

MAST90026 Computational Differential Equations: Week 3

Jesse Collis

Modified from Hailong Guo (2022)

Semester 1 2024

The University of Melbourne



Galerkin method

Consider the second order self-adjoint BVP:

$$-(D(x)u')' + q(x)u = f(x),$$

subject to Dirichlet boundary conditions $u(a) = \alpha$, $u(b) = \beta$. We also require $D(x) > 0$, $q(x) \geq 0$.

Idea of Galerkin method: require the projection of the residual onto V to vanish, i.e. require

$$\int_a^b [-(D(x)u')' + q(x)u - f(x)]v(x) dx = 0, \quad \forall v \in V,$$

where V is called the *test space*.

Integration by part

Idea: integrate by parts (if possible) and obtain the equation:

$$[-D(x)u'v]_a^b + \int_a^b D(x)u'v' dx + \int_a^b q(x)uv dx = \int_a^b fv dx, \quad \forall v \in V.$$

where $D(x)u'(x)$ is the flux.

Question: How to choose the test space V ?

Answer: Depends on the boundary condition.

- Neumann/Robin boundary conditions, the flux or u' is known on the boundaries.
- Dirichlet boundary conditions, do not know the flux on the boundary. Remove it by requiring $v = 0$ on the boundaries. The “don’t test where you know u ” principle

Motivation for weak derivative

If a function $f(x)$ is differentiable on $[a, b]$, then after integration by parts, for any smooth function $v(x)$ satisfying $v(a) = v(b) = 0$, we have

$$\int_a^b f(x)v'(x)dx = - \int_a^b f'(x)v(x)dx.$$

Example. The function $f(x) = |x|$ is not differentiable but we can define its weak derivative as the step function $g(x) = \begin{cases} 1, & x \geq 0, \\ -1, & x < 0. \end{cases}$

Weak derivative

Definition : (Weak derivative)

A function $g(x)$ is defined to be the weak derivative of f on $[a, b]$ if it satisfies

$$\int_a^b f(x)v'(x)dx = - \int_a^b g(x)v(x)dx, \quad \forall v(x) \in C_0^\infty([a, b]),$$

where $C^\infty([a, b])$ is the set of functions with derivatives of any order being continuous and $C_0^\infty([a, b])$ denotes that functions should also vanish at the boundaries $x = a$ and $x = b$.

Denote $f'(x) = g(x)$.

Weak derivative (continue)

Definition : (k th order weak derivative)

A function $g(x)$ is defined to be the k th order weak derivative of f on $[a, b]$ if it satisfies

$$\int_a^b f(x)v^{(k)}(x)dx = (-1)^k \int_a^b g(x)v(x)dx, \quad \forall v(x) \in C_0^\infty([a, b]).$$

Sobolev space

Definition : (Sobolev space)

For a positive index k , the Sobolev space $H^k(a, b)$ is the set of functions $u : (a, b) \rightarrow \mathbb{R}$ such that u and all (weak-) derivatives up to and including k are square integrable:

$$u \in H^k(a, b) \iff \int_a^b u^2 < \infty, \int_a^b (u')^2 < \infty, \dots, \int_a^b (u^{(k)})^2 < \infty$$

Note that $H^k(a, b)$ defines a Hilbert space with inner product

$$(u, w)_k = \int_a^b uw + \int_a^b u'w' + \int_a^b u''w'' + \dots + \int_a^b u^{(k)}w^{(k)},$$

and norm

$$\|u\|_k = \left(\int_a^b u^2 + \int_a^b (u')^2 + \dots + \int_a^b (u^{(k)})^2 \right)^{1/2}.$$

Returning to model equation

The appropriate space for V turns out to be

$$H_0^1(a, b) = \underbrace{\{u \in L_2(a, b), u' \in L_2(a, b); u(a) = 0, u(b) = 0\}}_{u \in H^1(a, b)}.$$

essential b.c.'s

This is a big surprise:

- There are no second derivatives in the definition of H_0^1 !
- First derivatives need not be continuous!

Weak form

Definition : (Variational form)

The variational form of the model equation is to find $u \in H^1(a, b)$ with $u(a) = \alpha$ and $u(b) = \beta$ such that

$$\int_a^b D(x)u'v' \, dx + \int_a^b q(x)uv \, dx = \int_a^b fv \, dx, \quad \forall v \in H_0^1(a, b).$$

Galerkin method

Principle 1: Make the residual orthogonal to a test space V .

Principle 2: Integrate by parts to lower smoothness requirements of the solution. (Seek a weak solution of the BVP).

Principle 3: Satisfy boundary conditions

- For Neumann boundary conditions, no restriction on test space, **natural boundary conditions**.
- For Dirichlet boundary conditions, remove boundary terms by restricting test space to have 0 on the boundaries. These are called **essential boundary conditions**.
- So we have the test space satisfying zero boundary conditions and the trial space satisfying specific boundary conditions for Dirichlet boundary conditions but no restrictions for Neumann boundary conditions.

Those 3 principles are already contained in the variational formulation.

Galerkin method (continue)

Principle 4: Choose trial and test space to be finite dimensional function spaces $U_N, V_N \subset H^1(a, b)$.

- For Dirichlet boundary conditions: $u_N = \phi_0 + \bar{u}$ where $\bar{u} \in V_N^0$ and ϕ_0 satisfies the boundary conditions. The space u_N exists in is an *affine space*. For V_N^0 choose a vector space of functions with zero boundary conditions, i.e. $V_N^0 \subset H_0^1(a, b)$.
- For Neumann boundary conditions
 $v \in V_N \subset H^1(a, b)$, $u \in U_N \subset H^1(a, b)$ where U_N and V_N are finite dimensional vector spaces of functions.

Galerkin equation

Principles 1 to 4 combine to give a **Galerkin method**.

So if we look back at our original problem, let $u = \sum_{j=1}^N c_j \phi_j(x)$.

$$\begin{aligned}(Du', v') + (qu, v) &= (f, v) + D(b)u'(b)v(b) - D(a)u'(a)v(a) \quad \forall v \in V_N \\ \implies (Du', \phi'_k) + (qu, \phi_k) &= (f, \phi_k) + D(b)u'(b)\phi_k(b) - D(a)u'(a)\phi_k(a) \\ \implies (D \sum_{j=1}^N c_j \phi'_j(x), \phi'_k) + (q \sum_{j=1}^N c_j \phi_j(x), \phi_k) &= (f, \phi_k) + B_k \quad k = 1, \dots, N.\end{aligned}$$

So we have a system of equations $A\vec{c} = \vec{f}$, where

$$A_{kj} = \int_a^b D(x)\phi'_k(x)\phi'_j(x) + q(x)\phi_k(x)\phi_j(x) dx,$$

$$\vec{c} = (c_1, \dots, c_N)^T,$$

$$f_k = \int_a^b f(x)\phi_k(x) dx + B_k.$$

These are the *Galerkin equations*. We call \vec{f} the (global) load vector.

Subspace conditions

Recall that first derivatives of H^1 function need not be continuous.

Theorem

A piecewise infinitely differentiable function $v : (a, b) \rightarrow \mathbb{R}$ belongs to $H^k(a, b)$ if and only if $v \in C^{k-1}(a, b)$.

Note that the theorem is also true in higher dimensions.

Choice of finite element dimensional space V_N :

- Spaces with global support e.g. polynomials or trigonometric functions. These are called **Spectral Galerkin methods**.
- Piecewise polynomial spaces. So ϕ_j will have local support. We call these spaces **finite element spaces**.

Finite element space

In FEM we construct V_h using piecewise polynomials in C_0 . Denote the finite element space by X_h^k .



For X_h^k , each piece is a polynomial of degree k

1. $n(k + 1)$ parameters

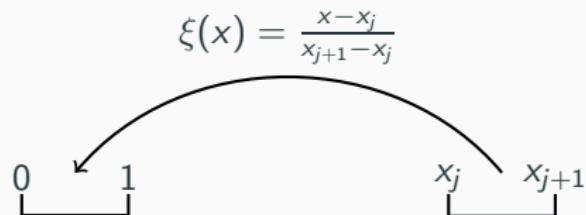
$n(k+1)$ parameters

- $(n-1)$ match conditions at mesh points to ensure C^0 For
 $\frac{nk+1}{nk+1}$ parameters = dimension of X_h^k

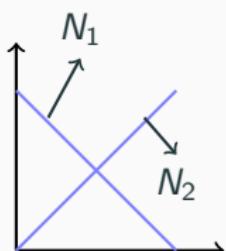
$X_{h,0}^k = X_h^k \cap H_0^1(a, b)$, zero value at 2 boundaries $\Rightarrow nk - 1$ parameters.

See $nk + 1$ basis functions. Degree of freedom (DOF = $nk + 1$).

Construction of linear basis function



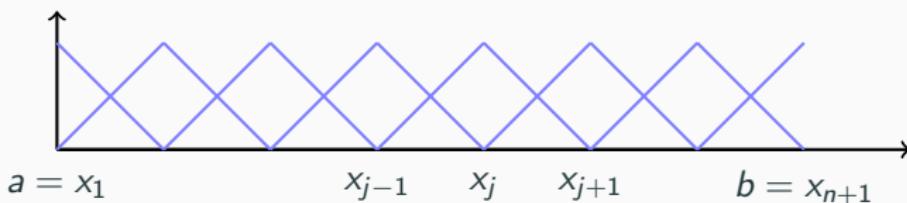
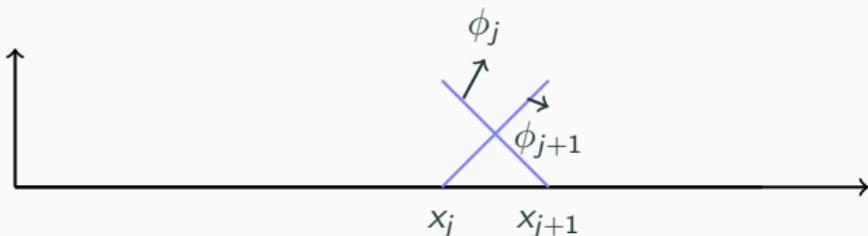
The reference element has 2 basis functions called **shape function**.



Linear functions with cardinal property $N_1(0) = 1, N_1(1) = 0$ and $N_2(0) = 0, N_2(1) = 1$
 $\Rightarrow N_1(\xi) = 1 - \xi, N_2(\xi) = \xi$.

Global linear basis functions

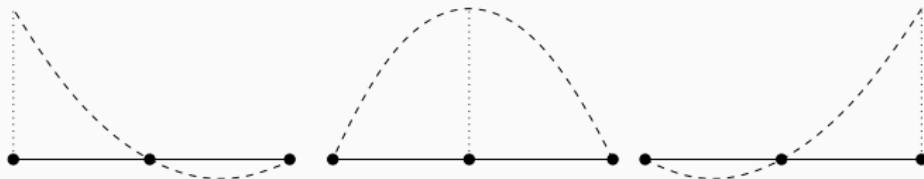
Under the map and the shape function, we get 2 basis functions defined on each element $E_j = (x_j, x_{j+1})$: $\phi_j = N_1(\xi(x))$ and $\phi_{j+1} = N_2(\xi(x))$



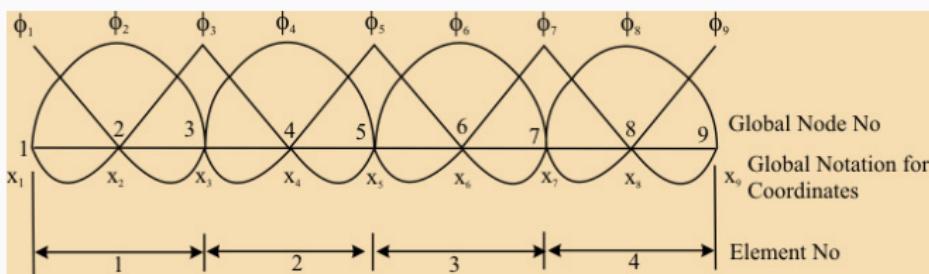
$\Rightarrow (n+1)$ basis fun $\{\phi_j\} \in X_h^1$ (Linear Lagrange element) $\Rightarrow \phi_j(x_i) = \delta_{ij}$ and local support, i.e. $\phi_j \neq 0$ only at $K_{j-1} \cup K_j$.

Construction of quadratic basis function

$k = 2, 2n + 1$ parameters $\Rightarrow 2n + 1$ DOFs $\Rightarrow (n+1)$ mesh points + n extra nodes



Shape functions on master element: $N_1 = (1 - \xi)(1 - 2\xi)$,
 $N_2 = 4(1 - \xi)\xi$ and $N_3 = \xi(2\xi - 1)$.



Implementation of finite element method

FEM implementations differ from others in 2 ways:

1. The stiffness/mass matrices and load vector are computed by doing integrals over each element, called *element stiffness matrices/load vectors*, and assembled into the final global matrices/vectors.
2. Rather than treat Dirichlet boundary conditions using general functions with $u = \phi_0 + \bar{u}$ ($\bar{u} \in V_h^0$). Choose $\phi_0 \in X^k \setminus X_0^k$ so $u \in X^k$ with non zero end basis functions. We do integrals using $u = \sum_{j=1}^{n+1} u_h \phi_j$ then move the corresponding known Dirichlet node values to the right hand side of $A\vec{u} = \vec{b}$.

Implementation of linear finite element method

For linear Lagrange element, we have n elements and $n + 1$ nodes giving global matrices of size $(n + 1) \times (n + 1)$. For each element we provide a list of nodes that make up the element in 1D so K_ℓ has nodes X_ℓ and $X_{\ell+1}$. The stiffness matrix and load vector are

$$K_{ij} = \int_a^b D(x) \phi'_i \phi'_j dx, \quad \text{for } i, j = 1, \dots, \text{degrees of freedom}$$

$$F_i = \int_a^b f(x) \phi_i(x) dx + \text{boundary terms},$$

Observation

$K_{ij} = 0$ unless ϕ'_i and ϕ'_j overlap so there are a lot of 0 terms. Instead we break things over the elements E_k :

$$\begin{aligned} K_{ij} &= \int_a^b D(x) \phi'_i \phi'_j \, dx \\ &= \sum_{\ell=1}^n \int_{x_\ell}^{x_{\ell+1}} D(x) \phi'_i \phi'_j \, dx \\ &= \sum_{\ell=1}^n \int_{E_\ell} D(x) \phi'_i \phi'_j \, dx, \end{aligned}$$

where each individual term in the sum is an element stiffness matrix.
Similarly

$$F_i = \sum_{\ell=1}^n \int_{E_\ell} f(x) \phi_i \, dx.$$

Observation (continue)

The only basis functions that are non zero on element ℓ are ϕ_ℓ and $\phi_{\ell+1}$.

$$F_{E_\ell} = \begin{bmatrix} \int_{E_\ell} f(x) \phi_\ell \, dx \\ \int_{E_\ell} f(x) \phi_{\ell+1} \, dx \end{bmatrix},$$

$$K_{E_\ell} = \begin{bmatrix} \int_{E_\ell} D(x) \phi_\ell'^2 \, dx & \int_{E_\ell} D(x) \phi_\ell' \phi_{\ell+1}' \, dx \\ \int_{E_\ell} D(x) \phi_{\ell+1}' \phi_\ell' \, dx & \int_{E_\ell} D(x) \phi_{\ell+1}'^2 \, dx \end{bmatrix}.$$

Assemble load vector

$$\vec{f} = \begin{bmatrix} (f, \phi_1) \\ (f, \phi_2) \\ \vdots \\ \vdots \\ (f, \phi_{n+1}) \end{bmatrix} = \begin{bmatrix} \int_{x_1}^{x_2} f \phi_1 \\ \int_{x_1}^{x_3} f \phi_2 \\ \vdots \\ \vdots \\ \int_{x_n}^{x_{n+1}} f \phi_{n+1} \end{bmatrix} = \begin{bmatrix} \int_{x_1}^{x_2} f \phi_1 \\ \int_{x_1}^{x_2} f \phi_2 \\ 0 \\ \vdots \\ 0 \end{bmatrix} + \begin{bmatrix} 0 \\ \int_{x_2}^{x_3} f \phi_2 \\ \int_{x_2}^{x_3} f \phi_3 \\ 0 \\ \vdots \end{bmatrix} + \cdots + \begin{bmatrix} 0 \\ \vdots \\ \vdots \\ \int_{x_n}^{x_{n+1}} f \phi_n \\ \int_{x_n}^{x_{n+1}} f \phi_{n+1} \end{bmatrix}$$

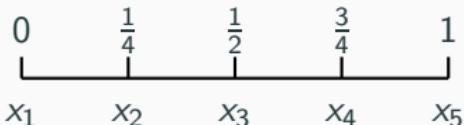
We only need compute element load vector F_{E_ℓ} and insert it into $(\ell : \ell + 1, 1)$ locations in F .

Assemble stiffness matrix

$$K = \begin{bmatrix} K_{E_1} & 0 & \cdots & 0 \\ 0 & 0 & \cdots & 0 \\ \vdots & \vdots & \ddots & 0 \\ 0 & 0 & \cdots & 0 \end{bmatrix} + \begin{bmatrix} 0 & 0 & \cdots & 0 \\ 0 & K_{E_2} & \cdots & 0 \\ \vdots & \vdots & \ddots & 0 \\ 0 & 0 & \cdots & 0 \end{bmatrix} + \cdots + \begin{bmatrix} 0 & 0 & \cdots & 0 \\ 0 & 0 & \cdots & 0 \\ \vdots & \vdots & \ddots & 0 \\ 0 & 0 & \cdots & K_{E_n} \end{bmatrix}$$

We only need compute element load vector K_{E_ℓ} and insert it into $(\ell : \ell + 1, \ell : \ell + 1)$ locations in K .

Data structure for implementing linear FEM



$$\text{node} = \begin{bmatrix} x_1 \\ x_2 \\ x_3 \\ x_4 \\ x_5 \end{bmatrix} = \begin{bmatrix} 0 \\ \frac{1}{4} \\ \frac{1}{2} \\ \frac{3}{4} \\ 1 \end{bmatrix} \quad \text{elem} = \begin{bmatrix} 1 & 2 \\ 2 & 3 \\ 3 & 4 \\ 4 & 5 \end{bmatrix}$$

elem is just the local numbering of local points to the global numbering of global points

Pseudo code

```
A = sparse(n+1, n+1);
b = zeros(n+1,1);
for j = 1:n
    xj = node(elem(j, 1));
    xj+1 = node(elem(j, 2));
    KEj = elem_stiff(xj, xj+1, D); % compute element stiffness matrix
    FEj = elem_load(xj, xj+1, f); % compute element load vector
    A(elem(j, :), elem(j, :)) = A(elem(j, :), elem(j, :)) + KEj;
    b(elem(j, :), 1) = b(elem(j, :), 1) + FEj;
end
```

You should write funs to compute element matrices/vectors which can be done by mapping into master element.

Computation of elementary matrices/vector

Map each integral to the master element and then integrate over $[0, 1]$.

$$\int_{x_j}^{x_{j+1}} f(x) \phi_j dx = h_j \int_0^1 f(x(\xi)) N_1(\xi) d\xi,$$
$$\int_{x_j}^{x_{j+1}} f(x) \phi_{j+1} dx = h_j \int_0^1 f(x(\xi)) N_2(\xi) d\xi.$$

Similarly,

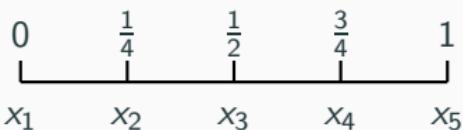
$$\begin{aligned} & \int_{x_j}^{x_{j+1}} \frac{d\phi_j(x)}{dx} \frac{d\phi_{j+1}(x)}{dx} dx = \int_0^1 \frac{d\phi_j(x(\xi))}{dx} \frac{d\phi_{j+1}(x(\xi))}{dx} \frac{dx}{d\xi} d\xi \\ &= \int_0^1 \frac{dN_1(\xi)}{d\xi} \frac{d\xi}{dx} \frac{dN_2(\xi)}{d\xi} \frac{d\xi}{dx} \frac{dx}{d\xi} d\xi = \int_0^1 \frac{dN_1(\xi)}{d\xi} \frac{1}{h_j} \frac{dN_2(\xi)}{d\xi} \frac{1}{h_j} h_j d\xi \\ &= \frac{1}{h_j} \int_0^1 \frac{dN_1(\xi)}{d\xi} \frac{dN_2(\xi)}{d\xi} d\xi. \end{aligned}$$

Elementary matrices/vector of quadratic element

$$F_{E_1} = \begin{bmatrix} (f, \phi_1) \\ (f, \phi_2) \\ (f, \phi_3) \end{bmatrix}, \quad F_{E_2} = \begin{bmatrix} (f, \phi_3) \\ (f, \phi_4) \\ (f, \phi_5) \end{bmatrix},$$

$$K_{E_1} = \begin{bmatrix} K_{11} & K_{12} & K_{13} \\ K_{21} & K_{22} & K_{23} \\ K_{31} & K_{32} & K_{33} \end{bmatrix}, \quad K_{E_2} = \begin{bmatrix} K_{33} & K_{34} & K_{35} \\ K_{43} & K_{44} & K_{45} \\ K_{53} & K_{54} & K_{55} \end{bmatrix},$$

Data structure for implementing quadratic FEM



$$\text{node} = \begin{bmatrix} x_1 \\ x_2 \\ x_3 \\ x_4 \\ x_5 \end{bmatrix} = \begin{bmatrix} 0 \\ \frac{1}{4} \\ \frac{1}{2} \\ \frac{3}{4} \\ 1 \end{bmatrix} \quad \text{elem} = \begin{bmatrix} 1 & 2 \\ 2 & 3 \\ 3 & 4 \\ 4 & 5 \end{bmatrix} \quad \text{elem2dof} = \begin{bmatrix} 1 & 6 & 2 \\ 2 & 7 & 3 \\ 3 & 8 & 4 \\ 4 & 9 & 5 \end{bmatrix}$$

elem2dof is just the numbering of local basis functions to the global numbering of global basis functions

Pseudo code of quadratic element

```
A = sparse(n+1, n+1);
```

```
b = zeros(n+1,1);
```

```
for j = 1:n
```

```
     $x_j = \text{node}(\text{elem}(j, 1));$ 
```

```
     $x_{j+1} = \text{node}(\text{elem}(j, 2));$ 
```

```
     $K_{E_j} = \text{elem\_stiffp2}(x_j, x_{j+1}, D);$  %element stiffness matrix
```

```
     $F_{E_j} = \text{elem\_loadp2}(x_j, x_{j+1}, f);$  % compute element load vector
```

```
     $A(\text{elem2dof}(j, :), \text{elem2dof}(j, :)) = A(\text{elem2dof}(j, :), \text{elem2dof}(j, :)) + K_{E_j};$ 
```

```
     $b(\text{elem2dof}(j, :), 1) = b(\text{elem2dof}(j, :), 1) + F_{E_j};$ 
```

```
end
```

You should write funs to compute element matrices/vectors which can be done by mapping into master element.

Galerkin orthogonality

The **bilinear form** (or inner product) defined by the ODE

$$a(u, v) = \int_a^b D(x)u'v' + q(x)uv \, dx,$$

which also induced a norm called **energy norm**

$$\|v\|_E^2 = \int_a^b D(x)|v'|^2 + q(x)v^2 \, dx.$$

Galerkin orthogonality:

$$a(u - u_h, v_h) = 0 \quad \forall v_h \in V_h,$$

Properties of bilinear form

Two properties:

- Coercivity: $c\|u\|_{H^1(a,b)}^2 \leq a(u, u)$ (Poincaré's Lemma).
- Continuity: $a(u, v) \leq C\|u\|_{H^1(a,b)}\|v\|_{H^1(a,b)}$ (Cauchy-Schwartz Inequality)

Lemma : (Lax-Miligram)

Let $a(\cdot, \cdot)$ be a bounded coercive and continuous bilinear form on a Hilbert space $H_0^1(a, b)$. Then for any function $f \in L^2(a, b)$, there exists a unique u in $H_0^1(a, b)$ such that

$$a(u, v) = (f, v), \quad v \in H_0^1(a, b).$$

Sufficient condition for this weak formulation to have a unique solution.

Ceà's Lemma

Lemma

Let u_h be the finite element solution of the model problem. Assume V_h is a subspace of H^1 , then we have

$$\|u - u_h\|_E = \min_{w_h \in V_h} \|u - w_h\|_E,$$

Best approximation property: the Galerkin solution is the best approximation to u from all the functions in V_h .

Error estimate

Theorem

Assume $u \in H^s$ for $s \geq 2$ and $u_h \in V_h = X_h^k$. Then:

$$\|u - u_h\|_{H_0^l} \leq Ch^l \|u\|_{H^{l+1}},$$

where $l = \min\{k, s - 1\}$.

Also,

$$\|u - u_h\|_{L^2} \leq Ch^{l+1} \|u\|_{H^{l+1}}.$$

Assume u is smooth enough. k th order in H^1 norm and $(k + 1)$ th order in L^2 norm.

End of week 3!