

CIV100 - MECHANICS - QUIZ #2 - 90 MINUTES - DECEMBER 2, 2015

READ CAREFULLY THE RULES:

No aids are permitted.

Only one of the two specified non-programmable calculators are permitted:

- CasioFX991 - SharpEL520

Answers must include the appropriate units.

Draw a Free Body Diagram (FBD) for each problem.

Use 5 significant digits (two decimal points for angles) in the calculations and 3 significant digits in the answers.

Instructor: Prof. Grasselli

Section

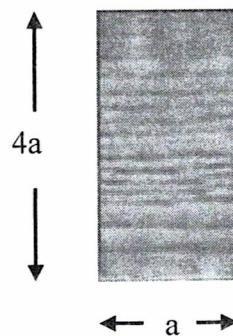
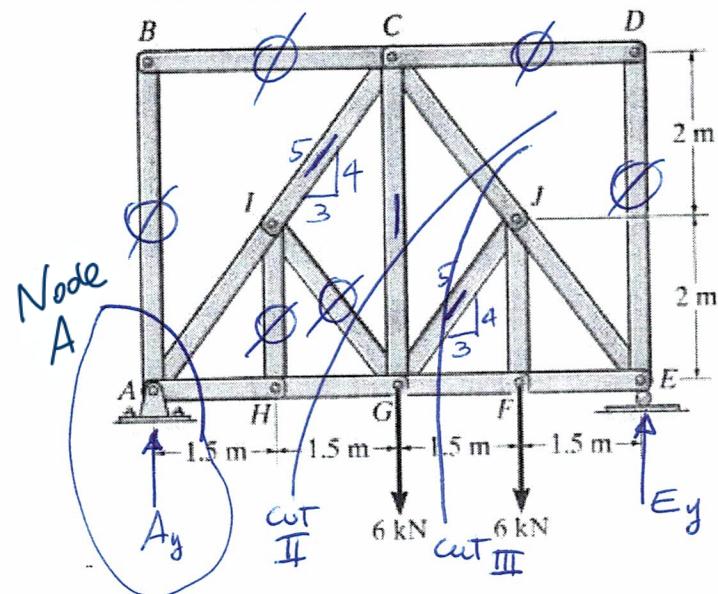
Family name:

Given name:

Student ID

- YOU MUST REPORT ALL RESULTS IN THE SPACES PROVIDED -
-ONLY THOSE REPORTED RESULTS WILL COUNT TOWARDS THE QUIZ GRADE-

SOLUTIONS

Question #1 [10 points]

1 - List all zero force members.

[2 point]

Zero force members are: AB, BC, IH, IG, CD, DE

2 - Determine the force in members IC, GJ, and CG of the truss and state if these members are in tension or compression.

IC= 5.625 kN in C	GJ= 3.75 kN in C	CG= 9.00 kN in T
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[4 point]

3 - Assuming that the force in member GF is 5.625 kN in tension, calculate the minimum dimension for member GF knowing the cross section is rectangular as shown above. The maximum tensile strength for the wood is 12 MPa; the load factor is 2.0; and the side dimension a is only available in increments of 5 mm.

a= 15.3 mm	Chosen cross section = 20 X 80
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[2 point]

4 - Assuming the member AH, having cross-sectional area is equal to $15 \times 60 \text{ mm}^2$, to be subjected to a service load of 3.5 kN in tension, calculate its elongation. For the calculations assume the Young's modulus to be equal to $12 \times 10^3 \text{ MPa}$.

Elongation = 0.486 mm

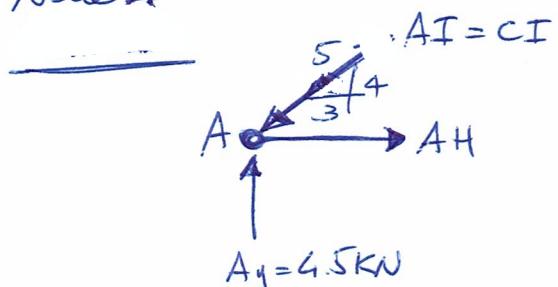
[2 point]

(2) Reaction forces
 $\text{@ } A \sum M_A = -6 \cdot 3 - 6 \cdot 4.5 + E_y \cdot 6 = 0 \rightarrow E_y = 7.5 \text{ kN}$

$\text{@ } E \sum M_E = 0 = 6 \cdot 1.5 + 6 \cdot 3 - A_y \cdot 6 = 0 \rightarrow A_y = 4.5 \text{ kN}$

Verify that $\sum F_y = 0 = 7.5 - 6 - 6 + 4.5 = 0 \checkmark$

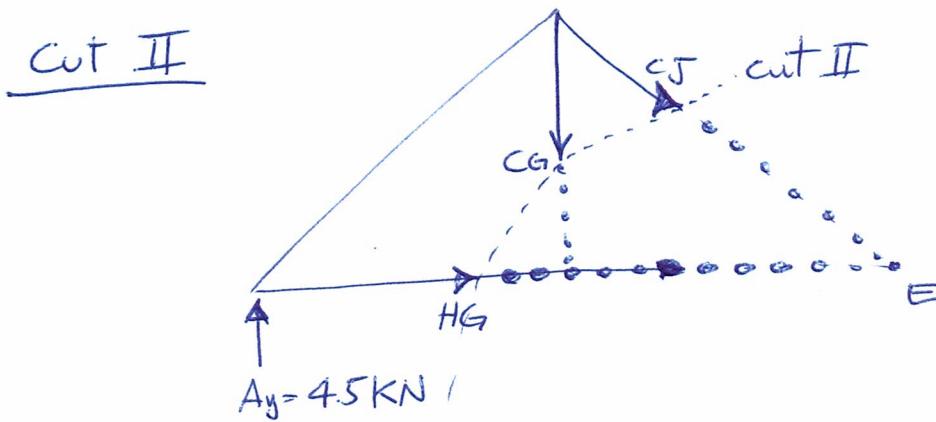
Node A



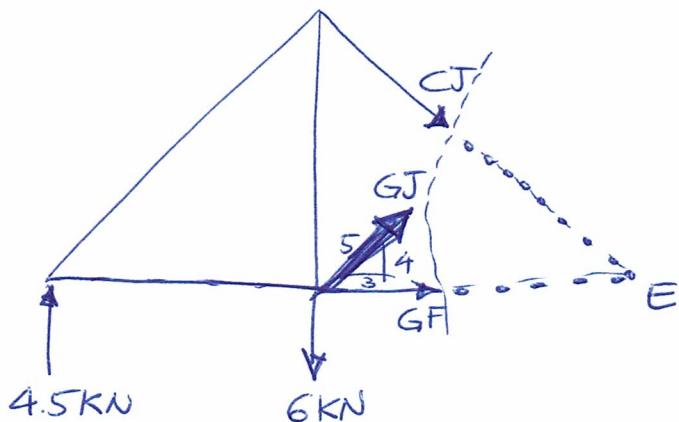
$$\frac{4}{5} AI = A_{Iy} = A_y$$



$$AI = \frac{5}{4} \cdot 4.5 = \underline{\underline{5.625 \text{ KN}}} \quad (C)$$



$$@ E \Rightarrow \sum M_E = 0 = -4.5 \cdot 6 + CG \cdot 3 = 0 \Rightarrow \underline{\underline{CG = 9 \text{ kN} (+)}}$$

Cut III

$$\begin{aligned} GJ_y &= \frac{4}{5} GJ \\ GJ &\rightarrow \\ \frac{3}{5} GJ &= GJ_x \end{aligned}$$

$$@ E \Rightarrow \sum M_E = 0 = -4.5 \cdot 6 - GJ_y \cdot 3 + 6 \cdot 3 \Rightarrow \underline{\underline{GJ = \frac{15}{4} = 3.75 \text{ kN} (C)}}$$

③ $\text{Area} = \alpha \cdot 4\alpha = 4\alpha^2$

$$\underline{\underline{T_y = \frac{F \cdot LF}{\text{Area}}}} \rightarrow \text{Area} = \frac{F \cdot LF}{T_y} \Rightarrow 4\alpha^2 = \frac{5.625 \cdot 10^3 \cdot 2}{12}$$

$$\alpha^2 = 234.375 \text{ mm}^2 \rightarrow \underline{\underline{\alpha = 15.31 \text{ mm}}}$$

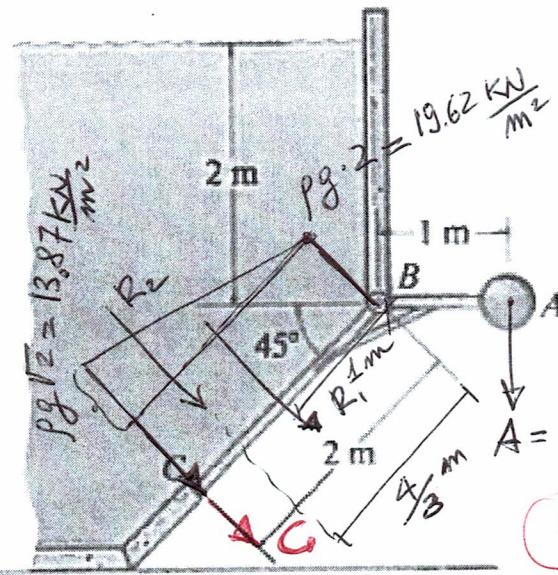
$\rightarrow \underline{\underline{\text{Section } 20 \times 80}}$

④ $\text{Area} = 15 \cdot 60 = 900 \text{ mm}^2$

$$L = 1.5 \text{ m} = 1500 \text{ mm}$$

$$\underline{\underline{T = \frac{F}{\text{Area}} = \frac{3.5 \cdot 10^3}{900} = 3.889 \text{ MPa}}}$$

$$\underline{\underline{T = E \cdot \frac{\Delta L}{L} \rightarrow \Delta L = \frac{T \cdot L}{E} = \frac{3.889 \cdot 1500}{12 \cdot 10^3} = 0.486 \text{ mm}}}$$

Question #2 [5 points]

If the mass of the counterweight at A is 6500 kg, determine the force the gate exerts on the smooth stop at C.

The gate is hinged at B and is 1 m wide.

The density of water is $\rho_w = 1 \text{ Mg/m}^3$

$$A = 6.5 \cdot 10^3 \cdot 9.81 = 63.765 \text{ KN}$$

(3)

1	Resultant force due to Water pressure =	53.11 kN	C= 3.02	kN	2	[5 point]
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$$P_B = \rho g \cdot h_B = 1000 \cdot 9.81 \cdot 2 = 19.62 \frac{\text{KN}}{\text{m}^2}$$

$$\Delta p = \rho g (h_c - h_B) = 1000 \cdot 9.81 \cdot \sqrt{2} = 13.87 \frac{\text{KN}}{\text{m}^2}$$

$$P_c = P_B + \Delta p = 53.11 \frac{\text{KN}}{\text{m}^2}$$

$$R_1 = P_c \cdot 2 \cdot 1 = 39.24 \text{ KN}$$

$$R_2 = \Delta p \cdot 2 \cdot 1 \cdot \frac{1}{2} = 13.87 \text{ KN}$$

$$R = R_1 + R_2 = 53.11 \text{ KN}$$

$$@B \sum M_B = 0 = -63.765 + 39.24 \cdot 1 + 13.87 \cdot \frac{4}{3} + C_y \cdot 2$$

$$\rightarrow C_y = 3.02 \text{ KN}$$

Question #3 [10 points]

- 1- Draw **NEATLY** the Shear and Bending Moment diagrams for the beam ABCDEF in the space provided below the beam. [2 point]
 (Locate and indicate values at supports, loads, and local maxima and minima)

- 2- Find the magnitude of the Maximum Bending moment.

Maximum Bending moment = kN m

[2 point]

- 3- Find the location, distance measured from A, where the bending moment is maximum.

Distance from A = m

[2 point]

- 5 - If the maximum bending moment is 15 kN m, for the given cross section below, calculate the value of the maximum stress due to the applied bending moment.

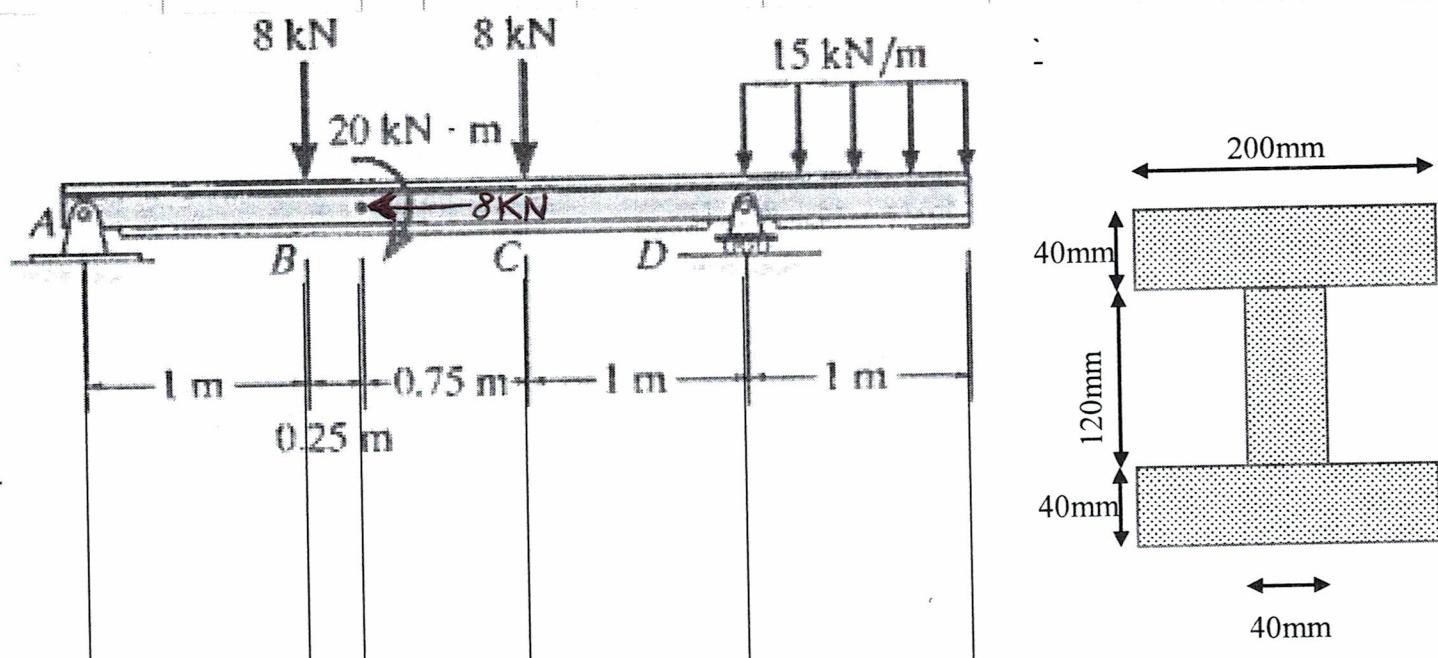
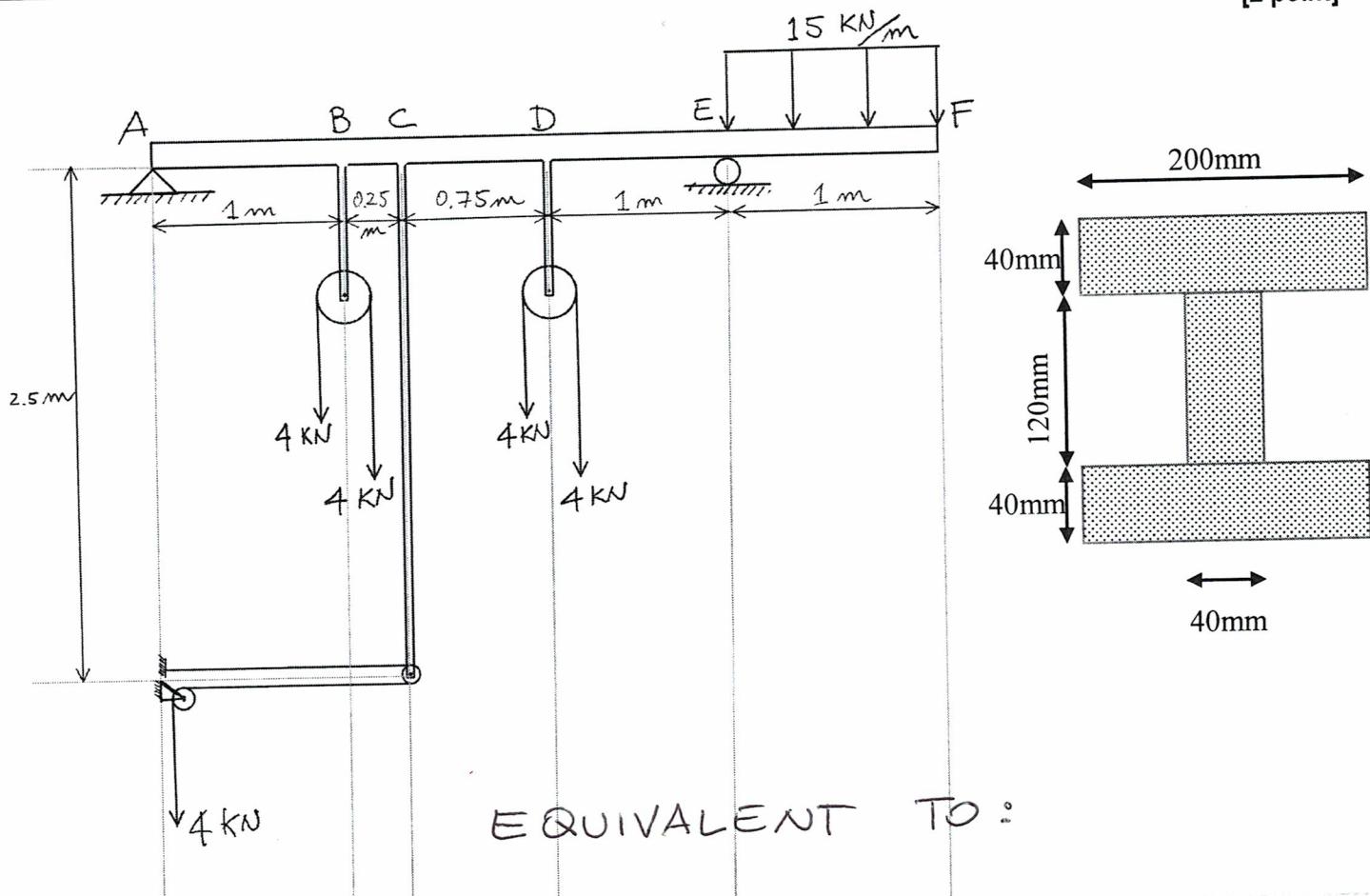
Maximum stress = MPa

[2 point]

- 6 - If the yield stress for the material in the beam is 400 MPa, state if the beam is safe or not

The beam is

[2 point]



Question #3 [10 points]

1- Draw NEATLY the Shear and Bending Moment diagrams for the beam ABCDEF in the space provided below the beam. [2 point]
 (Locate and indicate values at supports, loads, and local maxima and minima)

2- Find the magnitude of the Maximum Bending moment.

Maximum Bending moment = 16.54 kN m

[2 point]

3- Find the location, distance measured from A, where the bending moment is maximum.

Distance from A = 1.25 m

[2 point]

5 - If the maximum bending moment is 15 kN m, for the given cross section below, calculate the value of the maximum stress due to the applied bending moment.

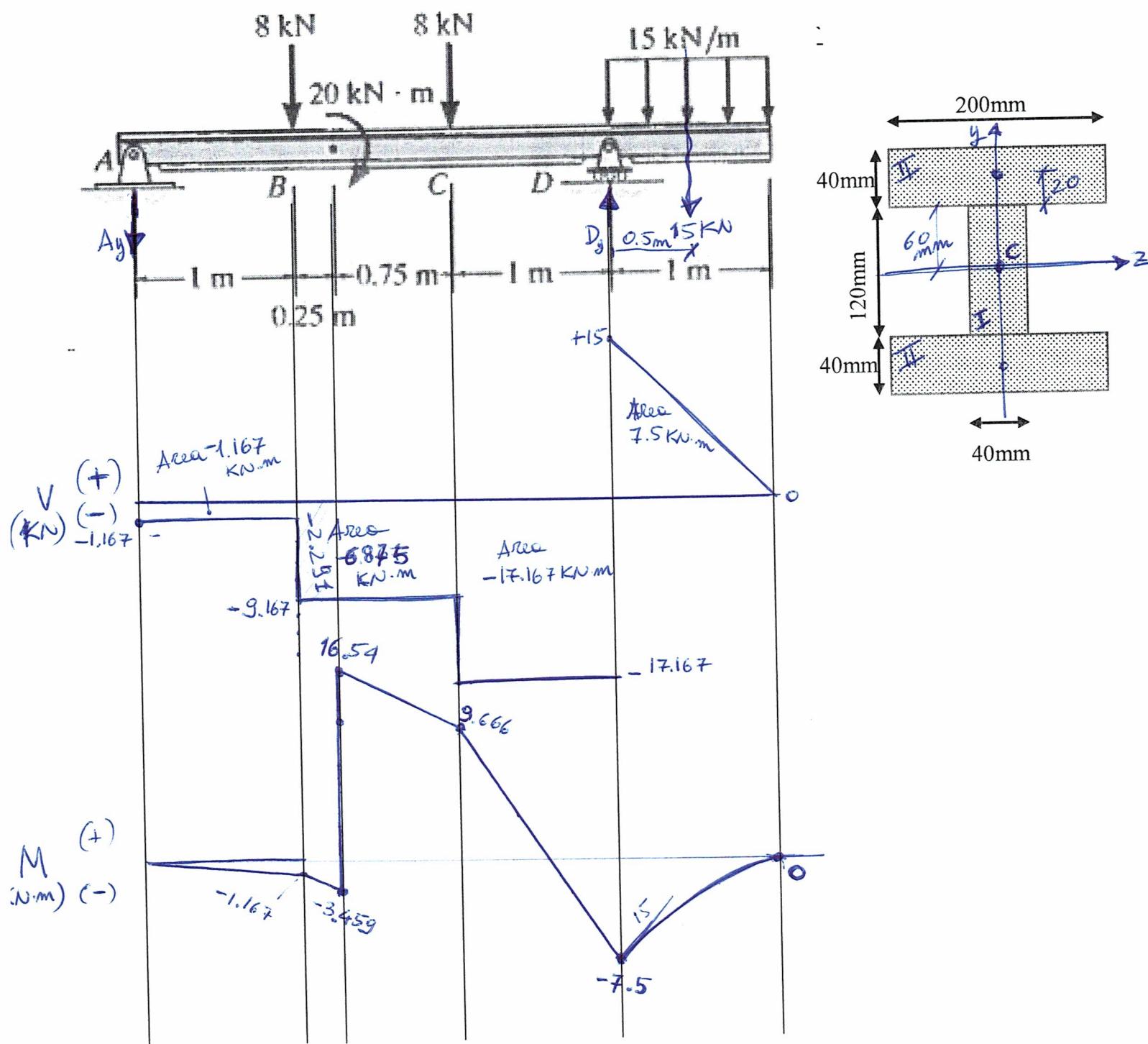
Maximum stress = 13.6 MPa

[2 point]

6 - If the yield stress for the material in the beam is 400 MPa, state if the beam is safe or not

The beam is SAFE

[2 point]



$$\textcircled{1} \quad @ A \rightarrow M_A = 0 = -20 - 8 \cdot 1 - 8 \cdot 2 - 15 \cdot 3.5 + D_y \cdot 3$$

$$D_y = 32.167 \text{ KN} (\uparrow)$$

$$@ D \rightarrow M_D = 0 = -15 \cdot 0.5 + 8 \cdot 1 + 8 \cdot 2 - 20 + A_y \cdot 3$$

$$A_y = 1.167 \text{ KN} (\downarrow)$$

$$\text{Check: } \sum F_y = 1.167 - 8 - 8 + 32.167 - 15 = 0 \checkmark$$

(5)

$$M_{z_{\max}} = 15 \text{ KN.m}$$

$$\sigma = \frac{M_{z_{\max}} \cdot y_{\max}}{I_z}$$

$$y_{\max} = 100 \text{ mm}$$

Part	I_z	$d_y(\text{mm})$	$A(\text{mm}^2)$	$d^2 A$	$I_z + d^2 A (\text{mm}^4)$
I	$\frac{1}{12} \cdot 40 \cdot 120^3$	0	4800	0	$5.76 \cdot 10^6$
2x II	$\frac{1}{12} \cdot 200 \cdot 40^3$ $= 1.067 \cdot 10^6$	80	8000	$512 \cdot 10^6$	$52.267 \cdot 10^6 \times 2$
					$I_z = 110.294 \cdot 10^6 (\text{mm}^4)$

$$\sigma = \frac{15 \cdot 10^6 \cdot 100}{110.294 \cdot 10^6} = 13.6 \text{ MPa}$$

(6)

$$\sigma = 13.6 \text{ MPa} < 400 \text{ MPa} \Rightarrow \text{Safe!}$$