

# Tutorial T04 – Elasticity stress and strain

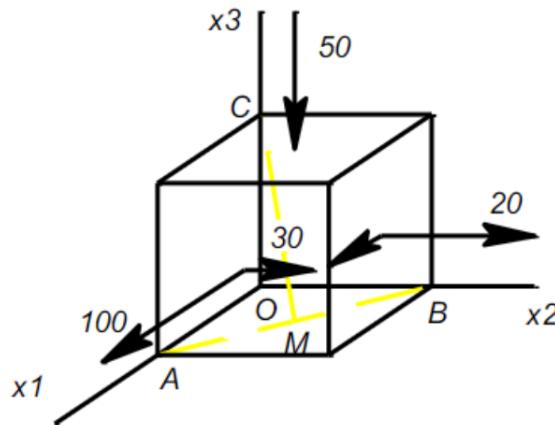
Answer the following questions as they could come up in an exam.

## 1 Stress basics (geometry and stress vector)

... based on section 3.1 (Exercise V1 in old material before 2022)

**Given:**

$$E = 200 \text{ GPa}, \nu = 0.25 \\ OA = OB = a \text{ and } OC = \frac{1}{2}\sqrt{2} \cdot a$$



**Questions:**

- a) Find normal stress  $\sigma_{ABC}$  and shear stress  $\tau_{ABC}$  acting on the area  $ABC$ .
- b) What are the components of the strain-tensor  $\varepsilon_{ij}$ ?
- c) What are the eigen-strains?
- d) In this stress-state, the maximal principal stress must not be larger than: 150 MPa.  
Is this stress state allowed according to the hypotheses of Tresca and von Mises?

**Answers:**

a) The stress Tensor is:  $[\sigma_{ij}] = \begin{bmatrix} 100 & 30 & 0 \\ 30 & 20 & 0 \\ 0 & 0 & -50 \end{bmatrix}$  MPa,

given the arrows, using symmetry, direction of arrows (sign), and non-existing (zero).

First, find the normal to the plane: *this can be done by taking the cross-product of two line vectors (that describe a plane).*

$$\vec{AC} \times \vec{AB} = \begin{pmatrix} -a \\ 0 \\ \frac{a}{\sqrt{2}} \end{pmatrix} \times \begin{pmatrix} -a \\ a \\ 0 \end{pmatrix} = \frac{-a^2}{\sqrt{2}} \begin{pmatrix} 1 \\ 1 \\ \sqrt{2} \end{pmatrix} = a \begin{pmatrix} 1 \\ 1 \\ \sqrt{2} \end{pmatrix}$$

Normalizing the vector using the normality condition ( $\hat{n}_1^2 + \hat{n}_2^2 + \hat{n}_3^2 = 1$ ), one can find  $a$ :

$$a^2 (1^2 + 1^2 + \sqrt{2}^2) = 1 \implies a = \frac{1}{2}$$

After using the cross-product, with the vectors in random order we pay close attention to the fact that the normal is facing outside the plane. With the normal you indicate which side the material is. To make the normal point away from the material, we can choose  $a$  positive.

**Cauchy:** Stress or traction vector:  $p_i = \sigma_{ij}n_j$ , so that:

$$\rightarrow [p] = \begin{bmatrix} p_1 \\ p_2 \\ p_3 \end{bmatrix} = [\sigma] \begin{bmatrix} \hat{n}_1 \\ \hat{n}_2 \\ \hat{n}_3 \end{bmatrix} = \begin{bmatrix} 100 & 30 & 0 \\ 30 & 20 & 0 \\ 0 & 0 & -50 \end{bmatrix} \frac{1}{2} \begin{pmatrix} 1 \\ 1 \\ \sqrt{2} \end{pmatrix} = \begin{bmatrix} 65 \\ 25 \\ -25\sqrt{2} \end{bmatrix}$$

The normal stress on the plane  $ABC$  is:  $[\sigma] = [\hat{n}]^T [p] = 20 \text{ MPa}$

The shear stress on the plane  $ABC$ , using Pythagoras, is:

$$\tau^2 = p^2 - \sigma^2 = [p_1^2 + p_2^2 + p_3^2] - \sigma^2 = 6100 - 400 = 5700 \text{ MPa}^2,$$

so that  $\tau = 75.5 \text{ MPa}$ .

b)

Hooke's law for strain  $\varepsilon_{ij} = \frac{1}{E}[(1+\nu)\sigma_{ij} - \nu\sigma_{kk}\delta_{ij}]$ , with  $\sigma_{kk} = \sigma_{11} + \sigma_{22} + \sigma_{33}$ , allows to obtain:  $\varepsilon_{11} = \frac{\sigma_{11}}{E} - \nu\frac{\sigma_{22}}{E} - \nu\frac{\sigma_{33}}{E}$ ,  $\varepsilon_{12} = \frac{\sigma_{12}}{2G}$ , with  $G = \frac{E}{2(1+\nu)}$ , and - similarly - the other components.

$$[\varepsilon] = \begin{bmatrix} 5.375 & 1.875 & 0 \\ 1.875 & 0.375 & 0 \\ 0 & 0 & -4 \end{bmatrix} 10^{-4} = \frac{1}{8} \begin{bmatrix} 43 & 15 & 0 \\ 15 & 3 & 0 \\ 0 & 0 & -32 \end{bmatrix} 10^{-4}$$

c)

Principal strains are computed, like for stress, solving:

$$\det(\varepsilon_{ij} - \varepsilon\delta_{ij}) = \begin{bmatrix} \varepsilon_{11} - \varepsilon & \varepsilon_{12} & \varepsilon_{13} \\ \varepsilon_{12} & \varepsilon_{22} - \varepsilon & \varepsilon_{23} \\ \varepsilon_{13} & \varepsilon_{23} & \varepsilon_{33} - \varepsilon \end{bmatrix} = 0$$

$$\varepsilon^3 - E_1\varepsilon^2 + E_2\varepsilon - E_3 = 0$$

$$E_1 = \varepsilon_{11} + \varepsilon_{22} + \varepsilon_{33} = (14/8) 10^{-4}$$

$$E_2 = \varepsilon_{11}\varepsilon_{22} + \varepsilon_{22}\varepsilon_{33} + \varepsilon_{33}\varepsilon_{11} - \varepsilon_{12}^2 - \varepsilon_{13}^2 - \varepsilon_{23}^2 = ???$$

$$E_3 = \det(\varepsilon) = ???$$

with solutions:  $\varepsilon_I = 6 10^{-4}$ ,  $\varepsilon_{II} = -0.25 10^{-4}$ ,  $\varepsilon_{III} = -4 10^{-4}$ , sorted.

d)

To compute the allowable stress, we first need to compute the principal stresses:  $\sigma_I = 110 \text{ MPa}$ ,  $\sigma_{II} = 10 \text{ MPa}$ ,  $\sigma_{III} = -50 \text{ MPa}$ , sorted.

According to Tresca:  $\sigma_{eq}^{Tresca} = \sigma_I - \sigma_{III} = 160 \text{ MPa}$ .

According to von Mises:  $\sigma_{eq}^{vonMises} = \sqrt{\frac{1}{2} [(\sigma_I - \sigma_{II})^2 + (\sigma_I - \sigma_{III})^2 + (\sigma_{II} - \sigma_{III})^2]} = 140 \text{ MPa}$ .

Allowed stress means:  $\sigma_{eq} \leq \bar{\sigma} = 150 \text{ MPa}$ . Thus von Mises is allowed, whereas Tresca is not.

## 2 Stress tensor basics

... based on sections 3.1-3.3. (Exercise V2 in old material before 2022)

**Given:**

- Linear elastic isotropic material with modulus  $E = 2 \cdot 10^5$  N/mm<sup>2</sup>
- The stress cube, below, in units of N/mm<sup>2</sup>
- One principal stress is: 8 N/mm<sup>2</sup>

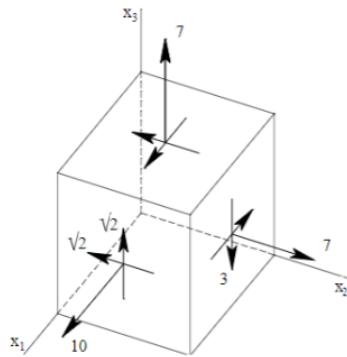


Figure 1: Stress cube → write down the stress matrix

**Questions:**

- Find the other principal (eigen) stresses
- Find the eigen-directions and plot these in a graph.
- What is the maximal shear strain for a given volumetric strain of  $\varepsilon_V = 0.6 \cdot 10^{-4}$ ?
- What are the equivalent stresses according to the hypotheses of Tresca and von Mises?

**Answers:**

- The stress tensor from the cube is:

$$[\sigma_{ij}] = \begin{bmatrix} 10 & -\sqrt{2} & \sqrt{2} \\ -\sqrt{2} & 7 & -3 \\ \sqrt{2} & -3 & 7 \end{bmatrix} \text{ MPa}$$

Note that the first index denotes the direction of the normal to the according surface on which this stress component works, while the second index gives the direction of the stress component.

Next get the characteristic equation from:

$$\det(\sigma_{ij} - \sigma\delta_{ij}) = \begin{bmatrix} 10 - \sigma & -\sqrt{2} & \sqrt{2} \\ -\sqrt{2} & 7 - \sigma & -3 \\ \sqrt{2} & -3 & 7 - \sigma \end{bmatrix}$$

$$\sigma^3 - I_1\sigma^2 + I_2\sigma - I_3 = 0$$

$$I_1 = \sigma_{11} + \sigma_{22} + \sigma_{33} = 24 \text{ MPa}$$

$$I_2 = \sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{33}\sigma_{11} - \sigma_{12}^2 - \sigma_{31}^2 - \sigma_{32}^2 = 176 \text{ MPa}^2$$

$$I_3 = \det(\sigma) = 384 \text{ MPa}^3$$

Here, the characteristic equation is not easily solvable; one way is to use the given eigen-value,  $\sigma = 8 \text{ N/mm}^2$ , and polynomial division (units dropped for simplicity, but must be added for final answer). Take the characteristic equation and divide by  $(\sigma - 8)$ :

$$(\sigma^3 - 24\sigma^2 + 176\sigma - 384) \setminus (\sigma - 8) = \sigma^2 - 16\sigma + 48$$

$$\underline{(\sigma^3 - 8\sigma^2)}$$

$$- 16\sigma^2 + 176\sigma - 384$$

$$\underline{(-16\sigma^2 + 128\sigma)}$$

$$+ 48\sigma - 384$$

$$\underline{( + 48\sigma - 384)}$$

%

The result is a second order polynomial, which can be solved as:

$$\sigma_{1,2} = (16 \pm \sqrt{16^2 - 4 \times 48})/2 = 12 \text{ and } 4 \text{ MPa.}$$

Therefore, the sorted eigen-values are:  $\sigma_I = 12 \text{ MPa}$ ,  $\sigma_{II} = 8 \text{ MPa}$ ,  $\sigma_{III} = 4 \text{ MPa}$ .

b) Direction of  $\sigma_I = 12 \text{ MPa}$

*There are various ways to solve for eigen-vectors, here is one example ...*

*Insert values, solve the system of equations, and normalize the solution.*

$$\begin{bmatrix} \sigma_{11} - \sigma_I & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & \sigma_{22} - \sigma_I & \sigma_{23} \\ \sigma_{13} & \sigma_{23} & \sigma_{33} - \sigma_I \end{bmatrix} \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \quad \text{and} \quad n_1^2 + n_2^2 + n_3^2 = 1$$

$\Rightarrow$

$$-2n_1 - \sqrt{2}n_2 + \sqrt{2}n_3 = 0$$

$$-\sqrt{2}n_1 - 5n_2 - 3n_3 = 0$$

$$\sqrt{2}n_1 - 3n_2 - 5n_3 = 0$$

The eigen-direction associated to the first, largest eigen-value:

$$\Rightarrow \hat{n}^{(I)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \frac{1}{2} \begin{bmatrix} \sqrt{2} \\ -1 \\ 1 \end{bmatrix}$$

Direction of  $\sigma_{II} = 8\text{MPa}$

$$\begin{bmatrix} \sigma_{11} - \sigma_{II} & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & \sigma_{22} - \sigma_{II} & \sigma_{23} \\ \sigma_{13} & \sigma_{23} & \sigma_{33} - \sigma_{II} \end{bmatrix} \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \quad \text{and} \quad n_1^2 + n_2^2 + n_3^2 = 1$$

$\implies$

$$2n_1 - \sqrt{2}n_2 + \sqrt{2}n_3 = 0$$

$$-\sqrt{2}n_1 - n_2 - 3n_3 = 0$$

$$\sqrt{2}n_1 - 3n_2 - n_3 = 0$$

The eigen-direction associated to the second, intermediate eigen-value:

$$\implies \hat{n}^{(II)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \frac{1}{2} \begin{bmatrix} \sqrt{2} \\ 1 \\ -1 \end{bmatrix}$$

Direction of  $\sigma_{III} = 4\text{MPa}$

$$\begin{bmatrix} \sigma_{11} - \sigma_{III} & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & \sigma_{22} - \sigma_{III} & \sigma_{23} \\ \sigma_{13} & \sigma_{23} & \sigma_{33} - \sigma_{III} \end{bmatrix} \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \quad \text{and} \quad n_1^2 + n_2^2 + n_3^2 = 1$$

$\implies$

$$6n_1 - \sqrt{2}n_2 + \sqrt{2}n_3 = 0$$

$$-\sqrt{2}n_1 + 3n_2 - 3n_3 = 0$$

$$\sqrt{2}n_1 - 3n_2 + 3n_3 = 0$$

The eigen-direction associated to the third, smallest eigen-value:

$$\implies \hat{n}^{(III)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \frac{\sqrt{2}}{2} \begin{bmatrix} 0 \\ 1 \\ 1 \end{bmatrix}$$

The directions are unspecified, indicated by the plus-minus from taking a square-root; all three direction vectors are normalized (check it, if enough time in exam),  $(n_i)^2 = 1$ ; furthermore, all three normal (eigen) vectors must be pair-wise perpendicular on each other, i.e.  $n_i^{(a)} n_i^{(b)} = 0$ , for all  $a, b = I, II, III$  with  $a \neq b$ . This perpendicularity allows to obtain, alternatively, one eigen-vector by a cross-product, e.g. above  $\hat{n}^{(III)} = \hat{n}^{(I)} \times \hat{n}^{(II)}$ .

c)

$$\varepsilon_v = \varepsilon_{xx} + \varepsilon_{yy} + \varepsilon_{zz} = 0.6 \times 10^{-4}$$

Since the material is isotropic:

$$\varepsilon_v = 3\varepsilon_{xx} = 0.6 \times 10^{-4}$$

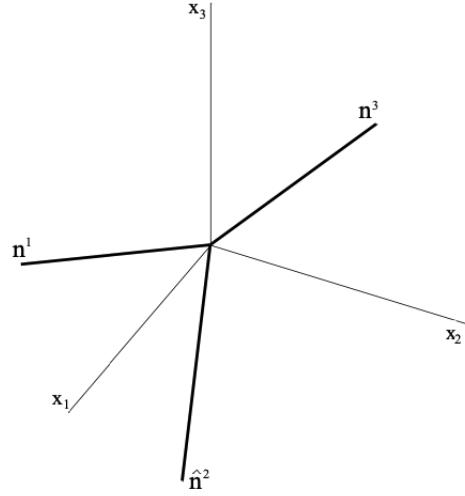


Figure 2: Sketch of the eigen-vectors (with coordinates and vectors)

$$\varepsilon_{xx} = 0.2 \times 10^{-4}$$

$$\sigma_{xx} = E \varepsilon_{xx} = 2 \times 10^5 \cdot 0.2 \times 10^{-4} = 4 \text{ MPa}$$

d)

$$\sigma_{Tresca} = \max\{|\sigma_I - \sigma_{II}|, |\sigma_{II} - \sigma_{III}|, |\sigma_{III} - \sigma_I|\} = \max\{4, 4, 8\} = 8 \text{ MPa}$$

$$\sigma_{von-Mises} = \sqrt{\frac{(\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2}{2}} = 6.92 \text{ MPa}$$

Then, Tresca is safer since it is larger and thus reaches the limit stress earlier.

### 3 Stress tensor basics

... based on sections 3.1-3.3. (Exercise V3 in old material before 2022)

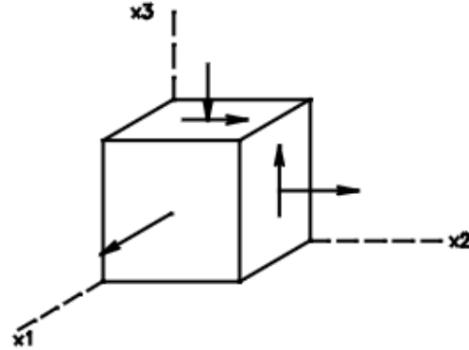


Figure 3: Stress cube, empty → fill it

**Given:**

The stress-state is described by the matrix:  $\begin{bmatrix} 60 & 0 & 0 \\ 0 & 20 & 20\sqrt{3} \\ 0 & 20\sqrt{3} & -20 \end{bmatrix}$  N/mm<sup>2</sup>,  
with  $E = 2 \cdot 10^5$  N/mm<sup>2</sup>, and  $\nu = 0.25$ .

**Questions:**

- a) Compute the principal stresses
- b) Compute the eigen-directions
- c) Compute the maximal shear-stress

**Answers:**

a)

The sorted eigen-values are:  $\sigma_I = 60$  MPa,  $\sigma_{II} = 40$  MPa,  $\sigma_{III} = -40$  MPa.

b)

Without calculation necessary (due to the special structure of this plane stress):

$$\hat{n}^{(I)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \begin{bmatrix} 1 \\ 0 \\ 0 \end{bmatrix}$$

The other eigen-directions are obtained from  $(\sigma_{ij} - \sigma \delta_{ij})n_j = 0$ , with normalization  $n_j^2 = 1$ :

$$\hat{n}^{(II)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \frac{1}{2} \begin{bmatrix} 0 \\ \sqrt{3} \\ 1 \end{bmatrix}$$

$$\hat{n}^{(III)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \begin{bmatrix} 0 \\ 1 \\ -\sqrt{3} \end{bmatrix}$$

Insert values, for example  $\sigma_{II}$ , solve the system of equations, and normalize the solution.

$$\begin{bmatrix} \sigma_{11} - \sigma_{II} & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & \sigma_{22} - \sigma_{II} & \sigma_{23} \\ \sigma_{13} & \sigma_{23} & \sigma_{33} - \sigma_{II} \end{bmatrix} \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \quad \text{and} \quad n_1^2 + n_2^2 + n_3^2 = 1$$

$\implies$

$$-20n_2 + 20\sqrt{3}n_3 = 0$$

$$20\sqrt{3}n_2 - 60n_3 = 0$$

$\implies$

$$n_2 = \sqrt{3}n_3$$

$$n_2 = (3/\sqrt{3})n_3 = \sqrt{3}n_3 \text{ (identical due to dependency)}$$

$\implies$

$$n_2^2 + n_3^2 = (1+3)n_3^2 = 1$$

$\implies$

$$n_3 = \sqrt{1/4} = \pm 1/2 = \pm 0.5$$

The eigen-direction associated to the second, intermediate eigen-value:

$$\hat{n}^{(II)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \frac{1}{2} \begin{bmatrix} 0 \\ \sqrt{3} \\ 1 \end{bmatrix}$$

The third eigenvalue calculation is similar (not shown).

c)

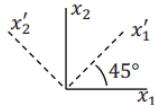
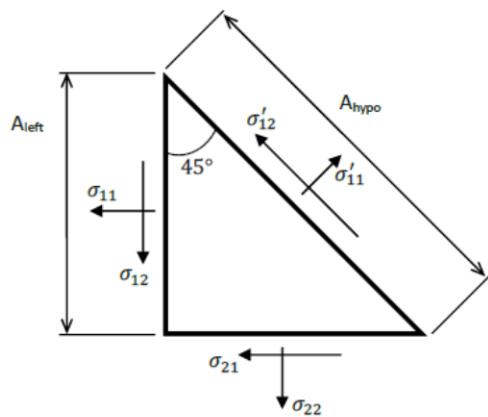
The maximum shear stress is:  $\tau_{max} = (\sigma_I - \sigma_{III})/2 = 50$  MPa.

## 4 Stress tensor and transformation

... based on sections 3.1-3.4. (Exercise V10 in old material before 2022)

**Given:**

- A plane-stress state in a point P of a body with  $\sigma_{13} = \sigma_{23} = \sigma_{33} = 0$
- Given are these (mixed) stress components:  
 $\sigma_{11} = 92 \text{ MPa}$   
 $\sigma'_{11} = 194 \text{ MPa}$   
 $\sigma'_{12} = -42 \text{ MPa}$   
where the prime indicates the new (transformed) coordinate system.
- The material is linear elastic with  $E = 2 \cdot 10^5 \text{ MPa}$  and  $\nu = 0.25$ .



**Questions:**

- Give the stress tensor in the original  $x_1x_2x_3$  system.
- Give the stress tensor in the new  $x'_1x'_2x'_3$  coordinate system, as obtained by a rotation of the coordinates about 45° around the  $x_3$ -axis, as sketched above.
- Compute the eigen-stresses and the eigen-directions.

**Answers:**

a)

There are two ways to solve this problem. The triangle given represents all stresses on all sides, but only part of the stress components are known. By considering force equilibrium and using the respective stress components, divided by the side-lengths of the triangle (which also has a third dimension outside the plane, not shown). Assume the sides have unit-length, then the hypotenuse has, according to Pythagoras, length  $\sqrt{2}$ . Further assume the thickness also to be unit-length. The ratio between sides and hypotenuse is then:

$$\frac{A_l}{A_h} := \frac{A_{left}}{A_{hypo}} = \frac{1}{\sqrt{2}} = \frac{1}{2}\sqrt{2}$$

With this we get:

Force balance in  $x_1$  direction:

$$\begin{aligned}
A_h \sigma'_{11} \cos(45^\circ) - A_h \sigma'_{12} \sin(45^\circ) - A_l \sigma_{11} - A_l \sigma_{12} &= 0 \\
\Rightarrow \sigma'_{11} \cos(45^\circ) - \sigma'_{12} \sin(45^\circ) - \frac{A_l}{A_h} (\sigma_{11} + \sigma_{12}) &= 0 \\
\Rightarrow (\sigma'_{11} - \sigma'_{12}) \frac{\sqrt{2}}{2} - \frac{\sqrt{2}}{2} (\sigma_{11} + \sigma_{12}) &= 0 \\
\Rightarrow \sigma_{12} \equiv \sigma_{21} &= \sigma'_{11} - \sigma'_{12} - \sigma_{11} \\
\Rightarrow \sigma_{12} \equiv \sigma_{21} &= 194 - (-42) - 92 = 144 \text{ MPa.}
\end{aligned}$$

Force balance in  $x_2$  direction:

$$\begin{aligned}
A_h \sigma'_{11} \sin(45^\circ) + A_h \sigma'_{12} \cos(45^\circ) - A_l \sigma_{12} - A_l \sigma_{22} &= 0 \\
\Rightarrow \sigma'_{11} \sin(45^\circ) + \sigma'_{12} \cos(45^\circ) - \frac{A_l}{A_h} (\sigma_{12} + \sigma_{22}) &= 0 \\
\Rightarrow (\sigma'_{11} + \sigma'_{12}) \frac{\sqrt{2}}{2} - \frac{\sqrt{2}}{2} (\sigma_{12} + \sigma_{22}) &= 0 \\
\Rightarrow \sigma_{22} &= \sigma'_{11} + \sigma'_{12} - \sigma_{12} \\
\Rightarrow \sigma_{22} &= 194 + (-42) - 144 = 8 \text{ MPa.}
\end{aligned}$$

The stress tensor in the  $x_1x_2x_3$  system is thus:

$$[\sigma_{ij}] = \begin{bmatrix} 92 & 144 & 0 \\ 144 & 8 & 0 \\ 0 & 0 & 0 \end{bmatrix} \text{ MPa}$$

b)

The stress tensor in the  $x'_1x'_2x'_3$  system is obtained by rotation of the original system around  $45^\circ$ , as sketched, in index notation,  $\sigma'_{pq} = R_{pi}R_{qj}\sigma_{ij}$ , or:

$$[\sigma'] = [R] [\sigma] [R^T] = \begin{bmatrix} 194 & -42 & 0 \\ -42 & -94 & 0 \\ 0 & 0 & 0 \end{bmatrix} \text{ MPa,}$$

using the transformation matrix:

$$[R] = \begin{bmatrix} \sqrt{2}/2 & \sqrt{2}/2 & 0 \\ -\sqrt{2}/2 & \sqrt{2}/2 & 0 \\ 0 & 0 & 1 \end{bmatrix}.$$

The *alternative* way is to use the symbolic transformation rule and solve the system of equations, for each component, for the unknowns  $\sigma_{12}$ ,  $\sigma_{22}$ , and  $\sigma'_{22}$ .

$$[\sigma'] = [R] [\sigma] [R^T] = \begin{bmatrix} \sqrt{2}/2 & \sqrt{2}/2 & 0 \\ -\sqrt{2}/2 & \sqrt{2}/2 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 92 & \sigma_{12} & 0 \\ \sigma_{12} & \sigma_{22} & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} \sqrt{2}/2 & -\sqrt{2}/2 & 0 \\ \sqrt{2}/2 & \sqrt{2}/2 & 0 \\ 0 & 0 & 1 \end{bmatrix} = \begin{bmatrix} 194 & -42 & 0 \\ -42 & \sigma'_{22} & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

which allows to solve without geometry and force balance, after matrix multiplications:

$$[\sigma'] = (1/\sqrt{2}) \begin{bmatrix} 92 + \sigma_{12} & \sigma_{12} + \sigma_{22} & 0 \\ -92 + \sigma_{12} & -\sigma_{12} + \sigma_{22} & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} \sqrt{2}/2 & -\sqrt{2}/2 & 0 \\ \sqrt{2}/2 & \sqrt{2}/2 & 0 \\ 0 & 0 & 1 \end{bmatrix} = \begin{bmatrix} 194 & -42 & 0 \\ -42 & \sigma'_{22} & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

$$(1/2) \begin{bmatrix} 92 + 2\sigma_{12} + \sigma_{22} & -92 + \sigma_{22} & 0 \\ -92 + \sigma_{22} & 92 - 2\sigma_{12} + \sigma_{22} & 0 \\ 0 & 0 & 0 \end{bmatrix} = \begin{bmatrix} 194 & -42 & 0 \\ -42 & \sigma'_{22} & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

which yields from the non-diagonal 12-component  $\sigma_{22} = 8$  MPa. Inserted into the 11-component, one finds  $\sigma_{12} = 144$  MPa, and all inserted into the 22-component results in  $\sigma'_{22} = -94$  MPa. These results are identical to the above geometry and force balance considerations.

c)

The principal stresses and eigen-directions can now be computed the usual way from

$$\det(\sigma_{ij} - \sigma\delta_{ij}) = 0 ,$$

and  $(\sigma_{ij} - \sigma\delta_{ij})n_j = 0$ , with normalization  $n_j^2 = 1$ .

c.1) This stress tensor describes a plane-stress state and thus has one eigenvalue  $\sigma = 0$ . The remaining characteristic equation is:

$$\sigma^2 - 100\sigma + 736 - 144^2 = 0$$

with solutions:  $\sigma_{1,2} = (100 \pm \sqrt{100^2 - 4(736 - 144^2)})/2 = (100 \pm \sqrt{9.10^4})/2 = 50 \pm 150$  MPa.

The sorted eigen-values are thus:  $\sigma_I = 200$  MPa,  $\sigma_{II} = 0$  MPa,  $\sigma_{III} = -100$  MPa.

c.2) Insert values, solve the system of equations, and normalize the solution.

$$\begin{bmatrix} \sigma_{11} - \sigma_I & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & \sigma_{22} - \sigma_I & \sigma_{23} \\ \sigma_{13} & \sigma_{23} & \sigma_{33} - \sigma_I \end{bmatrix} \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \quad \text{and} \quad n_1^2 + n_2^2 + n_3^2 = 1$$

$\implies$

$$(92 - 200)n_1 + 144n_2 + 0n_3 = 0$$

$$144n_1 + (8 - 200)n_2 + 0n_3 = 0$$

$\implies$

$$-108n_1 + 144n_2 + 0n_3 = 0$$

$$144n_1 - 192n_2 + 0n_3 = 0$$

$\implies$

$$n_2 = (108/144)n_1 = (3/4)n_1$$

$\implies$

$$n_1^2 + n_2^2 = (1 + 9/16)n_1^2 = 1$$

$\implies$

$$n_1 = \sqrt{16/25} = \pm 4/5 = \pm 0.8$$

The eigen-direction associated to the first, largest eigen-value:

$$\hat{n}^{(I)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \begin{bmatrix} 0.8 \\ 0.6 \\ 0 \end{bmatrix}$$

Similarly (no details given), the eigen-direction associated to the third, smallest eigen-value:

$$\hat{n}^{(III)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \begin{bmatrix} 0.6 \\ -0.8 \\ 0 \end{bmatrix}$$

and without calculation necessary (due to structure of the matrix), for the second, intermediate:

$$\hat{n}^{(II)} = \begin{bmatrix} n_1 \\ n_2 \\ n_3 \end{bmatrix} = \pm \begin{bmatrix} 0 \\ 0 \\ 1 \end{bmatrix}$$

## 5 Stress equilibrium

... based on sections 3,5 (Exercise V12 in old material before 2022)

In a linear elastic ( $E = 2 \cdot 10^5$  MPa,  $\nu = 0.25$ ) body under load, the stress-field is given (with four free parameters), with respect to the Cartesian  $x_1 - x_2 - x_3$  coordinate system as:

$$\sigma_{11}(x_1, x_2, x_3) = \sigma_0 \left[ 20 + \alpha_1 \left( \frac{x_1}{L} \right) - 10 \left( \frac{x_2}{L} \right) + \alpha_2 \left( \frac{x_1}{L} \right)^2 \right]$$

$$\sigma_{22}(x_1, x_2, x_3) = \sigma_0 \left[ 10 + 8 \left( \frac{x_1}{L} \right) + \beta_1 \left( \frac{x_2}{L} \right) + \beta_2 \left( \frac{x_2}{L} \right)^2 \right]$$

$$\sigma_{12}(x_1, x_2, x_3) = \sigma_0 \left[ 12 - 10 \left( \frac{x_1}{L} \right) + 7 \left( \frac{x_2}{L} \right) - 8 \left( \frac{x_1}{L} \right) \left( \frac{x_2}{L} \right) \right]$$

$\sigma_{13}(x_1, x_2, x_3) = \sigma_{23}(x_1, x_2, x_3) = \sigma_{33}(x_1, x_2, x_3) = 0$ , and  
with reference stress  $\sigma_0 = 1$  MPa and reference length  $L = 1$  m.

Note: Question (a) is general, symbolic, with variables  $x_1, x_2, x_3$  and coefficients  $\alpha_1, \alpha_2, \beta_1, \beta_2$ ;  
only from question (b) on, use the single, chosen point P( $x_1 = 0, x_2 = 0, x_3 = 0$ ).

### Questions:

... based on section 3

- a) Does the stress field agree with the stress-equilibrium equations in absence of volume-forces?  
Which relations have to be valid for the free coefficients  $\alpha_1, \alpha_2, \beta_1, \beta_2$  due to stress equilibrium?
- b) Compute the eigen-stresses in point P using linear algebra, mathematics -- not circle of Mohr.  
Describe and name the state of stress in point P (and in all other points in the body).
- c) Compute the eigen-direction of the major eigen-stress.
- d) Draw the relevant circle of Mohr and confirm graphically the results of (b) and (c); explain.

... based on section 5

- e) Compute the equivalent stress according to Tresca.  
What is the origin of the limit-stress hypothesis of Tresca?
- f) Compute the equivalent stress according to von Mises.  
What is the origin of the limit-stress hypothesis of von Mises?
- g) Compute the specific elastic energy  $\pi_{el}$  in point P.

### Answers:

a)

Given was the plane stress-field, independent of  $x_3$ , in absence of body forces  $f_i = 0$ :

$$\begin{aligned}\sigma_{11}(x_1, x_2) &= \sigma_0 \left[ 20 + \alpha_1 \frac{x_1}{L} - 10 \frac{x_2}{L} + \alpha_2 \left( \frac{x_1}{L} \right)^2 \right] \\ \sigma_{22}(x_1, x_2) &= \sigma_0 \left[ 10 + 8 \frac{x_1}{L} + \beta_1 \frac{x_2}{L} + \beta_2 \left( \frac{x_2}{L} \right)^2 \right] \\ \sigma_{12}(x_1, x_2) &= \sigma_0 \left[ 12 - 10 \frac{x_1}{L} + 7 \frac{x_2}{L} - 8 \frac{x_1}{L} \frac{x_2}{L} \right]\end{aligned}$$

Using the respective stress-equilibrium equations, in this case two, one obtains:

$$\begin{aligned}\frac{d}{dx_1} \sigma_{11}(x_1, x_2) + \frac{d}{dx_2} \sigma_{12}(x_1, x_2) &= \sigma_0 \left[ \frac{\alpha_1}{L} + 2\alpha_2 \frac{x_1}{L^2} \right] + \sigma_0 \left[ \frac{7}{L} - 8 \frac{x_1}{L^2} \right] = 0 \\ \frac{d}{dx_1} \sigma_{12}(x_1, x_2) + \frac{d}{dx_2} \sigma_{22}(x_1, x_2) &= \sigma_0 \left[ \frac{-10}{L} - 8 \frac{x_2}{L^2} \right] + \sigma_0 \left[ \frac{\beta_1}{L} + 2\beta_2 \frac{x_2}{L^2} \right] = 0\end{aligned}$$

From these equations, one gets the coefficients that solve them:  $\alpha_1 = -7$ ,  $\alpha_2 = 4$ ,  $\beta_1 = 10$ ,  $\beta_2 = 4$ .

Because the field equations must be valid for all constants and points  $x_1, x_2, x_3$ , independently, one can group them accordingly: The constant terms from the first and second equations provide  $\alpha_1$  and  $\beta_1$ , respectively, while the  $x_1$  and  $x_2$  groups provide  $\alpha_2$  and  $\beta_2$ .

b)

The stress Tensor in point  $P = (x_1 = 0, x_2 = 0, x_3 = 0)$  is:  $[\sigma_{ij}] = \begin{bmatrix} 20 & 12 & 0 \\ 12 & 10 & 0 \\ 0 & 0 & 0 \end{bmatrix}$  MPa

From this stress tensor, the characteristic equation is:

$$\sigma^3 - I_1\sigma^2 + I_2\sigma - I_3 = \sigma^3 - 30\sigma^2 + 56\sigma - 0 = (\sigma^2 - 30\sigma + 56)(\sigma - 0) = 0.$$

Knowing/recognizing that one eigen-value is zero, i.e. also  $I_3 = 0$ , the principal stresses can be computed from the second order polynomial as:  $\sigma_I = 28$  MPa,  $\sigma_{II} = 2$  MPa,  $\sigma_{III} = 0$  MPa. This is a plane-stress state with all stresses on the  $x_3$ -surface equal to zero, which also has consequences for the eigen-directions ...

c)

The principal directions can be calculated the usual way, where  $\hat{\mathbf{n}}^{(III)} = (0, 0, 1)$  is directly visible from the tensor, due to the zero shear stresses in the  $x_3$ -direction.

The eigen-direction of the major stress  $\sigma_I = 28$  MPa is obtained solving:

$$\begin{bmatrix} \sigma_{11} - \sigma_I & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & \sigma_{22} - \sigma_I & \sigma_{23} \\ \sigma_{13} & \sigma_{23} & \sigma_{33} - \sigma_I \end{bmatrix} \begin{bmatrix} n_1^{(I)} \\ n_2^{(I)} \\ n_3^{(I)} \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \text{ and normalization: } n_1^{(I)} + n_2^{(I)} + n_3^{(I)} = 1$$

so that:  $-8n_1^{(I)} + 12n_2^{(I)} = 0 \rightarrow n_1^{(I)} = (3/2)n_2^{(I)}$  and thus:  $[(9/4) + 1]n_2^{(I)} = 1 \rightarrow n_2^{(I)} = 2/\sqrt{13}$

$$\Rightarrow \begin{bmatrix} n_1^{(I)} \\ n_2^{(I)} \\ n_3^{(I)} \end{bmatrix} = \pm \begin{bmatrix} 3/\sqrt{13} \\ 2/\sqrt{13} \\ 0 \end{bmatrix}$$

The eigen-direction of the intermediate stress,  $\sigma_{II} = 2$  MPa was not asked, just for completeness:

$$\begin{bmatrix} \sigma_{11} - \sigma_{II} & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & \sigma_{22} - \sigma_{II} & \sigma_{23} \\ \sigma_{13} & \sigma_{23} & \sigma_{33} - \sigma_{II} \end{bmatrix} \begin{bmatrix} n_1^{(II)} \\ n_2^{(II)} \\ n_3^{(II)} \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \text{ and normalization: } n_1^{(II)} + n_2^{(II)} + n_3^{(II)} = 1$$

so that:  $18n_1^{(II)} + 12n_2^{(II)} = 0 \rightarrow n_1^{(II)} = -(2/3)n_2^{(II)}$  and thus:  $[(4/9) + 1]n_2^{(II)} = 1 \rightarrow n_2^{(II)} = 3/\sqrt{13}$

$$\Rightarrow \begin{bmatrix} n_1^{(II)} \\ n_2^{(II)} \\ n_3^{(II)} \end{bmatrix} = \pm \begin{bmatrix} -2/\sqrt{13} \\ 3/\sqrt{13} \\ 0 \end{bmatrix}$$

d)

Mohr's circle

Consider only the two non-zero eigenvalues that characterise the plane-stress state in point P.

The circle centre is:  $M = \sigma_{avg} = \frac{\sigma_{xx} + \sigma_{yy}}{2} = \frac{20+10}{2} = 15$  MPa,

and its radius is:  $R = \sqrt{\left(\frac{\sigma_{xx} - \sigma_{yy}}{2}\right)^2 + \sigma_{xy}^2} = \sqrt{\left(\frac{20-10}{2}\right)^2 + (12)^2} = 13$  MPa.

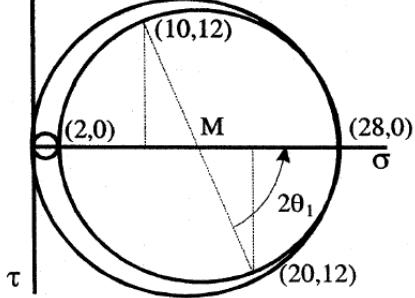


Figure 4: Sketch of a Mohr circle, focus is on the right, inner circle.

The eigenvalues are therefore:

$$\sigma_I = M + R = 28 \text{ MPa}, \sigma_{II} = C - R = 2 \text{ MPa}.$$

The eigen-directions are:

$\tan 2\theta = \frac{2\sigma_{xy}}{\sigma_{xx} - \sigma_{yy}} = \frac{24}{10} = 2.4 \implies \theta_I = (1/2) \arctan(2.4) = 67.38^\circ/2 = 33.69^\circ$ , which corresponds to the orientation of the first eigenvector relative to the horizontal  $\theta_I = \arcsin(2/\sqrt{13}) = \arccos(3/\sqrt{13})$ ; and  $\theta_{II} = (180^\circ + 67.3^\circ)/2 = 247.3^\circ/2 = 123.7^\circ = \arccos(-2/\sqrt{13})$ .

The maximum shear stress is just the radius:  $\tau^{max} = R = 13 \text{ MPa}$

e) Failure criteria according to the (double) maximal shear stress:

$$\tau_{max} = \frac{1}{2} |\sigma_{max} - \sigma_{min}| = 14 \text{ MPa}, \sigma_{eq}^{Tresca} = 2\tau_{max} = 28 \text{ MPa}$$

f) Failure criteria according to the shape-change (distortion) energy:

$$\sigma_{eq}^{von-Mises} = \sqrt{\frac{1}{2}[(\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2]} \approx 27 \text{ MPa}$$

$\sigma_{eq}^{von-Mises} < \sigma_{eq}^{Tresca}$ , thus the Tresca criterion is safer, since the limit stress is reached earlier.

g) Hooke's law for strain as function of stress:  $\varepsilon_{ij} = \frac{1}{E}[(1+\nu)\sigma_{ij} - \nu\sigma_{kk}\delta_{ij}]$ , with  $\sigma_{kk} = \sigma_{11} + \sigma_{22} + \sigma_{33}$ , using  $E = 2.10^5 \text{ MPa}$  and  $\nu = 1/4$ , one obtains:

$$\varepsilon_{11} = \frac{\sigma_{11}}{E} - \nu \frac{\sigma_{22}}{E} - \nu \frac{\sigma_{33}}{E} = 20/E - (10/4)/E = (35/2) \text{ MPa } E^{-1},$$

$$\varepsilon_{22} = \frac{\sigma_{22}}{E} - \nu \frac{\sigma_{11}}{E} - \nu \frac{\sigma_{33}}{E} = 10/E - (20/4)/E = 5 \text{ MPa } E^{-1},$$

$$\varepsilon_{33} = \frac{\sigma_{33}}{E} - \nu \frac{\sigma_{11}}{E} - \nu \frac{\sigma_{22}}{E} = -(10/4)/E - (20/4)/E = -(15/2) \text{ MPa } E^{-1},$$

$$\varepsilon_{12} = \frac{\sigma_{12}}{2G} = 6/G = 15 \text{ MPa } E^{-1} \text{ (with } G = \frac{E}{2(1+\nu)} = 2E/5),$$

and  $\varepsilon_{13} = \varepsilon_{23} = 0$ . Note that  $\varepsilon_{33} \neq 0$ , even though  $\sigma_{33} = 0$ .

$$[\varepsilon] = \begin{bmatrix} 35/2 & 15 & 0 \\ 15 & 10/2 & 0 \\ 0 & 0 & -15/2 \end{bmatrix} \text{ MPa } E^{-1} = \begin{bmatrix} 35 & 30 & 0 \\ 30 & 10 & 0 \\ 0 & 0 & -15 \end{bmatrix} \frac{10^{-5}}{4} = \begin{bmatrix} 87.5 & 75 & 0 \\ 75 & 25 & 0 \\ 0 & 0 & -37.5 \end{bmatrix} 10^{-6}.$$

Elastic energy:

$$\pi_{el} = 0.5 \sum_{i=1}^3 \sum_{j=1}^3 \sigma_{ij} \varepsilon_{ij} = \sigma_{ij} \varepsilon_{ij} = 1.9 \cdot 10^{-3} \text{ MPa} \left( = \frac{\text{Energy}}{\text{Volume}} \right)$$

in detail (diamonds entries not needed):

$$\begin{aligned}
\pi_{el} &= \begin{bmatrix} 20 & 12 & 0 \\ 12 & 10 & 0 \\ 0 & 0 & 0 \end{bmatrix} \cdot \begin{bmatrix} 87.5 & 75 & 0 \\ 75 & 25 & 0 \\ 0 & 0 & -37.5 \end{bmatrix} \frac{10^{-6}}{2} \text{ MPa} \\
&= \text{tr} \begin{bmatrix} 20 * 87.5 + 12 * 75 & \diamond & 0 \\ \diamond & 12 * 75 + 10 * 25 & 0 \\ 0 & 0 & 0 \end{bmatrix} \frac{10^{-6}}{2} \text{ MPa} \\
&= (1750 + 900 + 900 + 250) \frac{10^{-6}}{2} \text{ MPa} = 1.9 \cdot 10^{-3} \text{ MPa}
\end{aligned}$$

*Note that stress and energy density have the same units.*

## 6 Stress and transformation

... based on sections 3, 4, 5.1 (Exercise V4 in old material before 2022)

**Given:**

$$E = 2 \cdot 10^{11} \text{ Pa}, \nu = 0.25$$

Stress-state in point P:  $[\sigma] = \begin{bmatrix} 19 & -5 & -\sqrt{6} \\ -5 & 19 & -\sqrt{6} \\ -\sqrt{6} & -\sqrt{6} & 10 \end{bmatrix} \text{ MPa}$

**Questions:**

- a) Show that the principal stresses are 8, 16 and 24 MPa.
- Compute the directional cosines (transformation matrix entries) of the smallest eigen-stress.  
... based on sections 4,5
- b) Compute the volumetric (isotropic) strain.
- c) What is the largest angle-change (not shear-strain) in P?  
... based on section 5
- d) Which material property is implicitly used/assumed in Hooke's law?

**Answers:**

a)

From  $\det(\sigma_{ij} - \sigma\delta_{ij}) = 0$ , the characteristic equation follows as:

$$\sigma^3 - I_1\sigma^2 + I_2\sigma - I_3 = \sigma^3 - 48\sigma^2 + 704\sigma - 3072 = 0.$$

Given the eigenvalues,  $\sigma$ , one can test their validity by inserting one by one; or one can factorize the equation, e.g. by polynomial division; or one computes the invariants from the eigen-values and confirms the characteristic equation. *Watch the signs in the definitions.*

Sorting the eigen-values is convention and part of the answer:

$$\sigma_I = 24 \text{ MPa}, \sigma_{II} = 16 \text{ MPa}, \text{ and } \sigma_{III} = 8 \text{ MPa}.$$

it allows to refer a certain eigen-value, e.g. the smallest and its eigen-direction.

The eigen-direction of the minor eigen-stress,  $\sigma_{III} = 8 \text{ MPa}$  is obtained by solving:

$$\begin{bmatrix} \sigma_{11} - \sigma_{III} & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & \sigma_{22} - \sigma_{III} & \sigma_{23} \\ \sigma_{13} & \sigma_{23} & \sigma_{33} - \sigma_{III} \end{bmatrix} \begin{bmatrix} n_1^{(III)} \\ n_2^{(III)} \\ n_3^{(III)} \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \text{ and normalization: } n_1^{(III)} + n_2^{(III)} + n_3^{(III)} = 1$$

so that (dropping the superscript for brevity):

$$11n_1 - 5n_2 - \sqrt{6}n_3 = 0 \rightarrow n_1 = (5/11)n_2 + (\sqrt{6}/11)n_3$$

$$-5n_1 + 11n_2 - \sqrt{6}n_3 = 0 \rightarrow n_2 = (5/11)n_1 + (\sqrt{6}/11)n_3$$

$$-\sqrt{6}n_1 - \sqrt{6}n_2 + 2n_3 = 0 \rightarrow n_3 = (\sqrt{6}/2)n_1 + (\sqrt{6}/2)n_2$$

Subtracting line 2 from 1 yields:  $n_1 - n_2 = (5/11)(n_2 - n_1) \rightarrow n_1 = n_2$

$$\text{Inserting into line 3 yields: } n_3 = \sqrt{6}n_1, \text{ so that: } \begin{bmatrix} n_1^{(III)} \\ n_2^{(III)} \\ n_3^{(III)} \end{bmatrix} = \pm c \begin{bmatrix} 1 \\ 1 \\ \sqrt{6} \end{bmatrix}$$

where the unknown  $c = 1/\sqrt{8} = \sqrt{2}/4$  is obtained from normalization, resulting in:

$$\implies \begin{bmatrix} n_1^{(III)} \\ n_2^{(III)} \\ n_3^{(III)} \end{bmatrix} = \pm \begin{bmatrix} \sqrt{2}/4 \\ \sqrt{2}/4 \\ \sqrt{3}/2 \end{bmatrix}$$

b)

For the volumetric (isotropic) strain, we can use the short-cut (not the full strain tensor), as:  
 $\varepsilon_V = \varepsilon_1 + \varepsilon_2 + \varepsilon_3 = \left[ \frac{1-2\nu}{E} \right] \sigma_{kk} = 12 \cdot 10^{-5}$ .

c)

The largest change of angle is:  $\gamma_{max} = \frac{\tau_{max}}{G} = \frac{1}{2} \frac{\sigma_I - \sigma_{III}}{G} = 10^{-4}$ , using  $G = \frac{E}{2(1+\nu)} = (4/5) \cdot 10^5$  MPa,  
 where the largest shear strain is just half of that:  $\varepsilon_{max} = \gamma_{max}/2$ .

d)

Isotropic (direction independent) material behavior is intrinsically assumed in the law of Hooke.

## 7 Displacement, strain and stress relation

... based on sections 3,4,5 (Exercise V7 in old material before 2022)

Given is the displacement field:

$$u_1 = x_1 x_3, \quad u_2 = -x_1 x_2, \quad \text{and} \quad u_3 = x_1^2 - x_3^2$$

and material properties  $E = 2$  and  $\nu = 0.25$

(discuss the units, but drop them in calculations to save space).

### Questions:

- a) Compute the stress tensor (components).
- b) In the point  $(x_1, x_2, x_3) = (0, 0, z_0)$  compute the eigen-stresses and maximum shear stress.

## 8 Displacement, strain and stress equilibrium

... based on sections 3,4,5 (Exercise V8 in old material before 2022)

In a homogenous body made of a linear elastic, isotropic material, the displacement field is given:

$$u_1 = \frac{p}{E} a \left[ \frac{x_2}{a} + 2 \frac{x_1 x_2}{a^2} - \frac{x_2^2}{a^2} \right]$$

$$u_2 = \frac{p}{E} a \left[ \frac{x_1}{a} + \alpha \frac{x_1^2}{a^2} + \beta \frac{x_1 x_2}{a^2} - 2 \frac{x_2^2}{a^2} \right]$$

$$u_3 = 0$$

with coordinates  $x_1$  and  $x_2$ , and constant coefficients  $p, E, a$ , and  $\nu = 0.25$ .

### Questions:

In the absence of volume forces, compute the magnitude of the parameters  $\alpha$  and  $\beta$ , using the information that the stress field is in mechanical equilibrium.

## 9 Displacement, strain, stress

*... based on sections 3,4,5 (Exercise V9 in old material before 2022)*

Within a homogeneous body made of a linear elastic, isotropic material the displacement field:

$$\begin{aligned} u_1 &= \frac{1}{3}(1-2\nu)x_1^3 - (3-2\nu)x_1x_2^2 - 3x_2 - 3x_3 \\ u_2 &= (1-2\nu)x_1^2x_2 + \frac{1}{3}(1+2\nu)x_2^3 + 3x_1 - 4x_3 \\ u_3 &= 3x_1 + 4x_2 \end{aligned}$$

the elasticity modulus  $E$ , and the Poisson-ratio  $\nu$  are given.

### Questions:

- Compute the components of the strain tensor.

*... based on section 5.1*

- Compute the components of the stress tensor.

*... based on section 3,5.1*

- Confirm that the stress-field is in equilibrium in the absence of volume forces.