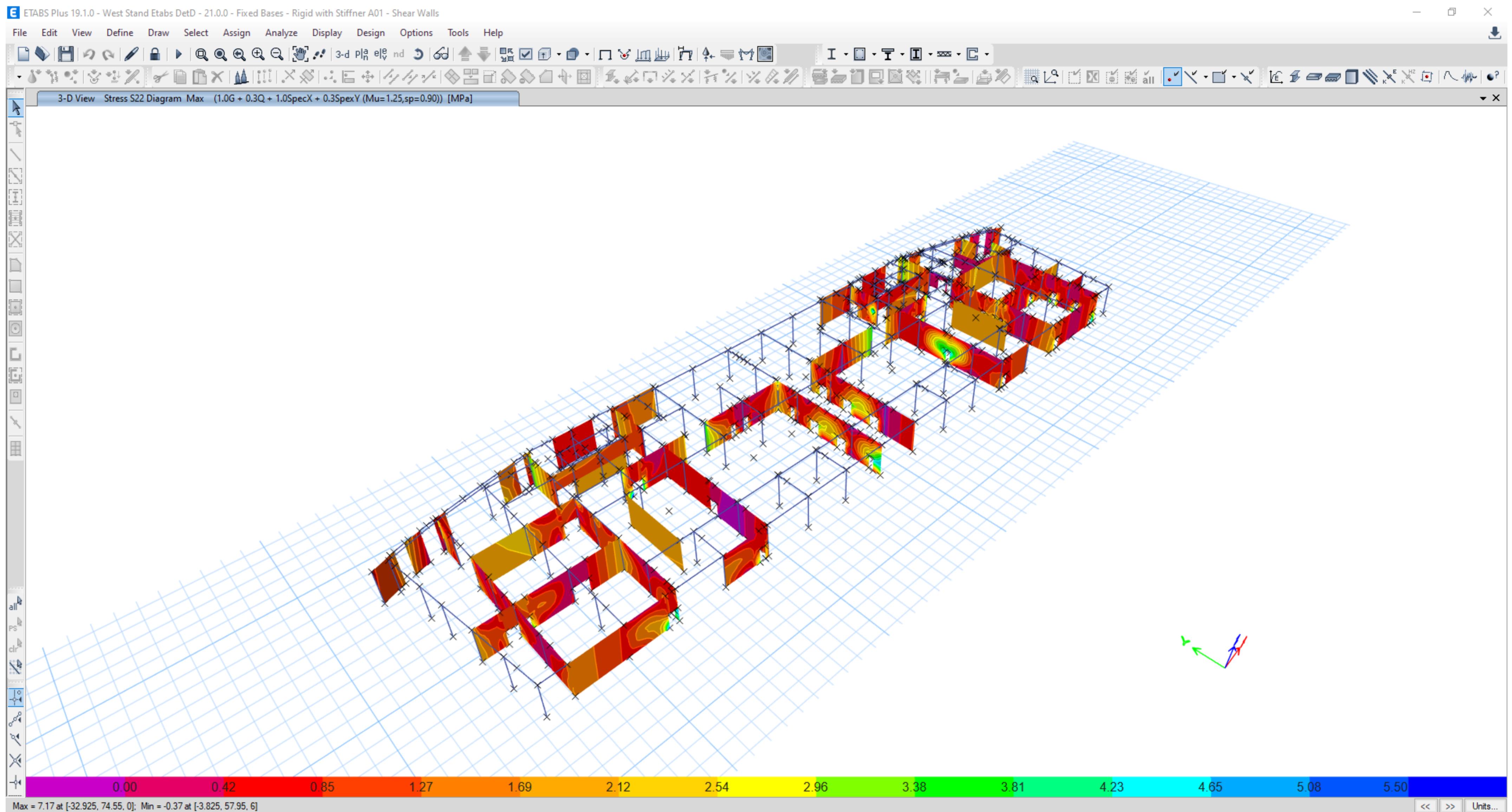


## West Stand Shear Wall Design

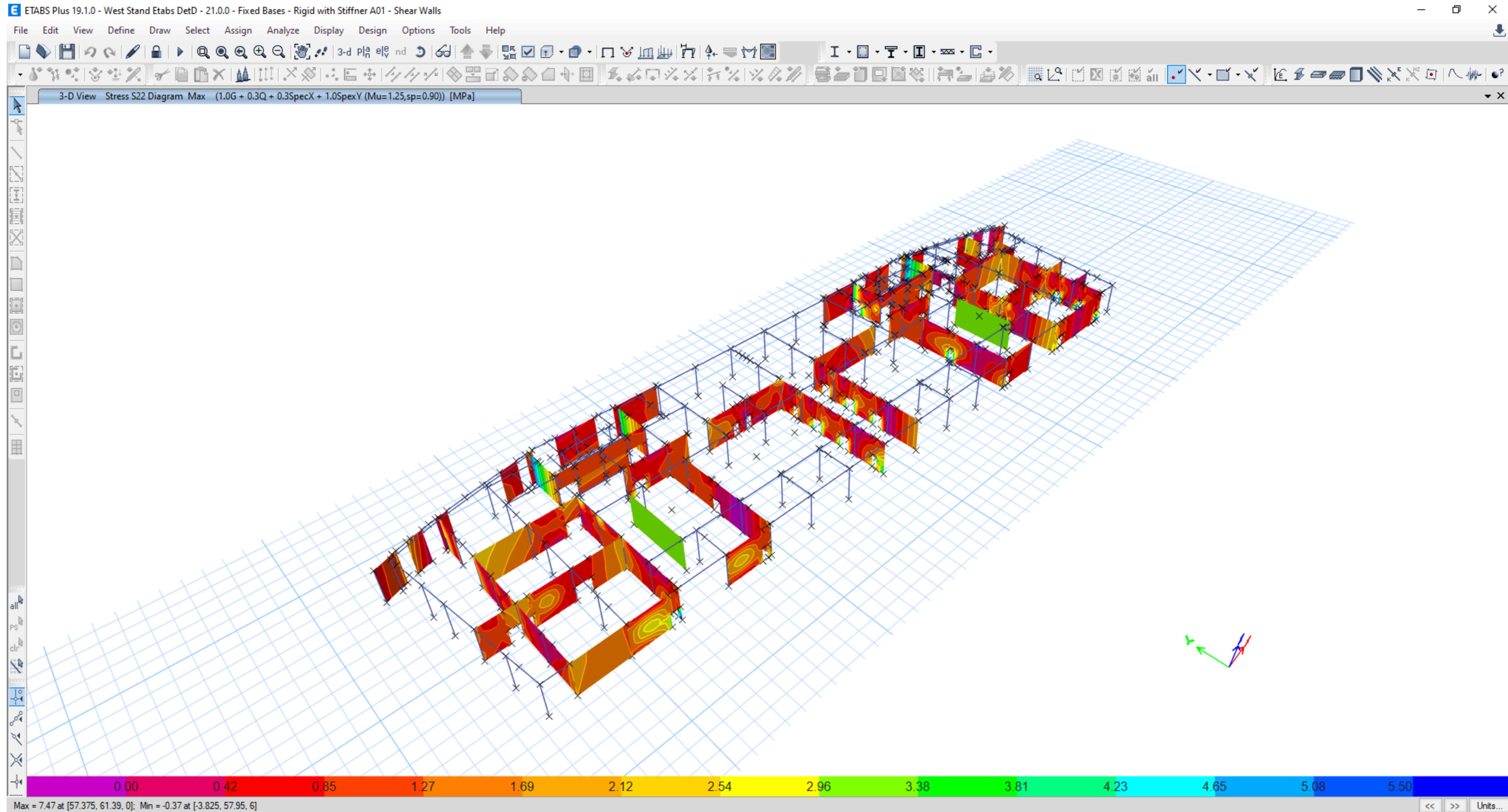
# General Stress Distribution (Vertical)

1.0G + 0.30Q + 1.0SpecX + 0.30SpecY [mu = 1.25, sp = 0.90]

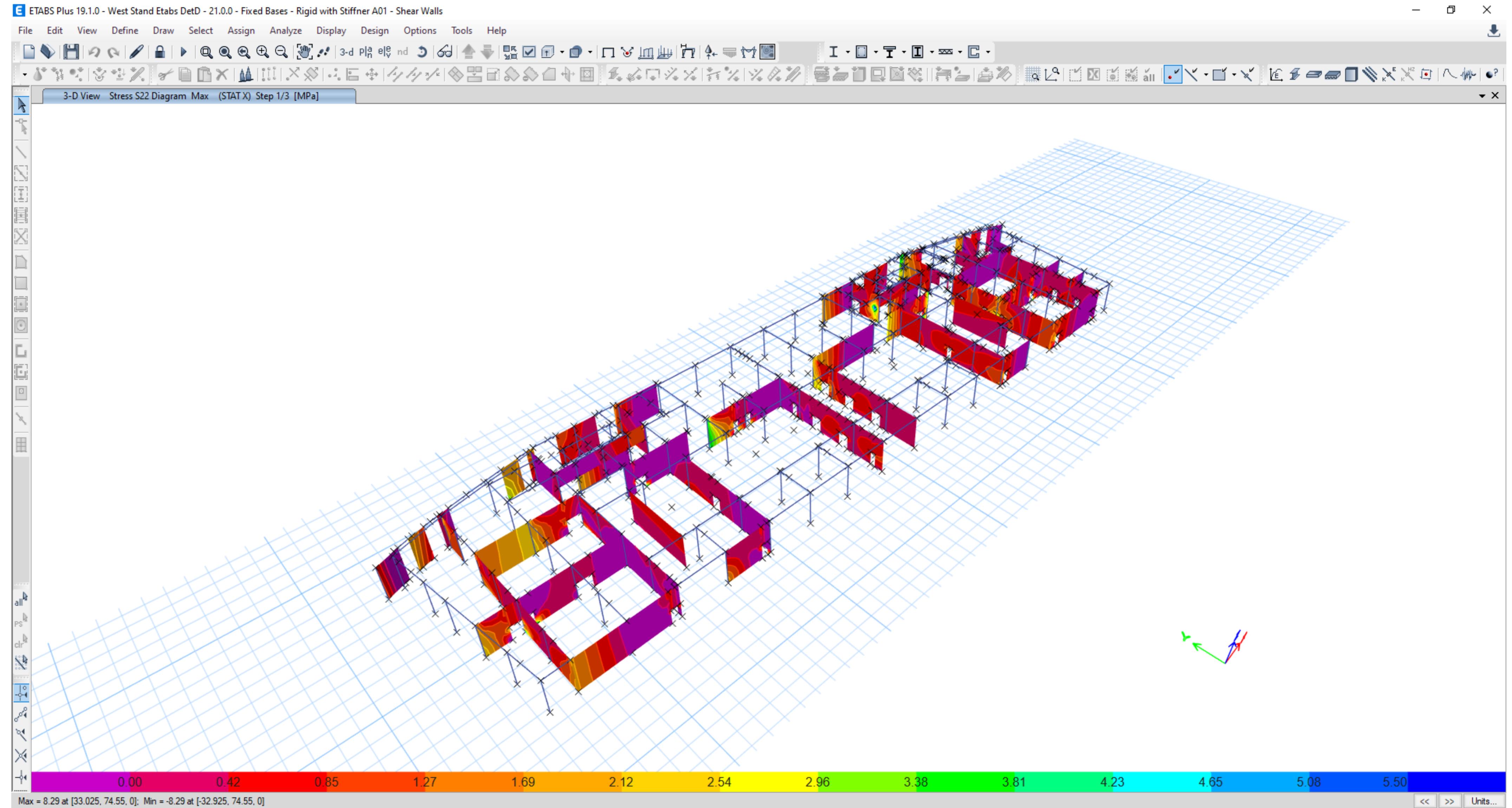


# General Stress Distribution (Vertical)

1.0G + 0.30Q + 0.30SpecX + 1.0SpecY [mu = 1.25, sp = 0.90]

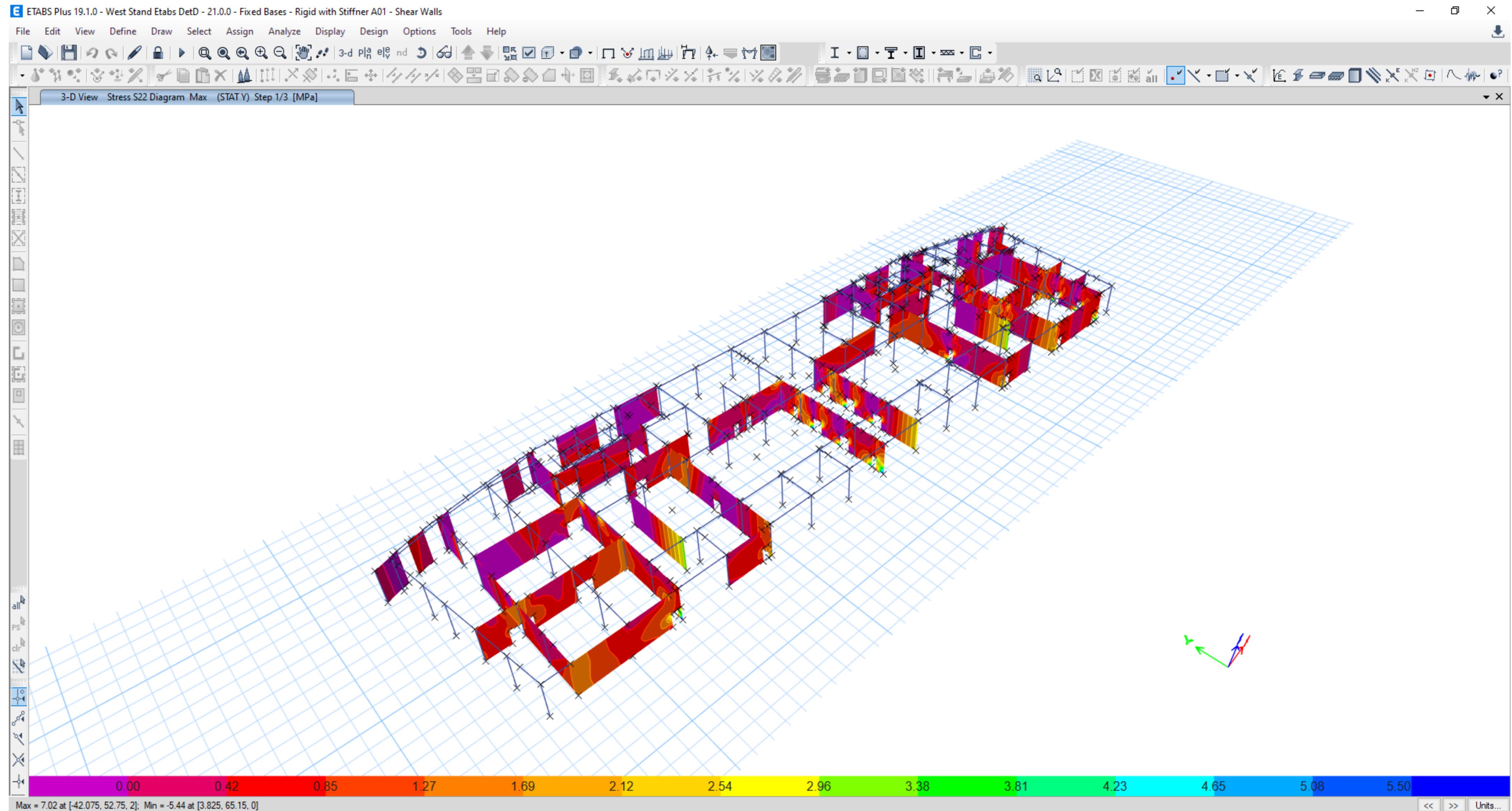


# General Stress Distribution (Vertical) STATX [ $\mu = 1.00$ , $sp = 0.90$ ]



# General Stress Distribution (Vertical)

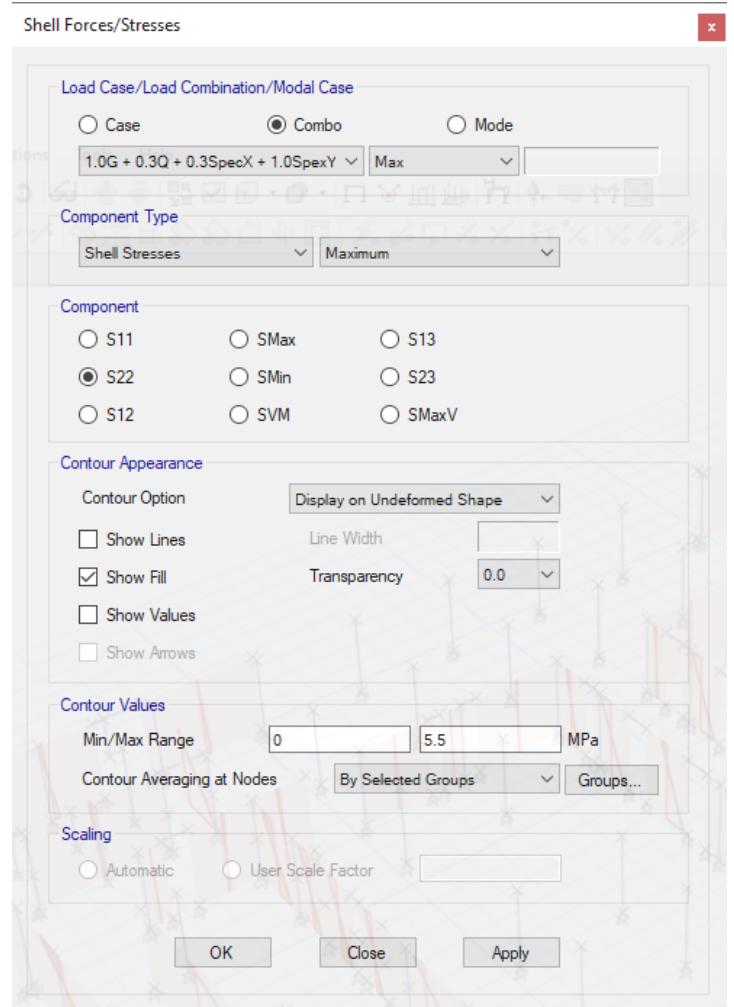
STATY [ $\mu = 1.00$ ,  $sp = 0.90$ ]



Stress, MPa	Wall Thickness, mm	Wall Depth, mm	Force, kN/m	Bar Dia, mm	Bar Area, mm <sup>2</sup>	Bar Spacing	Number of Bars	Number of Layers	Total Area of Reo, mm <sup>2</sup>	Yield Strength, Mpa	Cap, kN/m	Ult, %
0.5	350	1000	175	16	201.06	400	2.5	1	502.7	500	251.3274	70%
1	350	1000	350	20	314.16	400	2.5	1	785.4	500	392.6991	89%
1.5	350	1000	525	25	490.87	350	2.9	1	1402.5	500	701.2484	75%
2	350	1000	700	25	490.87	300	3.3	1	1636.2	500	818.1231	86%
2.5	350	1000	875	32	804.25	300	3.3	1	2680.8	500	1340.413	65%
3	350	1000	1050	32	804.25	300	3.3	1	2680.8	500	1340.413	78%
3.5	350	1000	1225	32	804.25	250	4.0	1	3217.0	500	1608.495	76%
4	350	1000	1400	32	804.25	250	4.0	1	3217.0	500	1608.495	87%
4.5	350	1000	1575	32	804.25	250	4.0	1	3217.0	500	1608.495	98%
5	350	1000	1750	32	804.25	225	4.4	1	3574.4	500	1787.217	98%
5.5	350	1000	1925	32	804.25	200	5.0	1	4021.2	500	2010.619	96%

## Shear Wall Design Methodology

1. Check all walls in accordance with NZS 3101:2006. Against the maximum bending (of all load combination which present a net axial tensile force in the wall) moment paired with the maximum tension.
2. In regards to  $V^*$ , Shear Forces use refined loads which consider the contribution of shear from the steel structure to be loaded with ( $\mu_u = 3.5 \times OS$ ) while the concrete structure is loaded at ( $\mu_u = 1.25$ ,  $s_p = 0.9$ ).
3. For walls which fail NZS 3101:2006 moment checks; inspect MAX - S22 (Vertical Tensile Stresses). Take 1000mm average stress and check reinforcement required. ETABS moments are conservative as they include consider locally high stresses.
4. Consider the shear friction capacity of columns connecting to shear walls.



## Refinement of Shear Forces in West Stand

Shear walls and other RC elements need to be designed to withstand the following load cases:-

- $1.0G + 0.30Q + 1.0\text{SpecX} + 0.30\text{SpecY}$  ( $\mu = 1.25$ ,  $Sp = 0.90$ )
- $1.0G + 0.30Q + 0.3\text{SpecX} + 1.0\text{SpecY}$  ( $\mu = 1.25$ ,  $Sp = 0.90$ )

Although the ground floor RC elements need to be designed for the ( $\mu = 1.25$ ,  $sp = 0.90$ ) with a 30% orthogonal force, the steel elements above the concourse need to be designed to:

- $1.0G + 0.30Q + 0.30\text{Spex} + 1.0\text{SpecY}$  ( $\mu = 3.5 \times OS$ )
- $1.0G + 0.30Q + 1.0\text{Spex} + 0.30\text{SpecY}$  ( $\mu = 3.5 \times OS$ )

ETABS does not account for this and will overestimate the amount of force the concrete elements will be seeing.

### **Method:**

To determine more accurate wall values of Shear Forces the following steps will be taken:

1. Take a section cut just above the concourse level to determine the total horizontal forces acting through the stand under equivalent static.
2. Take a section cut just below the concourse level to determine the total horizontal forces seen by the concrete elements at ground floor.
3. Find the difference between the two aforementioned values. This difference is the total contribution from the concourse only.
4. Apply the difference as a horizontal shell load on the concourse in a separate load case.
5. Obtain the shear values out of the wall and determine the proportional split of shear through the walls. The shear force in the walls are solely from concourse up.
6. C, % Force Contribution from Steelwork = Difference in Inertial Force  $V^*$  / STAT(X or Y)  $V^*$

Real Shear in Shear Walls =  $V^*.G+0.3Q + V^*.concourse + V^*.steelwork$

### **Where:**

$$V.concourse = V^* (100\%/30\%, \mu = 1.25, sp = 0.90) \times (1 - C)$$

$$V.steelwork = V^* (100\%, \mu = 3.5 \times OS) \times C$$



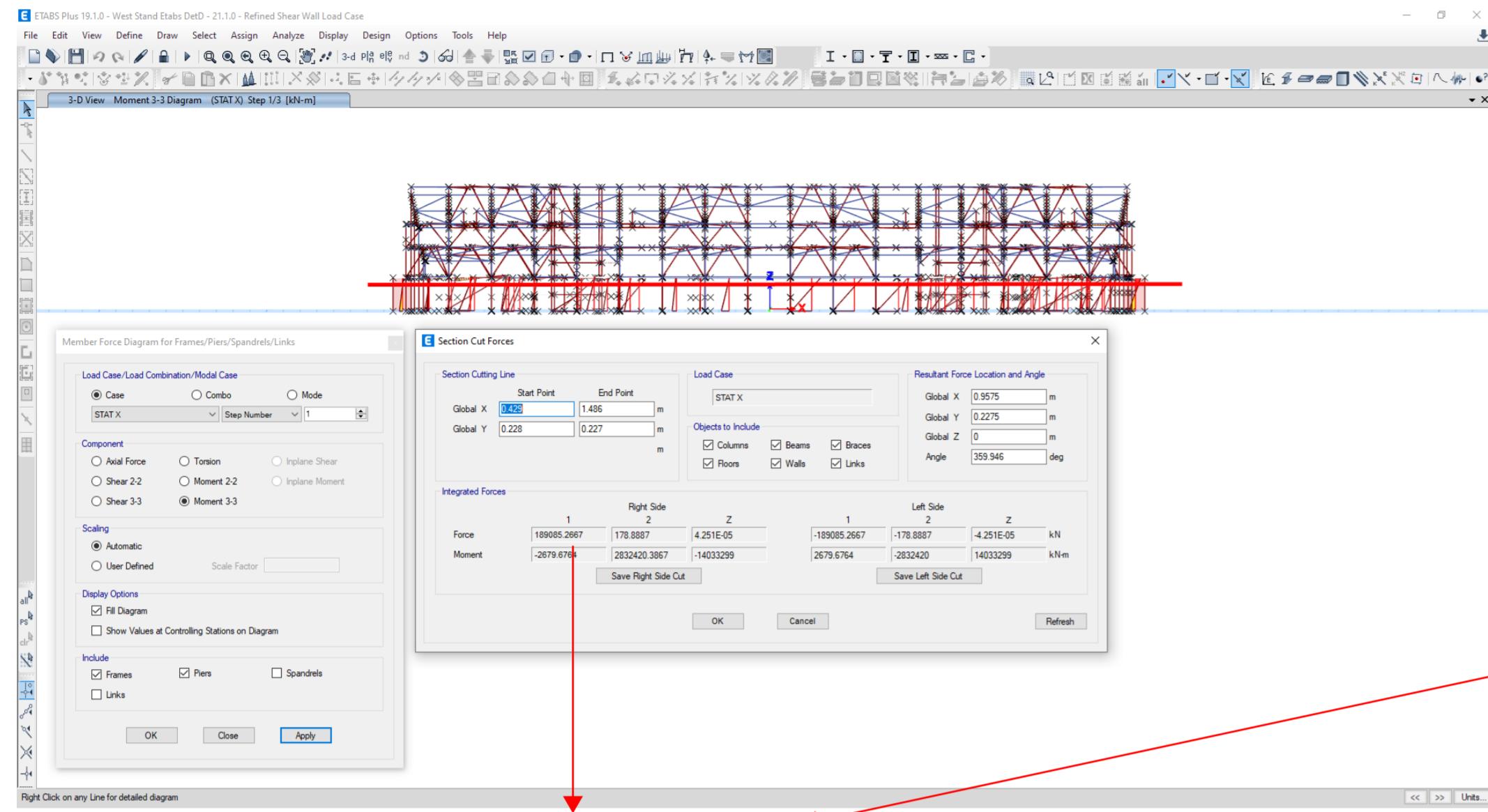
### **Over Strength Calculation:**

$$\text{Material Over-strength} = 1.5 / 0.9 = 1.67$$

$$\text{Building Over-strength} = 85\%$$

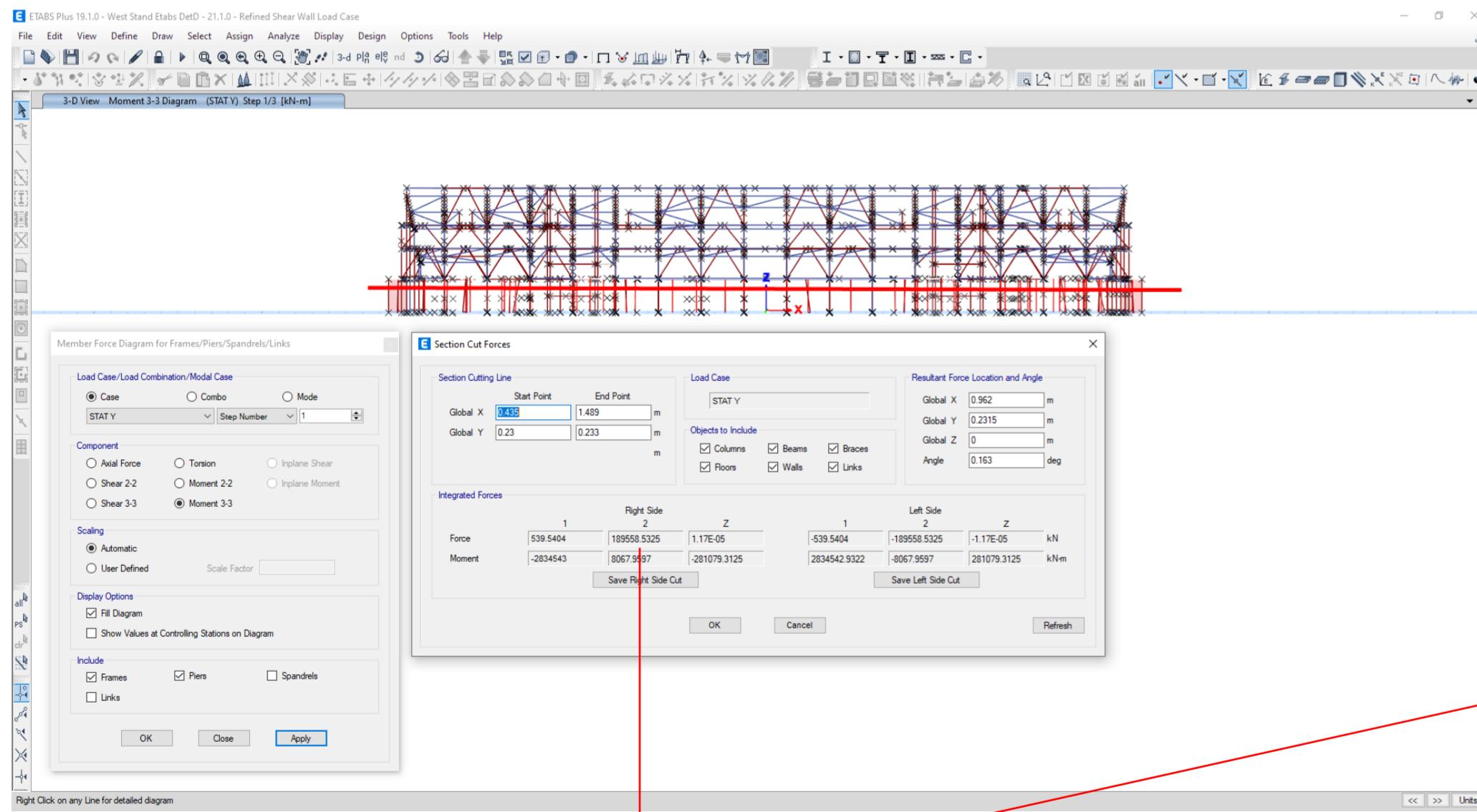
$$1.67 / 0.85 = 1.9647$$

## STATX, Section Cut, Below Concourse



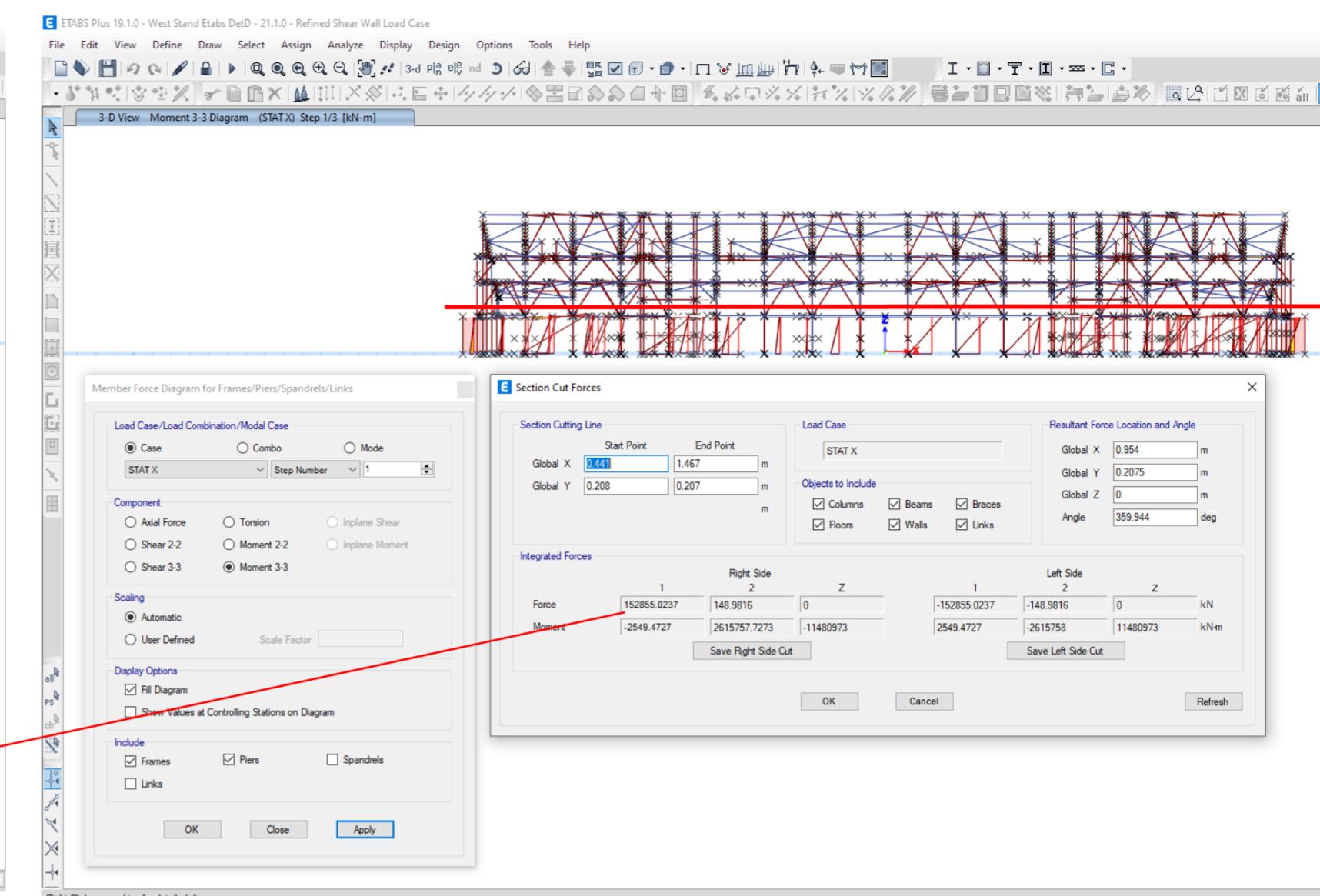
Inertial Force of Concourse =  $189,085\text{kN} - 152,855\text{kN} = 36,230\text{kN}$  (STATX)

## STATY, Section Cut, Below Concourse

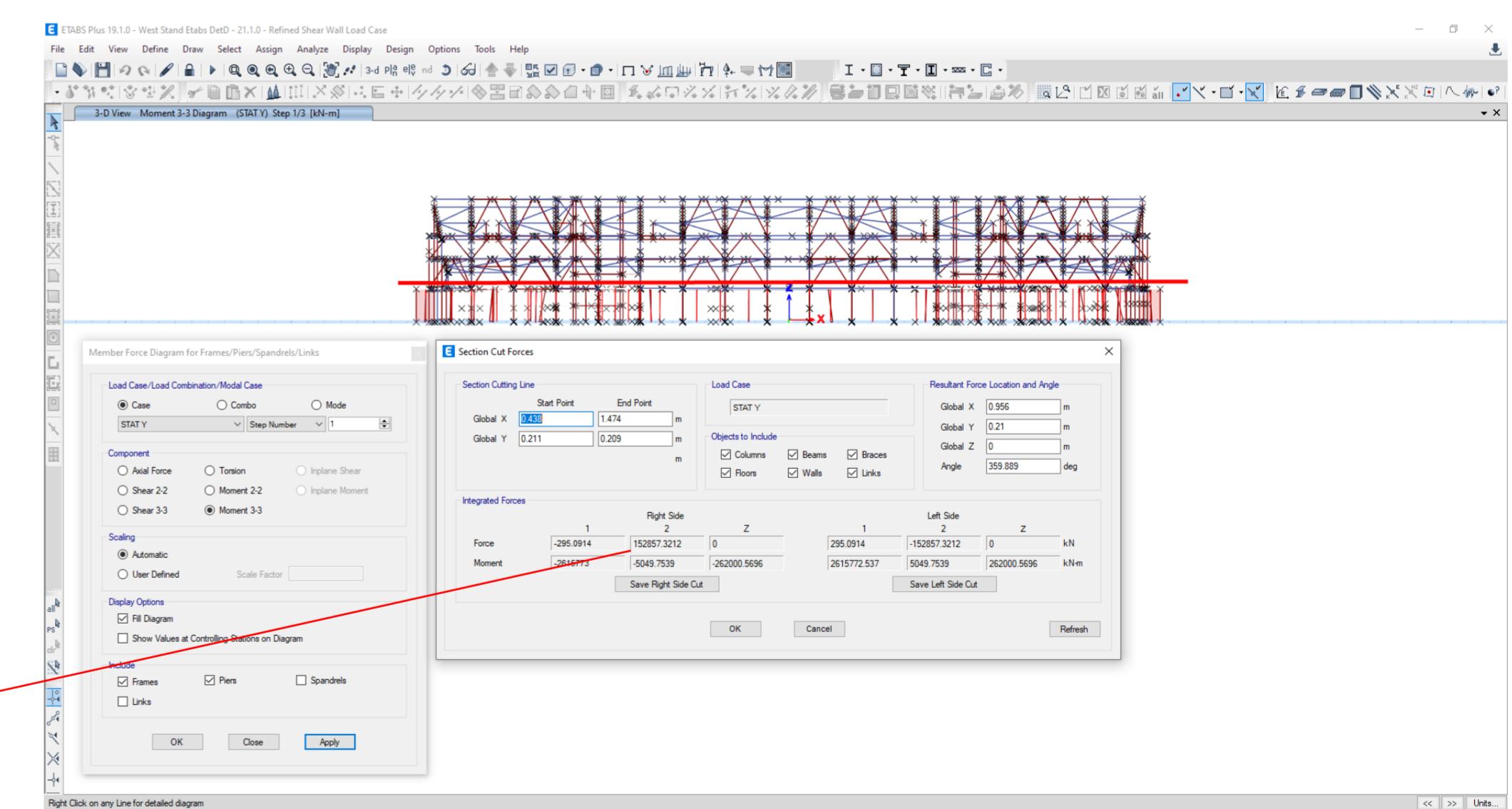


Inertial Force of Concourse =  $189,558\text{kN} - 152,857\text{kN} = 36,701\text{kN}$  (STATY)

## STATX, Section Cut, Above Concourse



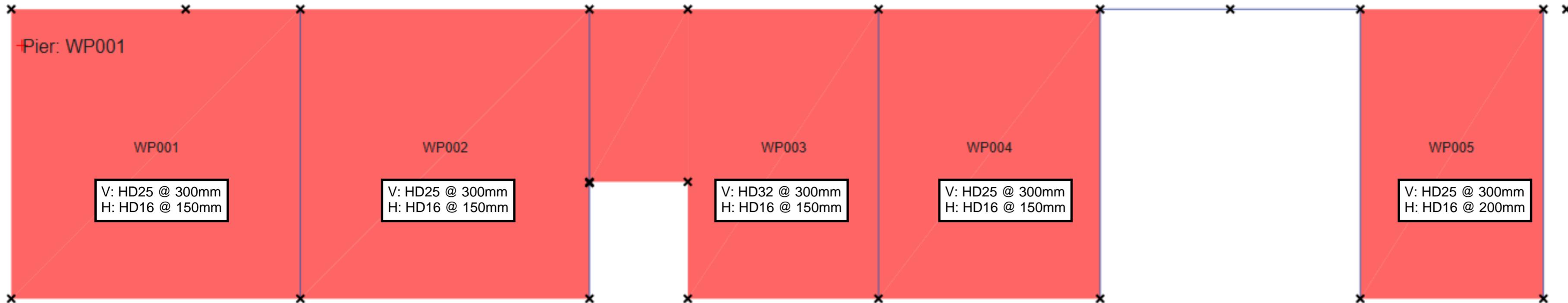
## STATY, Section Cut, Above Concourse



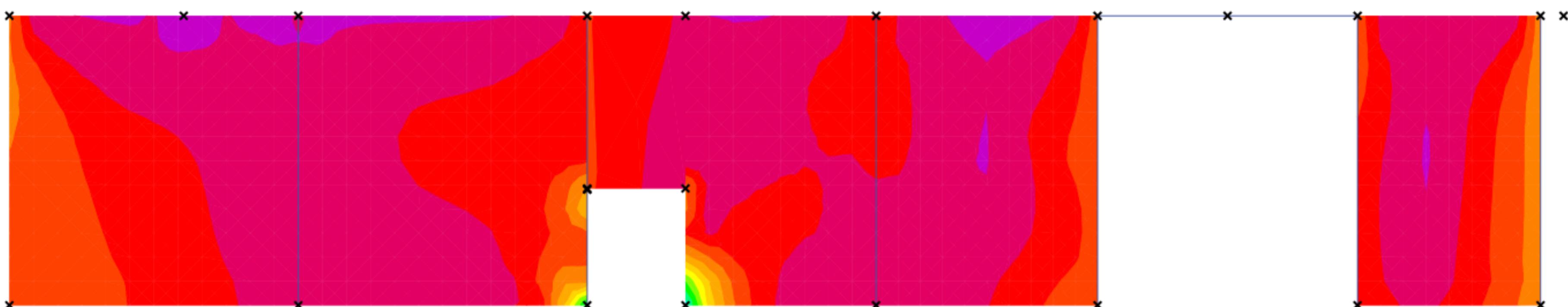
# Calculation of C, Steel Contribution Factor

V* (kN)	STAT X	STAT Y	User Seismic X	User Seismic Y	Steel Contribution Factor (X Direction)	Steel Contribution Factor (Y Direction)
WP001	-615.5445	7831.6912	-298.4041	689.6765		91%
WP002	975.959	10737.9126	-41.0979	889.1352		92%
WP003	441.6725	6210.9639	-17.778	480.1831		92%
WP004	1716.9513	5019.5845	97.6295	597.8356		88%
WP005	1168.3691	2163.5561	-15.4326	371.3075		83%
WP006	376.258	462.5531	62.1502	62.8992		86%
WP007	1018.7208	4683.9719	140.1109	520.4385		89%
WP008	-316.6604	10646.0882	-57.9002	937.6194		91%
WP009	2155.3759	8729.9508	63.7119	951.8954		89%
WP010	481.923	6772.5133	-66.5667	654.5277		90%
WP011	-1418.0316	2790.6637	-96.7872	1009.9435		64%
WP012	1079.095	6873.0474	118.9412	906.3042		87%
WP013	481.3209	8509.0906	-0.3005	1053.0651		88%
WP014	226.7793	6263.8372	-14.1256	736.8977		88%
WP015	306.1603	10669.4775	-45.7636	1358.0476		87%
WP016	990.9716	3373.4891	50.0356	1035.7186		69%
WP017	7.2273	468.5743	0.3974	72.541		85%
WP018	39.5124	3092.5932	2.3898	479.4009		84%
WP019	11.3045	1101.5362	0.9379	175.5275		84%
WP020	28.9512	4770.0284	2.1628	688.2302		86%
WP021	-75.181	3934.5448	-1.4942	480.7948		88%
WP022	-28.8743	208.5515	-1.1426	100.843		52%
WP023	-967.6842	2666.2076	-51.2492	393.4686		85%
WP024	42.5543	4870.849	10.534	791.7795		84%
WP025	13.6237	1171.8637	2.4804	191.5427		84%
WP026	151.0032	4922.5656	13.1215	727.7775		85%
WP027	537.7595	3862.2681	26.258	517.5108		87%
WP028	185.3272	271.1988	9.8653	121.8356		55%
WP029	1453.6789	1960.3107	58.5404	256.3197		87%
WP030	-911.0229	6795.7601	-136.5612	1078.0044		84%
WP031	-31.0978	6145.848	20.4792	959.6393		84%
WP032	199.3452	9002.8057	44.9688	1297.8974		86%
WP033	269.3977	6382.332	70.2615	1027.2292		84%
WP034	-386.3306	3468.2461	-31.8715	734.0678		79%
WP035	-1129.5871	7278.1762	-200.1658	1117.9318		85%
WP036	998.4562	5990.0834	82.7182	827.9005		86%
WP037	651.5012	1876.1776	24.5066	166.757		91%
WP038	-208.7338	2530.1905	4.6844	416.1008		84%
WP039	-148.9158	4156.394	22.3257	531.1486		87%
WP040	-16.1992	2087.8249	34.5891	302.6592		86%
WP041	277.8678	2729.1884	50.948	273.5381		90%
WP042	1819.2931	3186.9518	125.3749	1451.0994		54%
WP043	1267.7708	3640.2011	258.8515	559.1686		85%
WP044	528.2363	1792.8701	108.5214	264.2557		85%
WP045	92.5677	3743.7066	45.8281	538.6138		86%
WP046	-70.7721	4338.0048	4.4112	461.6903		89%
WP047	14.0382	1459.9811	6.3979	264.7658		82%
WP048	341.4927	6082.4142	38.5891	767.8872		87%
WP049	-1141.1112	5768.6779	-46.6941	1344.4487		77%
WP050	-836.4706	1890.7087	57.2331	943.8693		50%
WP051	619.4056	8934.7166	-34.573	1057.1152		88%
WP052	17.1723	9046.7513	58.1922	1454.8836		84%
WP101	6941.2897	-193.0734	1260.4998	-72.1617	82%	
WP102	5102.2169	1525.7148	894.8619	237.0274	82%	
WP103	4298.9325	1243.2766	752.414	190.3636	82%	
WP104	385.6503	252.4418	67.9158	41.2277	82%	
WP105	421.5147	-131.1139	76.3654	-22.4888	82%	
WP106	5136.6783	-290.5758	913.9608	-63.6106	82%	
WP107	1056.5661	-390.9241	183.4225	-66.2486	83%	
WP108	5392.3095	33.838	961.0781	-9.7178	82%	
WP109	6033.5608	2149.7424	1094.5386	323.0255	82%	
WP110	349.751	338.5989	66.5816	53.2312	81%	
WP111	1730.0563	514.266	479.0298	14.7768	72%	
WP112	6542.795	48.9505	1178.3312	59.843	82%	
WP113	5274.8388	1153.9033	1146.5157	56.6054	78%	
WP114	738.2749	0	151.2848	6.6988	80%	
WP115	4266.9217	84.827	825.5768	38.1318	81%	
WP116	3602.2533	4174.5152	578.0104	27.5586	84%	
WP117	3090.7466	1314.9235	478.8781	30.8794	85%	
WP118	5226.6473	-673.1583	1217.2951	43.7102	77%	
WP119	4010.5639	135.2217	701.0177	26.358	83%	
WP120	12018.6943	2593.4134	1652.9073	101.3208	86%	
WP121	2942.7601	155.7503	823.1441	-41.4044	72%	
WP122	2705.355	127.9775	181.7824	-3.6587	93%	
WP123	4384.4871	839.3885	619.3309	-36.942	86%	
WP124	3227.6803	-115.8324	469.6674	60.5168	85%	
WP125	2011.2438	265.5889	798.0373	-1.6033	60%	
WP126	1483.8182	20.482	245.0227	-2.3956	83%	
WP127	4610.3251	-124.8478	1070.6807	-80.9524	77%	
WP128	6114.8458	430.1168	1535.3566	66.2509	75%	
WP129	4494.6562	106.5385	1343.0351	1.9656	68%	
WP130	3204.9274	557.1318	465.5743	-33.464	85%	
WP131	4481.8567	-262.424	635.5365	101.6013	86%	
WP132	2600.7749	-14.8292	171.9593	6.9053	93%	
WP133	4442.2419	255.2504	1191.0045	56.4717	73%	
WP134	10863.9969	-1218.3706	1444.582	-156.9364	87%	
WP135	8233.2719	-28.2347	1413.9076	-35.2748	83%	
WP136	8625.5525	638.8542	1473.8026	-30.2425	83%	
WP137	676.9552	371.2689	106.6262	26.5592	84%	
WP138	945.2178	374.0445	152.2918	24.0412	84%	
WP139	1465.3051	510.0009	238.3553	30.8126	84%	
WP140	2129.7579	532.0246	361.2041	28.2687	83%	
WP141	963.9618	149.3296	168.8823	6.2235	82%	
WP142	5676.7588	563.4948	870.4455	-40.6267	85%	
WP143	4194.4512	459.8688	713.7048	-41.926	83%	
WP144	4734.8392	257.3477	804.7634	-45.7174	83%	
WP145	5708.5934	349.4133	876.4657	-66.8816	85%	
WP146	995.6082	29.1813	173.9225	-32.6306		

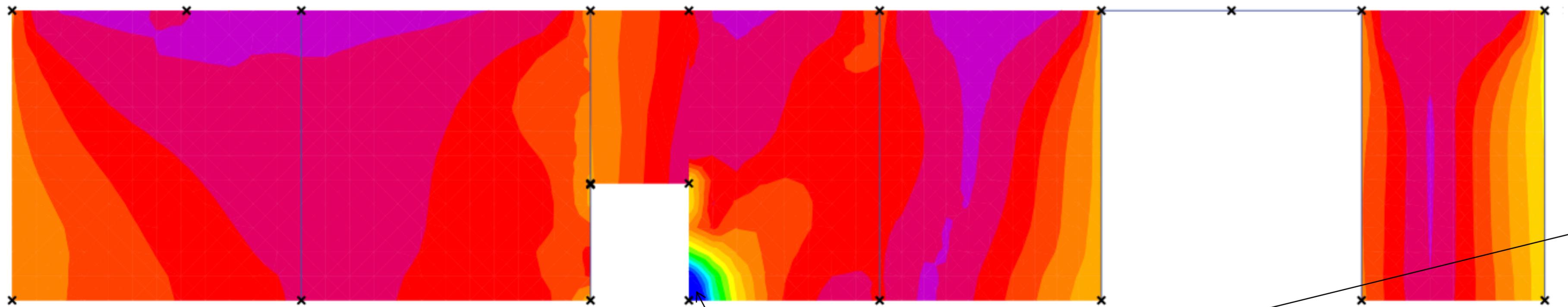
A08



1.0G + 0.30Q + 1.0SpecX + 0.30SpecY [mu = 1.25, sp = 0.90]



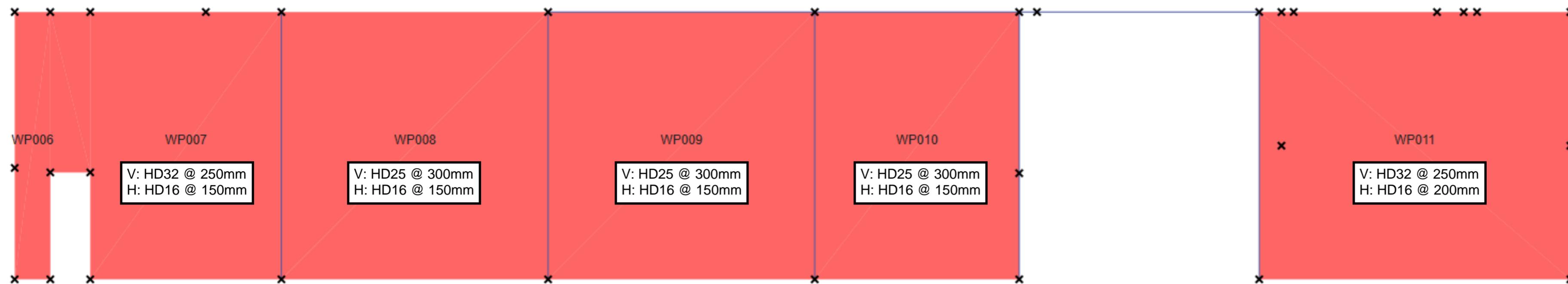
1.0G + 0.30Q + 0.30SpecX + 1.0SpecY [mu = 1.25, sp = 0.90]

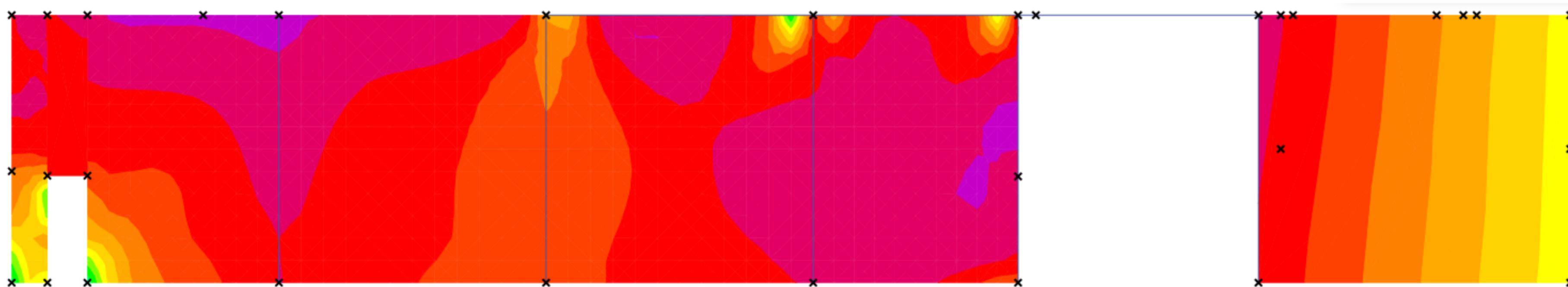


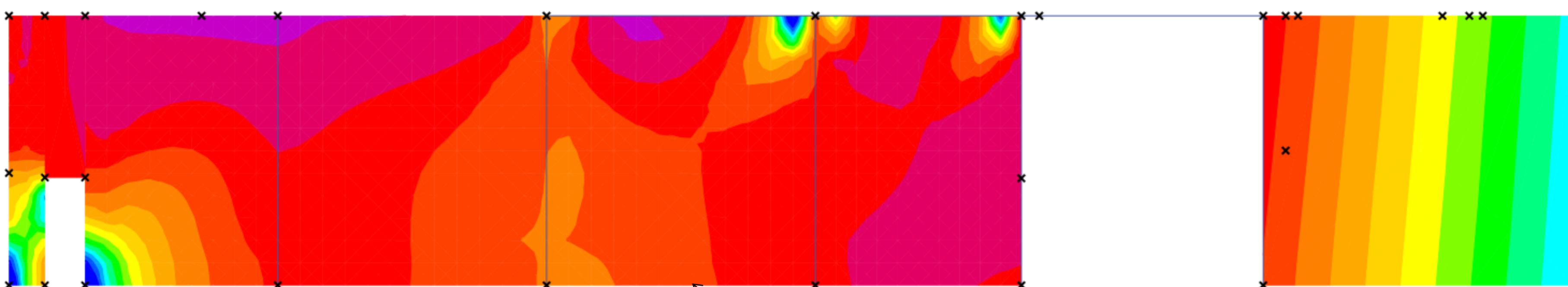
Can justify to HD32 - 300, when  
considering average stress over  
1000mm.



A06



$$1.0G + 0.30Q + 1.0\text{SpecX} + 0.30\text{SpecY} \quad [\mu = 1.25, sp = 0.90]$$


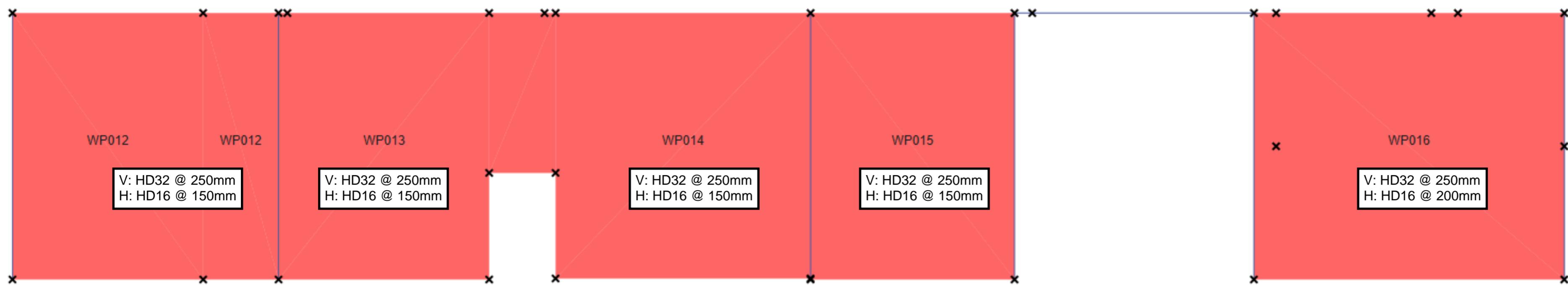
$$1.0G + 0.30Q + 0.30\text{SpecX} + 1.0\text{SpecY} \quad [\mu = 1.25, sp = 0.90]$$


Can justify to HD25 - 300, when  
considering average stress over  
1000mm.

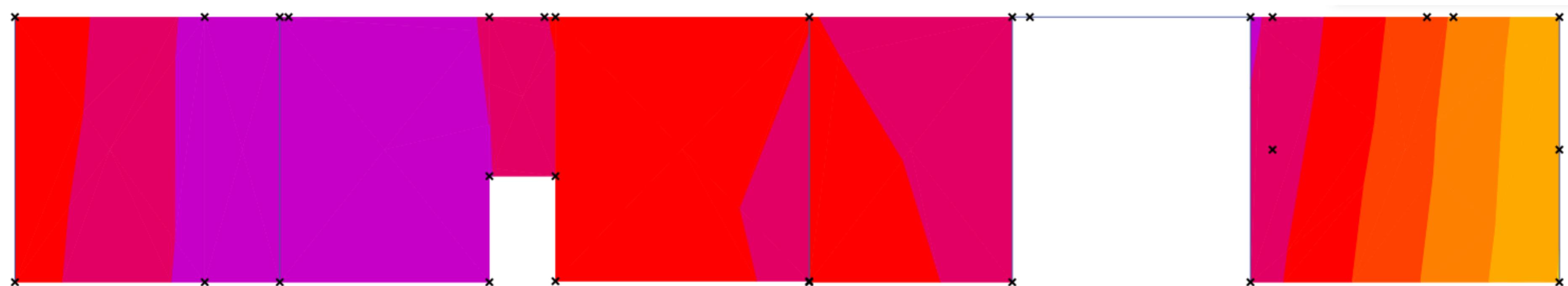
WP006	0.32
WP007	0.77
WP008	0.51
WP009	1.23
WP010	0.94
WP011	0.82



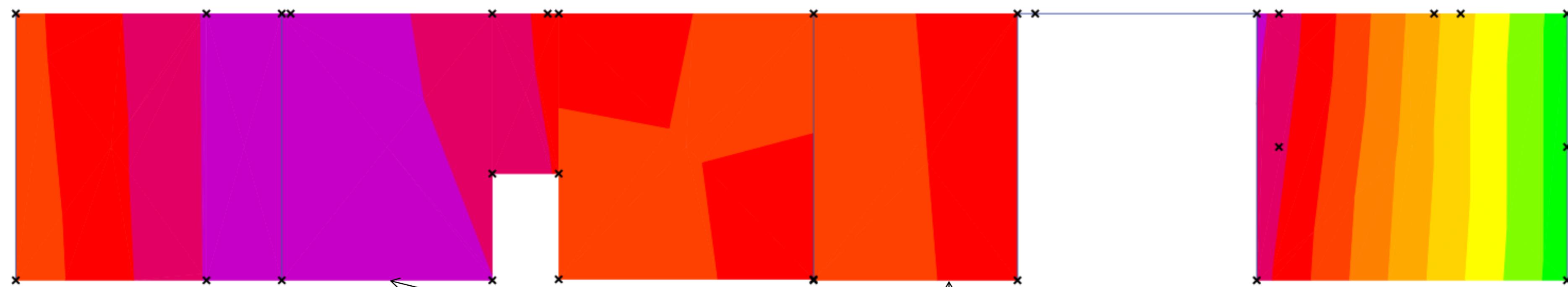
A04



1.0G + 0.30Q + 1.0SpecX + 0.30SpecY [mu = 1.25, sp = 0.90]



1.0G + 0.30Q + 0.30SpecX + 1.0SpecY [mu = 1.25, sp = 0.90]

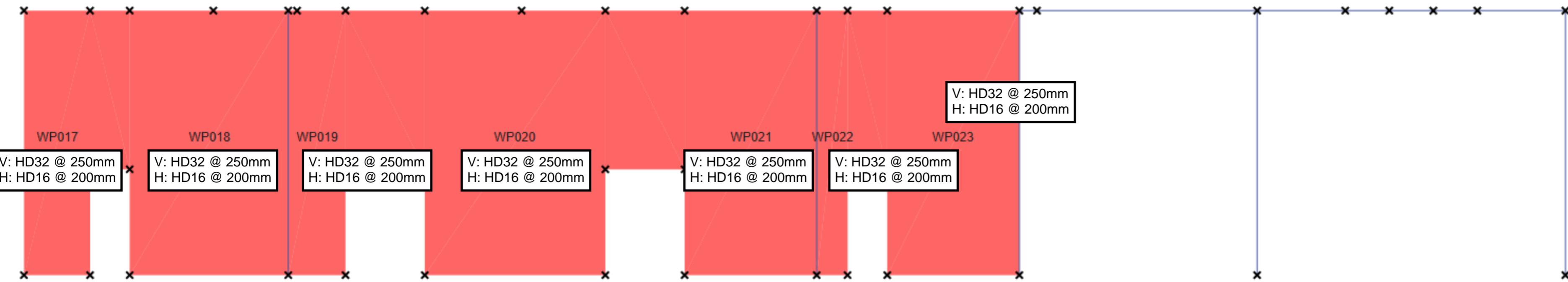
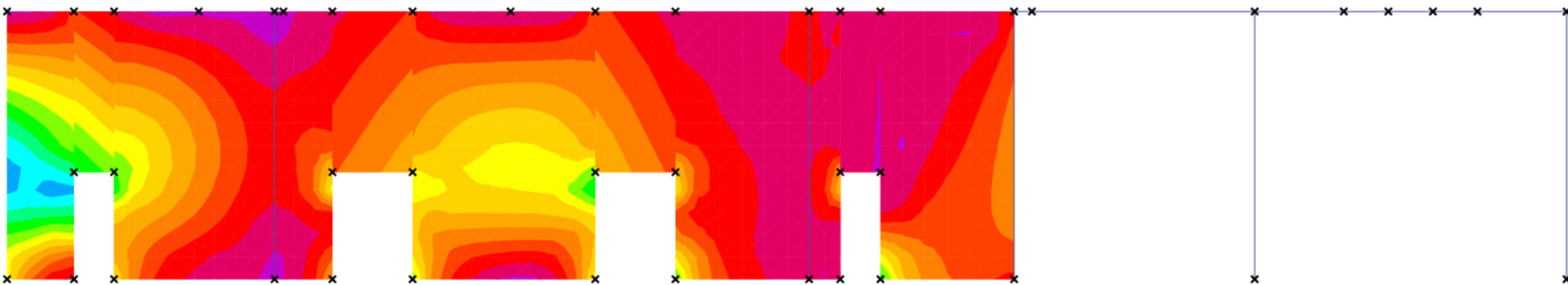
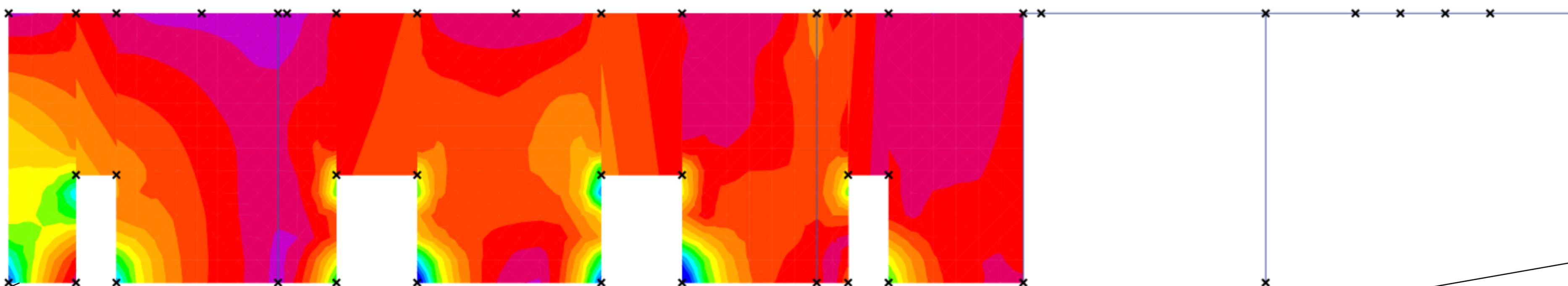


Can justify to HD32 - 250, when considering average stress over 1000mm.

WP012	0.98
WP013	1.35
WP014	0.84
WP015	1.72
WP016	0.76



A01

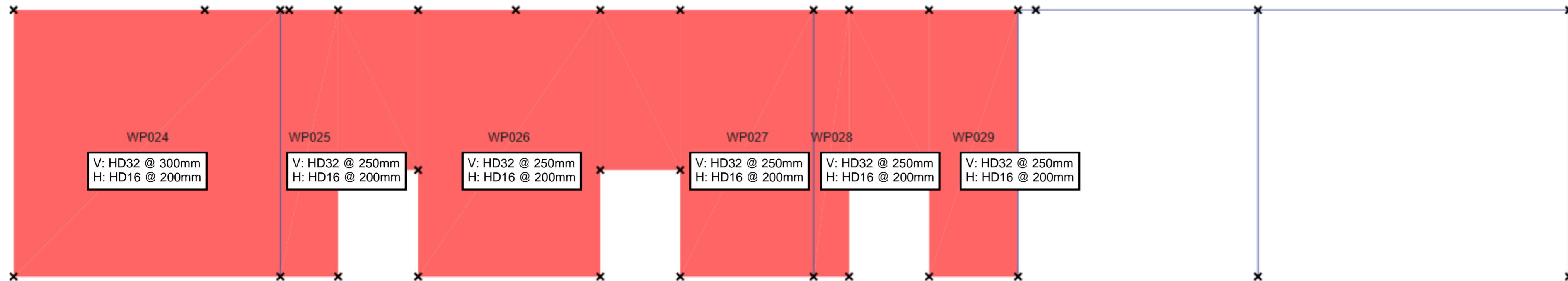

 $1.0G + 0.30Q + 1.0\text{SpecX} + 0.30\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]

 $1.0G + 0.30Q + 0.30\text{SpecX} + 1.0\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]


Can justify to HD32 - 250, when considering average stress over 1000mm.

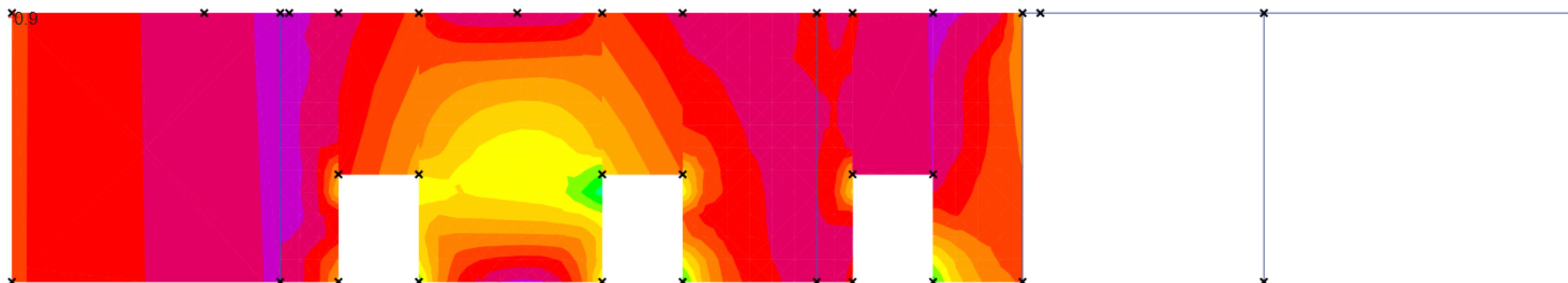
WP017	1.02
WP018	0.47
WP019	0.96
WP020	0.63
WP021	0.80
WP022	0.15
WP023	0.40



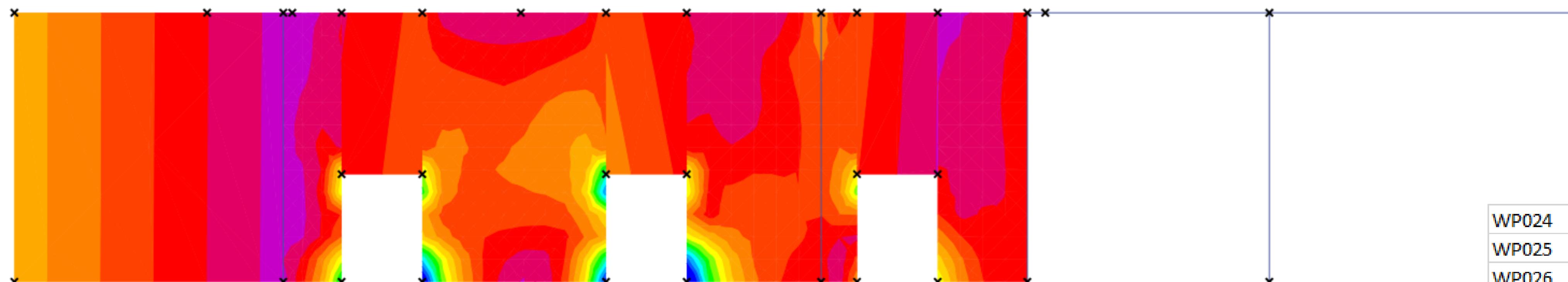
B01



**1.0G + 0.30Q + 1.0SpecX + 0.30SpecY [mu = 1.25, sp = 0.90]**



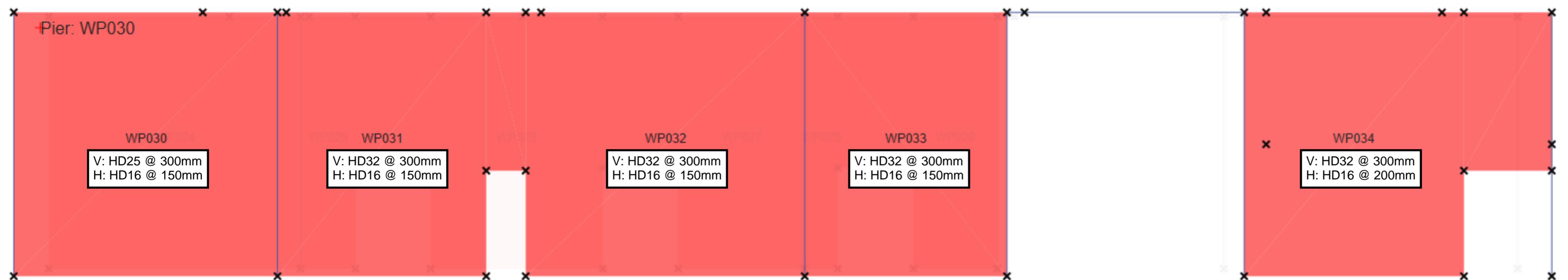
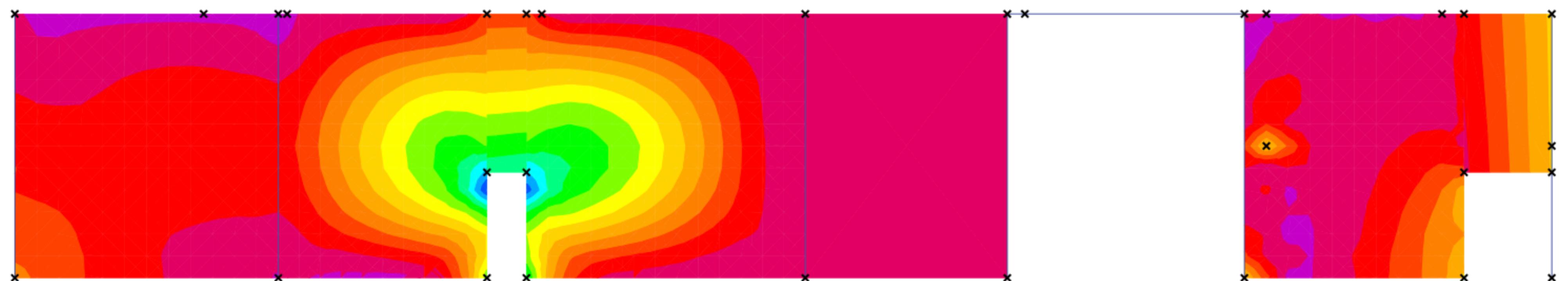
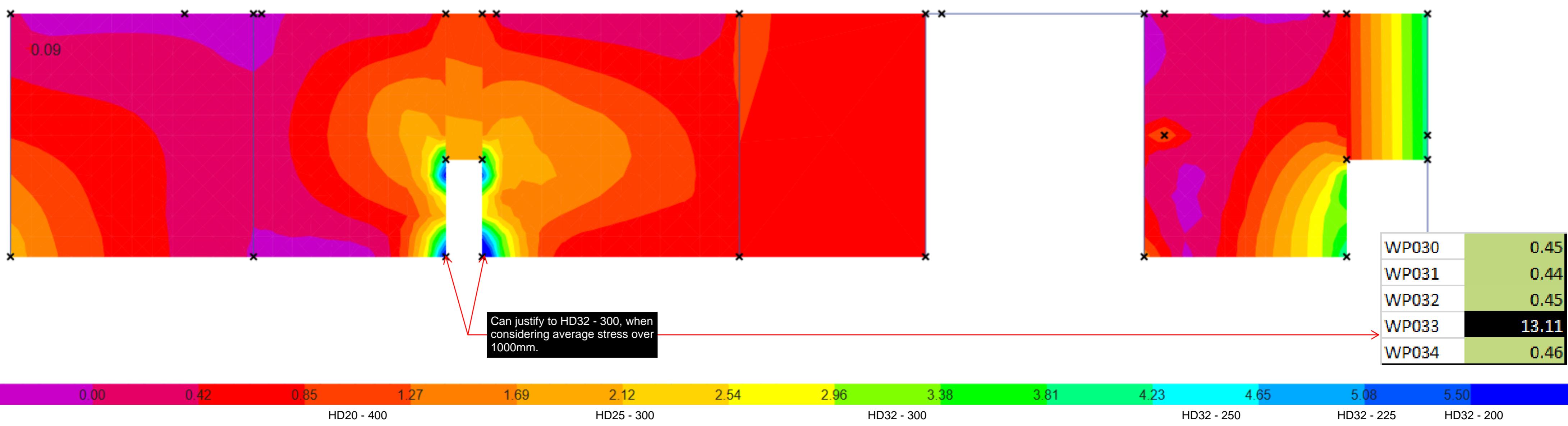
1.0G + 0.30Q + 0.30SpecX + 1.0SpecY [mu = 1.25, sp = 0.90]



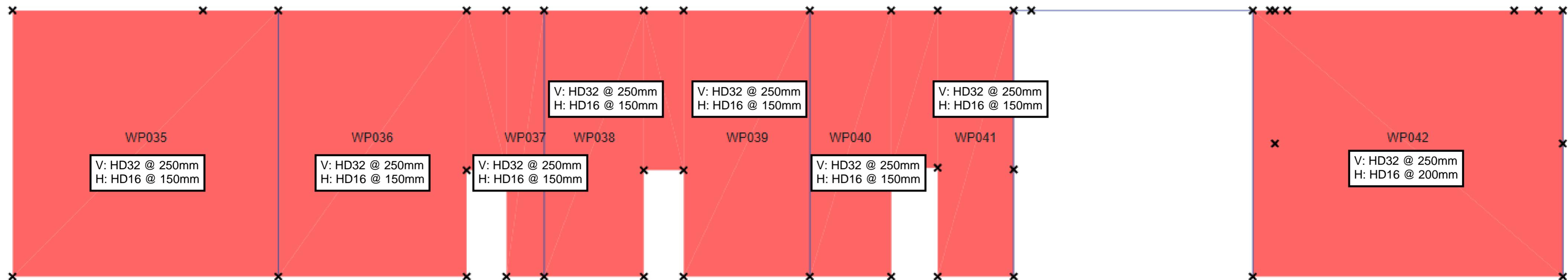
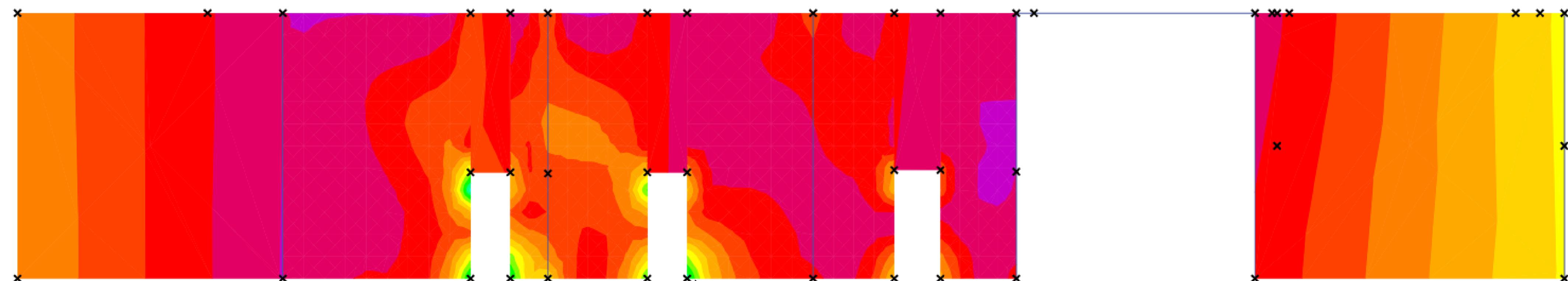
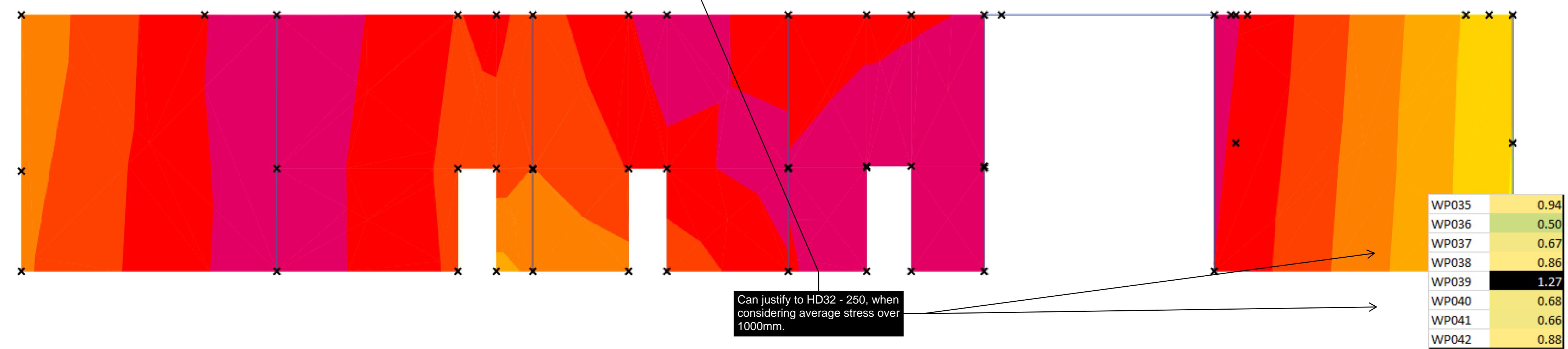
WP024	0.73
WP025	0.99
WP026	0.62
WP027	0.84
WP028	0.15
WP029	0.60



B04


 $1.0G + 0.30Q + 1.0\text{SpecX} + 0.30\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]

 $1.0G + 0.30Q + 0.30\text{SpecX} + 1.0\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]


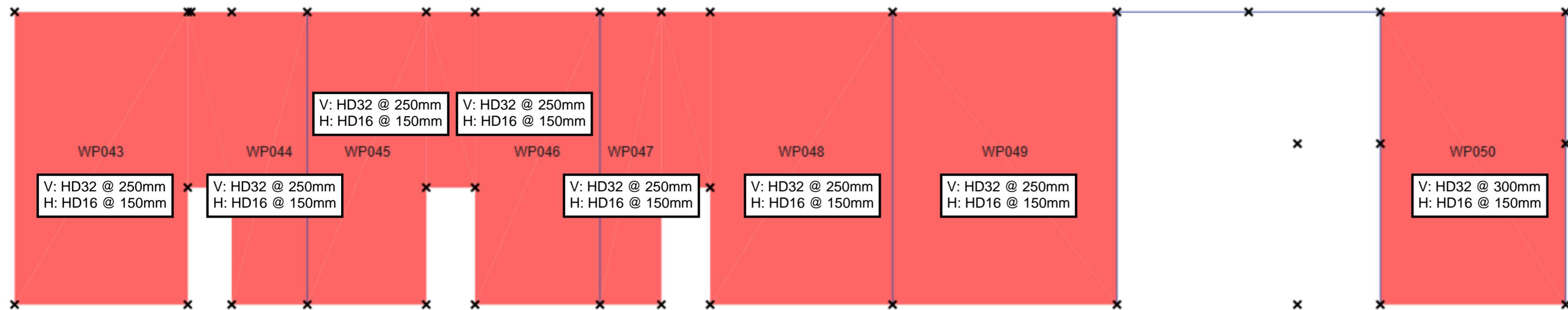
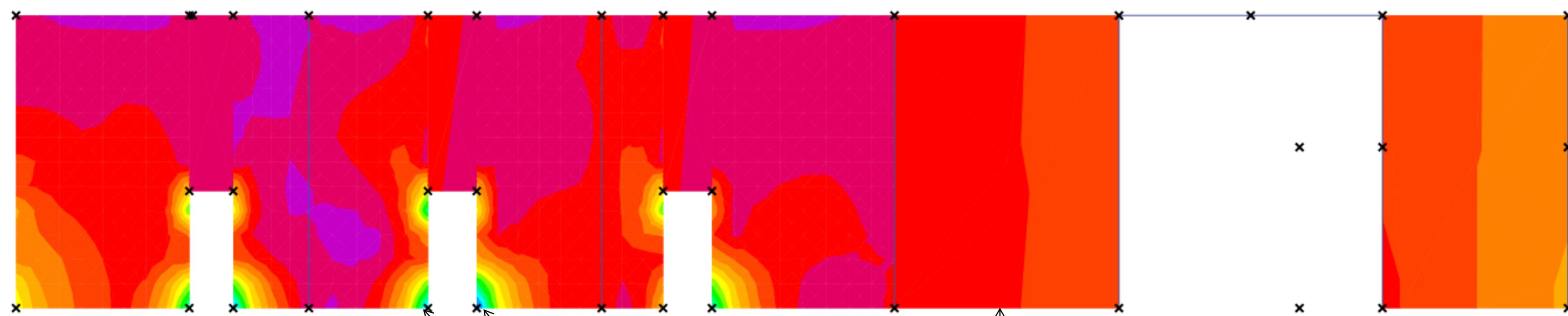
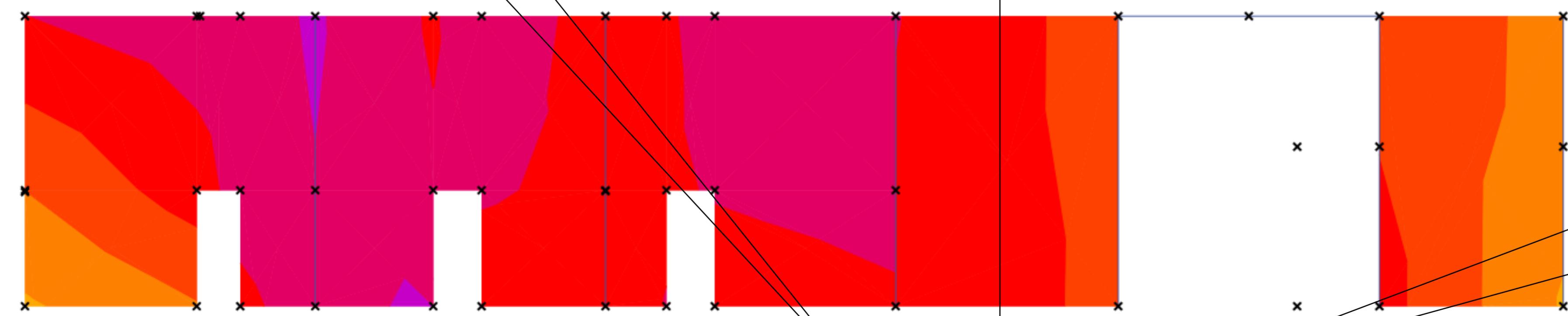
B06


 $1.0G + 0.30Q + 1.0\text{SpecX} + 0.30\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]

 $1.0G + 0.30Q + 0.30\text{SpecX} + 1.0\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]


0.00 0.42 0.85 1.27 1.69 2.12 2.54 2.96 3.38 3.81 4.23 4.65 5.08 5.50

HD20 - 400 HD25 - 300 HD32 - 300 HD32 - 250 HD32 - 225 HD32 - 200

B08

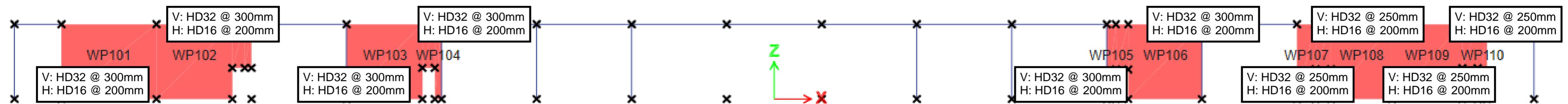

 $1.0G + 0.30Q + 1.0\text{SpecX} + 0.30\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]

 $1.0G + 0.30Q + 0.30\text{SpecX} + 1.0\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]


Can justify to HD32 - 250, when considering average stress over 1000mm.

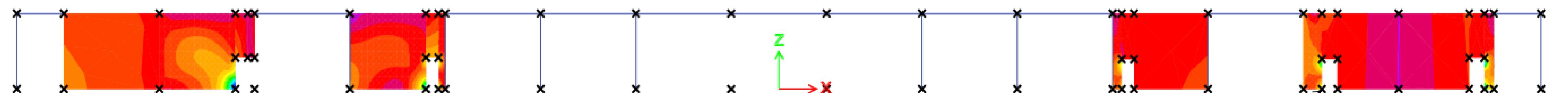
WP043	0.79
WP044	0.31
WP045	1.32
WP046	2.97
WP047	0.72
WP048	0.69
WP049	1.38
WP050	0.76



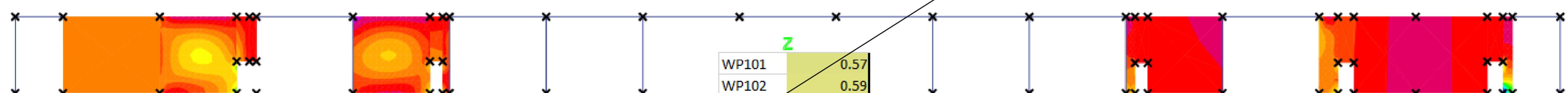
$Y = 51.95$



$1.0G + 0.30Q + 1.0\text{Spec}X + 0.30\text{Spec}Y$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]



$1.0G + 0.30Q + 0.30\text{Spec}X + 1.0\text{Spec}Y$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]

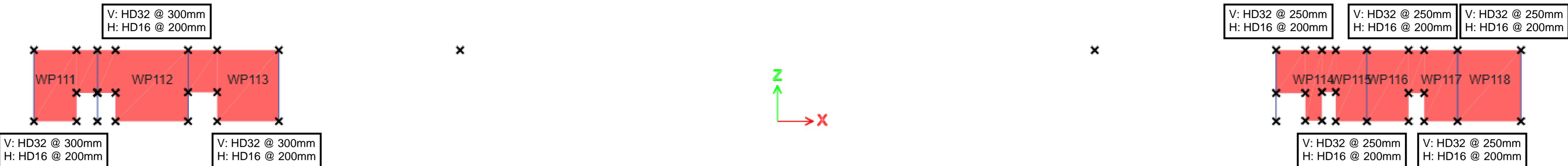


WP101	0.57
WP102	0.59
WP103	0.38
WP104	0.37
WP105	0.30
WP106	0.80
WP107	1.06
WP108	0.98
WP109	0.92
WP110	0.32

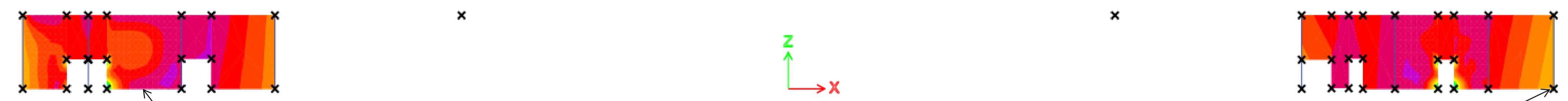
Can justify to HD32 - 300, when considering average stress over 1000mm.



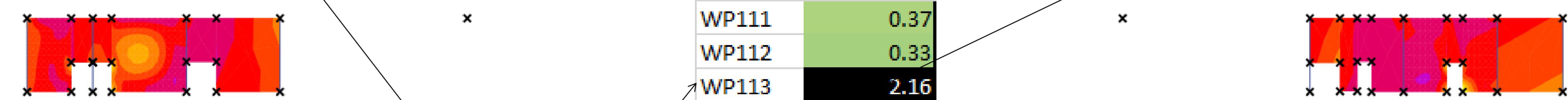
W-C1



1.0G + 0.30Q + 1.0SpecX + 0.30SpecY [mu = 1.25, sp = 0.90]



1.0G + 0.30Q + 0.30SpecX + 1.0SpecY [mu = 1.25, sp = 0.90]

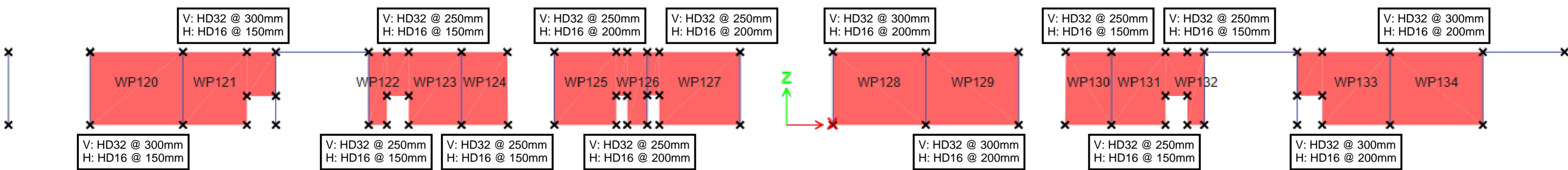


WP111	0.37
WP112	0.33
WP113	2.16
WP114	1.16
WP115	2.86
WP116	0.34
WP117	0.48
WP118	1.52

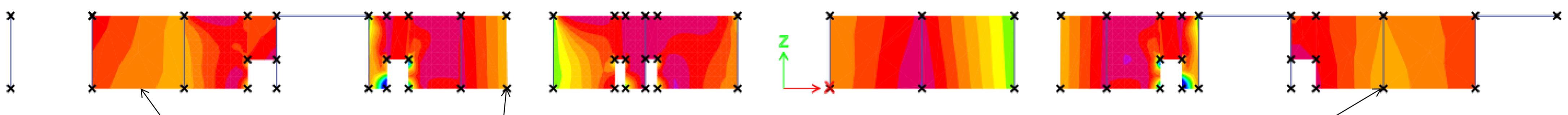
Can justify to HD32 - 300, when considering average stress over 1000mm.



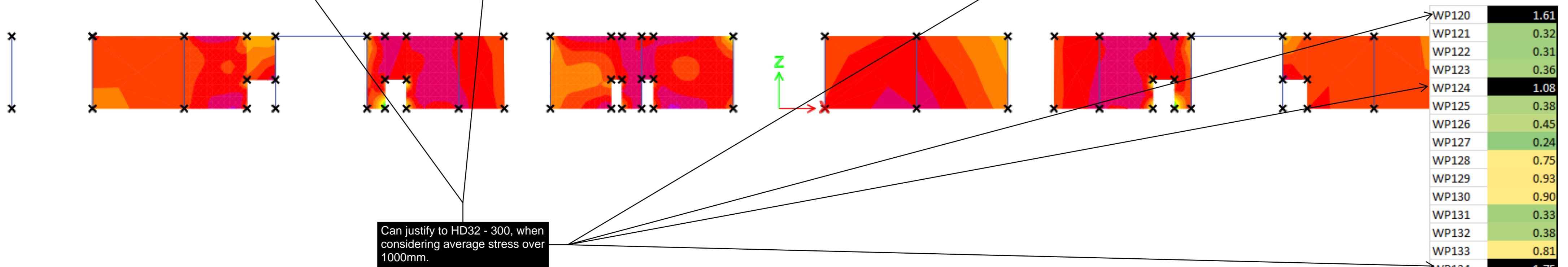
W-E



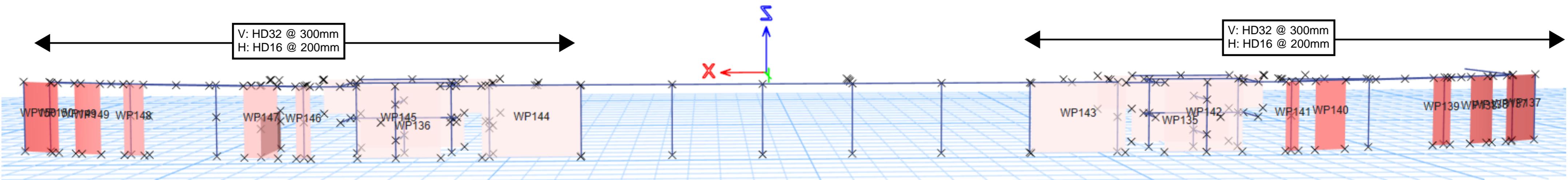
$1.0G + 0.30Q + 1.0\text{SpecX} + 0.30\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]



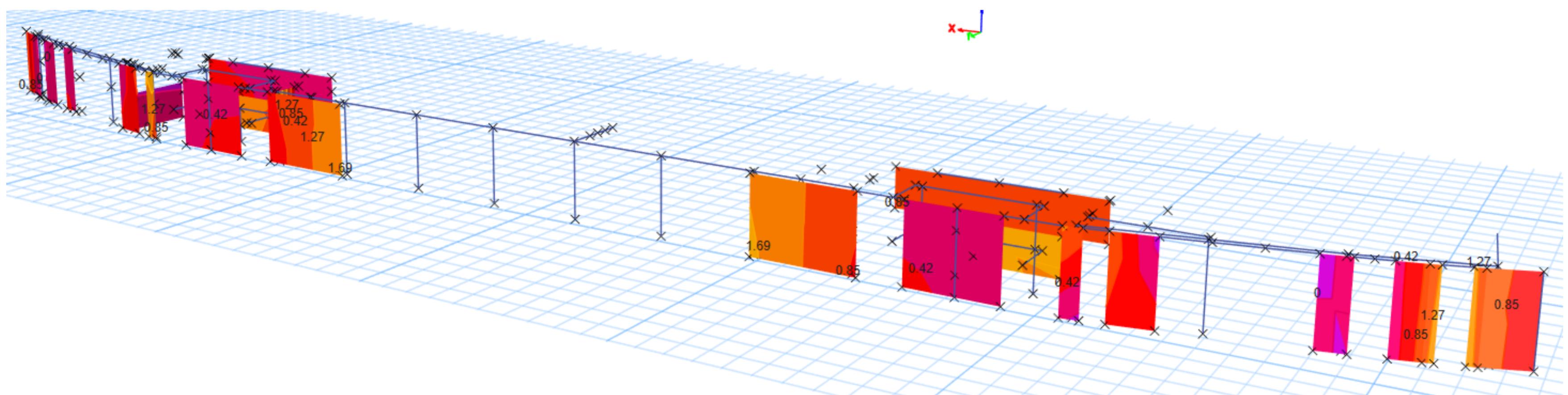
$1.0G + 0.30Q + 0.30\text{SpecX} + 1.0\text{SpecY}$  [ $\mu = 1.25$ ,  $sp = 0.90$ ]



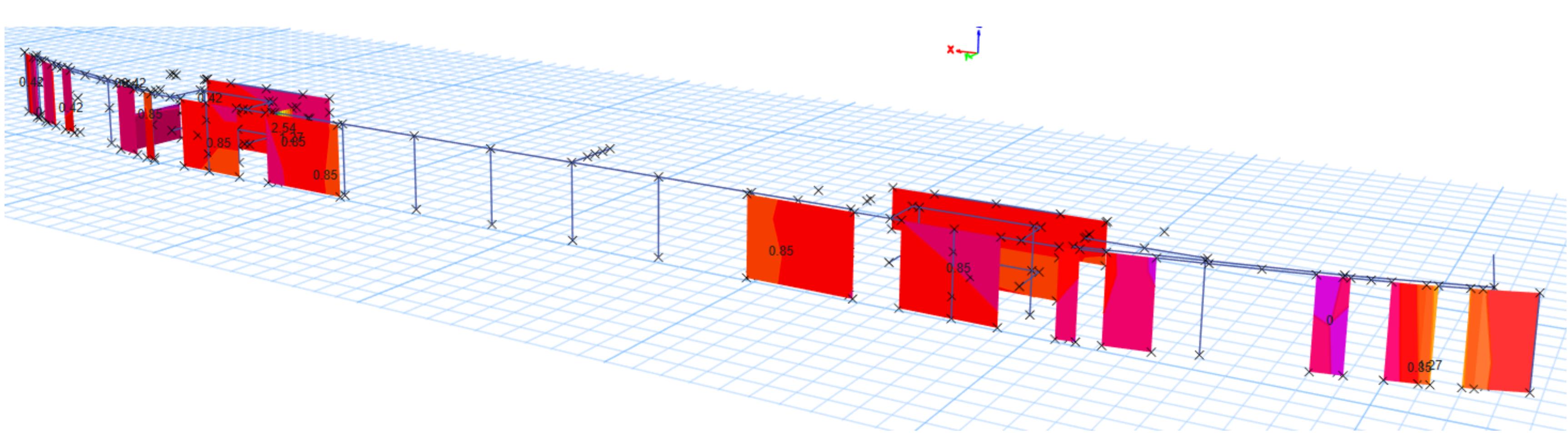
## Back of Bowl



1.0G + 0.30Q + 1.0SpecX + 0.30SpecY [mu = 1.25, sp = 0.90]



1.0G + 0.30Q + 1.0SpecX + 1.0SpecY [mu = 1.25, sp = 0.90]

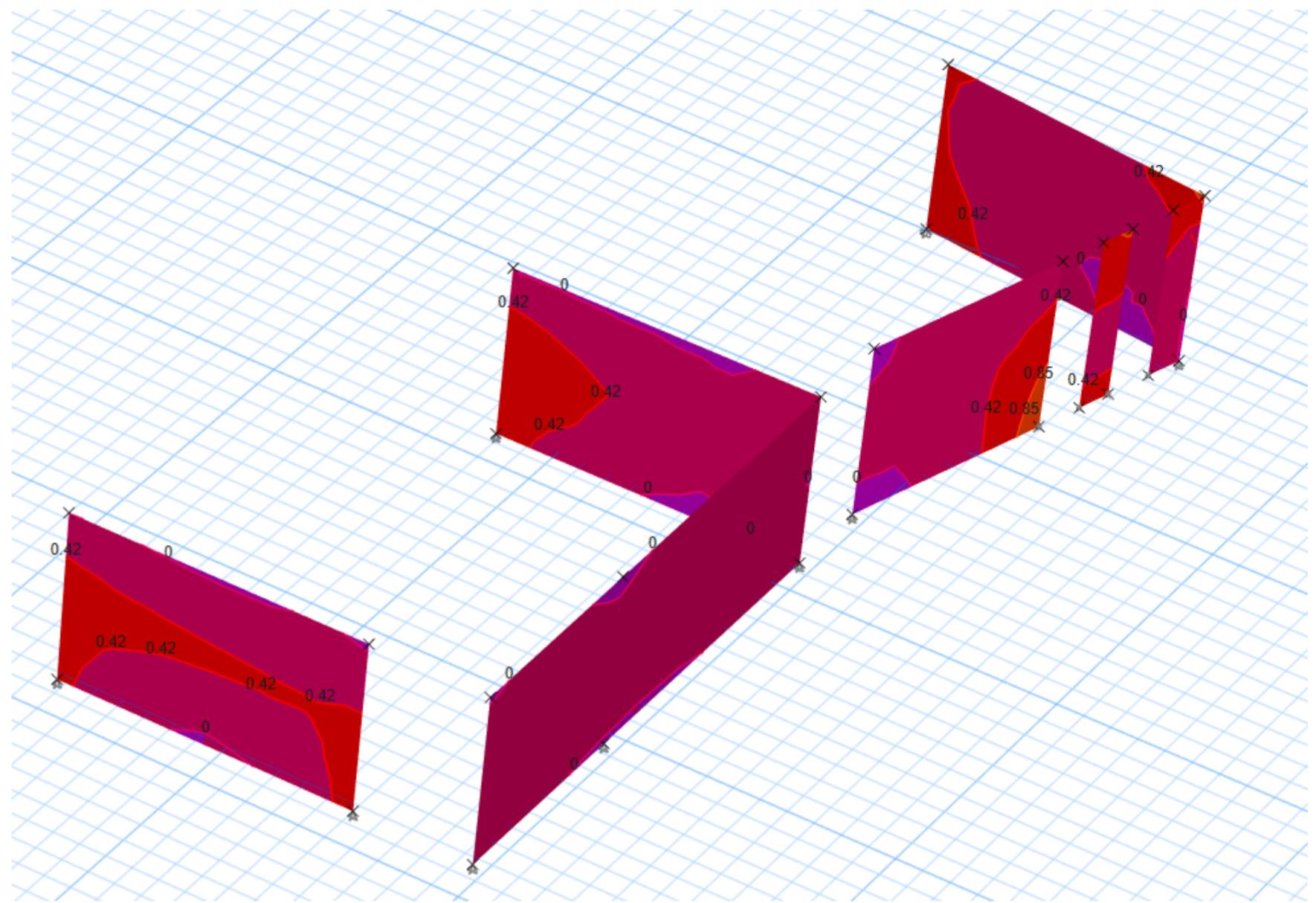
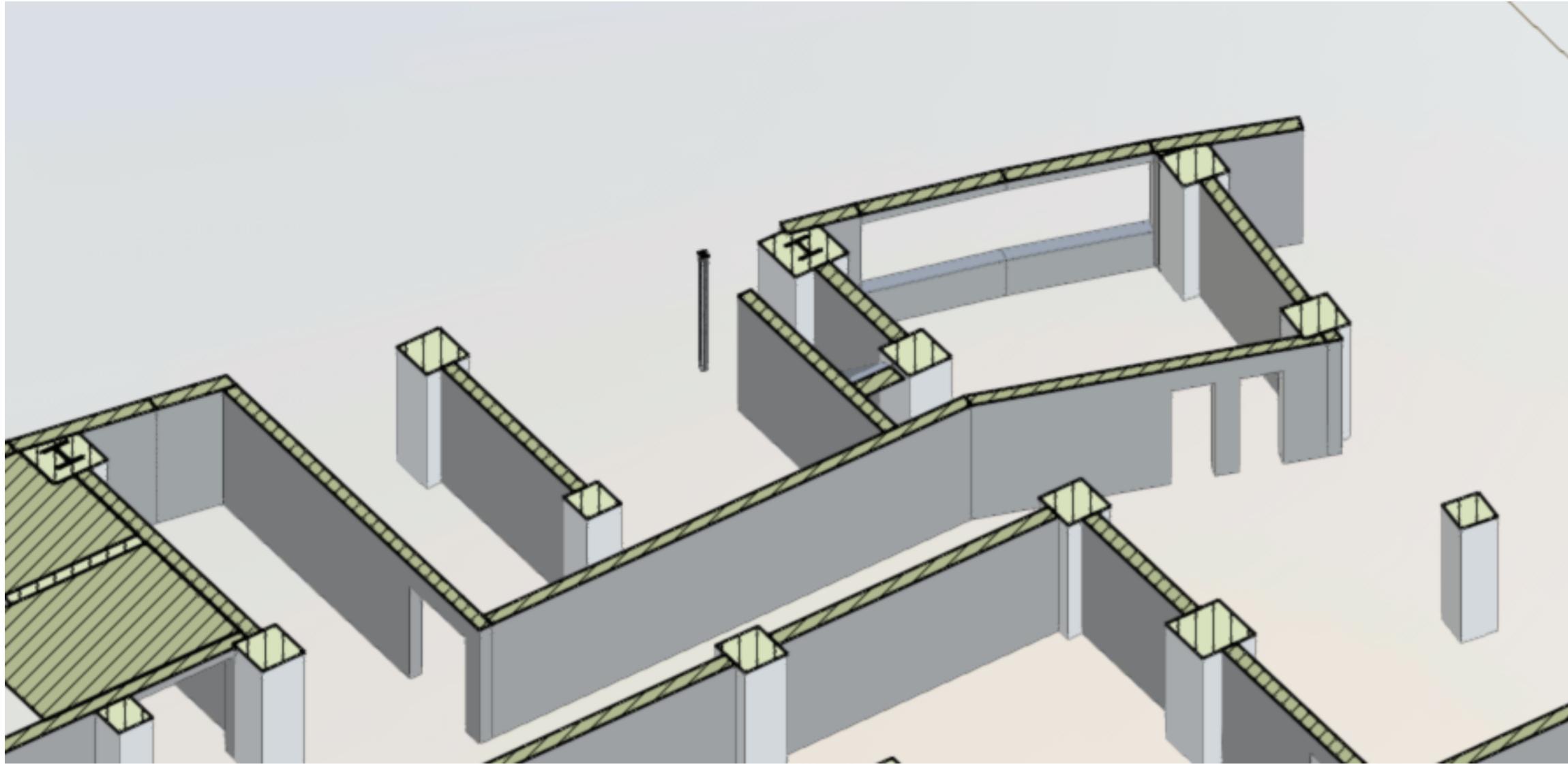


Stress Plots OKAY

	0.54
WP137	1.54
WP138	3.58
WP139	1.42
WP140	6.05
WP141	0.67
WP142	0.55
WP143	0.57
WP144	0.67
WP145	5.39
WP146	1.44
WP147	4.88
WP148	2.52
WP149	1.20
WP150	



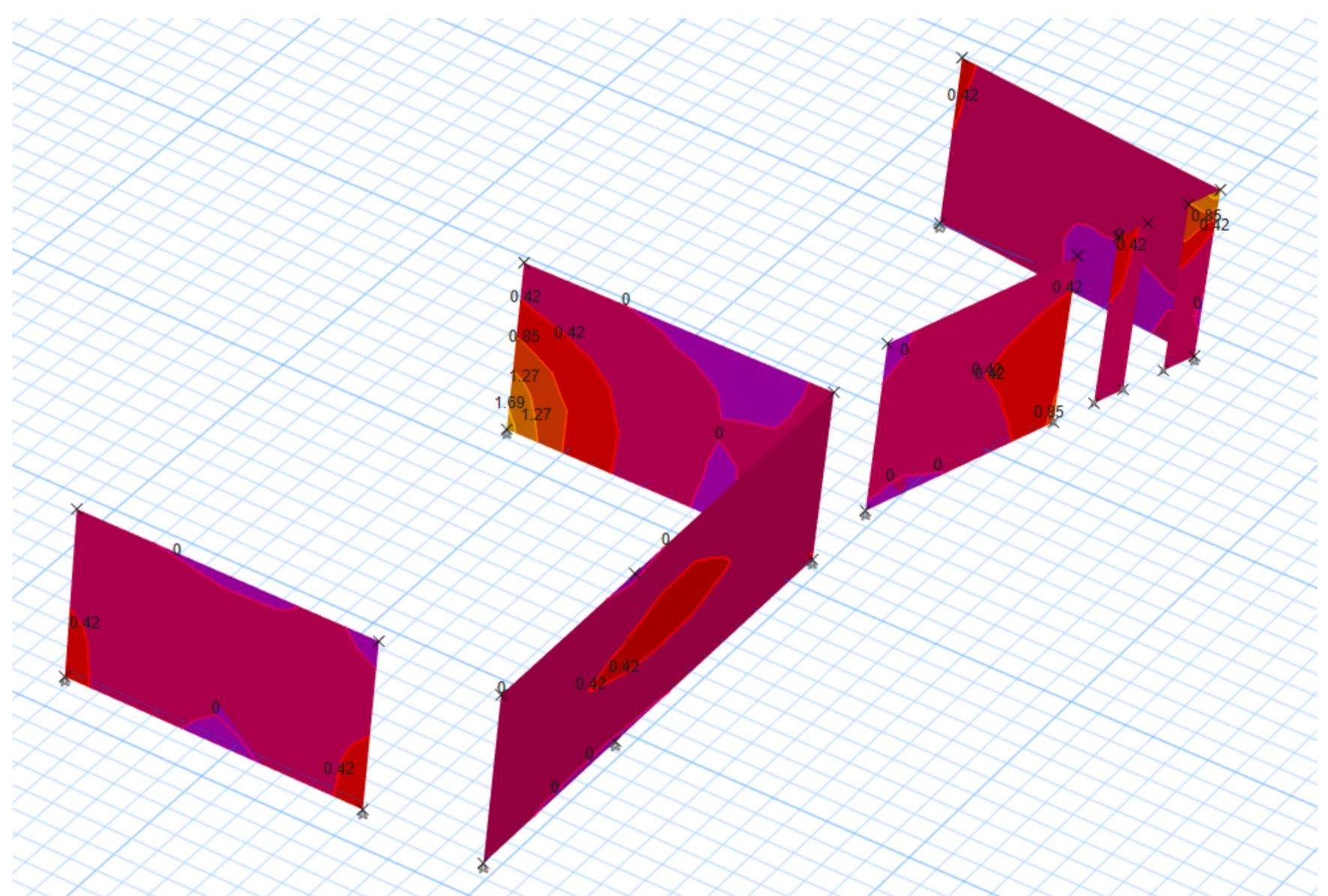
## Mezzanine Level in Top Left Corner of West Stand



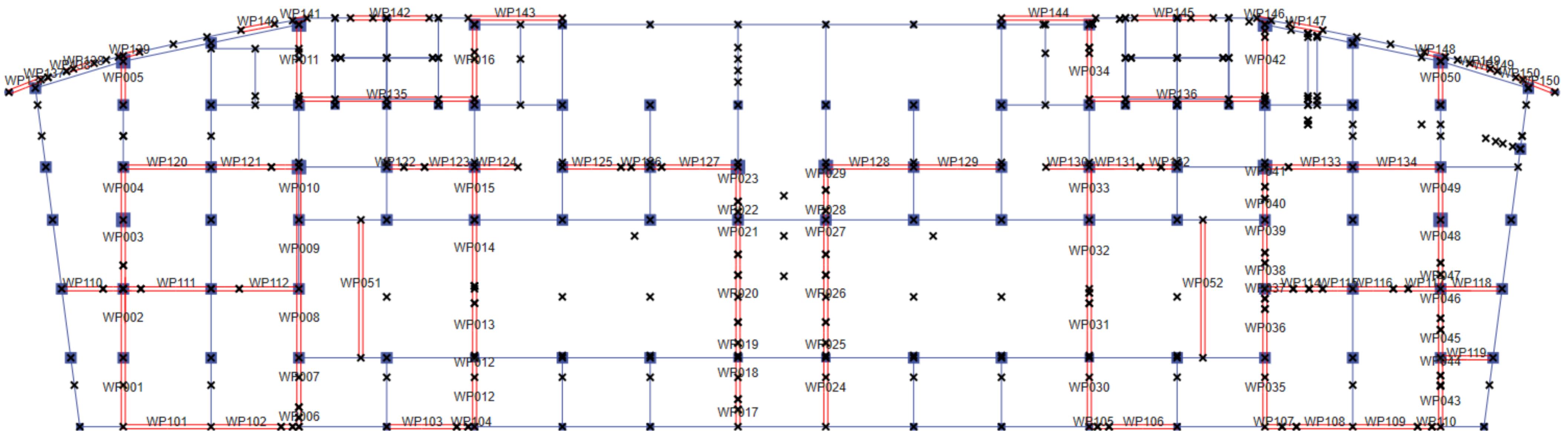
By inspection of the stress map apply:

V: HD25 - 300

H: HD16 - 200



## Pier Labels - Analytical Model



# Analytical NZS 3101:2006 Model Checks

	Pier	Height	Depth	Width	End Zone Length	Material	Cover	fc	Ec	fsy	Verti.Dia.	Verti. Spacing	Boundary Reo	Panel Reo	Horiz. Dia.	Horiz. Spacing	p	M3	V2	Maximum longitudinal bar diameter	Starter Dia Check	Maximum spacing of vertical reinforcement	Vertical reinforcement ratio	Minimum vertical reinforcement ratio	Neutral axis depth	M Uti of Starter Reo	v_n	Shear Stress Check	v_c	Shear Bar Spacing	Shear Bar Spacing (Entire Wall)	Max spacing of horizontal reinforcement	Horizontal Spacing Check for Shear	A_v	A_vmin	Shear friction uti of dowels	Area of longitudinal bars restrained by one horizontal stirrup	Vertical spacing of horizontal stirrups	Max vertical spacing of horizontal stirrups	Stirrup Spacing check	A_te	Selected diameter of stirrup tie for antibuckling	Stirrup diameter for confinement	Stirrup legs for confinement	No of stirrup dia check	Stirrup spacing for confinement zones			
0	WP001	6	6000	350	900	NZS3101-40MPa	35	40	29725	500	25	300	0.003	0.003	16	150	1938	2993	5981	50	1	450	1	0.005	0.003	1	271	0.47	4.75	1	2481.21	176	165	480	1	2289	490	2.98	491	150	250	1	30.68	10	1	-24.38	10	2	200
1	WP002	6	6000	350	900	NZS3101-40MPa	35	40	29725	500	25	300	0.003	0.003	16	150	920	3594	8258	50	1	450	1	0.005	0.003	1	367	0.41	6.55	1	2684.90	116	165	450	1	3469	490	3.04	491	150	250	1	30.68	10	1	-8.63	10	2	200
2	WP003	6	3958	350	594	NZS3101-40MPa	35	40	29725	500	32	300	0.003	0.003	16	150	2429	3752	4817	50	1	450	1	0.008	0.003	1	218	1.12	5.80	1	1406.58	127	165	450	1	3168	490	2.55	804	150	300	1	39.27	10	1	-14.56	10	2	200
3	WP004	6	4600	350	690	NZS3101-40MPa	35	40	29725	500	25	300	0.003	0.003	16	150	843	2348	3761	50	1	450	1	0.005	0.003	1	257	0.53	3.89	1	2030.90	248	165	450	1	1622	490	1.91	491	150	250	1	30.68	10	1	13.78	10	2	200
4	WP005	6	3800	350	570	NZS3101-40MPa	35	40	29725	500	25	300	0.003	0.003	16	200	710	1685	1633	50	1	450	1	0.005	0.003	1	218	0.51	2.05	1	1674.95	821	820	450	1	330	490	1.02	491	150	250	1	40.91	10	1	-12.31	10	2	200
5	WP006	6	800	350	800	NZS3101-40MPa	35	40	29725	500	40	100	0.003	0.003	16	150	967	192	310	50	1	450	1	0.036	0.003	1	182	0.32	1.85	1	23.26	329	175	160	1	1220	490	0.14	1257	150	300	1	49.09	10	1	153.92	10	2	200
6	WP007	6	4300	350	645	NZS3101-40MPa	35	40	29725	500	32	250	0.003	0.003	16	150	3357	3750	3159	50	1	450	1	0.009	0.003	1	279	0.78	3.50	1	1384.55	245	175	450	1	1644	490	1.35	804	150	300	1	39.27	10	1	-4.95	10	2	200
7	WP008	6	6000	350	900	NZS3101-40MPa	35	40	29725	500	25	300	0.003	0.003	16	150	1804	3391	7695	50	1	450	1	0.005	0.003	1	285	0.51	6.11	1	2507.96	124	175	450	1	3230	490	3.66	491	150	250	1	30.68	10	1	-22.09	10	2	200
8	WP009	6	6000	350	900	NZS3101-40MPa	35	40	29725	500	25	300	0.003	0.003	16	150	2251	6874	6148	50	1	450	1	0.005	0.003	1	237	1.23	4.88	1	2481.61	167	175	450	1	2408	490	3.44	491	150	250	1	30.68	10	1	-29.96	10	2	200
9	WP010	6	4600	350	690	NZS3101-40MPa	35	40	29725	500	25	300	0.003	0.003	16	150	460	4812	4821	50	1	450	1	0.005	0.003	1	295	0.94	4.99	1	2107.41	171	175	450	1	2349	490	2.15	491	150	250	1	30.68	10	1	-5.72	10	2	200
10	WP011	6	7000	350	1050	NZS3101-40MPa	35	40	29725	500	32	250	0.003	0.003	16	200	4106	14702	1993	50	1	450	1	0.009	0.003	1	600	0.82	1.36	1	1671.53	821	820	450	1	352	490	0.41	804	200	300	1	52.36	10	1	15.44	10	2	200
11	WP012	6	6000	350	900	NZS3101-40MPa	35	40	29725	500	32	250	0.003	0.003	16	150	1345	17152	4293	50	1	450	1	0.009	0.003	1	699	0.98	3.41	1	2599.75	309	230	450	1	1302	490	0.76	804	150	300	1	39.27	10	1	45.60	10	2	200
12	WP013	6	4750	350	713	NZS3101-40MPa	35	40	29725	500	32	250	0.003	0.003	16	150	1257	14210	5432	50	1	450	1	0.009	0.003	1	534	1.36	5.45	1	2019.72	146	230	450	1	2749	490	1.26	804	150	300	1	39.27	10	1	41.53	10	2	200
13	WP014	6	5750.8	350	863	NZS3101-40MPa	35	40	29725	500	32	250	0.003	0.003	16	150	13471	4023	50	1	450	1	0.009	0.003	1	671	0.84	3.33	1	2494.66	322	230	450	1	1247	490	0.74	804	150	300	1	39.27	10	1	45.76	10	2	200	
14	WP015	6	4600	350	690	NZS3101-40MPa	35	40	29725	500	32	250	0.003	0.003	16	150	1903	14053	6832	50	1	450	1	0.009	0.003	1	438	1.72	7.07	1	1818.78	101	230	450	1	3962	490	1.85	804	150	300	1	39.27	10	1	24.71	10	2	200
15	WP016	6	7000	350	1050	NZS3101-40MPa	35	40	29725	500	32	250	0.003	0.003	16	200	3789	14322	2058	50	1	450	1	0.009	0.003	1	628	0.76	1.40	1	1918.34	821	820	450	1	295	490	0.41	804	200	300	1	52.36	10	1	19.37	10	2	200
16	WP017	6	1500	350	525	NZS3101-40MPa	35	40	29725	500																																							

## Analytical NZS 3101:2006 Model Checks - Refinement of "Shear Friction Utilizations"

Pier	Shear friction util of dowels
WP001	2.99
WP002	3.04
WP003	2.56
WP004	1.91
WP005	1.02
WP006	0.14
WP007	1.36
WP008	3.67
WP009	3.45
WP010	2.16
WP011	0.41
WP012	0.76
WP013	1.26
WP014	0.74
WP015	1.85
WP016	0.41
WP017	0.38
WP018	0.71
WP019	0.78
WP020	0.70
WP021	1.52
WP022	0.06
WP023	0.88
WP024	0.92
WP025	0.86
WP026	0.69
WP027	1.51
WP028	0.06
WP029	1.16
WP030	1.94
WP031	1.06
WP032	1.48
WP033	5.89
WP034	0.73
WP035	1.86
WP036	1.10
WP037	0.63
WP038	1.06
WP039	2.12
WP040	0.97
WP041	1.50
WP042	0.45
WP043	0.97
WP044	0.29
WP045	2.08
WP046	4.10
WP047	1.09
WP048	1.94
WP049	1.54
WP050	0.56
WP051	0.54
WP052	0.56
WP101	1.44
WP102	1.02
WP103	0.53
WP104	0.17
WP105	0.13
WP106	0.76
WP107	0.91
WP108	0.82
WP109	0.86
WP110	0.17
WP111	0.40
WP112	0.88
WP113	1.67
WP114	0.34
WP115	1.77
WP116	0.69
WP117	1.07
WP118	1.03
WP119	0.53
WP120	2.04
WP121	0.43
WP122	0.43
WP123	0.69
WP124	0.85
WP125	0.35
WP126	0.81
WP127	0.43
WP128	0.82
WP129	0.69
WP130	0.83
WP131	0.67
WP132	0.45
WP133	0.75
WP134	1.72
WP135	0.64
WP136	0.68
WP137	0.15
WP138	0.34
WP139	0.61
WP140	0.52
WP141	0.61
WP142	0.57
WP143	0.38
WP144	0.43
WP145	0.57
WP146	0.63
WP147	0.53
WP148	0.79
WP149	0.60
WP150	0.43

The shear walls shall be "hooked" into the columns. Hence also consider the shear friction capacity of the adjacent column rebar which is full developed into the foundations.

**Shear Friction for Column:**  
 $\phi V = \phi \times u.s.f \times (A.s \times f.y)$

Where:  
 $\phi = 0.70$   
 $u.s.f = 1.0$

See next page for column shear friction capacities. Shear capacity consider the existing shear utilizations of the columns.  
 $V^*$  come from the summation of the  $V^*$  for each pier in range. See "Analytical NZS 3101:2006 Model Checks" for individual  $V^*$ .  
An example for WP001 - WP004 is given below:

**For WP001 - WP004 (A08):**  
 $2 \times 1000\text{mm} \times 1000\text{mm} (B) = 36\text{HD32} = 0.70 \times 1.0 \times (\pi \times 16^2 \times 500\text{MPa} \times 2 \times 36) / 1000 = 20267\text{kN}$   
 $2 \times 800\text{mm} \times 800\text{mm} (B) = 16\text{HD25} = 0.70 \times 1.0 \times (\pi \times 12.5^2 \times 500\text{MPa} \times 2 \times 16) = 5497\text{kN}$   
Total Additional Capacity = 25,765kN >  $V^* = 22,818\text{kN}$  (OKAY)

**WP005 (A08):**  
 $1250 \times 1250(B) + 1000 \times 1000(A) = 22,510\text{kN} + 15,028\text{kN} > V^* = 1,633\text{kN}$  (OKAY)

**WP006 - WP010 (A06):**  
 $1000 \times 1000 (B) + 1000 \times 1000 (B) + 1250 \times 1250 (C) = 9428\text{kN} + 9428\text{kN} + 10354\text{kN} = 29,210\text{kN} > V^* \text{ of } 22134\text{kN}$  (OKAY)

**WP012 - WP015 (A04):**  
 $1000 \times 1000 (B) + 1000 \times 1000(A) = 9428\text{kN} + 15,028\text{kN} = 24,456\text{kN} > V^* \text{ of } 20580\text{kN}$  (OKAY)

**WP017 - WP023 (A01):**  
 $1000 \times 1000 (B) + 1250 \times 1250(B) + 800 \times 800(B) = 9428\text{kN} + 22510\text{kN} + 2748\text{kN} = 34,686\text{kN} > V^* \text{ of } 10478\text{kN}$  (OKAY)

**WP024 - WP029 (B01):**  
 $1000 \times 1000 (B) + 1250 \times 1250(B) + 800 \times 800(B) = 9428\text{kN} + 22510\text{kN} + 2748\text{kN} = 34,686\text{kN} > V^* \text{ of } 10900\text{kN}$  (OKAY)

**WP030 - WP033 (B04):**  
 $1000 \times 1000 (B) + 1000 \times 1000(A) + 800 \times 800(B) = 9428\text{kN} + 15,028\text{kN} = 24,456\text{kN} > V^* \text{ of } 20803\text{kN}$  (OKAY)

**WP035 - WP041 (B06):**  
 $1000 \times 1000 (B) + 1000 \times 1000 (B) + 1250 \times 1250 (C) = 9428\text{kN} + 9428\text{kN} + 10354\text{kN} = 29,210\text{kN} > V^* \text{ of } 19376\text{kN}$  (OKAY)

**WP043 - WP049 (B08):**  
 $1250 \times 1250(B) + 1000 \times 1000(A) = 22,510\text{kN} + 15,028\text{kN} = 37,538\text{kN} > V^* \text{ of } 20890\text{kN}$  (OKAY)

**WP101 - 102:**  
 $800 \times 800(B) + 800 \times 800(B) = 2748\text{kN} + 2748\text{kN} = 5,496\text{kN} < V^* \text{ of } 8497\text{kN}$  (NOT OKAY)  
Further Refinement: Wall Reo Capacity + Column Reo Capacity =  $(5130/1.44 + 3367/1.02) + 5496 = 6863\text{kN} < V^* \text{ of } 8497\text{kN}$  (NOT OKAY, change vert reo to HD32 - 250)

**WP113 - WP118:**  
 $1000 \times 1000(B) + 800 \times 800(B) + 1250 \times 1250 (C) = 2748\text{kN} + 10354\text{kN} = 13,102\text{kN} < V^* \text{ of } 14434\text{kN}$

Further refinement: Wall Reo Capacity + Column Reo Capacity =  $[3266/1.67 + 468/0.34 + 2742/1.77 + 2430/0.69 + 2272/1.07 + 3255/1.03] + 13102 = 26,788\text{kN} >> V^* \text{ (OKAY)}$

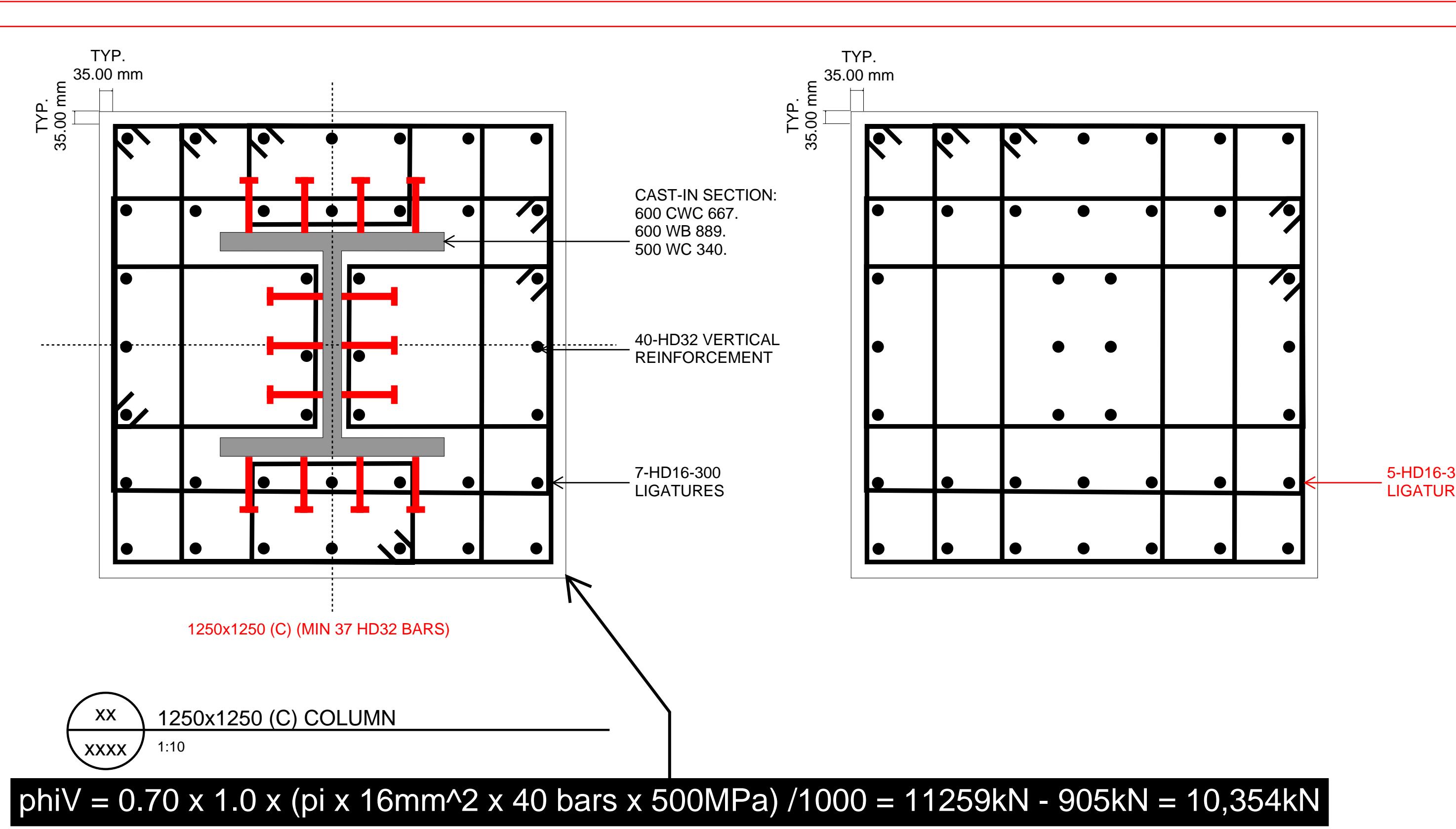
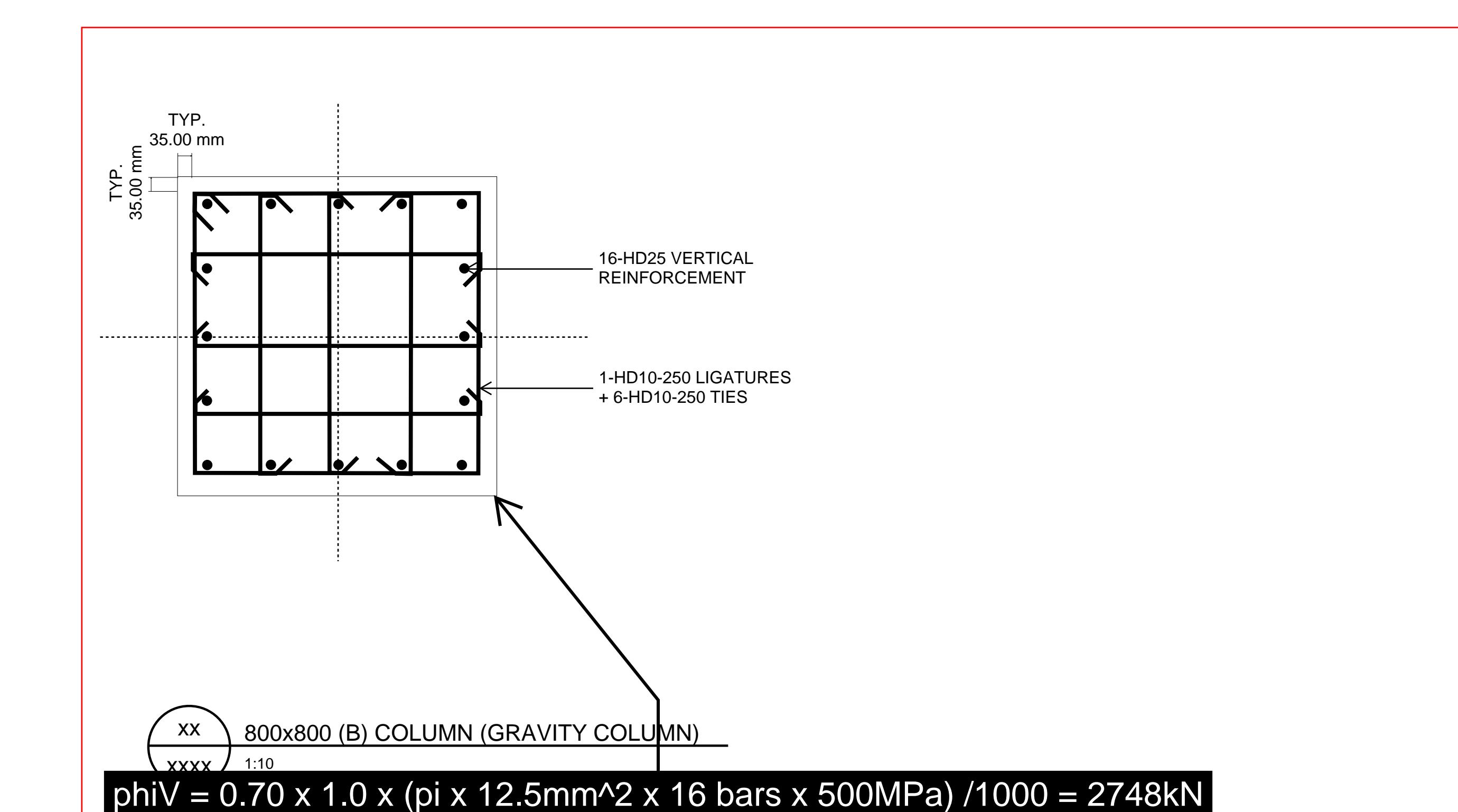
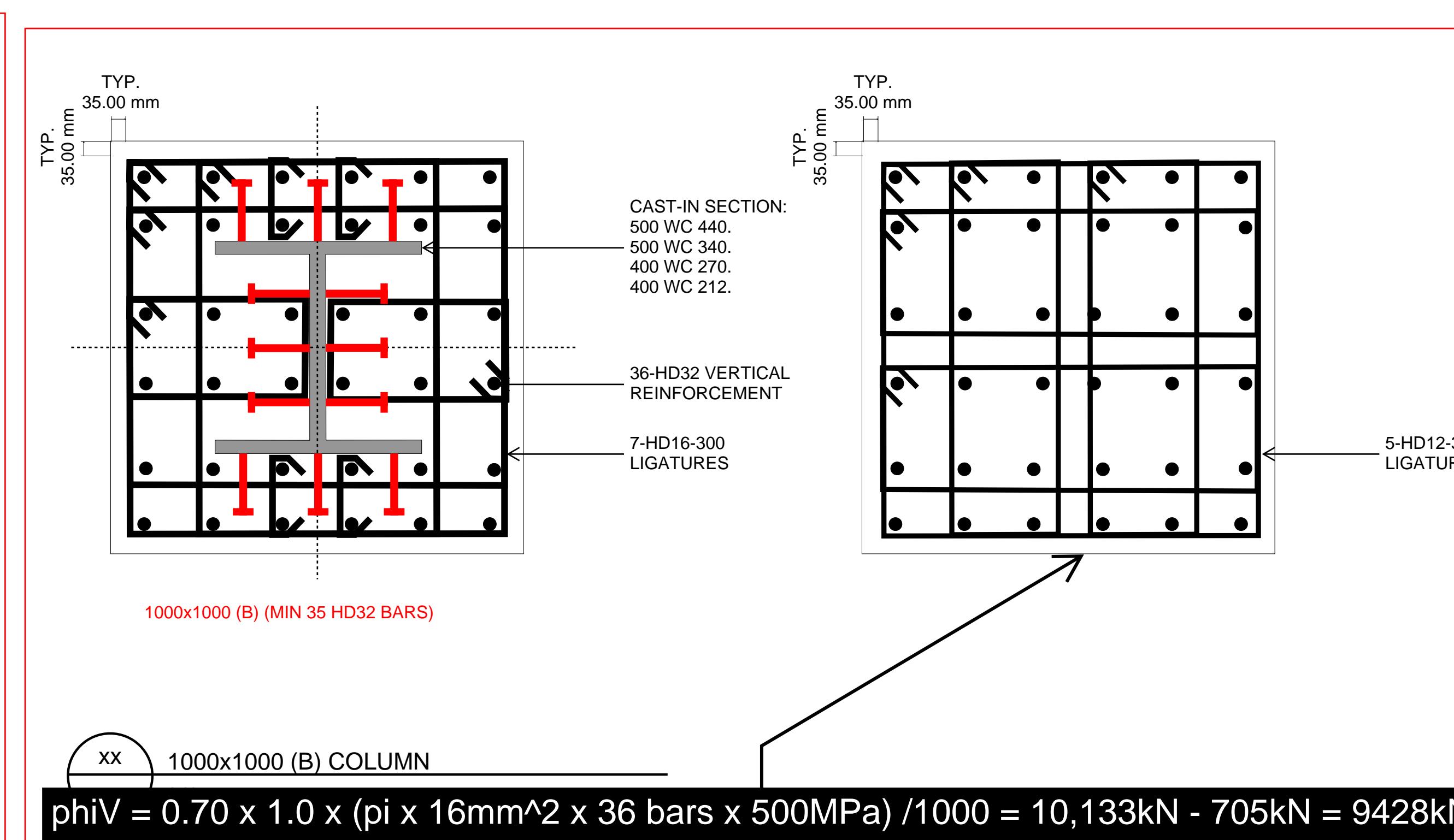
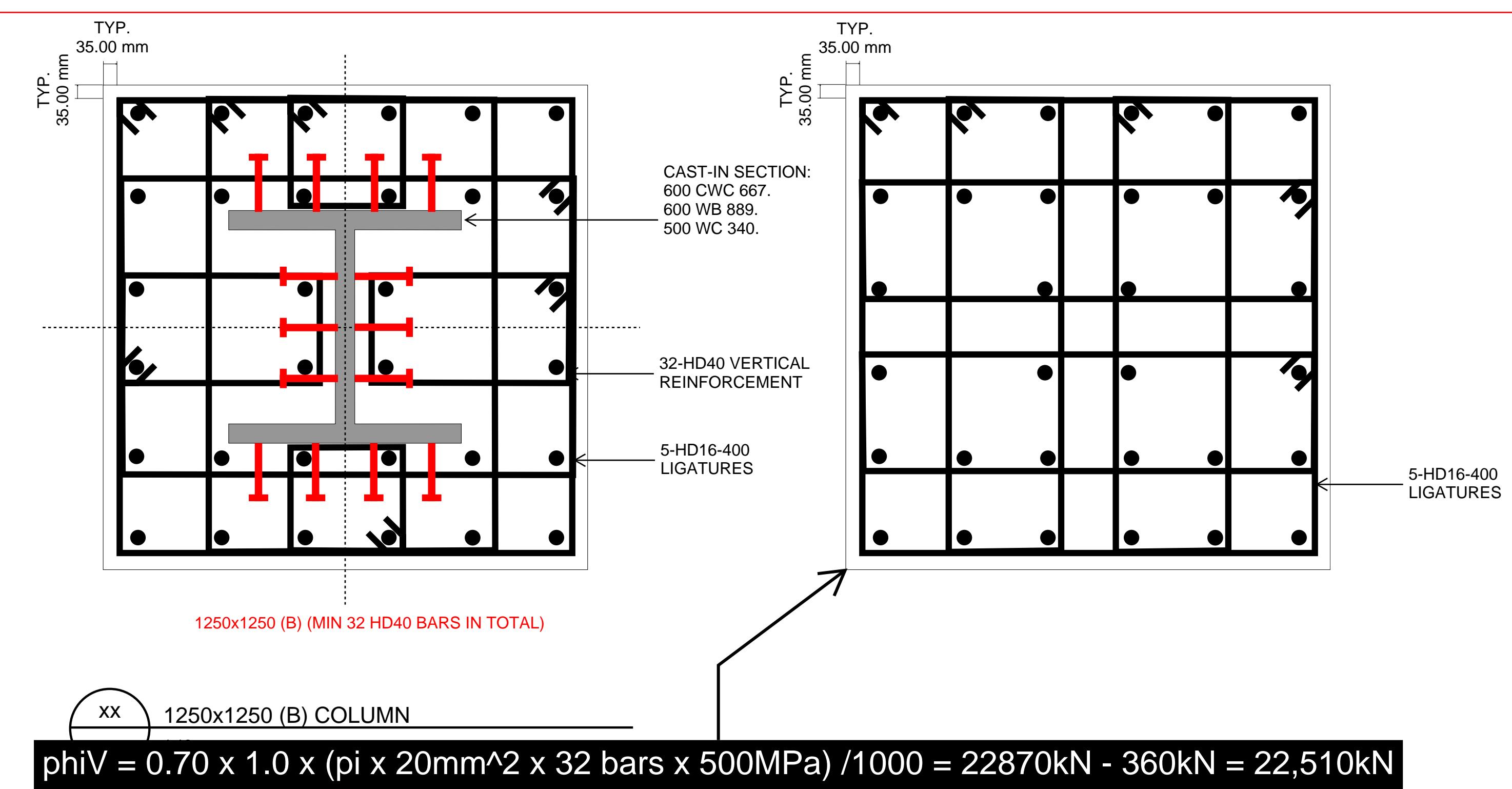
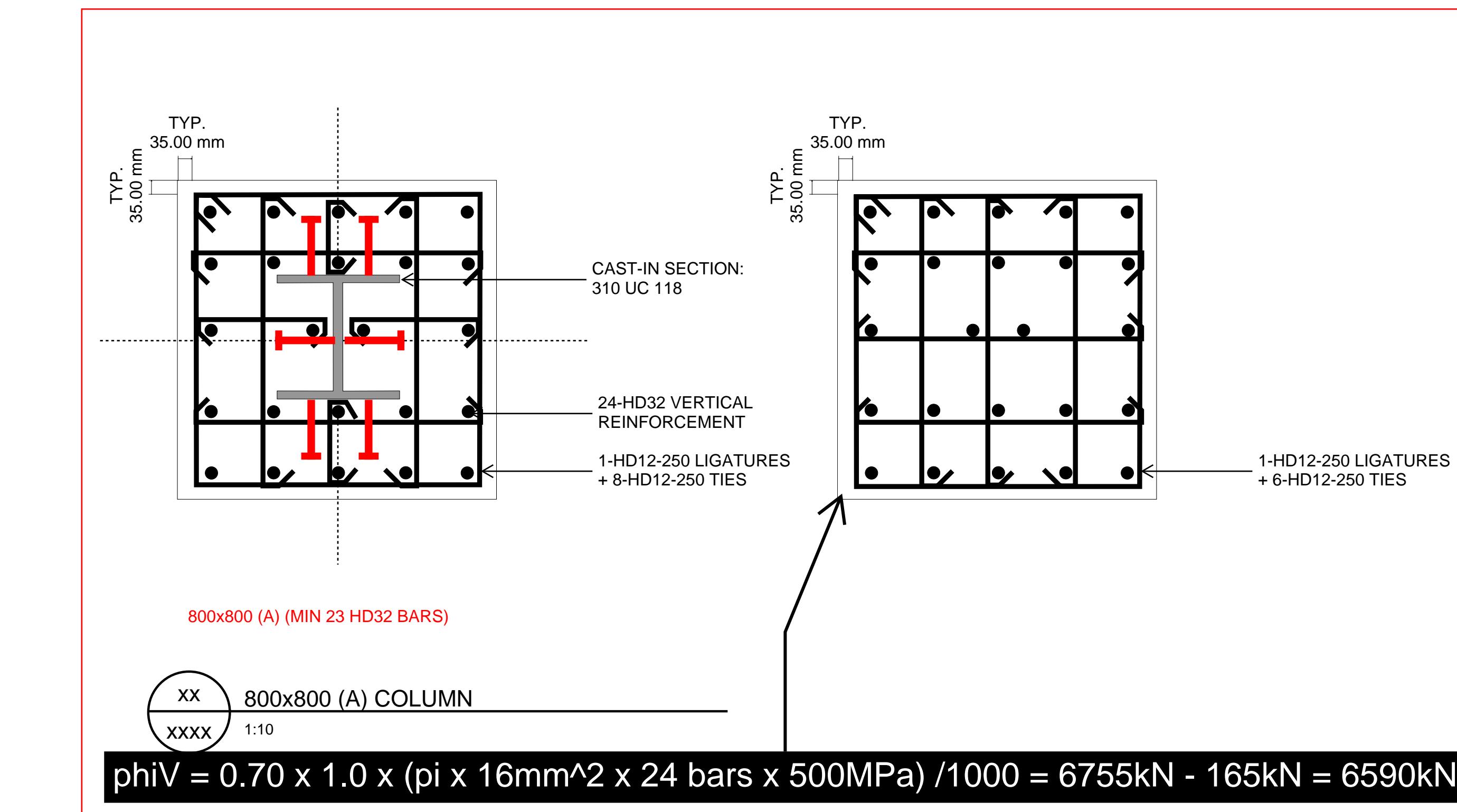
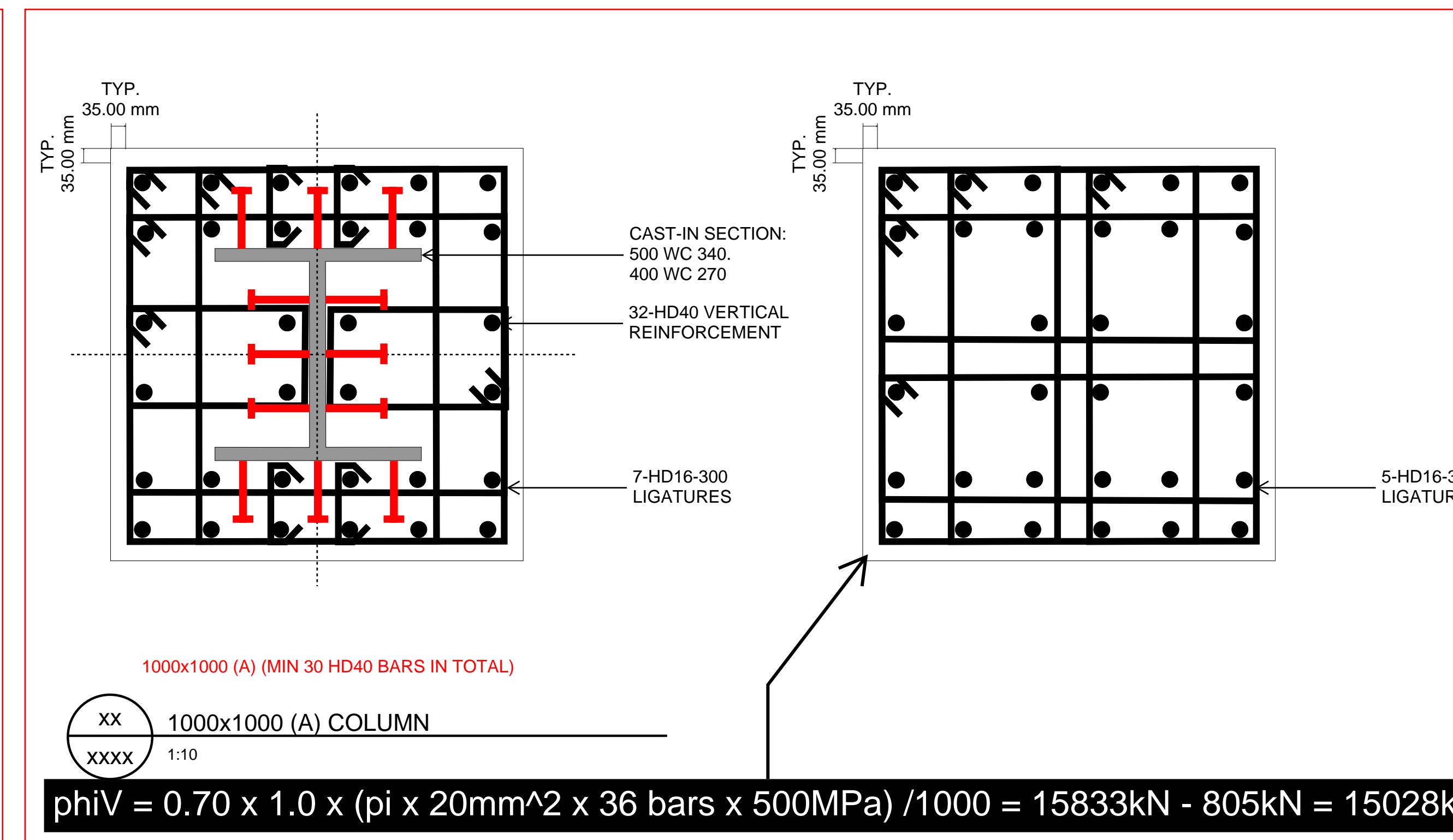
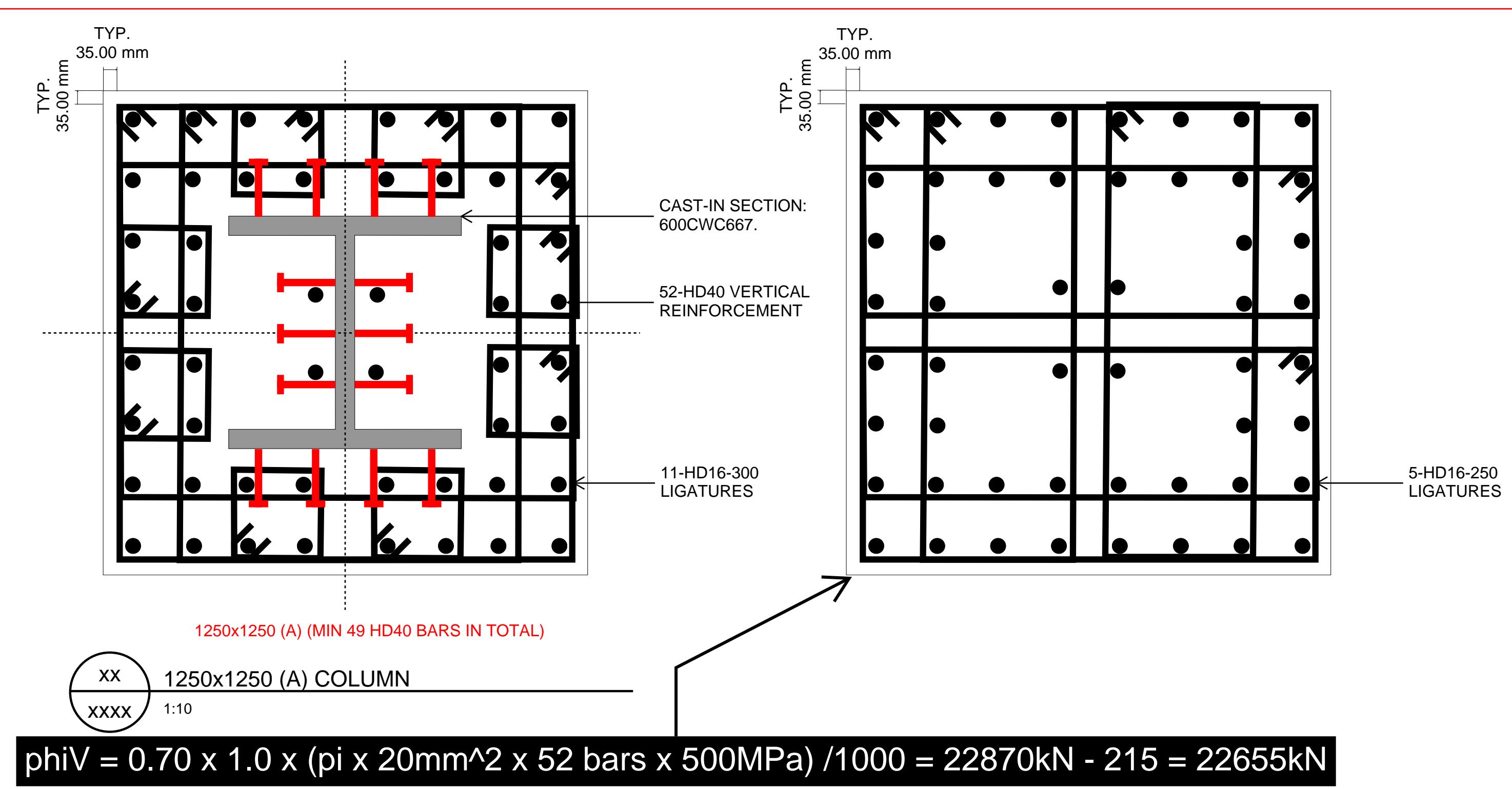
**WP120 - WP121:**  
 $1000 \times 1000(A) + 1000 \times 1000(A) + 1250 \times 1250 (C) = 15028\text{kN} + 10354\text{kN} = 25,382\text{kN} > V^* \text{ of } 9727\text{kN}$  (OKAY)

**WP133 - 134:**  
 $1250 \times 1250(C) + 1000 \times 1000(A) + 1000 \times 1000(A) = 10,354\text{kN} + 15028\text{kN} + 15028\text{kN} = 25,382\text{kN} > V^* \text{ (9854kN)}$  (OKAY)

WESTERN STAND - LEVEL 00  
COLUMN SHEAR FRICITION  
CAPACITIES

Shear Friction:  
 $\phi_i V = \phi_i \times u.s.f_x \times (A.s \times f.y)$

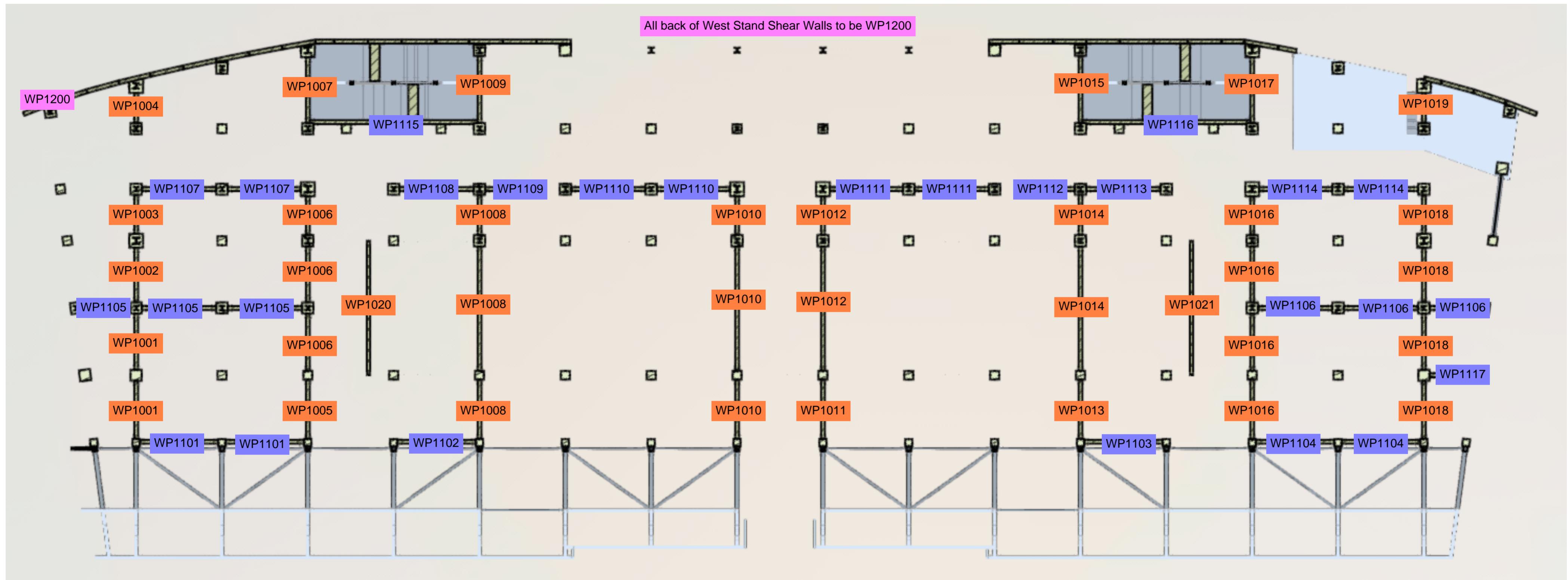
Where:  
 $\phi_i = 0.70$   
 $u.s.f = 1.0$



Subtract existing shear demands on columns.

Design Summary	
1250x1250 (A)	DESIGN LOADS
$N_c = 2550\text{kN}$	
$N_t = 25245\text{kN}$	
$M^{(total)} = 1265\text{ kNm (CRITICAL)}$	
$M^{(min)} = 75\text{ kNm (BASED ON MAX 30mm ECC.)}$	
$V^{(ETABS)} = 215\text{kN}$	
> REQUIRED VERTICAL REINFORCEMENT = 49 - 40HD	
1250x1250 (B)	DESIGN LOADS
$N_c = 2510\text{kN}$	
$N_t = 14761\text{kN}$	
$M^{(total)} = 1485\text{ kNm (CRITICAL)}$	
$M^{(min)} = 443\text{ kNm (BASED ON MAX 30mm ECC.)}$	
$V^{(ETABS)} = 360\text{kN}$	
> REQUIRED VERTICAL REINFORCEMENT = 32 - HD40	
1250x1250 (C)	DESIGN LOADS
$N_c = 695\text{kN}$	
$N_t = 1375\text{kN}$	
$M^{(total)} = 1965\text{ kNm (CRITICAL)}$	
$M^{(min)} = 251\text{ kNm (BASED ON MAX 30mm ECC.)}$	
$V^{(ETABS)} = 905\text{kN}$	
> REQUIRED VERTICAL REINFORCEMENT = 37 - HD32	
1000x1000 (A)	DESIGN LOADS
$N_c = 1405\text{kN}$	
$N_t = 13715\text{kN}$	
$M^{(total)} = 1510\text{ kNm (CRITICAL)}$	
$M^{(min)} = 411\text{ kNm (BASED ON MAX 30mm ECC.)}$	
$V^{(ETABS)} = 805\text{kN}$	
> REQUIRED VERTICAL REINFORCEMENT = 30 - HD40	
1000x1000 (B)	DESIGN LOADS
$N_c = 7840\text{kN}$	
$N_t = 7500\text{kN}$	
$M^{(total)} = 1380\text{ kNm (CRITICAL)}$	
$M^{(min)} = 400\text{ kNm (BASED ON MAX 30mm ECC.)}$	
$V^{(ETABS)} = 710\text{kN}$	
> REQUIRED VERTICAL REINFORCEMENT = 35 - HD32	
800x800 (A)	DESIGN LOADS
$N_c = 1510\text{kN}$	
$N_t = 4170\text{kN}$	
$M^{(total)} = 735\text{ kNm (CRITICAL)}$	
$M^{(min)} = 125\text{ kNm (BASED ON MAX 30mm ECC.)}$	
$V^{(ETABS)} = 165\text{kN}$	
> REQUIRED VERTICAL REINFORCEMENT = 23 - HD32	
800x800 (B) [GRAVITY ONLY]	DESIGN LOADS
$N_c = 4000\text{kN}$	
$N_t = 4000\text{kN}$	
CHECKED FOR	
$M = 150\text{ kNm (CRITICAL)}$	
$M^{(min)} = 120\text{ kNm (BASED ON MAX 30mm ECC.)}$	
> MIN REQUIRED VERTICAL REINFORCEMENT = 16 - HD25	

## Pier Labels - REVIT Model (Grouped)



## Schedule of Reinforcement based on REVIT Model Labels (Grouped)

Grouped Wall Label	Drossbach			Main wall			Vertical Reo Splice		Horizontal Shear		Confinement			
	Size	Duct Diameter	Spacing	Size (2 layers)	Spacing2	Central bar Length	Sleeve Length	Size4	Spacing5	Size7	Drossbach Zone Spacing	Cover (mm)	Concrete strength (MPa)	
WP1001	25	50	300	20	300	1300	1350	16	150	10	150	35	40	
WP1002	32	60	300	25	300	1550	1600	16	150	10	150	35	40	
WP1003	25	50	300	20	300	1300	1350	16	150	10	150	35	40	
WP1004	25	50	300	20	300	1300	1350	16	200	10	150	35	40	
WP1005	32	60	250	25	250	1550	1600	16	150	10	150	35	40	
WP1006	25	50	300	20	300	1300	1350	16	150	10	150	35	40	
WP1007	32	60	250	25	250	1550	1600	16	200	10	150	35	40	
WP1008	32	60	250	25	250	1550	1600	16	150	10	150	35	40	
WP1009	32	60	250	25	250	1550	1600	16	200	10	150	35	40	
WP1010	32	60	250	25	250	1550	1600	16	200	10	150	35	40	
WP1011	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1012	32	60	250	25	250	1550	1600	16	200	10	150	35	40	
WP1013	25	50	300	20	300	1300	1350	16	150	10	150	35	40	
WP1014	32	60	300	25	300	1550	1600	16	150	10	150	35	40	
WP1015	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1016	32	60	250	25	250	1550	1600	16	150	10	150	35	40	
WP1017	32	60	250	25	250	1550	1600	16	200	10	150	35	40	
WP1018	32	60	250	25	250	1550	1600	16	150	10	150	35	40	
WP1019	32	60	300	25	300	1550	1600	16	150	10	150	35	40	
WP1020	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1021	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1101	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1102	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1103	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1104	32	60	250	25	250	1550	1600	16	200	10	150	35	40	
WP1105	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1106	32	60	250	25	250	1550	1600	16	200	10	150	35	40	
WP1107	32	60	300	25	300	1550	1600	16	150	10	150	35	40	
WP1108	32	60	250	25	250	1550	1600	16	150	10	150	35	40	
WP1109	32	60	250	25	250	1550	1600	16	150	10	150	35	40	
WP1110	32	60	250	25	250	1550	1600	16	200	10	150	35	40	
WP1111	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1112	32	60	250	25	250	1550	1600	16	150	10	150	35	40	
WP1113	32	60	250	25	250	1550	1600	16	150	10	150	35	40	
WP1114	32	60	300	25	300	1550	1600	16	150	10	150	35	40	
WP1115	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1116	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
WP1117	32	60	225	25	225	1500	1550	16	200	10	150	35	40	
WP1200	32	60	300	25	300	1550	1600	16	200	10	150	35	40	
Mezzanine Walls	25	50	300	20	300	1300	1350	16	200	10	200	35	40	