

NAME: Solution

**PROBLEM 1:** The L-bracket shown is 12mm in diameter and is built into the wall at A. At point C a 10 kN load is applied as shown.

1a. Using the diagram below, draw the free-body diagram and determine the reactions at A. Using the diagram provided, illustrate the resultant bending moment, torque, normal force, and shear force at the wall.

$$(2) \quad \sum F_x = 0 = A_x + 8.66 \text{ kN} \Rightarrow \underline{A_x = -8.66 \text{ kN}}$$

$$(2) \quad \sum F_y = 0 = A_y - 5.0 \text{ kN} \Rightarrow \underline{A_y = 5.0 \text{ kN}}$$

$$(2) \quad \sum F_z = 0 = \underline{A_z}$$

$$\sum \vec{M}_A = \vec{0} = \vec{r}_{AC} \times \vec{F} = \begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ 1.5 \text{ m} & 0 & 0.5 \text{ m} \\ 8.66 \text{ kN} & 5 \text{ kN} & 0 \end{vmatrix} + \vec{M}_A$$

$$= [-(0.5 \text{ m})(5 \text{ kN})]\hat{i} - [-(0.5 \text{ m})(8.66 \text{ kN})]\hat{j} + [(1.5 \text{ m})(5 \text{ kN})]\hat{k} \\ + M_{Ax}\hat{i} + M_{Ay}\hat{j} + M_{Az}\hat{k}$$

Dotting with  $\hat{i}$

$$(2) \quad 0 = 2.5 \text{ kN}\cdot\text{m} + M_{Ax} \Rightarrow \underline{M_{Ax} = -2.5 \text{ kN}\cdot\text{m}}$$

Dotting with  $\hat{j}$

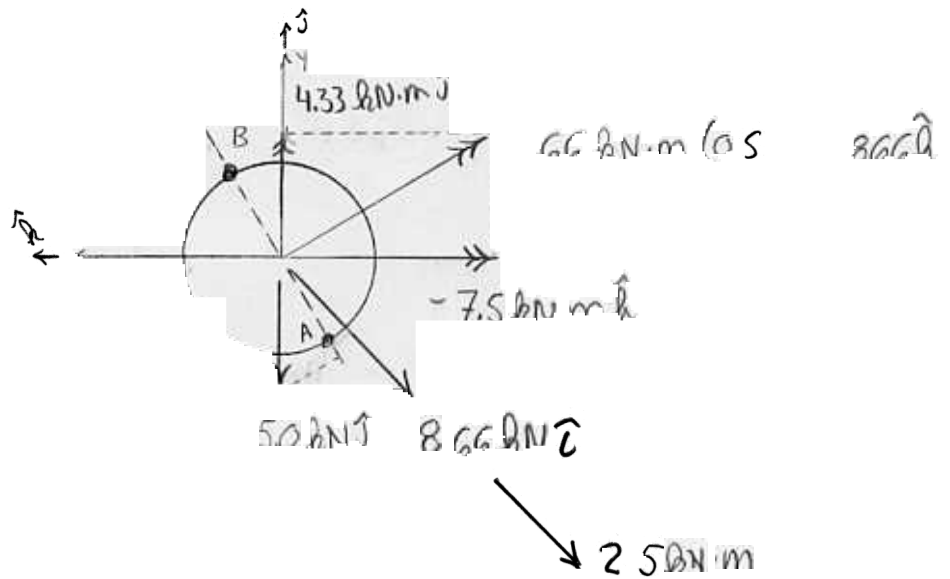
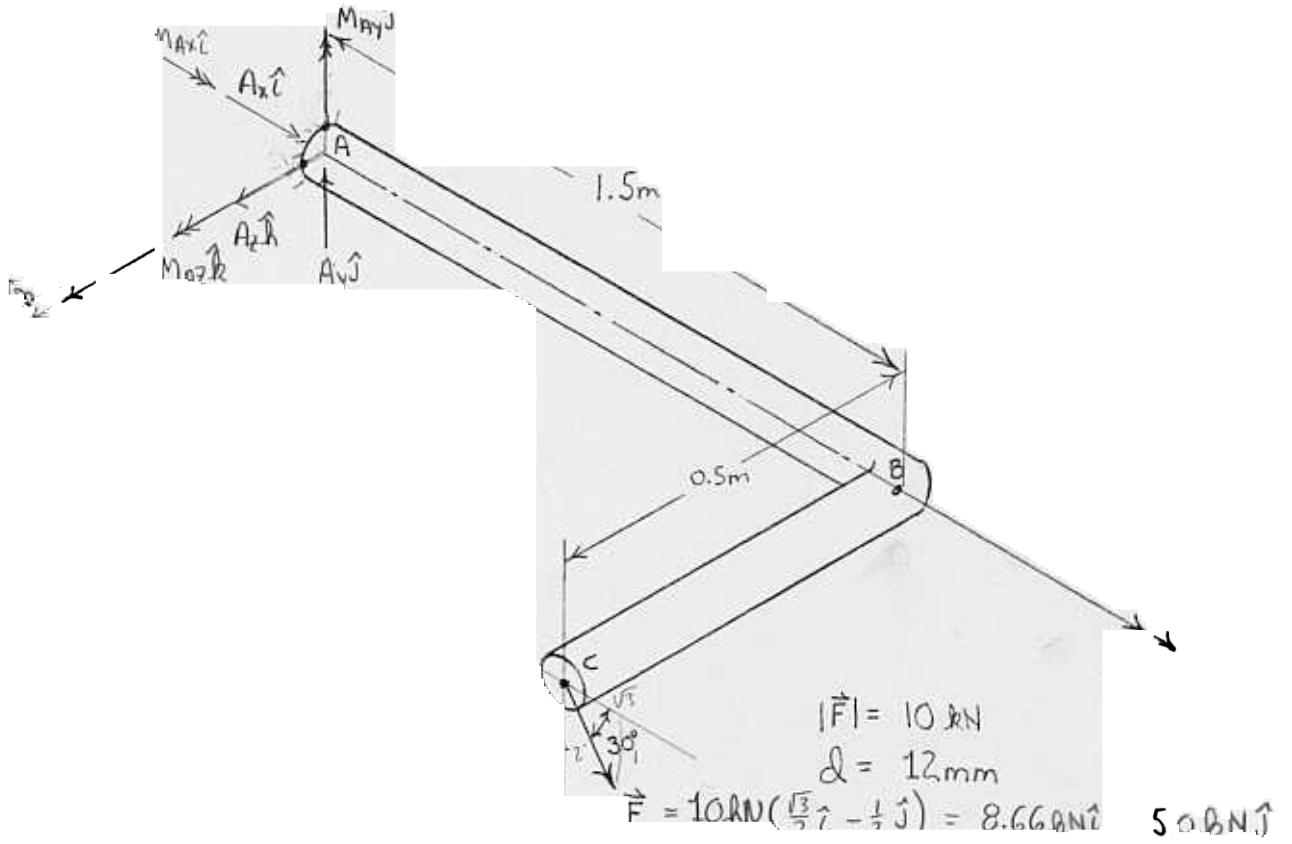
$$(2) \quad 0 = 4.33 \text{ kN}\cdot\text{m} + M_{Ay} \Rightarrow \underline{M_{Ay} = -4.33 \text{ kN}\cdot\text{m}}$$

Dotting with  $\hat{k}$

$$0 = -7.5 \text{ kN}\cdot\text{m} + M_{Az} \Rightarrow \underline{M_{Az} = 7.5 \text{ kN}\cdot\text{m}}$$

↑↑

4.)

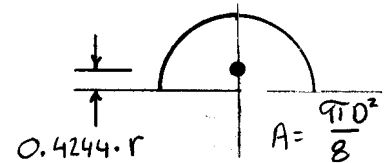


1b. At the point of the maximum bending stress in the beam at the wall, determine the complete stress state, designate the location on the previous illustration, and draw the resulting stress cube.

MAXIMUM TENSION AT A:  $46.56(10^9)$

$$\sigma_x = \frac{P}{A} + \frac{M c}{I} = \frac{8.66(10^3) \text{ N}}{\pi [12(10^{-3}) \text{ m}/2]^2} + \frac{8.66(10^3) \text{ N} \cdot \text{m} \cdot [12(10^{-3}) \text{ m}/2]}{\pi [12(10^{-3}) \text{ m}]^4} \cdot 64$$

$$51.07(10^9) \frac{\text{N}}{\text{m}^2} = \boxed{51.12 \text{ GPa}}$$



SHEAR STRESS DUE TO TORQUE

$$\tau_T = \frac{T r}{J} = \frac{2.5 \text{ kN} \cdot \text{m} \cdot 6(10^{-3}) \text{ m}}{\pi \cdot [12(10^{-3}) \text{ m}]^4} = 7.368(10^9) \frac{\text{N}}{\text{m}^2}$$

$$\boxed{7.368 \text{ GPa}}$$

SHEAR STRESS DUE TO SHEAR FORCE

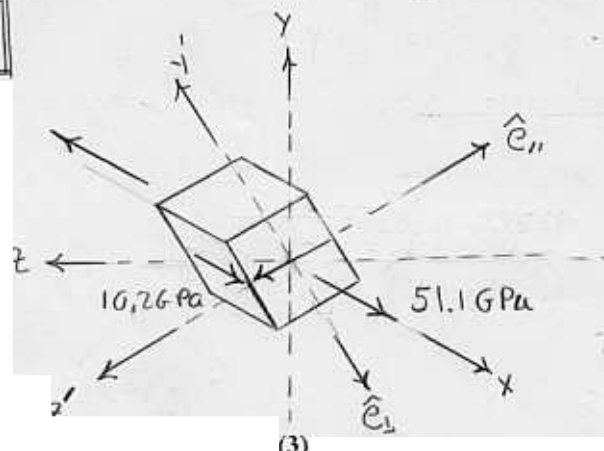
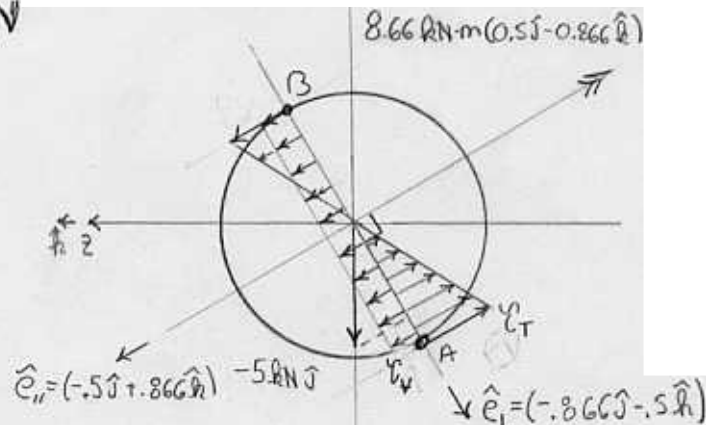
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$$5.8 \text{ kN} \hat{j} \cdot (-.5\hat{j} + .866\hat{k}) = 2.9 \text{ kN}$$

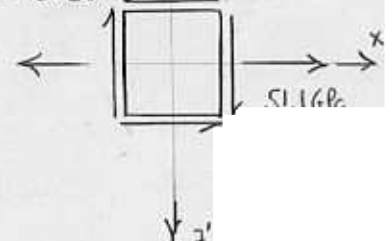
$$\tau_V = \frac{V Q}{I t} = \frac{2.9 \text{ kN} \cdot 0.4244 \cdot \left(\frac{12(10^{-3}) \text{ m}}{2}\right) \cdot \frac{\pi \cdot (12(10^{-3}) \text{ m})}{8}}{\pi \cdot [12(10^{-3}) \text{ m}]^4 \cdot 12(10^{-3}) \text{ m}}$$

$$2.849(10^9) \frac{\text{N}}{\text{m}^2}$$

$$\boxed{2.849 \text{ GPa}}$$



$$7.368 \text{ GPa} + 2.849 \text{ GPa} = 10.26 \text{ GPa}$$



**PROBLEM 2:** For the stress cube shown below, draw Mohr's circles and determine the principle stresses, maximum shear stress, and explain the orientations of these states of stress with respect to the original stress cube.

$$\sqrt{(2ksi)^2 + (2ksi)^2} = 2.828ksi$$

$$9.586ksi \quad 3.172ksi \quad 6.414ksi \quad 3)$$

$$\sigma_3 \quad 2ksi \quad 2.828ksi \quad 72ksi \quad 3)$$

$$\sigma_1 \quad 6ksi \quad 2.828ksi \quad 72ksi \quad 3)$$

$$\sigma_2 \quad 6ksi \quad 2.828ksi \quad 8.228ksi$$

$$\sigma_3 \quad 16ksi$$

$$\tau_{max1} \quad 2.828ksi$$

$$\tau_{max2} \quad 6ksi$$

$$\tau_{max3} \quad 172ksi / 2.586$$

$$\theta_{p1} \quad 22.5^\circ \quad \theta_{s1} \quad 22.5^\circ \quad 3)$$

$$\theta_{p2} = 0 \quad \theta_{s2} = -45^\circ \quad 3)$$

$$\theta_{p3} = 0 \quad \theta_{s3} = 45^\circ \quad 3)$$

