

PROBLEM 1.21 | A THIN, UNIFORMLY THICK PLATE OF THICKNESS " t " IS HANGING UNDER ITS WEIGHT AS SHOWN. THE CORRESPONDING DISPLACEMENT FIELD IN THE xy PLANE CAN BE APPROXIMATED BY.

$$u(x,y) = \frac{\rho \cdot g}{2 \cdot E} (2 \cdot b \cdot x - x^2 - \nu \cdot y^2) \quad (1)$$

$$v(x,y) = -\nu \cdot \frac{\rho \cdot g}{E} \cdot y \cdot (b - x) \quad (2)$$

WHERE ρ AND E ARE THE MASS DENSITY AND YOUNG'S MODULUS OF THE PLATE MATERIAL, RESPECTIVELY, AND g IS THE GRAVITY CONSTANT.

- DETERMINE THE CORRESPONDING PLANE STRESS FIELD $\sigma_x(x,y)$, $\sigma_y(x,y)$, AND $\tau_{xy}(x,y)$ AND COMMENT ON THE VALIDITY OF THE RESULTS.
- QUALITATIVELY, DRAW THE DEFORMED AND UNDEFORMED SHAPE OF THE EDGES ON THE SAME DRAWING. FOR THE SAKE OF CLARITY, EXAGGERATE THE DEFLECTIONS OF THE EDGES
- DETERMINE THE ROTATION OF THE PLATE AT POINTS "A" AND "B". DO THE ROTATIONS AGREE WITH THE SKETCH OF PART (b)?

GIVEN:

CONSTRAINTS

- PLATE OF DIMENSIONS $2a \times b$ THAT IS " t " THICK
- PLATE HAS DENSITY " ρ ", MODULUS OF ELASTICITY " E ", AND POISSON'S RATIO " ν "
- PLATE HANGES FROM THE PIN SUPPORT AT O.
- THE DISPLACEMENTS OF THE PLATE ARE APPROXIMATED BY u, v IN (1) & (2)

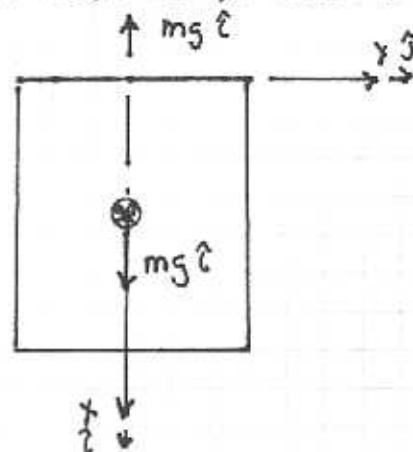
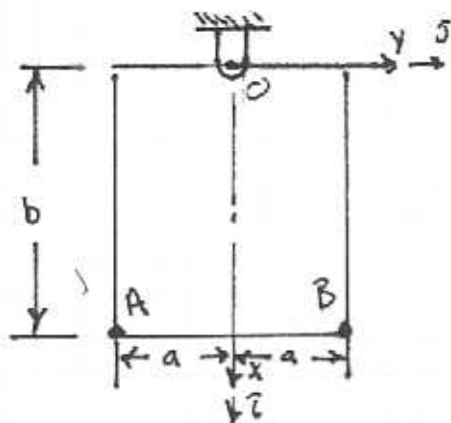
ASSUMPTIONS

- PLATE IS SUBJECTED TO SMALL DISPLACEMENTS AND STRAINS
- THE PLATE MATERIAL IS HOMOGENEOUS AND ISOTROPIC
- GRAVITY ACTS IN THE VERTICAL DIRECTION
- THE PIN AT "O" IS FRICTIONLESS

FIND:

- DETERMINE THE PLANE STRESS FIELD $\sigma_x(x,y)$, $\sigma_y(x,y)$, $\tau_{xy}(x,y)$
- DRAW THE UNDEFORMED AND DEFORMED EDGES OF THE PLATE.
- DETERMINE THE ROTATIONS OF THE PLATE AT POINTS "A" AND "B".

DIAGRAM



SOLUTION:

STARTING BY DETERMINING THE STRAINS FROM THE DISPLACEMENT FIELD

$$\epsilon_x = \frac{\partial u}{\partial x} = \frac{\partial}{\partial x} \left[\frac{\rho \cdot g}{2 \cdot E} (2 \cdot b \cdot x - x^2 - \nu \cdot y^2) \right]$$

$$= \frac{\rho \cdot g}{2 \cdot E} [2 \cdot b - 2 \cdot x] = \underline{\underline{\frac{\rho \cdot g}{E} [b - x]}} \quad (1)$$

$$\epsilon_y = \frac{\partial v}{\partial y} = \frac{\partial}{\partial y} \left[-\nu \cdot \frac{\rho \cdot g}{E} \cdot y \cdot (b - x) \right]$$

$$= -\nu \frac{\rho \cdot g}{E} [b - x] = \underline{\underline{-\nu \frac{\rho \cdot g}{E} \cdot (b - x)}} \quad (2)$$

$$\gamma_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} = \frac{\partial}{\partial y} \left[\frac{\rho \cdot g}{2 \cdot E} (2 \cdot b \cdot x - x^2 - \nu y^2) \right]$$

$$+ \frac{\partial}{\partial x} \left[-\nu \cdot \frac{\rho \cdot g}{E} \cdot y \cdot (b - x) \right]$$

$$= \frac{\rho \cdot g}{2 \cdot E} (-2 \cdot \nu \cdot y) - \nu \cdot \frac{\rho \cdot g}{E} \cdot y (-1)$$

$$= \frac{\rho \cdot g}{E} [-\nu \cdot y + \nu \cdot y] = \underline{\underline{0}} \quad (3)$$

USING THE ~~ST~~ PLANE STRESS STRESS-STRAIN RELATIONS, EXPRESSIONS FOR THE STATE OF STRESS CAN BE DERIVED

$$\sigma_x = \frac{E}{1 - \nu^2} [\epsilon_x + \nu \epsilon_y] = \frac{E}{1 - \nu^2} \left[\frac{\rho \cdot g}{E} (b - x) - \nu^2 \cdot \frac{\rho \cdot g}{E} (b - x) \right]$$

$$= \frac{E}{1 - \nu^2} \cdot \frac{\rho \cdot g}{E} \cdot (b - x) \cdot (1 - \nu^2) = \underline{\underline{\rho \cdot g \cdot (b - x)}} \quad (4)$$

$$\sigma_y = \frac{E}{1 - \nu^2} [\epsilon_y + \nu \epsilon_x] = \frac{E}{1 - \nu^2} \left[-\nu \frac{\rho \cdot g}{E} (b - x) + \nu \frac{\rho \cdot g}{E} (b - x) \right] = \underline{\underline{0}} \quad (5)$$

$$\tau_{xy} = \frac{E}{2(1 + \nu)} \cdot \gamma_{xy} = \frac{E}{2(1 + \nu)} \cdot (0) = \underline{\underline{0}}$$

IT APPEARS TO MAKE SENSE THAT THE ONLY STRESS IS IN THE ~~BAR~~ DIRECTION OF GRAVITY; HOWEVER, THE SOLUTION DOES NOT APPEAR TO ACCOUNT FOR THE REACTION AT "O" AND THE FREE SURFACE ALONG THE TOP OF THE PLATE.

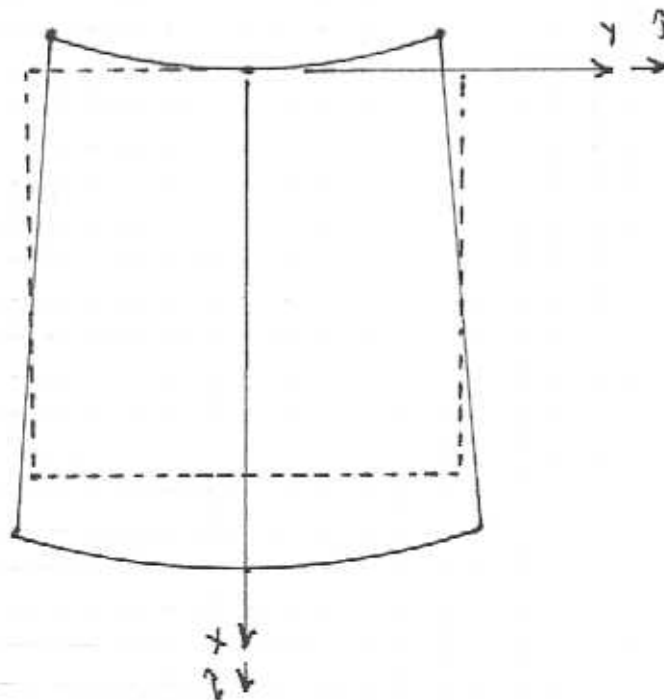
AN EXPRESSION FOR THE ROTATIONS ~~ARE~~ CAN BE DEVELOPED FROM THE DISPLACEMENTS

$$\begin{aligned}\Theta_{xy} &= \frac{1}{2} \left(\frac{\partial v}{\partial x} - \frac{\partial u}{\partial y} \right) = \frac{1}{2} \left\{ \frac{\partial}{\partial x} \left[-\nu \cdot \frac{p \cdot g}{E} \cdot y \cdot (b-x) \right] \right. \\ &\quad \left. - \frac{\partial}{\partial y} \left[\frac{p \cdot g}{2 \cdot E} \cdot (2 \cdot b \cdot x - x^2 - \nu \cdot y^2) \right] \right\} \\ &= \frac{1}{2} \left\{ \nu \cdot \frac{p \cdot g}{E} \cdot y - \frac{p \cdot g}{2 \cdot E} (-2 \cdot \nu \cdot y) \right\} = \frac{1}{2} \left\{ \nu \cdot \frac{p \cdot g}{E} \cdot y + \nu \cdot \frac{p \cdot g}{E} \cdot y \right\} \\ &= \underline{\underline{\frac{\nu \cdot p \cdot g}{E} \cdot y}}\end{aligned}$$

(6)

$$\Theta_{xy}^{(A)}(a) = -\frac{\nu \cdot p \cdot g}{E} \cdot a$$

$$\Theta_{xy}^{(B)}(a) = \frac{\nu \cdot p \cdot g}{E} a$$



SUMMARY:

THE PLACE WHERE THIS SOLUTION APPEARS TO FALL APART IS ON THE SURFACE WHERE IT IS PIN CONNECTED. SINCE THIS IS A PRESSURE FREE SURFACE, THE STRESS HERE SHOULD BE ZERO. THE ERROR IS A RESULT OF THE PIN CONNECTION. THE PIN CONNECTION IS A STRESS CONCENTRATION. THIS SOLUTION WOULD BE MORE APPROPRIATE FOR A PLATE FIXED TO THE WALL.