

PROBLEM 6.66 AN EXTRUDED BEAM HAS THE CROSS SECTION SHOWN. DETERMINE (a) THE LOCATION OF THE SHEAR CENTER O , (b) THE DISTRIBUTION OF THE SHEARING STRESS CAUSED BY THE 2.75 kIP VERTICAL SHEARING FORCE APPLIED AT O .

GIVEN:

1. 4 in BY 6 in BOX SECTION WITH ONE SIDE WALL CUT ALONG THE LENGTH
2. WALL THICKNESS OF $\frac{1}{8}$ in.
3. VERTICAL SHEARING FORCE OF 2.75 kips.

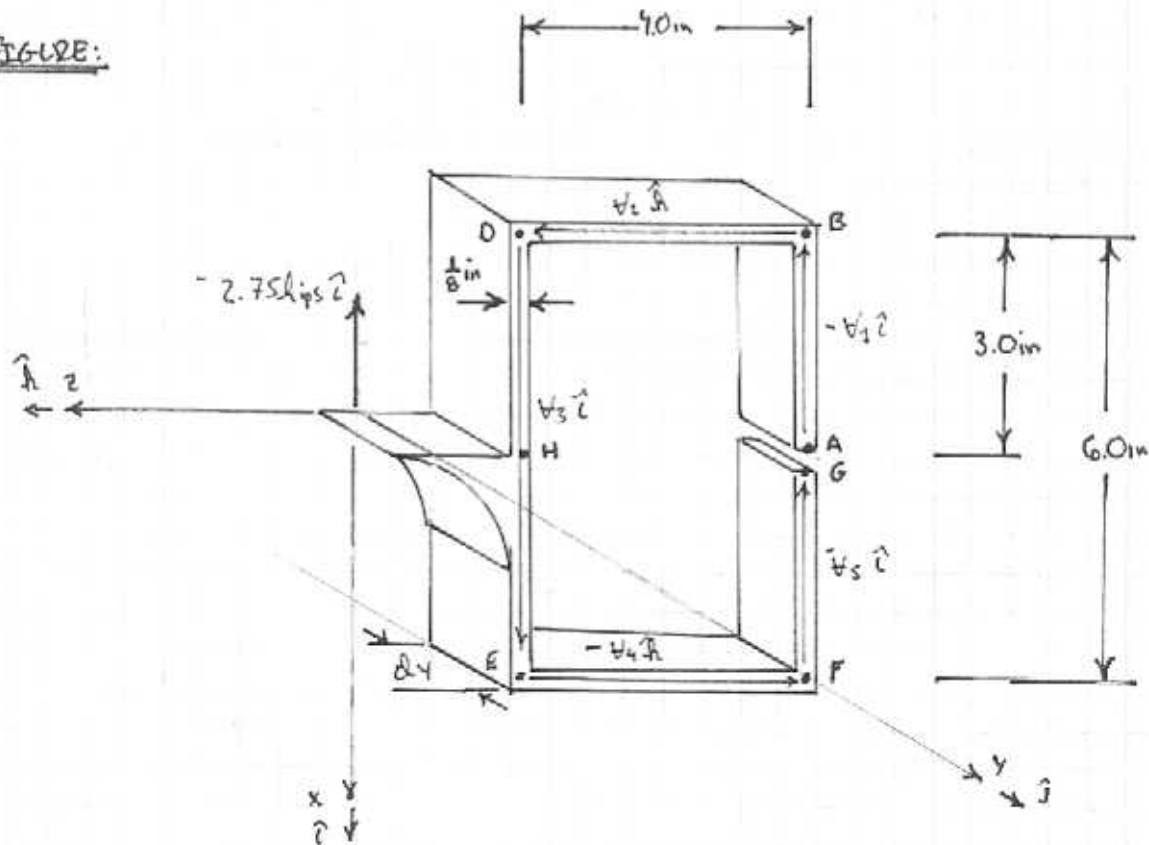
ASSUMPTIONS:

1. LINEAR ELASTIC MATERIAL
2. SMALL DEFORMATIONS

FIND:

1. THE LOCATION OF THE SHEAR CENTER
2. THE DISTRIBUTION OF THE SHEARING STRESS

FIGURE:



(a)

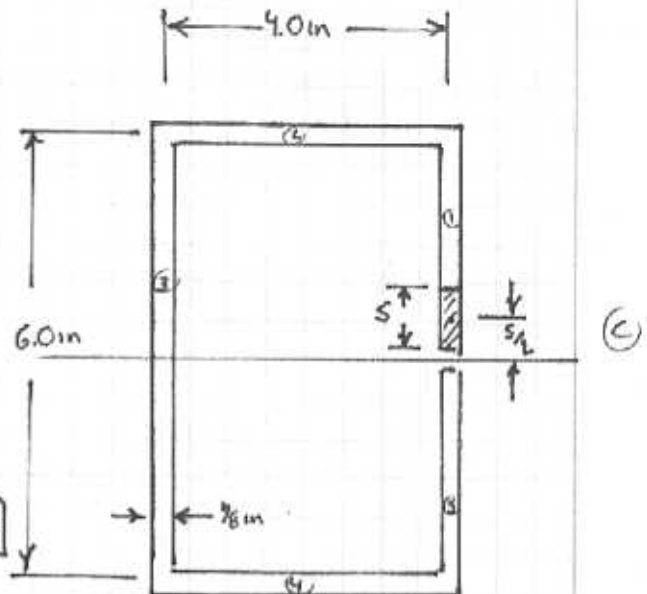
SOLUTION:

CALCULATING THE EXPRESSION FOR THE SHEARING STRESS IN THE FIRST SECTION OF THE BEAM.

$$\tau = \frac{V \cdot Q}{I \cdot t} \quad (1)$$

$$I = 2 \left[\frac{1}{12} \left(\frac{1}{8} \text{ in} \right) (3 \text{ in})^3 + \left(\frac{1}{8} \text{ in} \right) (3 \text{ in}) (1.5 \text{ in})^2 \right] + \frac{1}{12} \left(\frac{3}{8} \text{ in} \right) (6 \text{ in})^3 + 2 \left[\frac{1}{12} (4.0 \text{ in}) \left(\frac{1}{8} \text{ in} \right)^3 + (4.0 \text{ in}) \left(\frac{1}{8} \right) (3.0 \text{ in})^2 \right]$$

$$= 13.50 \text{ in}^4 \quad (2)$$



$$Q_1 = \bar{x} A = \left(\frac{5}{2} \right) \cdot \left(\frac{1}{8} \text{ in} \right) \cdot S = \frac{1}{16} \text{ in} \cdot S^2 \quad (3)$$

$$\tau_1 = \frac{V \cdot Q_1}{I t} = \frac{2.75(10^3) \text{ lb} \cdot \frac{1}{16} \text{ in} \cdot S^2}{13.50 \text{ in}^4 \cdot \frac{1}{8} \text{ in}} = 101.8 \frac{\text{lb}}{\text{in}^2} \cdot S^2 \quad (4)$$

AT THE END POINTS OF THIS SECTION

$$\tau_1(0) = 0 \quad (5)$$

$$\tau_1(3 \text{ in}) = 101.8 \frac{\text{lb}}{\text{in}^2} \cdot (3 \text{ in})^2 = 916.7 \frac{\text{lb}}{\text{in}^2} \quad (6)$$

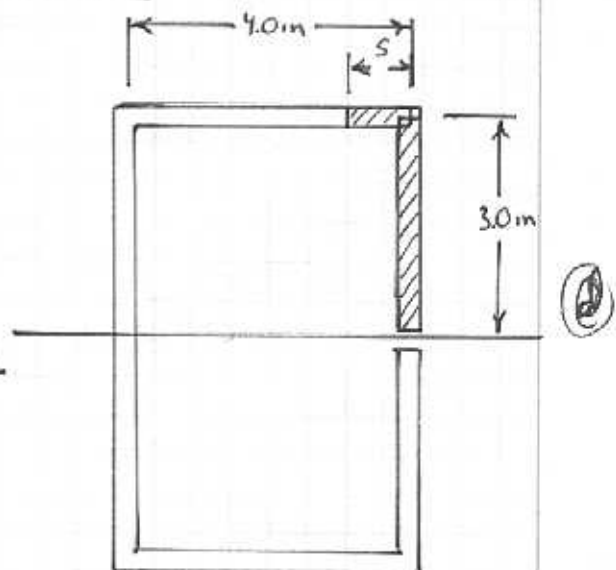
IN THE SECOND SECTION OF THE BEAM

$$Q_2 = \sum \bar{x} A = (1.5 \text{ in}) (3.0 \text{ in}) \left(\frac{1}{8} \text{ in} \right) + (3.0 \text{ in}) \left(\frac{1}{8} \text{ in} \right) \cdot S$$

$$= 0.5625 \text{ in}^3 + 0.3750 \text{ in}^2 \cdot S \quad (7)$$

$$\tau_2 = \frac{2.75(10^3) \text{ lb} \cdot (0.5625 \text{ in}^3 + 0.3750 \text{ in}^2 \cdot S)}{13.50 \text{ in}^4 \cdot \frac{1}{8} \text{ in}}$$

$$= 916.7 \frac{\text{lb}}{\text{in}^2} + 611.1 \frac{\text{lb}}{\text{in}^2} \cdot S \quad (8)$$



AT THE ENDS OF THIS SECTION

$$\tau_2(0) = 916.7 \frac{\text{lb}}{\text{in}^2} \quad (9)$$

$$\tau_2(4.0 \text{ in}) = 916.7 \frac{\text{lb}}{\text{in}^2} + 611.1 \frac{\text{lb}}{\text{in}^2} \cdot (4.0 \text{ in}) = 3361 \frac{\text{lb}}{\text{in}^2} \quad (10)$$

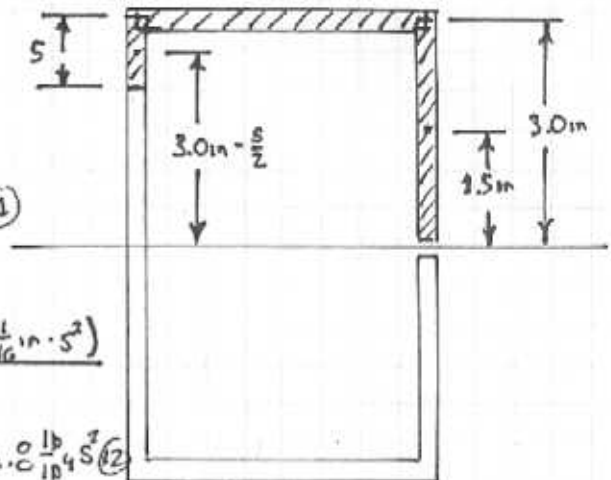
IN THE THIRD SECTION OF THE BEAM

$$Q_3 = \sum \bar{x}_i A_i = (1.5m)(3.0m)(\frac{1}{8}in) + (3.0m)(4.0m)(\frac{1}{8}in) + (3m - \frac{s}{2})(s)(\frac{1}{8}in)$$

$$= 2.062 in^3 + 0.375 in^2 \cdot s - \frac{1}{16} in \cdot s^2 \quad (11)$$

$$\tau_3 = \frac{2.75(10^3) lb \cdot (2.062 in^3 + 0.375 in^2 \cdot s - \frac{1}{16} in \cdot s^2)}{13.50 in^4 \cdot \frac{1}{8} in}$$

$$= 3361 \frac{lb}{in^2} + 611.1 \frac{lb}{in^3} \cdot s - 101.8 \frac{lb}{in^4} s^2 \quad (12)$$



AT THE ENDS OF THIS SECTION

$$\tau_3(0) = 3361 \frac{lb}{in^2} \quad (13)$$

$$\tau_3(3in) = 3361 \frac{lb}{in^2} + 611.1 \frac{lb}{in^3} \cdot (3in) - 101.8 \frac{lb}{in^4} (3in)^2$$

$$= 4277 \frac{lb}{in^2} \quad (14)$$

THE SHEAR STRESS CAN NOW BE INTEGRATED ALONG THE DIFFERENT SECTIONS OF THIS CROSS SECTION IN ORDER TO DETERMINE THE MAGNITUDE OF THE SHEAR FORCES IN EACH SECTION.

$$V_1 = V_5 = \int \tau_1 \cdot dA = \int \tau_1 \cdot t \cdot ds$$

$$= \int_0^{3m} 101.8 \frac{lb}{in^4} \cdot (\frac{1}{8} in) \cdot s^2 \cdot ds = 12.72 \frac{lb}{in^3} \cdot \frac{s^3}{3} \Big|_0^{3m}$$

$$= 4.242 \frac{lb}{in^3} [(3in)^3 - (0)^3] = 114.5 lb$$

$$= 0.1145 kips \quad (15)$$

$$V_2 = V_4 = \int \tau_2 \cdot dA = \int \tau_2 \cdot t \cdot ds$$

$$= \int_0^{4in} (916.7 \frac{lb}{in^2} + 611.1 \frac{lb}{in^3} \cdot s) (\frac{1}{8} in) ds$$

$$= \int_0^{4in} (114.5 \frac{lb}{in} + 76.39 \frac{lb}{in^2} \cdot s) ds$$

$$= [114.5 \frac{lb}{in} \cdot s + 76.39 \frac{lb}{in^2} \cdot \frac{s^2}{2}]_0^{4in} = [114.5 \frac{lb}{in} \cdot s + 38.20 \frac{lb}{in^2} \cdot s^2]_0^{4in}$$

$$= 114.5 \frac{lb}{in} \cdot (4in) + 38.20 \frac{lb}{in^2} \cdot (4in)^2 = 1069 lb$$

$$= 1.069 kips \quad (16)$$

$$\begin{aligned}
 V_3 &= 2 \cdot \int_0^{3\text{in}} \tau_3 dA = 2 \cdot \int_0^{3\text{in}} \tau_3 \cdot t \cdot ds \\
 &= 2 \cdot \int_0^{3\text{in}} \left[336 \frac{\text{lb}}{\text{in}^2} + 611.1 \frac{\text{lb}}{\text{in}^3} \cdot s - 101.8 \frac{\text{lb}}{\text{in}^4} \cdot s^2 \right] \left(\frac{1}{8} \text{in} \right) \cdot ds \\
 &= 2 \cdot \int_0^{3\text{in}} \left[420.1 \frac{\text{lb}}{\text{in}^2} + 76.39 \frac{\text{lb}}{\text{in}^3} \cdot s - 12.73 \frac{\text{lb}}{\text{in}^4} \cdot s^2 \right] ds \\
 &= 2 \cdot \left[420.1 \frac{\text{lb}}{\text{in}^2} \cdot s + \frac{76.39 \frac{\text{lb}}{\text{in}^3}}{2} \cdot s^2 - \frac{12.73 \frac{\text{lb}}{\text{in}^4}}{3} \cdot s^3 \right]_0^{3\text{in}} \\
 &= 2 \cdot \left[420.1 \frac{\text{lb}}{\text{in}^2} \cdot (3\text{in}) + 38.19 \frac{\text{lb}}{\text{in}^3} \cdot (3\text{in})^2 - 4.242 \frac{\text{lb}}{\text{in}^4} \cdot (3\text{in})^3 \right] \\
 &= \underline{\underline{2979 \text{ lb}}}
 \end{aligned}$$

FIGURE (C) SUMMARIZES THE FORCES THAT RESULT FROM THE SHEAR STRESS DISTRIBUTION AND THE EXTERNAL LOAD. NOW LETS CONSIDER EQUILIBRIUM

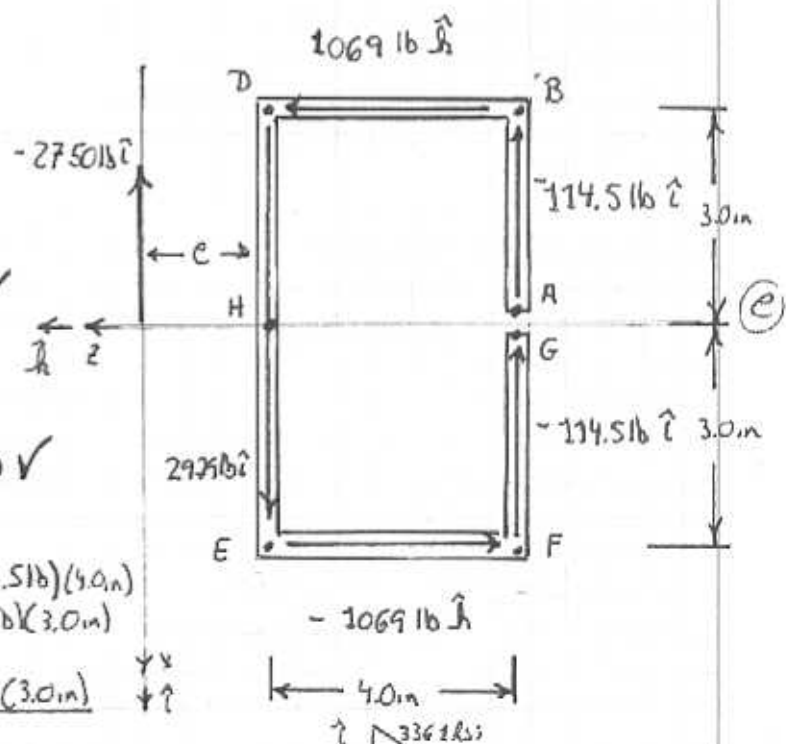
$$\sum F_z = 0 = 1069 \text{ lb} - 1069 \text{ lb} = 0 \checkmark$$

$$\sum F_x = 0 = -114.5 \text{ lb} - 114.5 \text{ lb} + 2979 \text{ lb} - 2750 \text{ lb} = 0 \checkmark$$

$$\sum M_{\text{topH}} = 0 = -(2750 \text{ lb}) \cdot e + 2 \cdot (114.5 \text{ lb}) (4.0 \text{ in}) + (1069 \text{ lb}) (3.0 \text{ in}) + (1069 \text{ lb}) (3.0 \text{ in})$$

$$e = \frac{2(114.5 \text{ lb})(4.0 \text{ in}) + 2(1069 \text{ lb})(3.0 \text{ in})}{2750 \text{ lb}}$$

$$= \underline{\underline{2.665 \text{ in}}}$$



SUMMARY:

FIGURE (F) SUMMARIZES THE DISTRIBUTION OF SHEAR IN THIS CROSS-SECTION. THE ONLY REASON WHY THE SHEAR STRESS IS THE SAME AT EACH OF THE REGION INTERSECTION POINTS IS BECAUSE THE WALL THICKNESS IS CONSTANT THROUGHT OUT THE SECTION.

