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PROBLEM 8.5-18 A horitantal bracket ABC consists of two perpendicular arms AB and BC, The later naving a length of 0.4m. Arm AB has a solid circular cross-section with diameter equal to 60mm. At point C a load P1=2.02 JeW acts vertically and a load P2=3.07 JeW acts horitantally parallel to arm AB. Considering only the forces P1 and P2, calculate the maximum tensile stress of, The maximum compressive stress of, The maximum shear Stress Compared at point P1, which is located at support A on the Side of the bracket at midheight.

GIVEN: CONST RAINTS

· Bracket That is fixed into a wall

hoads are applied in a cantilever mannor

2) Assumptions

· Linear elastic material response . The constraint at A does not allowary scrations or displacements

The maximum Tensile stress at P MayJ The maximum compassive stross at P The maximum shear stress at P 21 Αγĵ FREE BODY DIAGRAM: MAXT × ĉ 3.07.6N£ 0.4m 2.02 LNJ

STATICS:

The problem starts by solving for the reactions at A. Imposing Equilibrium

1

**(2)** 

**(3)** 

ZMZ/ATA = 0 = MAZ - (0.4m) (2.02 kN) => MAZ = 0.808 kN·m (6)

We have just found the leactions at A. To determine the stresses at P we need to determine the internal reactions in the beam just of the well. The Free body diagram to the right 1,228 hums 1.228 hum 1.228 hum 1.228 hum 1.228 hums 1.228 hum 1.228 hu Now we need to consider The stresses each of these scices and moments cause

-2,021NJ

3.07kNĥ -2.02.kn.l?

0.80elumb

7.02LN.L2 at paint p.

-0.805 LN m J

-1.272 hums

MECHANICS:

The force in the z direction along with the moments in The x and y directions give rise to norma stress in the 2 direction as follows

$$G_{z} = \frac{F_{z}}{A} + \frac{M_{\chi} \cdot y}{I_{\chi\chi}} - \frac{M_{\chi} \cdot x}{I_{\gamma\gamma}}$$

(7)

since point P is on The x axis (y=0) The second Term in 2) equals zero. The normal stress at point P is

$$\sigma_{z} = \frac{-3.07 \text{ kN}}{91 (0.03 \text{ m})^{2}} - \frac{(1.228 \text{ kN·m}) (80.03 \text{ m})}{91 (0.06 \text{ m})^{4}} = -59.0 (103) \frac{\text{kN}}{\text{m}^{2}}$$

07= 59.0 MPa

19.05mpa

The forces in The x and y directions along with the couple in the & direction all give rise to shearing stressos. Starting with the forces, there is no x directed force therefore the shear stress that lesurs from forces is given by

$$\mathcal{C}_{y\bar{z}} = \frac{V \cdot Q}{I \cdot c} = \frac{(2.07 \text{kN})(\frac{4 \cdot (0.03 \text{m})}{3 \cdot 71}) \cdot \frac{71}{z}(0.03 \text{m})^2}{\frac{91 \cdot (0.06 \text{m})^4 \cdot 0.06 \text{m}}{64} \cdot 0.06 \text{m}}$$

$$= 957.6 \frac{\text{kN}}{\text{m}^2}$$

$$= 0.9526 \text{m/R}$$

$$\frac{P_{x}}{\sqrt{1}}$$

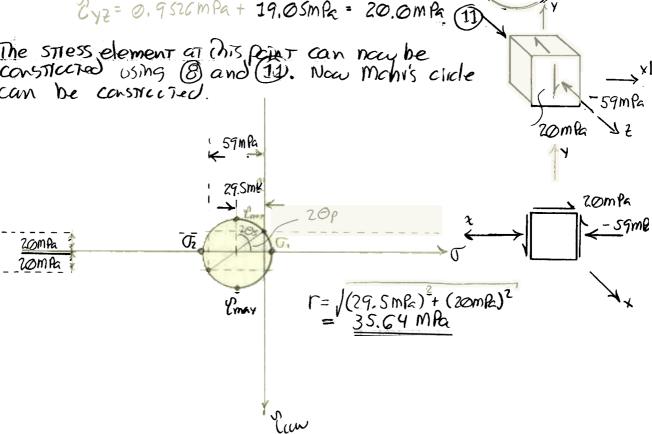
The shearing stress generated by the couple in the 2 direction is stren by

 $\gamma = \frac{T \cdot \Gamma}{J} = \frac{(0.808 \text{ kn·m}) (0.03\text{m})}{91 \cdot (0.06 \text{ m})^4} = \frac{19.0 \text{ smfa}}{19.0 \text{ smfa}} 13$ 

From the diagrams we see that Be total sheer stress at Pis given by

242= 0,9526mPa + 19,05mPa = 20.0mPa (1)

The stress element at this point can now be constructed using 8 and (1). Now Mohr's clicke can be consticited.



From the circle The maximum Tensile stress is given by  $U_1 = U_7 = U_6 = -29.5 \, \text{mRa} + 35.64 \, \text{mRa} = 6.141 \, \text{mRa}$   $U_7 = 6.14 \, \text{mRa}$ 

The maximum compressive stress is given by  $T_2 = T_2 = T_c = -29.5 \text{ m/h} - 35.64 \text{ m/h} = -65.14 \text{ m/h}$   $T_{c} = -65.1 \text{ m/h}$ 

And The maximum shear stress is given by

Lyz = 35.6 mPa

## Sommary:

The length of the biachet from A & B was not required in the solution to this peblom because of whole Pows located. No mitter what the length from A to B the compensat of mound strong that was exterior was 2 since I wis located along the contrained and parallel to this kad. Once the stress were dotes mined from the various confined of forces and moments the stress element at point P was constructed and mothers circle was drawn. This enabled the privaled stress of shearing strains to be determined.