2. 02 BN J

PROBLEM 8.5-18 1) h oritantal bracket ABC consists of Two perpendicular arms AB and BC. The later having a length of O.4m. Arm AB has a solid circular cross-section with

diameter egent to 60mm. At point Ca, load Py= 2.02 Jan. porallel to arm AB. Considering only the Jerces PlandBy

calculate The maximum tensile stress (t, The maximum compressive stress (t) and the maximum in-plane shear stress come at point P, which is located at support A on the side of the brocket at midheight.

GIVEN:

1) CONSTRAINIS

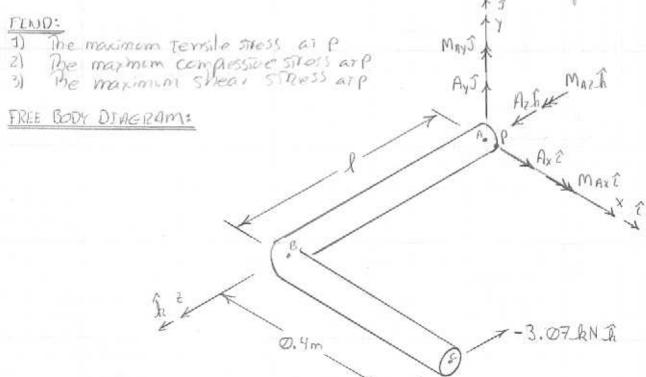
· Bracket That is fixed into a wall

hoads are applied in a confilerer manner

2) ASSCMPTIONS

· Linear elasic material regense

· The considerat at A class not allowary sciations or diploreneus



STATECS:

30 SHEETS 100 SHEETS 200 SHEETS

The problem starts by solving for the reactions at A. Imposing

ZFx=Ø=Ax

(1)

3

(3)

2M=/min = 0 = MAZ - (0.4m) (2.02 kN) => MAZ = 0.808 kN.m (6)

We have just found The reactions at A. To determine The stresses at P we need to determine The internal reactions in the beam just of the well. The The body diagram to the right 1.228hums & 2.08hus

-2.02MuJ

3.07hNh 3 K-1950.5-

O. BOR LAND &

Now we need to consider The stresses each of Prese forces and moments cause at point p.

7.02hv.12 3.0010m I

h = -0.508 bn mh

1.228 hwm3

MECHANICS:

The force in The & direction along with The moments in The x and y directions give rise to normal stress in the 2 direction as fellows

$$\sigma_z = \frac{F_z}{A} + \frac{M_x \cdot y}{I_{xx}} - \frac{M_y \cdot x}{I_{yy}}$$

(7)

since point P is on The x exis (y=0) The second Term in 3 equals zero. The Normal stress at point P is

 $G_{z} = \frac{-3.07 \text{ kN}}{97 (0.03 \text{ m})^{2}} - \frac{(1.228 \text{ kN·m}) (50.03 \text{ m})}{97 (0.06 \text{ m})^{4}} = -59.0 (103) \frac{\text{kN}}{\text{m}^{2}}$

07= 59.0 MPa

(8)

20MA WMA , 953MA

19.0 SMA

The forces in The x and y directions along with The couple in the & direction all give rise to shearing stressos. Starting with the Serves, there is no x directed force therefore the Shear stiess that lessess then forces is given by

$$\frac{V \cdot Q}{V_{YZ}} = \frac{(2.02 \text{kN})(\frac{4 \cdot (0.03 \text{m})}{3 \cdot \text{T}}) \cdot \frac{\text{T}}{\text{Z}}(0.03 \text{m})^{2}}{\frac{91 \cdot (0.06 \text{m})^{4} \cdot 0.06 \text{m}}{64} \cdot 0.06 \text{m}}$$

$$= 952.6 \frac{\text{kN}}{\text{m}^{2}}$$

$$= 0.9526 \text{ m/s}$$

$$= 0.9526 \text{ m/s}$$

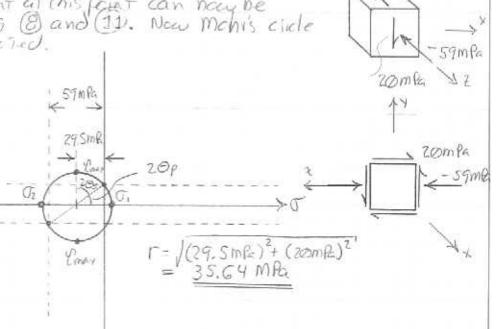
The shearing stress generated by the couple in the 2 direction is stren by

 $\tilde{\gamma} = \frac{T \cdot \Gamma}{J} = \frac{(0.808 \text{ linim}) (0.03m)}{91 \cdot (0.06 \text{ m})^{4}} = \frac{19.0 \text{ smfa}}{19.0 \text{ smfa}} \frac{15}{19}$ (10)

From The Diagrams we see that The total sheer silessat Pis given by

2vz= 0,9526mPa+ 19,05mPa = 20.0mPa (ID)

The stress element of this point can now be constituted using (8) and (11). Now Mohi's circle can be constituted.



22-141 50 SHEETS 22-142 100 SHEETS 22-144 200 SHEETS

From the circle The maximum Tensile Sitess is given by $O_1 = O_2 = O_2 = -29.5 \, \text{mps} + 35.64 \, \text{mps} = 6.141 \, \text{mps}$ $O_7 = 6.14 \, \text{mps}$

The maximum complessive stress is given by $\mathcal{T}_{z} = \mathcal{T}_{z} = \mathcal{T}_{c} = -29.5 \, \text{mps} - 35.64 \, \text{mps} = -65.14 \, \text{mps}$ $\mathcal{T}_{c} = -65.1 \, \text{mps}$

And The maximum shear stress is given by

Eyz = 35.6 MPa

Sommar:

The length of the Diachet Sem A & B was not required in the solution to this people because of whole Pacs located. No matter what the length from the 13 The compenent of Moimil 5 hors that was estern us 2 sice I was located along the contracted as a parallel to this kad. once the stress were dotermined from the thirties confined of fuces and memons the sitess element at point I was constituted all metris circle was drawn. This enabled the privaled sites of all sheeting strains to the determined.