

**PROBLEM 5** Construct shear-force and bending-moment diagrams for the beam ABC loaded as shown in the figure. The cable passes over a small frictionless pulley at C and supports a weight  $W = 5.0 \text{ kN}$ .

**GIVEN:**

1) Constraint

- Beam subjected to loading shown
- pin joints at A and B

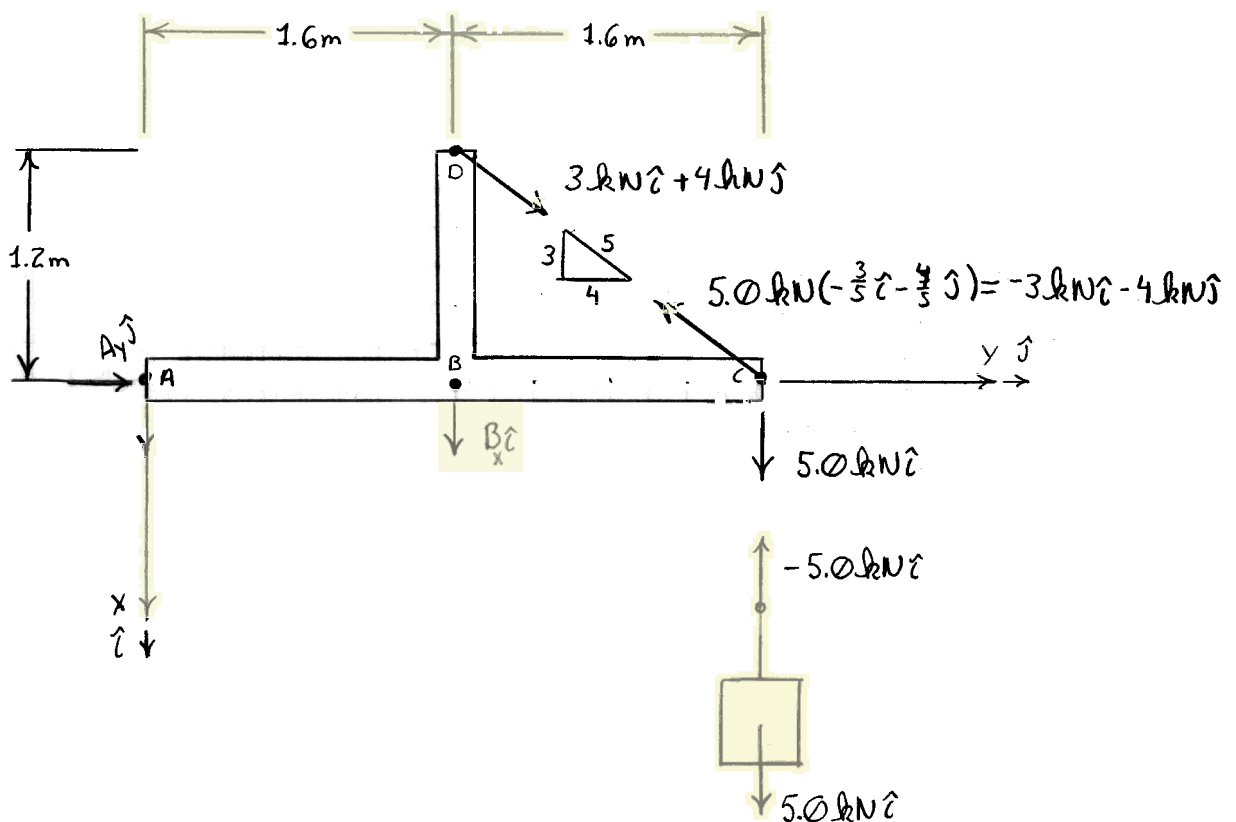
2) Assumptions

- All deflections caused by the loading are small
- pin joints do not provide any resistance to rotations
- frictionless pulley at C does not provide any resistance to the cable running over it.

**FIND:**

1) shear-force and bending moment diagram for ABC

**FREE BODY DIAGRAM:**



STATICS:

We start by using equilibrium to determine the reactions at A and B

$$\sum F_x = 0 = A_x + B_x + 3 \text{ kN} - 3 \text{ kN} + 5 \text{ kN} \Rightarrow \underline{A_x + B_x = -5 \text{ kN}} \quad (1)$$

$$\sum F_y = 0 = A_y - 4 \text{ kN} + 4 \text{ kN} \Rightarrow \underline{A_y = 0} \quad (2)$$

$$\sum \vec{M} = 0 = \vec{r}_{AD} \times \vec{F}_D + \vec{r}_{AB} \times \vec{B} + \vec{r}_{AC} \times \vec{F}_C$$

$$\vec{r}_{AD} = -1.2 \text{ m} \hat{i} + 1.6 \text{ m} \hat{j}$$

$$\vec{r}_{AB} = 1.6 \text{ m} \hat{j}$$

$$\vec{r}_{AC} = 3.2 \text{ m} \hat{j}$$

$$\vec{F}_D = 3 \text{ kN} \hat{i} + 4 \text{ kN} \hat{j}$$

$$\vec{B} = B_x \hat{i}$$

$$\vec{F}_C = (-3 \text{ kN} \hat{i} - 4 \text{ kN} \hat{j}) + 5 \text{ kN} \hat{i}$$

$$= 2 \text{ kN} \hat{i} - 4 \text{ kN} \hat{j}$$

$$0 = \begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ -1.2 \text{ m} & 1.6 \text{ m} & 0 \\ 3 \text{ kN} & 4 \text{ kN} & 0 \end{vmatrix} + \begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ 0 & 1.6 \text{ m} & 0 \\ B_x & 0 & 0 \end{vmatrix} + \begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ 0 & 3.2 \text{ m} & 0 \\ 2 \text{ kN} & -4 \text{ kN} & 0 \end{vmatrix}$$

$$= [(-1.2 \text{ m})(4 \text{ kN}) - (3 \text{ kN})(1.6 \text{ m})] \hat{k} + [-(B_x)(1.6 \text{ m})] \hat{k} + [-(2 \text{ kN})(3.2 \text{ m})] \hat{k}$$

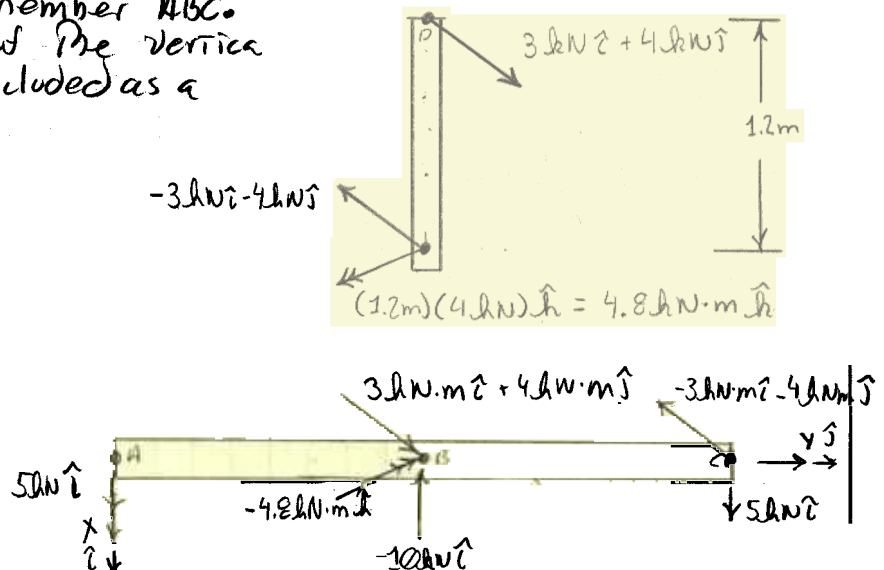
$$= [-9.6 \text{ kN} \cdot \text{m} - 1.6 \text{ m} \cdot B_x - 6.4 \text{ kN} \cdot \text{m}] \hat{k} = [-16 \text{ kN} \cdot \text{m} - 1.6 \text{ m} B_x] \hat{k}$$

$$\Rightarrow \underline{B_x = -10 \text{ kN}} \quad (3)$$

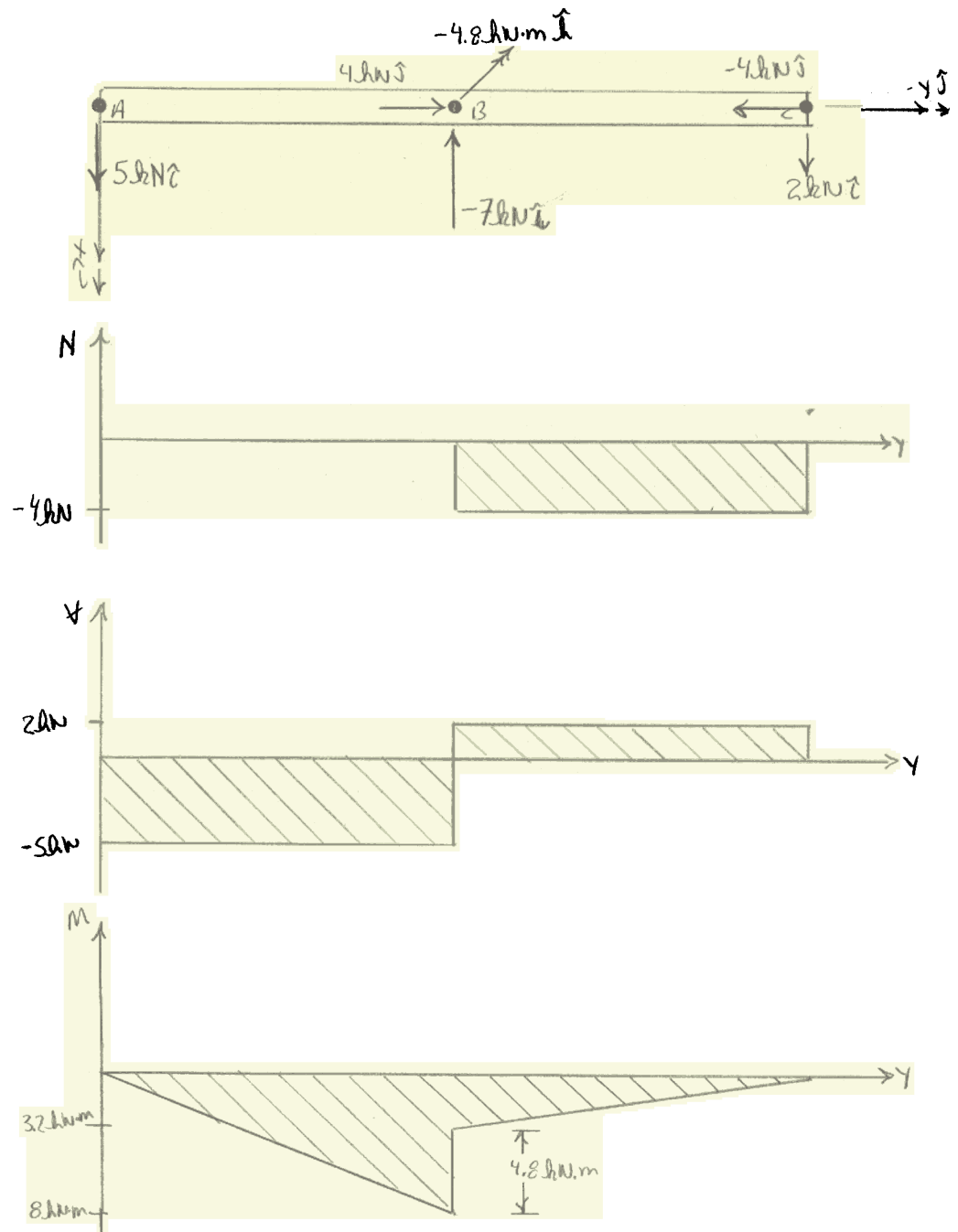
From (1)

$$A_x - 10 \text{ kN} = -5 \text{ kN} \Rightarrow \underline{A_x = 5 \text{ kN}}$$

Now the solution diagram is drawn in a way that isolates member ABC. Therefore the effect of the vertical extension BD must be included as a force and moment



Now the shear force and bending moment diagrams can be drawn using the direct integration method.



SUMMARY: This problem requires that you first understand that if a structure is in equilibrium the entire structure is in equilibrium. Then the normal force, shear force, and bending moment diagrams can be drawn showing the loading and the differential relationships between the load, shear force, and bending moment.

From (A) CONSIDERING EQUILIBRIUM

$$\sum F_x = 0 = A_x + B_x + 100 \text{ kN} \Rightarrow A_x + B_x = -100 \text{ kN} \quad (1)$$

$$\sum M_{z/eA} = 0 = -(1.6 \text{ m}) \cdot B_x - (3.2 \text{ m}) \cdot (100 \text{ kN}) \Rightarrow B_x = -\frac{3.2 \text{ m}}{1.6 \text{ m}} \cdot 100 \text{ kN} = -200 \text{ kN} \quad (2)$$

$$(2) \rightarrow (1) \Rightarrow A_x = 100 \text{ kN} \quad (3)$$

From (B), AN EQUIVALENT BEAM "ABC" IS CREATED, (C)

FIGURE (D) SHOWS THE SAME BEAM WITH THE NET FORCES SHOWN. NOW CONSIDER EXPRESSIONS FOR THE INTERNAL SHEAR AND BENDING MOMENT

$0 < y < 1.6 \text{ m}$  USING (E)

$$\sum F_x = 0 = 100 \text{ kN} + V \Rightarrow V = -100 \text{ kN}$$

$$\sum M_{z/eP} = 0 = M + 100 \text{ kN} \cdot y$$

$$\Rightarrow M = -100 \text{ kN} \cdot y$$

$1.6 \text{ m} < y < 3.2 \text{ m}$  USING (G)

$$\sum F_x = 0 = -V + 40 \text{ kN} \Rightarrow V = 40 \text{ kN}$$

$$\sum M_{z/eP} = 0 = -M - 40 \text{ kN} (3.2 \text{ m} - y)$$

$$\Rightarrow M = 40 \text{ kN} \cdot y - 128.0 \text{ kN} \cdot \text{m}$$

