



ENVIRONMENT

Manchester Metropolitan University
Ryebank Road
Chorlton
Flood Risk Statement and Conceptual
Drainage Strategy

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Flood Risk Statement and Conceptual Drainage Strategy

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EXECUTIVE SUMMARY

This Flood Risk Assessment and Conceptual Drainage Strategy has been prepared on behalf of Manchester Metropolitan University to support divestment of their asset at Ryebank Fields, Chorlton (approximate grid reference: SJ810945). The report is therefore not for the purposes of a planning application.

This report demonstrates that the proposed development is at not at a significant flood risk, subject to the flood mitigation strategies being implemented at the planning stage. Any future planning application at the site will require a site-specific Flood Risk Assessment (FRA) and Sustainable Drainage Statement (SDS) to be produced.

The site is shown to be entirely located within Flood Zone 1 (Low Probability). The nearest Flood Zone extents are located approximately 390m south west of the site, associated with the River Mersey and Chorlton Platt Gore. Environment Agency data shows the site to be elevated approximately 4.03m above the 1 in 100-year event and 3.92m AOD in the 1 in 100-year + 30% climate change event flood levels, therefore fluvial flood risk is considered to be low.

A drainage ditch is shown to run through the centre of the site from east to west and is thought to be culverted to the west of the site. A pluvial flow route is shown to run through the site, attributed to the ditch. However, existing levels prevent water from flowing through the ditch and the ditch is therefore not considered to be an active drainage feature.

The site is considered to be at a medium risk of groundwater flooding, and it is recommended that finished floor levels of any buildings are suitably raised above surrounding ground levels to mitigate the groundwater flood risk identified. There is also potential for groundwater to be encountered during construction. Where significant groundwater is encountered within excavations alternative dewatering systems will need to be employed. The advice of a suitably experienced groundwater contractor should be sought to determine the most viable option.

The proposed development has also been assessed against a further range of potential risk sources including surface water, canals, reservoirs and sewers. The site is considered to be at a low risk from these sources.

To mitigate the developments impact on the current runoff regime it is proposed to appropriately manage surface water and foul water separately, in order to ensure flood risk in the wider area is not increased. At this stage it is proposed to store storm water runoff within above ground detention basins and discharge surface water from the site to the existing surface water sewer running through the site. United Utilities have confirmed that foul water will be allowed to drain to the public combined sewer network, with the preferred connection to be to the existing 225mm public combined sewer in Ryebank Road, to the north of the site.

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1. INTRODUCTION

- 1.1 This Flood Risk Assessment (FRA) and Conceptual Drainage Strategy has been prepared on behalf of Manchester Metropolitan University to support divestment of their asset at Ryebank Fields, Chorlton.
- 1.2 A Development Framework document has been produced on behalf of Manchester Metropolitan University to inform future proposals. The framework has been endorsed by Manchester City Council's Executive, to guide and assist the assessment of future planning applications for the site.
- 1.3 This FRA and Conceptual Drainage Strategy is not intended to support a planning application and as such the level of detail included is commensurate and subject to the nature of the proposals.

Table 1.1: Site Summary

Site Name	Ryebank Road
Location	Chorlton
NGR (approx.)	SJ810945
Application Site Area (ha)	4.68 (approx.)
Development Type	Residential
Flood Zone Classification	Flood Zone 1
NPPF Vulnerability	More Vulnerable
Environment Agency Office	Greater Manchester, Merseyside and Cheshire
Lead Local Flood Authority	Manchester City Council
Local Planning Authority	Manchester City Council

Sources of Data

- i. Topographical Survey by CT Surveys, reference [Dwg No. 4692AB/1-5]
- ii. OS Explorer Series mapping
- iii. Environment Agency consultation
- iv. Manchester City Council Consultation
- v. Local Authority Surface Water Flood Risk Maps
- vi. Ryebank Road Development Framework document
- vii. Manchester City, Salford City and Trafford Council Strategic Flood Risk Assessment
- viii. Manchester City Council Preliminary Flood Risk Assessment

- ix. Site visit undertaken by BWB Consulting Ltd
- x. Manchester City Council Local Flood Risk Management Strategy
- xi. Ground Investigations undertaken by e3p, reference [13-355-R2-DRAFT and 13-533-R1-1]
- xii. CCTV Survey undertaken by Drain Alert Ltd
- xiii. Utility Assessment Report undertaken by Fuel Solutions UK Limited
- xiv. United Utilities Sewer Records
- xv. United Utilities Pre- Development Enquiry
- xvi. British Geological Survey Drift & Geology Maps

Existing Site

- 1.4 The site is located on the land of Ryebank Fields, Ryebank Road, Chorlton, approximately 4.3km south west of Manchester. The site is bound to the north and east by residential development, to the south by Longford Road and to the west by Longford Park. The existing site comprises open space with an area of hardstanding to the south of the site. The site's location is illustrated within **Figure 1.1**.

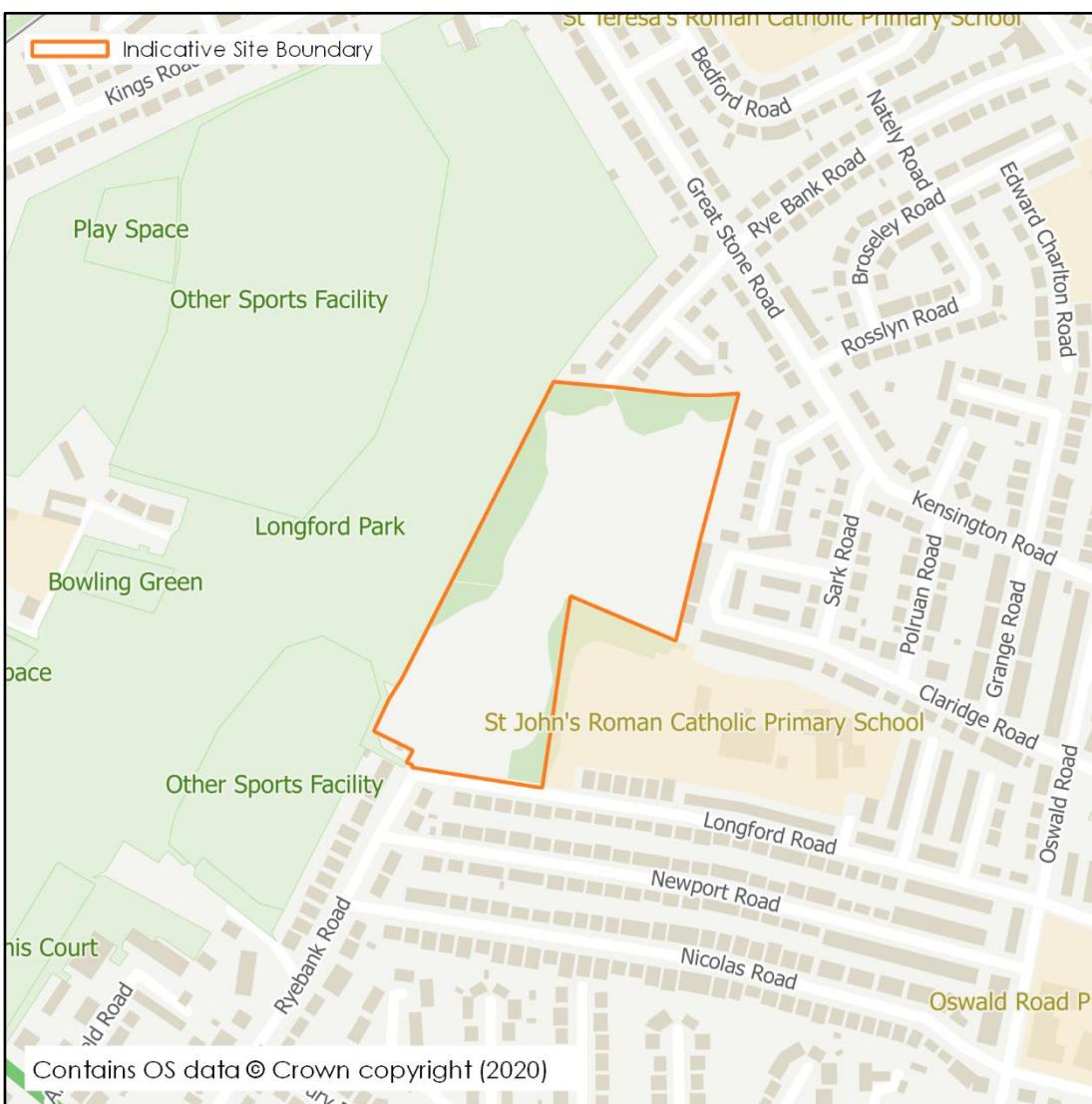


Figure 1.1: Site Location

- 1.5 A topographic survey has been undertaken and is included as **Appendix 1**. The site is shown to be relatively flat with a gentle fall in a north westerly direction. Site levels are shown to range from approximately 26.20m Above Ordnance Datum (AOD) in the north west to 28.62m AOD in the south. A ditch is shown to run through the centre of the site in a westerly direction with levels ranging from 27.11m AOD at the top of the banks to 26.20m AOD at the bottom of the banks. Parts of the site have not been surveyed due to densely overgrown vegetation.

Proposed Development

- 1.6 It is expected that the proposed development will be residential, split into two parcels in the north and south, comprising approximately 120 properties and including detached, semi-detached and terraced properties along with associated open space, as informed by the Development Framework. An extract of the Development Framework is included as **Appendix 2**. Proposed plans are not available at the time of writing.

2. LOCAL GUIDANCE

Strategic Flood Risk Assessment

- 2.1 A Strategic Flood Risk Assessment (SFRA) is a study carried out by one or more local planning authorities to assess the risk to an area from flooding from all sources, now and in the future.
- 2.2 The Manchester City, Salford City and Trafford Councils Level 1 SFRA¹ has been reviewed in the production of this FRA. The SFRA provides information specific to the site location in the form of fluvial, surface water and groundwater flood risk mapping, as well as records of historic flooding. Information from the Level 1 SFRA will be referenced within **Section 3.0** where applicable.
- 2.3 The Manchester City, Salford City and Trafford Councils Level 2 SFRA² was produced to facilitate the application of Sequential and Exception Tests to screen allocated development sites. The proposed application site is not referenced within the Level 2 SFRA. Information from the Level 2 SFRA will be referenced within **Section 3.0** where applicable.

Preliminary Flood Risk Assessment

- 2.4 A Preliminary Flood Risk Assessment (PFRA) is an assessment of floods that have taken place in the past and floods that could take place in the future. It generally considers flooding from surface water runoff, groundwater and ordinary watercourses, and is prepared by the Lead Local Flood Authorities.
- 2.5 The Manchester City Council PFRA³ considers flooding from surface water runoff, groundwater, ordinary watercourses and canals. However, no historical instances of flooding at the site are referenced. Information from the PFRA will be referenced within this report where applicable.

Local Flood Risk Management Strategy

- 2.6 A Local Flood Risk Management Strategy (LFRMS) is prepared by a Lead Local Flood Authority to help understand and manage flood risk at a local level. The LFRMS aims to ensure that the knowledge of local flood risk issues is communicated effectively so that they can be better managed. The LFRMS also aims to promote sustainable development and environmental protection.
- 2.7 The Manchester City Council LFRMS⁴ has been reviewed and will be referenced within this report where applicable.

¹ Level 1 Strategic Flood Risk Assessment (Manchester City, Salford City and Trafford Council, 2010)

² Level 2 Strategic Flood Risk Assessment (Manchester City, Salford City and Trafford Council, 2011)

³ Preliminary Flood Risk Assessment (Manchester City Council, 2011)

⁴ Local Flood Risk Management Strategy (Manchester City Council, 2014)

Greater Manchester Strategic Flood Risk Management Framework

- 2.8 The Greater Manchester Strategic Flood Risk Management Framework (SFRMF)⁵ aims to provide a spatial framework for flood risk management that highlights the key strategic flood risks including cross-boundary issues and recommends key priorities for intervention.
- 2.9 The Greater Manchester SFRMF has been reviewed and will be referenced within the report where applicable.

Development Framework

- 2.10 A Development Framework⁶ has been prepared on behalf of Manchester Metropolitan University for Ryebank Road, Chorlton, Manchester. The role of the framework is to define the broad parameters that future development can be set within. It does not provide detailed design of the development project; this is something that will be dealt with through individual planning applications.
- 2.11 The Framework aims to provide the conditions to bring forward a high quality residential neighbourhood in keeping with its surrounding context whilst supporting a new residential product that will enhance the existing residential offer of Chorlton and the wider South Manchester market.

⁵ Greater Manchester Strategic Flood Risk Management Framework (Greater Manchester Combined Authority, 2018)

⁶ Development Framework, Manchester Metropolitan University (2019)

3. POTENTIAL SOURCES OF FLOOD RISK

- 3.1 Flooding can occur from a variety of sources, or combination of sources, which may be natural or artificial. **Table 3.1** below identifies the potential sources of flood risk to the site in its current condition, and the impacts which the development could have in the wider catchment, prior to mitigation. These are discussed in greater detail in the forthcoming section. The mitigation measures proposed to address flood risk issues and ensure the development is appropriate for its location are discussed within **Section 4.0**.

Table 3.1: Pre-Mitigation Sources of Flood Risk

Flood Source	Potential Risk				Description
	High	Medium	Low	None	
Fluvial			X		The site is entirely located in Flood Zone 1. Longford Brook is located approximately 150m north west of the site.
Coastal				X	The site is shown to be located away from any coastal/tidal influence.
Canals			X		The site is shown to be located away from the Bridgewater Canal.
Groundwater		X			The site is shown to fall within an area predicted to be at a medium susceptibility to groundwater flooding.
Reservoirs and waterbodies			X		The site is shown to fall within the area at risk of reservoir failure. However, maintenance and safety checks mean this only poses a low residual risk to the site.
Pluvial runoff			X		The site is shown to be largely at a very low risk of pluvial flooding with a pluvial flow route shown to run through the centre of the site, associated with the existing ditch.
Sewers			X		The surface water sewer running through the site is of sufficient depth that it is not thought to pose a flood risk.
Effect of Development			X		Development will not result in impedance/loss of surface water route.

Flood Source	Potential Risk				Description
	High	Medium	Low	None	
on Wider Catchment		X			The development will increase the area of impermeable surfaces leading to a potential increase in runoff.

Fluvial Flood Risk

- 3.2 Flooding from watercourses occurs when flows exceed the capacity of the channel, or where a restrictive structure is encountered, which leads to water overtopping the banks into the floodplain. This process can be exacerbated when debris is mobilised by high flows and accumulates at structures.
- 3.3 The site is shown to be located entirely within Flood Zone 1, as shown in **Figure 3.1**. The nearest Environment Agency (EA) Main River is Longford Brook, located approximately 150m north west of the site in Longford Park and flows in a north westerly direction. The watercourse is assumed to be culverted with no Flood Zones associated with it within the vicinity of the site.

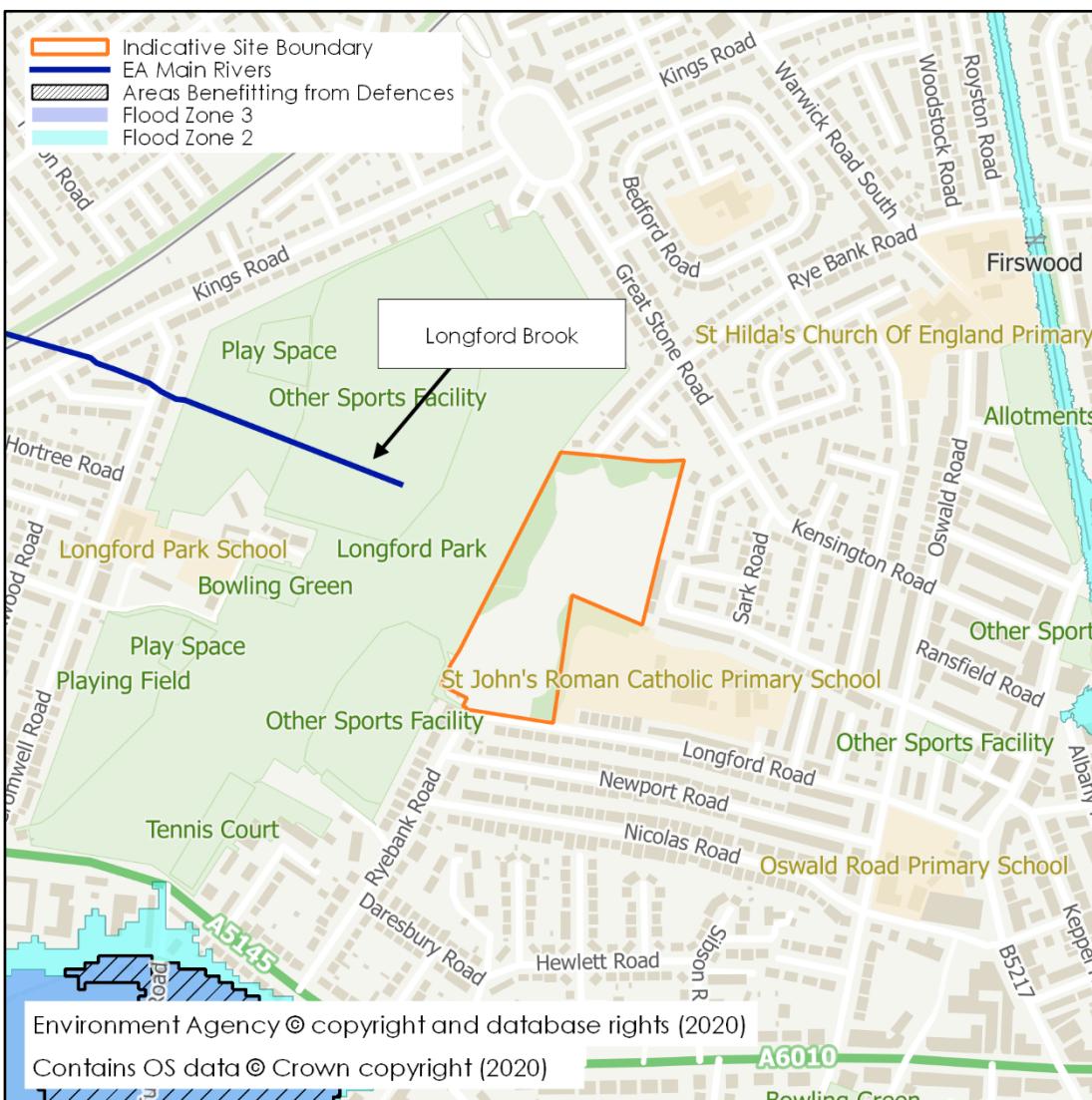


Figure 3.1: Flood Map for Planning

- 3.4 The nearest Flood Zone 2 and 3 extents are located approximately 390m south west of the site, associated with the River Mersey and Chorlton Platt Gore.
- 3.5 The EA were contacted to request flood information available at the site and surrounding area. The correspondence is included as **Appendix 3**. The Chorlton Platt Gore 2012 model data provided by the EA shows the modelled water level associated with Chorlton Platt Gore to be 24.59m AOD during the 1 in 100-year undefended and defended event at the nearest modelled node, located approximately 1km south east of the site. The modelled water level during the 1 in 100-year + 30% climate change event is shown to be 24.70m AOD in both the undefended and defended scenarios. The site is shown to be elevated approximately 4.03m above the 1 in 100-year event and 3.92m AOD in the 1 in 100-year + 30% climate change event.
- 3.6 The EA Historical Flood Map shows the nearest recorded flood outline to be located approximately 2.5km south east of the site, associated with a 1964 flood event. Correspondence with Manchester City Council as the LLFA (included as **Appendix 4**) notes no record of any flooding at the site or within the immediate vicinity of the site.

- 3.7 Based upon the distance and intervening topography, the site is considered to be at a low risk from fluvial sources.

Longford Brook/Nico Ditch

- 3.8 A drainage ditch is shown to pass through the centre of the site from east to west and is currently understood to serve a small catchment of approximately 1.4km. Historical mapping (referenced within the Phase I Geoenvironmental Site Assessment⁷) suggests that Longford Brook once passed through the site to the north of the existing ditch before being diverted along the eastern site boundary.
- 3.9 A site visit undertaken by BWB Consulting in October 2019 noted that the ditch is not well defined with large amounts of vegetation and no apparent flowing water, as shown in **Figure 3.2**. A site visit undertaken in November 2019, following vegetation clearance, also shows the ditch to be dry, with no flowing water, as shown in **Figure 3.3**. A further site visit, undertaken in March 2020, noted standing water within the ditch, as shown in **Figure 3.4**. The ditch is, therefore, not considered to be an active drainage feature due to the lack of flowing water within the feature.



Figure 3.2: Ditch, October 2019, Taken Facing East

⁷ Phase I Geoenvironmental Site Assessment, e3p (2020)



Figure 3.3: Ditch, November 2019, Taken Facing East



Figure 3.4: Ditch, March 2020, Taken Facing East

- 3.10 It is considered at this stage that there is no connectivity between the eastern boundary diversion of the Longford Brook and the drainage ditch, as the ditch was noted as not conveying any flows during a period of intense rainfall.
- 3.11 The ditch is understood to be culverted to the western boundary of the site and Longford Park, as shown in **Figure 3.5**. Standing water is shown to be present at the entrance to the culvert, implying that the culvert may be blocked, preventing the flow of water away from the site. Further investigation of the culvert is required in order to understand the connectivity of the ditch to the west of the site and confirm any connectivity to the Longford Brook. Recommendations for further investigation is included within **Section 5.0**.



Figure 3.5: Culvert, Taken Facing West

- 3.12 Overall, the fluvial flood risk surrounding the existing ditch is considered to be low.

Coastal/Tidal Flood Risk

- 3.13 Inundation of low lying coastal areas by the sea may be caused by seasonal high tides, storm surges and storm driven wave action. Coastal/Tidal flooding is most commonly a result of a combination of two or more of these mechanisms, which can result in the overtopping or breaching of sea defences. River systems may also be subject to tidal influences.
- 3.14 The site is shown to be located away from any coastal/tidal influence. Therefore, there is no risk of coastal/tidal flooding at the site.

Flood Risk from Canals

- 3.15 The Canal and River Trust (CRT) generally maintains canal levels using reservoirs, feeders and boreholes and manages water levels by transferring it within the canal system.
- 3.16 Water in a canal is typically maintained at predetermined levels by control weirs. When rainfall or other water enters the canal, the water level rises and flows out over the weir. If the level continues rising it will reach the level of the storm weirs. The control weirs and storm weirs are normally designed to take the water that legally enters the canal under normal conditions. However, it is possible for unexpected water to enter the canal or for the weirs to become obstructed. In such instances the increased water levels could result in water overtopping the towpath and flowing onto the surrounding land.
- 3.17 Flooding can also occur where a canal is impounded above surrounding ground levels and the retaining structure fails.
- 3.18 The Bridgewater Canal is located approximately 1km west of the site. The LFRMS identifies the site to be located outside of the canal breach zone and the SFRA notes limited canal overtopping in Manchester with overtopping events unlikely to affect areas beyond the canal towpath. Therefore, the flood risk associated with canals is thought to be low.

Groundwater Flood Risk

- 3.19 Groundwater flooding occurs when the water table rises above ground elevations. It is most likely to happen in low lying areas underlain by permeable geology. This may be regional scale chalk or sandstone aquifers, or localised deposits of sands and gravels underlain by less permeable strata such as that in a river valley.
- 3.20 According to British Geological Survey (BGS) mapping, the area is shown to be underlain by the Wilmslow Sandstone Formation, which is designated as a Principal Aquifer. Principal Aquifers are layers of rock or drift deposits that have high intergranular and/or fracture permeability, meaning they usually provide a high level of water storage. They may support water supply and/or base flow on a strategic scale.
- 3.21 Superficial deposits of Till (Devensian) are expected to be present throughout the majority of the site with a small portion to the north western corner of the site expected to be underlain by Glaciofluvial Sheet Deposits (Devensian). The superficial deposits are designated Secondary (undifferentiated) and Secondary A Aquifers.
 - i. Secondary (undifferentiated) Aquifers are assigned in cases where it has not been possible to attribute either Category A or B to a rock type.
 - ii. Secondary A Aquifers are permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers.
- 3.22 Ground investigations carried out by e3p in December 2019 encountered Made Ground deposits in all exploratory hole locations to a maximum depth of 12.70m Below Ground Level (BGL).

- 3.23 Site investigations including 30 trial pits and 16 borehole logs have been undertaken by e3p. Groundwater was encountered within 24 trial pits between depths of 1.4m BGL and 2.9m BGL. Several of the trial pits had to be terminated due to rapid groundwater ingress, particularly within the south of the site. Groundwater was encountered within 8 borehole logs between depths of 2.1m BGL and 4.3m BGL. Further information can be found within the Phase II Geoenvironmental Site Assessment⁸.
- 3.24 The Manchester City Council PFRA notes that the site is located within an area at risk of groundwater rebound and the LFRMS shows the site to be located within an area at risk of groundwater flooding; however, the SFRA notes no known records of groundwater flooding in Manchester and correspondence with the LLFA notes no record of flooding within the site.
- 3.25 A site visit undertaken by BWB Consulting in October 2019 noted muddy ground conditions, but no standing water appeared to be present. A further site visit in March 2020 also noted muddy ground conditions but with standing water pooling in the rutted areas, following a period of rainfall.
- 3.26 The proposed development is considered to be at a medium risk of groundwater flooding.
- 3.27 In the event of groundwater flooding, due to the relatively flat nature of the site, water is likely to pool across the site. Additionally, whilst there is scope for groundwater to rise through such deposits, in some cases, impermeable surfaces at the site would act as a barrier to groundwater emergence.
- 3.28 Mitigation measures are required to reduce the potential risk of groundwater flooding, especially during construction. Further details on which are included within **Section 5.0**.

Flood Risk from Reservoirs & Large Waterbodies

- 3.29 Flooding can occur from large waterbodies or reservoirs if they are impounded above the surrounding ground levels or are used to retain water in times of flood. Although unlikely, reservoirs and large waterbodies could overtop or breach leading to rapid inundation of the downstream floodplain.
- 3.30 To help identify this risk, reservoir failure flood risk mapping has been prepared, this shows the largest area that might be flooded if a reservoir were to fail and release the water it holds. The map displays a worst case scenario and is only intended as a guide. An extract from the mapping is included as **Figure 3.6**.

⁸ Phase II Geoenvironmental Site Assessment, e3p (2020)



Figure 3.6: Reservoir Failure Flood Risk Map

- 3.31 The site is shown to be located within the potential failure flood extent of up to four reservoirs, this could pose a hazard to the site in its existing conditions.
- 3.32 The EA data (included as **Appendix 3**) confirmed the residual risk of reservoir flooding to the site. The reservoirs are owned and operated by United Utilities Water plc, who have the ultimate responsibility for the safety of their reservoir assets. Their responsibilities include regular safety inspections, any necessary design or repairs undertaken where required and an annual statement produced on the operation and maintenance regime.
- 3.33 Based on the safety legislation in place and the maintenance and repair responsibilities of the reservoir owners, the actual probability of a significant failure is considered to be low. Therefore, the risk of flooding at the site from this source is also considered to be low.

Pluvial Flood Risk

- 3.34 Pluvial flooding can occur during prolonged or intense storm events when the infiltration potential of soils, or the capacity of drainage infrastructure is overwhelmed leading to the accumulation of surface water and the generation of overland flow routes.
- 3.35 Risk of flooding from surface water mapping has been prepared, this shows the potential flooding which could occur when rainwater does not drain away through the normal drainage systems or soak into the ground but lies on or flows over the ground instead. An extract from the mapping is included as **Figure 3.7**.

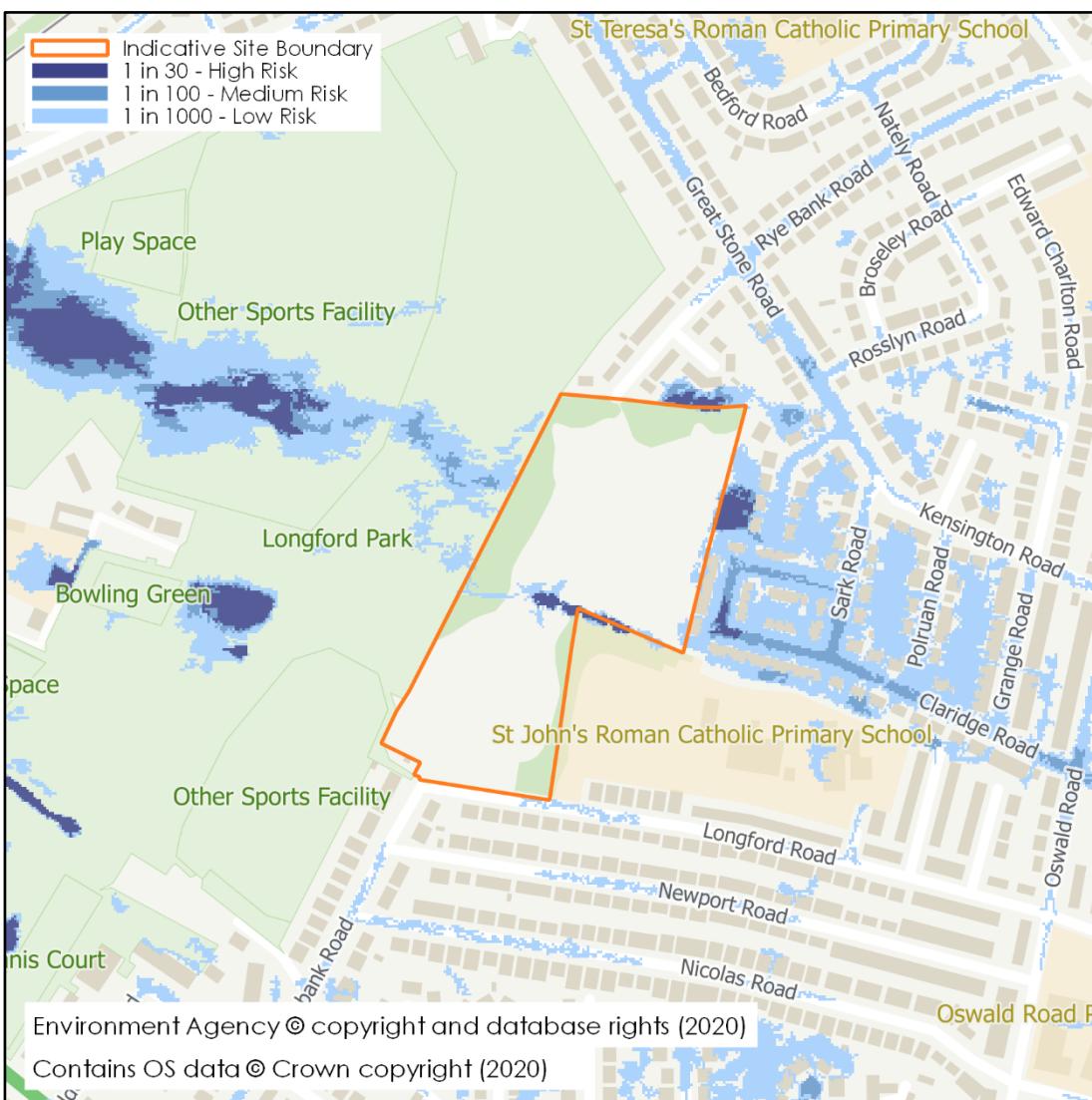


Figure 3.7: Surface Water Flood Risk Map

- 3.36 The site is largely shown to be at a very low risk of surface water flooding. A pluvial flow route is shown to flow through the centre of the site from east to west presenting a Low (1 in 1000-year) to High (1 in 30-year) risk of surface water flooding. This is thought to be associated with the ditch on site and is shown to be generally constrained. Due to existing levels of the ditch, water cannot flow and collects as standing water as a result of prolonged rainfall, as shown by the small areas of high susceptibility.

- 3.37 A large area of low to high surface water flood risk is shown to be present to the north east and east of the site. To the south of the site, Ryebank Road and Longford Road are largely shown to be at a very low risk of surface water flooding, therefore safe/dry access and egress is achievable.
- 3.38 The Greater Manchester SFRMF identifies surface water flood risk as a general issue for Greater Manchester, however it does not provide details on the exact areas deemed to be affected.
- 3.39 The overall flood risk to the site from pluvial sources is thought to be low.

Flood Risk from Sewers

- 3.40 Sewer flooding can occur when the capacity of the infrastructure is exceeded by excessive flows, or as a result of a reduction in capacity due to collapse or blockage, or if the downstream system becomes surcharged. This can lead to the sewers flooding onto the surrounding ground via manholes and gullies, which can generate overland flows.
- 3.41 The local sewerage undertaker is United Utilities Water. A copy of their sewer records is included as **Appendix 5**.
- 3.42 A public surface water sewer with a construction comprising of brick, a diameter of 1275mm and an average depth to invert of approximately 5.40m is shown to be present in the south western corner of the site. An overflow pipe with a diameter of 1500mm is located to the north of the site and is known to flow from the north where it serves a wider area, the depth of the pipe is not known. There are a number of public foul water sewers located to the north, east and south of the site, which serve the existing residential development.
- 3.43 A CCTV survey and investigation of the public surface water sewer were undertaken by Drain Alert (included as **Appendix 6**) in order to understand the connectivity between any sewers on site. Die testing and sounding out proved the connectivity of the surface water sewer in the south west of the site to the overflow sewer to the north of the site. Therefore, the public surface water sewer is shown to run through the entire site as shown in **Figure 3.8**. The location of the sewer is approximate and based on site investigations and existing United Utilities Sewer Records.
- 3.44 There is a need for any future development to consider the sewer by means of an easement. Further details are included within **Section 5.0**.
- 3.45 In the unlikely event of sewer flooding, water will follow local topography and may pool in places due to the relatively flat nature of the topography or flow in a north westerly direction.
- 3.46 Due to the depth of the sewer within the site, flood risk from sewers is considered to be low.

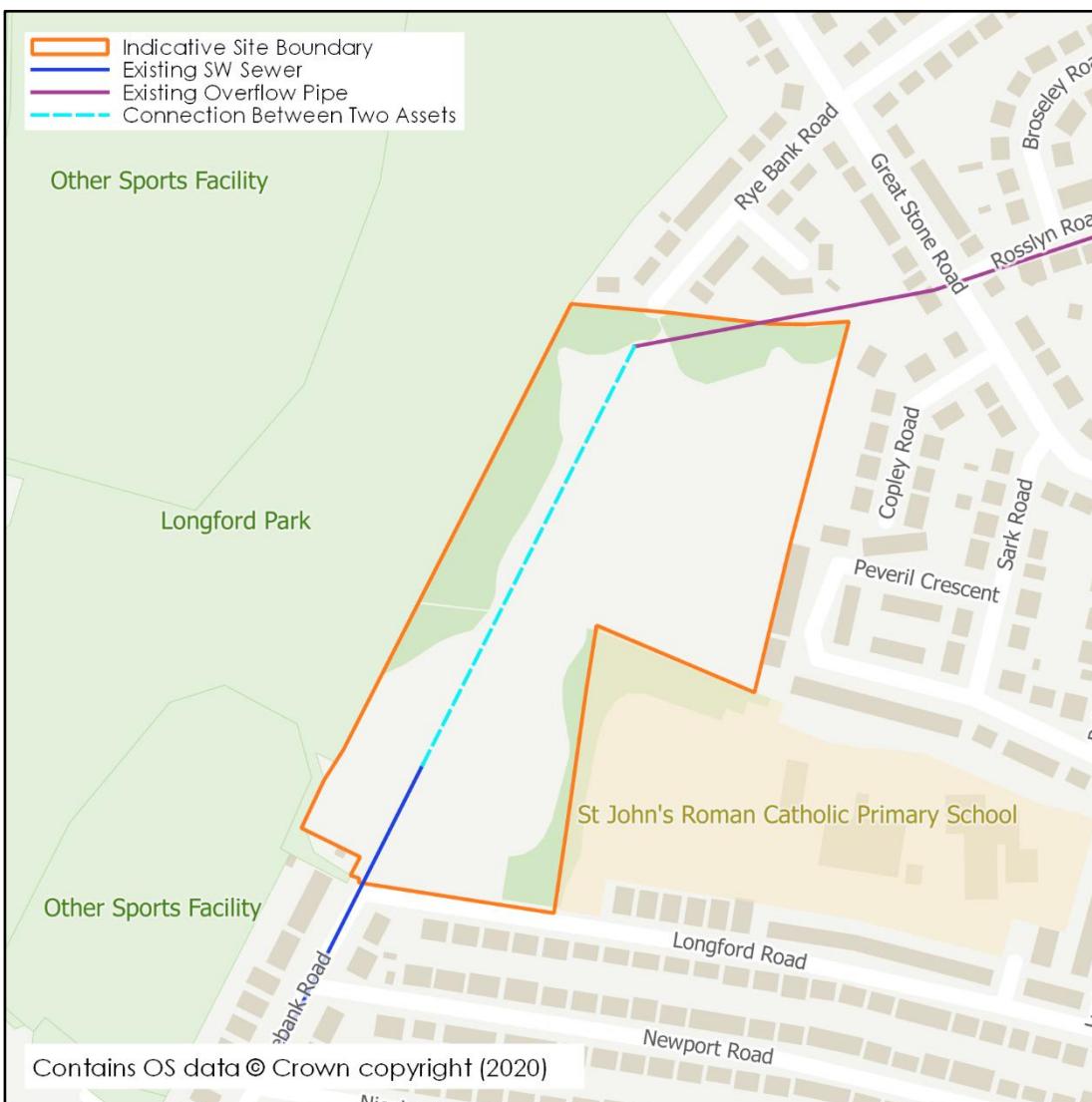


Figure 3.8: Approximate Location of United Utilities Sewerage Assets (taken from United Utilities Sewer Records and Drain Alert Site Investigations)

Effect of Development on Wider Catchment

Impediment of Flood Flows

- 3.47 The Development Framework suggests that development will be split into two parcels in the north and south of the site with green infrastructure located along the western site boundary and along the alignment of the existing ditch. Therefore, development will not impede any pluvial flow routes through the site.

Development Land Use

- 3.48 The proposed development will increase the area of impermeable surfaces on site. This will result in an increase in surface water runoff, which could increase flood risk downstream unless properly mitigated.

4. DRAINAGE

Surface Water

Drainage Requirements

- 4.1 Local and national policy requires the use of Sustainable Drainage Systems (SuDS) principles for new developments, which would necessitate consideration of the infiltration potential of the ground, in the first instance, as a means of surface water disposal, followed by discharge to a local waterbody. An allowance of attenuation and treatment of surface water will also be required.
- 4.2 Manchester City Council, in their role as LLFA, have published a SuDS Requirements for New Developments Update 2019⁹ which advises that SuDS are designed in line with the national Non-Statutory Technical Standards for SuDS¹⁰. This guidance has been used to inform this assessment and is included as **Appendix 4**.
- 4.3 Typically, an allowance for urban creep, to account for residents increasing the impermeable areas through driveways and patios etc, should be included and usually depends on the density of the residential development. Given the early and conceptual stage of this surface water drainage strategy, no allowance has been included for urban creep. Once an indicative masterplan is available, a more detailed strategy that includes an appropriate allowance for urban creep should be produced. Typically, an additional 10% of the proposed impermeable area would be required to accounted for as part of the site drainage.

Existing Conditions

- 4.4 The total site is approximately 4.68ha. The developable area, as measured by the two indicative catchments, is approximately 2.99ha.
- 4.5 The runoff rate, per hectare, for the site has been estimated using the IH124 method, with appropriate prorated adjustments for a site of less than 50ha. This was undertaken within Micro Drainage, which makes the necessary adjustments for small sites automatically. The results are summarised within **Table 4.1** and included as **Appendix 7**.

Table 4.1: Existing Greenfield Runoff Rates from the Site

Return Period (Yrs.)	Runoff Rate (l/s/ha)
1	2.0
Mean Annual Flow Rate (QBAR)	2.3
30	3.9
100	4.8

⁹ SuDS Requirements for New Developments, Manchester City Council (2019)

¹⁰ Sustainable Drainage Systems Non-statutory technical standards for sustainable drainage systems, DEFRA (2015)

Drainage Hierarchy

- 4.6 The Planning Policy Guidance¹¹ and the SuDS Manual identify that surface water runoff from a development should be disposed of as high up the following hierarchy as reasonably practicable:
- i. Into the ground (infiltration);
 - ii. To a surface water body;
 - iii. To a surface water sewer, highway drain, or another drainage system;
 - iv. To a combined sewer.
- 4.7 In-situ (falling) head permeability tests were undertaken by e3p as part of the Phase II investigations, within two environmental monitoring wells. As the site is underlain by Made Ground impacted by low-level inorganic and hydrocarbon compounds, soakaway drainage is not considered to be suitable for the proposed development.
- 4.8 A drainage ditch is shown to run through the centre of the site with a culvert located to the western site boundary. The ditch is not understood to be an active drainage feature with the existing levels preventing water from flowing within the ditch. At this stage, the ditch is not considered to be a suitable receiving body for surface water runoff.
- 4.9 Due to the presence of the surface water sewer running through the entire site, it is proposed to discharge surface water from the site to this asset following development. This is subject to confirmation from United Utilities.

Surface Water Drainage Strategy

- 4.10 The site has been split into two drainage catchments, with the existing drainage ditch separating the two parcels, in line with the Development Framework.
- 4.11 The Non-statutory technical standards for sustainable drainage systems requires that the peak runoff rate from the development to any highway drain, sewer or surface water body for the 1 in 100-year rainfall event should never exceed the peak greenfield runoff rate from the same event.
- 4.12 The greenfield runoff rate for each catchment has been calculated using the catchment area and the existing greenfield rate of 2.3l/s/ha. The results are shown in **Table 4.2**. The greenfield runoff rate for Catchment 2 has been increased to 3l/s in order to reduce the potential risk of blockage of flow restriction devices, this increased runoff rate has been used to calculate the attenuated storage requirement for the catchment.

¹¹ Planning Practice Guidance. <http://planningguidance.planningportal.gov.uk/>

Table 4.2: Restricted Discharge Rate per Catchment

Catchment No.	Catchment Area (ha)	Greenfield Runoff Rate (l/s)
1	1.95	4.5
2	1.04	2.4

- 4.13 A simulation has been run using Micro Drainage ‘Source Control’ to identify the necessary storage provision for each catchment. Using a restriction of 4.5l/s for Catchment 1 and 3l/s for Catchment 2, and a proposed impermeable area of 1.27ha for Catchment 1 and 0.68ha for Catchment 2, the volume of attenuated storage required for the development has been calculated for storm events up to the 1 in 100-year + 40% climate change storm. The results are summarised in **Table 4.3** and included as **Appendix 7**.

Table 4.3: Total Attenuated Storage Requirement per Catchment

Catchment No.	Critical Storm	Maximum Volume (m ³)
1	1440 min Winter	1019.4
2	960 min Winter	511.2

- 4.14 The catchment-based approach will treat and attenuate the surface water runoff as close to its source as possible. Storm water runoff from each catchment will be stored within an above ground detention basin. The water will then discharge at a controlled rate via a vortex flow control or similar device, into the receiving surface water network.
- 4.15 Water quality should be considered within the drainage strategy to capture any potential pollutants in the runoff from the development. The detention basins will provide the primary stage of treatment. However, to supply a minimum two stage treatment train, further features such as conveyance swales, bioretention areas and pervious pavements should be incorporated into the development and included within the detailed design. Rainwater harvesting measures should also be considered as a method of recycling surface water and reducing/delaying surface water entering the surrounding network.
- 4.16 As part of any future planning application, this conceptual drainage design should be developed into a more detailed drainage strategy which should be created alongside the masterplan to ensure that a suitable area is designated for SuDS in line with local and national guidance.
- 4.17 A Conceptual Surface Water Drainage Strategy is included as **Appendix 8** (Drawing reference: MMU-BWB-ZZ-XX-SK-CD-0001) and demonstrates how the required storage could be achieved within each catchment.

Foul Water

- 4.18 United Utilities have confirmed via a Pre-development Enquiry (included as **Appendix 9**), that foul will be allowed to drain to the public combined sewer network. Their preferred
- 4.19 connection would be to the existing 225mm public combined sewer in Ryebank Road, to the north of the site.
- 4.20 A formal application will have to be made to United Utilities in order to make a formal sewer connection.

5. RECOMMENDATIONS AND FURTHER WORKS

- 5.1 This report has been produced on behalf of Manchester Metropolitan University to support divestment of their asset at Ryebank Fields, Chorlton (approximate grid reference: SJ810945).
- 5.2 Any future planning application at the site will require a site-specific Flood Risk Assessment (FRA) and Sustainable Drainage Statement (SDS) to be produced.

Flood Risk

- 5.3 The site has been identified to fall within Flood Zone 1 (Low Probability). A drainage ditch is shown to run through the centre of the site from east to west; however, this is not considered to be an active drainage feature due to the lack of flowing water. The ditch is culverted to the west of the site, further investigation of the connectivity of the culvert to the west of the site is recommended.
- 5.4 An overland flow route is shown to be present through the centre of the site, attributed to the drainage ditch. Mitigation would be required to ensure obstructions are not placed over natural overland flow routes.
- 5.5 The site has been identified to be at a medium risk of groundwater flooding. It is recommended that finished floor levels of any buildings are suitably raised above surrounding ground levels to mitigate the groundwater flood risk identified. There is also potential for groundwater to be encountered during construction. Where significant groundwater is encountered within excavations alternative dewatering systems will need to be employed. The advice of a suitably experienced groundwater contractor should be sought to determine the most viable option.
- 5.6 Due to the presence of the surface water sewer passing through the site there is a need for any future development to consider this by means of an easement, where no built development or planting can take place. Initial correspondence from United Utilities would suggest that such a sewer would require an easement of 3m either side of the sewer centre line. However, due to its depth and diameter it is thought there is the potential for an easement of circa 8m either side of the sewer. Further clarification has been sought from United Utilities on this matter, at the time of writing a response is yet to be received.
- 5.7 The site has also been assessed against a range of potential flood risk sources including surface water, sewers, canals and reservoirs. None of these flood sources are thought to represent a potential barrier to development.

Surface Water Drainage

- 5.8 Due to the increase in impermeable area and the resulting implication upon the surface water runoff regime, a Sustainable Drainage Strategy which incorporates SuDS into the development in line with the latest guidance would be required.


- 5.9 It is proposed at this stage to discharge surface water from the site to the existing surface water sewer running through the site. This is subject to confirmation from United Utilities.
- 5.10 A catchment-based approach has been applied and the site has been divided into two catchments based on the Development Framework. This approach aims to treat and attenuate the surface water runoff as close to its source as possible. Storm water runoff from each catchment will be stored within above ground detention basins. The water will then discharge at the equivalent greenfield rate up to the 1 in 100-year storm with a 40% allowance for climate change via a vortex flow control into the receiving surface water network.

Foul Water

- 5.11 United Utilities have confirmed in their Pre-Development Enquiry response that foul will be allowed to drain to the public combined sewer network. Their preferred connection would be to the existing 225mm public combined sewer in Ryebank Road, to the north of the site.
- 5.12 A formal application will have to be made to United Utilities, at the appropriate juncture in order to make a formal sewer connection.

APPENDICES

APPENDIX 1: Topographic Survey



ABBREVIATIONS

AR	ASSUMED ROUTE	OSA	OUTSIDE SURVEY AREA
AV	AIR VALVE	P	POST POLE
BP	BALL VALVE	PL	PIPE LINE
BH	BORSHOLE	RE	RODING EYE
BT	BURIED TELECOM CHAMBER	BS	BUSS BAR
CATV	CABLE TV	RW	RIVETED WIRE
CL	COVER LEVEL	SC	STOP COCK
CO	COOLER	SV	STOP VALVE
DP	DOWN PIPE	SP	SPRING PIPE
EP	ELECTRICITY POLE	TC	TRAFFIC CONTROL
FH	FIRE HYDRANT	TR	TOP ROD
FL	FLAT FENCE LEVEL	TRR	TAKEN FROM RECORDS
FR	FLAT ROOF LEVEL	TL	TRAFFIC LIGHTS
IC	INSPECTION CHAMBER	UTR	UNABLE TO READ
GPR	TRACED BY G.P.R.	UTA	UNABLE TO GAIN ACCESS
JP	JUNCTION POINT	UTB	UNABLE TO BURROW
JB	JUNCTION BOX	UTR	UNABLE TO RAISE
LP	LADDER POSITION	UTT	UNABLE TO TRACE
LB	LITTER BIN OR LETTER BOX	VP	VENT PIPE
MB	METER BOX	WAL	WALL LEVEL
MM	MANHOLE	WP	WASH OUT VALVE
NT	NOT SURVEYED	WV	WATER PIPE
b	BEAM HEIGHT	c	CILL HEIGHT
ch	CHIMNEY HEIGHT	dh	DOOR HEIGHT
dp	DOWNPIPE HEIGHT	fl	FLOOR HEIGHT
fsl	FLOOR LEVEL	hd	HIGH DOOR LEVEL
hd	HIGH DOOR LEVEL	hh	HEAD HEIGHT
rp	RIDGE BOARD HEIGHT	sp	SPRING PIPE
sp	SPRING POINT	st	STEPS UP

FENCE LINETYPES

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SURVEY NOTES:
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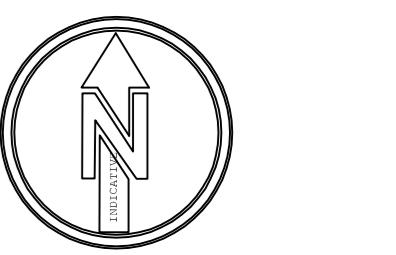
SURVEY REVISIONS
PREVIOUS DWG NO: -
REVISION NO: -
ORIGINAL SURVEY DATE: -

REVISION NOTES:


LAYOUT VIEWS

Cushman & Wakefield
PROJECT TITLE:
Land off Ryebank Rd/Langford Rd,
Manchester, M21 9WW

DRAWING NUMBER:
4692AB/2



SCALE: 1:200 @ A0 SURVEY DATE: November 2019
LEVEL DATUM: OSGB by GPS GRID SYSTEM: OSGB fixed at ST06
BENCH MARK USED: Nail at ST06 surveyed by GPS BM VALUE: 28.167m SHEET NUMBER: 2 of 5



ABBREVIATIONS

AR	ASSUMED ROUTE	OSA	OUTSIDE SURVEY AREA
AV	AIR VALVE	OP	OUTLINE POLE
BH	BORSHOLE	P	POST POLE
BT	BURIED TELECOM CHAMBER	RE	RODDED EYE
CATV	CABLE TV	RS	RUNNING PIPE
CL	COVER LEVEL	SC	STOP COCK
CO	COVER	SV	STOP VALVE
DR	DOWN PIPE	SVF	STOP VALVE FOR
EP	ELECTRICITY POLE	TC	TRAFFIC CONTROL
FH	FIRE HYDRANT	TR	TAKEN FROM
FL	FLAT FENCE LEVEL	TRR	TAKEN FROM RECORDS
FR	FLAT ROOF LEVEL	TL	TRAFFIC LIGHTS
IC	INSPECTION CHAMBER	UTR	UNABLE TO RASE
GPR	TRACED BY G.P.R.	UTA	UNABLE TO GAIN ACCESS
JB	JUNCTION BOX	UTB	UNABLE TO BURROW
LW	LITTER BIN OR LETTER BOX	UTC	UNABLE TO CANCEL
MB	MANHOLE	UTT	UNABLE TO TRACE
NC	NOT CANCELLED	VP	VENT PIPE
NT	NOT TRACED	WV	WASH OUT VALVE
b	BEAM HEIGHT		
c	CILL HEIGHT		
dH	DOOR HEIGHT		
dh	DRAUGHT HEIGHT		
fL	FLOOR LEVEL		
hL	HALL LEVEL		
HL	HEAD HEIGHT		
r	RIDGE BOARD HEIGHT		
SP	SPRING POINT		
ST	STEPS UP		

FENCE LINETYPES

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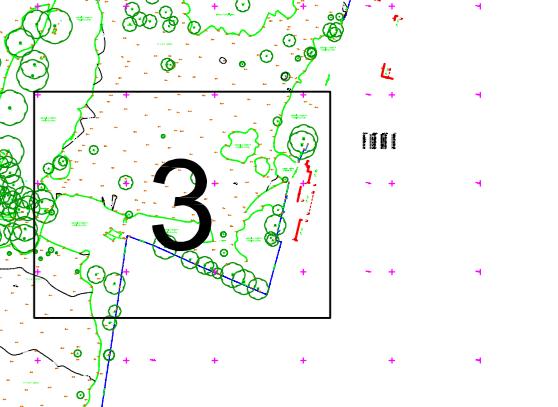
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REVISION NO:	-
ORIGINAL SURVEY DATE:	-

REVISION NOTES:

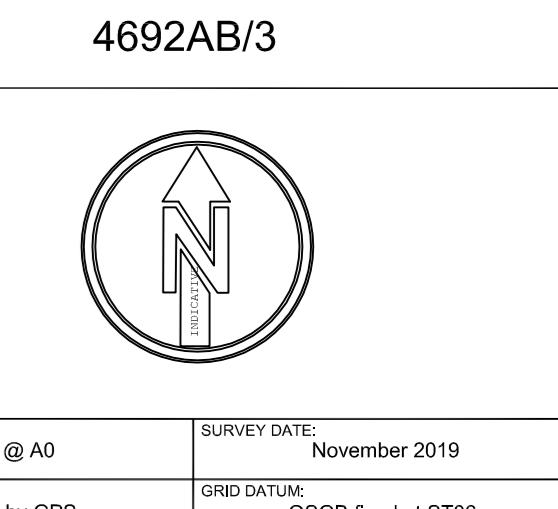
LAYOUT VIEWS



Cushman & Wakefield

PROJECT TITLE:
Land off Ryebank Rd/Langford Rd,
Manchester, M21 9WW

DRAWING NUMBER:
4692AB/3



SCALE: 1:200 @ A0 SURVEY DATE: November 2019
LEVEL DATUM: OSGB by GPS GRID SYSTEM: OSGB fixed at ST06
BENCH MARK USED: Nail at ST06 surveyed by GPS BM VALUE: 28.167m SHEET NUMBER: 3 of 5

ABBREVIATIONS

AR	ASSUMED ROUTE	OSA	OUTSIDE SURVEY AREA
AV	AIR VALVE	P	POST POLE
BH	BORHOLE	R	RODDED EYE
BH	BORHOLE	RS	RODDED SIGHT
BT	BURIED TELECOM CHAMBER	RW	RIVETED PIPE
CATV	CABLE TV	SP	STOPPIPE
CL	COVER LEVEL	SV	STOP COCK
CR	CROSS ROD	SV	STOP VALVE
DR	DOWN PIPE	TC	TRAFFIC CONTROL
EP	ELECTRICITY POLE	TR	TOP RECORD
FH	FIRE HYDRANT	TRR	TAKEN FROM RECORDS
FL	FLAT ROOF LEVEL	TH	THICKNESS
FR	FLAT ROOF LEVEL	TL	TRAFFIC LIGHTS
GR	GRAVEL	TR	TRANSITION
GPR	TRACED BY G.P.R.	UTR	UNABLE TO GET
IC	INSPECTION CHAMBER	UVR	UNABLE TO RAISE
JB	JUNCTION BOX	UVL	UNABLE TO LOWER
LW	LIDGE	UTR	UNABLE TO TRACE
LB	LITTER BIN OR LETTER BOX	VP	VENT PIPE
MB	MANHOLE	W	WASH OUT VALVE
NT	NOT SURVEYED	WP	WATER PIPE

b BEAM HEIGHT
c CILL HEIGHT
d DOOR HEIGHT
e FLOOR HEIGHT
f FLOOR LEVEL
g GROUND ADJUST LEVEL
h HEAD HEIGHT
i RIDGE BOARD HEIGHT
j SPANNING POINT
k STEPS UP

FENCE LINETYPES

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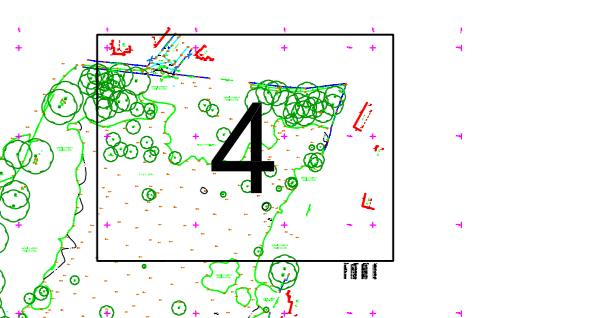


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REVISION NO: -
ORIGINAL SURVEY DATE: -

REVISION NOTES:

LAYOUT VIEWS




Cushman & Wakefield

PROJECT TITLE:
Land off Ryebank Rd/Langford Rd,
Manchester, M21 9WW

DRAWING NUMBER:
4692AB/4



SCALE: 1:200 @ A0 SURVEY DATE: November 2019
LEVEL DATUM: OSGB by GPS GRID SYSTEM: OSGB fixed at ST06
BENCH MARK USED: Nail at ST06 surveyed by GPS BM VALUE: 28.167m SHEET NUMBER: 4 of 5



ABBREVIATIONS

AR	ASSUMED ROUTE	OSA	OUTSIDE SURVEY AREA
AV	ARM VALVE	P	POST POLE
CA	CABLE	R	RODDED EYE
BH	BORSHOLE	RE	RODDED EYE
BT	BURIED TELECOM CHAMBER	BS	BUICKS PIPE
CATV	CABLE TV	RW	RIVETED PIPE
CL	COVER LEVEL	SC	STOP COCK
CO	COVER	SV	STOP VALVE
DP	DOWN PIPE	SP	STOPPIPE
EP	ELECTRICITY POLE	TC	TRAFFIC CONTROL
FH	FIRE HYDRANT	TR	TAKEN FROM RECORDS
FL	FLAT FENCE LEVEL	TH	THREE HORN
FR	FLAT ROOF LEVEL	TL	TRAFFIC LIGHTS
GR	GRADIENT	UTR	UNABLE TO READ
GPR	TRACED BY G.P.R.	UTA	UNABLE TO ACCESS
IC	INSPECTION CHAMBER	UTR	UNABLE TO RAISE
JB	JUNCTION BOX	UVR	UNABLE TO VERIFY
LP	LITTER BIN	VP	VENT PIPE
MN	MANHOLE	WV	WASH OUT VALVE
NT	NOT TRACED	WP	WATER PIPE

→ STATION POINTS & WALL LEVEL

b	BEAM HEIGHT
c	CILL HEIGHT
dH	DOOR HEIGHT
dh	DOOR HEIGHT
fl	FLOOR HEIGHT
fL	FLOOR LEVEL
hL	HALL LEVEL
hA	HEAD HEIGHT
r	ROOF
rd	ROOF DRAIN
RP	ROOF PITCH
SP	SPRING POINT
STP	STATION POINT
↑	STATION POINTS & WALL LEVEL

FENCE LINETYPES

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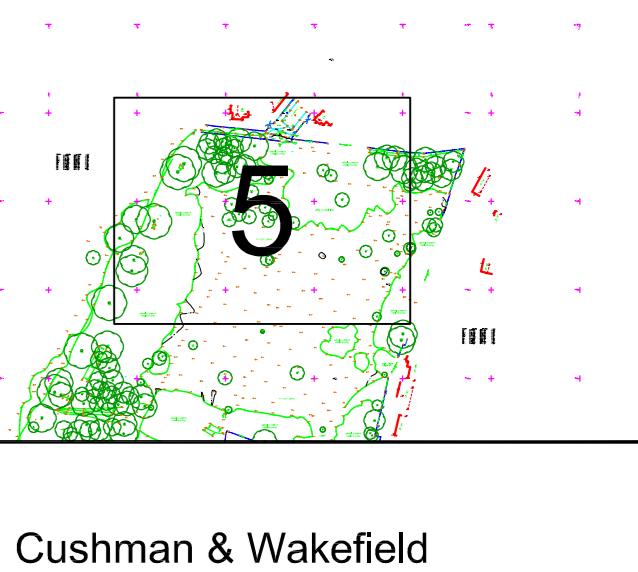


SURVEY NOTES:
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SURVEY REVISIONS
PREVIOUS DWG NO:
REVISION NO:
ORIGINAL SURVEY DATE:

REVISION NOTES:

LAYOUT VIEWS



Cushman & Wakefield

PROJECT TITLE:
Land off Ryebank Rd/Langford Rd,
Manchester, M21 9WW

DRAWING NUMBER:
4692AB/5



SCALE: 1:200 @ A0 SURVEY DATE: November 2019
LEVEL DATUM: OSGB by GPS GRID SYSTEM: OSGB fixed at ST06
BENCH MARK USED: Nail at ST06 surveyed by GPS BM VALUE: 28.167m SHEET NUMBER: 5 of 5

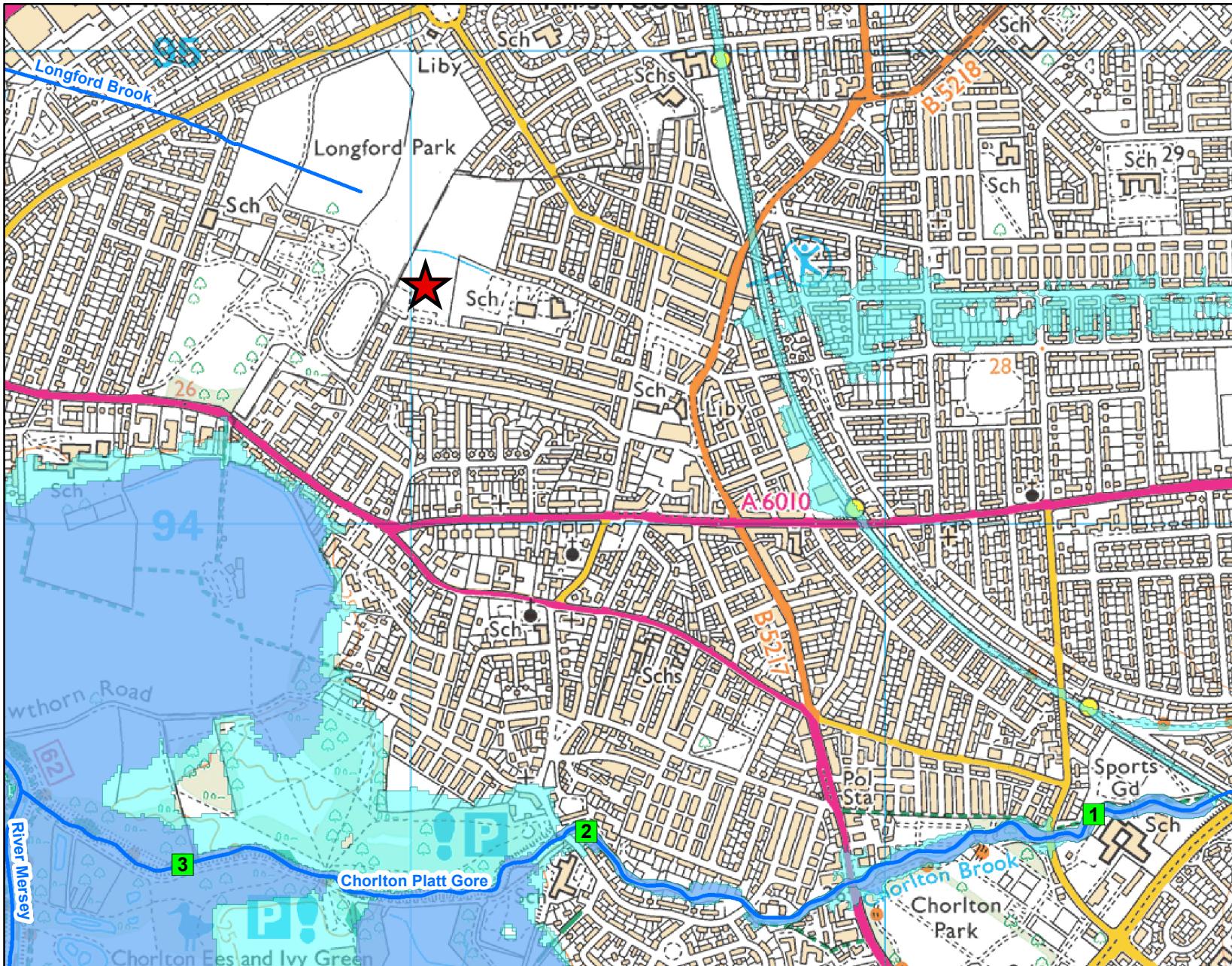
APPENDIX 2: Development Framework

3.2 Summary of Opportunities



APPENDIX 3: EA Correspondence

Detailed Flood Map centred on Ryebank Road, Chorlton, M21 9SN. Created on 31/10/2019 [GMMC147886AB]



1:12,001



Legend

- Site Location (Red Star)
- Model measurements (Green Square)
- Main River (Blue Line)
- Flood Zone 3 (Blue Shaded Area)
- Flood Zone 2 (Cyan Shaded Area)

Map Reference	Model Node Reference	Easting	Northing	Data	Undefended								Defended							
					10 % AEP (1 in 10 year)	4 % AEP (1 in 25 year)	1.33 % AEP (1 in 75 year)	1 % AEP (1 in 100 year)	1 % AEP (1 in 100 year) + 30yr Climate Change	1 % AEP (1 in 100 year) + 35yr Climate Change	1 % AEP (1 in 100 year) + 70yr Climate Change	0.1 % AEP (1 in 1000 year)	10 % AEP (1 in 10 year)	4 % AEP (1 in 25 year)	1.33 % AEP (1 in 75 year)	1 % AEP (1 in 100 year)	1 % AEP (1 in 100 year) + 30yr Climate Change	1 % AEP (1 in 100 year) + 35yr Climate Change	1 % AEP (1 in 100 year) + 70yr Climate Change	0.1 % AEP (1 in 1000 year)
1	ea013_Model_CHOP01_00034	382443	393388	Modelled Water Level (m aodN)	26.03	26.39	26.78	26.86	27.29	27.35	27.46	27.48	26.03	26.39	26.79	26.87	27.33	27.37	27.46	27.49
				Modelled Flow (cumecs)	10.79	13.53	16.12	16.67	19.52	19.99	21.02	21.18	10.79	13.54	16.14	16.68	19.83	20.19	21.12	21.24
2	ea013_Model_CHOP01_00054	381376	393354	Modelled Water Level (m aodN)	24.44	24.51	24.58	24.59	24.70	24.71	24.75	24.78	24.44	24.51	24.58	24.59	24.70	24.72	24.75	24.78
				Modelled Flow (cumecs)	10.87	13.30	15.63	16.06	18.56	18.92	20.15	20.79	10.87	13.31	15.65	16.07	18.80	19.09	20.23	20.99
3	ea013_Model_CHOP01_00062	380518	393278	Modelled Water Level (m aodN)	24.34	24.35	24.36	24.36	24.39	24.39	24.40	24.38	24.34	24.35	24.36	24.36	24.39	24.39	24.40	24.38
				Modelled Flow (cumecs)	8.99	10.79	12.55	12.85	11.30	11.54	12.39	16.63	8.99	10.80	12.56	12.86	11.44	11.64	12.43	16.79

Model data taken from Chorlton Platt Gore 2012

AEP - Annual Exceedence Probability

m aodn - metres above ordnance datum Newlyn

cumecs - cubic metres per second

Notes: *Climate Change Scenario - 30%, 35% and 70% increases in flow calculated for the 2080's (2070 - 2115). Please see <https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances> for more information regarding the new climate change guidance. The location of the site and the type (vulnerability) of development determine the climate change allowances to consider in any flood risk assessment.

Reservoir Flood Map



The area within the red circle could be at risk of flooding from the following reservoirs:

Reservoir Name	Reservoir Owner	Location	Local Authority	Environment Agency Office
Gorton Upper	United Utilities Water plc	389893, 396254	Manchester	Environment Agency - Greater Manchester, Merseysid
Audenshaw No. 3	United Utilities Water plc	390934, 396835	Tameside	Environment Agency - Greater Manchester, Merseysid
Audenshaw No.2	United Utilities Water plc	391643, 396093	Tameside	Environment Agency - Greater Manchester, Merseysid
Audenshaw No.1	United Utilities Water plc	391852, 396483	Tameside	Environment Agency - Greater Manchester, Merseysid

 Maximum extent of flood

1:10,000

Reservoir Flood Map

This text must be read with the extract from the Reservoir Flood Map which we have sent to you

How to use the maps

Reservoir flood maps are available to help you find out if you could be affected by reservoir flooding. Even though reservoir flooding is very unlikely it may be helpful to you to find out if you live or work in an area that could be affected. If you do, you might want to think about what you would do if an emergency did happen.

For more information on what to do if you live or work near a reservoir, including some frequently asked questions, visit our website at <http://www.environment-agency.gov.uk/flood>.

The maps have been prepared for emergency planning purposes and for this reason they reflect a credible worst case scenario – this means that if a reservoir failure did occur it would most likely be far less severe than the scenario shown in the maps. We've mapped a credible worst case scenario so that emergency planners have all the information they might need to increase public safety.

Reservoir safety

Reservoirs in the UK have an extremely good safety record with no failures resulting in the loss of life since 1925. Reservoirs are more carefully maintained now. This means reservoir flooding is very unlikely to happen.

The Environment Agency is the enforcement authority for the Reservoirs Act 1975 in England. All large reservoirs that we think could endanger human life must be inspected and supervised by reservoir engineers. We ensure that reservoirs are regularly inspected and essential safety work is carried out.

For more information on reservoir safety visit our website at:

<https://www.gov.uk/guidance/reservoirs-owner-and-operator-requirements>.

Emergency planning

Lead Local flood authorities are responsible for coordinating emergency plans for reservoir flooding and ensuring communities are well prepared. Lead Local flood authorities work with other members of the Local Resilience Forum (LRF) to develop generic and site-specific emergency plans, depending on local circumstances and priorities.

If you want to find out about local emergency plans you should contact the responsible lead local flood authority as identified on the map.

APPENDIX 4: Manchester City Council Correspondence

Lucy Reeves

From: [REDACTED] on behalf of floodriskmanagement@manchester.gov.uk
Sent: 30 October 2019 12:13
To: Lucy Reeves
Cc: floodriskmanagement@manchester.gov.uk
Subject: Re: Request for Flood Risk Information (Ryebank Road, Chorlton)
Attachments: We found suspicious links; Indicative Site Boundary.png

Good afternoon

We do not have record of any flooding on the site or in immediate vicinity of the site.
From the OS maps it is visible that there is a drain that crosses the site - its connectivity and function needs to be investigated as part of the flood risk assessment.
Information on sewer flooding could be obtained from United Utilities.
information on surface water and ground water flooding and mapping of their extent is available from the Environment Agency website.

We are attaching guidance for developers on requirements related to surface water management that you might useful if you are also developing a drainage strategy for the site.

Regards

Flood Risk Management
Highways Service, Growth and Neighbourhoods Directorate
Manchester City Council
Tel: 0161 219 6295
Email: floodriskmanagement@manchester.gov.uk
Web: <http://www.manchester.gov.uk/>

POSTAL ADDRESS: Manchester City Council, Public Realm, The Neighbourhood Service, PO Box 532, Town Hall,
Manchester M60 2LA

Lead Local Flood Authority - Manchester City Council

SuDS Requirements for New Developments Update 2019

The Government has strengthened planning policy on the provision of sustainable drainage systems (SuDS) for 'major' planning applications which is being introduced from 6 April 2015 (Paragraph 103 of National Planning Policy Framework and Ministerial Statement on SuDS).

Changes were made to the Town and Country Planning Policy and Guidance to give Local Authority Planning Departments the responsibility for ensuring that new developments are drained in a sustainable way, through the planning process, in consultation with the Lead Local Flood Authorities. As per the guidance issued by the Department of Communities and Local Government (DCLG), all 'major' planning applications being determined from 6 April 2015, must consider sustainable drainage systems.

Decisions about the suitability of sustainable drainage provision are made by the Local Planning Authority. However, under the new consultation arrangements Manchester City Council, in its role as Lead Local Flood Authority, is a statutory consultee for all major applications with regards to sustainable drainage. All 'major' planning applications submitted from 15 April 2015 are required to include a Surface Water Drainage Statement.

Developers submitting the planning application to Manchester City Council are advised to:

- Assess the suitability of sustainable drainage systems in accordance with: paragraphs 051, 079 and 080 of the revised NPPF Planning Practice Guidance (PPG) for Flood Risk and Coastal Change.
- Design sustainable drainage systems in line with national Non-Statutory Technical Standards for SuDS: <https://www.gov.uk/government/publications/sustainable-drainage-systems-non-statutory-technical-standards> and local policies DM1, EN08, EN14 and EN17 of the Core Strategy.
- Implement strategy that supports surface water runoff control as near to its source as possible through a sustainable drainage approach to surface water management, preferably through green types of SuDS wherever practicable.
- Maximise use of Green Infrastructure to manage surface water runoff (sustainable drainage systems – SuDS) as feasibility of use of green types of SuDS will be evaluated by LLFA during planning application stage. Even on sites where infiltration is not possible due to the unfavourable soil conditions, utilisation of attenuating types of SuDS should be maximised as part of green infrastructure in order to bring surface water runoff peak and volume reduction but also provide additional environmental and social benefits in line with Policies DM1, EN08, EN14 and EN17 of the Core Strategy. Developers are encouraged to look into CIRIA SuDS Manual 2015 for details of these systems to manage surface water runoff.
- Assess surface water attenuation requirements that offer a reduction in surface water runoff rate in line with the 'Manchester City, Salford City and Trafford Councils Level 2 Hybrid SFRA, User Guide, Final, dated May 2010'. (Section 3.4.1 Critical Drainage Areas, page 31). Please note that all new connections to the watercourses shall comply with reduction of flows to Greenfield runoff rates.

http://www.manchester.gov.uk/downloads/download/3871/strategic_flood_risk_assessment-manchester_salford_trafford

- Submit hydraulic calculations and drawings to support the design along with proposed standards of operation and maintenance in accordance with paragraph 081 of NPF (PPG).
- It will be essential that the type of sustainable drainage system for a site, along with details of its extent/position, is identified at the design stage of the whole scheme. This information will be required for both outline and full applications so it is clearly demonstrated that the SuDS can be accommodated within the development that is proposed. It will no longer be acceptable to leave the design of SuDS to a later stage to be dealt with by planning conditions.

Applicants are strongly advised to discuss their proposals with relevant planning officers at the pre-application stage to ensure that an acceptable SuDS scheme is submitted.

Minimum requirements for approval

In order to avoid objection, the following three elements of evidence are required:

- Surface Water Management Statement for the site is submitted that is in line with requirement of NPPF PPG Paragraph 079:
 - “New development should only be considered appropriate in areas at risk of flooding if priority has been given to the use of green types of sustainable drainage systems as a primary mean of surface water management that could be accompanied by traditional attenuation systems if volumes required. Additionally, and more widely, when considering major development, as defined in the Town and Country Planning (Development Management Procedure) (England) Order 2015, sustainable drainage systems should be provided unless demonstrated to be inappropriate”.
 - The strategy should be accompanied by at least an outline layout of the proposed drainage systems with space allocation for the proposed attenuation in line with relevant flow reduction requirements.
- Evidence that drainage hierarchy has been applied in line with NPPF PPG Paragraph 080. The aim should be to discharge surface run off as high up the following hierarchy of drainage options as reasonably practicable:
 - into the ground (infiltration);
 - to a surface water body;
 - to a surface water sewer, highway drain, or another drainage system;
 - to a combined sewer.
- Evidence that one viable solution for draining the site is secured and agreed in principle with relevant authority:
 - Results of ground investigation carried out under Building Research Establishment Digest 365. Site investigations should be undertaken in locations and at proposed depths of the proposed infiltration devices. Proposal of the attenuation that is achieving half emptying time within 24 hours. If no ground

investigations are possible or infiltration is not feasible on site, evidence of alternative surface water disposal routes (as follows) is required.

- Where surface water is connected to Main River, any works within or adjacent to the river that would affect it would require consent from Environment Agency. An email of acceptance of proposed flows and/or new connection will suffice.
- Where surface water is connected to the public sewer, agreement in principle from United Utilities is required that there is adequate spare capacity in the existing system taking future development requirements into account. An email of acceptance of proposed flows and/or new connection will suffice.

Where surface water is connected to the ordinary watercourse, agreement in principle from Manchester City Council as Lead Local Flood Authority is required. Please note that all new connections to the watercourses shall comply with reduction of flows to Greenfield runoff rates. An email of acceptance of proposed flows and/or new connection will suffice.

- Where surface water is connected to ordinary watercourse, any works within or adjacent to the watercourse that would affect it would require consent from Manchester City Council as Lead Local Flood Authority. Consent forms can be obtained on the website at:

http://www.manchester.gov.uk/downloads/download/5567/land_drainage_consent_and_guidance.com

Avoiding or discharging drainage condition

- Evidence of consideration of green SuDS solution wherever practicable and viable in line with NPPF PPG Paragraph 051. SuDS should be designed to:
 - reduce the causes and impacts of flooding;
 - remove pollutants from urban run-off at source;
 - combine water management with green space with benefits for amenity, recreation and wildlife wherever practicable.
- For Greenfield site developments, details of surface water attenuation that offers a reduction in surface water runoff rate in line with the Manchester Trafford and Salford Strategic Flood Risk Assessment, i.e. to the Greenfield runoff rates;
- For Brownfield site developments, for all sites located within Conurbation Core Critical Drainage Areas, details of surface water attenuation that offers a reduction in surface water runoff rate in line with the Manchester Trafford and Salford Strategic Flood Risk Assessment, i.e. at least a 50% reduction in runoff rate compared to the existing rates with an aim to reduce to Greenfield runoff rates wherever practicable;
- Where surface water runoff is to be disposed through infiltration, details of surface water attenuation that is adequate for infiltration capacity of the underlying soil is required. This capacity has to be assessed through site investigation carried out in line with Building Research Establishment Digest 365. Site investigations should be undertaken in locations and at proposed depths of the proposed infiltration devices. Half emptying time of proposed attenuation within 24 hours should be achieved.

- Evidence that the drainage system has been designed (unless an area is designated to hold and/or convey water as part of the design) so that flooding does not occur during a 1 in 100 year rainfall event in any part of a building; The NPPF suggests that ‘for events with a return-period in excess of 30 years, surface flooding of open spaces such as landscaped areas or car parks is acceptable for short periods, but the layout and landscaping of the site should aim to route water away from any vulnerable property, and avoid creating hazards to access and egress routes. No flooding of property should occur as a result of a one in 100 year storm event (including an appropriate allowance for climate change)’.
- Assessment of overland flow routes for extreme events that is diverted away from buildings providing long and cross sections for the proposed drainage system and finished floor levels;
- Evidence that runoff volume in the 1 in 100 year, 6 hours rainfall shall be constrained to a value as close as is reasonably practical to the Greenfield runoff volume for the same event, but never to exceed the runoff volume from the development site prior to redevelopment;
- Hydraulic calculation of the proposed drainage system;
- Design construction drawings of proposed SuDS elements and flow controls;
- For sites where proposed development would cause unusual pollution risk to surface water (large car park areas (>50 parking spaces) or industrial estates), evidence of pollution control measures (preferably through SuDS) is required.
- Where an application is part of a larger site which already has planning permission it is essential that the new proposal does not compromise the drainage scheme already approved.

SuDS Maintenance

Maintenance of SuDS is essential for its proper operation and a clear management and maintenance plan for the lifetime of the development is required as part of the planning application.

In considering a development that includes a sustainable drainage system, Manchester City Council as local planning authority will want to be satisfied that the proposed minimum standards of operation are appropriate and that there are clear arrangements in place for ongoing maintenance. Information sought by Manchester City Council would be no more than necessary, having regard to the nature and scale of the development concerned in line with NPPF Paragraph 081.

In some instances where no clear adoption of the drainage system is proposed, a drainage maintenance condition could be attached to the planning applications. In order to discharge this condition, the following evidence should be provided following *construction*:

- Verification report providing photographic evidence of construction as per design drawings;
- As built construction drawings if different from design construction drawings;

- Management and maintenance plan for the lifetime of the development which shall include the arrangements for adoption by any public body or statutory undertaker, or any other arrangements to secure the operation of the sustainable drainage scheme throughout its lifetime.

Applicants are strongly advised to discuss their proposals with relevant planning officers at the pre-application stage to ensure that an acceptable SuDS scheme is submitted.

APPENDIX 5: Sewer Records

BWB Consulting Ltd

5th Floor, Waterfront House
Station Street,
Nottingham,
NG23DQ

FAO:

How to contact us:

United Utilities Water Limited
Property Searches
Haweswater House
Lingley Mere Business Park
Great Sankey
Warrington
WA5 3LP

Telephone: 0370 7510101

E-mail: propertysearches@uuplc.co.uk

Your Ref: MCW2136_POR028705
Our Ref: UUPS-ORD-131189
Date: 28/10/2019

Dear Sirs

Location: Ryebank Fields

I acknowledge with thanks your request dated 25/10/2019 for information on the location of our services.

Please find enclosed plans showing the approximate position of United Utilities' apparatus known to be in the vicinity of this site.

The enclosed plans are being provided to you subject to the United Utilities terms and conditions for both the wastewater and water distribution plans which are shown attached.

If you are planning works anywhere in the North West, please read United Utilities' access statement before you start work to check how it will affect our network. <http://www.unitedutilities.com/work-near-asset.aspx>.

I trust the above meets with your requirements and look forward to hearing from you should you need anything further.

If you have any queries regarding this matter please [contact us](#).

Yours Faithfully,



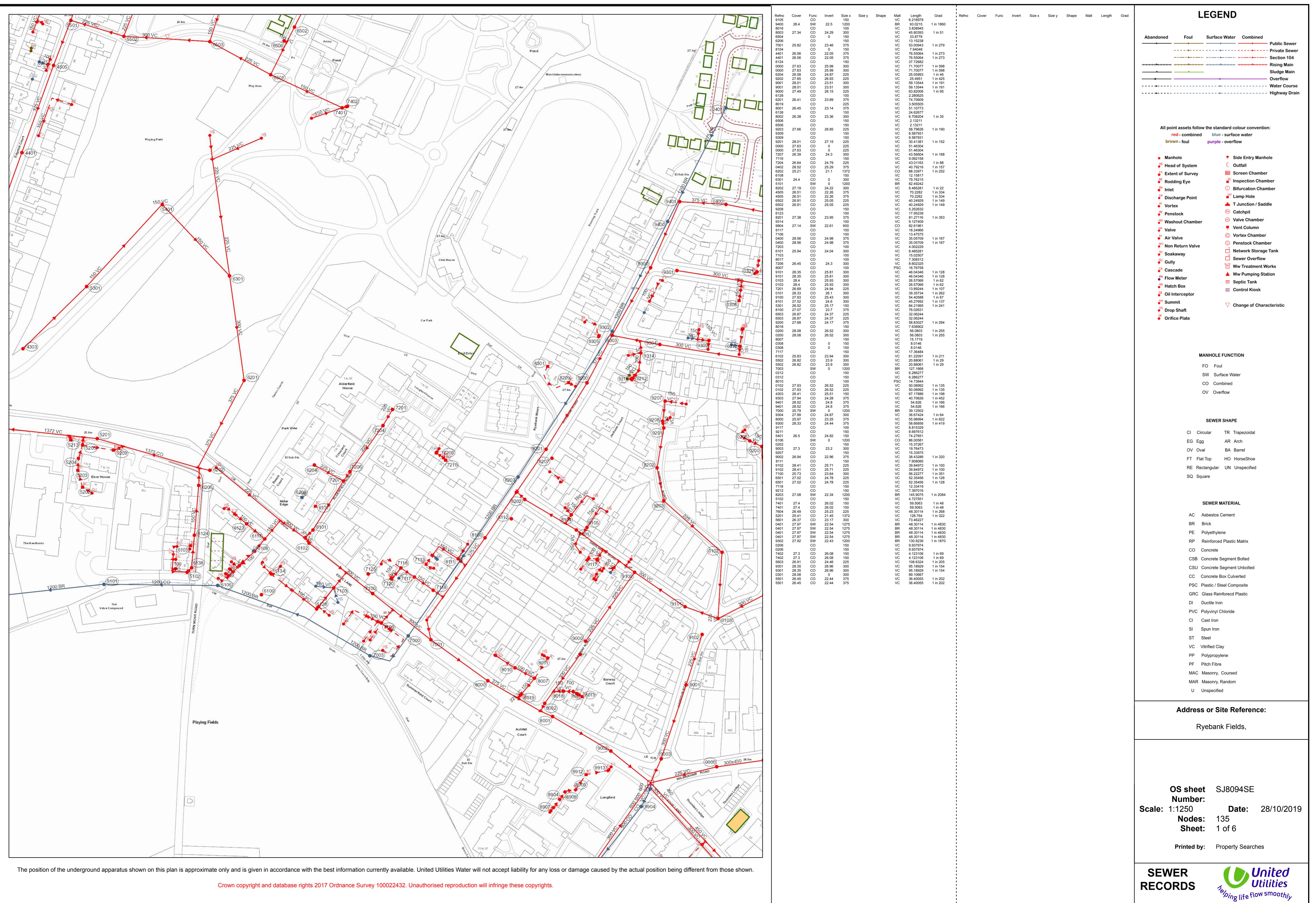
Karen McCormack
Property Searches Manager

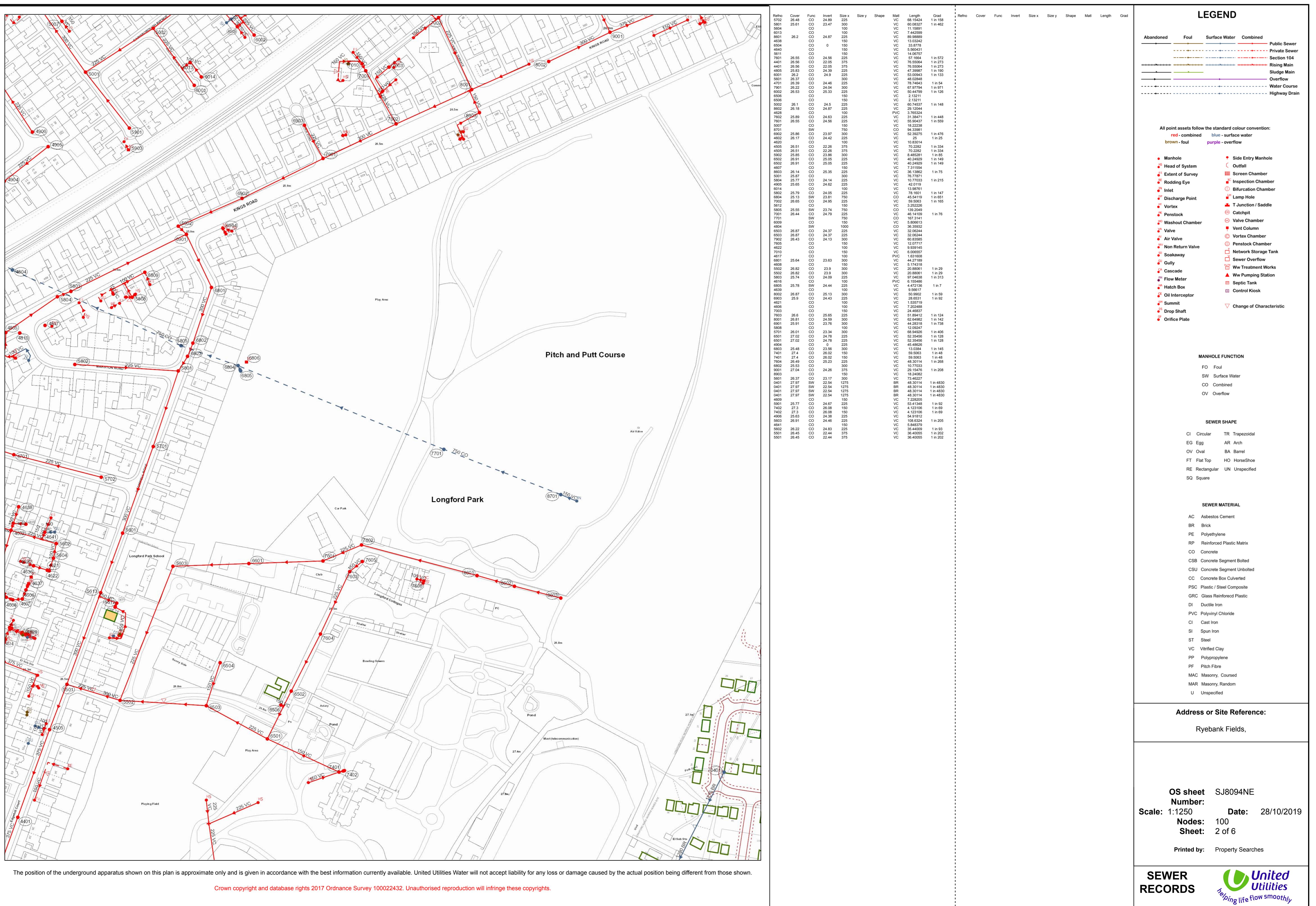
TERMS AND CONDITIONS - WASTEWATER AND WATER DISTRIBUTION PLANS

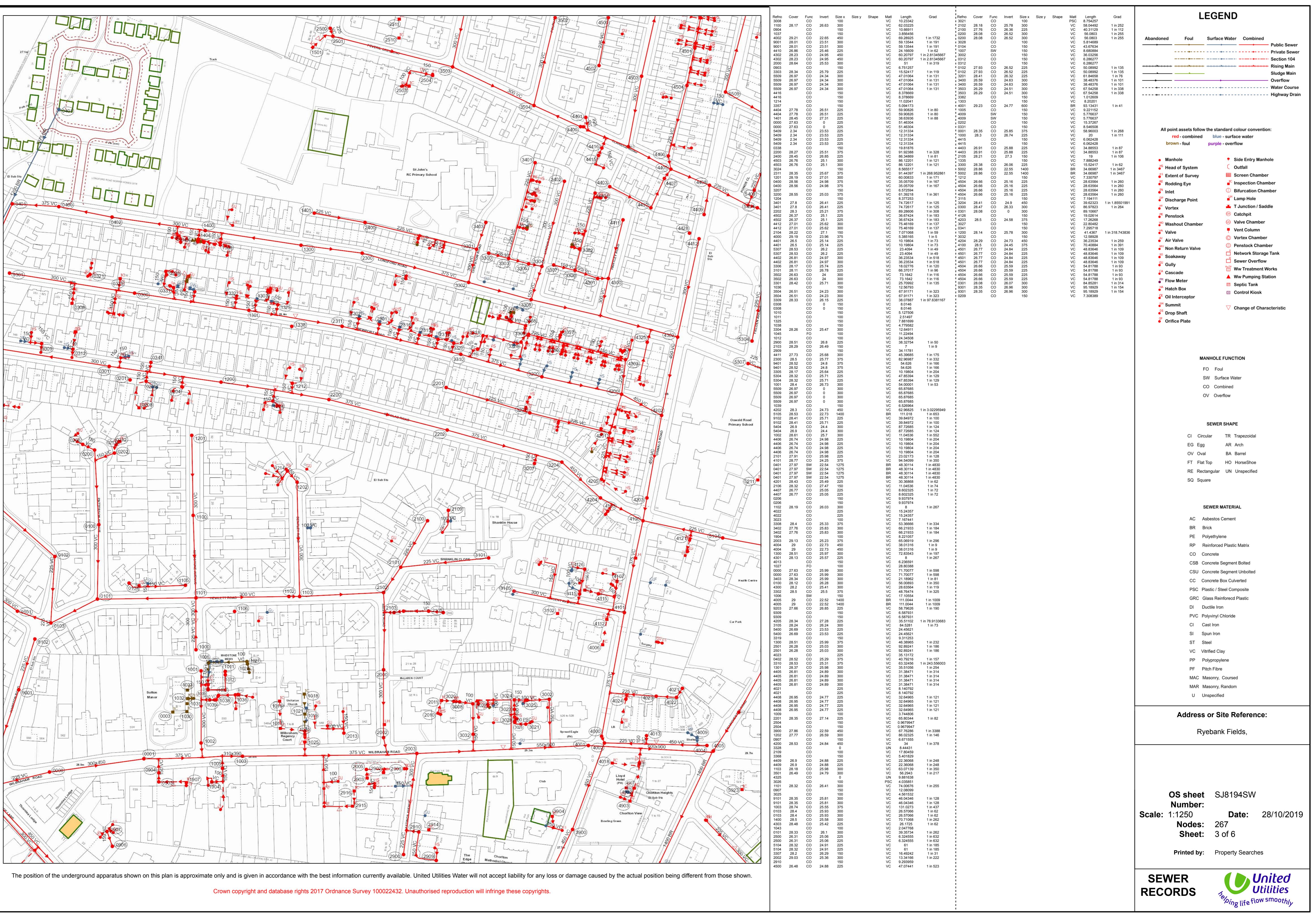
These provisions apply to the public sewerage, water distribution and telemetry systems (including sewers which are the subject of an agreement under Section 104 of the Water Industry Act 1991 and mains installed in accordance with the agreement for the self construction of water mains) (UUWL apparatus) of United Utilities Water Limited "(UUWL)".

TERMS AND CONDITIONS:

- This Map and any information supplied with it is issued subject to the provisions contained below, to the exclusion of all others and no party relies upon any representation, warranty, collateral contract or other assurance of any person (whether party to this agreement or not) that is not set out in this agreement or the documents referred to in it.
- This Map and any information supplied with it is provided for general guidance only and no representation, undertaking or warranty as to its accuracy, completeness or being up to date is given or implied.
- In particular, the position and depth of any UUWL apparatus shown on the Map are approximate only. UUWL strongly recommends that a comprehensive survey is undertaken in addition to reviewing this Map to determine and ensure the precise location of any UUWL apparatus. The exact location, positions and depths should be obtained by excavation trial holes.
- The location and position of private drains, private sewers and service pipes to properties are not normally shown on this Map but their presence must be anticipated and accounted for and you are strongly advised to carry out your own further enquiries and investigations in order to locate the same.
- The position and depth of UUWL apparatus is subject to change and therefore this Map is issued subject to any removal or change in location of the same. The onus is entirely upon you to confirm whether any changes to the Map have been made subsequent to issue and prior to any works being carried out.
- This Map and any information shown on it or provided with it must not be relied upon in the event of any development, construction or other works (including but not limited to any excavations) in the vicinity of UUWL apparatus or for the purpose of determining the suitability of a point of connection to the sewerage or other distribution systems.
- No person or legal entity, including any company shall be relieved from any liability howsoever and whensoever arising for any damage caused to UUWL apparatus by reason of the actual position and/or depths of UUWL apparatus being different from those shown on the Map and any information supplied with it.
- If any provision contained herein is or becomes legally invalid or unenforceable, it will be taken to be severed from the remaining provisions which shall be unaffected and continue in full force and effect.
- This agreement shall be governed by English law and all parties submit to the exclusive jurisdiction of the English courts, save that nothing will prevent UUWL from bringing proceedings in any other competent jurisdiction, whether concurrently or otherwise.

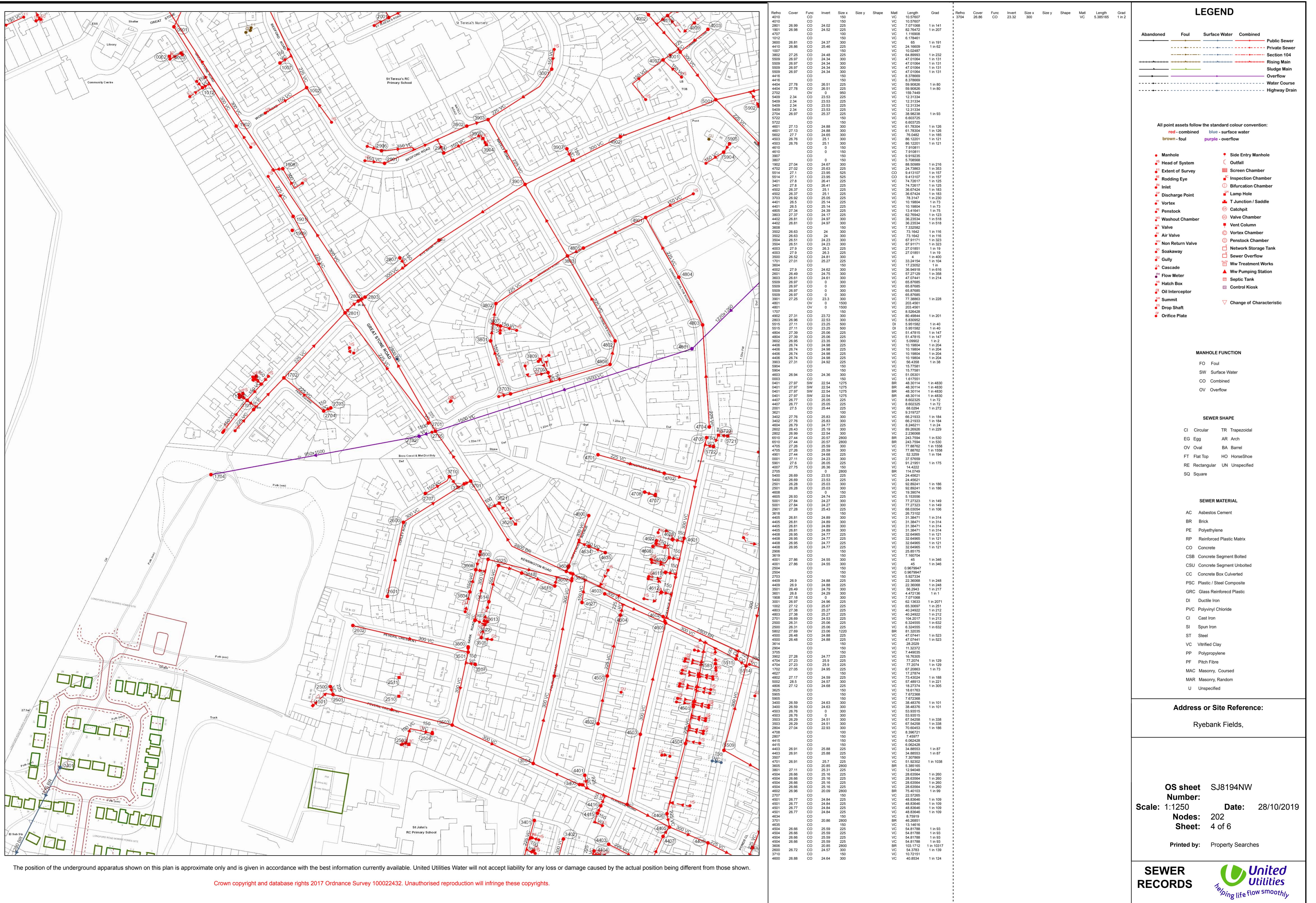


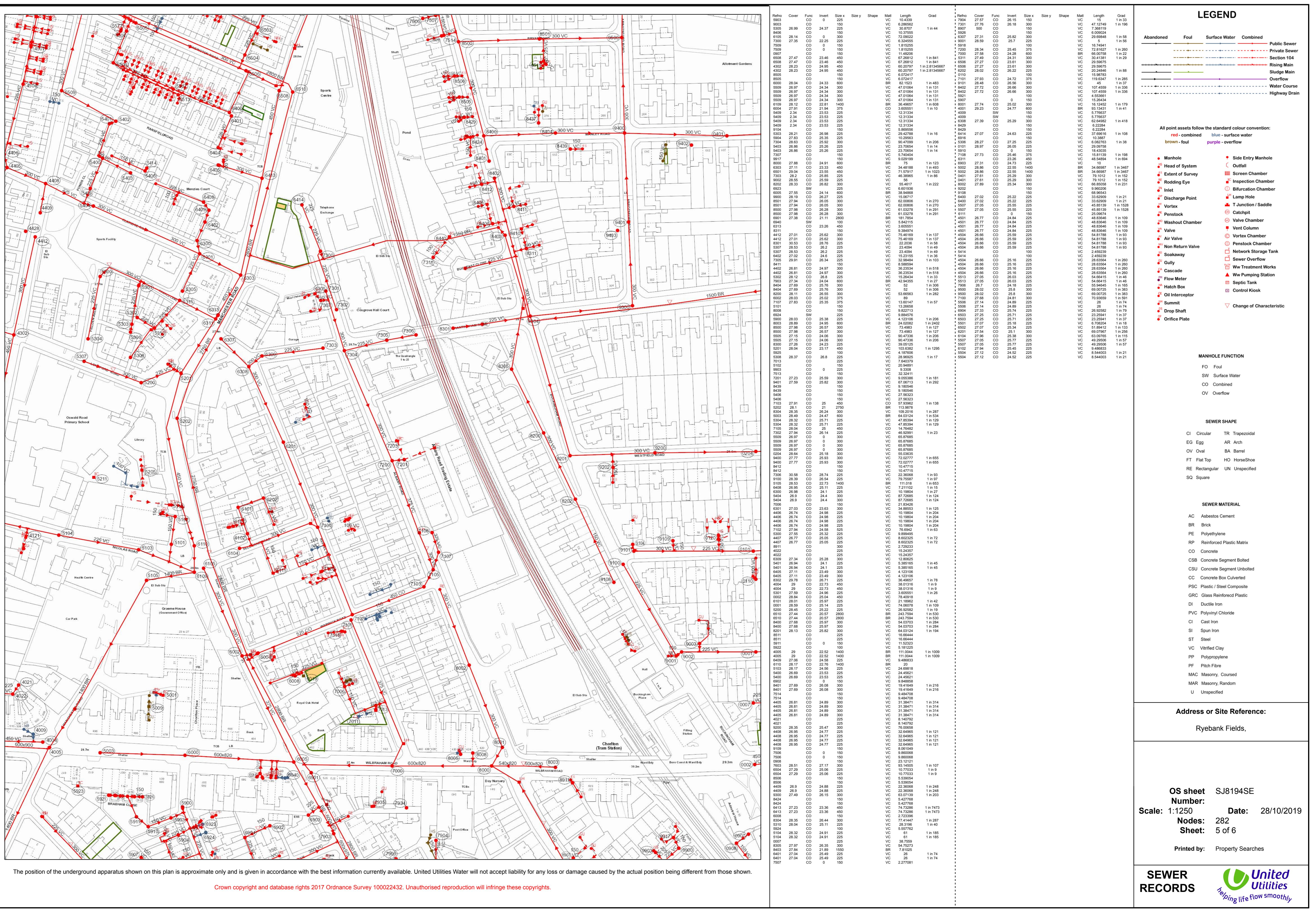


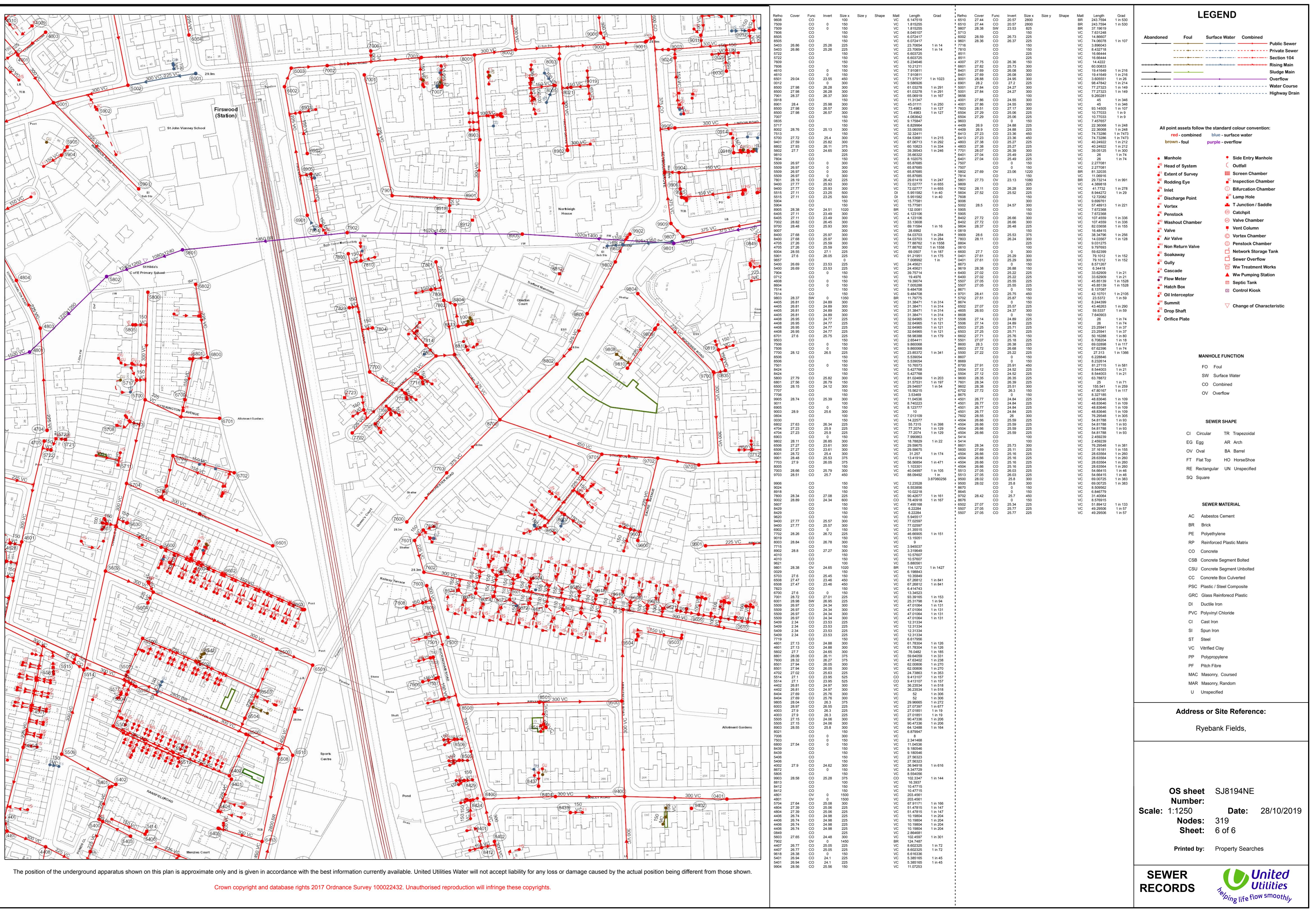


The position of the underground apparatus shown on this plan is approximate only and is given in accordance with the best information currently available. United Utilities Water will not accept liability for any loss or damage caused by the actual position being different from those shown.

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APPENDIX 6: Drain Alert Report



Cripplegate Lane, Hoghton, Preston. PR5 0RR
Tel: 01254 851500 Fax: 01254854004 service@drain-alert.co.uk

BWB Consulting Ltd
11 Portland Street
Manchester
M1 3HU

3rd March 2020

Dear Keith,

Reference: - JN29928, Ryebank Fields.

May we thank you for your valued custom. As requested, we have conducted a CCTV inspection at the above premises. We have emailed a link to you to access the Wincan VX video clips and documents via the cloud.

We trust that the report is to your satisfaction; however, should you have any queries then please do not hesitate to contact me.

Yours sincerely,

Mr S W Ormisher, B.A.(Hons.),
Technical Services Consultant

Service areas: Preston • Bolton • Wigan • Salford • Tameside • Rochdale • Cheshire • Fylde • Burnley
Company Reg. No. 029502950360 • Reg. No. 448 2116 57





Plan of the drainage system, not to scale

Enclosed

Conclusion

As requested, a CCTV survey and investigation of United Utilities surface water drainage lines was carried out. Upon arrival visible inspection found various manholes to be in light road, footpath and field areas, further investigation found silt debris to be minimal in manholes, therefore no jetting was carried out prior to the survey.

The survey was conducted upstream and downstream from manholes marked and identified on plan accordingly. Evidence from the survey found drainage lines to be of 1275mm brick construction throughout.

The general condition of lines surveyed was found to be reasonable and in expected working order throughout although one fault was seen which will require remedial work to be carried out to prevent problems occurring in the future.

Faults Found:

Section 1 UUMH1704-UUMH0401

Obstruction seen at 63.60mtrs causing reduced efficiency within the drain, restricting the survey.

Recommendation is to use the high-pressure tanker wagon to remove the debris allowing the CCTV to be carried out successfully.

We trust that the above is acceptable; however, should you require any further information, please do not hesitate to contact me.

Yours sincerely,

A handwritten signature in black ink, appearing to read "S W Ormisher".

Mr S W Ormisher,
Technical Services Consultant



Disclaimer - Please note that any dimensions, levels and drainage layout drawings that are provided by Drain Alert, should be checked before being relied upon. All updated drawings are not to scale. It is the responsibility of the client to verify all information given with regards to the drainage prior to commencing any design or work site.

**Project****Project Name:** 29928 Ryebank Fields**Project Description:** Converted project from v8 project**Project Number:** 1**Project Date:** 26/02/2020**Inspection Standard:** MSCC3 Sewers & Drainage GB (SRM4 Scoring)



Drain Alert
Cripplegate Barn, Cripplegate Lane
Tel. 01254 851500
www.drain-alert.co.uk

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Section: 2; UUMH0401 > UUMH9400		3



Drain Alert

Cripplegate Barn, Cripplegate Lane

Tel. 01254 851500

www.drain-alert.co.uk

Project Information

Project Name
29928 Ryebank Fields

Project Number
1

Project Date
26/02/2020

Client

Company: BWB Consulting Limited
Street: Ryebank Road
Town or City: Chorlton

Site

Company: Ryebank Fields
Street: Ryebank Road
Town or City: Chorlton

Contractor

Company: Drain Alert
Contact: Stephen Ormisher
Department: Director
Street: Cripplegate Barn
Town or City: Cripplegate Lane
County: Hoghton
Post Code: Preston PR5 0RR
Phone: 01254 851500
Fax: 01254 854004
Email: www.drain-alert.co.uk



Drain Alert

Cripplegate Barn, Cripplegate Lane
Tel. 01254 851500
www.drain-alert.co.uk

Section Inspection - 26/02/2020

Section 1	Inspection 1	Date 26/02/20	Time 10:27	Client's Job Ref Not Specified	Weather Showers	Pre Cleaned Not Specified	PLR Not Specified
Operator L Hilton	Vehicle PK65 HFB	Camera P235 Tractor		Preset Length Not Specified	Legal Status Not Specified	Alternative ID 1	

Town or Village:	Upstream Node: UU MH 1704
Road: Ryebank Road	Upstream Pipe Depth:
Location:	Downstream Node: UU MH 0401
Surface Type:	Downstream Pipe Depth:

Use: Surface water	Pipe Shape: Circular
Type of Pipe:	Dia/Height: 1,275 mm
Year Constructed:	Pipe Material: Brick
Flow Control:	Lining: No Lining
Inspection Purpose: Sample survey to determine asset condition	Lining Material: No Lining

Comments:
Recommendations:

Scale: 1:554	Position [m]	Code	Observation	MPEG	Photo	Grade
Depth: m						
UU MH 0401	0.00	ST	Start of survey	00:00:23		
UU MH 1704	63.60	OB	Obstruction, from 9 to 5 o'clock, 30% height loss: Heavy Debris	00:03:31	_2f6700ca-1ec4-4ca3-8fb1-c54	5
Depth: m	63.60	FH	Finish survey: Limit of CCTV Due to Debris	00:03:37		

Construction Features					Miscellaneous Features						
Structural Defects					Service & Operational Observations						
STR No.	Def	STR Peak	STR Mean	STR Total	STR Grade	SER No.	Def	SER Peak	SER Mean	SER Total	SER Grade
0		0.0	0.0	0.0	1.0	1		10.0	0.2	10.0	5.0

Section Pictures - 26/02/2020

Section 1	Inspection Direction Upstream	PLR	Client's Job Ref	Contractor's Job Ref
--------------	----------------------------------	-----	------------------	----------------------



_2f6700ca-1ec4-4ca3-8fb1-c542ec392b47.jpg, 00:03:31,
63.60 m
Obstruction, from 9 to 5 o'clock, 30% height loss, Heavy Debris



Drain Alert
Cripplegate Barn, Cripplegate Lane
Tel. 01254 851500
www.drain-alert.co.uk

Section Inspection - 26/02/2020

Section 2	Inspection 1	Date 26/02/20	Time 11:27	Client's Job Ref Not Specified	Weather Showers	Pre Cleaned Not Specified	PLR Not Specified
Operator L Hilton	Vehicle PK65 HFB	Camera P235 Tractor		Preset Length Not Specified	Legal Status Not Specified	Alternative ID 2	

Town or Village:	Inspection Direction:	Downstream	Upstream Node:	UU MH 0401
Road:	Inspected Length:	95.40 m	Upstream Pipe Depth:	
Location:	Total Length:	95.40 m	Downstream Node:	UU MH 9400
Surface Type:	Joint Length:	0.00 m	Downstream Pipe Depth:	

Use: Surface water	Pipe Shape: Circular
Type of Pipe:	Dia/Height: 1,275 mm
Year Constructed:	Pipe Material: Brick
Flow Control:	Lining: No Lining
Inspection Purpose: Sample survey to determine asset condition	Lining Material: No Lining

Comments:
Recommendations:

Scale: 1:830	Position [m]	Code	Observation	MPEG	Photo	Grade
Depth: m						
UU MH 0401						
 0.00	ST	Start of survey		00:00:10		
						
95.40	FH	Finish survey: UU MH 9400		00:04:36	_3db50f29 -7db9-4df9 -83fd-ffc15	
 UU MH 9400	Depth: m					

Construction Features					Miscellaneous Features						
Structural Defects					Service & Operational Observations						
STR No.	Def	STR Peak	STR Mean	STR Total	STR Grade	SER No.	Def	SER Peak	SER Mean	SER Total	SER Grade
0		0.0	0.0	0.0	1.0	0		0.0	0.0	0.0	1.0

Section Pictures - 26/02/2020

Section 2	Inspection Direction Downstream	PLR	Client's Job Ref	Contractor's Job Ref
--------------	------------------------------------	-----	------------------	----------------------



_3db50f29-7db9-4df9-83fd-ffc15a0464f2.jpg, 00:04:36, 95.40

m

Finish survey, UU MH 9400



Manhole Record Card

Number	UU0401	Date Of Survey	26/02/2020
Status	PU	Function	S
		Type	M

Cover Details:

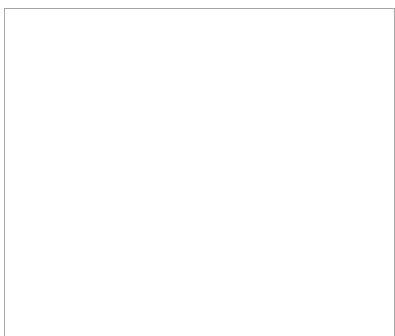
Square Recta Double Triang Single Triangl Circular Multiple Hinged Lockable

CoverLevel

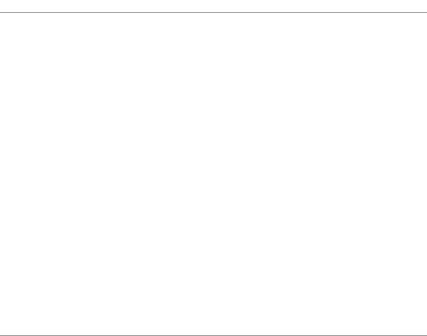
Cover Load Class

Cover	600	X	600	Chamber	<input type="text"/>	X	0	EvidenceOfSurcharge	<input type="checkbox"/>		
Shaft	0	X	0	ShaftDepth	<input type="text"/>	0		ToxicAtmosphere	<input type="checkbox"/>		
Brick	<input checked="" type="checkbox"/>	Precast Concrete	<input type="checkbox"/>	PVC	<input type="checkbox"/>	Segments	<input type="checkbox"/>	No.RegCourses	<input type="checkbox"/>		
Reducing Slab	<input type="checkbox"/>	Taper	<input type="checkbox"/>	Side Entry	<input type="checkbox"/>	No.Land	<input type="text" value="0"/>	Step Irons	<input checked="" type="checkbox"/>	Ladder	<input type="checkbox"/>

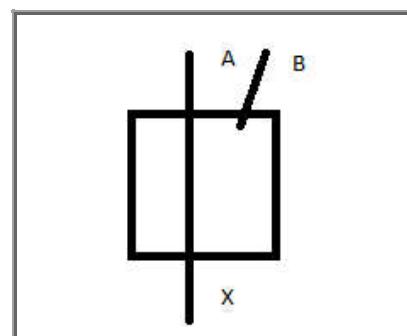
PlanPhoto



LocationPhoto



PlanofManhol

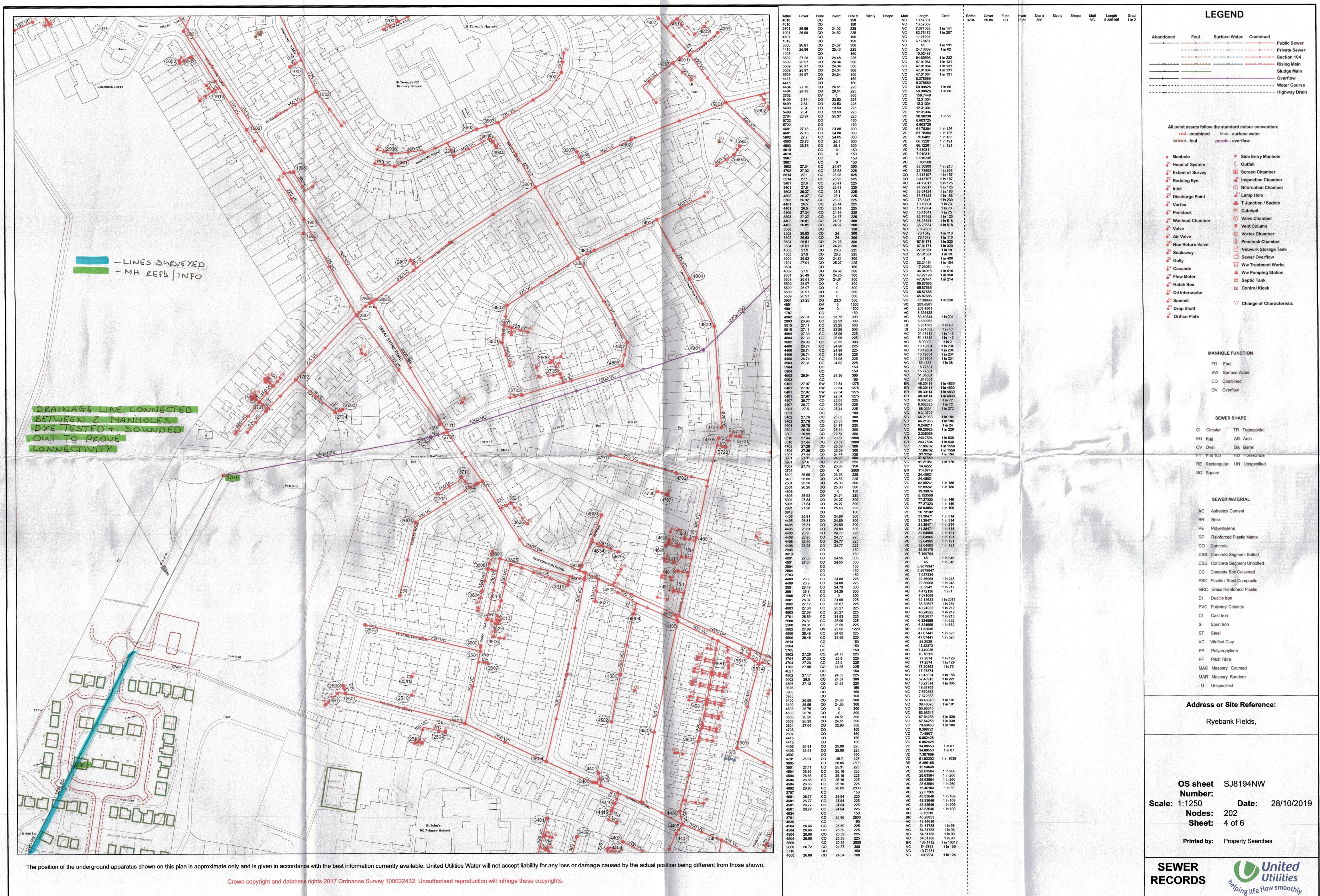

Chamber Conditions:

Cover	<input type="text" value="OK"/>	Shaft	<input type="text" value="OK"/>
Irons/Ladder	<input type="text" value="OK"/>	Chamber	<input type="text" value="OK"/>
Benching/Channel	<input type="text" value="OK"/>		

PHOTOS NOT DOWLOADING, WILL RE TAKE WHEN WE GO BACK TO SITE

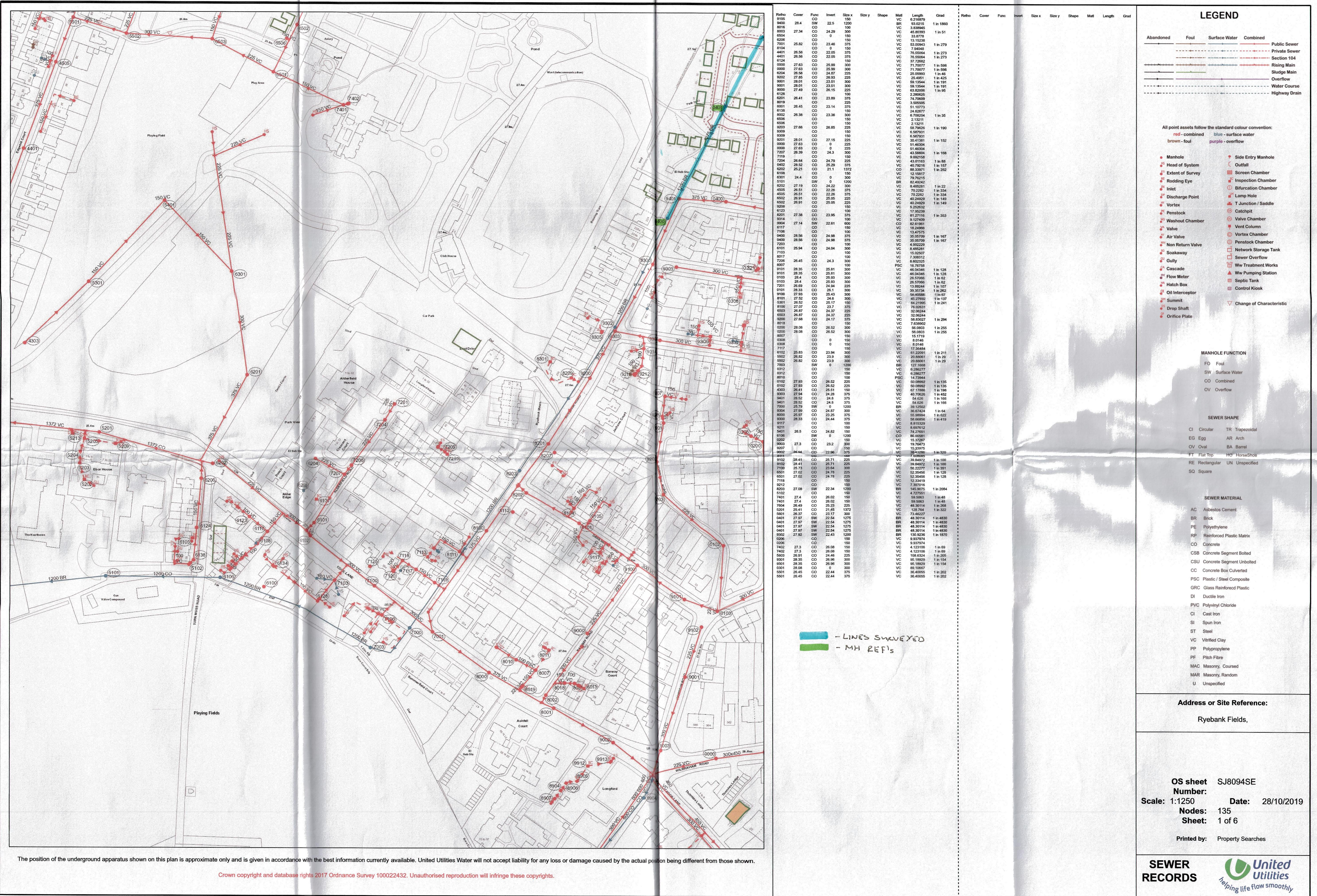
Pipe	Invert L	Depth Fr	Upstream Re	Downstream	Pipe Sh	Size	Height	Size	Width	Pipe Material	Lining Material
A	-5.400	5.40	MH1704		C			1275		B	
B	-2.730	2.73			C			150		CI	
X	-5.400	5.40		MH9400	C			1275		B	

Disclaimer - Any dimensions and levels provided on this form should be checked before being relied upon. It is the responsibility of the customer to verify all information given with regards to the drainage prior to designing or commencing any work on site.



The position of the underground apparatus shown on this plan is approximate only and is given in accordance with the best information currently available. United Utilities Water will not accept liability for any loss or damage caused by the actual position being different from those shown.

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APPENDIX 7: Micro Drainage Calculations

BWB Consulting Ltd		Page 1
5th Floor, Waterfront House 35 Station Street Nottingham, NG2 3DQ		
Date 03/03/2020 08:23	Designed by keith.alger	
File	Checked by	
XP Solutions	Source Control 2018.1.1	



ICP SUDS Mean Annual Flood

Input

Return Period (years)	2	Soil	0.300
Area (ha)	1.000	Urban	0.000
SAAR (mm)	851	Region Number	Region 10

Results 1/s

QBAR Rural 2.3
QBAR Urban 2.3

Q2 years 2.1

Q1 year 2.0
Q30 years 3.9
Q100 years 4.8

BWB Consulting Ltd 4th Floor Carvers Warehouse 77 Dale Street Manchester M1 2HG		Page 1
Date 04/03/2020 15:08 File Catchment 1 Storage.SRCX	Designed by Lucy.Reeves Checked by	
Micro Drainage	Source Control 2018.1.1	

Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	99.431	0.431	4.5	470.8	O K
30 min Summer	99.488	0.488	4.5	537.5	O K
60 min Summer	99.549	0.549	4.5	611.2	O K
120 min Summer	99.614	0.614	4.5	690.4	O K
180 min Summer	99.652	0.652	4.5	737.4	O K
240 min Summer	99.678	0.678	4.5	769.8	O K
360 min Summer	99.711	0.711	4.5	812.0	Flood Risk
480 min Summer	99.731	0.731	4.5	837.6	Flood Risk
600 min Summer	99.744	0.744	4.5	853.5	Flood Risk
720 min Summer	99.751	0.751	4.5	863.3	Flood Risk
960 min Summer	99.763	0.763	4.5	878.9	Flood Risk
1440 min Summer	99.764	0.764	4.5	879.7	Flood Risk
2160 min Summer	99.743	0.743	4.5	852.2	Flood Risk
2880 min Summer	99.721	0.721	4.5	823.9	Flood Risk
4320 min Summer	99.673	0.673	4.5	763.4	O K
5760 min Summer	99.625	0.625	4.5	704.3	O K
7200 min Summer	99.573	0.573	4.5	639.2	O K
8640 min Summer	99.523	0.523	4.5	579.1	O K
10080 min Summer	99.476	0.476	4.5	523.8	O K
15 min Winter	99.480	0.480	4.5	528.0	O K
30 min Winter	99.543	0.543	4.5	603.3	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
		(m³)	(m³)	
15 min Summer	200.129	0.0	379.8	27
30 min Summer	114.737	0.0	379.6	41
60 min Summer	65.780	0.0	626.0	72
120 min Summer	37.713	0.0	703.0	130
180 min Summer	27.237	0.0	719.3	190
240 min Summer	21.621	0.0	716.8	250
360 min Summer	15.615	0.0	707.3	368
480 min Summer	12.396	0.0	698.2	488
600 min Summer	10.363	0.0	690.3	606
720 min Summer	8.952	0.0	683.3	726
960 min Summer	7.164	0.0	670.7	964
1440 min Summer	5.233	0.0	650.3	1442
2160 min Summer	3.823	0.0	1310.3	1868
2880 min Summer	3.059	0.0	1325.0	2256
4320 min Summer	2.223	0.0	1219.5	3036
5760 min Summer	1.772	0.0	1619.8	3880
7200 min Summer	1.486	0.0	1698.5	4680
8640 min Summer	1.288	0.0	1765.5	5440
10080 min Summer	1.140	0.0	1824.3	6160
15 min Winter	200.129	0.0	380.7	27
30 min Winter	114.737	0.0	374.7	41

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Micro Drainage	Source Control 2018.1.1	

Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
60 min Winter	99.611	0.611	4.5	686.8	O K
120 min Winter	99.684	0.684	4.5	776.9	O K
180 min Winter	99.726	0.726	4.5	830.9	Flood Risk
240 min Winter	99.755	0.755	4.5	868.5	Flood Risk
360 min Winter	99.794	0.794	4.5	918.6	Flood Risk
480 min Winter	99.818	0.818	4.5	950.1	Flood Risk
600 min Winter	99.834	0.834	4.5	970.9	Flood Risk
720 min Winter	99.844	0.844	4.5	984.5	Flood Risk
960 min Winter	99.862	0.862	4.5	1007.6	Flood Risk
1440 min Winter	99.871	0.871	4.5	1019.4	Flood Risk
2160 min Winter	99.854	0.854	4.5	997.2	Flood Risk
2880 min Winter	99.824	0.824	4.5	957.8	Flood Risk
4320 min Winter	99.764	0.764	4.5	879.9	Flood Risk
5760 min Winter	99.700	0.700	4.5	798.2	Flood Risk
7200 min Winter	99.631	0.631	4.5	711.6	O K
8640 min Winter	99.549	0.549	4.5	610.4	O K
10080 min Winter	99.474	0.474	4.5	521.1	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
60 min Winter	65.780	0.0	694.1	70
120 min Winter	37.713	0.0	719.6	128
180 min Winter	27.237	0.0	713.1	188
240 min Winter	21.621	0.0	706.2	246
360 min Winter	15.615	0.0	695.5	362
480 min Winter	12.396	0.0	687.8	480
600 min Winter	10.363	0.0	681.9	596
720 min Winter	8.952	0.0	677.1	712
960 min Winter	7.164	0.0	670.1	942
1440 min Winter	5.233	0.0	664.5	1394
2160 min Winter	3.823	0.0	1378.6	2040
2880 min Winter	3.059	0.0	1344.5	2368
4320 min Winter	2.223	0.0	1250.9	3252
5760 min Winter	1.772	0.0	1814.6	4208
7200 min Winter	1.486	0.0	1902.1	5120
8640 min Winter	1.288	0.0	1977.5	5888
10080 min Winter	1.140	0.0	2043.4	6656

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Rainfall Details

Rainfall Model	FEH
Return Period (years)	100
FEH Rainfall Version	1999
Site Location GB 380950 394650 SJ 80950 94650	
C (1km)	-0.026
D1 (1km)	0.317
D2 (1km)	0.345
D3 (1km)	0.332
E (1km)	0.303
F (1km)	2.456
Summer Storms	Yes
Winter Storms	Yes
Cv (Summer)	0.750
Cv (Winter)	0.840
Shortest Storm (mins)	15
Longest Storm (mins)	10080
Climate Change %	+40

Time Area Diagram

Total Area (ha) 1.270

Time (mins)	Area	Time (mins)	Area	Time (mins)	Area
From:	To:	(ha)	From:	To:	(ha)
0	4 0.423	4	8 0.423	8	12 0.423

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Model Details

Storage is Online Cover Level (m) 100.000

Tank or Pond Structure

Invert Level (m) 99.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	1016.3	1.000	1383.6

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0100-4500-1050-4500
Design Head (m)	1.050
Design Flow (l/s)	4.5
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	100
Invert Level (m)	98.950
Minimum Outlet Pipe Diameter (mm)	150
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.050	4.5
Flush-Flo™	0.310	4.5
Kick-Flo®	0.661	3.6
Mean Flow over Head Range	-	3.9

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)						
0.100	3.3	1.200	4.8	3.000	7.3	7.000	11.0
0.200	4.4	1.400	5.1	3.500	7.9	7.500	11.3
0.300	4.5	1.600	5.5	4.000	8.4	8.000	11.7
0.400	4.4	1.800	5.8	4.500	8.9	8.500	12.0
0.500	4.3	2.000	6.1	5.000	9.4	9.000	12.4
0.600	4.0	2.200	6.4	5.500	9.8	9.500	12.7
0.800	4.0	2.400	6.6	6.000	10.2		
1.000	4.4	2.600	6.9	6.500	10.6		

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Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	99.441	0.441	3.0	251.2	O K
30 min Summer	99.496	0.496	3.0	286.5	O K
60 min Summer	99.555	0.555	3.0	325.0	O K
120 min Summer	99.616	0.616	3.0	365.6	O K
180 min Summer	99.651	0.651	3.0	388.9	O K
240 min Summer	99.673	0.673	3.0	404.4	O K
360 min Summer	99.701	0.701	3.0	423.3	Flood Risk
480 min Summer	99.715	0.715	3.0	433.5	Flood Risk
600 min Summer	99.722	0.722	3.0	438.6	Flood Risk
720 min Summer	99.725	0.725	3.0	440.4	Flood Risk
960 min Summer	99.727	0.727	3.0	442.3	Flood Risk
1440 min Summer	99.712	0.712	3.0	431.5	Flood Risk
2160 min Summer	99.683	0.683	3.0	411.4	O K
2880 min Summer	99.656	0.656	3.0	392.2	O K
4320 min Summer	99.593	0.593	3.0	349.8	O K
5760 min Summer	99.529	0.529	3.0	307.9	O K
7200 min Summer	99.471	0.471	3.0	270.2	O K
8640 min Summer	99.417	0.417	3.0	236.3	O K
10080 min Summer	99.367	0.367	3.0	205.5	O K
15 min Winter	99.489	0.489	3.0	281.8	O K
30 min Winter	99.550	0.550	3.0	321.7	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
		(m³)	(m³)	
15 min Summer	200.129	0.0	243.7	27
30 min Summer	114.737	0.0	250.9	41
60 min Summer	65.780	0.0	335.2	70
120 min Summer	37.713	0.0	384.5	130
180 min Summer	27.237	0.0	416.4	190
240 min Summer	21.621	0.0	440.7	248
360 min Summer	15.615	0.0	466.9	368
480 min Summer	12.396	0.0	470.4	486
600 min Summer	10.363	0.0	468.7	606
720 min Summer	8.952	0.0	465.7	724
960 min Summer	7.164	0.0	457.9	962
1440 min Summer	5.233	0.0	440.9	1368
2160 min Summer	3.823	0.0	701.6	1712
2880 min Summer	3.059	0.0	748.6	2104
4320 min Summer	2.223	0.0	811.6	2904
5760 min Summer	1.772	0.0	867.2	3688
7200 min Summer	1.486	0.0	909.7	4464
8640 min Summer	1.288	0.0	945.4	5192
10080 min Summer	1.140	0.0	976.8	5952
15 min Winter	200.129	0.0	250.7	27
30 min Winter	114.737	0.0	250.4	41

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Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
60 min Winter	99.616	0.616	3.0	365.6	O K
120 min Winter	99.684	0.684	3.0	412.0	O K
180 min Winter	99.723	0.723	3.0	439.1	Flood Risk
240 min Winter	99.749	0.749	3.0	457.4	Flood Risk
360 min Winter	99.781	0.781	3.0	480.6	Flood Risk
480 min Winter	99.800	0.800	3.0	493.9	Flood Risk
600 min Winter	99.810	0.810	3.0	501.5	Flood Risk
720 min Winter	99.815	0.815	3.0	505.4	Flood Risk
960 min Winter	99.823	0.823	3.0	511.2	Flood Risk
1440 min Winter	99.816	0.816	3.0	505.7	Flood Risk
2160 min Winter	99.780	0.780	3.0	479.8	Flood Risk
2880 min Winter	99.747	0.747	3.0	455.8	Flood Risk
4320 min Winter	99.668	0.668	3.0	400.5	O K
5760 min Winter	99.573	0.573	3.0	336.8	O K
7200 min Winter	99.478	0.478	3.0	275.1	O K
8640 min Winter	99.394	0.394	3.0	222.5	O K
10080 min Winter	99.320	0.320	3.0	177.4	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
60 min Winter	65.780	0.0	375.4	70
120 min Winter	37.713	0.0	430.6	128
180 min Winter	27.237	0.0	462.9	186
240 min Winter	21.621	0.0	471.8	244
360 min Winter	15.615	0.0	471.9	362
480 min Winter	12.396	0.0	468.6	478
600 min Winter	10.363	0.0	464.8	594
720 min Winter	8.952	0.0	461.1	708
960 min Winter	7.164	0.0	454.0	936
1440 min Winter	5.233	0.0	441.4	1378
2160 min Winter	3.823	0.0	785.9	1932
2880 min Winter	3.059	0.0	838.4	2228
4320 min Winter	2.223	0.0	831.9	3160
5760 min Winter	1.772	0.0	971.5	4040
7200 min Winter	1.486	0.0	1018.6	4768
8640 min Winter	1.288	0.0	1058.9	5536
10080 min Winter	1.140	0.0	1094.4	6256

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Rainfall Details

Rainfall Model	FEH
Return Period (years)	100
FEH Rainfall Version	1999
Site Location GB 380950 394650 SJ 80950 94650	
C (1km)	-0.026
D1 (1km)	0.317
D2 (1km)	0.345
D3 (1km)	0.332
E (1km)	0.303
F (1km)	2.456
Summer Storms	Yes
Winter Storms	Yes
Cv (Summer)	0.750
Cv (Winter)	0.840
Shortest Storm (mins)	15
Longest Storm (mins)	10080
Climate Change %	+40

Time Area Diagram

Total Area (ha) 0.680

Time (mins)	Area	Time (mins)	Area	Time (mins)	Area
From:	To:	(ha)	From:	To:	(ha)
0	4 0.227	4	8 0.227	8	12 0.227

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Model Details

Storage is Online Cover Level (m) 100.000

Tank or Pond Structure

Invert Level (m) 99.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	515.1	1.000	784.8

Hydro-Brake® Optimum Outflow Control

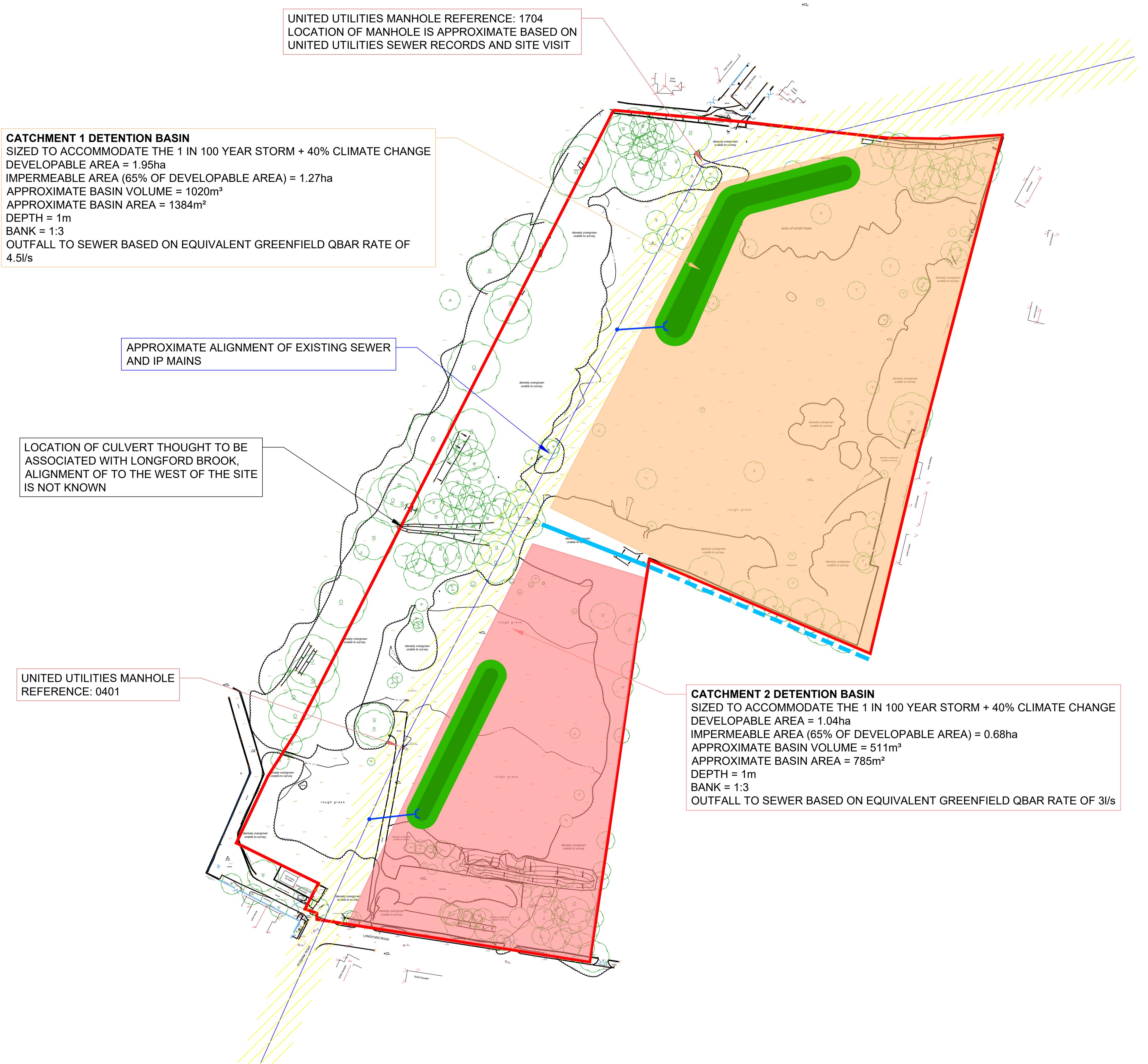
Unit Reference	MD-SHE-0082-3000-1050-3000
Design Head (m)	1.050
Design Flow (l/s)	3.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	82
Invert Level (m)	98.950
Minimum Outlet Pipe Diameter (mm)	100
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.050	3.0
Flush-Flo™	0.320	3.0
Kick-Flo®	0.658	2.4
Mean Flow over Head Range	-	2.6

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)						
0.100	2.4	1.200	3.2	3.000	4.9	7.000	7.3
0.200	2.9	1.400	3.4	3.500	5.2	7.500	7.5
0.300	3.0	1.600	3.6	4.000	5.6	8.000	7.7
0.400	3.0	1.800	3.8	4.500	5.9	8.500	8.0
0.500	2.9	2.000	4.0	5.000	6.2	9.000	8.2
0.600	2.7	2.200	4.2	5.500	6.5	9.500	8.4
0.800	2.6	2.400	4.4	6.000	6.8		
1.000	2.9	2.600	4.6	6.500	7.0		

APPENDIX 8: Conceptual Drainage Strategy



- Notes**
- Do not scale this drawing. All dimensions must be checked/verified on site. If in doubt ask.
 - This drawing is to be read in conjunction with all relevant architects, engineers and specialists drawings and specifications.
 - All dimensions in millimetres unless noted otherwise. All levels in metres unless noted otherwise.
 - Any discrepancies noted on site are to be reported to the engineer immediately.
 - Do not construct based on this drawing.
 - Location of existing surface water sewer/IP Mains is approximate and taken from Drain Alert site investigations and Fuel Solutions UK Limited Utility Assessment Report.
 - Location of the Nico Ditch is approximate.
 - Topographical Survey undertaken by CT Surveys.
 - All SuDS features are indicative and subject to further development of a masterplan, planning and detailed design.
 - Outfall to the existing surface water sewer subject to confirmation from United Utilities.

Legend

- Indicative Site Boundary
- Nico Ditch
- Existing Surface Water Sewer/IP Mains
- Existing Manhole
- 16m Easement Associated with Surface Water Sewer
- Catchment 1
- Catchment 2
- Detention Basin
- Headwall and Pipe
- Proposed Manhole

P01	05.03.20	Preliminary Issue	LR	KA
Rev	Date	Details of Issue / revision	Drw	Rev

Issues & Revisions

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Client
MANCHESTER METROPOLITAN UNIVERSITY

Project Title
RYEBANK ROAD, CHORLTON

Drawing Title
CONCEPTUAL SURFACE WATER DRAINAGE STRATEGY

Drawn: L. Reeves Reviewed: K. Alger

BWB Ref: MCW2136 Date: 05.03.20 Scale@A1: NTS

Drawing Status:

PRELIMINARY

Project - Originator - Zone - Level - Type - Role - Number Status Rev

MMU-BWB-ZZ-XX-SK-CD-0001 S2 P01

APPENDIX 9: Pre-Development Enquiry

Lucy Reeves

From: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Sent: 07 November 2019 13:44
To: Lucy Reeves
Cc: Wastewater Developer Services
Subject: RE: Wastewater Pre-Development Enquiry: Ryebank Road, Chorlton (MCW2136)
4200028845

Good Afternoon

We have carried out an assessment of your application which is based on the information provided; this pre development advice will be valid for 12 months

Foul

Foul will be allowed to drain to the public combined sewer network road at an unrestricted rate. Our preferred point of connection would be to the 225mm Combined sewer in Rye Bank Road, to the North of the site.

Surface Water

Surface water from this site should drain to either soak away or directly to watercourse. Discharge rates and consents must be discussed and agreed with all interested parties.

Connection Application

Although we may discuss and agree discharge points & rates in principle, please be aware that you will have to apply for a formal sewer connection. This is so that we can assess the method of construction, Health & Safety requirements and to ultimately inspect the connection when it is made. Details of the application process and the form itself can be obtained from our website by following the link below

<http://www.unitedutilities.com/connecting-public-sewer.aspx>

Sewer Adoption Agreement

You may wish to offer the proposed new sewers for adoption. United Utilities assess adoption application based on Sewers adoption 6th Edition and for any pumping stations our company addenda document. Please refer to link below to obtain further guidance and application pack:

<http://www.unitedutilities.com/sewer-adoption.aspx>

Existing Sewers Crossing the Site

A public sewer crosses this site and we will require unrestricted access to the sewer for maintenance purposes, we would ask that you maintain a minimum clearance of 6m, which is measured 3m from the centre line of the pipe. If you cannot achieve this then you may wish to consider diverting the public sewer.

Please refer to the link below to obtain full details of the processes involved in sewer diversion.

<http://www.unitedutilities.com/sewer-diversion.aspx>

Please be aware that on site drainage must be designed in accordance with Building Regulations, National Planning Policy, and local flood authority guidelines, we would recommend that you speak and make suitable agreements with the relevant statutory bodies.

Please note, if you intend to put forward your wastewater assets for adoption by United Utilities, the proposed detail design will be subject to a technical appraisal by an Adoption Engineer as we need to be sure that the proposals meets the requirements of Sewers for adoption and United Utilities Asset Standards. The proposed design should give consideration to long term operability and give United Utilities a cost effective proposal for the life of the assets. Therefore, further to this enquiry should you wish to progress a Section 104 agreement, we strongly recommend that no construction commences until the detailed drainage design, submitted as part of the Section 104 agreement, has been assessed and accepted in writing by United Utilities. Any works carried out prior to the technical assessment being approved is done entirely at the developers own risk and could be subject to change.

Regards

Matthew Dodd

Assistant Developer Engineer
Developer Services and Planning
Network Delivery
United Utilities
T: 01925 679369 (internal 79369)
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