

List of Tables

List of Figures

This report describes the structural design residential solar canopy in the City of Larkspur, California. It includes the design of a concrete slab, stem wall, steel tube frame, and attachments of solar panels to the frame.

The report is divided into the following three divisions:

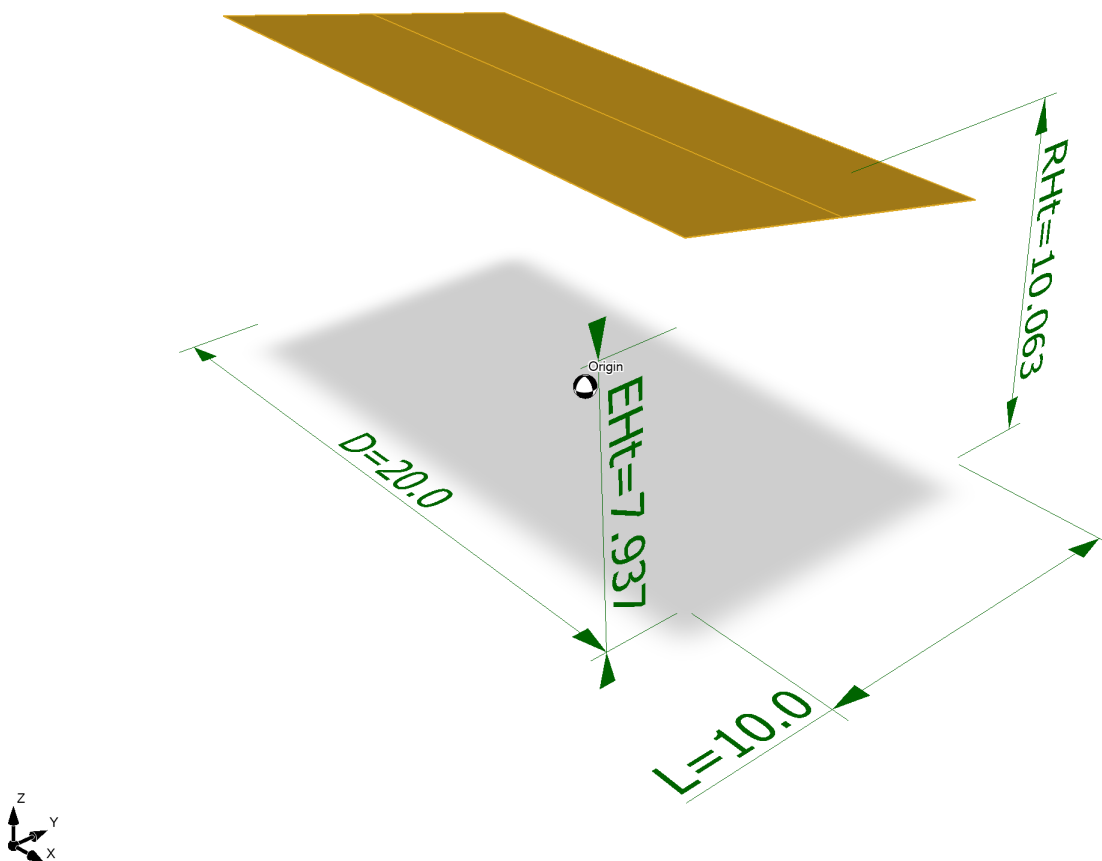
- 01 Loads: gravity, wind and seismic
- 02 Frame: steel tubes, connections and clips
- 03 Foundation: slab and stem wall

Client:

Date:

Location:

The project is located in Larkspur, California.

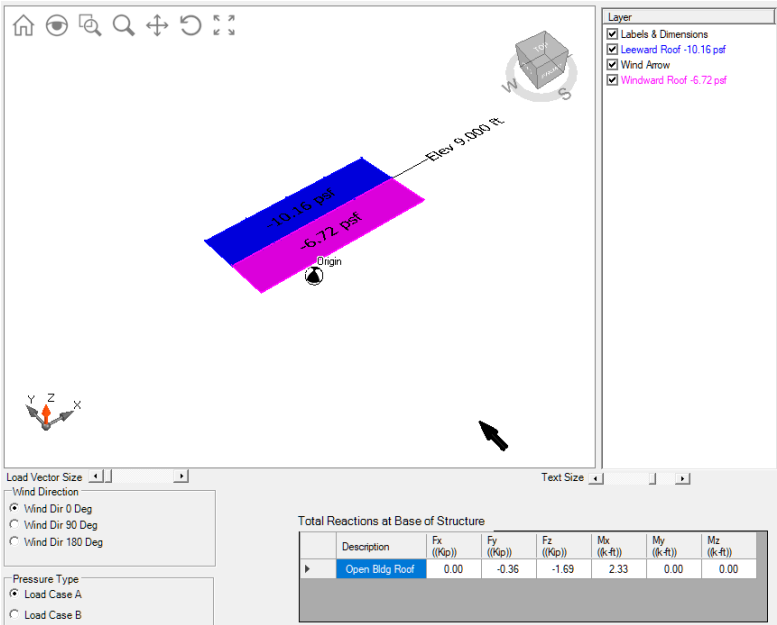


****Fig. 02 - **Wind load 1**

F02 - 02

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****Fig. 03 - **Wind load 2** F03 - 02

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The permit approval is under the jurisdiction of the City of Larkspur, California which adopted the 2019 California Building Code [CBC] and the 2019 California Residential Code [CRC] as the basis for permitting requirements of the CBC.
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Category Standard Year

Loading ASCE-7 2016

Concrete ACI-318 2014

Wood-National Design Specifications AWC-NDS 2018

Wood-Special Design Provisions for Wind and Seismic AWC-SDPWS 2015

Wood Frame Construction Manual AWC-WFCM 2018

Basic loads and load combinations are derived from the California Building and Residential Codes.

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Sym Load Effect Notes

D Dead load See IBC 1606 and Chapter 3 of this publication

E Combined effect of horizontal and vertical earthquake- induced forces as defined in ASCE/SEI 12.4.2 See IBC 1613, ASCE/SEI 12.4.2 and Chapter 6 of this publication

Em Maximum seismic load effect of horizontal and vertical forces as set forth in ASCE/SEI 12.4.3 See IBC 1613, ASCE/SEI 12.4.3 and Chapter 6 of this publication

H Load due to lateral earth pressures, ground water pressure or pressure of bulk materials See IBC 1610 for soil lateral loads

L Live load, except roof live load, including any permitted live load reduction See IBC 1607 and Chapter 3 of this publication

Li Roof live load including any permitted live load reduction See IBC 1607 and Chapter 3 of this publication

R Rain load See IBC 1611 and Chapter 3 of this publication

W Load due to wind pressure See IBC 1609 and Chapter 5 of this publication

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CBC 2019 reference Equation

Equation 16-1 $1.4(D + F)$

Equation 16-2 $1.2(D + F) + 1.6(L + H) + 0.5(L$

Equation 16-3 $1.2(D + F) + 1.6(L_r \text{ or } S \text{ or } R) + 1.6H + (f_1L \text{ or } 0.5W)$

Equation 16-4 $1.2(D + F) + 1.0W + f_1L + 1.6H + 0.5(L_r \text{ or } S \text{ or } R)$

Equation 16-5 $1.2(D + F) + 1.0E + f_1L + 1.6H + f_2S$

Equation 16-6 $0.9D + 1.0W + 1.6H$

Equation 16-7 $0.9(D + F) + 1.0E + 1.6H$

Some filler text

****Table 02 - **Roof unit dead loads**

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variable	value	[value]	description
ld1	2.0 psf	0.10 KPa	Urethane foam (4 inch thick)
ld2	1.0 psf	0.05 KPa	Three-ply roofing
ld3	5.0 psf	0.24 KPa	Doug Fir decking 2-in.
ld4	1.0 psf	0.05 KPa	Doug Fir beams 4x12 at 12 ft o.c.
-----	-----	-----	-----
roofdl1	9.0 psf	0.43 KPa	Total roof unit load

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****Table 03 - **Floor unit dead loads**

T03 - 03

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variable	value	[value]	description
ld1	3.0 psf	0.14 KPa	3/4 in. hardwood flooring
ld2	2.0 psf	0.10 KPa	1/2 in. plywood subfloor
ld3	4.0 psf	0.19 KPa	2x10 joists at 16 in. o.c.
ld4	1.5 psf	0.07 KPa	fixtures
-----	-----	-----	-----
floordl1	10.5 psf	0.50 KPa	Total floor unit load
=====	=====	=====	=====

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****Table 04 - **Interior wall unit dead loads**

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variable	value	[value]	description
ld1	5.5 psf	0.26 KPa	5/8" sheet rock (2)
ld2	2 psf	0.10 KPa	2x4 studs at 16" o.c.
ld3	1.5 psf	0.07 KPa	fixtures
intwalld1	9 psf	0.43 KPa	Total interior wall unit load

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****Table 05 - ****Exterior wall unit dead loads

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variable	value	[value]	description
ld1	2.0 psf	0.10 KPa	1/2 in plywood sheathing
ld2	2.0 psf	0.10 KPa	2x4 studs at 16 in o.c.
ld3	3.0 psf	0.14 KPa	5/8 in sheet rock
ld4	1.5 psf	0.07 KPa	fixtures
-----	-----	-----	-----
extwalldl1	8.5 psf	0.41 KPa	Total exterior wall unit load
=====	=====	=====	=====

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****Table 06 - **Areas**

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****Equ. 02 - **Roof weight**

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****Equ. 02 - **** $\text{rfwt1} = \text{arearf1} * \text{roofdl1}$

E02 - 03

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****Equ. 03 - ****Floor weight

E03 - 03

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****Equ. 03 - **** $\text{flrwt1} = \text{areaflr1} * \text{floordl1}$

E03 - 03

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****Equ. 04 - ****Partition weight

E04 - 03

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****Equ. 04 - ****partwt1 = htwall1 * lenwall1 * intwalldl1

E04 - 03

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****Equ. 05 - ****Exterior wall weight

E05 - 03

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****Equ. 05 - ****exwallwt1 = htwall1 * lenwall2 * extwalldl1

E05 - 03

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****Equ. 06 - ****Total building weight

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****Equ. 06 - **** $\text{totwt1} = \text{rfwt1} + \text{flrwt1} + \text{partwt1} + \text{exwallwt1}$

E06 - 03

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****Table 07 - ****Weights

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Because the T&G roof is relatively more flexible, the effective floor load for seismic models is calculated as the sum of the floor and all of the partition weight.

****Equ. 07 - ****Effective model floor load

E07 - 04

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****Equ. 07 - **** $\text{eflrdl1} = (\text{flrwt1} + \text{partwt1})/(\text{areaflr1})$

E07 - 04

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****Equ. 08 - **** Effective model floor density

E08 - 04

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****Equ. 08 - **** $\text{eflrdens1} = \text{eflrdl1}/(0.5 \cdot \text{IN})$

E08 - 04

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****Equ. 09 - ****Effective model roof density

E09 - 04

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****Equ. 09 - ****erfdens1 = roofdl1/(1.5*IN)

E09 - 04

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****Equ. 10 - ****Effective model wall density

E10 - 04

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****Equ. 10 - **** $\text{ewalldens1} = \text{extwalldl1}/(0.5 \cdot \text{IN})$

E10 - 04

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****Table 08 - ****Model loads

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