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List of Tables

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List of Figures

This report describes the structural design residential solar canopy in the City of Larkspur, California. It includes the design of a concrete slab, stem wall, steel tube frame, and attachments of solar panels to the frame.

The report is divided into the following three divisions:

• 01 Loads: gravity, wind and seismic

• 02 Frame: steel tubes, connections and clips

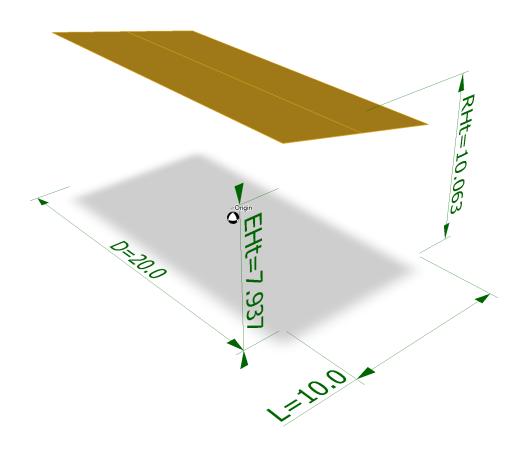
• 03 Foundation: slab and stem wall

Client:

Date:

Location:

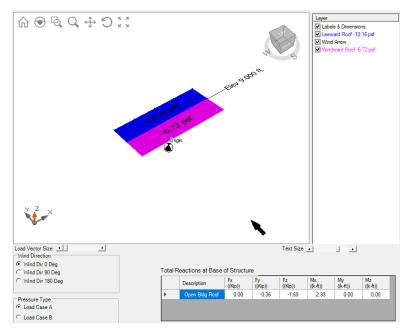
The project is located in Larkspur, California.





Wind load 1 Fig. 02 [02]

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Wind load 2 Fig. 04 [02]

The permit approval is under the jurisdiction of the City of Larkspur, California which adopted the 2019 California Building Code [CBC] and the 2019 California Residential Code [CRC] as the basis for permiting requirements of the CBC.

1.0leftleftleft

Category Standard Year

Loading ASCE-7 2016

Concrete ACI-318 2014

Wood-National Design Specifications AWC-NDS 2018

Wood-Special Design Provisions for Wind and Seismic AWC-SDPWS 2015

Wood Frame Construction Manual AWC-WFCM 2018

Basic loads and load combinations are derived from the California Building and Residential Codes. 1.0leftleftleft Sym Load Effect Notes

D Dead load See IBC 1606 and Chapter 3 of this publication

E Combined effect of horizontal and vertical earthquake- induced forces as defined in ASCE/SEI 12.4.2 See IBC 1613, ASCE/SEI 12.4.2 and Chapter 6 of this publication

Em Maximum seismic load effect of horizontal and vertical forces as set forth in ASCE/SEI 12.4.3 See IBC 1613, ASCE/SEI 12.4.3 and Chapter 6 of this publication

H Load due to lateral earth pressures, ground water pressure or pressure of bulk materials See IBC 1610 for soil lateral loads

L Live load, except roof live load, including any permitted live load reduction See IBC 1607 and Chapter 3 of this publication

Li Roof live load including any permitted live load reduction See IBC 1607 and Chapter 3 of this publication

R Rain load See IBC 1611 and Chapter 3 of this publication

W Load due to wind pressure See IBC 1609 and Chapter 5 of this publication

1.0centercenter CBC 2019 reference Equation

```
Equation 16-1 1.4(D +F)
```

Equation 16-2 1.2(D + F) + 1.6(L + H) + 0.5(L

Equation 16-3 1.2(D + F) + 1.6(Lr or S or R) + 1.6H + (f1L or 0.5W)

Equation 16-4 1.2(D + F) + 1.0W + f1L + 1.6H + 0.5(Lr or S or R)

Equation 16-5 1.2(D + F) + 1.0E + f1L + l.6H + f2S

Equation 16-6 0.9D+ l.0W+ l.6H

Equation 16-7 0.9(D + F) + 1.0E + 1.6H

| Some filler text ** Table: 02 [03]** ====== variation | Table: 02 [03] ======== iable value [value] descrip- |
|---|---|
| tion ==================================== | ======== ld1 2.0 psf 0.10 |
| KPa Urethane foam (4 inch thick) ld2 1.0 psf 0.05 KPa Three-ply roofing le | d3 5.0 psf 0.24 KPa Doug Fir |
| decking 2-in. ld4 1.0 psf 0.05 KPa Doug Fir beams $4x12$ at 12 ft o.c. $$ | roofdl1 9.0 psf 0.43 |
| KPa Total roof unit load ==================================== | ======================================= |
| ** Table: 04 [03]** | Table: 04 |
| [03] ==================================== | L 3 |
| description ==================================== | ======== ld1 3.0 psf 0.14 |
| KPa 3/4 in. hardwood flooring ld2 2.0 psf 0.10 KPa 1/2 in. plywood su 2x10 joists at 16 in. o.c. ld4 1.5 psf 0.07 KPa fixtures ———————————————————————————————————— | ordl1 10.5 psf 0.50 KPa Total |
| [03]** | Table: |
| 06 [03] ==================================== | |
| [value] description ======== =========================== | |
| 5.5 psf 0.26 KPa 5/8" sheet rock (2) ld2 2 psf 0.10 KPa 2x4 studs at 16" o | |
| tures —— —— intwalldl1 9 psf 0.43 KPa Total interior wall unit le | - |
| ======= ** Table: 08 [03]** | Table: |
| 08 [03] ==================================== | ====================================== |
| [value] description ==================================== | ====== ld1 |
| 2.0 psf 0.10 KPa 1/2 in plywood sheathing ld2 2.0 psf 0.10 KPa 2x4 studs | |
| KPa 5/8 in sheet rock ld4 1.5 psf 0.07 KPa fixtures —— —— extw | alldl1 8.5 psf 0.41 KPa Total |
| exterior wall unit load ======== =========================== | |
| exterior wan unit load | |
| ** Table: 10 [03]** | Table: 10 [03] |
| ** Table: 10 [03]** Roof weight | Table: 10 [03] Equ. 02 [03] |
| ** Table: 10 [03]** Roof weight rfwt1 = arearf1 * roofdl1 | Table: 10 [03] Equ. 02 [03] Equ. 03 [03] |
| ** Table: 10 [03]** Roof weight rfwt1 = arearf1 * roofdl1 Floor weight | Table: 10 [03] Equ. 02 [03] Equ. 03 [03] Equ. 05 [03] |
| ** Table: 10 [03]** Roof weight rfwt1 = arearf1 * roofdl1 Floor weight flrwt1 = areaflr1 * floordl1 | Table: 10 [03] Equ. 02 [03] Equ. 03 [03] Equ. 05 [03] Equ. 06 [03] |
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| ** Table: 10 [03]** Roof weight rfwt1 = arearf1 * roofdl1 Floor weight flrwt1 = areaflr1 * floordl1 Partition weight partwt1 = htwall1 * lenwall1 * intwalldl1 Exterior wall weight exwallwt1 = htwall1 * lenwall2 * extwalldl1 Total building weight totwt1 = rfwt1 + flrwt1 + partwt1 + exwallwt1 ** Table: 12 [03]** Because the T&G roof is relatively more flexible, the effective floor load lated as the sum of the floor and all of the partition weight. Effective model floor load eflrdl1 = (flrwt1 + partwt1)/(areaflr1) Effective model floor density eflrdens1 = eflrdl1/(0.5*IN) | Table: 10 [03] |

ewalldens1 = extwalldl1/(0.5*IN) ** Table: 14 [04]**

Equ. 27 [04]

Table: 14 [04]**

Table: 14 [04] .. raw:: latex