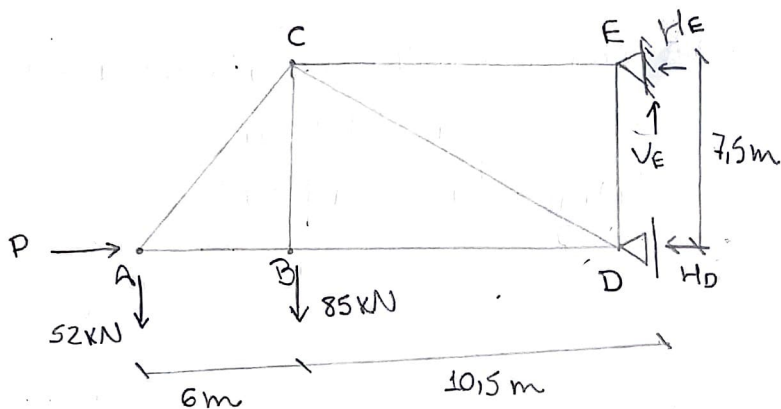


Mecânica dos Sólidos 3 - AB2.2

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1.



$$A = 1600 \text{ mm}^2 = 0,0016 \text{ m}^2$$

$$E = 200 \text{ GPa} = 2 \cdot 10^8 \text{ kPa}$$

* Reações de Apoio

$$\sum F_y = 0$$

$$-P - 85 + V_E = 0 \rightarrow V_E = 52 + 85$$

$$\boxed{V_E = 137 \text{ kN}}$$

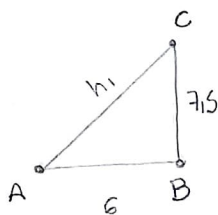
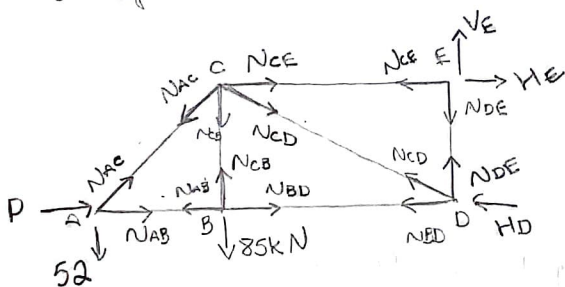
$$\sum M_E = 0$$

$$52 \cdot 16,5 + P \cdot 7,5 + 85 \cdot 10,5 - H_D \cdot 7,5 = 0 \rightarrow H_D = \frac{858 + 7,5P + 892,5}{7,5}$$

$$H_D = \frac{1750,5 + 7,5P}{7,5}$$

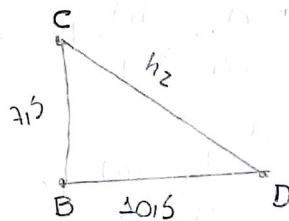
$$\boxed{H_D = 233,4 + P}$$

* Diagrama de corpo livre (Método dos Nós)



$$h_1 = \sqrt{6^2 + 7,5^2}$$

$$h_1 = 9,6$$



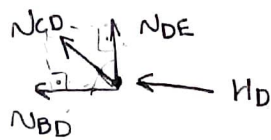
$$h_2 = \sqrt{7,5^2 + 10,5^2}$$

$$h_2 = 12,9$$

* Nô F

$$\sum F_y = 0 \rightarrow V_E - N_{DE} = 0 \rightarrow 52 + 85 - N_{DE} = 0 \rightarrow N_{DE} = 52 + 85 \rightarrow \boxed{N_{DE} = 137 \text{ kN}}$$

$$\sum F_x = 0 \rightarrow H_E - N_{CE} = 0 \rightarrow -233,4 - N_{CE} = 0 \rightarrow \boxed{N_{CE} = -233,4 \text{ kN}}$$



* Nº D

$$\sum F_y = N_{DE} + N_{CD} \cdot \left(\frac{7,5}{12,9} \right) = 0 \rightarrow 137 + 0,581 N_{CD} = 0 \rightarrow \boxed{N_{CD} = -235,7 \text{ kN}}$$

$$\sum F_x = 0 \rightarrow -N_{BD} - H_D - N_{CD} \left(\frac{10,5}{12,9} \right) = 0 \rightarrow N_{BD} = -233,4 - P + 235,7 \cdot 0,814 = 0$$

$$N_{BD} = -233,4 - P + 191,85$$

$$\boxed{N_{BD} = -41,6 - P} \text{ kN}$$

* Nº B

$$\sum F_y = 0 \rightarrow N_{CB} - 85 = 0 \rightarrow \boxed{N_{CB} = 85 \text{ kN}}$$

$$\sum F_x = 0 \rightarrow -N_{AB} + N_{BD} = 0 \rightarrow N_{AB} = N_{BD} \rightarrow \boxed{N_{AB} = (-41,6 - P) \text{ kN}}$$

* Nº A

$$\sum F_y = 0 \rightarrow -52 + N_{AC} \left(\frac{7,5}{9,6} \right) = 0 \rightarrow \boxed{N_{AC} = 66,59 \text{ kN}}$$

* Teorema de Castigliano ($P=0$)

$$\Delta_{DE} = \int_0^{7,5} \frac{N_{DE}}{EA} \cdot \frac{dN}{dP} dx = 0 \text{ m}$$

$$\Delta_{CE} = \int_0^{10,5} \frac{N_{CE}}{EA} \cdot \frac{dN}{dP} dx = 0 \text{ m}$$

$$\Delta_{CD} = \int_0^{12,9} \frac{N_{CD}}{EA} \cdot \frac{dN}{dP} dx = 0 \text{ m}$$

$$\Delta_{BD} = \int_0^{10,5} \frac{N_{BD}}{EA} \cdot \frac{dN}{dP} dx = \int_0^{10,5} (-41,6 - P) \cdot (-1) dx = 1,365 \cdot 10^{-6} \text{ m}$$

$$\Delta_{CB} = \int_0^{7,5} \frac{N_{CB}}{EA} \cdot \frac{dN}{dP} dx = 0 \text{ m}$$

$$\Delta_{AB} = \int_0^6 \frac{N_{AB}}{EA} \cdot \frac{dN}{dP} dx = \int_0^6 (-41,6 - P) \cdot (-1) dx = 7,8 \cdot 10^{-7} \text{ m}$$

$$\Delta_{AC} = \int_0^{9,6} \frac{N_{AC}}{EA} \cdot \frac{dN}{dP} dx = 0$$

sendo assim, o deslocamento horizontal em A:

$$\Delta_A = \Delta_{DE} + \Delta_{CE} + \Delta_{CD} + \Delta_{BD} + \Delta_{CD} + \Delta_{BD} + \Delta_{CB} + \Delta_{AB} + \Delta_{AC}$$

$$\Delta_A = 2,145 \cdot 10^{-3} \text{ m (esquerda para direita)}$$

2.

$$\sigma_{adm} = 250 \text{ MPa}$$

$$E = 200 \text{ GPa}$$

$$I_z = 128 \cdot 10^6 \text{ mm}^4$$

$$I_y = 18,4 \cdot 10^6 \text{ mm}^4$$

$$r_z = 130 \text{ mm}$$

$$n_f = 2$$

Utilizando a tabela para uma configuração com um engaste e uma extremidade livre $\rightarrow K=2$

* Carga Crítica (plano XZ)

$$P_{cr} = \frac{\pi^2 \cdot E \cdot I_y}{(KL)^2} = \frac{\pi^2 \cdot 200 \cdot 10^9 \cdot (18,4 \cdot 10^6 \cdot 10^{-12})}{(2 \cdot 0,7)^2}$$

$$P_{cr} = 18530,6 \text{ kN}$$

* Carga Crítica (plano XY)

$$P_{cr} = \frac{\pi^2 \cdot E \cdot I_z}{(KL)^2} = \frac{\pi^2 \cdot 200 \cdot 10^9 \cdot (128 \cdot 10^6 \cdot 10^{-12})}{(2 \cdot 0,9)^2}$$

$$P_{cr} = 779,821 \text{ kN}$$

então, a carga crítica adotada será a menor ($P_{cr} = 779,821 \text{ kN}$)

* Tensão

$$\sigma_{cr} = \frac{\pi^2 \cdot E}{\left(\frac{KL}{r}\right)^2} = \frac{\pi^2 \cdot 200 \cdot 10^9}{\left(\frac{2 \cdot 0,9}{130 \cdot 10^{-3}}\right)^2} = 102,9 \text{ MPa}$$

Como $\sigma_{cr} < \sigma_{adm}$, os cálculos são válidos.

* Utilizando os critérios de estabilidade

$$\sigma \leq \frac{\sigma_{cr}}{n}$$

$$\frac{P}{A} \leq \frac{P_{cr}}{A \cdot n}$$

sendo assim,

$$P \leq \frac{P_{cr}}{n}$$

$$P \leq \frac{779,821}{2}$$

$$P \leq 389,91 \text{ kN}$$

Então,

a carga admissível é 389,91 kN