

A detailed free-body diagram of a truss structure. The truss consists of joints A, B, C, D, and E. Joints A and B are supports at the bottom left, while D and E are roller supports at the bottom right. The top chord includes joints C and E. Horizontal dimensions are 6 m between A and B, and 10.5 m between B and D. Vertical dimensions are 7.5 m from the bottom chord to joint C, and another 7.5 m from joint C to joint E. External loads include a horizontal force P at joint A pointing left, vertical downward forces of 52 kN at A and 85 kN at B, and reaction forces R_{vd} (vertical) and R_{hd} (horizontal) at joint E. Internal member forces are labeled as N_{ac}, N_{bc}, N_{cd}, N_{ce}, N_{de}, N_{bd}, N_{ab}, and N_{be}. Angles θ₁ and θ₂ are indicated at joints A and D respectively, relative to the horizontal members.

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10)

Segue a aplicação do 2º teorema de Castiglione

$$\text{usando } \Delta = \sum_{i \in A} \int_{\epsilon A} \frac{U_i}{A \cdot E} \cdot \frac{\partial U_i}{\partial P} dx_i$$

$$\begin{aligned} -2.357037123 \cdot 10^5, Nce = 2.334000000 \cdot 10^5, Nde = 1.37000 \cdot 10^5, Rhd = -2.334000000 \cdot 10^5 \\ + P, Rhe = 2.334000000 \cdot 10^5, Rve = 1.37000 \cdot 10^5 \end{aligned}$$

assign(%)

$$\Delta_{ab} := \text{subs}\left(P=0, \frac{Nab}{A \cdot E} \cdot \frac{d}{dP} (Nab) \cdot 6\right) \quad -0.0007800000000 \quad (4)$$

$$\Delta_{bd} := \text{subs}\left(P=0, \frac{Nbd}{A \cdot E} \cdot \frac{d}{dP} (Nbd) \cdot 10.5\right) \quad -0.001365000000 \quad (5)$$

$$\Delta_{ac} := \text{subs}\left(P=0, \frac{Nac}{A \cdot E} \cdot \frac{d}{dP} (Nac) \cdot Dac\right) \quad 0. \quad (6)$$

$$\Delta_{bc} := \text{subs}\left(P=0, \frac{Nbc}{A \cdot E} \cdot \frac{d}{dP} (Nbc) \cdot 7.5\right) \quad 0. \quad (7)$$

$$\Delta_{cd} := \text{subs}\left(P=0, \frac{Ncd}{A \cdot E} \cdot \frac{d}{dP} (Ncd) \cdot Dcd\right) \quad -0. \quad (8)$$

$$\Delta_{de} := \text{subs}\left(P=0, \frac{Nde}{A \cdot E} \cdot \frac{d}{dP} (Nde) \cdot 7.5\right) \quad 0. \quad (9)$$

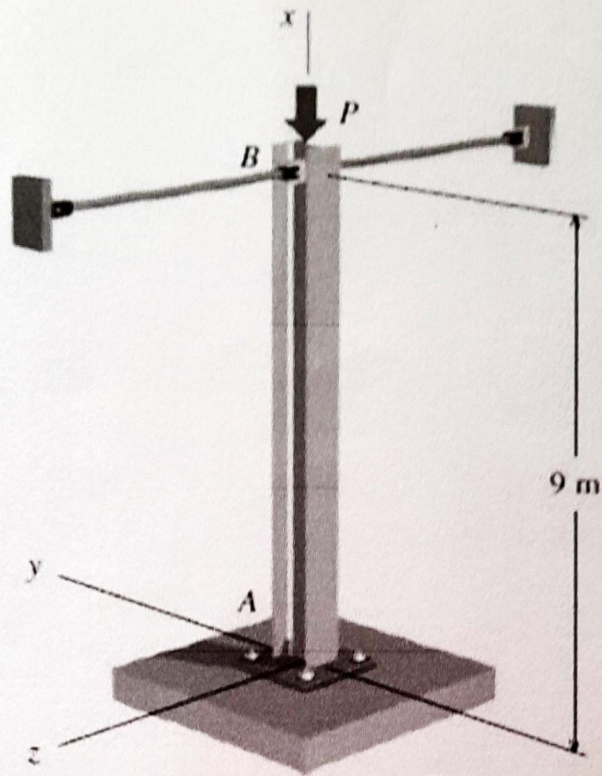
$$\Delta_{ce} := \text{subs}\left(P=0, \frac{Nce}{A \cdot E} \cdot \frac{d}{dP} (Nce) \cdot 10.5\right) \quad 0. \quad (10)$$

$$\Delta := \Delta_{ab} + \Delta_{bd} + \Delta_{ac} + \Delta_{bc} + \Delta_{cd} + \Delta_{de} + \Delta_{ce} \quad -0.002145000000 \quad (11)$$

Como P está para a esquerda, o sinal negativo indica que o deslocamento em A é de 2,145 mm para a direita

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Todas as cortes foram feitas usando o Maple

2.)

Convertendo dados

$$\begin{aligned} & \text{restart} \\ & E := \text{convert}(200, \text{units}, \text{GPa}, \text{Pa}) && 200000000000 && (1) \\ & L := 9 && 9 && (2) \\ & \sigma_{adm} := \text{convert}(250, \text{units}, \text{MPa}, \text{Pa}) && 250000000 && (3) \\ & I_z := \text{convert}(128 \cdot 10^6, \text{units}, \text{mm}^4, \text{m}^4) && 0.0001280000000 && (4) \\ & I_y := \text{convert}(18.4 \cdot 10^6, \text{units}, \text{mm}^4, \text{m}^4) && 0.00001840000000 && (5) \\ & r_z := \text{convert}(130., \text{units}, \text{mm}, \text{m}) && 0.1300000000 && (6) \end{aligned}$$

$$A := \text{solve}\left(r_z = \sqrt{\frac{I_z}{A}}, A\right) \rightarrow \text{Achando área através do raio de giração} \\ 0.007573964498 \quad (7)$$

Estabilidade

$$x_1 := \text{solve}\left(\frac{x_1}{A} = \sigma_{adm}, x_1\right) \rightarrow \text{Carga considerando critério de resistência} \\ 1.893491124 \cdot 10^6 \quad (8)$$

Carga adm

$$\left\{ \sigma_y := \frac{\pi^2 \cdot E \cdot I_y}{A \cdot (L \cdot 0.699)^2} \right\} \rightarrow \text{Tensões crítica considerando flambagem em torno do eixo y} \\ 1.227680007 \cdot 10^7 \pi^2 \quad (9)$$

$$\left\{ \frac{\sigma_y}{2} \cdot A \right\} \rightarrow \text{Cálculo da força com coef. de segurança} \\ 46492.02394 \pi^2 \quad (10)$$

$$\xrightarrow{\text{em 5 dígitos}} 4.5886 \cdot 10^5 \quad (11)$$

$$x_2 := \text{convert}(4.5886 \cdot 10^5, \text{units}, \text{N}, \text{kN}) \\ 458.8600000 \text{ kN (Força adm)} \quad (12)$$

Carga adm

$$\left\{ \sigma_z := \frac{\pi^2 \cdot E \cdot I_z}{A \cdot (L \cdot 2)^2} \right\} \rightarrow \text{Tensões crítica considerando flambagem em torno do eixo z} \\ 1.043209876 \cdot 10^7 \pi^2 \quad (13)$$

$$\xrightarrow{\text{em 5 dígitos}} 1.0296 \cdot 10^8 \quad (14)$$

$$\left\{ \frac{\sigma_z}{2} \cdot A \right\} \rightarrow \text{Cálculo da força com coef. de segurança}$$

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$$39506.17282 \pi^2 \quad (15)$$

em 5 dígitos →

$$3.8991 \cdot 10^5 \quad (16)$$

$$x_3 := \text{convert}(3.8991 \cdot 10^5, \text{units}, N, kN)$$

$$389.9100000 \text{ kN} \quad (\text{força adm}) \quad (17)$$

Avaliando os três forças encontrados com
 os critérios de resistência e estabilidade,
 chegamos a conclusão que $P_{adm} = \underline{389,91 \text{ kN}}$
 (~~caso~~ caso em que a barra flamba em torno
 do eixo 3)