

3)

↳ Para o plano XY, $m_1 = 2 \rightarrow$ fixo - livre

$$P_{cr} = \frac{\pi^2 EI_z}{(K \cdot L)_z^2} = \frac{\pi^2 (200.000 \text{ N/mm}^2) (128 \times 10^6 \text{ mm}^4)}{[2 \cdot 9000 \text{ mm}]^2}$$

↳ $P_{cr} = 779,8 \text{ kN}$

• Índice de esbeltez:

$$\lambda = \frac{L_e}{r_z} = \frac{K \cdot L}{r_z} = \frac{2 \cdot 9000 \text{ mm}}{130 \text{ mm}} = 138,5$$

↳ Para o plano XZ, $m_2 = 0,7 \rightarrow$ fixo - articulado

$$P_{cr} = \frac{\pi^2 EI}{(KL)_y^2} = \frac{\pi^2 (200.000 \text{ N/mm}^2) (118,4 \times 10^6 \text{ mm}^4)}{[0,7 \cdot 9000 \text{ mm}]^2}$$

↳ $P_{cr} = 915,09 \approx \underline{915,1 \text{ kN}}$

Logo, o menor P_{cr} é 779,8 kN.

↳ Tensão crítica:



• Tensão crítica:

$$\sigma_{cr} = \frac{\pi^2 \cdot E}{([\lambda]^2)} = \frac{\pi^2 \cdot 200.000}{[138,5]^2} = \underline{102,9 \text{ MPa}}$$

• $102,9 \text{ MPa} < 250 \text{ MPa}$, logo o critério de resistência é válido.

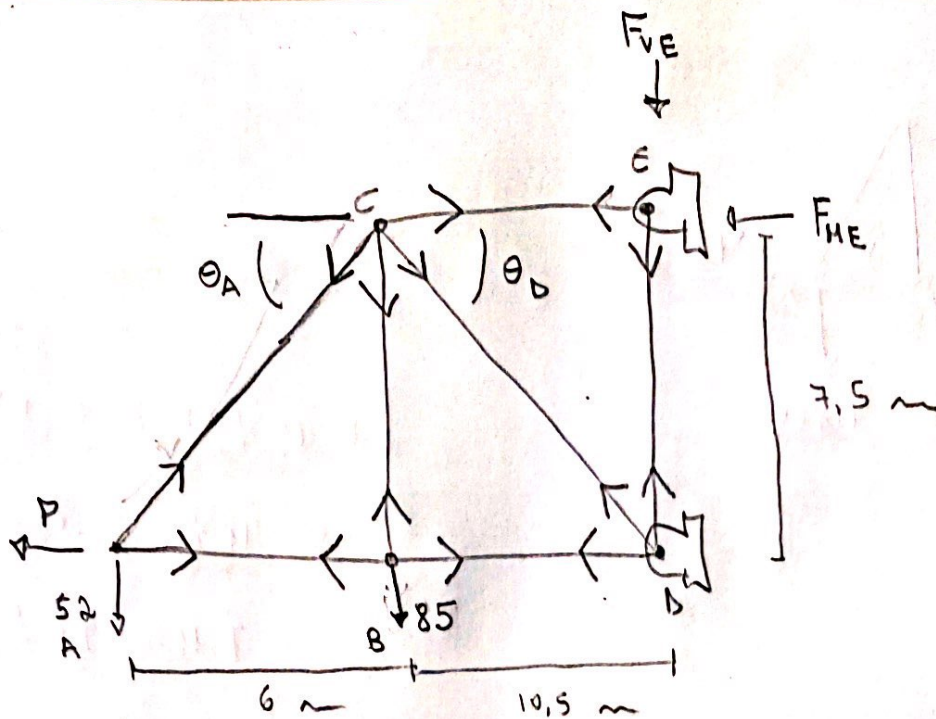
↳ Critério de estabilidade

$$\sigma \leq \frac{\sigma_{cr}}{m_1}$$

$$\frac{P}{A} \leq \frac{P_{cr}}{A \cdot m_1} \rightarrow P \leq \frac{779,88}{2} \Rightarrow P \leq \underline{389,9 \text{ kN}}$$

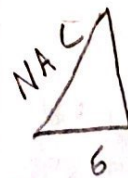
• Ou seja, a carga admissível no sistema é 389,9 kN

2)

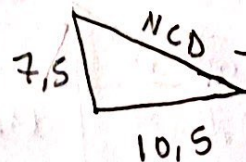


$$\theta_A = \tan^{-1} \left(\frac{7.5}{6} \right) = 0.89$$

$$\theta_D = \tan^{-1} \left(\frac{7.5}{10.5} \right) = 0.62$$



$$N_{AC} = 9.6 \text{ m}$$



$$N_{CD} = 12.9 \text{ m}$$

$$\sum F_{HA} = N_{AB} - P + N_{AC} \cdot \cos \theta_A = 0$$

$$\sum F_{VA} = -52 + N_{AC} \cdot \sin \theta_A = 0$$

$$\sum F_{HB} = -N_{AB} + N_{BD} = 0$$

$$\sum F_{VB} = N_{CB} - 85 = 0 \rightarrow N_{CB} = 85$$

$$\sum F_{HC} = -N_{AB} \cdot \cos \theta_A + N_{CE} + N_{CD} \cdot \cos \theta_D = 0$$

$$\sum F_{VC} = -N_{CB} - N_{AC} \cdot \sin \theta_A - N_{CD} \cdot \sin \theta_D = 0$$

$$\sum F_{HD} = -N_{BD} - N_{CD} \cdot \cos \theta_D - H_D = 0$$

$$\sum F_{VD} = N_{ED} + N_{CD} \cdot \sin \theta_D = 0$$

$$\sum F_{HE} = -H_E - N_{CB} = 0$$

$$\sum F_{VE} = -V_E - N_{ED} = 0$$

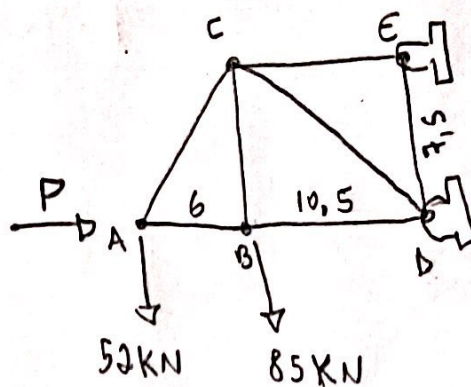
$$H_E = 233.4 \text{ kN} \rightarrow$$

$$V_E = 137.0 \text{ kN} \uparrow$$

$$H_D = P + 233.4 \leftarrow$$

2)

↳ Continuação:



$$\bullet F_{HB} = P + 233,4 \text{ kN} \leftarrow$$

$$\bullet F_{HE} = 233,4 \text{ kN} \rightarrow$$

$$\bullet F_{VE} = 137 \text{ kN} \uparrow$$

Tramo	F	$\frac{\delta F}{\delta P}$	F (P=0 kN)	L	$\left(\frac{\delta F}{\delta P}\right) \cdot F \cdot L$
AB	$-P - 43,6 \text{ kN}$	-1	-43,6 kN	6.000 mm	-2619,6 kN·mm
AC	66,59 kN	0	66,59 kN	9.604,6 mm	0
BC	85 kN	0	85 kN	7.500 mm	0
BD	$-P - 43,6 \text{ kN}$	-1	-43,6 kN	10.500 mm	-4587,6 kN·mm
CD	-235,7 kN	0	-235,7 kN	12.903,4 mm	0
CE	233,4	0	233,4 kN	10.500 mm	0
DE	137 kN	0	137 kN	7.500 mm	0

• Castiglione: $\Delta_A = \frac{1}{AE} \sum N \cdot \frac{\delta N}{\delta P} \cdot L$

$$\Delta_A = \frac{1}{AE} \left(N_{AB} \cdot \frac{\delta N_{AB}}{\delta P} \cdot L_{AB} + N_{BD} \cdot \frac{\delta N_{BD}}{\delta P} \cdot L_{BD} \right)$$

$$\Delta_A = \frac{1}{3,2 \times 10^5} \cdot \left((-43,6) \cdot (-1) \cdot 6 + (-43,6) \cdot (-1) \cdot 10,5 \right)$$

$$\Delta_A = \frac{(686,400,000 \text{ kN})}{(3,2 \times 10^5 \text{ kN/mm}^2)} = 0,00215 \text{ m ou } 2,15 \text{ mm}$$