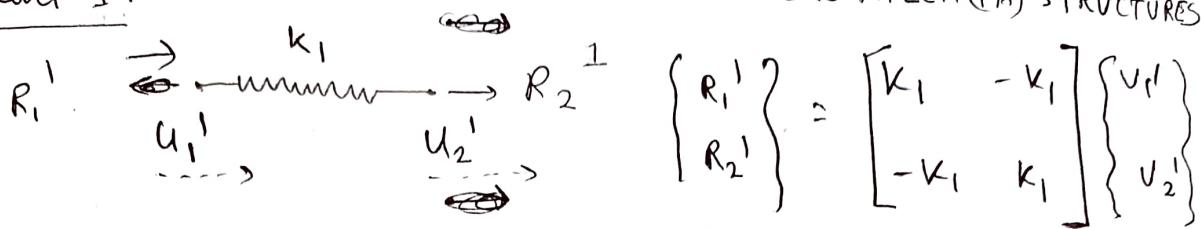
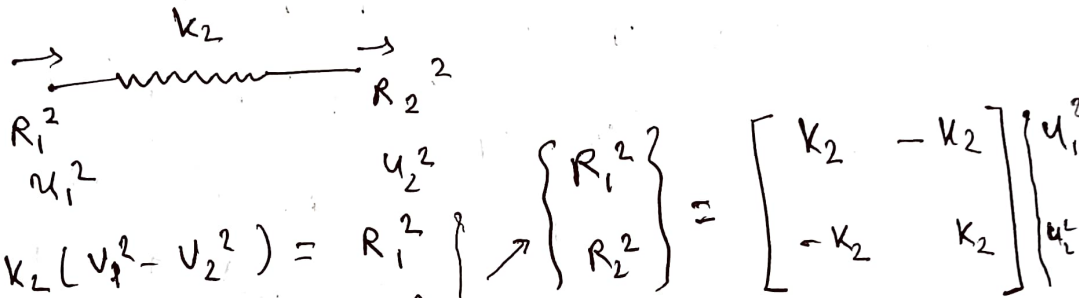


1. Using Direct Method:  
Element-1:



$$\left. \begin{aligned} k_1(-u_2^1 + u_1^1) &= R_1^1 \\ k_1(-u_1^1 + u_2^1) &= R_2^1 \end{aligned} \right\} \rightarrow$$

Element-2:



$$\left. \begin{aligned} k_2(u_2^2 - u_1^2) &= R_1^2 \\ k_2(u_1^2 - u_2^2) &= R_2^2 \end{aligned} \right\} \rightarrow$$

Similarly for nth element

$$\begin{Bmatrix} R_1^n \\ R_2^n \end{Bmatrix} = \begin{bmatrix} k_n & -k_n \\ -k_n & k_n \end{bmatrix} \begin{Bmatrix} u_1^n \\ u_2^n \end{Bmatrix}$$

Writing equilibrium eq<sup>n</sup>

@ 4 nodes.

Leftmost node is taken as 4th node.

@ 4th node,  $R_1^1 = R_4$

@ 1st node,  $R_2^1 + R_1^2 + R_1^3 + R_1^4 = R_1$

@ 2nd node,  $R_2^2 + R_2^3 + R_1^5 = R_2$

@ 3rd node,  $R_2^4 + R_2^5 = R_3$

Assembling all the matrices we get

$$\begin{Bmatrix} R_4 \\ R_1 \\ R_2 \\ -R_3 \end{Bmatrix} = \begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 [k_1+k_2+k_3+k_4] & -k_2-k_3 & -k_4 \\ 0 & -k_2-k_3 & k_2+k_3+k_5 & -k_5 \\ 0 & -k_4 & -k_5(k_4+k_5) \end{bmatrix} \begin{Bmatrix} U_4 \\ U_1 \\ U_2 \\ U_3 \end{Bmatrix} \quad - (A)$$

Energy method:

$$\begin{aligned} \Pi &= \frac{1}{2} k_1 (U_2^1 - U_1^1)^2 - R_1^1 U_1^1 - R_2^1 U_2^1 \\ &+ \frac{1}{2} k_2 (U_2^2 - U_1^2)^2 - R_1^2 U_1^2 - R_2^2 U_2^2 \\ &+ \frac{1}{2} k_3 (U_2^3 - U_1^3)^2 - R_1^3 U_1^3 - R_2^3 U_2^3 \\ &+ \frac{1}{2} k_4 (U_2^4 - U_1^4)^2 - R_1^4 U_1^4 - R_2^4 U_2^4 \\ &+ \frac{1}{2} k_5 (U_2^5 - U_1^5)^2 - R_1^5 U_1^5 - R_2^5 U_2^5 \\ &= \frac{1}{2} k_1 (U_1 - U_4)^2 + \frac{1}{2} k_2 (U_2 - U_1)^2 + \frac{1}{2} k_3 (U_2 - U_1)^2 \\ &+ \frac{1}{2} k_4 (U_3 - U_1)^2 + \frac{1}{2} k_5 (U_3 - U_2)^2 \\ &- R_4 U_4 - R_1 U_1 - R_2 U_2 - R_3 U_3 \end{aligned}$$

Now,

$$\frac{\partial \Pi}{\partial U_1} = 0 \Rightarrow \frac{1}{2} k_1 \times 2 (U_1 - U_4) - \frac{1}{2} k_2 \times 2 (U_2 - U_1) (k_2 + k_3) - \frac{1}{2} k_4 \times 2 (U_3 - U_1) - R_1 = 0$$

$$R_1 = U_1 (k_1 + k_2 + k_3 + k_4) - k_1 U_4 - U_2 (-k_2 - k_3) - k_4 U_3 \quad - (I)$$

$$\frac{\partial \Pi}{\partial U_2} = 0$$

$$R_2 = U_1 (-k_2 - k_3) + U_2 (k_2 + k_3 + k_5) + U_3 (-k_5) \quad - (2)$$

$$\frac{\partial \Pi}{\partial U_3} = 0$$

$$R_3 = U_1(-k_4) + U_2(-k_5) + U_3(k_4 + k_5) \quad \text{--- (3)}$$

$$\frac{\partial \Pi}{\partial U_4} = 0$$

$$R_4 = U_4(k_1) + U_1(-k_1) \quad \text{--- (4)}$$

From eq<sup>n</sup> (1), (2), (3) & (4), we get back  
 Same  $\{F\} = [K]\{U\}$  matrix A eq<sup>n</sup> (A).

# PYTHON CODE FOR FEM ASSIGNMENT 1

## question 2

### PART-B

```
In [1]: import numpy as np
import pandas as pd
```

```
In [2]: elementNodes=np.array([[1, 2],[2, 3],[2, 3],[2,4],[3, 4]])
numberElements=len(elementNodes)
numberNodes=4
```

for structure:

displacements: displacement vector

force : force vector

stiffness: stiffness matrix

```
In [3]: displacements=np.zeros((numberNodes,1))
force=np.zeros((numberNodes,1))
stiffness=np.zeros((numberNodes,numberNodes))
```

apply loads at node 2,3,4

```
In [4]: force[1 : 4] = 1
```

assembly of stiffness matrix

```
In [5]: for i in range(numberElements):
    elementDof=elementNodes[i,]
    rows=np.array([[elementDof[0]-1, elementDof[0]-1],
                    [elementDof[1]-1, elementDof[1]-1]], dtype=np.intp)
    columns=np.array([[elementDof[0]-1, elementDof[1]-1],
                      [elementDof[0]-1, elementDof[1]-1]], dtype=np.intp)
    #print(elementDof)
```

```
stiffness[rows,columns]=stiffness[rows,columns]+np.array([[1,-1],[-1,1]])
print(stiffness)
```

```
[[ 1. -1.  0.  0.]
 [-1.  4. -2. -1.]
 [ 0. -2.  3. -1.]
 [ 0. -1. -1.  2.]]
```

## boundary conditions and solution

```
In [6]: prescribedDof=np.array([[0]]) # corresponding to reaction (restricted DOF)
activeDof=np.setdiff1d(np.linspace(0,numberNodes-1,num=numberNodes),prescribedDof)

In [7]: reduced_stiffness1=np.delete(stiffness,prescribedDof , axis=0)
reduced_stiffness=np.delete(reduced_stiffness1,prescribedDof , axis=1)
reduced_force=np.delete(force,prescribedDof , axis=0)
displacements=np.linalg.solve(reduced_stiffness, reduced_force)

In [8]: displacements
final_displacement_vector=np.zeros((numberNodes,1))
final_displacement_vector[activeDof.astype(int)]=final_displacement_vector[activeDof.

In [9]: final_force_vector=np.matmul(stiffness,final_displacement_vector)

In [10]: final_force_vector
Out[10]: array([[ -3.],
               [  1.],
               [  1.],
               [  1.]])

In [11]: final_displacement_vector
Out[11]: array([[0. ],
               [3. ],
               [3.6],
               [3.8]])

In [12]: stiffness
Out[12]: array([[ 1., -1.,  0.,  0.],
               [-1.,  4., -2., -1.],
               [ 0., -2.,  3., -1.],
               [ 0., -1., -1.,  2.]])
```

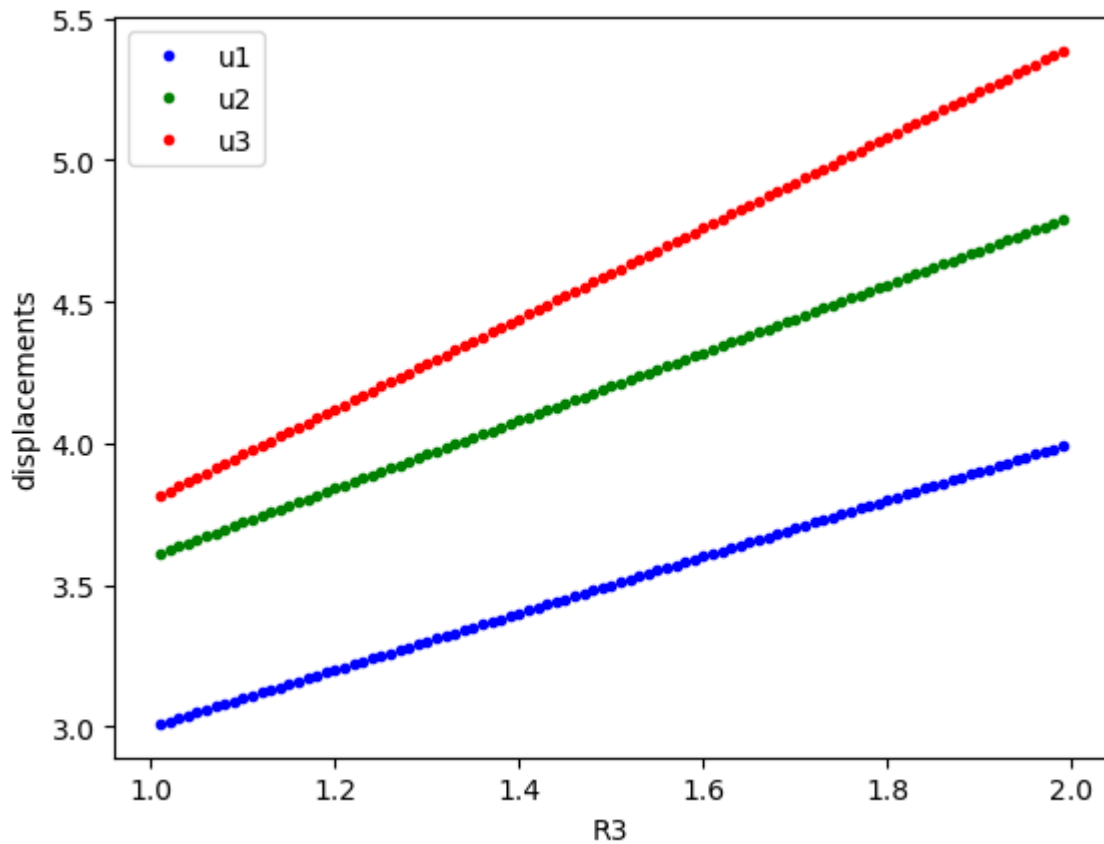
## PART-C

```
In [13]: import matplotlib.pyplot as plt

In [14]: varying_force=reduced_force
accuracy=100
for i in range(1,accuracy):
    varying_force[2,0]=varying_force[2,0]+1/accuracy
    displacements1=np.linalg.solve(reduced_stiffness, varying_force)
    plt.plot(varying_force[2,0],displacements1[0], 'b.')
```

```
plt.plot(varying_force[2,0],displacements1[1],'g.')
plt.plot(varying_force[2,0],displacements1[2],'r.')
plt.xlabel("R3")
plt.ylabel("displacements")
plt.legend(["u1", "u2","u3"], loc=0, frameon=True)
```

Out[14]: <matplotlib.legend.Legend at 0x26080ce2a60>



In [15]: `displacements1` # values at  $R3=2$  for  $u2$   $u3$  and  $u4$  respectively

Out[15]: `array([[3.99 ],  
[4.788],  
[5.384]])`

In [ ]: