

[1] Load Combinations <I>

Table 1: ASCE 7-05 Load Effects



Beam Geometry _[I]

Bending Stress Formula [Eq 1]

$$\sigma = \frac{M}{S}$$

[2] Loads and Geometry <V>

Table 2: Beam Loads and Properties

variable	value	[value]	description
D_1	3.80 psf	0.18 kPA	joists DL
D_2	2.10 psf	0.10 kPA	plywood DL
D_3	10.00 psf	0.48 kPA	partitions DL
D_4	1.00 klf	14.59 kN_m	fixed machinery DL
L_1	40.00 psf	1.92 kPA	ASCE7-05 LL

Beam Geometry [file: beam1-v.csv]

variable	value	[value]	description
W_1	2.00 ft	0.61 m	beam spacing
S_1	14.00 ft	4.27 m	beam span

dead load : ASCE7-05 2.3.2 [Eq 2]

$$dl_1 = 1.2 * (W_1 * (D_1 + D_2 + D_3) + D_4)$$

dl_1	[dl_1]		D_3	D_4	D_1	W_1	D_2
1.24 klf	18.07 kN_m		10.00 psf	klf	3.80 psf	2.00 ft	2.10 psf

live load : ASCE7-05 2.3.2 [Eq 3]

$$ll_1 = 1.6 * W_1 * L_1$$

ll_1	[ll_1]		L_1	W_1
0.13 klf	1.87 kN_m		40.00 psf	2.00 ft

total load : ASCE7-05 2.3.2 [Eq 4]

$$\omega_1 = d_1 + I_1$$

ω_1	$[\omega_1]$		I_1	d_1
1.37 klf	19.94 kN_m		128.00 ft·psf	1.24 klf

[3] Beam Section Properties <V>

rectangular section modulus [Eq 5]

$$S_1 = \text{rectsect}(10 \text{ inch}, 18 \text{ inch})$$

S_1	$[S_1]$		inch
540.00 in ³	8849.01 cm ³		inch

rectangular moment of inertia [Eq 6]

$$I_1 = \text{rectinertia}(10 \text{ inch}, 18 \text{ inch})$$

I_1	$[I_1]$		inch
4860.0 in ⁴	202288.5 cm ⁴		inch

[4] Force and Stress <V>

mid-span UDL moment [Eq 7]

$$M_1 = \omega_1 \cdot S_1^2 / 8$$

M_1	$[M_1]$		ω_1	S_1
33.47 ftkip	45.38 mKN		1.37 klf	14.00 ft

bending stress [Eq 8]

$$f_b_1 = M_1 / S_1$$

f_b_1	$[f_b_1]$		M_1	S_1
743.8 psi	5.1 MPA		33.5 ft ² ·klf	540.0 inch ³