

## [ 1i ] Load Combinations

**Table 1:** ASCE 7-05 Load Effects

Equation No.	Load Combination
16-1	1.4(D+F)
16-2	1.2(D+F+T) + 1.6(L+H) + 0.5(Lr or S or R)
16-3	1.2(D+F+T) + 1.6(Lr or S or R) + (f1L or 0.8W)



Beam Geometry

Bending Stress [Eq 1]

$$\sigma_1 = \frac{M_1}{S_1}$$

## [ 2v ] Loads and Geometry

**Table 2:** Unit Loads

variable	value	[value]	description
D_1	3.80 psf	0.18 kPa	joists DL
D_2	2.10 psf	0.10 kPa	plywood DL
D_3	10.00 psf	0.48 kPa	partitions DL
D_4	1.00 klf	14.59 kN_m	fixed machinery DL
L_1	40.00 psf	1.92 kPa	ASCE7-05 LL

**Table 3:** Beam Geometry [file: beam1.csv]

variable	value	[value]	description
W_1	2.00 ft	0.61 m	beam spacing
S_1	14.00 ft	4.27 m	beam span

Uniform Distributed Loads

dead load : ASCE7-05 2.3.2 [Eq 2]



$$dl_1 = 1.2 \cdot (D_4 + W_1 \cdot (D_1 + D_2 + D_3))$$

<b>dl_1</b>	<b>[dl_1 ]</b>			
1.24 klf	18.07 kN_m			
<b>D_1</b>	<b>D_2</b>	<b>D_4</b>	<b>D_3</b>	<b>W_1</b>
3.80 psf	2.10 psf	klf	10.00 psf	2.00 ft

live load : ASCE7-05 2.3.2 [Eq 3]

$$ll_1 = 1.6 \cdot L_1 \cdot W_1$$

<b>ll_1</b>	<b>[ll_1 ]</b>
0.13 klf	1.87 kN_m
<b>L_1</b>	<b>W_1</b>
40.00 psf	2.00 ft

total load : ASCE7-05 2.3.2 [Eq 4]

$$\omega_1 = dl_1 + ll_1$$

<b>omega_1</b>	<b>[omega_1 ]</b>
1.37 klf	19.94 kN_m
<b>ll_1</b>	<b>dl_1</b>
128.00 ft·psf	1.24 klf

### [ 3v ] Beam Section Properties

[ Python file read: sectprop.py ]

function: rect sect modulus [Eq 5]

```
section1 = rectsect(10·inch, 18·inch)
```

<b>section_1</b>	<b>[section_1 ]</b>
540.00 in <sup>3</sup>	8849.01 cm <sup>3</sup>

inch

inch



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function: rect moment inertia [Eq 6]

```
inertia1 = rectinertia(10·inch, 18·inch)
```

<b>inertia_1</b>	<b>[inertia_1 ]</b>
4860.0 in4	202288.5 cm4

inch

inch

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#### [ 4v ] Force and Stress

mid-span UDL moment [Eq 7]

$$m_1 = \frac{s_1^2 \cdot \omega_1}{8}$$

<b>m_1</b>	<b>[m_1 ]</b>
33.47 ftkip	45.38 mkN
<b>omega_1</b>	<b>s_1</b>
1.37 klf	14.00 ft

bending stress [Eq 8]

$$fb_1 = \frac{m_1}{\text{section}_1}$$

<b>fb_1</b>	<b>[fb_1 ]</b>
743.8 psi	5.1 MPA
<b>m_1</b>	<b>section_1</b>
33.5 ft <sup>2</sup> ·klf	540.0 inch <sup>3</sup>



fb\_1 < 20\*ksi | ok, not ok, blue, red [Eq 9]

