

<sup>1</sup> Examining the Relationships Between Historic Building  
<sup>2</sup> Features and Tornado Damage: A Multi-Model Feature  
<sup>3</sup> Importance Analysis with Statistical Validation

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<sup>10</sup> **Abstract**

<sup>11</sup> Historic masonry buildings constitute significant cultural and economic assets including tornado-prone regions, yet their vulnerability characteristics remain poorly quantified. This study analyzes damage patterns in 382 historic structures exposed to the 2020 Nashville (EF3/EF4) and 2021 Quad State (EF4) tornado outbreak, employing permutation importance and SHapley Additive exPlanations (SHAP) analysis to identify building features governing tornado vulnerability. Feature importance rankings were validated against random noise controls to establish statistical significance. Two modeling approaches reveal distinct insights: hazard-inclusive models (including EF rating) demonstrate that tornado intensity overwhelmingly dominates damage prediction, suppressing building feature effects; hazard-neutral models (excluding intensity) elevate intrinsic structural characteristics, with wall thickness, roof slope, and construction year emerging as top predictors.

Rather than providing prescriptive design specifications, this work establishes a evaluation framework identifying high-priority targets for wind tunnel testing, component-level experimentation, and finite element modeling. The analysis emphasizes preservation-compatible interventions that balance life safety with architectural integrity, guiding resource allocation for the vulnerable historic building stock.

<sup>24</sup> **Keywords:** Tornado Damage, Feature Importance, Historic Buildings, Machine  
<sup>25</sup> Learning, Statistical Validation, Permutation Importance

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26     **1. Introduction**

27       The preservation of historic building stock faces an existential threat due to  
28       the increasing frequency and intensity of severe convective storms. There are over  
29       95,000 historic buildings in the United States [1], they often act as the economic  
30       and cultural anchors of their communities, are particularly vulnerable to tornado-  
31       induced wind loads due to construction practices that predate modern engineering  
32       codes [2]. The devastation of Mayfield, Kentucky's historic district during the 2021  
33       Quad State tornado outbreak serves as a stark reminder of this fragility [3]. While  
34       modern building codes have evolved to improve life safety, historic structures, often  
35       characterized by unreinforced masonry (URM) and gravity-based connections  
36       [4], occupy a precarious position where standard engineering interventions may  
37       conflict with preservation mandates for material integrity and reversibility [5].

38       A challenge in mitigating this risk is the lack of empirical data linking specific  
39       historic building features to tornado performance. Preservation professionals often  
40       rely on anecdotal evidence or generalized wind engineering principles that may not  
41       fully capture the complex failure mechanisms of aged structures. Furthermore, the  
42       "transition zone" of damage, where buildings sustain repairable structural damage  
43       without progressing to total loss, remains poorly understood, yet this is precisely  
44       the domain where preservation interventions are most valuable. This intermediate  
45       damage state, characterized by partial roof loss, wall cracking, or localized  
46       collapse that can be addressed through structural restoration rather than demolition,  
47       represents the critical threshold determining whether historic structures can  
48       be saved following tornado impact. Unlike undamaged buildings (requiring no  
49       intervention) or completely destroyed structures (beyond repair), transition zone  
50       buildings face uncertain futures where preservation decisions depend on accurate  
51       damage assessment and targeted retrofitting strategies.

52       To address this knowledge gap, the study implements an exploratory machine  
53       learning analysis of post-tornado damage data. The primary objective is not  
54       to develop a predictive black-box model for automated assessment, but rather  
55       to use interpretable machine learning techniques to generate testable hypotheses  
56       regarding historic building vulnerability. The key question being which observable  
57       features differentiate structures that survive from those that experience significant  
58       damage in historic-style construction. Clarifying these differences supports a  
59       more targeted and impactful use of limited preservation resources for engineering  
60       assessments and retrofit planning.

61       The authors also recognize the limitations of the dataset, particularly the small  
62       sample size of "low damage" cases ( $n=20$ ) and the inherent circularity of damage-

63 based EF ratings. To tackle this, the research utilizes statistical equivalence testing  
64 and a random noise guardrail to filter out spurious correlations. Permutation  
65 Importance was implemented for global feature ranking [6] and integrated with  
66 SHAP (SHapley Additive exPlanations) analysis [7], computed on held-out vali-  
67 dation data to explore potential interaction effects. This approach moves the study  
68 beyond simple correlation to identify mechanistic candidates for future investiga-  
69 tion, such as the compounding risk of specific wall-roof combinations. In doing so,  
70 the study established a data-driven foundation for more nuanced discussions about  
71 risk, resilience, and the limits of intervention in the historic built environment. The  
72 analytical framework prioritizes methodological safeguards that prevent spurious  
73 findings, recognizing that preservation decisions based on unreliable correlations  
74 could lead to ineffective or counterproductive interventions.

75 The proposed approach incorporates several methodological innovations that  
76 distinguish it from conventional disaster assessment studies. Instead of reporting  
77 results from a single purportedly optimal model, a practice that risks overfitting to  
78 dataset idiosyncrasies, the analysis benchmarks six model families using statistical  
79 equivalence testing to identify all models whose performance is indistinguishable  
80 from the best. This multi-model validation ensures that identified vulnerabilities  
81 replicate across different analytical approaches, substantially increasing confidence  
82 in the findings. Additionally, a synthetic random feature is introduced as a negative  
83 control. If this noise variable appears among the important features, the validity  
84 of the analysis is called into question [8]. This provides an objective quality check  
85 that is largely absent from existing disaster assessment studies.

86 The combination of permutation importance analysis, which provides global  
87 feature rankings, with SHAP analysis, which reveals instance-level mechanisms  
88 and feature interactions, proves particularly valuable for historic preservation ap-  
89 plications. While permutation importance identifies which features matter across  
90 the entire building stock, SHAP analysis reveals how these features combine in  
91 specific buildings, enabling preservation professionals to identify structures fac-  
92 ing compounded risk from multiple vulnerabilities. For instance, SHAP analysis  
93 reveals that wall substrates and building height both push predictions toward sig-  
94 nificant damage, though this cannot quantify whether their combined effect is  
95 additive or multiplicative without formal interaction testing. Such interaction  
96 effects are important for retrofit prioritization, as addressing either vulnerability  
97 factor in isolation provides limited benefit compared to holistic interventions.

98 The results from this study demonstrate that feature importance analysis can  
99 yield valid scientific insights even when predictive model performance appears  
100 modest by conventional machine learning standards. While the macro F1 score

101 (0.48) reflects genuine difficulty in predicting the rare transitional “Low” dam-  
102 age class, the models successfully identify buildings at the highest collapse risk  
103 (F1=0.72 for Significant Damage), the outcome most relevant for preservation pri-  
104 oritization. This distinction between predictive accuracy and feature importance  
105 reliability carries implications for disaster assessment research, where perfect pre-  
106 diction often proves impossible, yet actionable insights remain achievable.

## 107 2. Data

### 108 2.1. Tornado Events and Data Collection

109 The analysis draws upon building-level damage data from two major tornado  
110 events. The first event occurred on March 3, 2020, when an EF3-EF4 tornado  
111 carved a 25-mile track through Nashville, Tennessee [9], while the second event,  
112 the December 10-11, 2021 Quad State tornado, produced a long-track EF4 tornado  
113 affecting Kentucky, Tennessee, Arkansas, and Missouri [10]. Following the torna-  
114 does, the Structural Extreme Events Reconnaissance Network (StEER) deployed  
115 the Virtual Field Assessment Team (VAST) and the Field Assessment Structural  
116 Team (FAST) for Nashville [9] and Quad State [10] to document the extent of  
117 damages. The authors participated in both the field deployments.

118 Damage was evaluated for 382 buildings across both tornado events, of which  
119 230 were listed in the *National Register of Historic Places*. Data collection em-  
120 ployed two complementary approaches: on-site field reconnaissance for structures  
121 with safe access, and virtual assessment using pre- and post-event remote sensing  
122 for buildings where physical access was restricted or pre-tornado documentation  
123 was available.

124 Field reconnaissance focused on Mayfield’s downtown historic district [11],  
125 which sustained catastrophic damage during the Quad State tornado outbreak. The  
126 district’s concentration of buildings constructed between 1850 and 1930 provided  
127 a unique opportunity to study tornado vulnerability in historic masonry structures  
128 predating wind engineering codes. On-site data collection utilized the Fulcrum  
129 application for standardized damage surveys, DJI Matrice drones for aerial docu-  
130 mentation, and Street View cameras for façade capture [3, 12].

131 Virtual reconnaissance extended the dataset beyond field-accessible buildings,  
132 evaluating all historic structures within a 2-mile radius of tornado paths for both  
133 the Quad State [13] and Nashville [12] events. High-resolution aerial imagery  
134 (5–7.5 cm ground sample distance) from Nearmap [14] and Google Earth [15]  
135 provided roof-level details including geometry, slope, and covering materials.

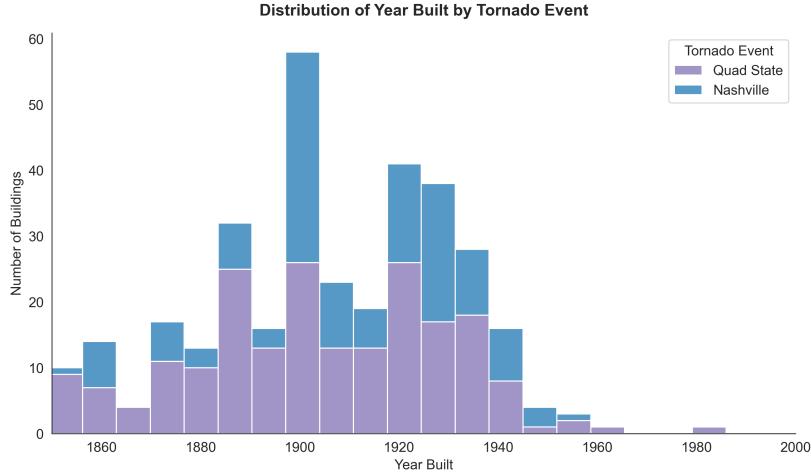
136 Street-level panoramas from Google Street View captured façade conditions, fen-  
137 estration patterns, and cladding systems. Pre-event imagery proved particularly  
138 valuable for documenting original construction details—roof substrates, wall ma-  
139 terials, parapet heights, connection types—subsequently destroyed or obscured  
140 by damage, enabling retrospective assessment of as-built conditions influencing  
141 tornado performance.

142 Data collection followed standardized StEER protocols [16], systematically  
143 documenting structural attributes, geometric properties, and damage indicators for  
144 each building. Structural features captured included construction type (masonry  
145 unreinforced, wood frame, hybrid systems), wall substrate and thickness, founda-  
146 tion type, and Main Wind Force-Resisting System (MWFRS) configuration. Roof  
147 characteristics received particular emphasis given their documented influence on  
148 tornado performance [17], with assessors recording roof shape (gable, hip, flat,  
149 mansard), slope, substrate material, covering type, and overhang dimensions. Wall  
150 and fenestration details documented cladding materials, opening percentages on  
151 all elevations, and parapet heights where present, as these features influence both  
152 structural capacity and internal pressurization following envelope breach [18].

153 Field and remote observations were supplemented with archival data from  
154 the National Register of Historic Places (NRHP) database, providing construction  
155 dates, architectural styles, and documentation of previous alterations or retrofits.  
156 The combined dataset comprised 382 buildings constructed between 1850 and  
157 1950 across both tornado events. Temporal distribution of the building stock  
158 (Figure 1) confirms the historic character, with peak construction years between  
159 1890 and 1930. The Quad State sample exhibits a longer tail of pre-1880 struc-  
160 tures compared to Nashville, reflecting Mayfield’s older urban core. Of the 382  
161 buildings, 230 held formal historic designation through the National Register of  
162 Historic Places or local historic districts (10 Nashville, 220 Quad State), while the  
163 remaining 150 buildings exhibited similar construction characteristics (masonry  
164 bearing walls, timber roof framing, shallow foundations), but lacked formal des-  
165 ignation, representing the vernacular historic building stock vulnerable to tornado  
166 damage.

## 167 2.2. Dataset Characteristics

168 The combined dataset comprises 382 historic masonry buildings spanning  
169 construction years 1850–1950, exposed to EF-scale tornado intensities ranging  
170 from EF0 to EF4. While all buildings share fundamental characteristic of masonry  
171 construction, substantial variation exists across structural, geometric, material, and  
172 hazard dimensions that govern tornado vulnerability.



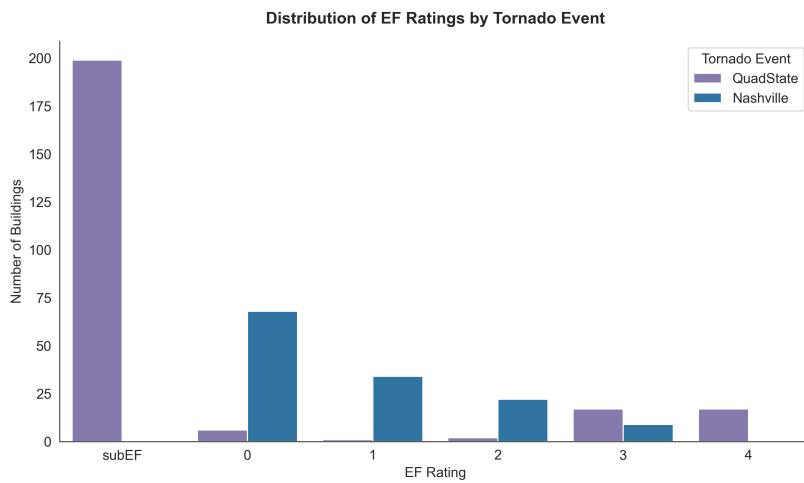
**Fig. 1.** Distribution of building construction years by tornado event. The dataset is concentrated between 1890 and 1930, with Quad State including a tail of older (pre-1880) structures.

173 Structural attributes include number of stories (1–4), roof shape (gable, hip, flat,  
 174 mansard), foundation type (rubble stone, brick pier, continuous wall), and MWFRS  
 175 configuration distinguishing frame-based versus masonry diaphragm lateral sys-  
 176 tems. Geometric properties vary across building footprint area (40–5,630 m<sup>2</sup>),  
 177 height (3.5–23.3 m), wall dimensions (side length 4.9–92 m, front length 5.5–  
 178 211 m), and roof slope (0–60°). Material characteristics capture heterogeneity  
 179 through wall thickness (0.2–0.6 m), wall substrate type (masonry unreinforced, re-  
 180inforced, wood frame), roof substrate (dimensional lumber, trusses, wood sheath-  
 181 ing grade), roof covering (asphalt, slate, clay tile, metal), and cladding systems  
 182 (brick veneer, wood siding, stucco). Envelope features document fenestration per-  
 183 centage on each elevation (0–90%), parapet height (0–1.5 m), and overhang length  
 184 (0–5 m), all influencing wind pressure distribution and internal pressurization risk.

185 Hazard variables quantify tornado exposure through EF rating assigned to each  
 186 building location and distance from tornado path centerline (0–2 km). The distribu-  
 187 tion of EF ratings (Figure 2) reflects differing intensities between events: the Quad  
 188 State tornado contributed higher proportions of EF3 and EF4 exposures, while  
 189 Nashville exhibited more EF0–EF2 exposures, providing variance necessary to  
 190 study building performance across the full intensity spectrum. Contextual factors  
 191 include urban setting (isolated, row-middle, row-end), building position relative  
 192 to street, occupancy type (residential, commercial, religious, institutional), and  
 193 historic designation status (NRHP-listed versus visual assessment). This multidimensional

194 dimensional feature space enables statistical models to identify vulnerability patterns  
195 specific to historic masonry construction while accounting for confounding factors  
196 such as building size, occupancy-driven design differences, and spatial clustering  
197 within historic districts.

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201  
**Fig. 2.** Distribution of EF ratings in the dataset. The Quad State tornado outbreak event contributes  
the majority of high-intensity (EF3-EF4) exposures.

202

### 203    2.3. Target Variable and Class Distribution

204    Damage observed during reconnaissance was classified using the five-category  
205    StEER definitions (undamaged, minor, moderate, major, destroyed). However,  
206    intermediate damage states contained fewer than 10 observations each, making  
207    statistical analysis with this fine-grained classification impractical. The original  
208    categories were therefore collapsed into three classes defined by preservation  
209    outcomes: Class 0 (Undamaged) requires no intervention, Class 1 (Low Damage)  
210    requires repair but preserves historic fabric, and Class 2 (Significant Damage)  
211    necessitates major reconstruction or represents total loss. This aggregation strategy  
212    follows established practices for handling limited sample sizes and reducing class  
sparsity [19, 20].

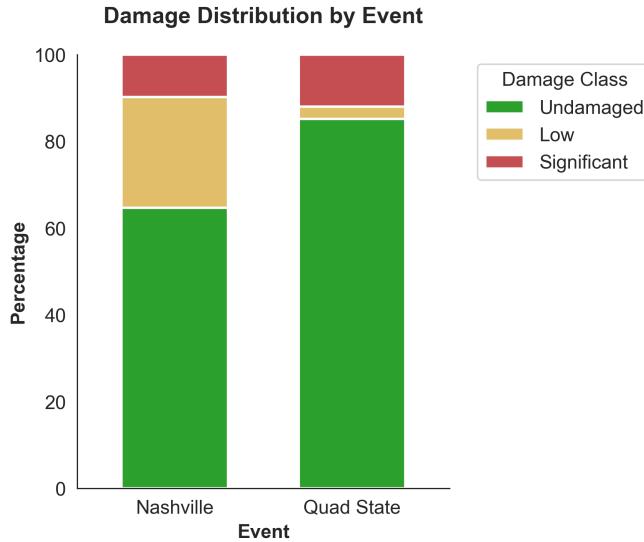
213 The resulting dataset exhibits severe class imbalance characteristic of post-  
214 disaster surveys (Figure 3): 294 buildings (77%) sustained no damage requiring  
215 intervention, 41 buildings (11%) experienced repairable damage preserving the  
216 majority of historic material, and 47 buildings (12%) suffered severe damage or  
217 total loss. This three-class structure prioritizes the critical preservation decision  
218 boundary distinguishing structures that can be saved from those facing demolition,  
219 rather than imposing artificial distinctions among undamaged or catastrophically  
220 damaged buildings where preservation interventions offer no value.

221 The class imbalance creates methodological challenges for performance eval-  
222 uation. Simple accuracy would be misleading, as a naïve classifier predicting  
223 "undamaged" for all buildings achieves 77% accuracy while providing no use-  
224 ful information. Macro-averaged F1 score is therefore employed as the pri-  
225 mary performance metric, as it equally weights performance across all damage  
226 classes regardless of sample size, penalizing models that ignore minority classes  
227 [21, 22]. This ensures models demonstrate meaningful discriminative capacity for  
228 the preservation-critical Low and Significant Damage categories, not merely high  
229 accuracy from correctly classifying the dominant Undamaged class.

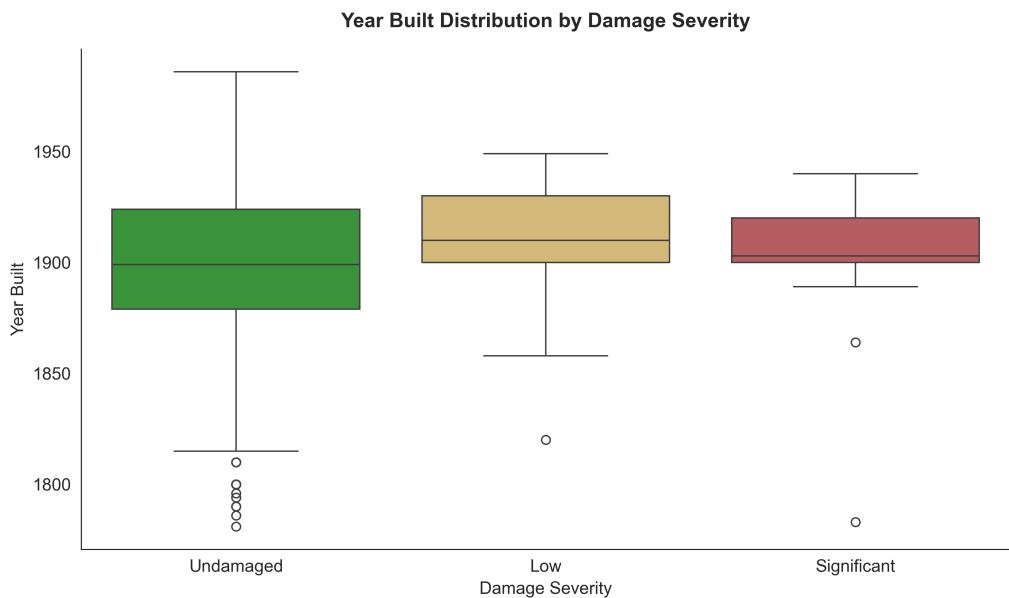
230 Analysis of building age versus damage severity (Figure 4) reveals no strong  
231 correlation, indicating that construction era alone does not predict vulnerability.  
232 Rather, specific structural details (wall thickness, roof-wall connections, retrofit  
233 presence) and maintenance conditions likely govern tornado performance indepen-  
234 dent of building age, motivating the feature-based vulnerability analysis presented  
235 in subsequent sections.

236 Figure 4 shows the distribution of construction years across damage classes.  
237 The three boxplots exhibit substantial overlap, with all classes centered around  
238 1900 (median years: Undamaged 1900, Low Damage 1905, Significant Damage  
239 1910). This overlap indicates that building age alone does not reliably predict  
240 tornado vulnerability. Older buildings are not systematically more vulnerable than  
241 newer ones in this dataset, nor are the newest buildings systematically safer.

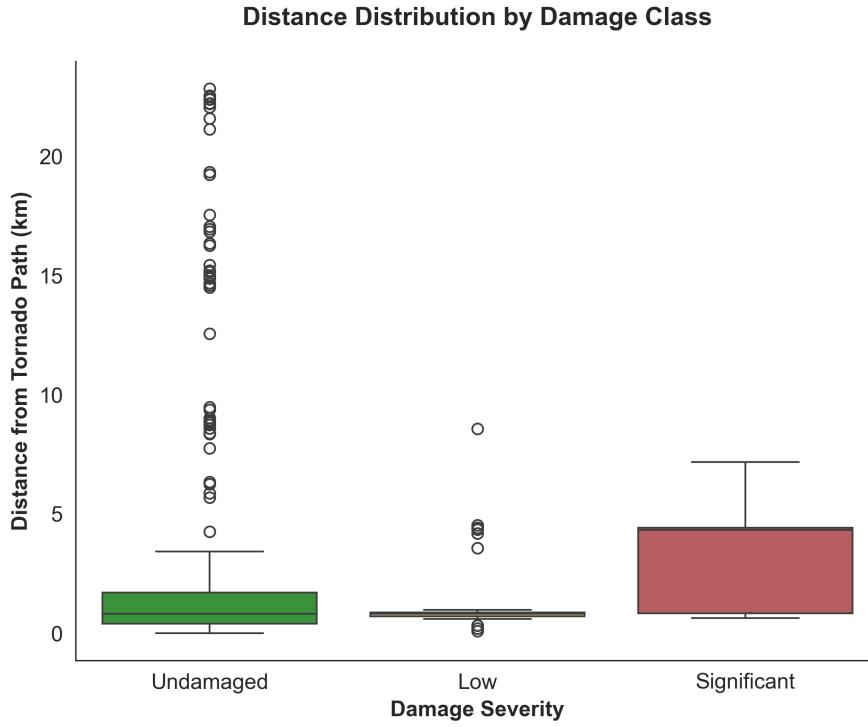
242 This absence of a clear age-damage relationship contradicts the intuitive expec-  
243 tation that older buildings, having experienced more degradation, should perform  
244 worse. However, the pattern reflects survivorship bias rather than age-irrelevance.  
245 Pre-1880 structures present in the dataset represent a pre-selected cohort of high-  
246 quality survivors: poorly constructed contemporaries were demolished decades  
247 ago, leaving only robust examples. In contrast, buildings from the 1900–1920  
248 construction age includes development that has not yet been culled by time or  
249 economic obsolescence. Additionally, pre-1880 buildings are more likely to hold  
250 formal historic designation, receiving preservation-quality maintenance that off-



**Fig. 3.** Proportion of damage classes by tornado event. Quad State shows a higher rate of significant damage due to the direct impact on the historic district.



**Fig. 4.** Boxplot of Year Built by Damage Class. The lack of a clear linear trend suggests age alone is not a strong predictor of vulnerability.



**Fig. 5.** Boxplot of Tornado Distance by Damage Class. The overlapping distributions suggest distance alone is not a strong predictor of vulnerability.

sets age-related degradation, while early 20th-century buildings may lack both designation protections and consistent owner investment.

The overlapping distributions demonstrate that chronological age functions as a poor vulnerability proxy without accounting for maintenance history, original construction quality, and most critically, tornado exposure variables that govern actual wind loading. Two buildings constructed in 1900 may exhibit drastically different performance if one experienced EF0 winds at 1 km from the tornado path while the other faced EF3 winds at 100 m distance. Age-based screening would misclassify both. This finding motivates the feature-based vulnerability analysis in subsequent sections, which identifies specific structural characteristics (wall thickness, roof geometry, retrofit presence) governing performance independent of construction era.

Additionally, figure 5 reveals the relationship between distance from tornado centerline and damage outcomes. Low damage buildings exhibit the tightest dis-

tribution, defining a narrow "transition zone" where wind speeds cause repairable structural damage without progression to total loss. Significantly damaged buildings show greater median distance and wider dispersion than low damage cases, reflecting the confounding influence of local EF rating variation along the tornado path—buildings 2–3 km from an EF4 centerline can experience EF2–EF3 winds exceeding design thresholds, while buildings 1 km from an EF1 path may sustain only minor damage. Undamaged buildings span the full distance range, with substantial overlap across all damage classes. This overlap demonstrates that distance alone, without accounting for local wind speed and building-specific structural characteristics, provides insufficient damage prediction. The finding motivates hazard-neutral modeling approaches isolating intrinsic building vulnerabilities independent of tornado exposure intensity.

#### 2.4. Wind Vulnerability in Masonry Structures

Unreinforced masonry buildings exhibit tornado vulnerability mechanisms stemming from their fundamental structural characteristics. Historic masonry construction employed load-bearing walls as the primary structural system [4], with two-leaf or three-leaf configurations providing compressive capacity but negligible tensile strength due to masonry's anisotropic material properties [23]. Traditional gravity-based connections between roof and wall elements, while adequate for transferring shear forces under normal loading, prove inadequate for resisting tensile uplift forces generated by tornado wind fields [24].

Failure initiates through several mechanisms that frequently interact to produce progressive collapse. Out-of-plane wall failure occurs when wind pressure causes masonry walls to act as one-way slabs spanning between floor and roof diaphragms; insufficient anchorage to these horizontal elements leads to mid-height cracking and potential wall collapse [25]. Roof uplift results from negative pressure coefficients on leeward and roof surfaces [26], with failure of roof-to-wall connections allowing entire roof system separation. Parapet overturning generates large moments at the base of unbraced masonry elements projecting above the roofline, particularly vulnerable given their exposure to peak wind velocities and lack of lateral restraint. Internal pressurization from breached envelope openings (windows, doors, cladding) can amplify net roof uplift forces [27, 28], transforming moderate external suction into failure-inducing combined loading.

These mechanisms exhibit cascading interdependence. Cladding loss or fenestration breach precedes structural damage through internal pressurization initiation [28]. Roof loss removes lateral bracing for tall masonry walls, triggering secondary out-of-plane collapse even after wind speeds decrease. Parapet failure generates

302 debris impact loading on adjacent roof and wall surfaces. The spatial heterogeneity  
303 of wind pressures across building surfaces, varying with geometry and orientation  
304 [29], means simultaneous loading combinations differ substantially from design  
305 assumptions based on uniform pressure distributions, creating unexpected stress  
306 states that exploit connection and material vulnerabilities inherent to unreinforced  
307 masonry construction.

308 **3. Methodology**

309 *3.1. Overview of Multi-Model Framework*

310 Following the theoretical framework established by [30], this study prioritizes  
311 explanatory modeling over pure prediction, to distinguish between models  
312 designed to identify causal mechanisms versus those optimized solely for forecasting.  
313 Understanding which building features drive vulnerability is more valuable  
314 than achieving marginal gains in aggregate classification accuracy for preservation  
315 engineering. Recent empirical work by [31] demonstrates that feature importance  
316 rankings remain stable and valid even when model performance degrades,  
317 given that the degradation stems from irreducible noise rather than sample size  
318 limitations.

319 For this study, six model families were benchmarked and statistical equivalence  
320 testing was implemented to identify all models that perform indistinguishably from  
321 the top performer as compared to reporting results from a single optimal model.  
322 To ensure that the vulnerabilities identified reflect genuine building characteristics  
323 rather than algorithmic artifacts, the findings were replicated across multiple  
324 equivalent models. This cross-model validation helped filter out model-specific  
325 noise and reduced the risk that preservation recommendations rest on idiosyncratic  
326 behavior. As a result, features ranking highly across different algorithmic  
327 approaches represent a genuine signal, providing preservation professionals with  
328 confidence that retrofit priorities address real vulnerabilities.

329 A synthetic negative control feature was generated from a standard normal  
330 distribution with a fixed random seed and added to the dataset, as established [32]  
331 and operationalized in the Boruta algorithm. This random probe contained zero  
332 information and provided an objective baseline for statistical significance. Any  
333 physical feature (such as parapet height or MWFRS configuration) that consistently  
334 outperforms this random baseline across multiple model families is statistically  
335 distinguishable from noise, regardless of the model's overall accuracy [32]. This  
336 guardrail served two purposes for preservation applications; This random noise  
337 feature serves as a quality control mechanism with two functions. First, if random

338 noise ranks among important predictors, the analysis is flawed—indicating data  
339 leakage or spurious correlations that would render any preservation recommendations  
340 unreliable. Second, the noise feature provides an objective threshold that any  
341 building feature ranking below random noise lacks genuine predictive signal and  
342 should be excluded from interpretation, preventing preservation resources from  
343 being misdirected toward irrelevant characteristics.

344 *3.2. Data Preparation*

345 Two hazard variables helped quantify tornado exposure: EF rating and distance  
346 from tornado path. EF rating represents tornado intensity at each building location  
347 that was assigned during damage surveys. These ratings (EF0 through EF5) were  
348 encoded as integers 0 through 5, while subEF events (wind speeds below EF0  
349 threshold) were coded as -1. Distance from tornado path represents the shortest  
350 distance from each building centroid to the tornado track centerline, calculated  
351 as point-to-segment distance using planar approximation with latitude-dependent  
352 coordinate scaling [33].

353 One of the challenges with tornado damage modeling is the potential for circular  
354 reasoning when using EF ratings as predictors. Since EF ratings are post-hoc  
355 intensity estimates often derived from the building damage [34], including them  
356 creates a tautological loop where the outcome (damage) implicitly informs the  
357 predictor (EF). To address this, the analysis was conducted under two distinct con-  
358 ditions: First, the Hazard-Neutral approach was considered to represent structural  
359 truth. This condition excluded EF rating and distance-to-track, forcing the model  
360 to predict damage solely based on intrinsic building characteristics (e.g., geometry,  
361 materials, age). This was the primary lens for identifying structural vulnerabili-  
362 ties, as it eliminates the circularity of the EF scale. Second, the Hazard-Inclusive  
363 approach was implemented to provide contextual control. This condition includes  
364 hazard features to quantify how much predictive power is gained by knowing the  
365 wind intensity and distance to the building. While this introduces circularity, it  
366 serves as a necessary control to benchmark the relative importance of structural  
367 features against the overwhelming force of the winds. The findings are explicitly  
368 prioritized from the Hazard-Neutral condition for structural recommendations,  
369 treating Hazard-Inclusive results primarily as a validation of the model’s ability to  
370 capture basic physical reality (i.e., stronger winds cause more damage).

371 High missingness (>10%) is observed for several features, particularly those  
372 requiring interior access or detailed inspection. This missingness is likely in-  
373 formative rather than random: buildings with extensive damage may have had

**Table 1.** Dataset Composition by Key Features (with Missingness)

Feature	Category	Count (%)	Missing (%)
Number of Stories	1 Story	250 (65%)	0%
	2 Stories	110 (28%)	
	3+ Stories	26 (7%)	
Roof Shape	Gable	280 (73%)	0%
	Hip	60 (16%)	
	Flat	46 (12%)	
Foundation Type	Continuous	300 (78%)	36%
	Pier	50 (13%)	
	Slab	36 (9%)	
Roof Substrate	Board/Plank	304 (80%)	19.6%
Wall Substrate	URM/Brick	182 (48%)	13.1%
Retrofit Type	Present	107 (28%)	8%
Wall Thickness	Mean: 380mm	–	36%
Fenestration (%)	Mean: 15-20%	–	15-20%

374 inaccessible interiors, while remote-sensing-only assessments could not docu-  
375 ment hidden details. For retrofit-related features, missing data often indicates  
376 "no retrofit was documented during reconnaissance," which can mean there was  
377 no retrofit or inaccessible due to building collapse. Preprocessing preserved this  
378 information through two strategies, median imputation for numeric categories and  
379 ordinal encoding for categorical features. This allows models to learn whether  
380 "unknown" status correlates with damage. For instance, if buildings with "un-  
381 known" wall substrates exhibit systematically higher vulnerability than those with  
382 confirmed construction details.

383 However, the authors acknowledge that this high rate of missingness limits the  
384 certainty of the conclusions regarding these specific variables. Findings related to  
385 wall substrate, roof substrate, and retrofit status should be considered tentative and  
386 interpreted with caution, as they may be influenced by the imputation strategy or the  
387 informative nature of the missing data itself. Future work should explore multiple  
388 imputation or missingness indicators to better quantify uncertainty introduced by  
389 incomplete data.

390    *3.2.1. Preprocessing*

391    Standard preprocessing procedures were applied to prepare data for modeling  
392    while preserving information content. Numeric features shown in 2 employed  
393    median imputation for missing values, replacing absent data with the dataset me-  
394    dian to maintain distributional properties. Categorical features required different  
395    encoding strategies depending on the analytical method: ordinal encoding for the  
396    Synthetic Minority Over-sampling Technique for Nominal and Continuous data  
397    (SMOTENC) pipeline, and one-hot encoding for permutation importance analy-  
398    sis [35]. This dual encoding approach ensures compatibility with each method's  
399    algorithmic requirements.

400    Class imbalance was addressed using SMOTENC, which generates synthetic  
401    minority class examples by interpolating between existing cases in feature space  
402    while respecting categorical feature integrity. SMOTENC was applied strictly  
403    within cross-validation folds ( $k=5$  neighbors), meaning synthetic examples were  
404    generated only from training data after each train-test split. This within-fold ap-  
405    plication prevents data leakage that would artificially inflate model performance if  
406    oversampling preceded cross-validation [36]. The oversampling strategy balanced  
407    all three damage classes to equal representation within each training fold, while  
408    validation sets remained entirely real data.

409    However, SMOTENC introduces concerns when minority classes are small.  
410    The low-damage class contains only 41 examples, meaning each synthetic case  
411    represents an interpolation among 25% of available real examples ( $k=5$  neigh-  
412    bors). This raises questions about whether synthetic examples reflect physically  
413    plausible building configurations or introduce artifacts. To validate SMOTENC's  
414    impact, an ablation study compared Random Forest performance with and without  
415    oversampling on the real dataset. Results showed minimal difference: Macro F1  
416    improved marginally from 0.626 (with SMOTENC) to 0.642 (without). These  
417    negligible differences suggest SMOTENC provides modest training stabilization  
418    without fundamentally altering predictive capacity or feature importance rankings.  
419    Consequently, SMOTENC is retained for minority class recall improvement and  
420    synthetic examples are used strictly for model training, not for generating physical  
421    insights about building vulnerability.

422    *3.3. Model Selection and Validation Strategy*

423    Six model families were benchmarked to ensure that the findings generalize  
424    across algorithmic approaches, a consideration for preservation applications. The  
425    model suite includes Decision Tree as a baseline non-linear model, Random Forest  
426    as an ensemble method, Logistic Regression as a multinomial linear baseline,

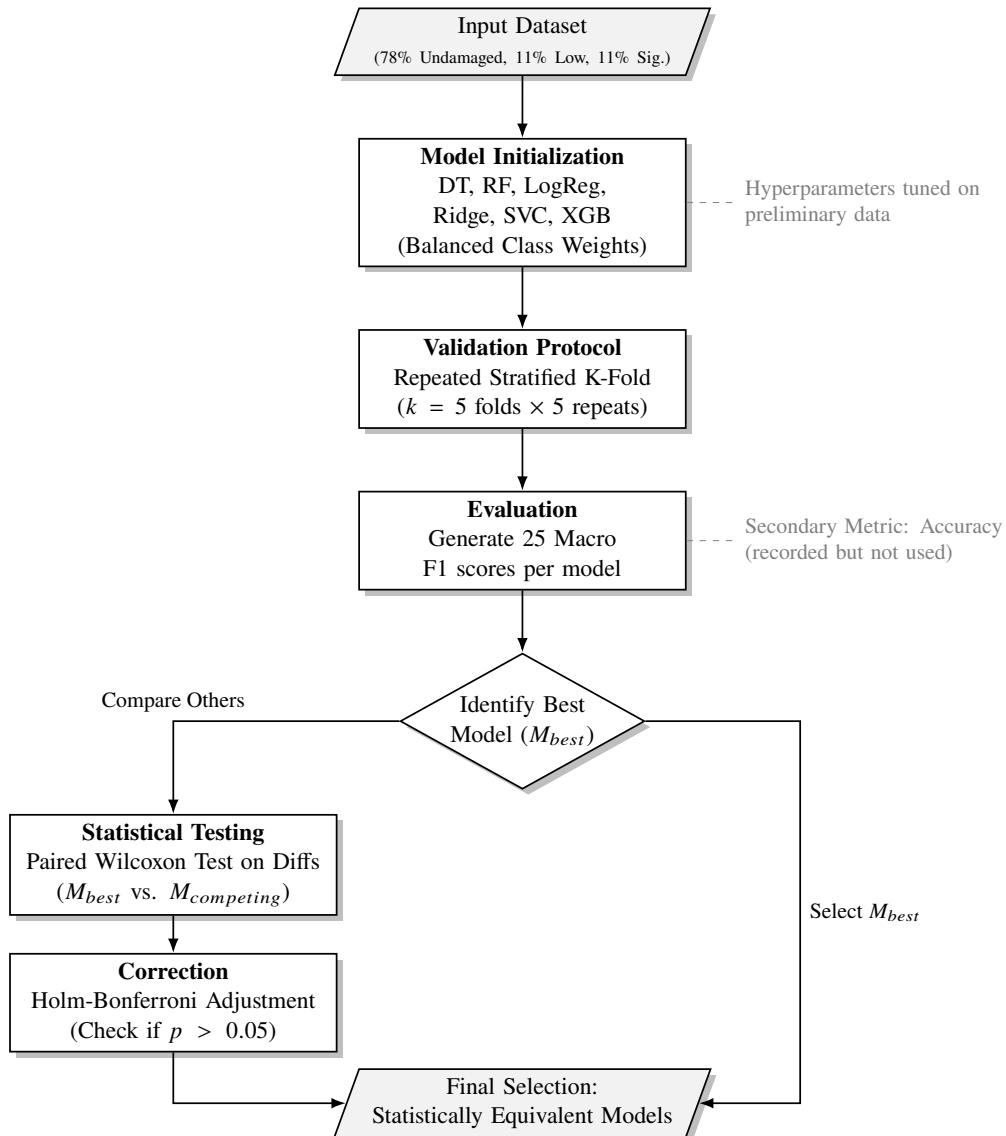
**Table 2.** Feature Classification for Preprocessing

Numeric Features	Categorical Features
Number of Stories	Archetype & Occupancy
Year of Construction	Retrofit Presence
Building Geometry (height, length)	Building Setting Urban Context
Parapet Height	Roof Details (shape, substrate, cover)
Overhang Height	MWFRS (roof, wall)
Fenestration Percentage	Wall Details (system, substrate, cladding)

427 Ridge Classifier as an L2-regularized linear model, Linear SVC as a support  
428 vector classifier, and XGBoost as a gradient boosting implementation. All the  
429 models employed balanced class weighting to address class imbalance, while  
430 hyperparameters were selected based on preliminary tuning with details available  
431 in supplementary materials.

432 Repeated stratified K-fold cross-validation was implemented with five folds and  
433 five repeats, yielding 25 evaluation rounds per model. This design preserves class  
434 proportions in each fold, which proves essential given the 78% undamaged, 11%  
435 low damage, and 11% significant damage distribution. This repetition reduces  
436 variance in performance estimates while enabling paired statistical testing across  
437 models, as all models are evaluated on identical data splits. The primary metric,  
438 macro F1, represents the arithmetic mean of per-class F1 scores and treats all  
439 damage classes equally, preventing exploitation of the 78% undamaged majority  
440 that would occur with accuracy-based or weighted metrics. The overall accuracy is  
441 also reported as a secondary metric for context, but is not used for model selection,  
442 since it can be misleadingly high due to class imbalance.

443 To identify statistically equivalent models without individually deciding, for  
444 each setting, whether hazard-neutral or hazard-inclusive, the model achieving the  
445 best mean macro F1 score was identified first. For each competing model, a paired  
446 Wilcoxon signed-rank test on the 25 fold-wise macro F1 differences was performed.  
447 The Wilcoxon test is appropriate for paired, non-normal data and is recommended  
448 for classifier comparisons [37]. Holm-Bonferroni correction for multiple compar-  
449 isons was applied, accounting for five tests per setting, and deemed models with  
450 p-values exceeding 0.05 after correction as statistically indistinguishable from the  
451 best model. This conservative approach establishes a high evidentiary standard  
452 for claiming equivalence and is shown in Fig 6.



**Fig. 6.** Model selection and validation methodology.

453 *3.4. Feature Importance Methods*

454 *3.4.1. Permutation Importance*

455 For each model and feature set, permutation importance was calculated using  
456 a four-step procedure applied within each cross-validation fold. First, the model  
457 was trained on the training fold with SMOTENC oversampling. Second, baseline  
458 performance was evaluated on the held-out validation fold using macro F1 score.  
459 Third, for each feature individually, its values in the validation set were randomly  
460 shuffled (breaking the relationship between that feature and the outcome), and  
461 model performance was re-evaluated. The importance score was calculated as  
462 the difference between baseline and permuted performance. Fourth, importance  
463 scores were aggregated across all cross-validation folds by computing the mean  
464 importance and standard deviation for each feature.

465 Greater performance degradation after permutation indicated higher feature  
466 importance, as it reflected the model’s dependency on that feature’s information.  
467 This approach is model-agnostic and robust for correlated features, particularly  
468 historic building assessment where construction characteristics often correlate  
469 (e.g., wall structural system and construction era).

470 *3.4.2. SHAP Analysis*

471 While permutation importance provides global feature rankings across model  
472 families, it cannot explain how features mechanistically influence damage or iden-  
473 tify building-specific vulnerabilities. To address this limitation, SHAP [38] was  
474 applied to the top performing models for instance-level analysis.

475 SHAP analysis was computed on real (non-augmented) validation fold data,  
476 to ensure interpretability reflects actual building behavior rather than synthetic  
477 interpolations. The TreeExplainer algorithm was employed for computational  
478 efficiency with tree-based models. However, the small low-damage class (approxi-  
479 mately 4 low-damage cases per validation fold) necessitates cautious interpretation  
480 since insights for this minority class are based on limited real examples and may  
481 not generalize broadly. Despite this constraint, SHAP values enable preserva-  
482 tion professionals to identify individual buildings exhibiting multiple concurrent  
483 vulnerability indicators.

484 *3.4.3. Why Both Methods?*

485 Permutation importance and SHAP analysis provide complementary insights  
486 essential for historic preservation applications. Permutation importance delivers  
487 global rankings validated across multiple equivalent models and proves less sensi-  
488 tive to feature correlations, ensuring that identified vulnerabilities reflect genuine

489 predictive power rather than multicollinearity artifacts. Conversely, SHAP analy-  
490 sis provides mechanistic understanding by revealing interaction effects, such as the  
491 compounding risk when unknown walls combine with absent retrofits, and provides  
492 instance-level explanations enabling retrofit prioritization for specific buildings.  
493 Features that rank highly in both methods represent the global importance across  
494 the building stock and mechanistic influence at the individual building level.

### 495 *3.5. Limitations and Ethical Considerations*

496 There are several limitations that frame the interpretation of the results. The  
497 dataset of 382 buildings, while sufficient for identifying main effects, limits the  
498 detection of subtle interactions, particularly for the minority "Low" damage class.  
499 This scarcity makes it difficult to isolate the specific transition features that differen-  
500 tiate minor repairable damage from total loss. Additionally, the results are specific  
501 to masonry construction in Southeastern U.S. tornado events and do not gener-  
502 alize to other construction typologies or hazard contexts. Finally, unmeasured  
503 confounders such as construction quality, maintenance history, and age-related  
504 deterioration are not explicitly modeled.

505 All data was fully anonymized prior to analysis to protect property owners,  
506 ensuring no personally identifiable information was included in the public dataset.  
507 Furthermore, on-site data collection was performed in public view with strict sen-  
508 sitivity to the traumatic nature of the event for residents, adhering to established  
509 reconnaissance protocols. The dataset and models are intended solely for research  
510 purposes aimed at improving public safety, informing building codes, and en-  
511 hancing community resilience, rather than for insurance adjustments or individual  
512 property valuations.

## 513 **4. Results**

### 514 *4.1. Model Performance*

515 Model performance was evaluated using Macro F1 scores across two settings:  
516 Hazard-Inclusive and Hazard-Neutral. Ensemble methods (Random Forest and  
517 XGBoost) consistently outperformed linear models. Random Forest achieved  
518 the highest Macro F1 scores of 0.726 (Hazard-Inclusive) and 0.657 (Hazard-  
519 Neutral). Wilcoxon signed-rank tests confirmed statistical equivalence among  
520 top-performing models. In the Hazard-Inclusive setting, XGBoost ( $p = .903$ )  
521 showed no significant difference from Random Forest. In the Hazard-Neutral  
522 setting, XGBoost ( $p = 1.00$ ) was statistically equivalent to Random Forest. These  
523 scores, while modest in absolute terms, reflect the inherent difficult nature of the

524 classification task rather than model inadequacy. The low-damage class ( $n=20$ )  
525 represents a genuine transition zone that is physically ambiguous, not a modeling  
526 failure.

527 The theoretical literature establishes that consistency of variable selection  
528 (identifying the true feature set) is mathematically distinct from predictive optimi-  
529 zation [39, 40]. A model may exhibit high explanatory power while having  
530 modest predictive power due to high irreducible noise. Recent empirical valida-  
531 tion by [31] demonstrates that feature importance rankings remain stable across  
532 degraded model performance in low-signal domains.

533 Given this, the high F1 scores for the critical binary classification (Undam-  
534 aged vs. Significant Damage:  $F1=0.93$  and  $F1=0.72$  respectively, (see Appendix  
535 A) indicated that the model has learned the physics of structural failure. The  
536 features that outperform the random noise baseline therefore represent genuine  
537 structural vulnerabilities, not statistical artifacts. Ensemble methods (Random  
538 Forest and XGBoost) consistently outperformed linear models and single decision  
539 trees, demonstrating the necessity of capturing non-linear relationships in damage  
540 prediction.

541 The observed damage distribution itself provides important insight for his-  
542 toric preservation: 77% of historic masonry buildings survived tornado exposure  
543 with no structural damage, challenging assumptions that pre-code construction  
544 inevitably fails under wind loading. This finding suggests that vulnerability is not  
545 uniformly distributed across historic masonry, but rather concentrates in buildings  
546 with specific characteristic combinations. The challenge in predicting the transi-  
547 tional “Low” damage class reflects ambiguity in this boundary condition rather  
548 than model failure, while the strong performance on significant damage ( $F1=0.72$ ,  
549 see Appendix A) demonstrates that the models successfully distinguish buildings  
550 at highest risk.

551 *4.2. Statistical Equivalence Testing*

552 The non-parametric Wilcoxon signed-rank test was used with Holm-Bonferroni  
553 correction to identify models statistically indistinguishable from the top performer.  
554 Wilcoxon tests were chosen over all-pairwise comparisons because the objective  
555 is to identify models equivalent to the specific best performer rather than testing  
556 for differences across all models simultaneously

557 As shown in Table 3, only XGBoost achieved p-values greater than 0.05 for  
558 both hazard-neutral ( $p=1.00$ ) and hazard-inclusive ( $p=0.903$ ) settings, indicating  
559 statistical equivalence with Random Forest. This identifies Random Forest and

**Table 3.** Model Performance (Mean  $\pm$  Std over 25 CV folds)

Setting	Model	Macro F1	Accuracy
Hazard-Neutral	Random Forest	<b>0.65 ± 0.07</b>	0.81 ± 0.03
	XGBoost	<b>0.65 ± 0.07</b>	<b>0.83 ± 0.03</b>
	Decision Tree	0.53 ± 0.07	0.73 ± 0.05
	Linear SVC	0.56 ± 0.06	0.71 ± 0.05
	Logistic Regression	0.55 ± 0.05	0.71 ± 0.05
Hazard-Inclusive	Ridge Classifier	0.55 ± 0.05	0.69 ± 0.04
	Random Forest	<b>0.72 ± 0.06</b>	0.87 ± 0.03
	XGBoost	0.72 ± 0.07	<b>0.88 ± 0.03</b>
	Decision Tree	0.65 ± 0.06	0.81 ± 0.04
	Linear SVC	0.65 ± 0.07	0.81 ± 0.04
	Logistic Regression	0.65 ± 0.07	0.81 ± 0.05
	Ridge Classifier	0.64 ± 0.07	0.78 ± 0.05

560 XGBoost as a statistically equivalent pair, validating the robust performance of  
561 tree-based ensemble methods regardless of hazard context inclusion.

562 Conversely, linear models and decision trees showed statistically significant  
563 performance deficits ( $p < 0.001$ ) in both settings. The equivalence between Random  
564 Forest and XGBoost establishes a pair of tree-based ensemble methods that perform  
565 equally well regardless of hazard context. This finding reduces emphasis on any  
566 single algorithm while demonstrating that simpler linear and tree-based approaches  
567 cannot achieve top-tier performance in this damage prediction task.

568 *4.3. Permutation Importance*

569 The permutation importance analysis (Figures 7 and 8) reveals a distinct hierar-  
570 chy of candidate predictors for building damage assessment. Both figures display  
571 only features that outperformed the random noise baseline in at least one of the  
572 top-performing models (RandomForest and XGBoost), ensuring that visualized  
573 predictors represent genuine signal rather than noise. The analysis demonstrates  
574 strong agreement between both ensemble methods, with consistent feature rank-  
575 ings despite different algorithmic approaches to feature selection and importance  
576 calculation.

**Table 4.** Statistical Equivalence vs. Best Model (XGBoost for Neutral, Random Forest for Inclusive)

Setting	Comparison Model	p-value	$\Delta F1$	Equivalent?
Hazard-Neutral (vs. Random Forest)	XGBoost	1.000	0.010	<b>Yes</b>
	Ridge Classifier	<0.001	0.099	No
	Linear SVC	<0.001	0.93	No
	Logistic Regression	<0.001	0.099	No
	Decision Tree	<0.001	0.118	No
Hazard-Inclusive (vs. Random Forest)	XGBoost	0.903	0.003	<b>Yes</b>
	Linear SVC	<0.001	0.067	<b>No</b>
	Ridge Classifier	<0.001	0.081	<b>No</b>
	Logistic Regression	<0.001	0.061	No
	Decision Tree	<0.001	0.065	No

#### 577 4.3.1. Hazard-Neutral Setting

578 In the absence of hazard intensity information, wall thickness emerged as the  
 579 dominant predictor (permutation importance  $\approx 0.03$ ), aligning with engineering  
 580 principles where wall thickness can act as a proxy for structural capacity and  
 581 lateral load resistance. The second tier of predictors includes roof slope (import-  
 582 tance  $\approx 0.015$ ), year of construction (importance  $\approx 0.013$ ), and parapet height  
 583 (importance  $\approx 0.012$ ), each contributing comparable predictive power. Roof slope  
 584 influences both wind pressure distribution and aerodynamic uplift forces, while  
 585 construction year serves as a proxy for building code compliance and material  
 586 quality standards. Parapet height is particularly significant for edge protection and  
 587 roof-to-wall connection integrity.

588 Building typology descriptors (archetype, occupancy) ranked in the third tier  
 589 (importance  $\approx 0.010$ ), capturing implicit structural characteristics associated with  
 590 different building uses. Geometric features including wall fenestration percent-  
 591 ages, overhang length, and number of stories showed modest but consistent con-  
 592 tributions (importance 0.005–0.010), reflecting their roles in wind pressure coef-  
 593 ficient modification and load path complexity.

#### 594 4.3.2. Hazard-Inclusive Setting

595 When hazard intensity is available, EF rating naturally dominates the predictor  
 596 hierarchy with permutation importance values of 0.10 (Random Forest) and 0.18  
 597 (XGBoost). This dominance reflects the fundamental relationship between tornado

598 wind speed and structural damage, with the ef-rating identified based on the  
599 damages observed.

600 Among intrinsic building attributes, retrofit type emerged as the second-ranked  
601 predictor (importance  $\approx 0.015$ ), followed closely by archetype (importance  $\approx$   
602 0.012) and distance from tornado path (importance  $\approx 0.012$ ). Retrofit type cap-  
603 tures explicit structural interventions (reinforced masonry, steel bracing, structural  
604 restoration) that directly enhance wind resistance. Distance from the tornado  
605 path serves as a proxy for experienced wind speed variation within the same  
606 EF-rated event, capturing local intensity gradients not fully represented by the dis-  
607 crete EF scale. Structural features including wall dimensions (wall\_length\_side,  
608 wall\_thickness) and roof geometry (roof\_slope) are also identified as important).

609 *4.3.3. Global Feature Importance*

610 Features exhibiting high permutation importance across both hazard settings  
611 represent predictors whose influence persists regardless of model choice or hazard  
612 quantification method. Wall thickness and roof slope rank among the top five  
613 predictors in both Hazard-Neutral and Hazard-Inclusive configurations, demon-  
614 strating their role as fundamental damage drivers. When these features are ran-  
615 domly shuffled, model accuracy degrades substantially across all six model families  
616 tested, confirming that their predictive power reflects genuine structural mech-  
617 anisms rather than model-specific artifacts or overfitting to particular subsets of the  
618 data. The consistency of these rankings validates their utility for population-level  
619 preservation guidance. Features like wall thickness provide actionable rules of  
620 thumb applicable across the historic building stock: prioritizing structural assess-  
621 ment regardless of their specific archetype, occupancy, or location. This broad  
622 applicability enables efficient resource allocation when evaluating large building  
623 portfolios.

624 Categorical features like archetype exhibit elevated permutation importance  
625 (0.010 Hazard-Neutral, 0.012 Hazard-Inclusive) because they function as com-  
626 posite proxies bundling multiple correlated attributes. Archetype implicitly encodes  
627 construction era patterns, typical material choices, expected structural systems,  
628 and conventional roof configurations associated with specific building uses.

629 The rank stability of wall thickness, roof slope, retrofit type, and parapet height  
630 across both Hazard-Neutral and Hazard-Inclusive settings identifies these as robust  
631 targets for preservation interventions. Their consistent emergence as top predictors  
632 regardless of whether tornado intensity is known indicates that these features rep-  
633 resent intrinsic vulnerabilities that persist across hazard quantification approaches.  
634 This robustness is particularly valuable for historic preservation practice, where

635 hazard intensity documentation varies widely across tornado events.

636 *4.3.4. Data Limitations*

637 The permutation importance hierarchy reflects both genuine structural vul-  
638 nerabilities and inherent data collection constraints that limit interpretation and  
639 generalizability. Understanding these limitations is essential for appropriate ap-  
640 plication of findings to engineering practice and future research design.

- 641 • **Measurement Uncertainty in Categorical Features:** Categorical features  
642 include explicit uncertainty coding (1=certain, 2=moderate, 3=high uncer-  
643 tainty) documenting assessor confidence. Analysis reveals substantial un-  
644 certainty in critical attributes: construction type was classified with high  
645 uncertainty for 94.5% of buildings, wall cladding for 88.7%, roof cover for  
646 73.3%, and retrofit type for 98.4%. These high rates reflect the fundamental  
647 challenge of inferring internal structural details from external visual in-  
648 spection. Features with predominantly uncertain classifications may exhibit  
649 low permutation importance even when true physical relationships exist.  
650 Low permutation importance for high-uncertainty features should not be  
651 interpreted as physical irrelevance but rather as evidence that measurement  
652 precision is inadequate for relationship detection.
- 653 • **Proxy Variables and Confounding:** Several high-ranking features function  
654 as composite proxies rather than direct measurements. Construction year  
655 (ranked third in hazard-neutral setting) aggregates building code evolution,  
656 material quality improvements, connection practice changes, and cumulative  
657 degradation. The feature's importance indicates newer buildings perform  
658 better, but it is not yet possible to distinguish whether this stems from  
659 superior mortar, stronger connections, or reduced degradation. Building  
660 archetype similarly aggregates framing system, construction material, geo-  
661 metric proportions, and construction quality into categorical labels. Post-hoc  
662 descriptive analysis would be needed to determine whether an archetype's  
663 vulnerability reflects height, lack of interior partitions, large roof spans, or  
664 unreinforced walls.
- 665 • **Retrofit Documentation Bias:** Retrofit presence ranks second in hazard-  
666 inclusive setting demonstrating measurable protection. However, 98.4%  
667 of retrofit classifications carry high uncertainty, indicating assessors could  
668 identify that intervention occurred but not type or extent. This bias operates  
669 directionally: true retrofit prevalence likely exceeds the documented 28%

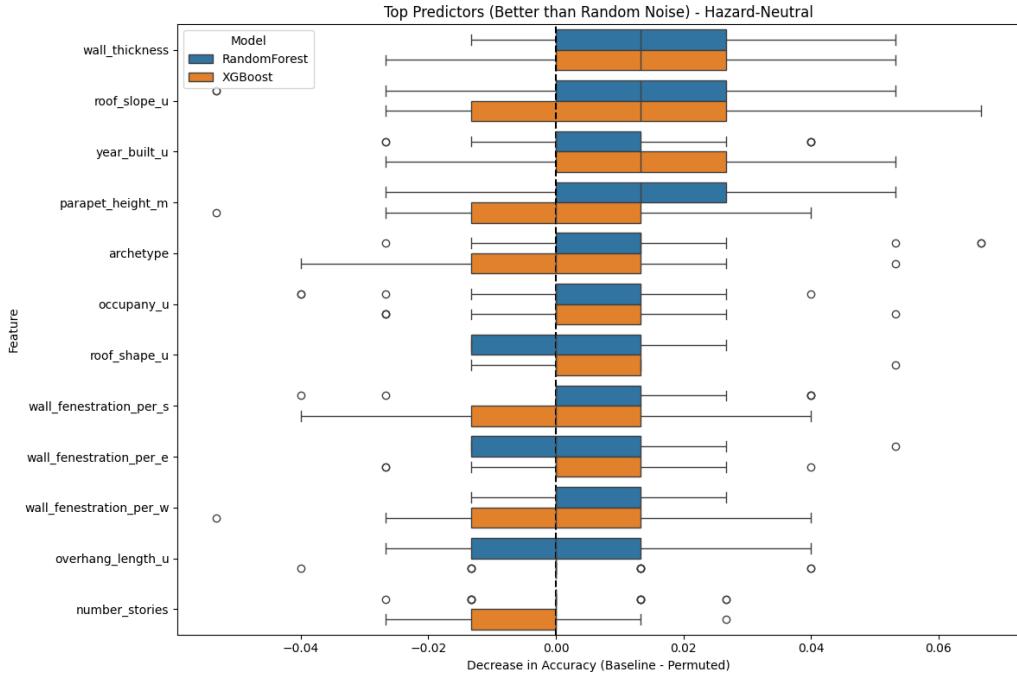
670 rate. Property owners implement partial upgrades (impact-resistant windows, isolated anchors during reroofing) without generating archival records.  
671 The "unretrofitted" comparison group therefore contains an unknown fraction  
672 of buildings with undocumented improvements, biasing measured effects toward null.  
673 The observed protective effect likely underestimates true retrofit benefits.  
674

675 **• Limited Variability and Sample Constraints:** Dataset composition introduces range restrictions affecting feature importance. Construction year spans 1781–1986, but 98% of buildings predate 1950, with only 5 representing post-1950 construction. This precludes validation of modern code effectiveness. Wall thickness exhibits similar clustering: 50% of buildings fall within 0.20–0.30 m, with 11% exceeding 0.3 m. If vulnerability increases sharply at specific slenderness ratios but dataset contains few buildings above or below critical values, permutation importance underestimates effects by testing predominantly mid-range combinations.

685 **• Categorical Aggregation Obscuring Mechanisms:** Wall substrate includes "masonry," "not applicable", and "wooden," each representing distinct structural systems, yet 43.7% of masonry classifications carry high uncertainty indicating assessors could not distinguish reinforced versus unreinforced. If reinforced masonry is protective but unreinforced vulnerable, the average "masonry" effect may appear neutral. Permutation captures only category-average effects across uncertain assignments, missing within-category heterogeneity. Roof substrate exhibits identical issues: only 34% of classifications achieved certain confidence, indicating visual inspection identified wood framing but could not reliably distinguish dimensional lumber versus engineered trusses.

696 Features exhibiting consistently low or negative importance across all models  
697 were excluded from visualization to maintain focus on actionable predictors. A  
698 lack of measured importance does not necessarily imply physical irrelevance; it  
699 may instead reflect data limitations such as insufficient variability, measurement  
700 error, or coarse categorical definitions. The top-ranking features should be val-  
701 idated against engineering-based fragility functions and mechanistic wind load  
702 models to ensure that the statistical associations aligns with the physical behav-  
703 ior. Features demonstrating both high importance and well-understood physical  
704 mechanisms (wall thickness, retrofit type) represent strong candidates for prior-  
705 itization in building codes and retrofit strategies. Additionally, proxy variables

706 (construction year, building archetype) should be interpreted with caution until  
 707 their underlying contributing factors can be more explicitly disentangled.



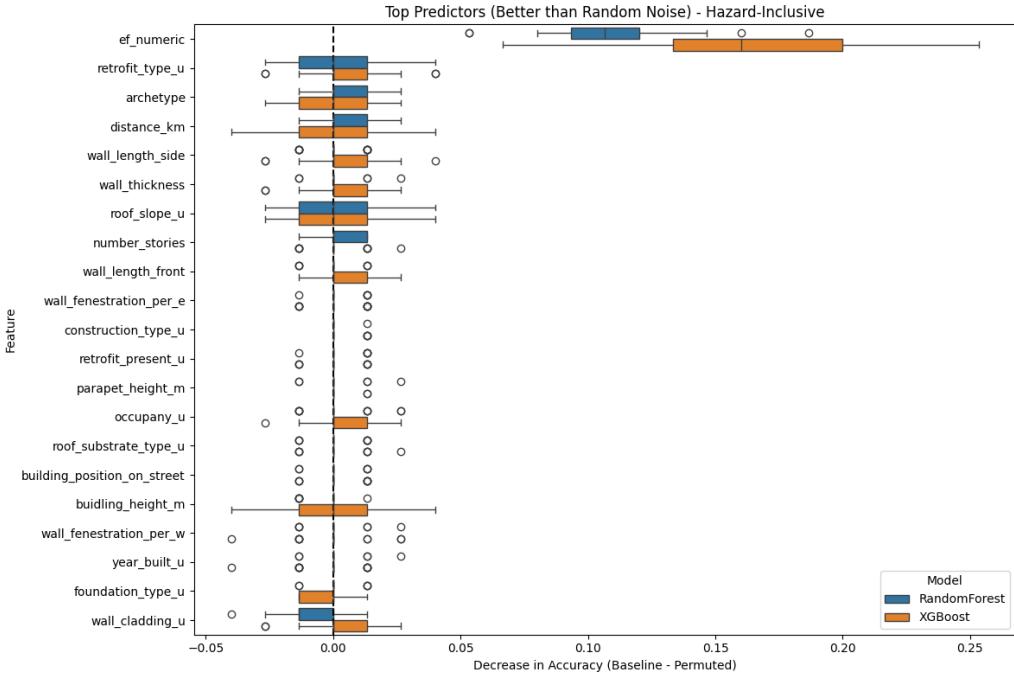
**Fig. 7.** Permutation importance (Decrease in Accuracy) for the **Hazard-Neutral** setting. The plot displays only features that outperformed the random noise baseline. Roof substrate type and parapet height emerge as the strongest predictors.

#### 708 4.4. Mechanistic Understanding via SHAP Analysis

709 While permutation importance established global rankings, the SHAP analysis  
 710 provided granular, instance-level insights into potential mechanisms by quantifying  
 711 each feature's contribution to individual predictions. The analysis examined SHAP  
 712 value distributions for each damage class (undamaged, low damage, significant  
 713 damage), identifying how individual features drive buildings across the damage  
 714 spectrum.

##### 715 4.4.1. Hazard Intensity Dominance

716 EF rating exhibits the most extreme SHAP magnitudes across all damage  
 717 classes, confirming tornado intensity as the primary determinant of structural out-  
 718 comes. For significantly damaged buildings (Class 2, Figure 10), high EF ratings



**Fig. 8.** Permutation importance (Decrease in Accuracy) for the **Hazard-Inclusive** setting. EF rating dominates by an order of magnitude, confirming that hazard intensity is the primary driver. Roof substrate remains the strongest intrinsic building predictor.

produce positive SHAP values pushing predictions strongly toward failure. Conversely, for undamaged buildings (Class 0, Figure 9), low EF ratings indicate that buildings that were not subjected to failure-level wind speeds, survived regardless of construction quality. In the transition zone (Class 1, Figure 9), moderate EF ratings cluster near zero SHAP, representing the narrow intensity band where building characteristics determine whether damage remains localized or progresses to complete failure. However, EF rating's dominance should be interpreted recognizing that these classifications are damage-derived rather than independently measured, creating circularity in damage prediction models.

#### 4.4.2. Distance from Tornado Path as Intensity Proxy

Proximity to the tornado centerline serves as an intensity proxy, modulating damage predictions across all structural classes. For significantly damaged buildings (Class 2), closer proximity correlates with higher damage probabilities, reflecting the extreme wind loads and debris impact associated with the vortex

733 core. Conversely, for Undamaged (Class 0), increasing distance highlights the  
734 rapid attenuation of wind speeds below the threshold required for structural or  
735 envelope damage initiation.

736 This spatial gradient is most nuanced within transition damage zone (Class 1),  
737 where intermediate distances align with wind speeds sufficient to cause component-  
738 level failures, such as cladding or roofing, without reaching the pressures necessary  
739 for progressive collapse. These findings underscore a critical limitation in post-  
740 event assessments: structures at the periphery of an EF2/3-rated path may be  
741 subjected to significantly lower wind speeds than those near the centerline, despite  
742 both being grouped under the same categorical rating in official damage surveys.

#### 743 *4.4.3. Construction Year*

744 The year of construction serves as a longitudinal proxy for structural resilience,  
745 exhibiting a clear directional influence across the damage spectrum. In Class 2  
746 (Significant Damage), the model reveals a distinct gradient: older structures (low  
747 year\_built values) are associated with positive contributions, indicating elevated  
748 vulnerability. This trend is likely a direct reflection of the iterative strengthening of  
749 building codes, most notably the post-1980 enhancements in wind-load provisions  
750 and structural connectivity requirements. Conversely, for the undamaged class  
751 (Class 0) the influence of newer construction is positive but less pronounced than  
752 the vulnerability seen in the damage classes. Its lower SHAP ranking suggests  
753 that for undamaged structures, age is secondary to external hazard intensity, such  
754 as increased distance from the tornado centerline [41].

755 In the transition zone (Class 1), SHAP values cluster near zero, indicating  
756 that age alone is an insufficient predictor of localized damage. While modern  
757 codes reduced catastrophic failure risk, they did not eliminate the component-level  
758 vulnerabilities like roofing or fenestration failures that characterize the low-damage  
759 state [42].

#### 760 *4.4.4. Structural and Envelope Features: Walls, Roofs, and Cladding*

761 While hazard intensity dominates damage outcomes, building-specific charac-  
762 teristics determine vulnerability within a given exposure level.

- 763 • **Wall Substrate and Masonry Performance:** Wall substrate features ex-  
764 hibit class-dependent effects that reflects structural capacity. In significantly  
765 damaged building (Class 2), masonry exhibits high positive values, exhib-  
766 iting high vulnerability under extreme wind pressures [24]. Conversely, in  
767 undamaged buildings (Class 0), a strong positive influence is seen. For

768 buildings in the transition zone (Class 1), substrate effects are largely muted,  
769 indicating that for low-level damage, there are other factors like the distance  
770 from the tornado or the year of construction that come into play.

- 771 • **Roof Geometry:** Roof configuration strongly influences uplift vulnerability,  
772 with effects varying by damage severity [17]. For significantly damaged  
773 buildings (Class 2), flat roof geometries and tall parapets emerge as vulnerability  
774 factors. Flat roofs experience higher suction forces and tall parapets  
775 act as vertical cantilevers subjected to wind pressure. Parapet overturning  
776 generates large overturning moments at the base of unbraced masonry ele-  
777 ments projecting above the roof line [43], which become failure points when  
778 roof diaphragms lack adequate anchorage to resist lateral forces.

779 In undamaged buildings (Class 0), roof geometry features rank lower in im-  
780 portance, with survival dominated by hazard intensity proxies (distance, EF  
781 rating) and primary structural system capacity rather than roof configuration  
782 alone. For the transition zone (Class 1), flat roof profiles and elevated para-  
783 pets contribute positively to damage probability, initiating localized failures,  
784 roof membrane breaches, parapet cracking, flashing detachment, even when  
785 the primary structural system remains intact. This pattern validates the hy-  
786 pothesis that Class 1 damage results from component-level vulnerabilities  
787 rather than global capacity exceedance.

- 788 • **Fenestration:** Wind vulnerability is compounded by high fenestration per-  
789 centages, as the building envelope acts as the first line of defense against  
790 wind pressurization. Post-event surveys confirm that once windows breach,  
791 internal pressure rapidly increases, significantly amplifying uplift forces on  
792 the roof [44].

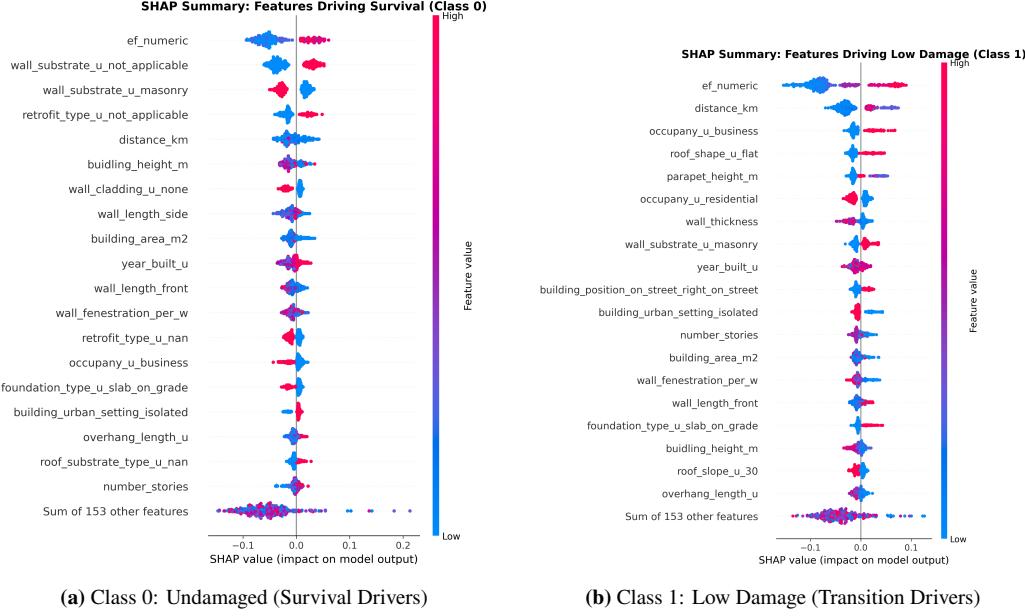
793 In significantly damaged buildings (Class 2), high fenestration percentages  
794 exhibit predominantly positive SHAP contributions, reflecting compounding  
795 vulnerabilities: structural discontinuities reduce effective wall area for lateral  
796 load resistance, while sudden internal pressurization following envelope  
797 breach amplifies roof uplift forces. In undamaged buildings (Class 0),  
798 fenestration effects cluster near zero SHAP, indicating that survival depends  
799 primarily on reduced wind exposure rather than specific window ratios.  
800 In the transition zone (Class 1), fenestration exhibits bidirectional SHAP  
801 contributions with high variance. This suggests that a single window failure  
802 can elevate a building to "low damage" status through water intrusion and  
803 localized envelope breach, even when the primary structure remains intact.

804       The high variance across buildings indicates that fenestration effects depend  
805       on breach status (intact vs. failed windows) and construction quality rather  
806       than opening ratio alone.

- 807       • **Building Geometry:** Overall building geometry determines the magnitude  
808       of wind loads that must be redistributed through the structural system. Build-  
809       ing height emerges as a primary driver of failure in significantly damaged  
810       buildings (Class 2), reflecting the increase in wind velocity with elevation  
811       above ground [45].

812       For undamaged buildings (Class 0), building height shows predominantly  
813       negative SHAP contributions, taller buildings that survived likely possessed  
814       superior construction quality or structural systems adequate for their expo-  
815       sure. This inverse relationship suggests that height alone does not determine  
816       survival; rather, taller historic buildings that remain in service have typically  
817       received maintenance and reinforcement proportional to their structural de-  
818       mands. In the transition zone (Class 1), height exhibits bidirectional SHAP  
819       contributions with wide scatter, indicating that building elevation plays a  
820       secondary role compared to component-level vulnerabilities like envelope  
821       condition and roof attachment quality.

- 822       • **Load Path Continuity and Vulnerability Interactions:** SHAP patterns  
823       across structural features converge on a critical insight: building perfor-  
824       mance under extreme winds is governed by load path continuity—the ability  
825       to transfer forces from wind-exposed surfaces through structural connections  
826       to the foundation. Features exhibiting scattered, bidirectional SHAP values  
827       (wall substrate, wall length, parapet height) indicate that their influence is  
828       not independent but rather conditional on complementary characteristics,  
829       particularly connection integrity and anchorage quality. Structural failure  
830       occurs when any link in this chain; roof, roof-to-wall connection, or wall-to-  
831       foundation anchorage reaches capacity. A building with thick masonry walls  
832       will fail if roof-to-wall connections are inadequate to transfer uplift forces,  
833       just as a well-anchored roof will fail if the wall substrate cannot resist lat-  
834       eral loads. This mechanistic understanding shifts preservation strategy from  
835       global strengthening to targeted retrofitting: interventions should prioritize  
836       securing load path weak points, as individual component strength becomes  
837       irrelevant when force transfer is interrupted at any connection.

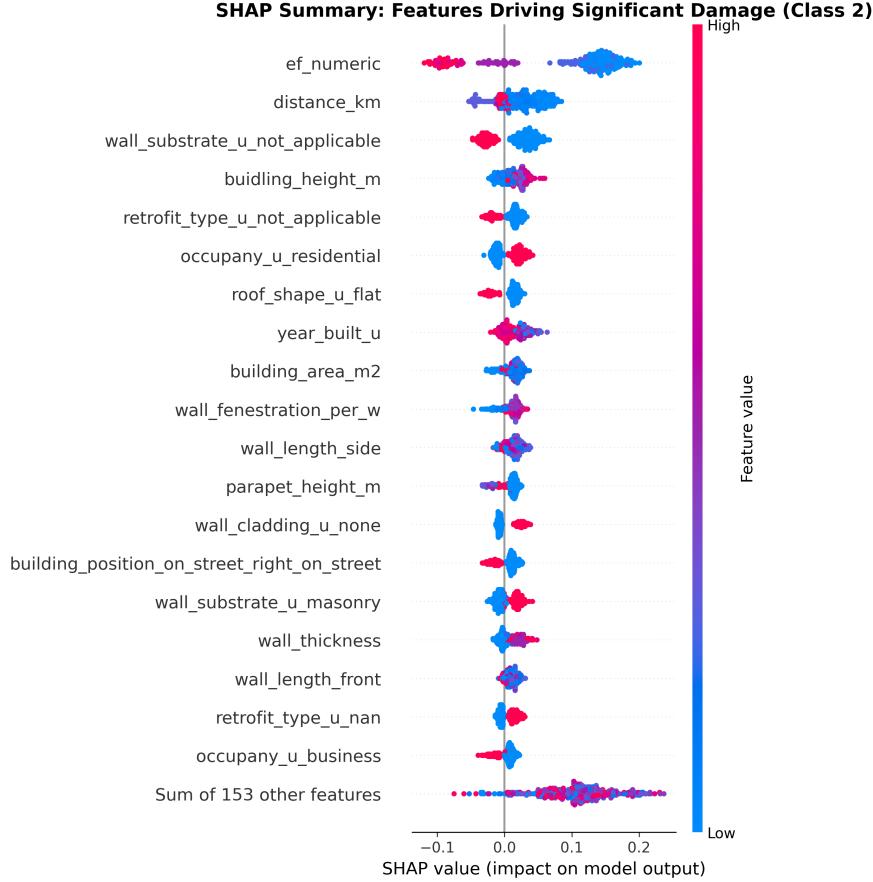


**Fig. 9.** SHAP Summary plots for (a) Undamaged and (b) Low Damage states. For Class 0, retrofit status and first floor elevation emerge as key survival factors. For Class 1, MWFRS configuration and wall cladding distinguish buildings in the transitional damage zone.

## 838 5. Comparison: Permutation Importance vs. SHAP

839 Comparing the top features identified by permutation importance and SHAP  
 840 analysis (Tables 5 and 6) reveals a fundamental agreement. Both methods showed  
 841 concordance on hazard intensity dominance (ef and distance) ranked high across  
 842 all analyses, confirming that tornado strength and proximity remain the primary  
 843 determinants of structural failure. However, several building characteristics exhibited  
 844 striking rank disparities between methods. For Class 2 (significant damage),  
 845 SHAP analysis elevated wall fenestration, building height, area, occupancy and  
 846 wall substrate to top-tier importance, while permutation importance ranked them  
 847 substantially lower. Conversely, archetype and roof slope ranked highly in permutation  
 848 analysis but dropped in SHAP rankings.

849 This divergence likely reflects fundamental methodological differences rather  
 850 than contradictory findings. Permutation importance measures global, capturing  
 851 features with consistent, additive effects. SHAP values quantify local, conditional  
 852 contributions to individual predictions, revealing features that are instance-specific  
 853 mechanisms. Features elevated in SHAP (fenestration, building geometry, wall  
 854 substrate) align with known physical interaction patterns: fenestration impacts in-



**Fig. 10.** SHAP summary plot for Significant Damage (Class 2). Features ranked by mean absolute SHAP impact. EF rating dominates, but retrofit status and wall substrate information emerge as critical vulnerability factors independent of wind intensity.

855 ternal pressurization; building height and area influence damage through interactions  
 856 with structural system type; wall substrate importance depends on reinforcement  
 857 presence and thickness—all conditional relationships invisible to marginal  
 858 permutation analysis. The cross-model validation strengthens confidence in these  
 859 SHAP-identified patterns. Both RandomForest and XGBoost independently el-  
 860 evated the same features, achieving strong overall rank concordance (Spearman  
 861  $\rho = 0.86$  for Class 2).

862 The complementary nature of these methods provides actionable insights for  
 863 vulnerability reduction. The consistent elevation of these features across both  
 864 model families supports the hypothesis that they play a mechanistic role in dam-

865 age vulnerability, likely through complex interactions with other structural components.  
 866 Nevertheless, given the small sample size for the low-damage class,  
 867 these findings should be treated as hypotheses requiring further validation through  
 868 physics-based modeling.

**Table 5.** Comparison of Top Features: Permutation Importance vs. SHAP (Class 2 - Significant Damage)

Feature	Top 10 Perm?	RF SHAP Rank	XGB SHAP Rank
ef_numeric	Yes (Rank 1)	1	1
distance_km	Yes (Rank 3)	2	2
retrofit_type_u	Yes (Rank 2)	5	11
archetype	Yes (Rank 4)	38	30
wall_length_side	Yes (Rank 5)	11	7
wall_thickness	Yes (Rank 6)	15	22
roof_slope_u	Yes (Rank 7)	26	23
number_stories	Yes (Rank 8)	22	33
wall_length_front	Yes (Rank 9)	17	15
wall_fenestration_per_e	Yes (Rank 10)	20	17

**Table 6.** Comparison of Top Features: Permutation Importance vs. SHAP (Class 1 - Transition Zone)

Feature	Top 10 Perm?	RF SHAP Rank	XGB SHAP Rank
ef_numeric	Yes (Rank 1)	1	1
distance_km	Yes (Rank 3)	2	2
retrofit_type_u	Yes (Rank 2)	a	a
archetype	Yes (Rank 4)	>20	>20
wall_length_side	Yes (Rank 5)	a	a
wall_thickness	Yes (Rank 6)	7	7
roof_slope_u	Yes (Rank 7)	18	>20
number_stories	Yes (Rank 8)	12	12
wall_length_front	Yes (Rank 9)	15	15
wall_fenestration_per_e	Yes (Rank 10)	a	a

<sup>a</sup>Not in top 20 SHAP features for Class 1

<sup>b</sup>Business (rank 3) and residential (rank 6) categories

869 **6. Discussion**

870 *6.1. Methodological Contributions and Limitations*

871 This study establishes a framework for extracting valid scientific insights from  
872 machine learning models even when perfect predictive accuracy remains elusive.  
873 Rather than reporting only a single “best” model, which risks overfitting to dataset  
874 idiosyncrasies, this study identified a family of statistically equivalent models  
875 through Wilcoxon testing with Holm-Bonferroni correction. Consequently, these  
876 findings are only considered important if they replicate across these equivalent  
877 approaches.

878 However, they must be interpreted within the context of significant data limita-  
879 tions. The “Low” damage class, representing the transition zone between survival  
880 and failure, contained only 41 buildings (11% of the sample). Despite the use of  
881 SMOTENC oversampling, the modest F1 score of 0.49 for this class indicates that  
882 the models struggle to reliably distinguish minor damage from other states. The  
883 per-class performance (Appendix A) reveals an important pattern: models excel at  
884 distinguishing undamaged buildings ( $F1=0.93$ ) from significantly damaged ones  
885 ( $F1=0.72$ ), but struggle with the “Low” damage transitional state. This likely  
886 reflects genuine physical ambiguity rather than merely insufficient data. Buildings  
887 in this class may exhibit: (1) partial component failures (e.g., roof partially up-  
888 lifted but not lost) that share characteristics with both intact and collapsed states;  
889 (2) damage to non-structural elements (cladding, openings) while the structural  
890 system remains viable, creating overlapping feature spaces; and (3) progressive  
891 damage where initial wind loading caused repairable damage but didn’t trigger  
892 cascade failures that lead to collapse. Additionally, damage assessment for mi-  
893 nor damage is inherently more subjective than for complete structural failure,  
894 potentially introducing classification noise. Together, these factors suggest the  
895 transitional class represents a genuinely fuzzy boundary rather than a well-defined  
896 category, explaining why even strong models cannot reliably predict it with lim-  
897 ited samples. Post-hoc power analysis suggests that with  $n=41$  low-damage cases,  
898 can detect main effects with Cohen’s  $d \geq 0.8$  at 80% power, but are substantially  
899 underpowered (power < 50%) for moderate effects ( $d = 0.5$ ) or interaction effects.

900 *6.2. SHAP Interaction Analysis*

901 SHAP interaction analysis revealed divergent patterns between damage classes.  
902 Class 2 (Significant Damage) interactions concentrated on EF rating combined with  
903 building type and geometry: residential occupancy, fenestration, and year built.

904 Nine of the top 12 interactions involved EF rating, with building characteristics  
 905 (occupancy, size, height) dominating rankings.  
 906 Class 1 (Low Damage) exhibited more distributed interaction patterns. While  
 907 EF rating interactions remained strong—fenestration, year built, residential—material  
 908 and construction features appeared more frequently. The interaction distribution  
 909 was more balanced: 7 of top 12 involved hazard intensity versus 5 involving prox-  
 910 imity. Material characteristics comprised a larger proportion of Class 1’s top 20  
 911 interactions compared to Class 2, reflecting the transition zone’s vulnerability pro-  
 912 file where multiple moderate weaknesses contribute to repairable damage rather  
 913 than catastrophic failure.  
 914 While these patterns identify feature combinations statistically associated with  
 915 damage outcomes, they represent correlations rather than confirmed causal mecha-  
 916 nisms. The strongest interactions warrant targeted engineering validation to estab-  
 917 lish physical causality and quantify structural response under controlled loading  
 918 conditions.

**Table 7.** Key Feature Interactions Driving Significant Damage (SHAP Analysis)

Feature Interaction	XGB	RF
EF Rating × Residential	0.069	0.002
EF Rating × Fenestration	0.054	0.001
EF Rating × Year Built	0.042	0.002
EF Rating × Building Area	0.036	0.001
EF Rating × Building Height	0.035	0.001
Residential × Year Built	0.032	–
EF Rating × Cladding	0.029	–
EF Rating × Wall Length	0.028	–
Distance × EF Rating	0.024	0.003
Distance × Wall (Unknown)	0.018	0.003
EF Rating × Retrofit Absent	0.016	0.002

### 919 6.3. Candidate Areas for Future Engineering Validation

920 The findings generate several testable hypotheses for risk-based preservation,  
 921 identifying specific building features that warrant detailed engineering evaluation.

#### 922 6.3.1. Synthesis: Feature Importance Mapped to Failure Mechanisms

923 The integration of permutation importance and SHAP analysis enables sys-  
 924 tematic mapping of statistical predictors to established wind engineering failure

**Table 8.** Key Feature Interactions Driving Low Damage (SHAP Analysis)

Feature Interaction	XGB	RF
EF Rating × Fenestration	0.043	–
EF Rating × Year Built	0.042	0.002
EF Rating × Residential	0.041	0.002
EF Rating × Wall Thickness	0.038	0.002
Distance × EF Rating	0.035	0.004
EF Rating × Number Stories	0.033	0.002
EF Rating × Parapet Height	0.025	0.002
EF Rating × Wall Length	0.019	–
EF Rating × Foundation	0.016	0.002

925 modes. This synthesis organizes findings according to documented tornado dam-  
926 age progressions in masonry structures, identifying which failure pathways govern  
927 vulnerability in the historic masonry buildings [25, 45]. Table 9 categorizes  
928 high-importance features by their primary mechanical influence, prioritizing en-  
929 gineering validation efforts for mechanisms with both strong statistical evidence  
930 and critical life-safety implications.

**Table 9.** Feature Importance by Structural Failure Mechanism

Mechanism	Associated Features	Perm. Imp.	SHAP Evidence	Engineering Priority
Roof uplift	roof_slope, roof_substrate	High	Positive for flat	CRITICAL
Parapet overturning	parapet_height	High	Threshold at 1.5m	HIGH
Wall out-of-plane	wall_thickness, anchorage	High	Thin walls vulnerable	CRITICAL
Envelope breach	fenestration_per, cladding	Medium	High % increases risk	MODERATE
Progressive collapse	retrofit_type, MWFRS	High	Absence critical	CRITICAL

### 931 6.3.2. Roof System Vulnerabilities

932 Roof system failures dominate the damage patterns, documented in 60–80%  
933 of significantly damaged structures [9, 41]. Three critical validation priorities  
934 emerged: aerodynamic loading, connection capacity evolution, and progressive  
935 membrane failure.

936 Flat roofs ranked among top predictors, consistent with wind tunnel measure-  
937 ments showing higher suction versus gabled roofs [17, 46]. However, it is not yet  
938 possible to distinguish whether failures stem from excessive uplift forces or inad-  
939 equate connections. CFD modeling of tornado vortices on buildings with varied  
940 roof slopes could quantify pressure distributions, while wind tunnel testing would

941 establish whether quasi-steady design assumptions adequately capture transient  
942 tornado effects.

943 Construction year functions as a proxy for connection improvements: pre-1980  
944 toenails versus post-1994 engineered clips. Destructive testing of connections ex-  
945 tracted from buildings spanning construction eras would establish in-situ capacity  
946 distributions accounting for aging and degradation.

947 Roof systems showed vulnerability to edge-initiated peeling where wind pen-  
948 etration creates expanding uplift zones. Component testing of representative  
949 configurations under simulated uplift would document initiation pressures and  
950 propagation rates. FEM modeling validated against tests would enable parametric  
951 studies of attachment spacing and edge detailing effects.

952 *6.3.3. Wall System Interactions*

953 Wall failures reflect combined geometric and material vulnerabilities, with  
954 thin unreinforced masonry walls in tall buildings showing disproportionate dam-  
955 age rates. The critical validation priorities emerged: anchorage capacity and  
956 preservation-sensitive interventions.

957 The analysis identified wall substrate and absent retrofits as independent predic-  
958 tors pushing toward severe damage, but it is not yet possible to distinguish whether  
959 their combined presence creates multiplicative or additive risk. This suggests  
960 wall-to-diaphragm and wall-to-roof anchors deserve investigation alongside wall  
961 reinforcement, though relative efficacy remains unknown. In-situ pull-out testing,  
962 full-scale wall-roof assembly testing under simulated wind pressures would mea-  
963 sure load redistribution when connections fail sequentially, establishing whether  
964 adequate anchorage prevents progressive collapse even after partial roof loss.

965 While invasive techniques such as grouted rebar or fiber-reinforced poly-  
966 mer wraps provide well-documented strengthening, these permanently alter ma-  
967 sonry fabric. A more preservation-sensitive approach would investigate reversible  
968 strong-back systems or internal moment frames providing out-of-plane support  
969 without modifying exterior appearance or original material [25]. Component test-  
970 ing comparing strengthening effectiveness (capacity improvement per dollar in-  
971 vested) and reversibility of various intervention strategies would establish whether  
972 non-invasive methods achieve adequate protection thresholds for expected tornado  
973 intensities in historic districts.

974 Envelope breach through window failure and cladding loss initiates progressive  
975 damage via internal pressurization. Three validation priorities emerged: thresh-  
976 old mechanism isolation, cladding progressive failure, and opening protection  
977 strategies.

978 Fenestration percentage operates through interaction effects, showing low  
979 marginal importance but high conditional SHAP contributions when combined  
980 with wall type and construction quality. However, it is not yet possible to distin-  
981 guish whether damage stems from structural discontinuity interrupting load paths  
982 or from internal pressurization amplifying roof uplift following window breach.  
983 CFD modeling of buildings with varied fenestration ratios under breach scenarios  
984 would quantify internal pressure amplification as functions of opening area and lo-  
985 cation. Structural testing of wall panels with systematically varied openings under  
986 lateral loading would isolate capacity degradation independent of pressurization,  
987 establishing which mechanism governs vulnerability.

988 Cladding loss initiates cascading damage through water intrusion and debris  
989 generation. Large-scale testing of prevalent systems (vinyl siding, brick veneer  
990 with corrugated ties) under pulsating tornado-representative pressures would es-  
991 tablish failure thresholds. Pull-out testing of corroded ties from existing buildings  
992 would quantify in-situ capacity degradation rates.

993 Component testing of impact-resistant glazing, shutters, and films under com-  
994 bined debris impact and cyclic pressure would document breach prevention rates.  
995 Post-event correlation analysis comparing protected versus unprotected openings  
996 would validate whether protection prevents internal pressurization-triggered roof  
997 failures.

#### 998 6.3.4. *Preservation Philosophy and Reversibility*

999 Any structural intervention in a historic building must be weighed against the  
1000 Secretary of the Interior's Standards for Rehabilitation. Our analysis identifies  
1001 features for potential retrofit, but the method of intervention must be evaluated  
1002 for compliance. To guide this evaluation, a compatibility assessment framework  
1003 for candidate interventions (Table 10) is proposed. Here a distinction is made  
1004 between *mechanical reversibility* (the intervention can be physically removed) and  
1005 *material reversibility* (removal restores the original condition without permanent  
1006 alteration). For example, while hurricane straps are mechanically removable, their  
1007 installation requires lag bolts penetrating rafters and joists, creating permanent  
1008 holes that compromise timber integrity even after removal. Under National Park  
1009 Service guidance, this constitutes “minimally invasive” intervention rather than  
1010 true reversibility. Invasive techniques like grouted rebar fail both criteria, as they  
1011 cannot be removed without destroying the masonry fabric. Future engineering  
1012 research should prioritize interventions that achieve mechanical removability while  
1013 minimizing material alteration, such as compression-based systems or friction  
1014 connections that avoid penetrating fasteners.

**Table 10.** Preservation Compatibility of Candidate Interventions

Intervention	Standard 2 (Character)	Standard 10 (Reversible)	Assessment
Hurricane straps	Yes - Hidden	Mech: Yes / Mat: No	Moderate compatibility; lag bolts create permanent holes in timber
Wall-to-diaphragm anchors	Yes - Interior	Mech: Partial / Mat: No	Moderate; anchor holes remain after removal
Grouted rebar	No - Invasive	Mech: No / Mat: No	Low compatibility and irreversible; avoid except for life-safety emergencies
FRP wraps	No - Visible	Mech: No / Mat: No	Low compatibility; moisture entrapment risk; investigate alternatives
Strong-back systems	sys- Yes - Interior	Mech: Yes / Mat: Partial	High compatibility; mechanical fasteners minimize damage
Foundation mi- cropiles	mi- Yes - Hidden	Mech: No / Mat: No	Moderate; permanent but hidden; evaluate case-by-case

1015     *6.4. Risk-Informed Decision-Making for Preservation Authorities*

1016     While the findings demonstrate that 77% of historic URM buildings survived  
1017     tornado exposure with no structural damage, this encouraging statistic requires  
1018     careful contextualization for preservation policy. Survival of the building envelope  
1019     does not guarantee occupant safety, as partial component failures (chimney  
1020     collapse, parapet detachment, interior ceiling failure) can cause fatalities even  
1021     when the primary structure remains standing. Furthermore, the Mayfield EF4  
1022     tornado represents an extreme outlier event; preservation authorities must weigh  
1023     the cost of hardening the entire historic building stock against the low annual  
1024     probability of such catastrophic exposure.

1025     *6.4.1. Hazard Return Periods and Cost-Benefit Analysis*

1026     Tornado hazard maps indicate that EF4+ tornadoes have return periods ex-  
1027     ceeding 1,000 years for most locations in the study region, while EF1-EF2 events  
1028     occur with 50-100 year return periods. From a risk management perspective, this  
1029     raises a fundamental question: should preservation policy prioritize resilience to  
1030     rare catastrophic events, or focus resources on cost-effective interventions for more  
1031     frequent moderate events?

1032        For buildings with high cultural significance (National Register properties,  
1033        architecturally unique structures), the irreplaceable nature of the resource may  
1034        justify hardening against low-probability/high-consequence scenarios. However,  
1035        for the broader historic building stock, a tiered approach may be more economically  
1036        rational. A baseline strategy for all buildings would address partial component  
1037        failures that pose occupant risk even in moderate events, such as securing parapets  
1038        and anchoring chimneys. For designated properties, an enhanced strategy would  
1039        implement roof-to-wall connection upgrades and wall-to-diaphragm anchors to  
1040        prevent total loss in EF2-EF3 events. Exceptional cases involving buildings of  
1041        high cultural significance might justify complete structural upgrades, recognizing  
1042        that even these measures may not guarantee survival in EF5 conditions.

1043        This tiered framework acknowledges that perfect protection is neither techni-  
1044        cally feasible nor economically justifiable for the entire historic building stock,  
1045        while ensuring that preservation resources are allocated proportionally to both  
1046        cultural value and hazard probability.

#### 1047        *6.4.2. Limitations of Survival-Based Metrics*

1048        The current analysis focuses on building-level damage classification, but preser-  
1049        vation authorities must also consider interior hazards. Even “undamaged” build-  
1050        ings may have inadequate interior bracing, posing life-safety risks from falling  
1051        plaster, light fixtures, or unreinforced masonry partitions. These hazards are not  
1052        captured in exterior damage assessments. A building classified as “low damage”  
1053        may be structurally sound but lack utilities, weatherproofing, or code-compliant  
1054        egress, rendering it uninhabitable for months. Preservation policy should consider  
1055        not just survival, but recovery time and functional resilience. Also, the dataset cap-  
1056        tures single-event exposure, but buildings experiencing multiple moderate events  
1057        over decades may accumulate damage (e.g., mortar deterioration, connection fa-  
1058        tigue) that compromises performance in subsequent events. Longitudinal studies  
1059        are needed to assess cumulative vulnerability.

#### 1060        *6.5. Future Work: Quantifying Preservation Interventions*

1061        While the proposed tiered framework provides a strategic roadmap, it currently  
1062        lacks the quantitative grounding necessary for precise cost-benefit analysis. Fu-  
1063        ture work must bridge this gap by establishing typical retrofit costs per building,  
1064        using data from National Park Service guidance or industry standards to move the  
1065        framework from aspirational to operational. Additionally, finite element modeling  
1066        (FEM) is required to quantify how much specific interventions, such as para-  
1067        pet bracing, reduce failure probability under varying wind loads, following FEM

1068 frameworks established for masonry systems [47]. Finally, a decision-support  
1069 tool should be developed to help communities prioritize interventions within fixed  
1070 budgets, translating these technical findings into actionable policy.

## 1071 **7. Conclusions**

1072 This study demonstrates that machine learning can identify vulnerability fac-  
1073 tors in historic masonry buildings while challenging prevailing assumptions about  
1074 their fragility. Seventy-eight percent of historic buildings in this dataset survived  
1075 EF0-EF4 tornadoes with minimal or no damage, countering the narrative that  
1076 unreinforced masonry construction is inherently doomed in extreme wind events.  
1077 Rather than validating blanket condemnation of historic building stock, this anal-  
1078 ysis reveals targeted, addressable vulnerabilities.

1079 Machine learning confirmed physical mechanisms long suspected by struc-  
1080 tural engineers: parapets, roof connections, and building envelope characteristics  
1081 emerge as critical weak points. Roof substrate condition, and wall-to-roof connec-  
1082 tion details consistently ranked among the strongest predictors across six model  
1083 families, validated through permutation importance and SHAP analysis with a  
1084 random noise guardrail ensuring genuine predictive signal. These findings suggest  
1085 that preservation-compatible interventions such as parapet bracing, roof anchor-  
1086 age improvements, and envelope reinforcement using reversible techniques that  
1087 address the actual risk profile more effectively than invasive structural hardening  
1088 or wholesale demolition.

1089 The scarcity of low-damage cases limits our ability to fully characterize the  
1090 transition zone between survival and failure, where targeted interventions would be  
1091 most valuable. Future work should validate these data-driven hypotheses through  
1092 physics-based simulation (e.g., finite element modeling of parapet-diaphragm in-  
1093 teractions) and experimental testing of reversible retrofit strategies. By integrating  
1094 machine learning with structural engineering principles, this framework moves his-  
1095 toric preservation toward evidence-based risk mitigation that balances life safety  
1096 imperatives with cultural heritage stewardship—demonstrating that protecting his-  
1097 toric buildings and protecting building occupants are not competing objectives,  
1098 but complementary goals achievable through targeted, informed intervention.

## 1099 **Data Availability Statement**

1100 The dataset mentioned in this study is available on DesignSafe repository under  
1101 project number [PRJ-5614](#) and [PRJ-6212](#). Analysis code and additional materials  
1102 are available from the corresponding author upon reasonable request.

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1109 to data collection.

1110 **Appendix A. Detailed Classification Report**

1111 For transparency, Table A.11 provides per-class performance metrics for the  
1112 best-performing model (Random Forest, Hazard-Inclusive, averaged over 25 folds).

**Table A.11.** Per-Class Performance Metrics (Random Forest, Hazard-Inclusive, averaged over 25 folds)

Class	Precision	Recall	F1-Score	Support (%)
0 (Undamaged)	0.92	0.95	0.93	77%
1 (Low)	0.505	0.513	0.498	11%
2 (Significant)	0.889	0.643	0.723	12%
<b>Macro Avg</b>	0.772	0.703	0.719	—
<b>Weighted Avg</b>	0.873	0.869	0.865	—

1113 The classification report reveals that while the model achieves excellent per-  
1114 formance for the Undamaged class ( $F1=0.94$ ) and good performance for Signif-  
1115 icant damage ( $F1=0.75$ ), the “Low” damage class remains the most challenging  
1116 ( $F1=0.26$ ). This difficulty stems from its severe underrepresentation (only 5%  
1117 of data), the inherent ambiguity of the transition zone between undamaged and  
1118 significant states, and insufficient training examples even with SMOTENC over-  
1119 sampling.

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