# CEE432/CEE532/MAE541 Developing Software for Engineering Applications

Lecture 22: Finite Element Modeling of Structural Systems

### Assumptions

- Linear stress-strain relationship
  - Same behavior in tension and compression
  - No damage
  - No unloading and/or reloading
- Elastic behavior
- Small displacements
- Small strains

### Type of Analyses

• Static

$$\mathbf{K}_{n\times n}\mathbf{D}_{n\times m}=\mathbf{F}_{n\times m}$$

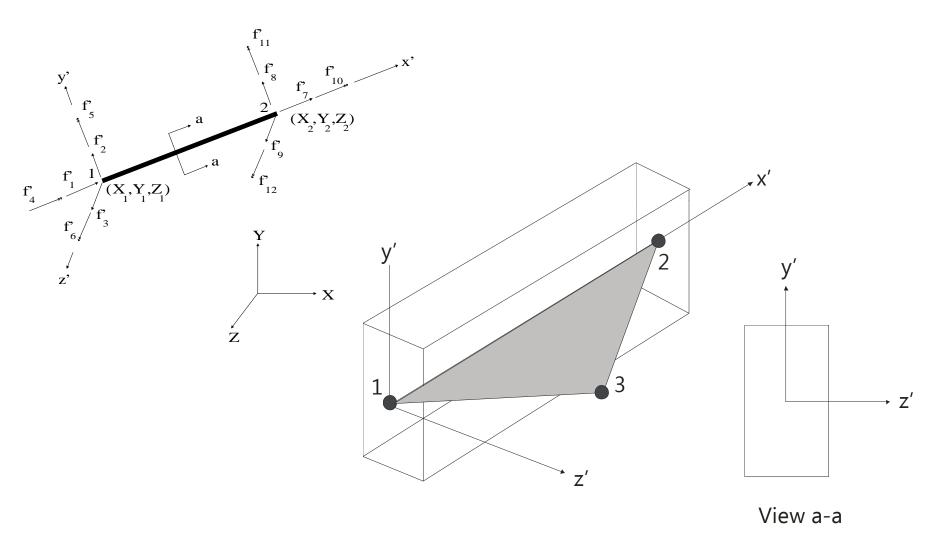
Modal

$$\mathbf{K}_{n\times n}\mathbf{\Phi}_{n\times n}=\mathbf{\Lambda}_{n\times n}\mathbf{M}_{n\times n}\mathbf{\Phi}_{n\times n}$$

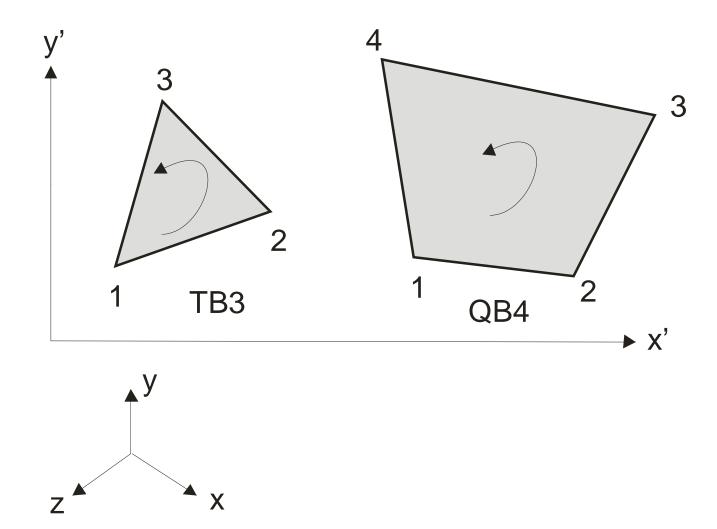
• Dynamic

$$\mathbf{M}_{n\times n}\ddot{\mathbf{D}}_{n\times m} + \mathbf{C}_{n\times n}\dot{\mathbf{D}}_{n\times m} + \mathbf{K}_{n\times n}\mathbf{D}_{n\times m} = \mathbf{F}(t)_{n\times m}$$

### Beam Finite Element



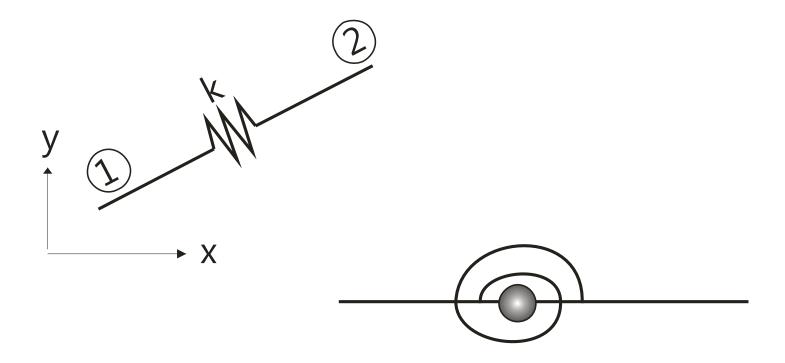
### Plate/Shell Finite Element



### Plate vs Shell

- Shell (TB3/QB4)
  - 6 dof/node
  - Flat and thin
  - Membrane plus bending
- Plate
  - 3 dof/node
  - Flat and thin
  - Bending only
  - Special case of shell

### Spring Elements



### Element Response

Element	Response
Beam	Element nodal forces
Shell	$\{\sigma_x,\sigma_y, au_{xy}\}$
Translational Spring	Element axial force
Torsional Spring	Element torsional moment

### Material Model

- Metal
  - Steel
  - Aluminum
- Brittle Composites
  - Concrete
  - Brick
- Elastic behavior
- Isotropic (E, CTE, Poisson's Ratio)

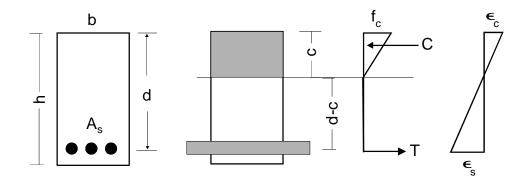
### Element Properties

Concrete Modulus of Elasticity

$$E_c = w_c^{1.5} 33\sqrt{f_c'} psi$$

$$E_c = 57000\sqrt{f_c'} psi$$

• Moment of Inertia



$$I_{gt} = \frac{bc^3}{3} + nA_s(d - c)^2$$

$$I_e = I_{gt} + \left(\frac{M_{cr}}{M_a}\right)^3 \left(I_g - I_{gt}\right)$$

### Loading

- Dead Load
  - Self-weight
  - Others
- Live Load
- Wind Load
- Snow Load
- Rain Load
- Seismic Load

### **Connection Points**

- Centroid of cross-section
- Mid-plane of shell element
- Beam and shell elements are compatible
  - Eccentricities in connections?

### Modeling Approximations

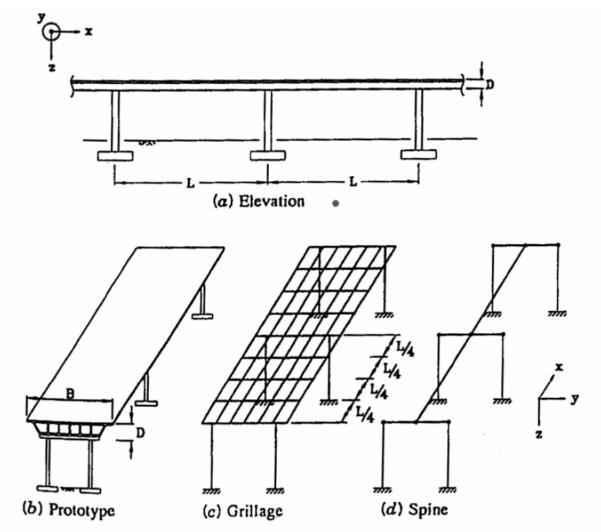


Figure 4.2-4 Superstructure Models (Priestley, et al 1996).

### Modeling Approximations

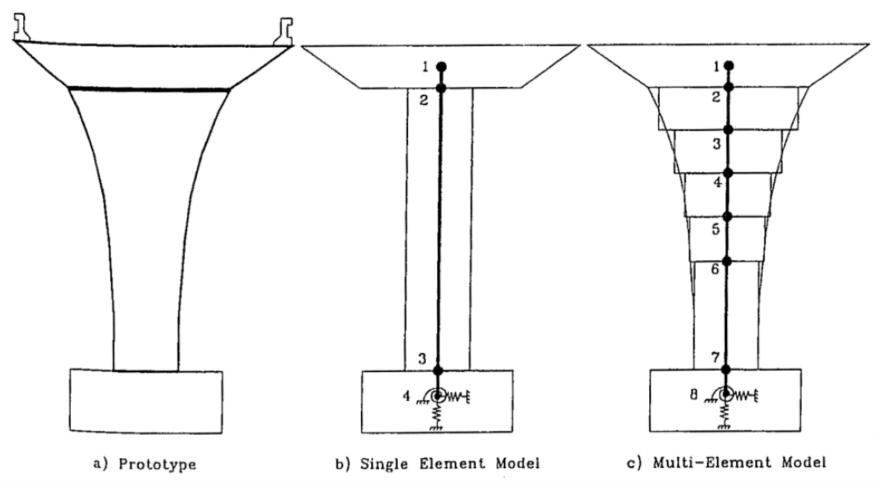
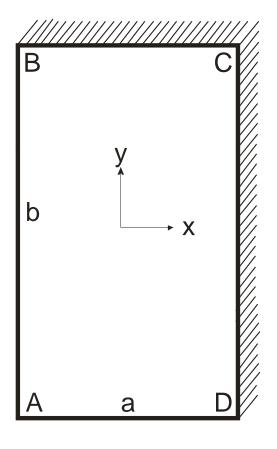


Figure 4.2-5 Single-Column Bent Models (Priestley et al, 1996).

### Case Study: Thin Steel Plate



$$t = 0.015 m$$

$$a \times b = 0.15 m \times 0.25 m$$

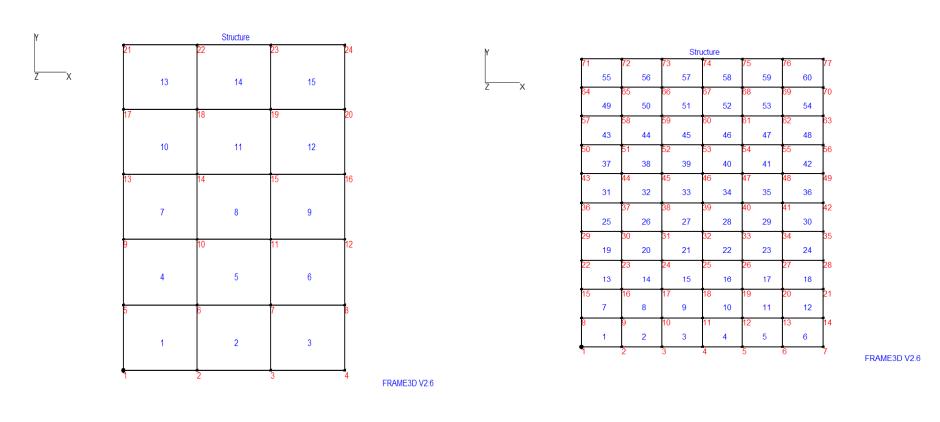
$$E = 200 GPa$$

$$v = 0.3$$

$$\rho = 7850 kg/m^3$$

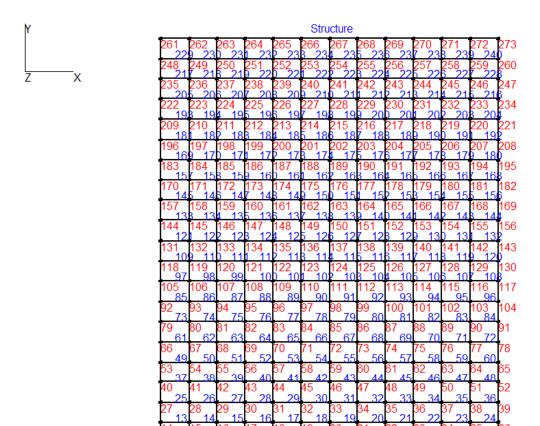
$$p = 2000 N/m^2$$

### Convergence Study



P1M1 P1M2

### Convergence Study



FRAME3D V2.6

**P1M3** 

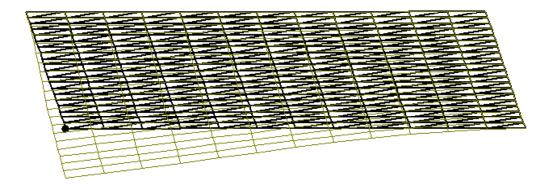
### FEA Results

Model Name	Element mesh size (m)	No. of nodes	Max. von Mises stress (Pa)	Max. Z-Nodal displacement (m)
P1M1	0.05	24	568812	2.43 x 10 <sup>-6</sup>
P1M2	0.025	77	645999	2.38 x 10 <sup>-6</sup>
P1M3	0.0125	273	684336	2.37 x 10 <sup>-6</sup>

### Deformed Shape Plot



Deformed Shape



#### Frame3D V2.72

Nodes: 273 Elements: 240 DOF: 1440 Load Cases: 1

(Model, Viewing Box) Limits

X Min: (0, 0) X Max: (0.15, 0.15)

Y Min: (0, 0)

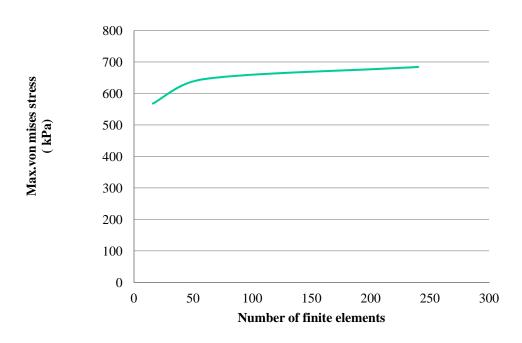
Y Max: (0.25, 0.25) Z Min: (0, 0) Z Max: (0, 0)

#### Model Info

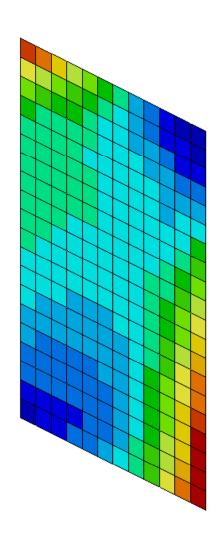
Max. Displacement: 2.36811e-006 Max. Rotation: 1.98766e-005 Magnification Factor: 6813.74

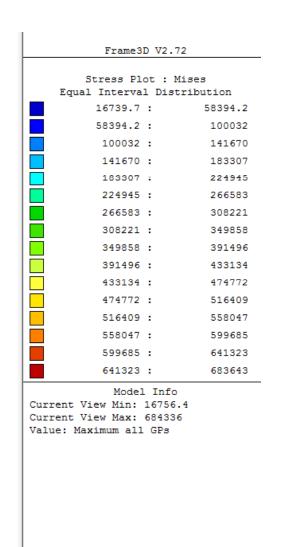
### Stress Convergence

#### **Convergence Check**

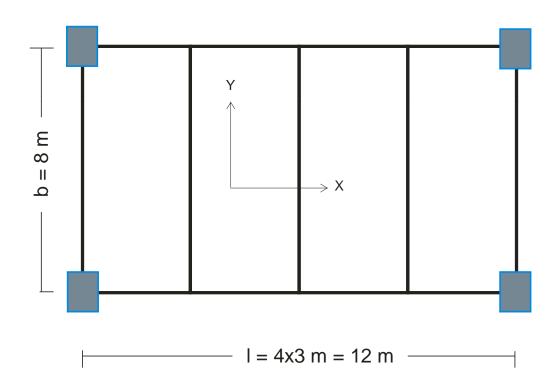


### von Mises Stress Plot





## Case Study: Reinforced Concrete Slab-Beam-Column System



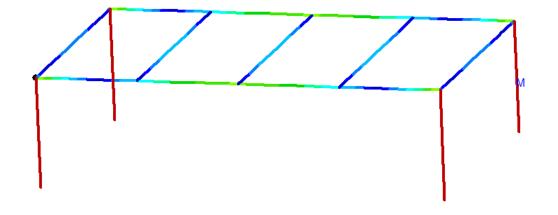
```
t = 0.1 m
h \times w = 0.4 m \times 0.25 m
E = 32 GPa
v = 0.15
\rho = 2400 kg/m^3
p = 5000 N/m^2
```

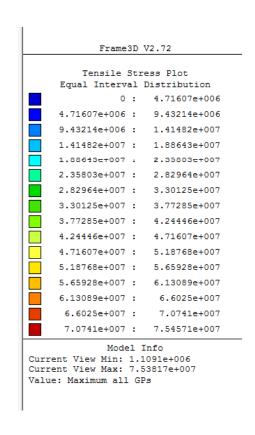
### FEA Results

<b>Model Name</b>	Element mesh size (m)	No. of elements	Max. Slab Compressive Stress (MPa)	Max. Slab Tensile Stress (MPa)	Max. Z-Nodal displacement (m)
P3M0	1.0	164	10.7	10.1	0.08440
P3M1	0.5	384	11.9	10.2	0.08455
P3M2	0.25	1536	12.5	10.2	0.08447

### Max. Tensile Stress Plot

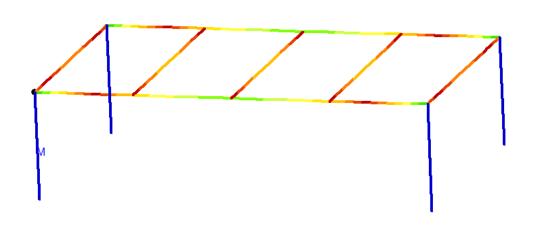




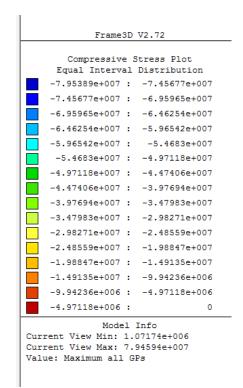


### Max. Compressive Stress Plot

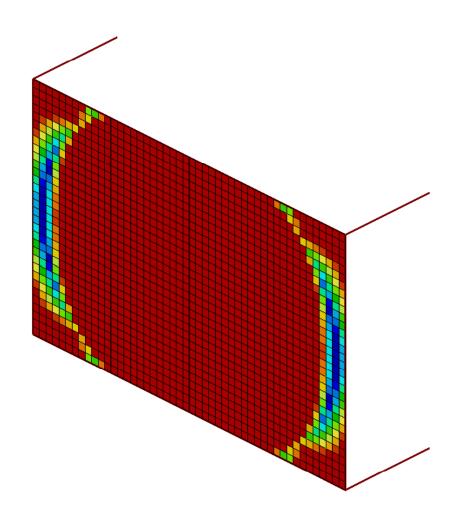


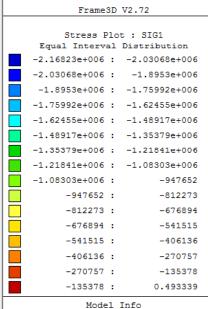


Compressive Stress



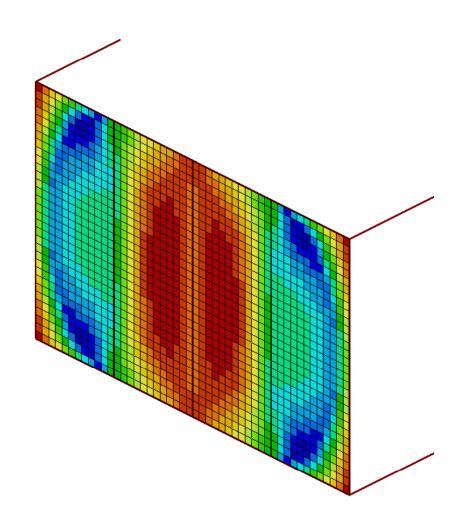
### Principal Stress 1 Plot

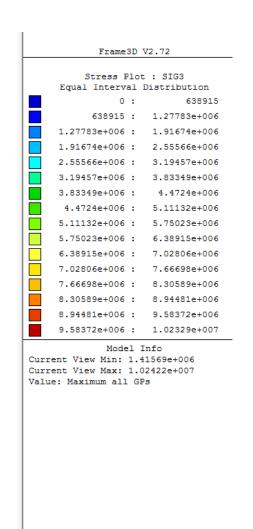




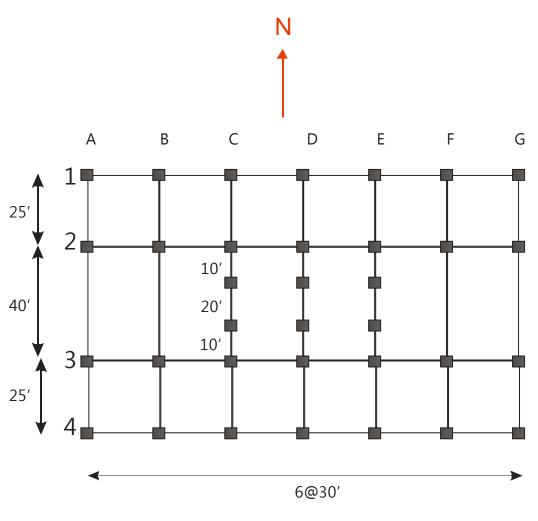
Current View Min: -2.44394e+006 Current View Max: 0.493441 Value: Maximum all GPs

### Principal Stress 3 Plot





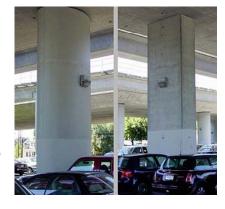
# Project 4: Modeling and Analysis of a Multi-Storied Building



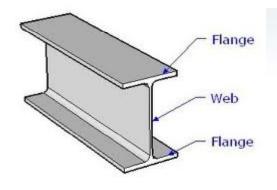
Floor	Height (ft)
Basement	16
Other floors	13

### Structural Elements

- Beams and Columns
  - Wide-flange sections, welded box beam
  - High strength reinforced concrete
- Floor system
  - 3" cellular deck plus 2.5" concrete slab









### Loading

- Live load
  - 100 psf (1st floor), 80 psf (other floors), 20 psf (roof)
- Dead Load
  - Self-weight
  - Roof: 48 psf (felt, gravel plus decking)
- Wind Load
  - N-S direction only
- 1 load case: DL + LL + WL

### Tutorial #1

 Model a 0.5" cantilever steel plate that is 60" long and 10" wide and is subjected to a uniformly distributed loading of 2 psf.
 Compute the largest transverse displacement and von Mises stress.

### Solution Strategy

- Can you compute an analytical, approximate solution (max. displacement, max. normal stress?
- What other response parameters do you know that can be used as a check?
  - Displacements
  - Stresses
  - Reactions

### Solution Steps

- Select consistent units (lbm, in, s, lb)
- Try a single element solution
  - Analyze and go through the checks
- Refine the model
  - Analyze and go through the checks
- Refine the model even more
  - Analyze and go through the checks
- Document the results and check for convergence

### Tutorial #2

• Model the building shown below using only beam elements.

### Tutorial #3

• Model the building shown below using beam and shell elements.