

FLOOR RESPONSE SPECTRA OF THREE-DEMENTIONALLY BASE-ISOLATED NUCLEAR POWER PLANT CONSIDERING SOIL-STRUCTURE INTERACTION EFFCET

LT. Ding¹, F. Wang², T. Wang³

¹ Master candidate, Key Laboratory of Earthquake Engineering and Engineering Vibration, Institute of Engineering Mechanics, CEA, Yanjiao, Sanhe, Hebei, 065201, China

² Master candidate, Key Laboratory of Earthquake Engineering and Engineering Vibration, Institute of Engineering Mechanics, CEA, Yanjiao, Sanhe, Hebei, 065201, China

³ Researcher, Key Laboratory of Earthquake Engineering and Engineering Vibration, Institute of Engineering Mechanics, CEA, Yanjiao, Sanhe, Hebei, 065201, China (wangtao2004@gmail.com)

ABSTRACT

The seismic mitigation effect of base isolation technique applied on nuclear power plants (NPP) shall be examined by considering the soil structure interaction (SSI) because current NPP seismic design codes allow foundation soil much softer than bed rocks. In this study, four multi-degree-of-freedom (MDOF) mass-spring models are built with different structure patterns: the prototype with fixed base (P), the prototype considering SSI (P_SSI), the three-dimensional isolation model with fixed base (3D), and the isolation model with SSI effect (3D_SSI). A set of time history analyses are then conducted on each model, The floor response spectra (FRS) are thus developed, which indicate that the base isolation technique effectively mitigate the seismic response, while SSI, particularly for those structures built on moderate hard soils, might amplify the structure responses.

INTRODUCTION

Fukushima Daiichi nuclear power plant accident caused by 3.11 East Japan earthquake has a great impact on Japan and even all the world. On April 12, 2011, Japanese authorities had raised the accident level to the most serious level 7, same as the Chernobyl nuclear accident. The seismic behavior of nuclear power plant had caught a lot more concerns from researchers since then. Base isolation technique has already been proved very effective to mitigate seismic responses of structures. The structure of nuclear power plant is so rigid that makes it very suitable for the base isolation technique^[1]. Several studies have experimentally and numerically examined the feasibility and effectiveness of base isolation applied to nuclear power plants^[2-5]. Floor response spectra (FRS) of base isolated nuclear power plant shift its dominated frequency to much lower frequency band compared with the one without isolation, thus significantly mitigating the responses of inside facilities^[6]. Soil structure interaction (SSI) is another concern during the seismic design of nuclear power plant^[7]. Specific issues, such as wave incoherence, passage effect, reflection and so on, have to be considered comprehensively because of the large foundation raft. However, very limited studies have been found to answer the influence of SSI on the base isolated nuclear power plants. This becomes very interesting since concerns have been raised to consider hard soil site rather than rock only. As a subsequent study of a three-dimensionally base isolated NPP technique^[6], this study examines the SSI effect using four MDOF mass-spring models. Time history analyses are conducted on each model using forty ground motions. All results are presented in the form of FRS which provide insight into the seismic mitigation effect of base isolation technique.

MDOF MASS-SPRING MODELING

Safety Shell and Internal Structure

An open source code, OpenSEES, developed by University of California at Berkeley, was employed to construct the MDOF mass-spring model. As shown in Figure.1, the model consists of the safety shell, internal structure and a common raft foundation under them. The weights are respectively 18.3 million tons, 16.1 million tons and 26.7 million tons. The internal structure is 37.5m high, The NPP structure is 60.18m high and has a diameter of 38.8m. The thickness of the shell is 900mm.

The simplified model is shown in Figure.2. The safety shell and internal structures are simplified as two separate MDOF mass-spring models based on a common raft foundation. The base isolation layer is inserted under the raft. Three springs represent the isolators, two in vertical and one in horizontal. The distance between the two vertical springs is calculated according to rocking stiffness. Below the base isolation layer is the foundation soil which is represented by distributed springs and dashpots. Parameters of soil springs and dashpots are described in the following section.

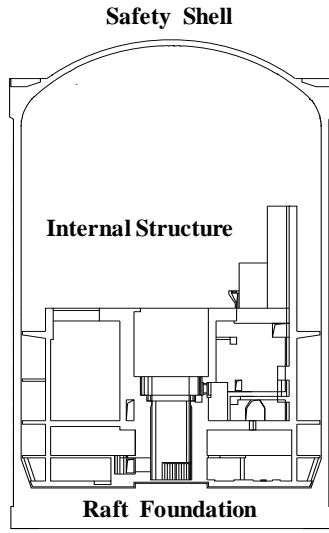


Figure 1. NPP prototype structure

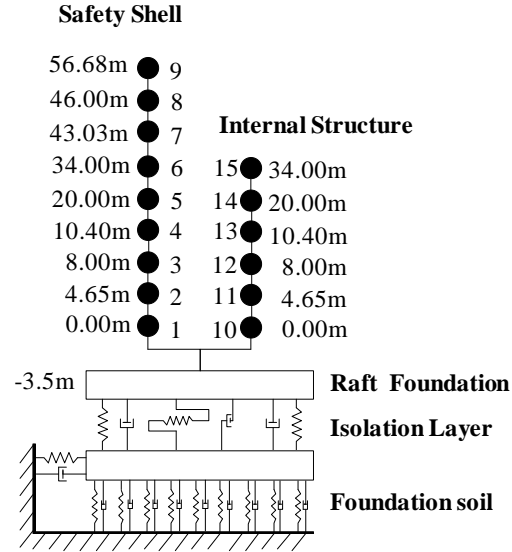


Figure 2. MDOF mass-spring model

Soil Spring-Damper Element

(1) Vertical foundation soil spring element

Winkler soil model^[8] is adopted in this study, as formulated in Eq.1:

$$p_i = ks_i \quad (1)$$

where, p_i denotes the intensity of pressure at one point on the soil surface; k denotes coefficient of subgrade reaction; and s_i is the vertical deformation at the same point on the soil surface.

Based on elastic half-space theory^[9], the vertical deformation at one point s_i is defined as follow:

$$s_i = \frac{p_i b(1-\nu^2)}{E} I \quad (2)$$

From Equations (1) and (2), k is thus derived as:

$$k = \frac{E}{b(1-\nu^2)I} \quad (3)$$

where, E is the modulus of elasticity of the soil; b is the width of the foundation; ν denotes the Poisson's ratio of the soil; and I is a coefficient relate to the shape and stiffness of foundation, for rectangle rigid foundation, $I = 0.88$.

(2) Horizontal foundation soil spring element

Based on elastic half-space theory, Horizontal equivalent coefficient of subgrade reaction of the rectangle rigid foundation is shown as follow:

$$K = 4(1 + \nu)G\beta\sqrt{BL} \quad (4)$$

where, G is the shear modulus of the soil; B equals to $1/2$ width of the foundation; L represents the length of the foundation; ν is the Poisson's ratio of the soil; and β is a coefficient relate to the shape and stiffness of foundation, for rectangle rigid foundation, $\beta = 0.97$.

(3) Vertical and horizontal dashpot element

Damping ratio of the soil can be obtained by soil's dynamic properties test. It is commonly small (about 5%) under the most common dynamic shear strain values. In this study, the damping ratio of soil is specified as 3%.

Classification of Foundation Soil

In this paper, three different foundation soils are studied: rock, hard soil and moderate hard soil, respectively with the shear wave speed of 1100m/s, 700m/s and 400m/s. Density of soil is 2500kg/m³, dynamic shear modulus of the soil is calculated from the following equation:

$$G = \rho c_s^2 \quad (5)$$

where, ρ denotes the density of soil; and c_s is the shear wave speed of soil.

MODAL ANALYSIS

For comparison, four models are built, i.e., the prototype with fixed base (P), the prototype considering SSI (P_SSI), the three-dimensional isolation model with fixed base (3D), and the isolation model with SSI effect (3D_SSI). The first five periods of these models are listed in Table 1. It is observed that isolated models have a much longer first period than non-isolated models no matter what foundation soil type is considered. For base fixed case, horizontal period (modal order 1) of P is 0.197s, while with three-dimensional base isolation, it is 2.515s, about 12.8 times longer. The vertical period (modal order 3) of 3D and P is respectively 0.117s and 0.06s, about 3.8 times longer. Soil springs elongated the period. And it seems the SSI effect is more pronounced for the model without isolation. The period is 0.341 s for the model sit on the moderate hard soil, almost twice of the fixed base model.

Table 1: First 5 periods of models: P and P_SSI (Unit: sec)

Model	Soil type	Modal order				
		1	2	3	4	5
		Safety shell horizontal mode	Internal structure horizontal mode	Safety shell vertical mode		
P	Base Fixed	0.1971	0.0999	0.0667	0.0597	0.0417
P_SSI	Rock	0.2181	0.1111	0.0830	0.0613	0.0553
	Hard soil	0.2490	0.1205	0.1149	0.0684	0.0589
	Moderate hard soil	0.3414	0.1919	0.1386	0.0990	0.0601

Table 2: First 5 periods of models: 3D and 3D_SSI (Unit: sec)

Model	Soil type	Modal order				
		1	2	3	4	5
		Overall horizontal mode	Overall rocking mode	Overall vertical mode		
3D	Base Fixed	2.5151	0.2853	0.2525	0.1167	0.0604
3D_SSI	Rock	2.5189	0.2980	0.2569	0.1179	0.0604
	Hard soil	2.5215	0.3217	0.2730	0.1192	0.0618
	Moderate hard soil	2.5342	0.3764	0.2956	0.1216	0.0987

FRS ANALYSIS

In this section, time history analyses are conducted on each model using forty ground motions^[10], including 20 near fault ground motions and 20 ground motions with exceeding probability of 2% in 50 years recorded in Los Angeles. Pseudo acceleration response spectra of these ground motions are shown in Figure.3, where the bold line represents the statistic value with 97.7% exceedance probability. The amplitudes of the 40 ground motions are scaled as 300gal. Each motion is input in both horizontal and vertical directions, and the vertical amplitude is 2/3 of the horizontal one.

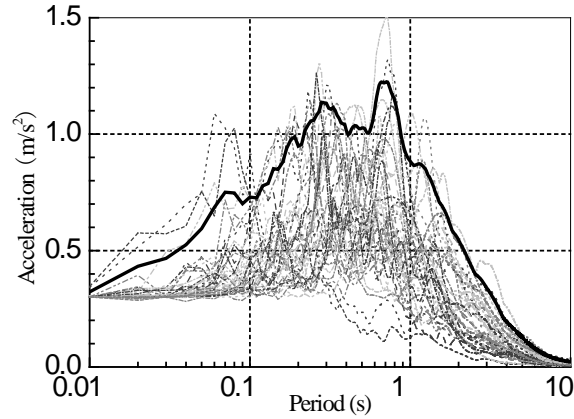


Figure 3. Pseudo acceleration response spectrum

Horizontal FRS Analyses

Horizontal FRS with 97.7% exceedance probability of the safety shell top node are shown in Figure.4-Figure.6, respectively for the rock, hard soil and moderate hard soil. Figure 7 to Figure 9 give the horizontal FRS of the top of internal structure. It is observed from these figures that:

- 1) FRS of base isolated NPP models have larger dominant periods than non-isolated models. Acceleration responses of both safety shell and internal structure are reduced significantly.
- 2) Considering SSI effect, Horizontal FRS amplitude increases. SSI effect has larger influence on the model without isolation. The softer the soil, the larger influence of SSI is observed. The peak floor acceleration is amplified almost twice for the one located at moderate hard soil site. P_SSI safety shell with rock, hard soil and moderate hard soil type, FRS amplitudes are 56.9 m/s^2 , 77.5 m/s^2 and 76.7 m/s^2 respectively. Compared to P, amplitude increase by 121%, 160% and 163%. While for base isolated models, amplitude increase is very limited. Similar phenomenon is observed as well for the internal structure. And the influence of SSI is much more pronounced for the model at moderate hard foundation. The reason might be the period after shifting happens to match the dominated frequency of excitations. P_SSI amplitude increases about 3 times than P model. The SSI effect cannot be ignored for the models without isolation, while the isolated models seem not sensitive to the SSI effect.

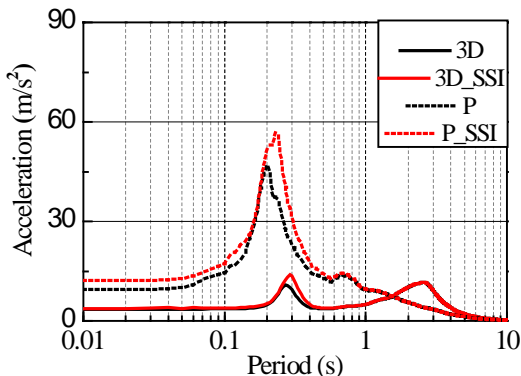


Figure 4. Safety shell horizontal FRS (rock)

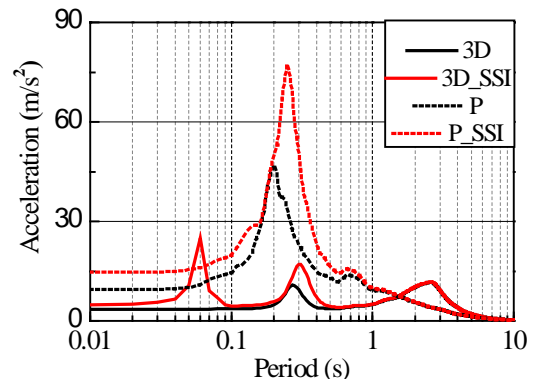


Figure 5. Safety shell horizontal FRS (hard soil)

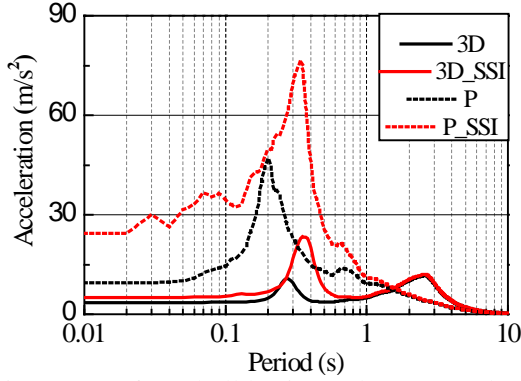


Figure 6. Safety shell horizontal FRS (moderate hard soil)

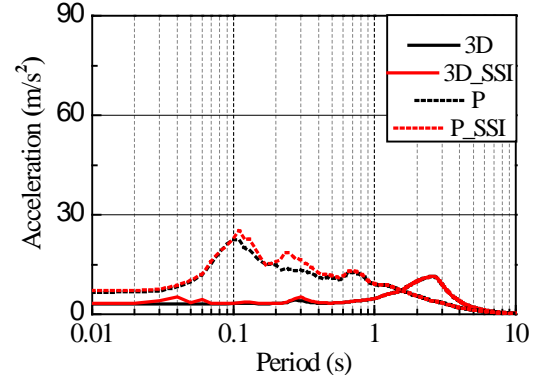


Figure 7. Internal structure horizontal FRS (rock)

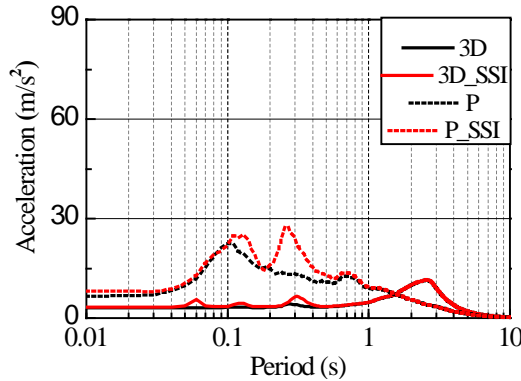


Figure 8. Internal structure horizontal FRS (hard soil)

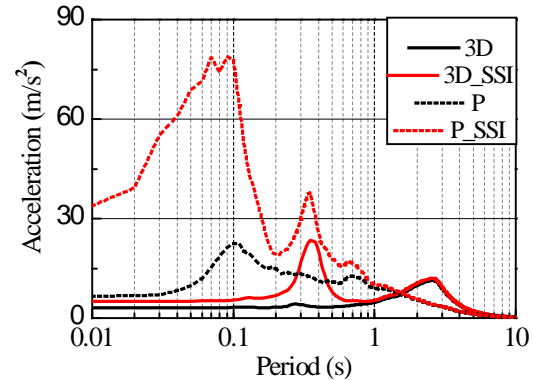


Figure 9. Internal structure horizontal FRS (moderate hard soil)

Vertical FRS Analyses

Nuclear facilities are more sensitive to vertical floor responses than horizontal ones. And this is the purpose to develop the three-dimensional isolation technique. In this study, the vertical FRS is closely examined considering different soil conditions. Shown in Figure 10 to 12 is the vertical FRS with 97.7% exceedance probability of the top of internal structure. It is observed that:

- 1) FRS of base isolated NPP models have longer predominant periods than non-isolated models. Acceleration response of the internal structure is mitigated significantly.
- 2) In the vertical direction, FRS amplitude of base isolated NPP increases compared to non-isolated structure. However, predominant periods are elongated and move away from the vibration frequency ranging from 10 - 20Hz, deemed as the dominated frequencies of facilities and pipes.
- 3) Considering SSI effect, vertical FRS amplitude increases. Softer site has larger influence on structure responses than harder sites. For 3D_SSI with moderate hard soil, FRS amplitude is 40.6 m/s^2 , while for 3D, it is 22.7 m/s^2 , while based on the rock foundation, 3D_SSI FRS amplitude is 24.8 m/s^2 , almost the same as the one not considering SSI.

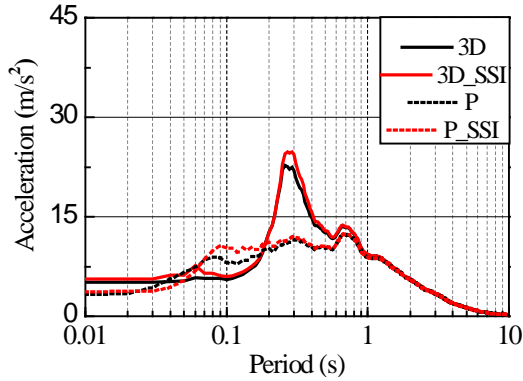


Figure 10. Internal structure vertical FRS (rock)

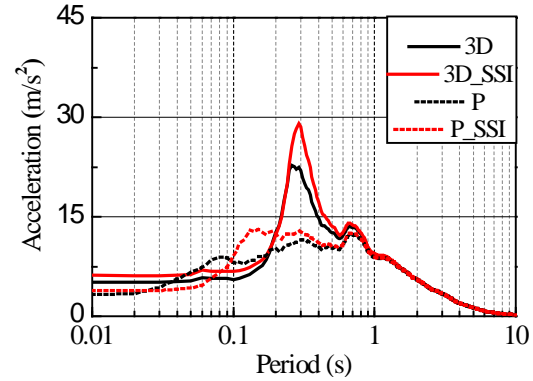


Figure 11. Internal structure vertical FRS (hard soil)

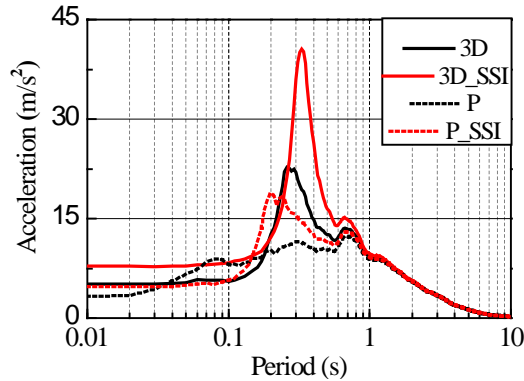


Figure 12. Internal structure vertical FRS (moderate hard soil)

CONCLUSION

1. Base isolation technique can effectively mitigate the horizontal response of superstructure, particularly for those located at hard soil foundation or rocks. Vertically, the mitigation effect is not pronounced because the vertical frequency is about 3Hz. However, the seismic effect is reduced within the frequency range dominating facility vibration.

2. SSI has significant influence on the structure located on moderate hard foundation. For the site with moderate hard soil, the response increases about 2 times. The influence of SSI on hard foundation, although smaller, cannot be ignored. In the vertical direction, it almost eliminates the base isolation effect. Vertical isolation on hard soil foundation shall be carefully implemented, and this is a future concern.

ACKNOWLEDGEMENT

This project was supported by National Department Public Benefit Research Foundation of China (201208013), Central Public-Interest Scientific Institution Basal Research Fund of China (2010B04), and National Natural Science Foundation of China (51008287). Any opinions, findings, and conclusion or recommendation expressed in this paper are those of the authors and do not necessarily reflect the views of the sponsors.

REFERENCES

- [1] Malushte S, Whittaker A. (2005). "Survey of past base isolation applications in nuclear power plants and challenges to industry acceptance", *18th SMiRT*, K10-7, Beijing, China.

- [2] Coladant Ch. (1991). "Seismic isolation of nuclear power plants - EDF's philosophy", *Nuclear Engineering and Design*, 127: 243-251.
- [3] Martelli A, Forni M, Bergamo G, Bonacina G, Cesari. (1995). "Proposal for design guidelines for isolated nuclear facilities", *13th SMiRT*. Italy.
- [4] Sun L, Gu FY. (1999). "A primary study of base isolation technology in NPP", *15th SMiRT*, K17-2, Korea.
- [5] XIE Lili, ZHAI Changhai. (2012). "A prospective study on applicability of base isolation in nuclear power plants", *Journal of Earthquake Engineering and Engineering Vibration*, CSTAM & IEM, China, 32(1):1-10.
- [6] Wang F, Wang T, Ding LT. (2012). "Numerical and experimental study on seismic behavior of base-isolated nuclear power plant", *15th WCEE*, Lisbon, Portugal.
- [7] Jim Xu, Sujit Samaddar. (2009). "Case Study: Effect of Soil-Structure Interaction and Ground Motion Incoherency on Nuclear Power Plant Structures", *PVP2009*, Prague, Czech Republic.
- [8] Zhou Jingxing, Li Guangxin, Yu Shimin, Wang Honghongjin. (2007). *Foundation Engineering*[M]. Tsinghua University Press, Beijing, China.
- [9] Wolf J P. (1994). *Foundation vibration analysis using simple physical models*[M]. Englewood Cliffs(NJ); Prentice-Hall Inc.
- [10] Somerville P. (1999). "Development of ground motion time histories for phase 2 of the FEMA/SAC steel project", *SAC Background Document*, Report No. SAC/BD-99-03.