

DESIGN CRITERIA:

BUILDING CODE - 2021 UTAH STATE BUILDING CODE (BASED ON IBC 2021)

1. RISK CATEGORY & IMPORTANCE FACTORS:

A. RISK CATEGORY.....	II
B. WIND FACTOR.....	1.0
C. SNOW FACTOR.....	1.0
D. SEISMIC FACTOR.....	1.0

2. DESIGN DEAD LOADS:

A. ROOF.....	22 PSF
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3. DESIGN LIVE LOADS\*:

A. ROOF.....	20 PSF
* LIVE LOADS ARE REDUCIBLE IN ACCORDANCE WITH THE BUILDING CODE.	

4. WIND LOADS:

A. ULTIMATE WIND SPEED.....	103 MPH
B. DIRECTIONALITY FACTOR (Kd).....	0.85
C. EXPOSURE CATEGORY.....	C
D. ENCLOSURE CLASSIFICATION.....	ENCLOSED BUILDING
E. GUST EFFECT FACTOR (G).....	0.85
F. COMPONENT AND CLADDING LOADS (100 SQ. FT., ZONES ARE PER ASCE-7)	

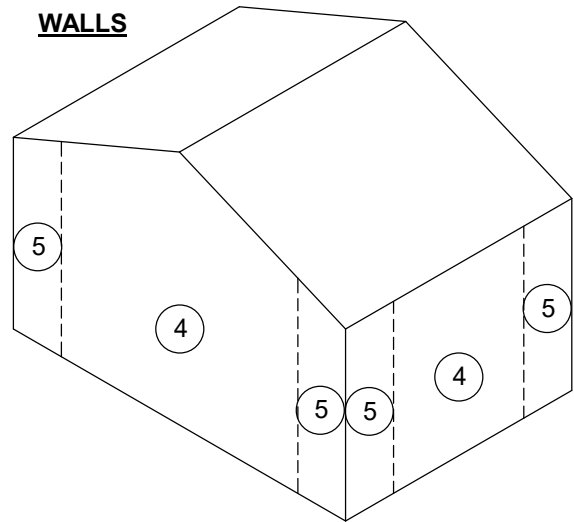
ROOFS	10 FT <sup>2</sup>	100 FT <sup>2</sup>
NEGATIVE ZONE 1	-38.30 PSF	-29.90 PSF
NEGATIVE ZONE 2	-22.00 PSF	-22.00 PSF
NEGATIVE ZONE 3	-50.50 PSF	-39.70 PSF
NEGATIVE ZONE 4	-50.50 PSF	-39.70 PSF
POSITIVE ZONES 1 & 1'	16.00 PSF	16.00 PSF
POSITIVE ZONES 2 & 3	22.00 PSF	18.80 PSF

OVERHANG ZONE 1&1'	-34.60 PSF	-32.60 PSF
OVERHANG ZONE 2	-46.80 PSF	-32.60 PSF
OVERHANG ZONE 3	-46.80 PSF	-32.60 PSF

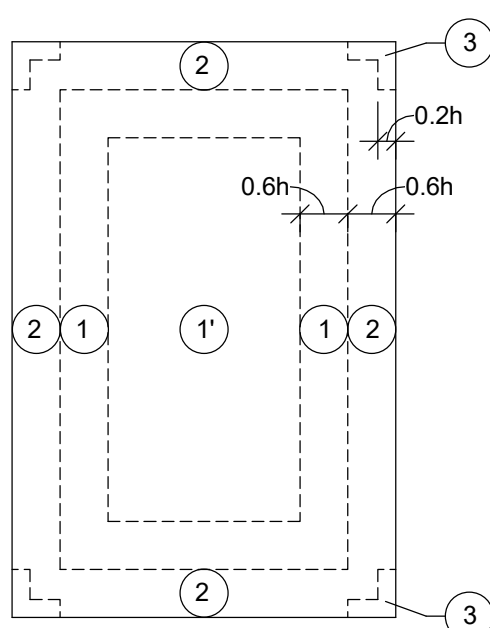
WALLS		
NEGATIVE ZONE 4	-23.80 PSF	-19.80 PSF
NEGATIVE ZONE 5	-29.30 PSF	-22.90 PSF
POSITIVE ZONES 4 & 5	22.00 PSF	18.80 PSF

REFERENCE ASCE 7 FOR EFFECTIVE WIND AREAS NOT LISTED. OTHERWISE, USE THE SMALLEST APPLICABLE AREA.

WALLS



ROOFS



h = 18'-0"

C & C WIND ZONES  
GABLES  $\theta \leq 7^\circ$ , h  $\leq 60'-0"$   
MONOSLOPES  $\leq 3^\circ$ , h  $\leq 60'-0"$

5. EARTHQUAKE LOADS:

A. SITE CLASS.....	D
B. Ss =.....	1.359
C. S1 =.....	0.505
D. SDS =.....	1.087
E. SD1 =.....	0.604
F. SEISMIC DESIGN CATEGORY.....	D
G. BASIC SEISMIC FORCE RESISTING SYSTEM =.....	LIGHT FRAME WOOD WALLS WITH STRUCTURAL WOOD SHEAR PANELS.
H. RESPONSE MODIFICATION COEFFICIENT, R =.....	6.5
I. OVER-STRENGTH FACTOR (Do).....	2.5
J. DEFLECTION AMPLIFICATION FACTOR (Cd).....	4
K. SEISMIC RESPONSE COEFFICIENT (Cs) =.....	0.167
L. LONG PERIOD TRANSITION PERIOD (TL).....	8
M. ANALYSIS PROCEDURE.....	EQUIVALENT LATERAL FORCE
N. DESIGN BASE SHEAR =.....	20.0K

6. SNOW LOADS:

A. GROUND SNOW LOAD.....	38.0 PSF
B. FLAT ROOF SNOW LOAD (Pf).....	26.6 PSF
C. THERMAL FACTOR (Ct).....	1.0
D. SNOW EXPOSURE FACTOR (Ce).....	1.0
E. RAIN ON SNOW SURCHARGE.....	0.0 PSF
F. UNIFORM ROOF SNOW LOAD.....	26.6 PSF
G. LEEWARD SNOW LOAD.....	35.8 PSF

7. RAIN LOADS:

A. RAINFALL INTENSITY (i).....	1.71 IN/HR
B. STATIC HEAD (ds).....	1.0 INCHES
C. HYDRAULIC HEAD (dh).....	1.62 INCHES
D. DESIGN RAIN LOAD.....	19.0 PSF

SPECIAL INSPECTIONS:

- SPECIAL INSPECTION AND A FINAL REPORT IN ACCORDANCE WITH IBC SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.
- THE OWNER WILL EMPLOY THE SERVICES OF ONE OR MORE SPECIAL INSPECTORS TO PROVIDE SPECIAL INSPECTIONS DURING CONSTRUCTION ACCORDING TO THE SCHEDULE OF SPECIAL INSPECTIONS.
- THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
- DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
  - THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE OR WAIVE ANY OF THE REQUIREMENTS OF THE DOCUMENTS.
  - THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE PROFESSIONAL OF RECORD, AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. THEN, IF UNCORRECTED, SUBMIT A COMPLETE LIST OF ALL OUTSTANDING DISCREPANCIES ON A WEEKLY BASIS TO THE OWNER, THE BUILDING OFFICIAL, AND THE PROFESSIONAL OF RECORD UNTIL ALL CORRECTIONS HAVE BEEN COMPLETED.
  - THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.
- DUTIES AND RESPONSIBILITIES OF THE CONTRACTOR:
  - NOTIFY THE SPECIAL INSPECTOR THAT SPECIAL INSPECTIONS ARE NEEDED.
  - COORDINATE THE SCHEDULING AND TIMELY NOTIFICATION OF THE SPECIFIC INDIVIDUALS NEEDED FOR THE SPECIAL INSPECTION.
  - PROVIDE DIRECT ACCESS TO THE APPROVED PLANS AND SPECIFICATIONS FOR THE PROJECT.
  - SUBMIT FABRICATORS CERTIFICATES OF COMPLIANCE, WELDER'S CERTIFICATES, AND OTHER REQUIRED DOCUMENTATION FOR REVIEW BY THE SPECIAL INSPECTOR.
  - PROVIDE SAFE ACCESS TO THE WORK TO BE INSPECTED AND DELIVER SAMPLES FOR TESTING WHEN NEEDED.
- WHERE SPECIAL INSPECTION REQUIREMENTS DUPLICATE THE REQUIREMENTS OF SPECIFIED QUALITY ASSURANCE TESTING, DUPLICATE INSPECTIONS SHALL NOT BE REQUIRED.

MISCELLANEOUS:

- STRUCTURAL DRAWINGS SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING PERTINENT ASPECTS OF ALL DISCIPLINES INTO THEIR SHOP DRAWINGS AND WORK, AND SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES OR OMISSIONS.
- NO OPENINGS OR MODIFICATIONS SHALL BE MADE IN OR TO ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.
- NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.
- OPENINGS 1'-4" OR LESS ON A SIDE ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SUCH OPENINGS.
- THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
- THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL THE TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
- DO NOT SCALE THESE DRAWINGS: USE DIMENSIONS. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS, SEE ARCHITECTURAL DRAWINGS.
- CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
- THE CONTRACTOR SHALL INFORM THE PROFESSIONAL OF RECORD, IN WRITING, OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE PROFESSIONAL OF RECORD. REVIEW OF SHOP DRAWINGS, OR NOT DATA, ETC. UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE PROFESSIONAL OF RECORD OF SUCH DEVIATION AT THE TIME OF SUBMISSION AND THE ARCHITECT HAS GIVEN THE WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
- WHERE A SECTION/DETAIL IS CUT ON THE PLAN, IT IS ASSUMED/UNDERSTOOD TO BE REPRESENTATIVE OF ALL LIKE OR SIMILAR CONDITIONS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS INTO THEIR SHOP DRAWINGS AND WORK.
- AT ALL TIMES THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF PERSONS AND PROPERTY. THE ARCHITECT'S OR ENGINEER'S PRESENCE AT THE JOB SITE OR REVIEW OF WORK DOES NOT IMPLY CONFIRMATION OF THE ADEQUACY OF THE CONTRACTOR'S MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH OSHA REGULATIONS.
- CONSULT ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR LOCATION, SIZE AND EXTENT OF CHASES, INSERTS, RECESSES, RIDGES, FINISHES, DEPRESSIONS, ETC., NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCIES.
- THE CONTRACTOR SHALL VERIFY ALL FLOOR AND ROOF MOUNTED MECHANICAL EQUIPMENT WEIGHTS AS WELL AS FLOOR AND/OR ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- THE CONTRACTOR SHALL NOTIFY, IN WRITING, THE STRUCTURAL ENGINEER OF RECORD OF CONDITIONS ENCOUNTERED IN THE FIELD WHICH ARE CONTRADICTORY TO THOSE SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS.
- STRUCTURAL CONTRACT DOCUMENTS SHALL NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR ANY MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR OR SUBCONTRACTOR.
- REFERENCE TO STANDARD SPECIFICATIONS OR ANY TECHNICAL SOCIETY, ORGANIZATION OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES SHALL MEAN THE LATEST STANDARD, CODE SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AND PUBLISHED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR FLOOR ELEVATIONS, SLOPE, AND LOCATION OF DEPRESSED FLOOR AREAS. THE CONTRACTOR SHALL COMPARE STRUCTURAL SECTIONS WITH THE ARCHITECTURAL SECTIONS AND REPORT ANY DISCREPANCY TO THE ARCHITECT PRIOR TO FABRICATING OR INSTALLING STRUCTURAL MEMBERS.
- PRINCIPAL OPENINGS THROUGH THE FRAMING ARE SHOWN ON THESE DRAWINGS. THE GENERAL CONTRACTOR SHALL EXAMINE THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR THE REQUIRED OPENINGS AND HE SHALL PROVIDE FOR ALL OPENINGS WHETHER SHOWN ON THE DRAWINGS OR NOT. HE SHALL VERIFY SIZE AND LOCATION OF ALL OPENINGS WITH THE MECHANICAL CONTRACTOR, ANY DEVIATION FROM THE OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR APPROVAL.

SUBMITTALS:

- CONTRACTOR SHALL SUBMIT A SCHEDULE OF SHOP DRAWING SUBMITTAL DATES AT LEAST 30 DAYS PRIOR TO FIRST SUBMITTAL. FAILURE TO SUBMIT DRAWINGS ON DESIGNATED DATES MAY IMPACT REVIEW SCHEDULE.
- ANY MATERIALS OR PRODUCTS SUBMITTED FOR APPROVAL THAT ARE DIFFERENT FROM THE MATERIAL OR PRODUCTS SPECIFIED IN THE STRUCTURAL CONTRACT DOCUMENTS WILL BE CONSIDERED ONLY IF THE FOLLOWING CRITERIA ARE SATISFIED:
  - A COST SAVINGS TO THE OWNER IS DOCUMENTED AND SUBMITTED WITH THE REQUEST.
  - THE MATERIAL OR PRODUCT HAS BEEN APPROVED BY THE ICC-ES, AND THE ICC-ES REPORT IS SUBMITTED WITH THE REQUEST. SUBMITTALS NOT SATISFYING THE ABOVE CRITERIA WILL NOT BE CONSIDERED.
  - A SIGNED AND SEALED LETTER OR CALCULATIONS BY A LICENSED ENGINEER IN THE PROJECT STATE SHOWING THE SUBSTITUTION IS OF EQUAL OR GREATER STRENGTH THAN WHAT IS SPECIFIED OR IS ADEQUATE FOR THE IMPOSED LOADS.
- REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER OF RECORD DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE STRUCTURAL ENGINEER OF RECORD. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS AND DIMENSIONS SPECIFIED IN METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION. SEE SPECIFIC PROVISIONS IN THE CONTRACT DOCUMENT DEALING WITH THE APPROPRIATE DESIGN RESPONSIBILITIES OF CONTRACTORS, SUBCONTRACTORS AND CONTRACT SUPPLIERS.
- THE USE OF REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES HIS ACCEPTANCE OF ALL INFORMATION SHOWN HEREIN AS CORRECT AND OBLIGATES HIM TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING FROM ANY ERRORS THAT MAY OCCUR HEREIN.

DEFERRED SUBMITTALS:

- DEFERRED SUBMITTALS ARE DEFINED AS THE FOLLOWING PORTIONS OF THE DESIGN THAT ARE NOT SUBMITTED AT THE TIME OF APPLICATION AND THAT ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD:
  - PRE-MANUFACTURED WOOD TRUSSES
  - STEEL STAIRS & LADDERS
  - HANDRAILS & GUARDS
- THE DEFERRED SUBMITTALS SHALL BE APPROVED BY THE PROJECT ARCHITECT AND/OR ENGINEER OF RECORD. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN AUTHORIZED BY THE BUILDING OFFICIAL.

FOUNDATIONS:

- SPREAD FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUSTAINING A NET ALLOWABLE BEARING PRESSURE OF 3.0 KSF FOR INDIVIDUAL COLUMN FOOTINGS AND 3.0 KSF FOR CONTINUOUS WALL FOOTINGS UNDER FULL SERVICE LIVE AND DEAD LOAD.
- THE FOOTINGS HAVE BEEN POSITIONED AT THE ESTIMATED ELEVATION WHICH WILL PROVIDE SUITABLE BEARING. HOWEVER, IF ADEQUATE BEARING CAPACITY IS NON-EXISTENT AT THESE ESTIMATED ELEVATIONS, THE FOOTING SHALL BE LOWERED TO AN ELEVATION WHERE THE PRESCRIBED SAFE BEARING CAPACITY EXISTS.
- FOOTINGS MAY BE CAST INTO AN EARTH-FORMED TRENCH IF SOIL CONDITIONS PERMIT.
- EXCAVATION FOR FOOTINGS SHALL BE CUT TO ACCURATE SIZES AND DIMENSIONS, AS SHOWN ON PLANS. ALL SOIL BELOW SLABS AND FOOTINGS SHALL BE PROPERLY COMPACTED AND SUBGRADE BROUGHT TO A REASONABLE TRUE AND LEVEL PLANE BEFORE PLACING CONCRETE.
- IN THE AREA OF THE BUILDING, EXISTING ORGANIC MATERIAL, UNSUITABLE SOIL, ABANDONED FOOTINGS AND ANY OTHER EXISTING UNSUITABLE MATERIALS AS IDENTIFIED BY THE GEOTECHNICAL INVESTIGATION REPORT SHALL BE REMOVED. ANY FILL MATERIAL REQUIRED AT THE SITE SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN THE GEOTECHNICAL INVESTIGATION REPORT AND APPROVED BY A SOILS ENGINEER. ROCKS OF A DIAMETER GREATER THAN THAT SPECIFIED SHALL BE EXCLUDED FROM STRUCTURAL FILL LIFTS. FILL MATERIAL SHALL BE PLACED IN LOOSE LIFTS ACCORDING TO THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS AND COMPACTED TO A SPECIFIED MAXIMUM DRY DENSITY AS DETERMINED BY THE MODIFIED COMPACTION TEST (ASTM D1557). ADEQUATE FIELD DENSITY AND MOISTURE CONTENT TESTS SHALL BE PERFORMED TO ENSURE COMPLIANCE.
- FOOTING CONCRETE SHALL BE CAST ON THE SAME DAY THE EXCAVATION IS APPROVED. IF THE BEARING SURFACE IS ALLOWED TO BECOME DISTURBED IN ANY WAY, IT SHALL BE REWORKED TO THE SATISFACTION OF THE TESTING ENGINEER PRIOR TO CASTING THE CONCRETE.
- ALL BEARING MATERIAL SHALL BE INSPECTED BY THE INDEPENDENT TESTING AGENCY PRIOR TO CONCRETE PLACEMENT. THE INDEPENDENT TESTING AGENCY SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED.
- BOTTOM OF EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 2'-6" BELOW FINAL GRADE FOR FROST PROTECTION.
- WHEN UNSATISFACTORY OR UNCONTROLLED FILL IS ENCOUNTERED, REMOVAL AND REPLACEMENT WILL BE PAID ON THE BASIS OF UNIT PRICES SET FORTH IN THE CONTRACT.
- DRAINAGE FILL SHALL BE AN EVENLY GRADED MIXTURE OF NATURAL OR CRUSHED STONE, CONFORMING TO THE REQUIREMENTS OF ASTM STANDARD C33, AND HAVING A GRADATION AS FOLLOWS:

100 % PASSING.....	A 3/4" SIEVE
10-30 % PASSING.....	A 1/2" SIEVE
0-10 % PASSING.....	A 3/8" SIEVE
0-5 % PASSING.....	A #4 SIEVE
- ANY FILL WITHIN 10'-0" OF THE BUILDING LIMIT SHALL CONFORM TO THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER FOR PREPARATION.
- BACKFILL AROUND AND OVER FOUNDATION ELEMENTS SHALL BE OF SUITABLE MATERIAL, INSPECTED AND PRE-APPROVED BY THE TESTING ENGINEER.
- BACKFILL AGAINST WALLS SHALL BE PLACED IN 8 INCH LIFTS AND SHALL BE DEPOSITED EVENLY AGAINST EACH SIDE OF THE WALL UNTIL THE LOWER FINAL GRADE IS REACHED. BACKFILL SHALL NOT BE PLACED AGAINST WALLS DEPENDENT UPON TOP AND BOTTOM SLAB/FOUNDATION FOR SUPPORT UNTIL SUCH SLABS HAVE ATTAINED MINIMUM DESIGN COMPRESSIVE STRENGTH. WALLS WITH SLAB-ON-GROUND AT THE TOP OF THE WALL SHALL BE SAFELY SHORED AND BRACED DURING BACKFILLING.
- MAXIMUM SLOPE OF EXCAVATIONS SHALL BE IDENTIFIED IN THE GEOTECHNICAL INVESTIGATION REPORT AND ADHERED TO. PROVIDE SHORING AND PROTECTION FOR EXCAVATION BANKS AS NECESSARY TO PRESERVE SAFETY AND PREVENT CAVING.
- ALL BEARING STRATA SHALL BE ADEQUATELY DRAINED BEFORE FOUNDATION CONCRETE IS PLACED.
- COLUMN FOOTINGS AND WALL FOOTINGS SHALL BE POURED MONOLITHIC WITH TOPS OF ADJACENT FOOTINGS AT THE SAME ELEVATION.
- THERE SHALL BE NO HORIZONTAL OR VERTICAL CONSTRUCTION JOINTS IN ANY FOOTING WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
- CONCRETE CAST ON SLOPING SURFACES SHALL BEGIN AT THE LOWEST ELEVATION AND CONTINUE MONOLITHICALLY TOWARD THE HIGHER ELEVATION UNTIL THE INTENDED POUR IS COMPLETED.
- FOUNDATION DESIGN IS BASED ON THE SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING EVALUATION DATED SEPTEMBER 26, 2024 AS PREPARED BY APPLIED GEOTECHNICAL ENGINEERING CONSULTANTS, INC. PROJECT NO. 1240637.

CONCRETE REINFORCEMENT LAP LENGTH SCHEDULE						
BAR SIZE	f <sub>c</sub> = 3,000 PSI		f <sub>c</sub> = 4,000 PSI		f <sub>c</sub> = 4,500 PSI	
	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER
#3	28"	22"	25"	19"	23"	18"
#4	38"	29"	33"	25"	31"	24"
#5	47"	36"	41"	31"	38"	30"
#6	56"	43"	49"	37"	46"	35"
#7	81"	63"	71"	54"	67"	51"
#8	93"	72"	81"	62"	76"	59"

NOTES:

- WHERE THE CLEAR SPACING BETWEEN BARS BEING SPICED IS LESS THAN (2) BAR DIAMETERS, INCREASE THE LAP LENGTH BY 50%.
- WHERE THE BAR COVER IS LESS THAN OR EQUAL TO THE BAR DIAMETER, INCREASE THE LAP LENGTH BY 50%.
- TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THE BARS.
- LAP SPICE LENGTHS ARE PROVIDED FOR NORMAL WEIGHT CONCRETE. WHERE LIGHTWEIGHT CONCRETE IS USED, INCREASE LAP SPICE LENGTHS BY 30%.
- SPICES OF HORIZONTAL REINFORCEMENT IN WALLS SHALL BE STAGGERED.
- SPICES OF HORIZONTAL REINFORCEMENT IN WALLS COMBINED TWO MATTS OF REINFORCEMENT SHALL NOT OCCUR IN THE SAME LOCATION.

CONCRETE:

- CODE: AMERICAN CONCRETE INSTITUTE (ACI) 318 (LATEST ADDITION)
- CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH IN ACCORDANCE WITH THE FOLLOWING:

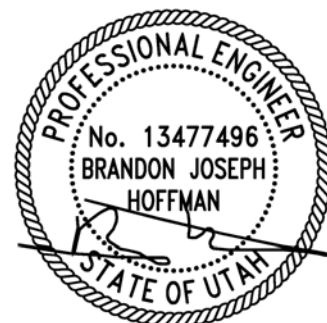
FOOTINGS & SLABS ON GRADE.....	3000 PSI
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- ALL CONCRETE SHALL HAVE A DENSITY OF 145 PCF UNLESS NOTED OTHERWISE.
- CONCRETE SHALL BE ENTRAINED AS REQUIRED TO CONFORM TO DURABILITY REQUIREMENTS OF ACI 318.
- CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS FOR ALL UNIQUE CONCRETE APPLICATIONS FOR REVIEW WELL IN ADVANCE OF CONCRETE PLACEMENT. CONCRETE MIX DESIGN SHALL BE CERTIFIED BY AN ENGINEER REGISTERED IN THE PROJECT STATE. MIX DESIGN TEST DATA SHALL COMPLY WITH ACI 318 AND SHALL INCLUDE (AT A MINIMUM) AVERAGE 28 DAY STRENGTH, NUMBER OF SAMPLES, AND STANDARD DEVIATION (IF APPLICABLE). TEST RESULTS SHALL NOT BE MORE THAN 24 MONTHS OLD AT TIME OF SUBMITTAL.
- REINFORCING SHALL CONFORM TO ASTM A615, GR60, UNLESS NOTED OTHERWISE.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, GRADE 60.
- WELDED WIRE FABRIC SHALL BE PLACED 1" BELOW T/SLAB, UNLESS NOTED OTHERWISE. LAP FABRIC 6" ON SIDES AND ENDS.
- ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH THE LATEST ADDITION OF THE ACI DETAILING MANUAL.
- ALL MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE.
- REINFORCEMENT LAP SPICES SHALL BE IN ACCORDANCE WITH ACI 318 (CLASS "B" WHERE APPLICABLE), UNLESS NOTED OTHERWISE. ALL CONTINUES REINFORCEMENT SHALL BE SPICED AS REQUIRED.
- PROVIDE STANDARD BAR CHAIRS WITH PROTECTIVE TIPS AND SPACERS AT 5'-0" CENTERS FOR ALL SLABS AND BEAMS ABOVE GRADE. PROVIDE 3" X 6" X 20 GAGE SHEET METAL BAR CHAIRS AT 4'-0" MAXIMUM CENTERS EACH WAY FOR ALL TOP REINFORCING FOR SLABS-ON-GRADE.
- PROVIDE #4 @ 12" CENTERS TEMPERATURE BARS AT RIGHT ANGLES TO MAIN BARS FOR ALL SOLID SLABS ABOVE GRADE, UNLESS NOTED OTHERWISE.
- SUBMIT REINFORCING PLACEMENT AND DETAIL (SHOP) DRAWINGS FOR REVIEW. NO REINFORCING BARS SHALL BE INSTALLED UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND RETURNED.
- PRODUCTS AND MATERIALS:
  - TYPE III PORTLAND CEMENT SHALL CONFORM TO ASTM-C150.
  - AGGREGATES SHALL CONFORM TO ASTM C-33.
  - REINFORCING BARS SHALL CONFORM TO ASTM A-615 (GRADE 60).
  - FORMING SHALL BE OF WOOD, STEEL, OR FIBERGLASS OF SATISFACTORY QUALITY AND CONDITION.
  - NO ADMIXTURES SHALL BE ADDED TO THE CONCRETE UNLESS APPROVED BY THE ENGINEER.
  - NON-SHRINK GROUT SHALL BE READY TO USE NON-METALLIC AGGREGATE AND DEVELOP A 7-DAY COMPRESSIVE STRENGTH OF 5000 PSI.
- ALL REINFORCING SHALL BE SUPPORTED IN FORMS SPACED WITH NECESSARY ACCESSORIES AND SHALL BE SECURELY WIRED TOGETHER IN ACCORDANCE WITH LATEST ADDITION OF THE CRSI "MANUAL OF STANDARD PRACTICE".

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH.....	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 BARS AND LARGER.....	2 INCHES
#5 BARS AND SMALLER.....	1-1/2 INCHES
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
BEAMS AND COLUMNS.....	1-1/2 INCHES
SLABS, WALLS, AND JOISTS.....	3/4 INCHES
- SCHEDULED OR DETAILED REINFORCING STEEL SHALL NOT BE TACK WELDED FOR ANY REASON. WELDED REINFORCING STEEL SPICES ARE NOT PERMITTED WITHOUT ENGINEER'S APPROVAL. WHERE WELDING IS APPROVED IT SHALL CONFORM TO AWS D1.4 STRUCTURAL WELDING CODE - REINFORCING STEEL.
- SLAB-ON-GRADE SHALL BE SAW CUT IMMEDIATELY AFTER CONCRETE HARDENS. THE CONTRACTOR SHALL SUBMIT LAYOUT AND CONSTRUCTION SCHEDULE ("SOFT CUT" ® INTERNATIONAL OR SIM.)
- CONTROL JOINTS IN SLABS ON GROUND SHALL BE LOCATED AT 15'-0" MAXIMUM SPACING AND SHALL CREATE SECTIONS OF SLAB WITH A MAXIMUM ASPECT RATIO OF 1.5:1. CONTROL JOINTS SHALL BE SAWN AND SHALL BE A MINIMUM OF 1/4 OF THE SLAB THICKNESS DEEP IF CUT WITH A CONVENTIONAL SAW, OR 1" DEEP IF CUT WITH AN EARLY-ENTRY DRY-CUT SAW. THE CONTROL JOINTS SHALL BE SAWN AS SOON AS THE SAW BLADE CAN CUT THE CONCRETE WITHOUT DISPLACING THE AGGREGATE. CUT EVERY OTHER MESH WIRE AT THE CONTROL JOINT LOCATION PRIOR TO PLACING CONCRETE.
- SAWN CONTROL JOINTS SHALL BE PLACED AS SOON AS CONCRETE IS ABLE TO BE SAWN WITHOUT PULLING AGGREGATE FROM FLOOR. SLABS SHALL NOT BE LEFT OVERNIGHT, OR ANY REASONABLE AMOUNT OF TIME, WITHOUT SAWING JOINTS. WEATHER IS CRITICAL TO THE SCHEDULE OF SAWN JOINTS. IF LARGE AREAS OF SLAB ARE POURED AT ONE TIME, SEVERAL SAWS MAY BE REQUIRED SO THAT JOINTS ARE PLACED IN TIME TO PREVENT SHRINKAGE CRACKING. PROPER JOINTING OF THE SLAB IS CRITICAL. REFER TO THE ACI MANUAL OF CONCRETE PRACTICE FOR PROPER JOINTING TECHNIQUES.
- THE FLATNESS AND LEVELNESS OF THE SLAB-ON-GRADE SHALL BE DETERMINED ACCORDING TO ASTM E-1155 OR ACI 117. SLAB CLASS 5 (ACI 302) STANDARD TEST METHOD USING F NUMBERS. THE SPECIFIC FLATNESS AND LEVELNESS SHALL BE F/-35 AND F/-20.
- WHERE FOOTINGS, WALLS, OR OTHER STRUCTURAL ELEMENTS INTERSECT, CORNER OR TEE, PROVIDE CORNER BARS WITH REQUIRED LAP LENGTHS TO PROVIDE CONTINUITY OF HORIZONTAL STEEL REINFORCING, UNLESS NOTED OTHERWISE.
- PROVIDE A MINIMUM OF 3" COVER FOR ANCHOR BOLTS AND LOCATE HORIZONTAL REINFORCEMENT TO THE OUTSIDE FOR ANCHOR BOLT CONTAINMENT, UNLESS NOTED OTHERWISE.
- WHERE DOWELS, BOLTS OR INSERTS ARE CALLED OUT TO BE ANCHORED TO CAST IN PLACE OR PRECAST CONCRETE ELEMENTS USING ADHESIVE ANCHORS, USE AN ANCHORAGE SYSTEM EQUAL TO "HILT" HIT HY-200. FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS. ALTERNATE ANCHORAGE SYSTEMS MAY BE USED WITH ENGINEER'S PRIOR APPROVAL.
- PROVIDE TEMPORARY SHORING AND BRACING OF ALL STRUCTURAL AND MISCELLANEOUS ELEMENTS UNTIL CONCRETE HAS OBTAINED 80% OF DESIGN STRENGTH AND ALL PERMANENT BRACING ELEMENTS ARE INSTALLED.
- PLACEMENT OF CONCRETE, COLD WEATHER AND HOT WEATHER PRECAUTIONS, MATERIAL AND PROPORTIONING REQUIREMENTS, REBAR COVER AND DETAILING SHALL CONFORM TO THE REQUIREMENTS OF THE ACI 318.



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GENERAL NOTES

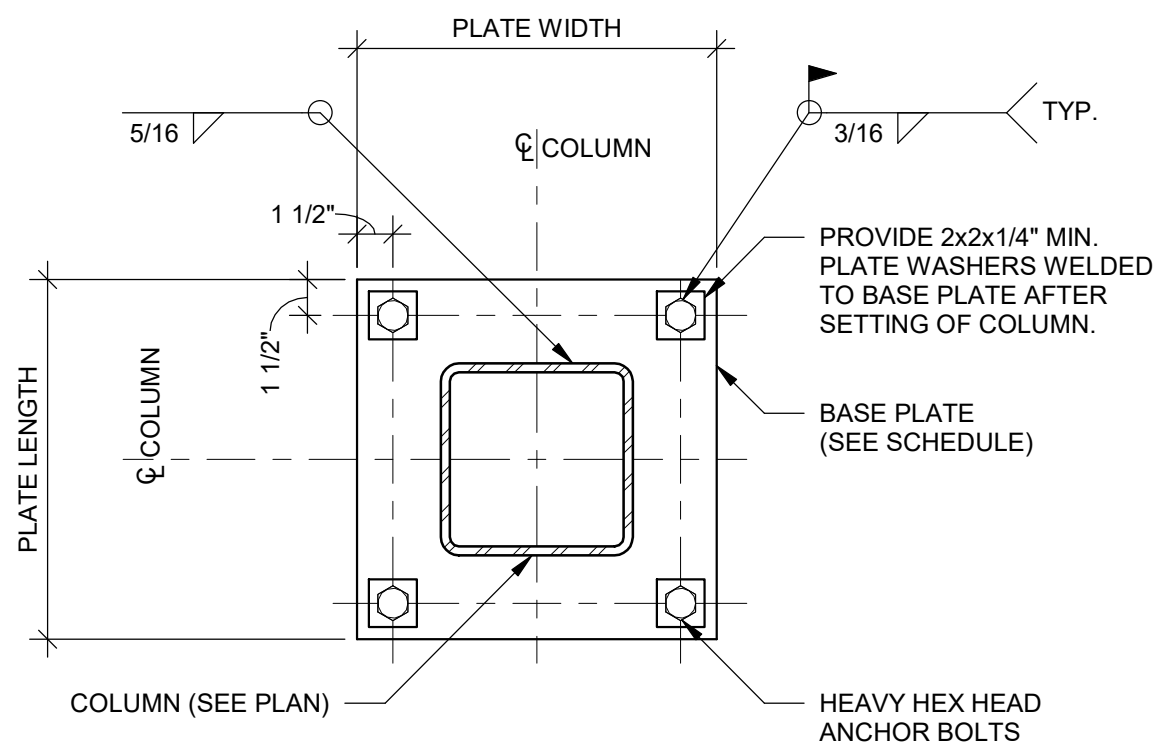
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HDA Job Number: 2426  
Date: 6-13-2025

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BASE PLATE SCHEDULE				
COLUMN SIZE	PLATE LENGTH	PLATE WIDTH	PLATE THICKNESS	ANCHOR BOLTS
HSS5x5	12"	12"	3/4"	(4) 3/4" DIA. x12" EMBED

FOOTING SCHEDULE				
MARK	LENGTH	WIDTH	THICKNESS	REINFORCEMENT
F4	4' - 0"	4' - 0"	1' - 4"	(5) #5 E.W.
F5	5' - 0"	5' - 0"	1' - 4"	(6) #5 E.W.

SHEAR WALL SCHEDULE								
SYMBOL	SHEATHING	FASTENER	MINIMUM PENETRATION OF FASTENER	FASTENER SPACING		END WALL POSTS	ANCHOR BOLT SIZES & SPACING	HOLDOWN
				PANEL EDGES	FIELD			
1	15/32" STRUCT 1 APA RATED, EXT GRADE	10d NAIL	1 1/2"	6" O.C.	12" O.C.	(3) 2x6	5/8" DIA SIMPSON TITEN HD @ 16" O.C. MIN EMBED 4"	SIMPSON HDUE9-SDS3.5
2	15/32" STRUCT 1 APA RATED, EXT GRADE	10d NAIL	1 1/2"	6" O.C.*	12" O.C.	(2) 2x6	5/8" DIA SIMPSON TITEN HD @ 16" O.C. MIN EMBED 4"	SIMPSON HDUE7-SDS3
3	15/32" STRUCT 1 APA RATED, EXT GRADE	10d NAIL	1 1/2"	4" O.C.	12" O.C.	6x6	5/8" DIA SIMPSON TITEN HD @ 16" O.C. MIN EMBED 4"	SIMPSON HDUE17-SDS4.5

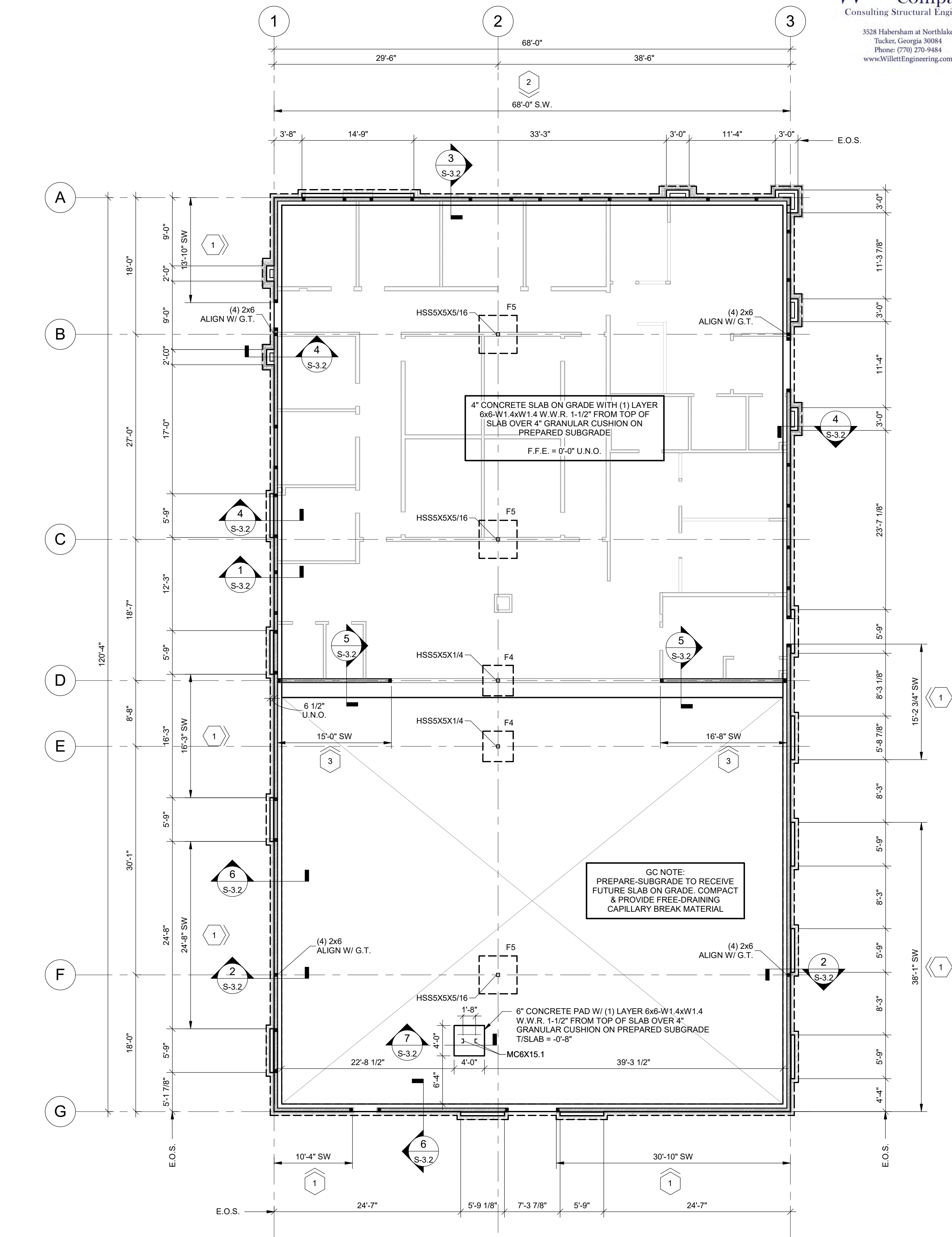
- NOTES:
1. WHEN NAIL SPACING IS 3" OR LESS, FRAMING MEMBER AND BLOCKING MEMBER IN THE SHEAR WALL SHALL BE A 3x MIN. OR (2) 2x, U.N.O.
  2. \* CONTINUE NAIL PATTERN ABOVE AND BELOW OPENINGS. SHEAR WALL END POSTS AT OPENINGS IS THE GREATER OF END POSTS OR HEADER KING STUDS.
  3. HOLDOWNS REPRESENT MODEL NUMBERS BY SIMPSON STRONG-TIE U.N.O.

FOUNDATION PLAN LEGEND

- INDICATES STEP IN FOUNDATION
- INDICATES ATYPICAL TOP OF FOOTING ELEVATION
- INDICATES A STEP IN THE SLAB ON GRADE
- INDICATES STRUCTURAL WALL 2x6 @ 16" O.C. U.N.O. - SEE ARCH. FOR NON-STRUCTURAL WALLS
- INDICATES SHEAR WALL

FOUNDATION PLAN NOTES:

1. TOP OF ALL EXTERIOR FOOTINGS SHALL BE -1'-0" BELOW FINISHED FLOOR, U.N.O.
2. TOP OF ALL INTERIOR FOOTINGS SHALL BE -1'-0" BELOW FINISHED FLOOR, U.N.O.
3. REFER TO ARCH'L AND CIVIL DRAWINGS FOR LOCATION OF MOISTURE BARRIER, CURBS, EXTERIOR SLABS, DRAINAGE, RAMPS, STEPS, WALKS, ETC.
4. BUILDING SLAB IS NOT DESIGNED TO SUPPORT CRANE LOADS, CONCRETE MIXING TRUCKS, OR OTHER SPECIFIC CONSTRUCTION LOADINGS.
5. FOOTINGS SHALL BE CENTERED ON THE CENTERLINE OF THE WALL AND/OR COLUMNS, U.N.O.
6. COORDINATE LOCATION OF LOWERED FOOTINGS WITH PLUMBING DRAWINGS.
7. REFER TO ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. COORDINATE SLAB ELEVATIONS AND SLOPES WITH ARCHITECTURAL PLANS.
8. REFER TO ARCHITECTURAL AND MEP DRAWINGS FOR SIZE AND LOCATION OF SLAB AND FOUNDATION PENETRATIONS.
9. THICKEN SLAB TO MAINTAIN THE SLAB THICKNESS AROUND FLOOR BOXES AND CONDUIT.

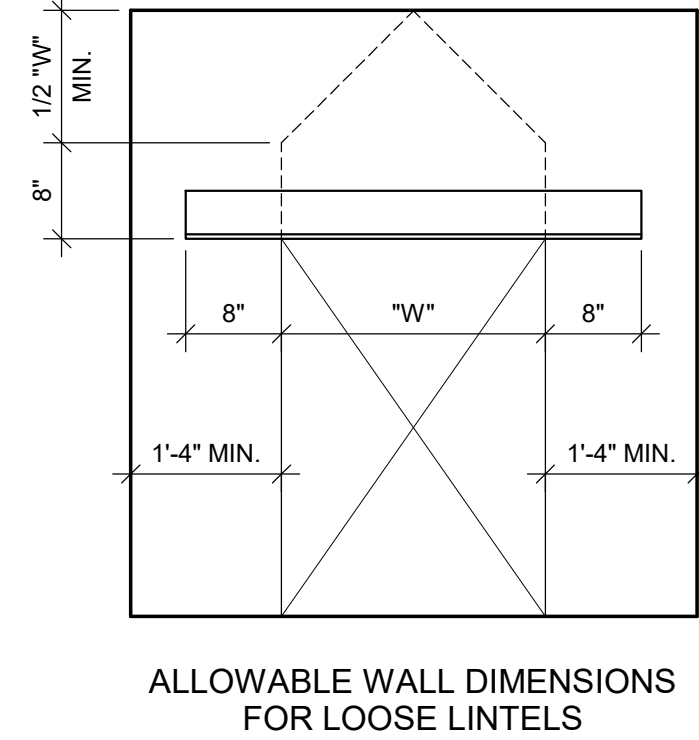


1 S-1.0  
SCALE: 1/8" = 1'-0"

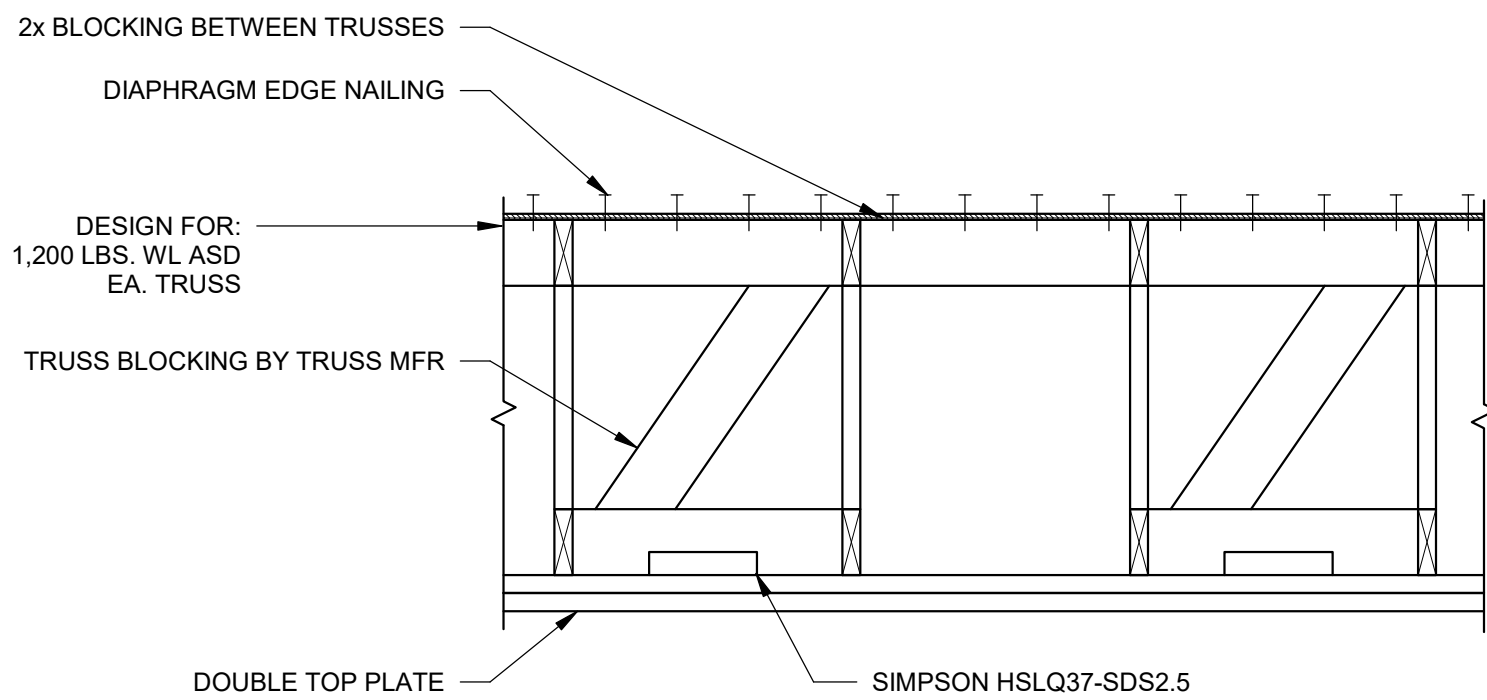


BRICK LINTEL SCHEDULE		
OPENING WIDTH	BRICK W/ AIR SPACE	BRICK W/ NO AIR SPACE
	SEE ARCH	
≤ 5'-0"	L4hx3/8 (h = 4" MIN)	L3-1/2x3-1/2x3/8
6'-0"	L6hx3/8 (h = 4" MIN)	L4x4x3/8
8'-0"	L6hx3/8 (h = 6" MIN)	L6x4x3/8 (LLV)
12'-0"	L6hx5/8 (h = 4" MIN)	L6x4x7/8 (LLV)

- NOTES:  
1. LOOSE LINTELS SHALL BE HOT-DIP GALVANIZED U.N.O.  
2. WHERE ANGLE SIZES ARE NOT AVAILABLE, BENT PLATES ARE ALLOWED TO BE SUBSTITUTED.  
3. "h" DENOTES ANGLE HORIZONTAL LEG VARIES BASED ON AIR SPACE AND BRICK DIMENSIONS.  
4. BEAR LOOSE LINTELS A MIN. OF 6" ON EACH SIDE OF OPENINGS.  
5. DO NOT USE THE BRICK LINTEL SCHEDULE WHERE DIMENSIONS DO NOT MEET MINIMUM WALL DIMENSIONS IN THE ALLOWABLE WALL DIMENSIONS DIAGRAM.



## LOOSE LINTEL SCHEDULE

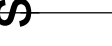



## TYP. TRUSS BLOCKING

ROOF TRUSS LOADS		
LOAD	TOP CHORD	BOTT. CHORD
DEAD	10.0 PSF	12.0 PSF
ROOF LIVE	20.0 PSF	0.0 PSF
LIVE	0.0 PSF	10.0 PSF <sup>1,2</sup>
WIND (DOWN)	SEE S-0.1	0.0 PSF
WIND (UP)	SEE S-0.1	0.0 PSF
RAIN	19.0 PSF	0.0 PSF
SNOW	35.8 PSF	0.0 PSF

1. NON-CONCURRENT WITH ROOF LIVE LOAD.  
2. TRUSS MANUFACTURER SHALL DESIGN FOR ADDITIONAL LOADS WHERE PARAMETERS FOR UNINHABITABLE ATTICS WITH STORAGE CRITERIA ARE MET PER ASCE 7.

### ROOF FRAMING PLAN LEGEND:

- <XXX> INDICATES AXIAL ASD (0.6W OR 0.7EQ) LOAD TO BE INCORPORATED INTO MEMBERS AND CONNECTION DESIGN  
 INDICATES BEAM SPLICE  
 INDICATES SNOW DRIFT APPLIED TO ROOF FRAMING. SEE SCHEDULE FOR DRIFT LOAD AND WIDTHS

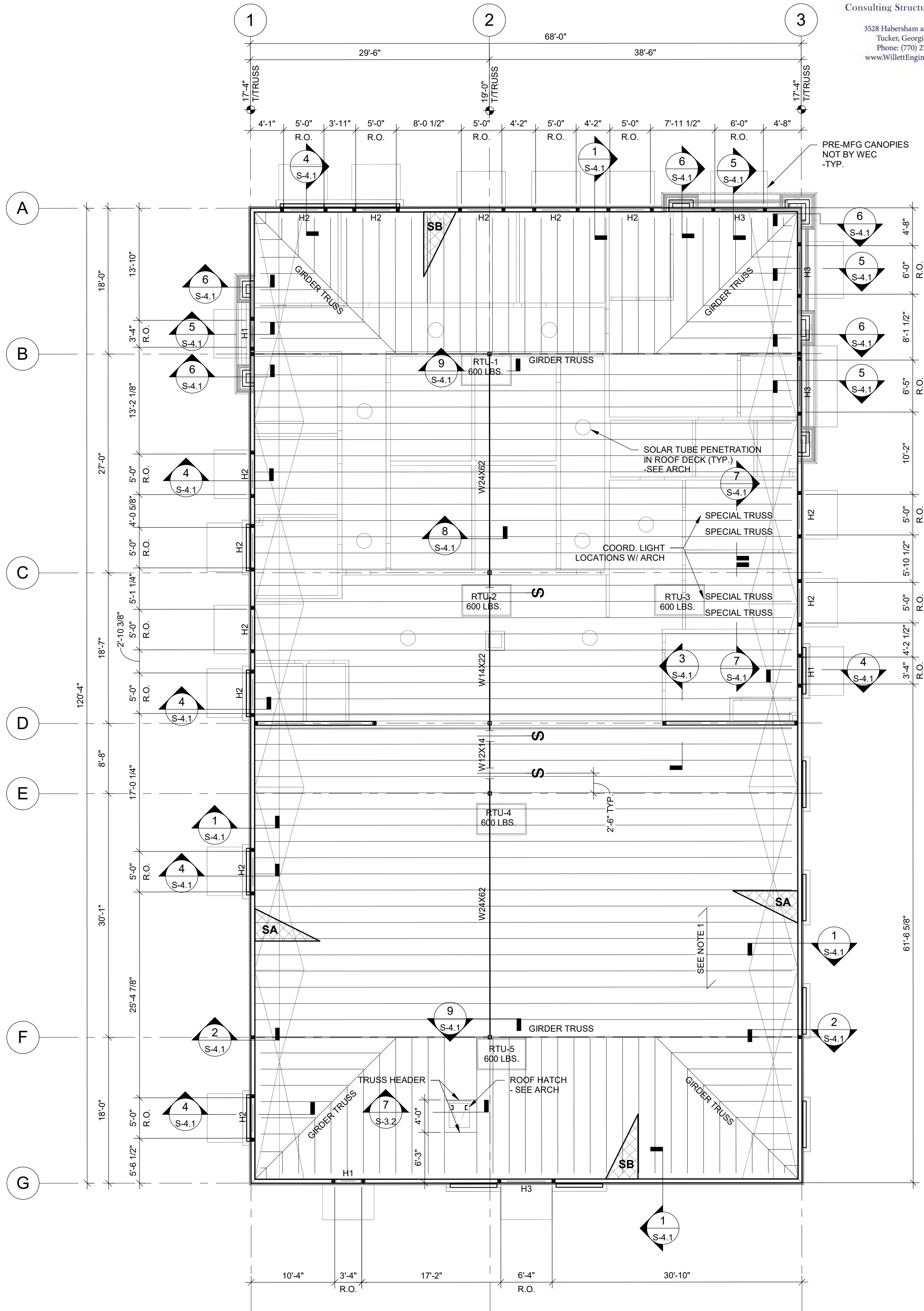
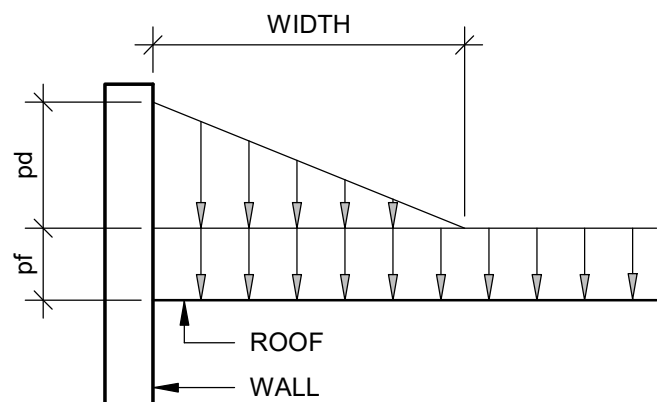
SNOW DRIFT LOADING SCHEDULE			
MARK	BALANCED SNOW "pd"	DRIFT SNOW "pd"	DRIFT WIDTH "WIDTH"
SA	26.6 PSF	44.3 PSF	9'-6"
SB	26.6 PSF	58.0 PSF	12'-3"

WALL OPENING SCHEDULE			
HEADER MARK	H1	H2	H3
HEADER SIZE	(3) 2x6	(3) 2x8	(3) 2x12
KING STUDS	(2) 2x6	(3) 2x6	(3) 2x6
JACK STUDS	(2) 2x6	(2) 2x6	(2) 2x6
SILL BEAM	(1) 2x6 IF REQ'D	(2) 2x6 IF REQ'D	(2) 2x6 IF REQ'D
ANCHORAGE	(1) A.B.	(1) A.B.	(1) A.B.

- NOTES:  
1. A.B. SHALL BE 5/8" DIA. SIMPSON TITEN HD MIN. EMBED 4".

### ROOF FRAMING PLAN NOTES

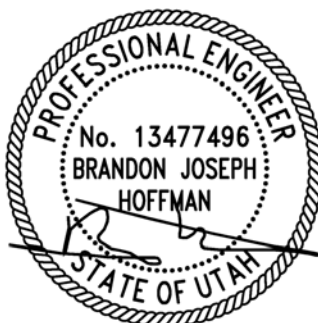
1. INDICATES SPAN OF 40/20 RATED, 5/8" PLYWOOD STRUCTURAL 1 ROOF. SEE NOTES & DETAILS FOR NAILING.  
2. TRUSS MANUFACTURER SHALL DESIGN AND PROVIDE ALL BRACING AND UPLIFT BRIDGING.  
3. B/TRUSS = 14'-0" U.N.O. T/BREAM = 18'-3 1/2" U.N.O.  
4. ROOF IS NOT DESIGNED TO SUPPORT ANY FUTURE ROOF TOP EQUIPMENT. WHAT IS INDICATED ON THIS DRAWING SHALL BE INCORPORATED INTO THE DESIGN BY THE TRUSS MFR.  
5. REFER TO ARCHITECTURAL AND MEP DRAWINGS FOR SIZE AND LOCATION OF DECK PENETRATIONS.



1  
S-2.0

## ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"



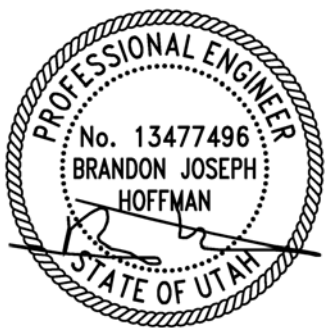
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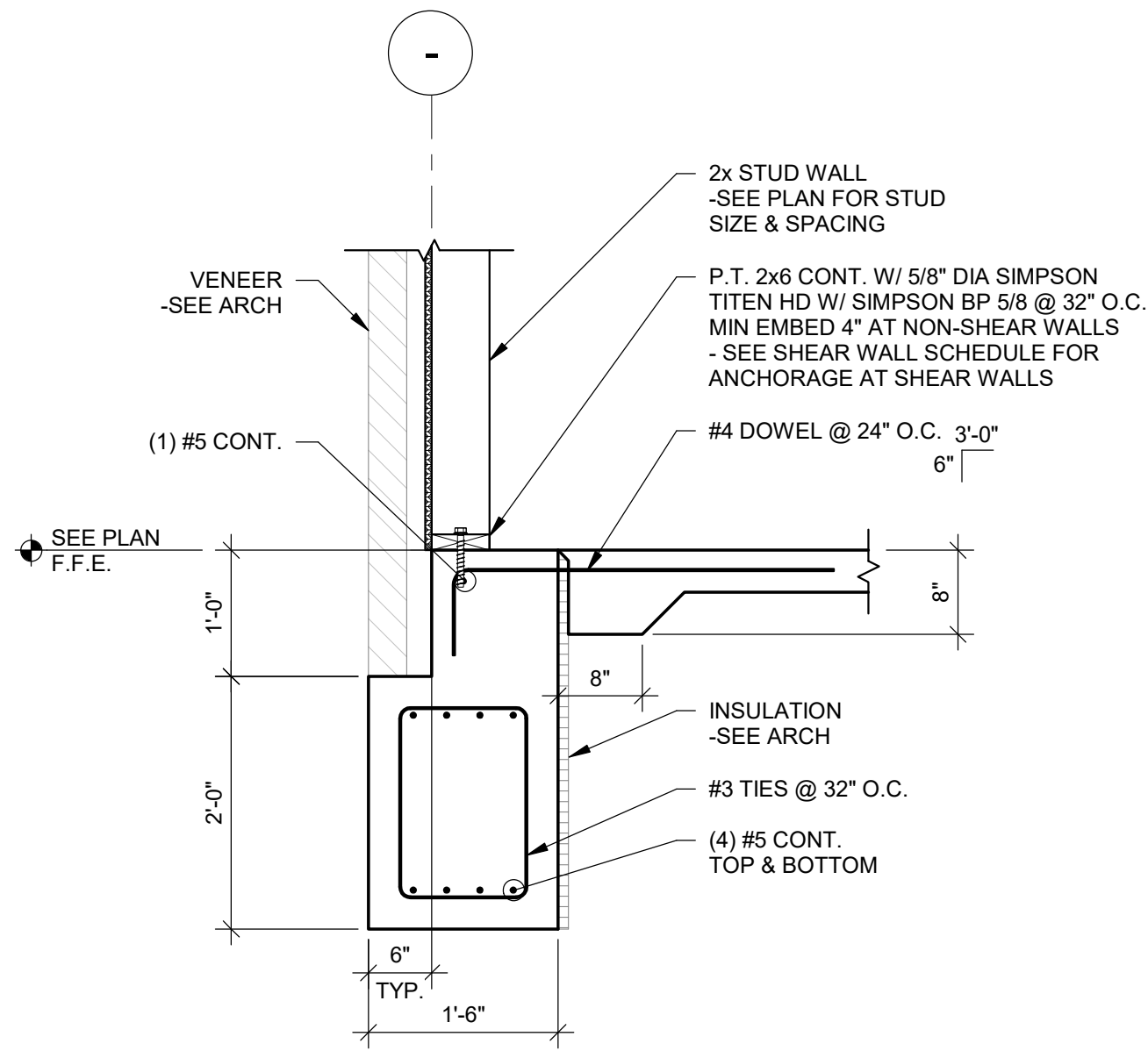


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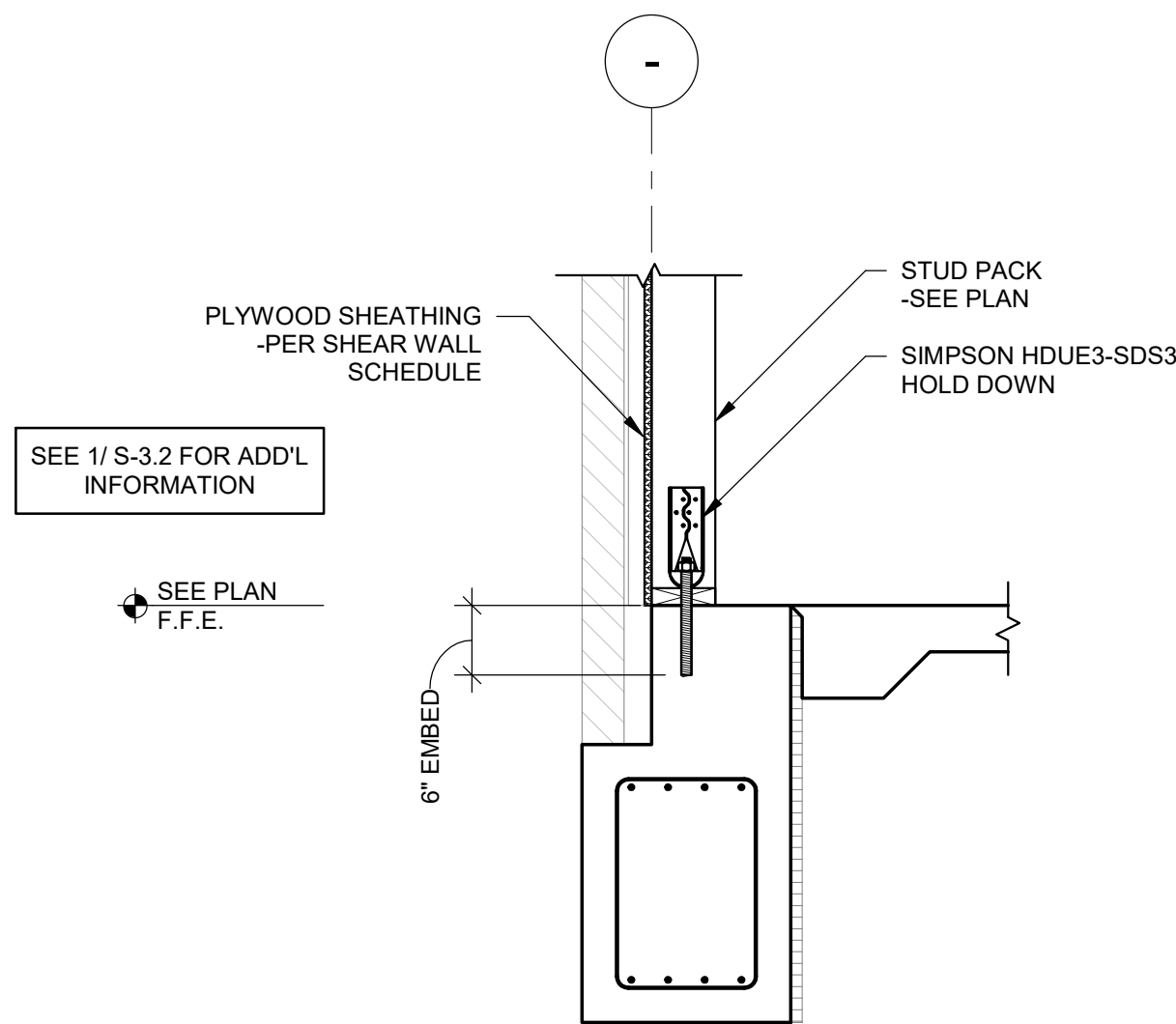
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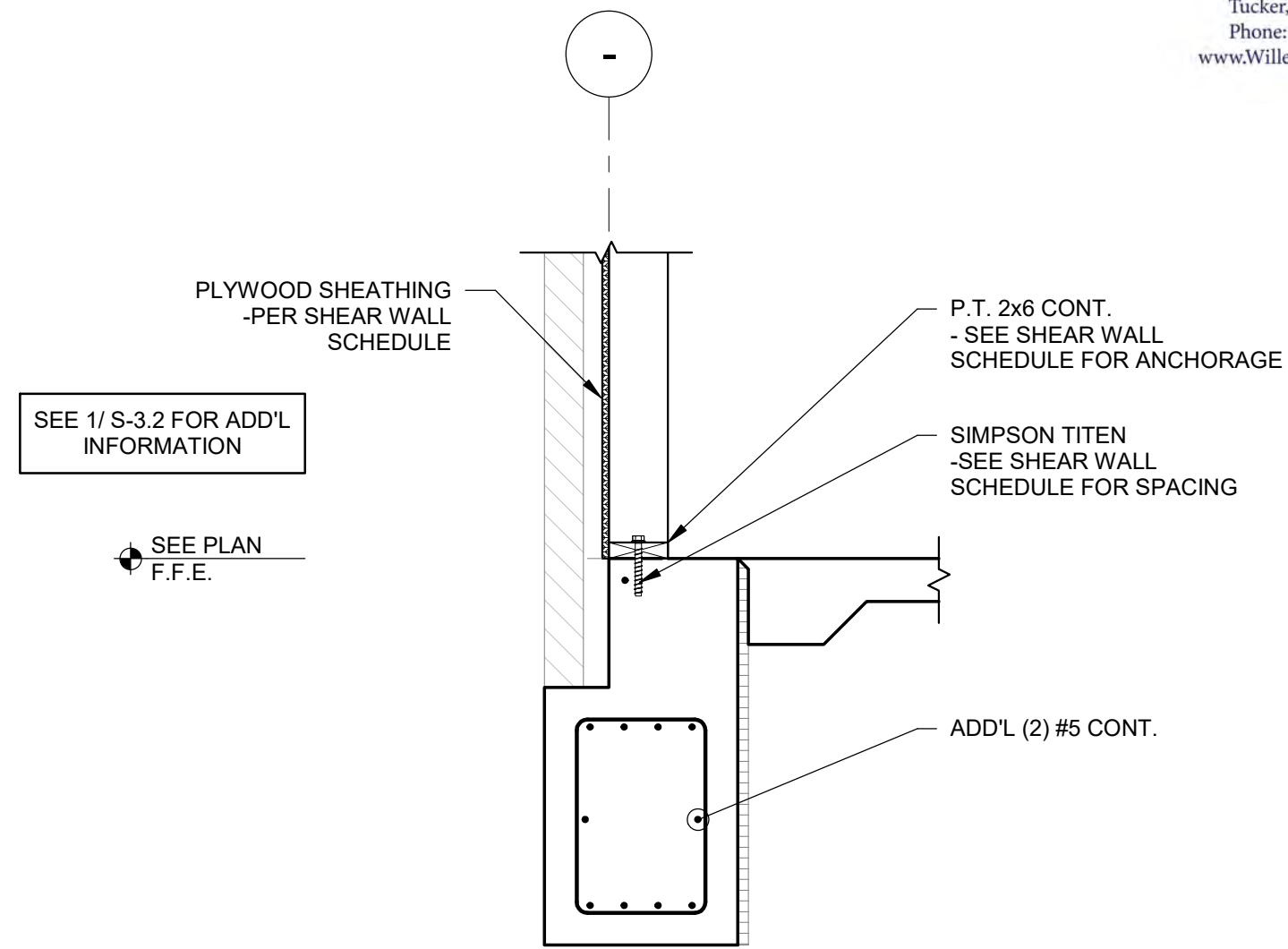
S-3.2



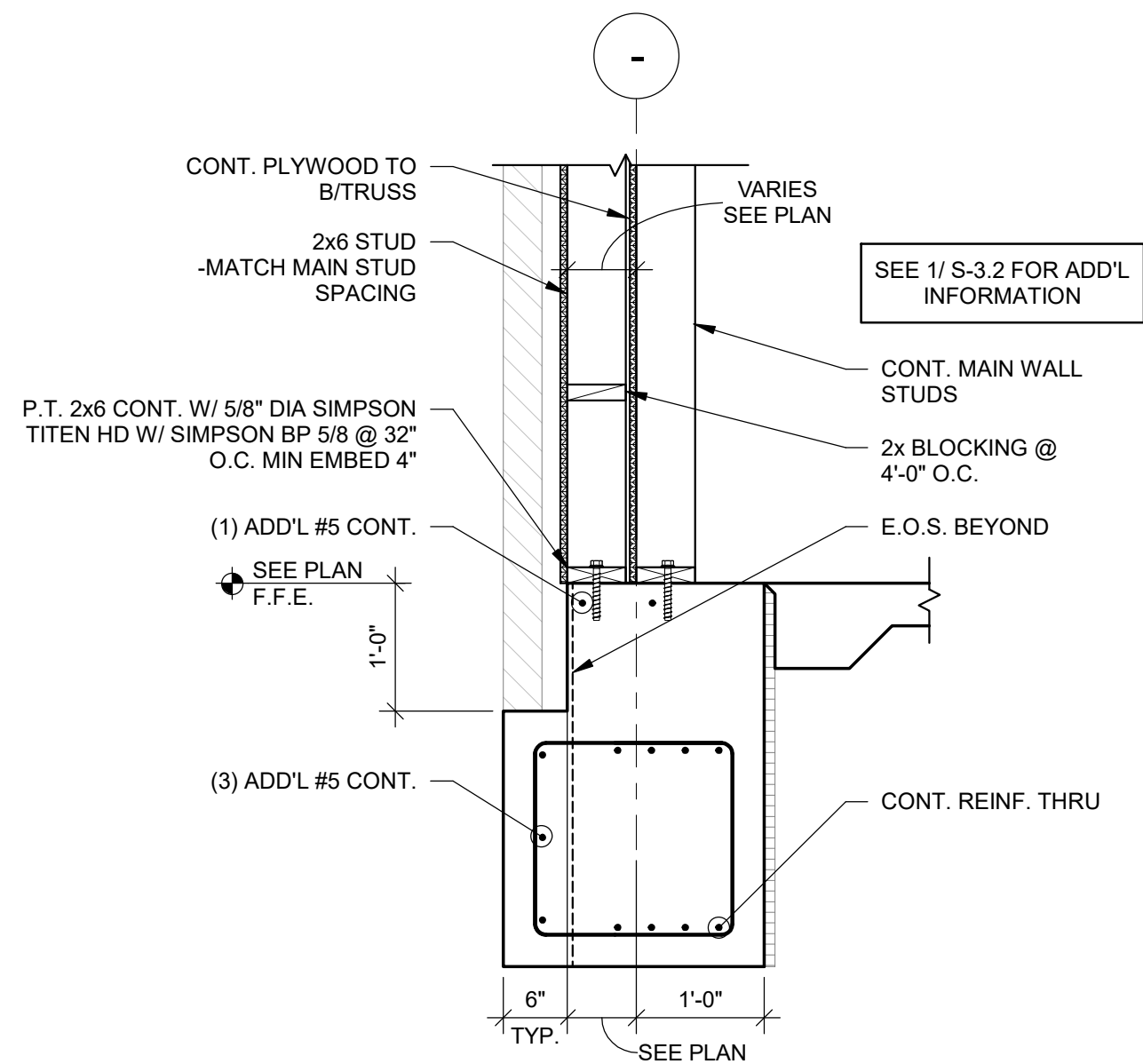
1  
S-3.2  
**TYP. EXT. WALL FOOTING**  
SCALE: 3/4" = 1'-0"



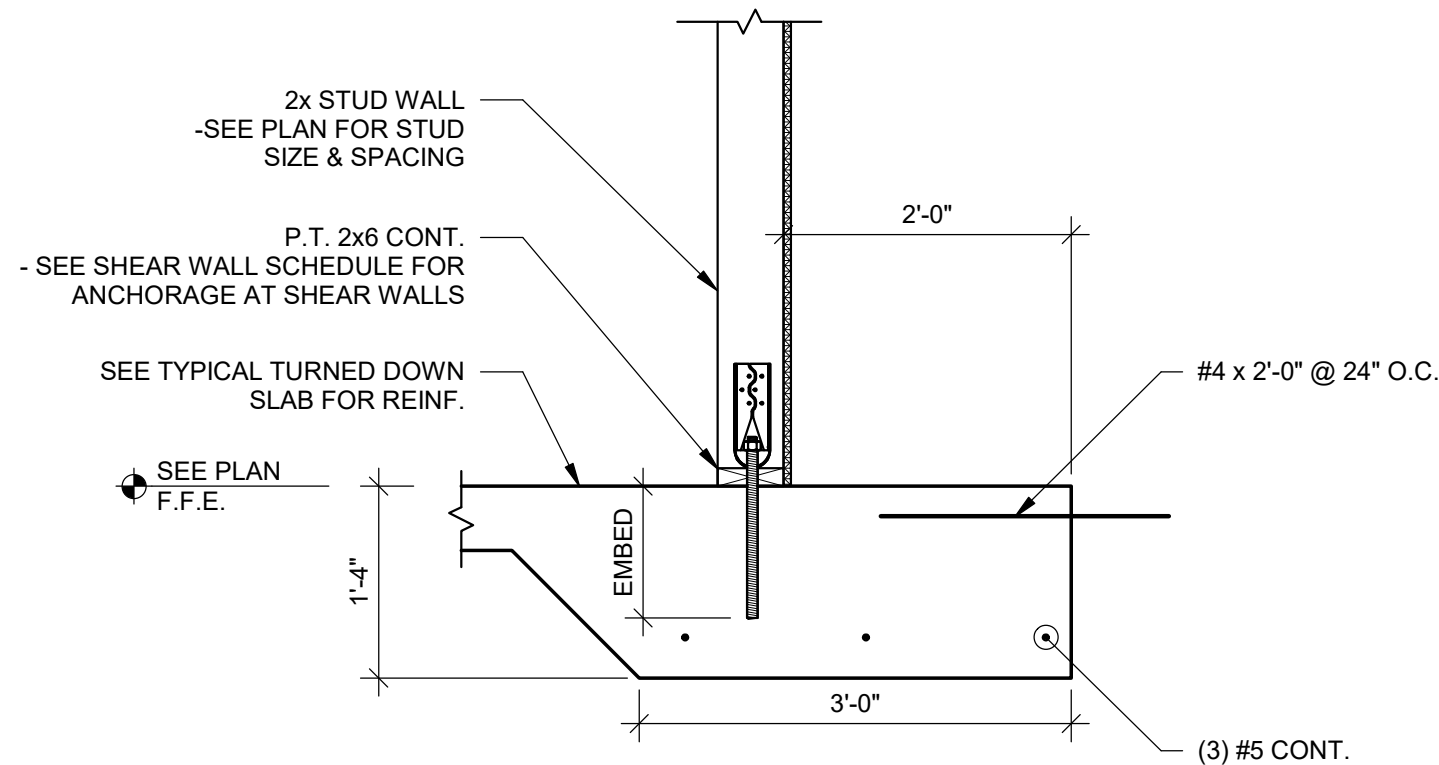
2  
S-3.2  
**WALL FOOTING @ HOLD DOWN**  
SCALE: 3/4" = 1'-0"



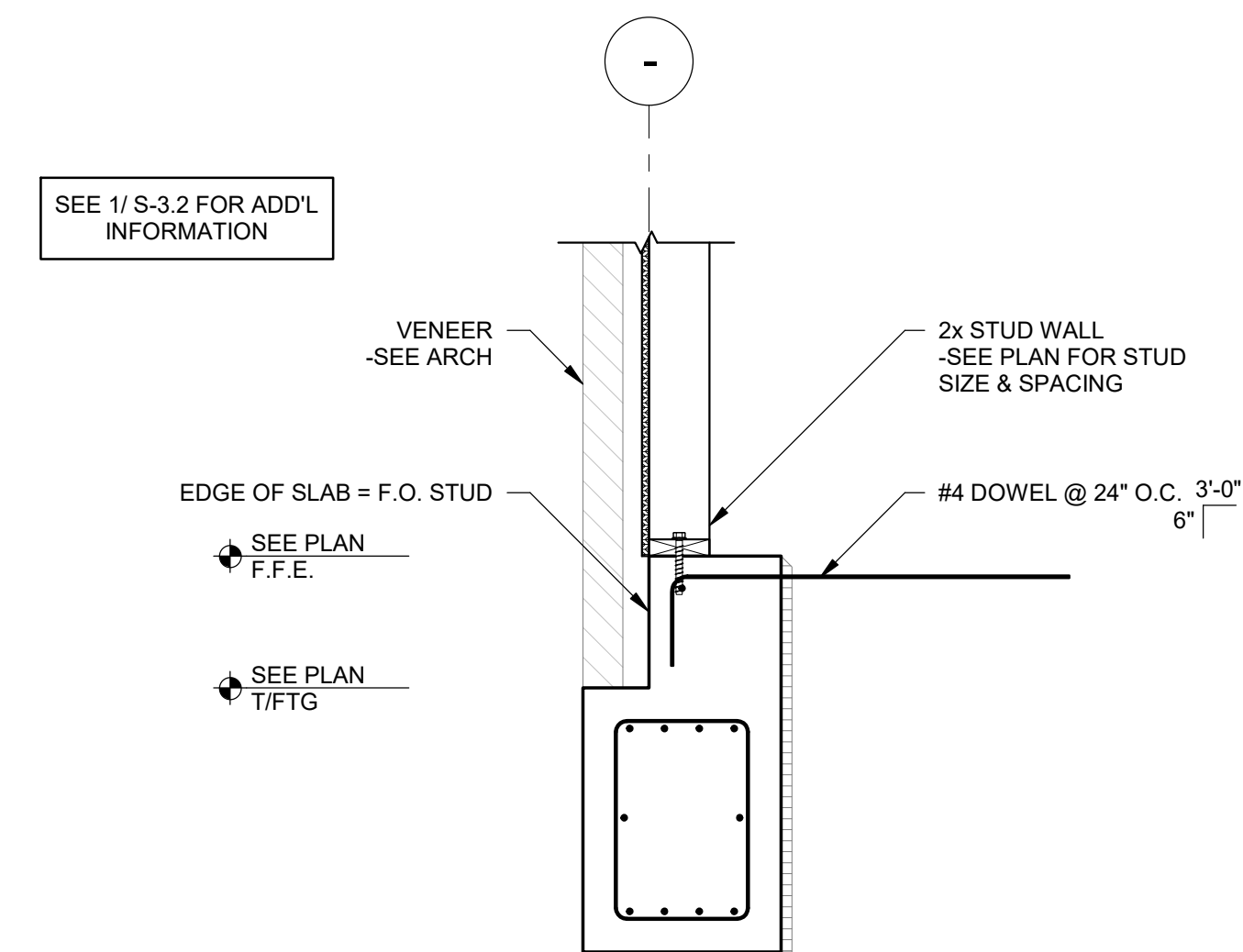
3  
S-3.2  
**WALL FOOTING @ SHEAR WALL**  
SCALE: 3/4" = 1'-0"



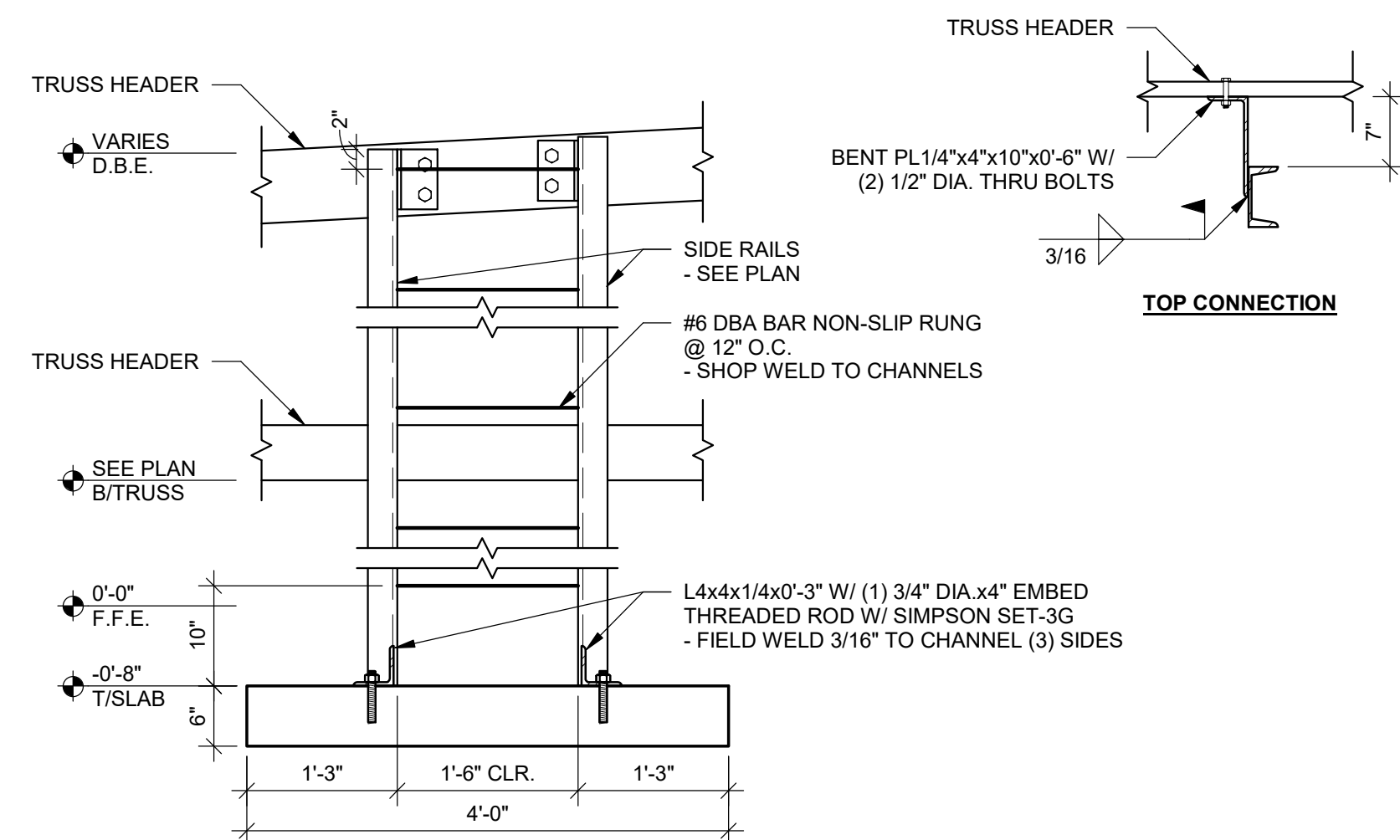
4  
S-3.2  
**WALL FOOTING AT FURRING WALL**  
SCALE: 3/4" = 1'-0"



5  
S-3.2  
**FOUNDATION @ INT. SHEAR WALL**  
SCALE: 3/4" = 1'-0"



6  
S-3.2  
**TYP. EXT. FOOTING AT PARTIAL SLAB**  
SCALE: 3/4" = 1'-0"



7  
S-3.2  
**LADDER DETAIL**  
SCALE: 3/4" = 1'-0"



Release	Description
06-13-25	PERMITTING/BID
10-27-25	PERMIT COMMENTS

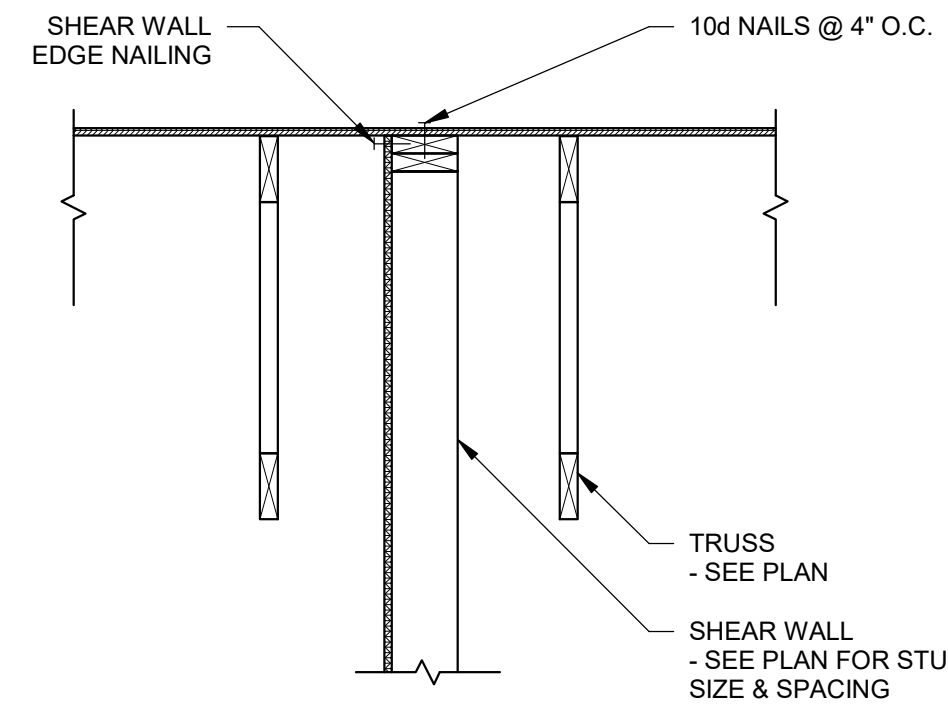
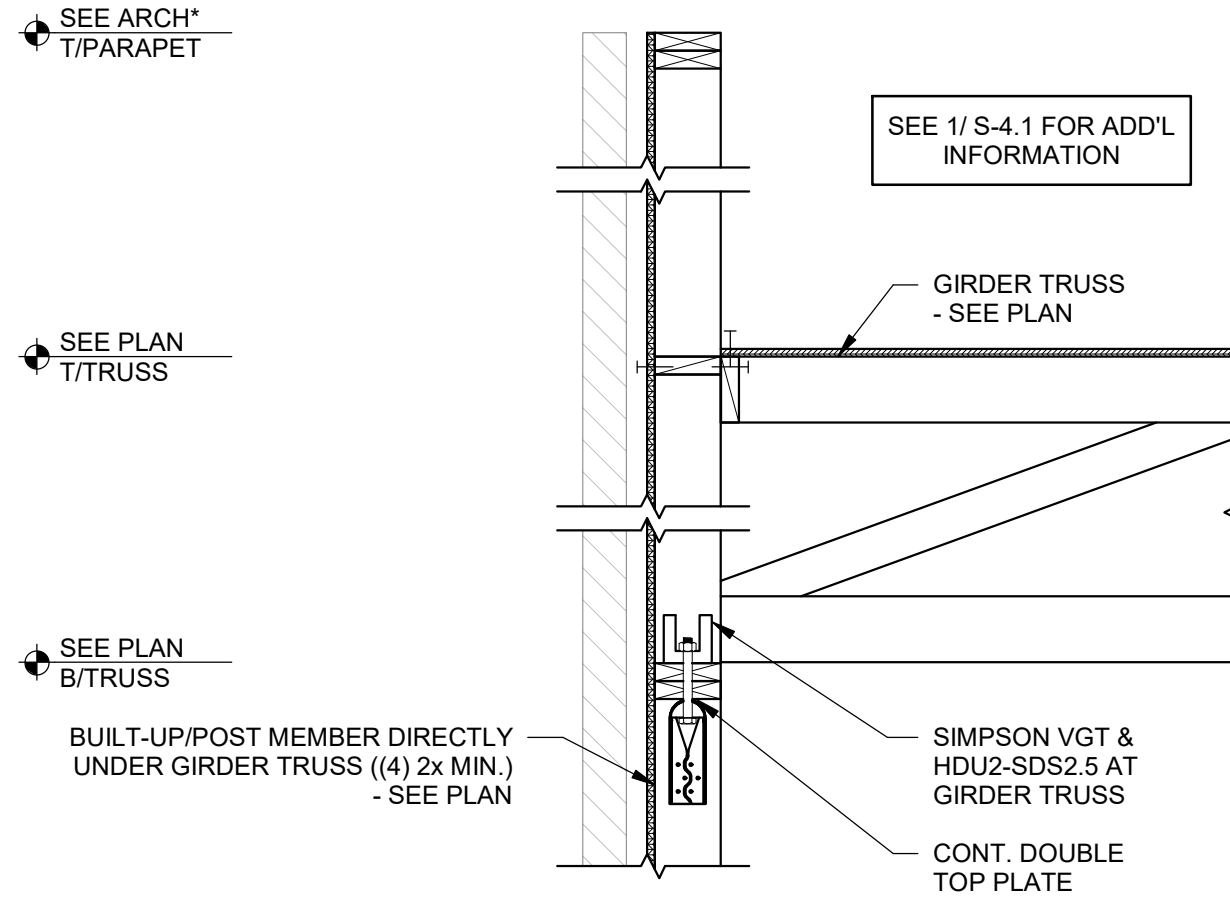
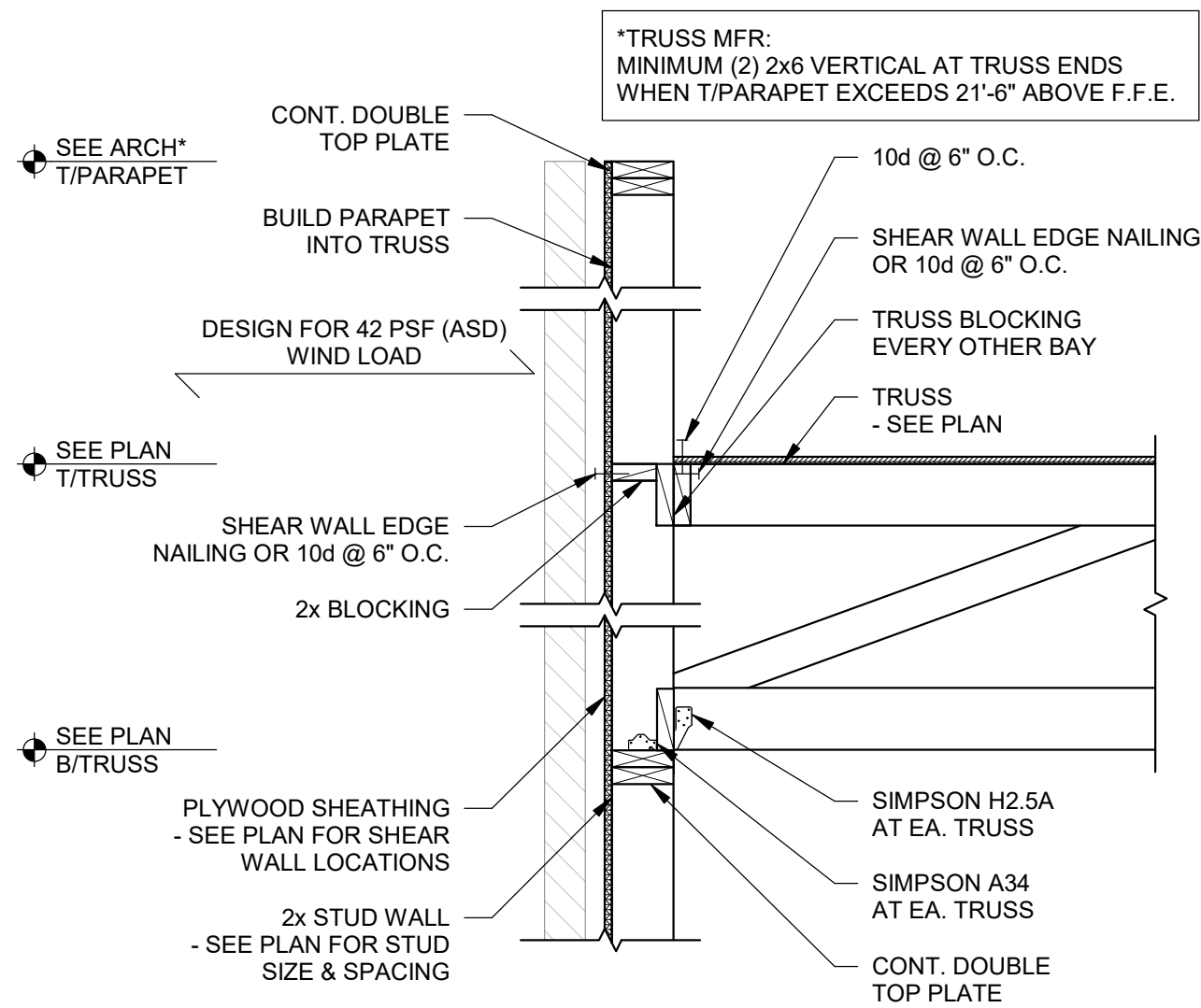
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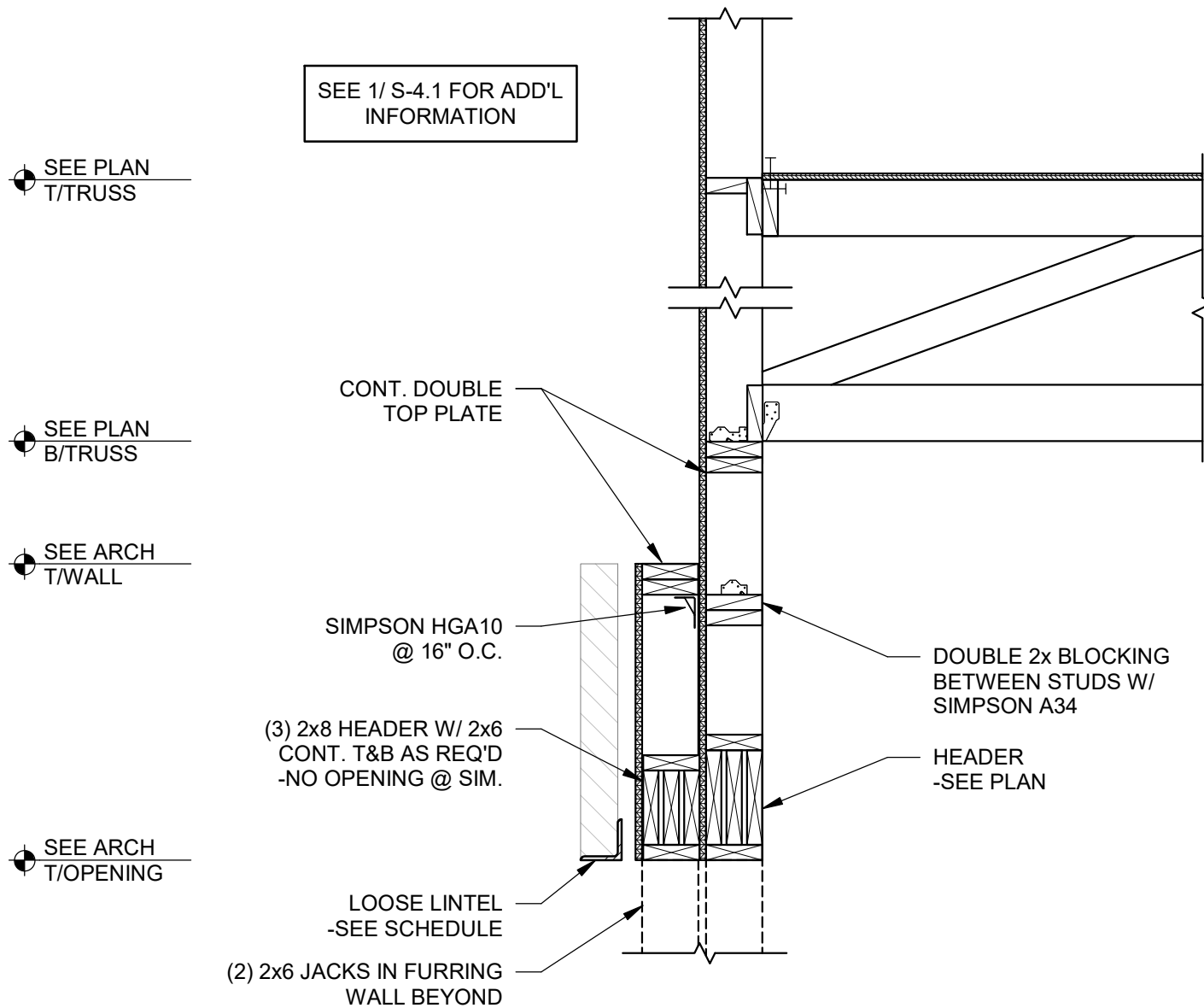
FOR CONSTRUCTION

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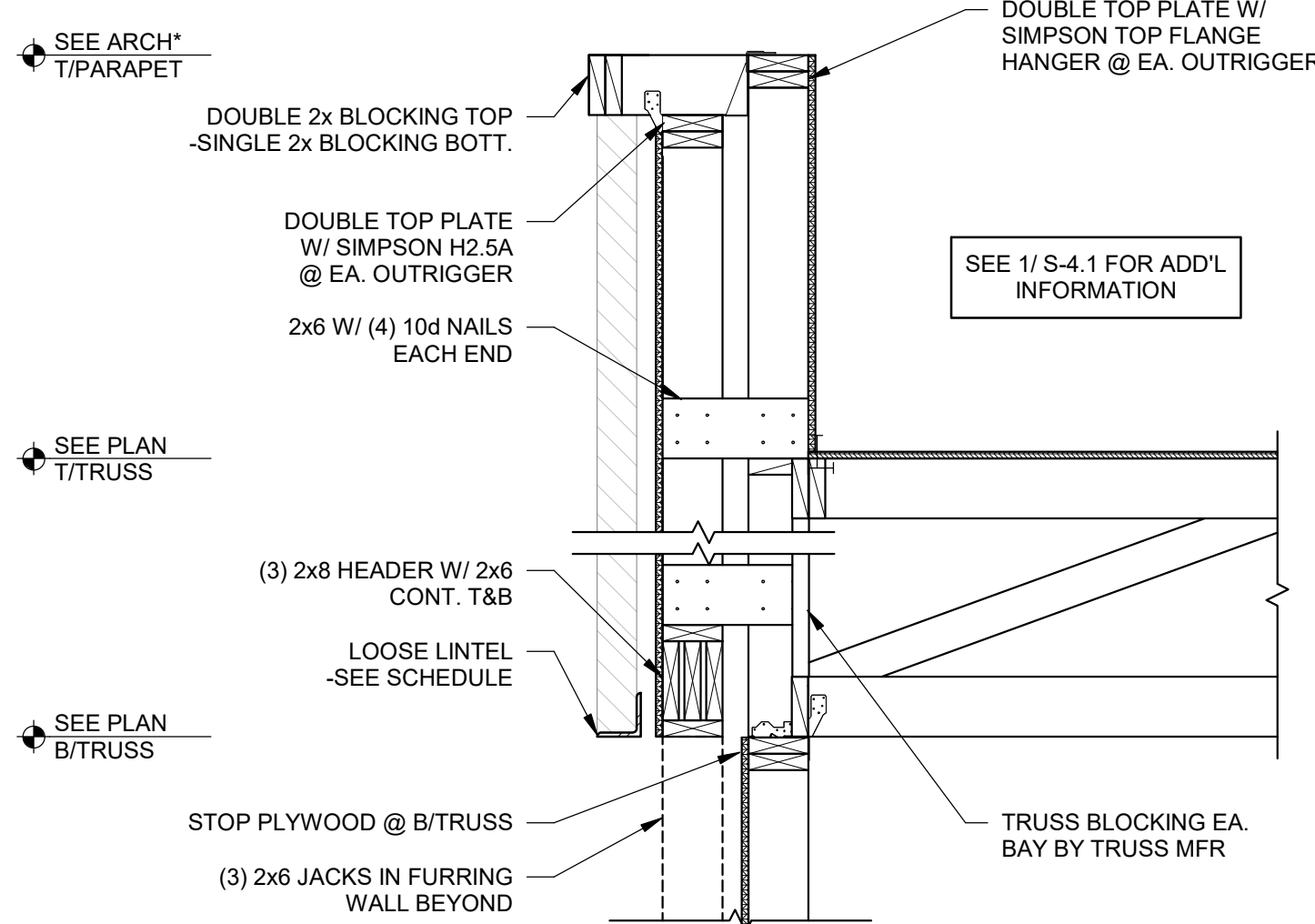
## 1 TYP. WALL SECTION

SCALE: 3/4" = 1'-0"



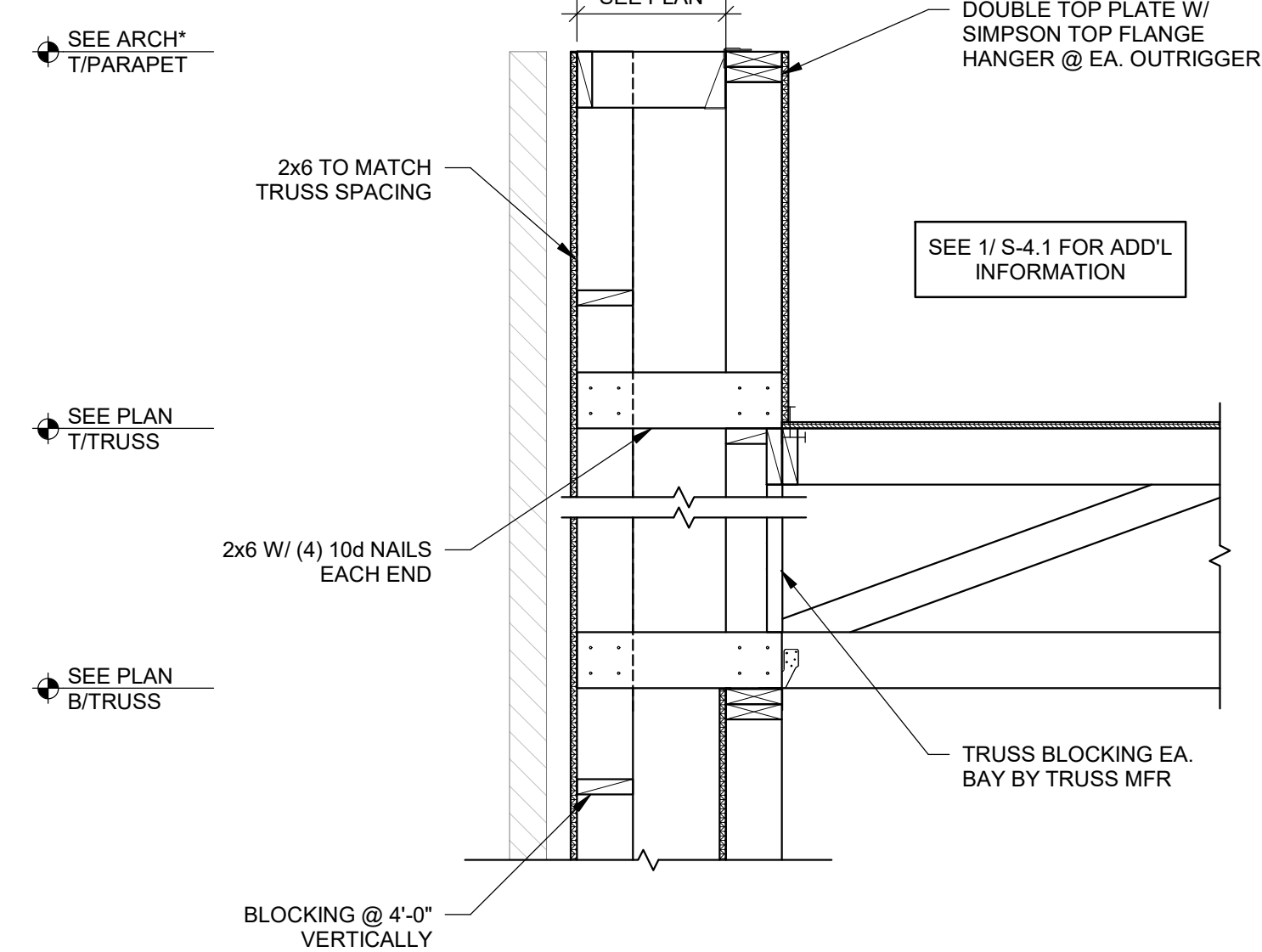
## 2 TYP. WALL SECTION @ GIRDER

SCALE: 3/4" = 1'-0"



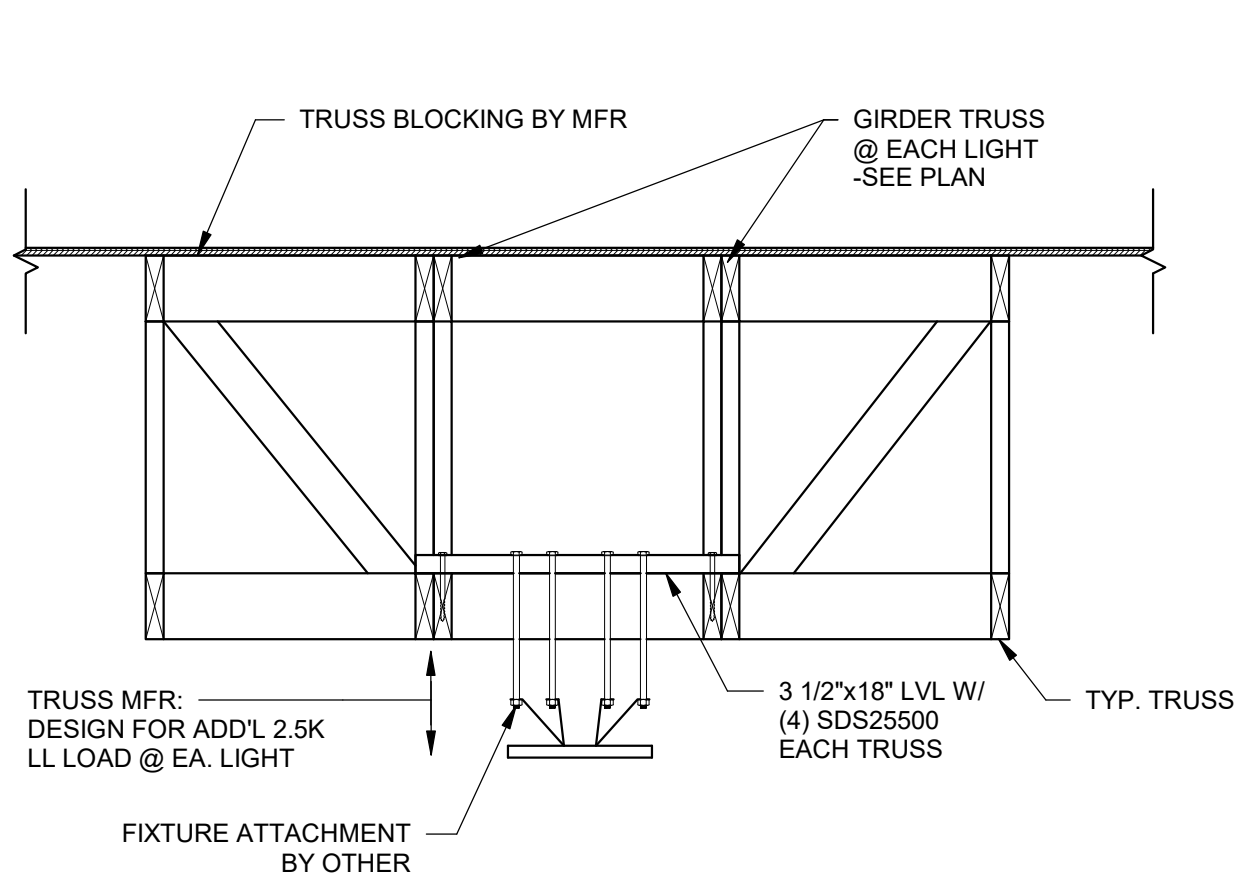
## 3 SECTION @ INT. SHEAR WALL

SCALE: 3/4" = 1'-0"



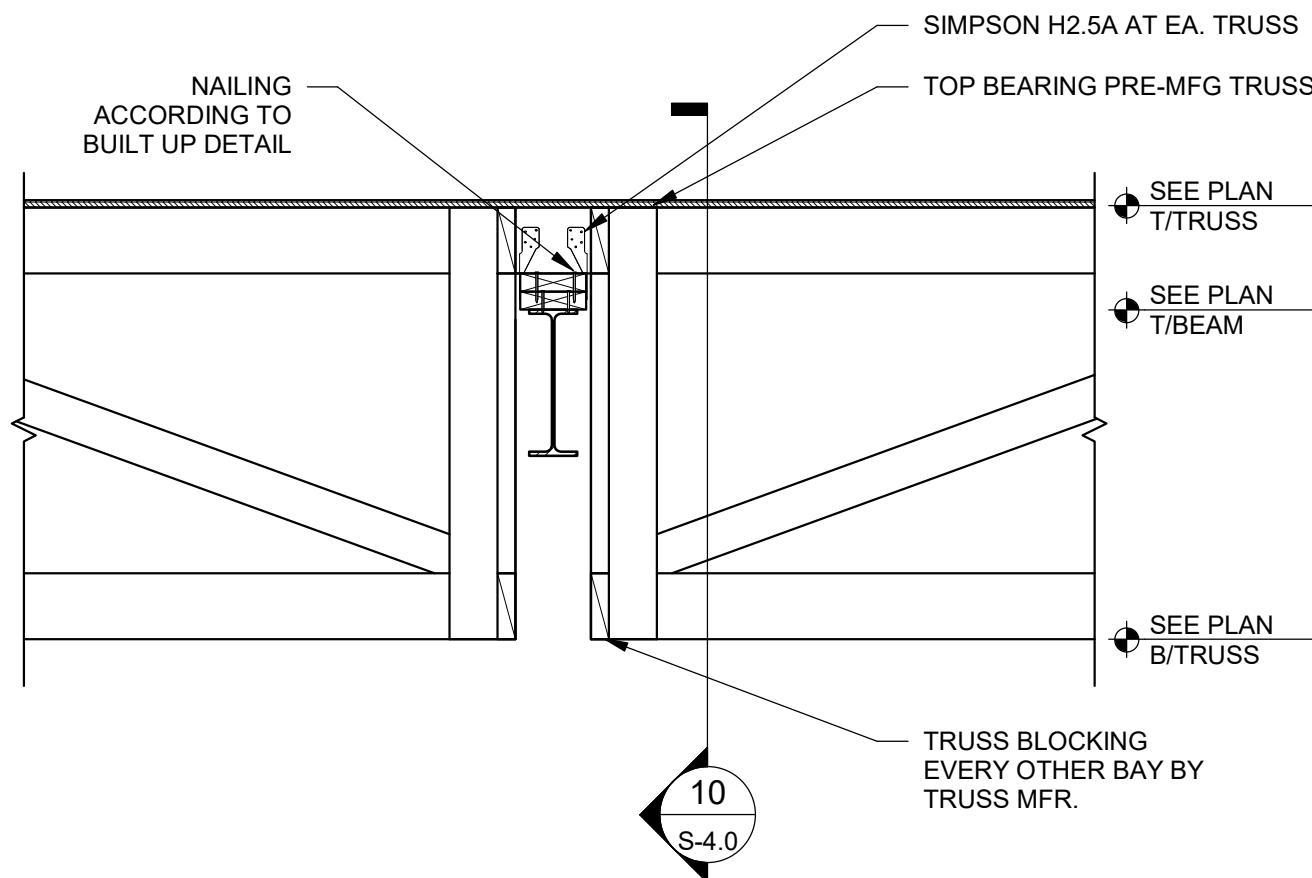
## 4 SECTION @ FURRING WALL

SCALE: 3/4" = 1'-0"



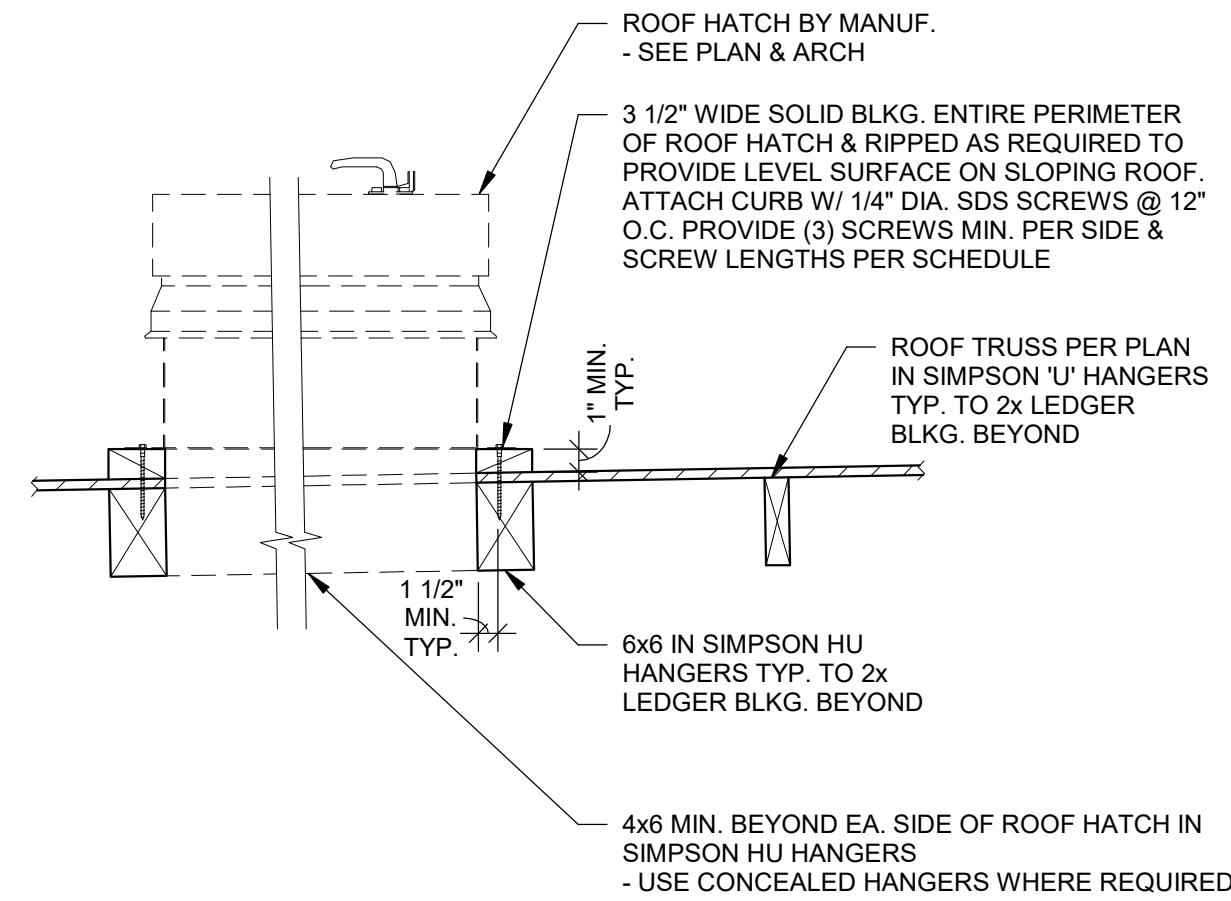
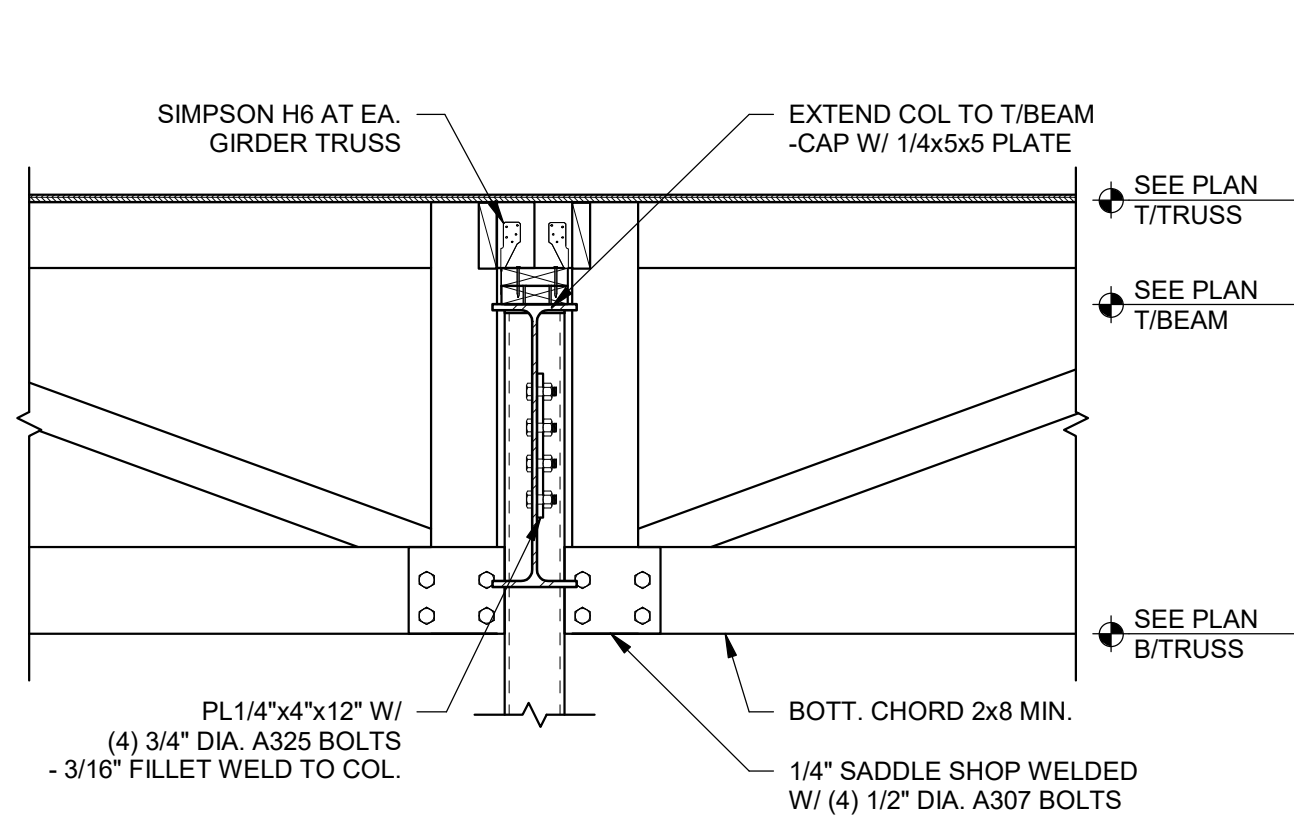
## 5 SECTION @ ENTRANCE

SCALE: 3/4" = 1'-0"



## 6 SECTION @ ENTRANCE

SCALE: 3/4" = 1'-0"



## 7 SECTION @ LIGHT

SCALE: 3/4" = 1'-0"



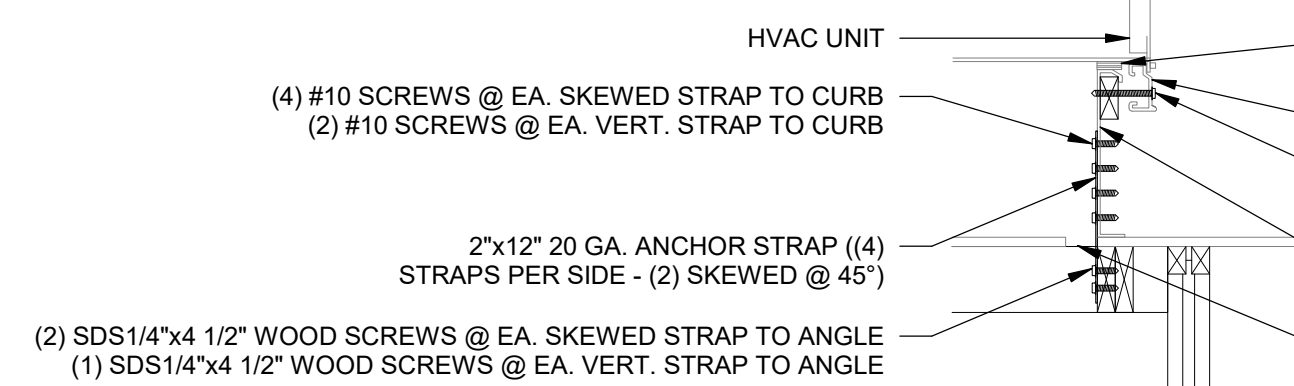
## 8 TYP. INT. TRUSS BRG.

SCALE: 3/4" = 1'-0"



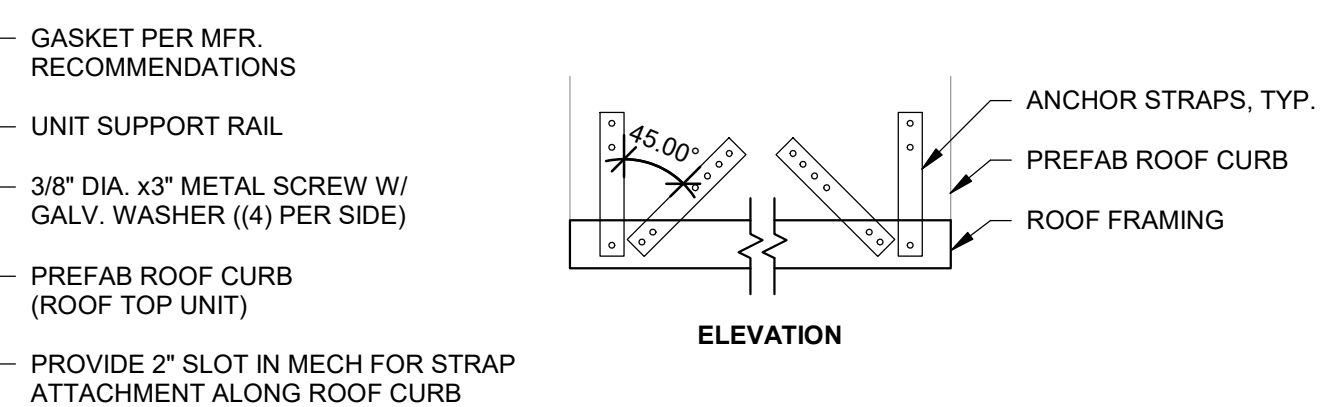
## 9 TYP. END COLUMN

SCALE: 3/4" = 1'-0"



## 10 SECTION @ ROOF HATCH

SCALE: 1" = 1'-0"



## 11 MECH UNIT SUPPORT

SCALE: 3/4" = 1'-0"

