In [10]:
```
#          ###########################################################################
#          #                        IN THE NAME OF ALLAH                             #
#          #        FREE VIBRATION  ANALYSIS OF INELASTIC SPRING WITH                #
#          #   INTIAL VELOCITY AND TWO SEISMIC ACCELERATION IN X AND Y DIRECTION     #
#          #                   AND SOIL-STRUCTURE INTERACTION                        #
#          #-------------------------------------------------------------------------#
#          #      THIS PROGRAM WRITTEN BY SALAR DELAVAR GHASHGHAEI (QASHQAI)         #
#          #                  EMAIL: salar.d.ghashghaei@gmail.com                    #
#          ###########################################################################
```

Modeling soil-structure interaction (SSI) with lateral and rotational springs in OpenSees involves defining the properties and behavior of the springs to simulate the interaction between the soil and structure. Considering the large variation of stiffness across various deposits or within the same soil, its required to conduct site or lab-tests to estimate a reliable value of stiffness (direct or coupled stiffness) under proper drainage condition. An SSI study with an incorrect stiffness would spoil the purpose of the study. Further, the designer need to have a good estimate of strain levels to assume a suitable value of damping. For soils exhibiting elastic or elastoplastic behavior, the stiffness and damping ratio can vary significantly. Here are some general estimates:

Stiffness (k in N/m)

- Clay soils: Stiffness can range from 5 MPa to 20 MPa (MegaPascals, equivalent to N/m²).
- Sandy soils: Stiffness can range from 10 MPa to 50 MPa.
- Gravelly soils: Stiffness can range from 20 MPa to 100 MPa.
- Stony soils: Stiffness can range from 200 MPa to 1500 MPa.

Damping Ratio (ζ)

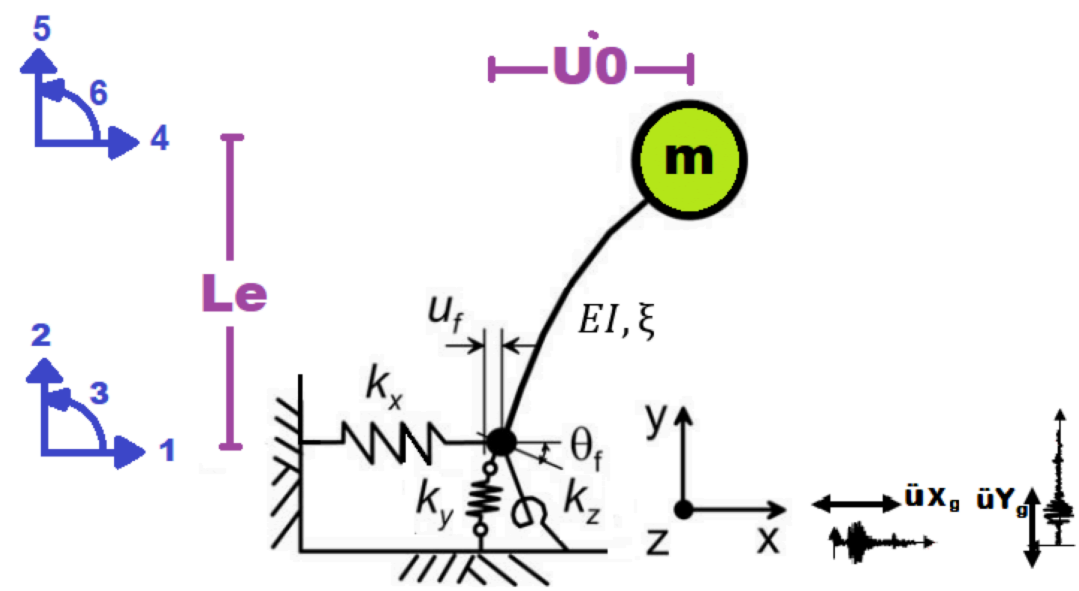
- Clay soils: Damping ratio typically ranges from 5% to 15%.
- Sandy soils: Damping ratio typically ranges from 2% to 10%.
- Gravelly soils: Damping ratio typically ranges from 1% to 5%.
- Stony soils: Damping ratio typically ranges from 0.5% to 3%.

These values are approximate and can vary based on factors such as moisture content, compaction, and the presence of organic material. For accurate assessments, it's best to conduct site-specific geotechnical investigations.

In [11]:
```
# REPORT: Soil-Structure Interaction for Building Structures
'https://www.nehrp.gov/pdf/nistgcr12-917-21.pdf'
```

Out[11]: 'https://www.nehrp.gov/pdf/nistgcr12-917-21.pdf'

In [12]:
```
# Load the image
def PLOT_IMAGE(image):
    import matplotlib.pyplot as plt
    import matplotlib.image as mpimg
    image = mpimg.imread(image_path)

    # Display the image
    plt.figure(figsize=(15, 8))
    plt.imshow(image)
    plt.axis('off')  # Hide axes
    plt.show()

image_path = 'OPENSEES_SOIL_STRUCTURE_INTERACTION_COLUMN_SEISMIC.png'
PLOT_IMAGE(image_path)
```



In [13]:
```
#import the os module
import os
import time
import numpy as np
import openseespy.opensees as op
import opsvis as opsv
import matplotlib.pyplot as plt
```

In [14]:
```
#to create a directory at specified path with name "Data"
os.mkdir('C:\\OPENSEESPY_SALAR')
```

```
---------------------------------------------------------------------------
FileExistsError                           Traceback (most recent call last)
Cell In[14], line 2
      1 #to create a directory at specified path with name "Data"
----> 2 os.mkdir('C:\\OPENSEESPY_SALAR')

FileExistsError: [WinError 183] Cannot create a file when that file already exists: 'C:\\OPENSEESPY_SALAR'
```

In [15]:
```python
FOLDER_NAME = 'SOIL_STRUCTURE_INTERACTION_COLUMN'
dir = f"C:\\OPENSEESPY_SALAR\\{FOLDER_NAME}\\"
if not os.path.exists(dir):
    os.makedirs(dir)
```

In [16]:
```python
# OUTPUT DATA ADDRESS:
SALAR_DIR = f'C://OPENSEESPY_SALAR//{FOLDER_NAME}//';
```

In [17]:
```python
## DELETE ALL FILES IN DIRECTORY
def DELETE_FOLDER_CONTANTS(folder_path):
    import os
    for filename in os.listdir(folder_path):
        file_path = os.path.join(folder_path, filename)
        try:
            if os.path.isfile(file_path) or os.path.islink(file_path):
                os.unlink(file_path)
        except Exception as e:
            print(f"Failed to delete {file_path}. Reason: {e}")
    print("Deletion done")

FOLDER_PATH = f'C:\\OPENSEESPY_SALAR\\{FOLDER_NAME}'  # Specify the folder path
#DELETE_FOLDER_CONTANTS(FOLDER_PATH)
```

In [18]:
```python
def PLOT_2D(X1, Y1, X2, Y2, XLABEL, YLABEL, TITLE):
    plt.figure(figsize=(10, 6))
    plt.plot(X1, Y1, label='Undamped', color='black')
    plt.plot(X2, Y2, label='Damped', color='red')
    plt.xlabel(XLABEL)
    plt.ylabel(YLABEL)
    plt.title(TITLE)
    plt.grid(True)
    #plt.semilogy()
    plt.legend()
    plt.show()

# ---------------------------------

def PLOT_SPRING():
    import matplotlib.pyplot as plt
    # Extract displacements and forces
    displacement_kh, force_kh = zip(*kh)
    displacement_kv, force_kv = zip(*kv)
    displacement_kr, force_kr = zip(*kr)

    # Plotting the force-displacement relation
    plt.figure(figsize=(15, 5))

    plt.subplot(1, 3, 1)
    plt.plot(displacement_kh, force_kh, label='Horizontal Spring', color='blue')
    plt.xlabel('Displacement (m)')
    plt.ylabel('Force (N)')
    plt.title('Horizontal Spring Force-Displacement Relation')
    plt.legend()

    plt.subplot(1, 3, 2)
    plt.plot(displacement_kv, force_kv, label='Vertical Spring', color='green')
    plt.xlabel('Displacement (m)')
    plt.ylabel('Force (N)')
    plt.title('Vertical Spring Force-Displacement Relation')
    plt.legend()

    plt.subplot(1, 3, 3)
    plt.plot(displacement_kr, force_kr, label='Rotational Spring', color='red')
    plt.xlabel('Rotation (rad)')
    plt.ylabel('Moment (N.mm)')
    plt.title('Rotational Spring Moment-Rotation Relation')
    plt.legend()

    plt.tight_layout()
    plt.show()

# ---------------------------------

def PLOT_4_CHART():
    # Plot the results
    plt.figure(figsize=(18, 28))
    # Displacement
    plt.subplot(7, 1, 1)
    P01 = displacement_undamped
    P02 = displacement_damped
    plt.plot(time_undamped, P01, label=f'Undamped: {np.max(np.abs(P01)):.5e}', color='black')
    plt.plot(time_damped, P02, label=f'Damped: {np.max(np.abs(P02)):.5e}', color='red')
    plt.xlabel('Time [s]')
    plt.ylabel('Displacement ')
    plt.title('Displacement Time History')
    plt.legend()
    # Velocity
    plt.subplot(7, 1, 2)
    P01 = velocity_undamped
    P02 = velocity_damped
    plt.plot(time_undamped, P01, label=f'Undamped: {np.max(np.abs(P01)):.5e}', color='black')
    plt.plot(time_damped, P02, label=f'Damped: {np.max(np.abs(P02)):.5e}', color='red')
    plt.xlabel('Time [s]')
    plt.ylabel('Velocity')
    plt.title('Velocity Time History')
    plt.legend()
    # Acceleration
    plt.subplot(7, 1, 3)
    P01 = acceleration_undamped
    P02 = acceleration_damped
    plt.plot(time_undamped, P01, label=f'Undamped: {np.max(np.abs(P01)):.5e}', color='black')
    plt.plot(time_damped, P02, label=f'Damped: {np.max(np.abs(P02)):.5e}', color='red')
    plt.xlabel('Time [s]')
```

```python
    plt.ylabel('Acceleration')
    plt.title('Acceleration Time History')
    plt.legend()
    # Spring Force - SHEAR
    plt.subplot(7, 1, 4)
    P01 = spring_force_H_undamped
    P02 = spring_force_H_damped
    plt.plot(time_undamped, P01, label=f'Undamped: {np.max(np.abs(P01)):.5e}', color='black')
    plt.plot(time_damped, P02, label=f'Damped: {np.max(np.abs(P02)):.5e}', color='red')
    plt.xlabel('Time [s]')
    plt.ylabel('Horizental Axial Spring Force')
    plt.title('Spring Force Time History')
    plt.legend()
    # Spring Force - AXIAL
    plt.subplot(7, 1, 5)
    P01 = spring_force_V_undamped
    P02 = spring_force_V_damped
    plt.plot(time_undamped, P01, label=f'Undamped: {np.max(np.abs(P01)):.5e}', color='black')
    plt.plot(time_damped, P02, label=f'Damped: {np.max(np.abs(P02)):.5e}', color='red')
    plt.xlabel('Time [s]')
    plt.ylabel('Vertical Axial Spring Force')
    plt.title('Spring Force Time History')
    plt.legend()
    # Spring Force - MOMENT
    plt.subplot(7, 1, 6)
    P01 = spring_force_M_undamped
    P02 = spring_force_M_damped
    plt.plot(time_undamped, P01, label=f'Undamped: {np.max(np.abs(P01)):.5e}', color='black')
    plt.plot(time_damped, P02, label=f'Damped: {np.max(np.abs(P02)):.5e}', color='red')
    plt.xlabel('Time [s]')
    plt.ylabel('Moment Spring Force')
    plt.title('Spring Force Time History')
    plt.legend()
    # Element Force
    plt.subplot(7, 1, 7)
    P01 = ele_force_undamped
    P02 = ele_force_damped
    plt.plot(time_undamped, P01, label=f'Undamped: {np.max(np.abs(P01)):.5e}', color='black')
    plt.plot(time_damped, P02, label=f'Damped: {np.max(np.abs(P02)):.5e}', color='red')
    plt.xlabel('Time [s]')
    plt.ylabel('Element Force')
    plt.title('Element Force Time History')
    plt.legend()
    # Display the plot
    plt.show()

# ---------------------------------

def SECTION_REC(sectionID, matID, B, H, numY, numZ):
    # numY , numZ = 1000, 1
    # Define fiber rectangular section
    op.section('Fiber', sectionID)
    #patch rect $matTag $numSubdivY $numSubdivZ $yI $zI $yJ $zJ
    op.patch('rect', matID, numY, numY, -0.5*H, -B, 0.5*H, B)  # Core

# --------------------

"""
When OK equals -1, it generally indicates that the command or operation was not executed
because it was already in progress or had already been completed. This can happen if you
try to run a command that is already running or has been completed in a previous step.

When OK equals -2, it typically indicates that the command or operation was not executed
because it was not recognized or not implemented. This could mean that the command
is either misspelled, not available in the current version of OpenSees, or not applicable to the current context.

When OK equals -3, it typically means that the command or operation failed.
This could be due to various reasons, such as incorrect input parameters,
syntax errors, or issues with the model setup.
"""
def ANALYSIS(OK, INCREMENT, TOLERANCE, MAX_ITERAIONS):
    import openseespy.opensees as op
    test = {1:'NormDispIncr', 2: 'RelativeEnergyIncr', 4: 'RelativeNormUnbalance',5: 'RelativeNormDispIncr', 6: 'NormUnbalance'}
    algorithm = {1:'KrylovNewton', 2: 'SecantNewton' , 4: 'RaphsonNewton',5: 'PeriodicNewton', 6: 'BFGS', 7: 'Broyden', 8: 'NewtonLineSearch'}

    for i in test:
        for j in algorithm:
            if OK != 0:
                if j < 4:
                    op.algorithm(algorithm[j], '-initial')

                else:
                    op.algorithm(algorithm[j])

                op.test(test[i], TOLERANCE, MAX_ITERAIONS)
                OK = op.analyze(INCREMENT)
                print(test[i], algorithm[j], OK)
                if OK == 0:
                    break
            else:
                continue

# --------------------
```

In [19]:

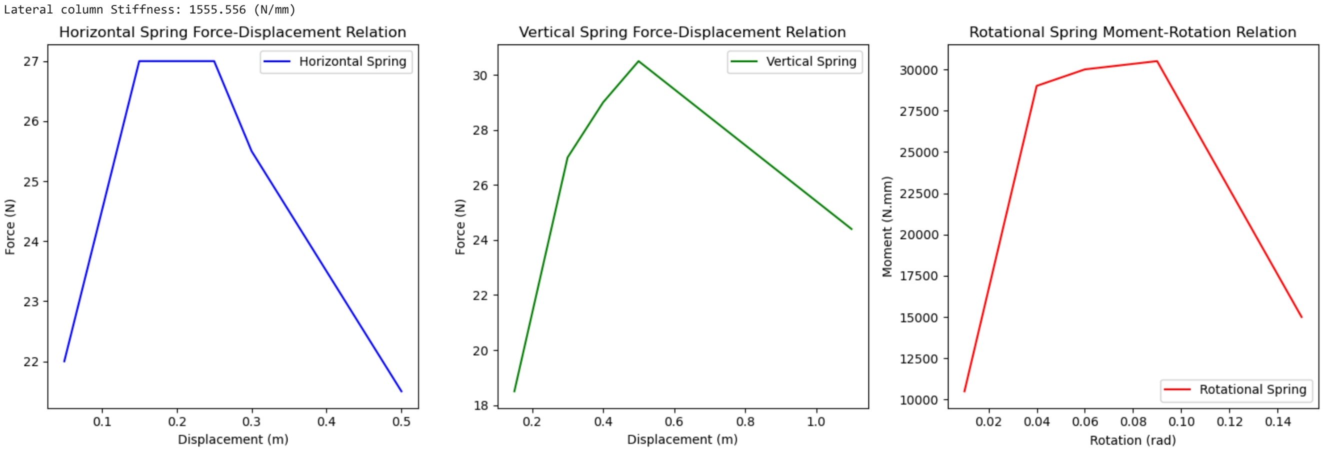
```python
import openseespy.opensees as ops
import numpy as np
import matplotlib.pyplot as plt

# Set up variables
Ea = 210.0e3  # [N/mm^2] Modulus of elasticity (Young's modulus)
B = 100 # [mm] Section Width
H = 200 # [mm] Section Height
Le = 3000.0  # [mm] Length of the column element
A = B * H # [mm^2] Cross-sectional area of the element
I = (B * H**3) / 12 # [mm^4]
KE = (3 * Ea * I) / Le**3 # [N/mm] Lateral column Effective Stiffness
print(f'Lateral column Stiffness: {KE:.3f} (N/mm)')
iv0 = 0.005                   # [mm] Initial velocity applied to the node
st_iv0 = 0.0                  # [s] Initial velocity applied starting time
damping_ratio = 0.05          # Damping ratio
soil_damping_ratio = 0.015    # Damping ratio for Stony soils
MS = 10                       # [kg] mass values for node 1
```

```
# Define masses for multiple nodes
mass_values = [100.0, 500.0]  # mass values for node 2

# Set analysis parameters
duration = 50.0 # [s] 50 Seconds
dt = 0.01

# Define ReinforcingSteel material properties
fy_m = 400.0    # [N/mm^2] Yield strength of steel
Es_m = 210.0e3  # [N/mm^2] Modulus of elasticity
fu_m = 600.0    # [N/mm^2] Ultimate strength
Esh_m = 20.0e3  # [N/mm^2] Hardening modulus
esh_m = 0.03    # [mm/mm] Strain at start of hardening
esu_m = 0.1     # [mm/mm] Ultimate strain

# Section Fibers
numY , numZ = 1000, 1

# Trigonometric (Harmonic) loading parameters
START_TIME = 2.0
END_TIME = 12.0
PERIOD = 3.2
LOAD_FACTOR = 15.0 # Amplitude Factor

MAX_ITERATIONS = 5000 # convergence iteration for test
TOLERANCE = 1.0e-10   # convergence tolerance for test

# Define soil-structure interaction springs at base (Node 1)
# ELASTIC SOIL & ELASTIC-NO TENSION SOIL:
# Spring constants for soil-structure interaction
k_horizontal = 210.0e4  # Horizontal spring constant
k_vertical = 150.0e4  # Vertical spring constant
k_rotational = 550.0e4  # Rotational spring constant

# ELASTO-PLASTIC SOIL:
# Horizontal spring parameters
fy_h = 400.0      # [N] Yield force
Es_h = 21000.0    # [N/mm] Lateral Siffness in Horizental Direction
fu_h = 410.0      # [N] Ultimate force
Esh_h = 200.0     # [N/mm] Hardening modulus
esh_h = 0.4       # [mm] Displacement at start of hardening
esu_h = 1.1       # [mm] Ultimate displacement

# Vertical spring parameters
fy_v = 450.0      # [N] Yield force
Es_v = 20000.0    # [N/mm] Lateral Siffness in Vertical Direction
fu_v = 460.0      # [N] Ultimate force
Esh_v = 180       # [N/mm] Hardening modulus
esh_v = 0.5       # [mm] Displacement at start of hardening
esu_v = 1.8         # [mm] Ultimate displacement

# Rotational spring parameters
fy_r = 50e3       # [N.mm] Yield force
Es_r = 50e5       # [N/mm/rad] Rotational Siffness in Horizental Direction
fu_r = 70e3       # [N.mm] Ultimate force
Esh_r = 50e4      # [N/mm/rad] Hardening modulus
esh_r = 0.06      # [Rad] Rotation at start of hardening
esu_r = 0.12      # [Rad] Ultimate rotation

# Define the MultiLinear material properties for springs
kh = np.array([[0.05, 22.0], [0.15, 27.0], [0.25, 27.0], [0.3, 25.5], [0.5, 21.5]])                   # k horizontal
kv = np.array([[0.15, 18.5], [0.3, 27.0], [0.4, 29.0], [0.5, 30.5], [1.1, 24.4]])                     # k vertical
kr = np.array([[0.01, 10500.0], [0.04, 29000.0], [0.06, 30000.0], [0.09, 30500.0], [0.15, 15000.0]])  # k rotational

# Create a DataFrame
import pandas as pd
data = {
    "Displacement_X": kh[:, 0],
    "Force (kh)": kh[:, 1],
    "Displacement_Y": kv[:, 0],
    "Force (kv)": kv[:, 1],
    "Rotation_Z": kr[:, 0],
    "Force (kr)": kr[:, 1]
}

#df = pd.DataFrame(data)
PLOT_SPRING()

def INTERACTION_ANALYSIS(MASS_ONE, DAMPING, SOIL_DAMPING, periodTF, LINEAR, SPRING_KIND):
    import openseespy.opensees as ops
    import numpy as np

    KE = (3 * Ea * I) / Le**3; # [N/mm] Lateral column Effective Stiffness
    #GMfact = 9810 # standard acceleration of gravity or standard acceleration
    GMfact = 1
    # Set up the model
    ops.wipe()
    ops.model('basic', '-ndm', 2, '-ndf', 3)

    # Define nodes
    ops.node(1, 0, 0)
    ops.node(2, 0, Le)
    ops.node(200, 0, 0)

    # Define boundary conditions (modified to include springs)
    ops.fix(200, 1, 1, 1)  # Node 200 is Fixed in all directions initially

    # Assign masses to nodes
    if MASS_ONE == True:
        ops.mass(2, mass_values[1], mass_values[1], 0.0)
        print('ONE MASS')

    if MASS_ONE == False:
        ops.mass(1, mass_values[0], mass_values[0], 0.0)
        ops.mass(2, mass_values[1], mass_values[1], 0.0)
        print('TWO MASSES')

    # Define material
    matID = 1
    if LINEAR == True:
        ops.uniaxialMaterial('Elastic', matID, Es_m)
```

```python
    if LINEAR == False:
        ops.uniaxialMaterial('ReinforcingSteel', 1, fy_m, Es_m, fu_m, Esh_m, esh_m, esu_m)

    # Define fiber rectangular section
    sectionID = 1
    ops.section('Fiber', sectionID)
    #patch rect $matTag $numSubdivY $numSubdivZ $yI $zI $yJ $zJ
    ops.patch('rect', matID, numY, numZ, -0.5*H, -B, 0.5*H, B)  # Core

    # Define geometric transformation
    ops.geomTransf('Linear', 1)

    # Define elements
    # element nonlinearBeamColumn $eleTag $iNode $jNode $numIntgrPts $secTag $transfTag
    eleID =  1
    ops.element('nonlinearBeamColumn', eleID, 1, 2, 5, sectionID, 1)

    # OUTPUT DATA
    ops.recorder('Node', '-file', f"{SALAR_DIR}DTH_DYN.txt",'-time', '-node', 2, '-dof', 1,2,3, 'disp')# Displacement Time History Node 2
    ops.recorder('Node', '-file', f"{SALAR_DIR}BTH_DYN.txt",'-time', '-node', 1, '-dof', 1,2,3, 'reaction')# Base Shear Time History Node 1
    ops.recorder('Element', '-file', f"{SALAR_DIR}DEF_DYN.txt",'-time', '-ele', 1, 'section', 5, 'deformations')# Curvature Time History

    # Define soil-structure interaction springs at base (Node 1)
    if SPRING_KIND == 1:
        # Elastic Material:
        ops.uniaxialMaterial('Elastic', 2, k_horizontal)  # Horizontal spring
        ops.uniaxialMaterial('Elastic', 3, k_vertical)    # Vertical spring
        ops.uniaxialMaterial('Elastic', 4, k_rotational)  # Rotational spring
        KS = k_horizontal # Soil Lateral Stiffness
    if SPRING_KIND == 2:
        # Define horizontal spring
        ops.uniaxialMaterial('ReinforcingSteel', 2, fy_h, Es_h, fu_h, Esh_h, esh_h, esu_h)
        # Define vertical spring
        ops.uniaxialMaterial('ReinforcingSteel', 3, fy_v, Es_v, fu_v, Esh_v, esh_v, esu_v)
        # Define rotational spring
        ops.uniaxialMaterial('ReinforcingSteel', 4, fy_r, Es_r, fu_r, Esh_r, esh_r, esu_r)
        KS = Es_h # Soil Lateral Stiffness
    if SPRING_KIND == 3:
        # Elastic-No Tension Material:
        ops.uniaxialMaterial('ENT', 2, k_horizontal)  # Horizontal spring
        ops.uniaxialMaterial('ENT', 3, k_vertical)    # Vertical spring
        ops.uniaxialMaterial('ENT', 4, k_rotational)  # Rotational spring
        KS = k_horizontal # Soil Lateral Stiffness
    if SPRING_KIND == 4:
        # Define soil-structure interaction springs at base (Node 1)
        #                                       STRAIN    STRESS
        ops.uniaxialMaterial('MultiLinear', 2, *kh.flatten()) # Horizontal spring
        ops.uniaxialMaterial('MultiLinear', 3, *kh.flatten()) # Vertical spring
        ops.uniaxialMaterial('MultiLinear', 4, *kh.flatten()) # Rotational spring
        KS = kh[0][1] / kh[0][0] # Soil Lateral Stiffness


    # Soil Natural frequency (rad/s)
    wns = (KS / MS) ** 0.5
    # Soil Damping coefficient (Ns/m)
    CS = 2 * wns * MS * soil_damping_ratio if SOIL_DAMPING else 0.0
    # Define materials for Soil damper
    ops.uniaxialMaterial('Elastic', 5, 0.0, CS)

    if SOIL_DAMPING == False:
        ops.element('zeroLength', 2, 200, 1, '-mat', 2, '-dir', 1) # Horizontal spring for soil stiffness
        ops.element('zeroLength', 3, 200, 1, '-mat', 3, '-dir', 2) # Vertical spring for soil stiffness
        ops.element('zeroLength', 4, 200, 1, '-mat', 4, '-dir', 3) # Rotational spring for soil stiffness
    if SOIL_DAMPING == True:
        ops.element('zeroLength', 2, 200, 1, '-mat', 2, 5, '-dir', 1, 1) # Horizontal spring for soil stiffness and damping
        ops.element('zeroLength', 3, 200, 1, '-mat', 3, 5, '-dir', 2, 2) # Vertical spring for soil stiffness and damping
        ops.element('zeroLength', 4, 200, 1, '-mat', 4, '-dir', 3) # Rotational spring for soil stiffness

    # Dynamic analysis setup
    ops.constraints('Transformation')
    ops.numberer('RCM')
    ops.system('UmfPack')
    ops.test('EnergyIncr', TOLERANCE, MAX_ITERATIONS)
    #ops.integrator('CentralDifference')
    #ops.integrator('HHT', 0.9)
    ops.integrator('Newmark', 0.5, 0.25)
    ops.algorithm('ModifiedNewton')

    # Define analysis type
    ops.analysis('Transient')

    # Define time series for input motion (Acceleration time history)
    ops.timeSeries('Path', 1, '-dt', 0.01, '-filePath', 'OPENSEES_SPRING_SEISMIC_01.txt', '-factor', GMfact, '-startTime', st_iv0) # SEISMIC-X
    ops.timeSeries('Path', 2, '-dt', 0.01, '-filePath', 'OPENSEES_SPRING_SEISMIC_02.txt', '-factor', GMfact) # SEISMIC-Y

    # Define load patterns
    # pattern UniformExcitation $patternTag $dof -accel $tsTag <-vel0 $vel0> <-fact $cFact>
    ops.pattern('UniformExcitation', 1, 1, '-accel', 1, '-vel0', iv0, '-fact', 1.0) # SEISMIC-X
    ops.pattern('UniformExcitation', 2, 2, '-accel', 2)                             # SEISMIC-Y

    # Perform eigenvalue analysis to determine modal periods
    if periodTF == True:
        #eigenvalues01 = ops.eigen('-genBandArpack', 1) # eigenvalue mode 1
        eigenvalues01 = ops.eigen('-fullGenLapack', 1) # eigenvalue mode 1
        #eigenvalues01 = ops.eigen('-symmBandLapack', 1) # eigenvalue mode 1
        #Omega = np.power(eigenvalues01, 0.5)
        #Omega = np.power(max(min(eigenvalues01), min(eigenvalues02), min(eigenvalues03)), 0.5)
        Omega = np.power(min(eigenvalues01), 0.5)
        modal_period = 2 * np.pi / np.sqrt(Omega)  # [Second]
        frequency = 1 / modal_period               # [Hertz]

    if periodTF == False:
        modal_period = 0.0            # [Second]
        frequency = 0.0               # [Hertz]


    # Dynamic analysis
    ops.constraints('Transformation')
    ops.numberer('RCM')
    ops.system('UmfPack')
    op.test('EnergyIncr', TOLERANCE, MAX_ITERATIONS) # determine if convergence has been achieved at the end of an iteration step
    #ops.integrator('CentralDifference') # Central Difference Method
```

```python
    ops.integrator('HHT', 0.9) # Hilber-Hughes-Taylor Method
    #ops.integrator('Newmark', 0.5, 0.25) # Newmark Method
    ops.algorithm('ModifiedNewton') # use Newton's solution algorithm: updates tangent stiffness at every iteration

    if DAMPING == True:
        # Calculate Rayleigh damping factors
        omega1 = np.sqrt(KE / mass_values[1])
        omega2 = 2 * omega1  # Just an assumption for two modes
        a0 = damping_ratio * (2 * omega1 * omega2) / (omega1 + omega2)
        a1 = damping_ratio * 2 / (omega1 + omega2)
        # Apply Rayleigh damping
        ops.rayleigh(a0, a1, 0, 0)

    ops.analysis('Transient')

    # Perform transient analysis and store results
    time = []
    displacement = []
    velocity = []
    acceleration = []
    spring_force_H = []
    spring_force_V = []
    spring_force_M= []
    ele_force = []

    stable = 0
    current_time = 0.0

    while stable == 0 and current_time < duration:
        stable = ops.analyze(1, dt)
        if SPRING_KIND != 3 or SPRING_KIND != 4: # if Elastic-No Tension Material
            ANALYSIS(stable, 1, TOLERANCE, MAX_ITERATIONS) # CHECK THE ANALYSIS
        current_time = ops.getTime()
        time.append(current_time)
        displacement.append(ops.nodeDisp(2, 1))
        velocity.append(ops.nodeVel(2, 1))
        acceleration.append(ops.nodeAccel(2, 1))
        spring_force_H.append(-ops.eleResponse(2, 'force')[0]) # SOIL REACTION - SHEAR
        spring_force_V.append(-ops.eleResponse(3, 'force')[1]) # SOIL REACTION - AXIAL
        spring_force_M.append(-ops.eleResponse(4, 'force')[2]) # SOIL REACTION - MOMENT
        ele_force.append(-ops.eleResponse(1, 'force')[0])      # COLUMN REACTION
        KE =  ele_force[-1] / displacement[-1] # Effective Lateral Stiffness

    print("Period: ", modal_period)
    print("Frequency: ", frequency)
    if DAMPING == False:
        print('Undamping Structure Dynamic Analysis Done.')
    if DAMPING == True:
        print('Damping Structure Dynamic Analysis Done.')

    return modal_period, time, displacement, velocity, acceleration, spring_force_H, spring_force_V, spring_force_M, ele_force
```

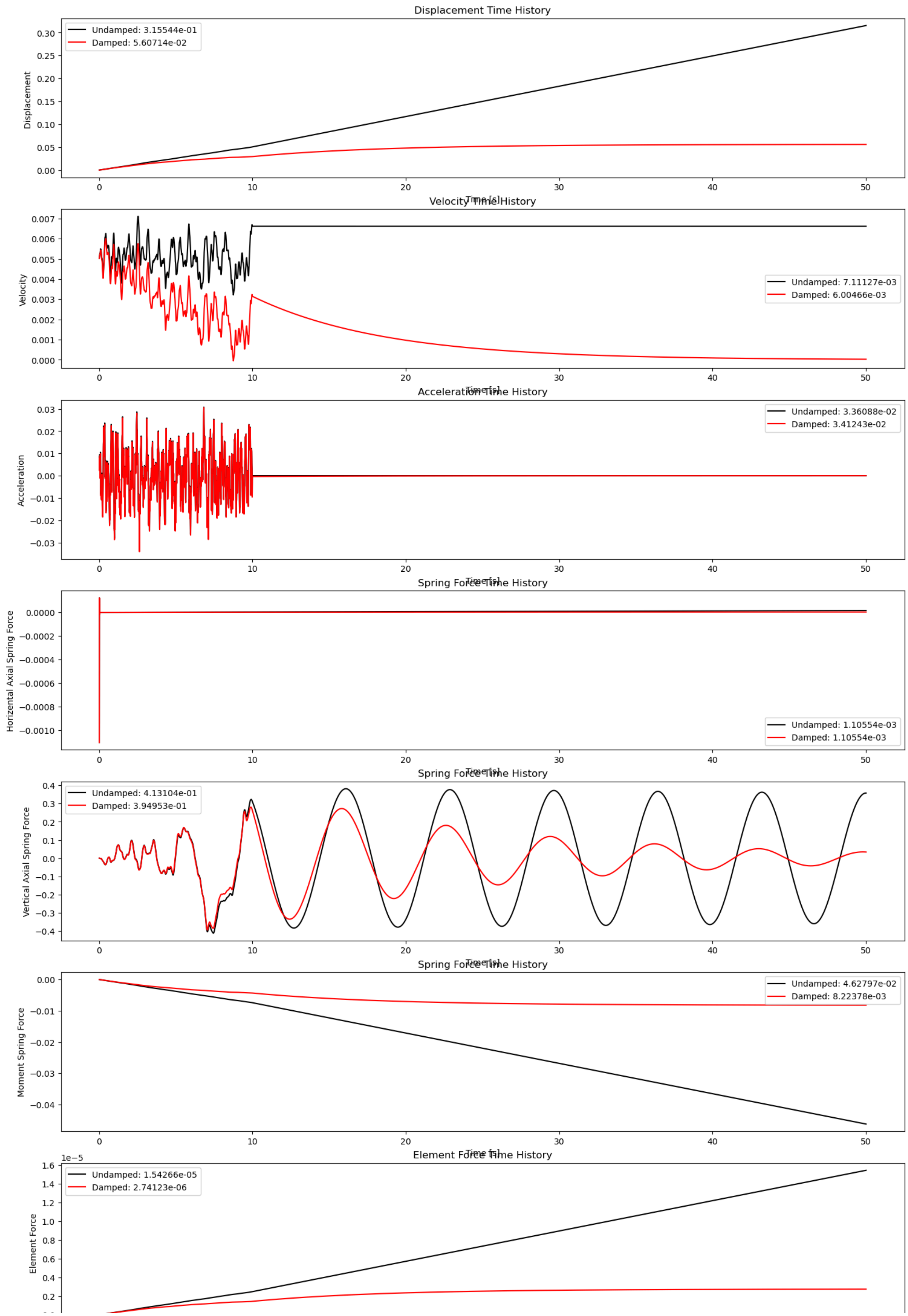
```
Lateral column Stiffness: 1555.556 (N/mm)
```



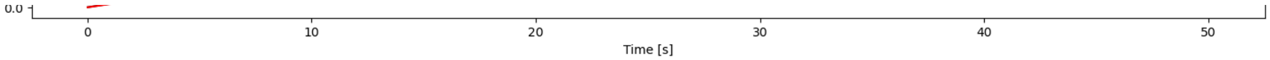
In [20]:
```python
### SOIL STRUCTURE INTERACTION WITH ONE MASS:
starttime = time.process_time()

# Perform analysis
undamped_period, time_undamped, displacement_undamped, velocity_undamped, acceleration_undamped, spring_force_H_undamped, spring_force_V_undamped, spring_force_M_undamped, ele_force_ur
damped_period, time_damped, displacement_damped, velocity_damped, acceleration_damped, spring_force_H_damped, spring_force_V_damped, spring_force_M_damped, ele_force_damped = INTERACTI

totaltime = time.process_time() - starttime
print(f'\nTotal time (s): {totaltime:.4f} \n\n')
```

```
ONE MASS
WARNING can't set handler after analysis is created
Period:  0.0
Frequency:  0.0
Undamping Structure Dynamic Analysis Done.
ONE MASS
WARNING can't set handler after analysis is created
Period:  0.0
Frequency:  0.0
Damping Structure Dynamic Analysis Done.

Total time (s): 76.2344
```

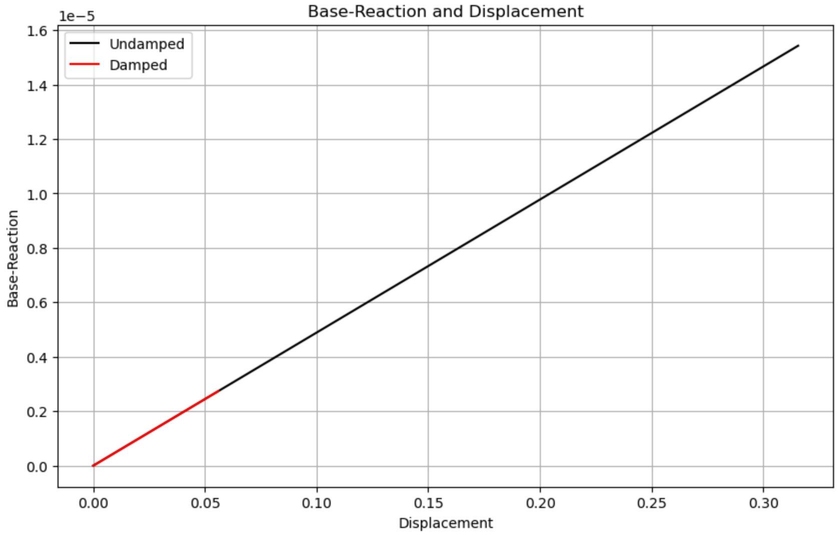
In [21]:
```python
### ONE MASS TIME HISTORY:
PLOT_4_CHART()
```



Displacement Time History
Undamped: 3.15544e-01
Damped: 5.60714e-02
Displacement
Time [s]
Velocity Time History
Undamped: 7.11127e-03
Damped: 6.00466e-03
Velocity
Time [s]
Acceleration Time History
Undamped: 3.36088e-02
Damped: 3.41243e-02
Acceleration
Time [s]
Spring Force Time History
Undamped: 1.10554e-03
Damped: 1.10554e-03
Horizental Axial Spring Force
Time [s]
Spring Force Time History
Undamped: 4.13104e-01
Damped: 3.94953e-01
Vertical Axial Spring Force
Time [s]
Spring Force Time History
Undamped: 4.62797e-02
Damped: 8.22378e-03
Moment Spring Force
Time [s]
Element Force Time History
1e−5
Undamped: 1.54266e-05
Damped: 2.74123e-06
Element Force

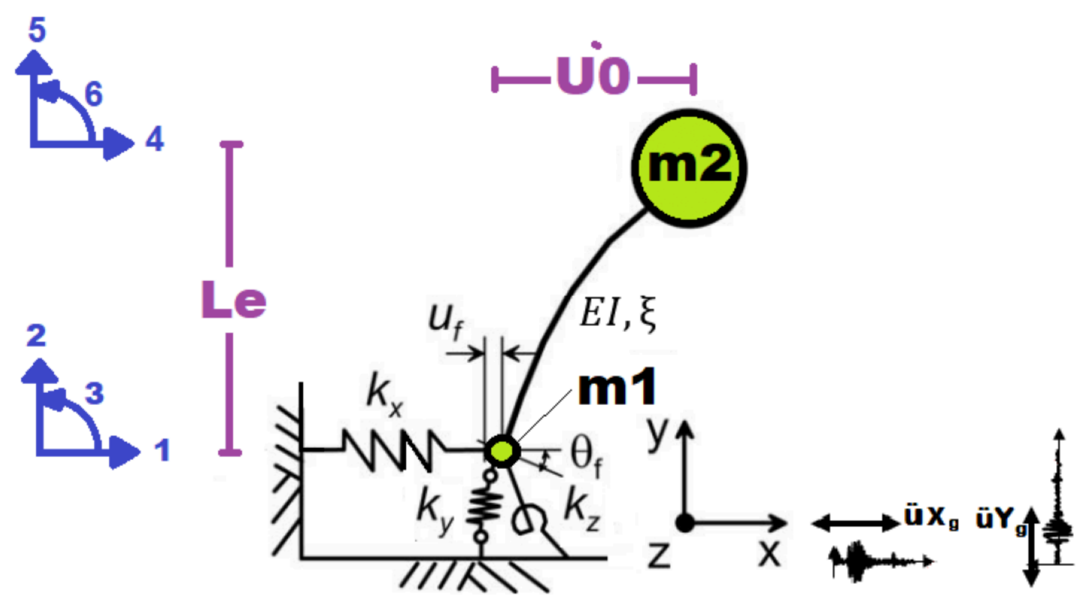


In [22]:
```
### ONE MASS BASE REACTION & DISPALCEMENT:
X1 = displacement_undamped
X2 = displacement_damped
Y1 = ele_force_undamped
Y2 = ele_force_damped
XLABEL = 'Displacement'
YLABEL = 'Base-Reaction'
TITLE = 'Base-Reaction and Displacement'

PLOT_2D(X1, Y1, X2, Y2, XLABEL, YLABEL, TITLE)
```



In [23]:
```
#          ####################################################################################
#          #                             IN THE NAME OF ALLAH                                 #
#          #      FREE VIBRATION ANALYSIS OF INELASTIC SPRING WITH TRIGONOMETRIC LOADING       #
#          #                   AND SOIL-STRUCTURE INTERACTION AND TWO MASSES                   #
#          #----------------------------------------------------------------------------------#
#          #              THIS PROGRAM WRITTEN BY SALAR DELAVAR GHASHGHAEI (QASHQAI)           #
#          #                        EMAIL: salar.d.ghashghaei@gmail.com                        #
#          ####################################################################################
```

In [24]:
```
# Load the image
image_path = 'OPENSEES_SOIL_STRUCTURE_INTERACTION_COLUMN_SEISMIC_MASS02.png'
PLOT_IMAGE(image_path)
```



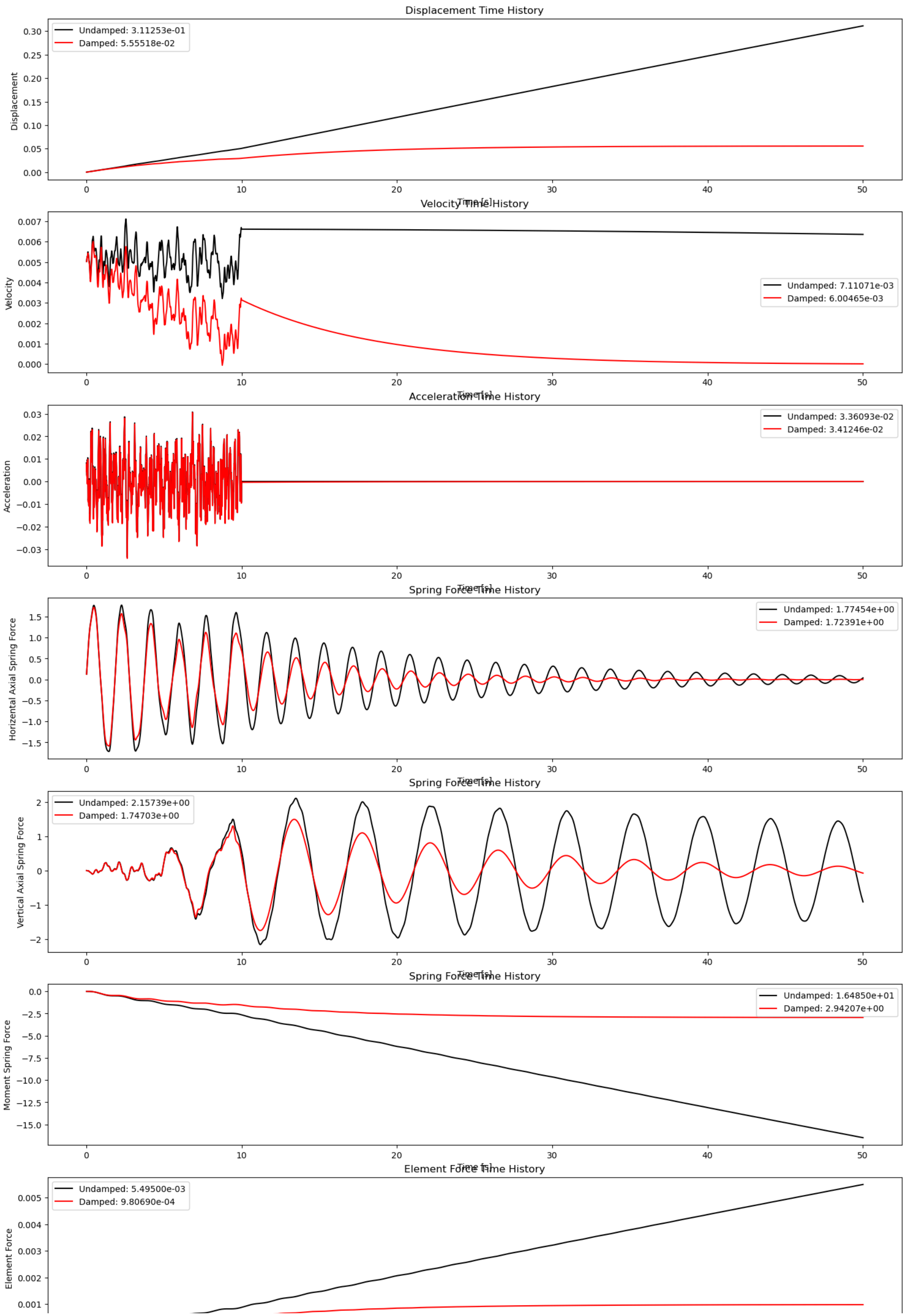
In [25]:
```
### SOIL STRUCTURE INTERACTION WITH TWO MASSES:
starttime = time.process_time()

# Perform analysis
undamped_period, time_undamped, displacement_undamped, velocity_undamped, acceleration_undamped, spring_force_H_undamped, spring_force_V_undamped, spring_force_M_undamped, ele_force_ur
damped_period, time_damped, displacement_damped, velocity_damped, acceleration_damped, spring_force_H_damped, spring_force_V_damped, spring_force_M_damped, ele_force_damped = INTERACTI

totaltime = time.process_time() - starttime
print(f'\nTotal time (s): {totaltime:.4f} \n\n')
```

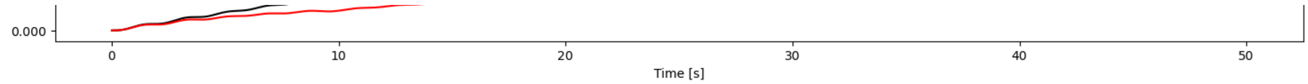
```
TWO MASSES
WARNING can't set handler after analysis is created
```

```
Period:  0.0
Frequency:  0.0
Undamping Structure Dynamic Analysis Done.
TWO MASSES
WARNING can't set handler after analysis is created
Period:  0.0
Frequency:  0.0
Damping Structure Dynamic Analysis Done.

Total time (s): 75.1250
```

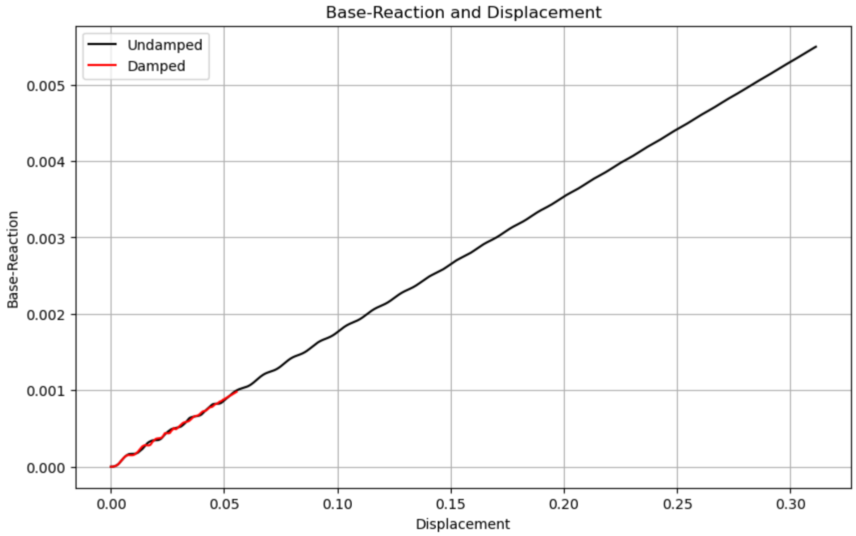
In [26]:

```
### TWO MASSES TIME HISTORY:
PLOT_4_CHART()
```


Displacement Time History
Undamped: 3.11253e-01
Damped: 5.55518e-02
Displacement
Time [s]
Velocity Time History
Undamped: 7.11071e-03
Damped: 6.00465e-03
Velocity
Time [s]
Acceleration Time History
Undamped: 3.36093e-02
Damped: 3.41246e-02
Acceleration
Time [s]
Spring Force Time History
Undamped: 1.77454e+00
Damped: 1.72391e+00
Horizontal Axial Spring Force
Time [s]
Spring Force Time History
Undamped: 2.15739e+00
Damped: 1.74703e+00
Vertical Axial Spring Force
Time [s]
Spring Force Time History
Undamped: 1.64850e+01
Damped: 2.94207e+00
Moment Spring Force
Time [s]
Element Force Time History
Undamped: 5.49500e-03
Damped: 9.80690e-04
Element Force



In [27]:
```
### TWO MASSES BASE REACTION & DISPALCEMENT:
X1 = displacement_undamped
X2 = displacement_damped
Y1 = ele_force_undamped
Y2 = ele_force_damped
XLABEL = 'Displacement'
YLABEL = 'Base-Reaction'
TITLE = 'Base-Reaction and Displacement'

PLOT_2D(X1, Y1, X2, Y2, XLABEL, YLABEL, TITLE)
```



In [ ]:

In [ ]: