

In [11]:

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# #####
#                                     IN THE NAME OF ALLAH                                     #
#                                     DYNAMIC TIME HISTORY ANALYSIS                               #
# # EXTERNAL OBJECT CONTACT EFFECTS WITH SOIL-STRUCTURE INTERACTIONS VIA THE SPRING METHOD #
# -----
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# #####
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Utilizing OpenSees for External Object Contact Effects with Soil-Structure Interaction via the Spring Method:

Modeling soil-structure interaction (SSI) with lateral and rotational springs in OpenSees involves defining the properties and behavior of the springs to simulate the interaction between the soil and structure. Considering the large variation of stiffness across various deposits or within the same soil, its required to conduct site or lab-tests to estimate a reliable value of stiffness (direct or coupled stiffness) under proper drainage condition. An SSI study with an incorrect stiffness would spoil the purpose of the study. Further, the designer need to have a good estimate of strain levels to assume a suitable value of damping. For soils exhibiting elastic or elastoplastic behavior, the stiffness and damping ratio can vary significantly. Here are some general estimates:

Stiffness (k in N/m)

- Clay soils: Stiffness can range from 5 MPa to 20 MPa (MegaPascals, equivalent to N/m²).
- Sandy soils: Stiffness can range from 10 MPa to 50 MPa.
- Gravelly soils: Stiffness can range from 20 MPa to 100 MPa.
- Stony soils: Stiffness can range from 200 MPa to 1500 MPa.

Damping Ratio (ζ)

- Clay soils: Damping ratio typically ranges from 5% to 15%.
- Sandy soils: Damping ratio typically ranges from 2% to 10%.
- Gravelly soils: Damping ratio typically ranges from 1% to 5%.
- Stony soils: Damping ratio typically ranges from 0.5% to 3%.

These values are approximate and can vary based on factors such as moisture content, compaction, and the presence of organic material. For accurate assessments, it's best to conduct site-specific geotechnical investigations.

Contact problems in finite element analysis (FEA) are quite fascinating and complex! They involve the interaction between two or more bodies that come into contact with each other. Here's a brief overview:

What are Contact Problems?

Contact problems occur when two or more bodies come into contact and interact with each other. This interaction can involve forces, displacements, and stresses at the contact interface. Examples include metal forming, vehicle crashes, and gear systems.

Lateral Springs in Contact Problems

Lateral springs are used in FEA to model the contact behavior between bodies. They provide a way to simulate the stiffness and damping properties of the contact interface. Here's how they work:

1. Representation: Lateral springs are represented as elements that connect the nodes of the contacting bodies.
2. Stiffness: The stiffness of the lateral springs determines how much resistance they provide against lateral movements.
3. Damping: Damping properties help in absorbing energy and reducing oscillations at the contact interface.

Challenges in Modeling Contact Problems

Contact problems are inherently nonlinear and can be challenging to solve. Some of the key challenges include:

- Nonlinearity: The contact force-displacement relationship is highly nonlinear.
- Convergence: Ensuring the numerical model converges can be difficult due to the complex nature of contact interactions.
- Computational Cost: Tracking contact and separation can be computationally expensive.

Applications

Contact problems are crucial in various engineering fields, such as:

- Automotive Engineering: For crash simulations and component interactions.
- Mechanical Engineering: For gear and bearing analysis.
- Manufacturing: For metal forming and stamping processes.

In [12]:

```
# REPORT: Soil-Structure Interaction for Building Structures
'https://www.nehrp.gov/pdf/nistgcr12-917-21.pdf'
```

Out[12]:

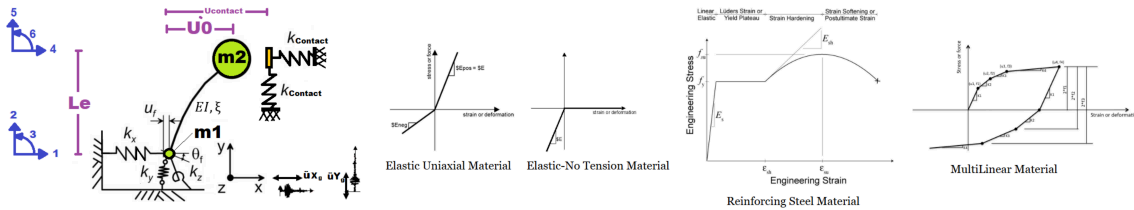
```
'https://www.nehrp.gov/pdf/nistgcr12-917-21.pdf'
```

In [13]:

```
# Load the image
def PLOT_IMAGE(image):
    import matplotlib.pyplot as plt
    import matplotlib.image as mpimg
    image = mpimg.imread(image_path)

    # Display the image
    plt.figure(figsize=(15, 8))
    plt.imshow(image)
    plt.axis('off') # Hide axes
    plt.show()

image_path = 'OPENSEES_SOIL_STRUCTURE_INTERACTION_COLUMN_SEISMIC_CONTACT.png'
PLOT_IMAGE(image_path)
```



```
In [14]: #import the os module
import os
import time
import numpy as np
import openseespy.opensees as op
import opsv as opsv
import matplotlib.pyplot as plt
```

```
In [15]: #to create a directory at specified path with name "Data"
os.mkdir('C:\\OPENSEESPY_SALAR')
```

```
-----
FileExistsError                                Traceback (most recent call last)
Cell In[15], line 2
      1 #to create a directory at specified path with name "Data"
----> 2 os.mkdir('C:\\OPENSEESPY_SALAR')

FileExistsError: [WinError 183] Cannot create a file when that file already exists: 'C:\\OPENSEESPY_SALAR'
```

```
In [16]: FOLDER_NAME = 'SOIL_STRUCTURE_INTERACTION_COLUMN_CONTACT'
dir = f'C:\\OPENSEESPY_SALAR\\{FOLDER_NAME}\\'
if not os.path.exists(dir):
    os.makedirs(dir)
```

```
In [17]: # OUTPUT DATA ADDRESS:
SALAR_DIR = f'C:\\OPENSEESPY_SALAR\\{FOLDER_NAME}\\';
```

```
In [18]: ## DELETE ALL FILES IN DIRECTORY
def DELETE_FOLDER_CONTANTS(folder_path):
    import os
    for filename in os.listdir(folder_path):
        file_path = os.path.join(folder_path, filename)
        try:
            if os.path.isfile(file_path) or os.path.islink(file_path):
                os.unlink(file_path)
            except Exception as e:
                print(f'Failed to delete {file_path}. Reason: {e}')
        print("Deletion done")

FOLDER_PATH = f'C:\\OPENSEESPY_SALAR\\{FOLDER_NAME}' # Specify the folder path
#DELETE_FOLDER_CONTANTS(FOLDER_PATH)
```

```
In [19]: def PLOT_2D(X1, Y1, X2, Y2, XLABEL, YLABEL, TITLE):
    plt.figure(figsize=(10, 6))
    plt.plot(X1, Y1, label='Undamped', color='black')
    plt.plot(X2, Y2, label='Damped', color='red')
    plt.xlabel(XLABEL)
    plt.ylabel(YLABEL)
    plt.title(TITLE)
    plt.grid(True)
    #plt.semilogy()
    plt.legend()
    plt.show()

# -----

def PLOT_SPRING():
    import matplotlib.pyplot as plt
    # Extract displacements and forces
    displacement_kh, force_kh = zip(*kh)
    displacement_kv, force_kv = zip(*kv)
    displacement_kr, force_kr = zip(*kr)

    # Plotting the force-displacement relation
    plt.figure(figsize=(15, 5))

    plt.subplot(1, 3, 1)
    plt.plot(displacement_kh, force_kh, label='Horizontal Spring', color='blue')
    plt.xlabel('Displacement (m)')
    plt.ylabel('Force (N)')
    plt.title('Horizontal Spring Force-Displacement Relation')
    plt.legend()

    plt.subplot(1, 3, 2)
    plt.plot(displacement_kv, force_kv, label='Vertical Spring', color='green')
    plt.xlabel('Displacement (m)')
    plt.ylabel('Force (N)')
    plt.title('Vertical Spring Force-Displacement Relation')
    plt.legend()

    plt.subplot(1, 3, 3)
    plt.plot(displacement_kr, force_kr, label='Rotational Spring', color='red')
    plt.xlabel('Rotation (rad)')
    plt.ylabel('Moment (N.mm)')
    plt.title('Rotational Spring Moment-Rotation Relation')
    plt.legend()

    plt.tight_layout()
    plt.show()

# -----

def PLOT_4_CHART(time_undamped, time_damped, displacement_n1x_undamped, displacement_n1x_damped,
    displacement_n2x_undamped, displacement_n2x_damped, velocity_n1_undamped,
    velocity_n1_damped, velocity_n2_undamped, velocity_n2_damped, acceleration_n1_undamped,
    acceleration_n1_damped, acceleration_n2_undamped, acceleration_n2_damped,
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        spring_force_H_undamped, spring_force_H_damped, spring_force_V_undamped,
        spring_force_V_damped, spring_force_M_undamped, spring_force_M_damped,
        ele_force_undamped, ele_force_damped):
import matplotlib.pyplot as plt
import numpy as np
# Plot the results
plt.figure(figsize=(18, 28))

# Displacement - NODE 01
plt.subplot(8, 1, 1)
P1 = displacement_n1x_undamped
P2 = displacement_n1x_damped
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Displacement X')
plt.title('Displacement Time History - NODE 01')
plt.legend()

plt.subplot(8, 1, 2)
P1 = displacement_n1y_undamped
P2 = displacement_n1y_damped
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Displacement Y')
plt.title('Displacement Time History - NODE 01')
plt.legend()

# Displacement - NODE 02
plt.subplot(8, 1, 3)
P1 = displacement_n2x_undamped
P2 = displacement_n2x_damped
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Displacement X')
plt.title('Displacement Time History - NODE 02')
plt.legend()

plt.subplot(8, 1, 4)
P1 = displacement_n2y_undamped
P2 = displacement_n2y_damped
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Displacement Y')
plt.title('Displacement Time History - NODE 02')
plt.legend()

# Velocity - NODE 01
P1 = velocity_n1_undamped
P2 = velocity_n1_damped
plt.subplot(8, 1, 5)
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Velocity')
plt.title('Velocity Time History - NODE 01')
plt.legend()

# Velocity - NODE 02
P1 = velocity_n2_undamped
P2 = velocity_n2_damped
plt.subplot(8, 1, 6)
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Velocity')
plt.title('Velocity Time History - NODE 02')
plt.legend()

# Acceleration - NODE 01
P1 = acceleration_n1_undamped
P2 = acceleration_n1_damped
plt.subplot(8, 1, 7)
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Acceleration')
plt.title('Acceleration Time History - NODE 01')
plt.legend()

# Acceleration - NODE 02
P1 = acceleration_n2_undamped
P2 = acceleration_n2_damped
plt.subplot(8, 1, 8)
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Acceleration')
plt.title('Acceleration Time History - NODE 02')
plt.legend()

# Display the plot
plt.show()

plt.figure(figsize=(18, 20))
# Spring Force - SHEAR
P1 = spring_force_H_undamped
P2 = spring_force_H_damped
plt.subplot(4, 1, 1)
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Horizontal Axial Spring Force')
plt.title('Spring Force Time History - SHEAR')
plt.legend()

# Spring Force - AXIAL
P1 = spring_force_V_undamped

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P2 = spring_force_V_damped
plt.subplot(4, 1, 2)
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Vertical Axial Spring Force')
plt.title('Spring Force Time History - AXIAL')
plt.legend()

# Spring Force - MOMENT
P1 = spring_force_M_undamped
P2 = spring_force_M_damped
plt.subplot(4, 1, 3)
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Moment Spring Force')
plt.title('Spring Force Time History - MOMENT')
plt.legend()

# Element Force
P1 = ele_force_undamped
P2 = ele_force_damped
plt.subplot(4, 1, 4)
plt.plot(time_undamped, P1, label=f'Undamped: {np.max(np.abs(P1)):.5e}', color='black')
plt.plot(time_damped, P2, label=f'Damped: {np.max(np.abs(P2)):.5e}', color='red')
plt.xlabel('Time [s]')
plt.ylabel('Element Force')
plt.title('Element Force Time History')
plt.legend()

# Display the plot
plt.show()

# -----

def SECTION_REC(sectionID, matID, B, H, numY, numZ):
    # numY, numZ = 1000, 1
    # Define fiber rectangular section
    op.section('Fiber', sectionID)
    #patch rect $matTag $numSubdivY $numSubdivZ $yI $zI $yJ $zJ
    op.patch('rect', matID, numY, numY, -0.5*H, -B, 0.5*H, B) # Core

# -----

"""
When OK equals -1, it generally indicates that the command or operation was not executed
because it was already in progress or had already been completed. This can happen if you
try to run a command that is already running or has been completed in a previous step.

When OK equals -2, it typically indicates that the command or operation was not executed
because it was not recognized or not implemented. This could mean that the command
is either misspelled, not available in the current version of OpenSees, or not applicable to the current context.

When OK equals -3, it typically means that the command or operation failed.
This could be due to various reasons, such as incorrect input parameters,
syntax errors, or issues with the model setup.
"""

def ANALYSIS(OK, INCREMENT, TOLERANCE, MAX_ITERATIONS):
    import openseespy.opensees as op
    test = {1: 'NormDispIncr', 2: 'RelativeEnergyIncr', 4: 'RelativeNormUnbalance', 5: 'RelativeNormDispIncr', 6: 'NormUnbalance'}
    algorithm = {1: 'KrylovNewton', 2: 'SecantNewton', 4: 'RaphsonNewton', 5: 'PeriodicNewton', 6: 'BFGS', 7: 'Broyden', 8: 'NewtonLineSearch'}

    for i in test:
        for j in algorithm:
            if OK != 0:
                if j < 4:
                    op.algorithm(algorithm[j], '-initial')

            else:
                op.algorithm(algorithm[j])

                op.test(test[i], TOLERANCE, MAX_ITERATIONS)
                OK = op.analyze(INCREMENT)
                print(test[i], algorithm[j], OK)
                if OK == 0:
                    break
            else:
                continue

# -----

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In [20]: import openseespy.opensees as ops
import numpy as np
import matplotlib.pyplot as plt

# Set up variables
Ea = 210.0e3 # [N/mm^2] Modulus of elasticity (Young's modulus)
B = 100 # [mm] Section Width
H = 200 # [mm] Section Height
Le = 3000.0 # [mm] Length of the column element
A = B * H # [mm^2] Cross-sectional area of the element
I = (B * H**3) / 12 # [mm^4]
KE = (3 * Ea * I) / Le**3 # [N/mm] Lateral column Effective Stiffness
print(f'Lateral column Stiffness: {KE:.3f} (N/mm)')
iv0 = 0.005 # [mm/s] Initial velocity applied to the node
st_iv0 = 0.0 # [s] Initial velocity applied starting time
damping_ratio = 0.05 # Damping ratio
soil_damping_ratio = 0.015 # Damping ratio for Stony soils
MS = 10000 # [kg] mass values for node 1

# Define masses for multiple nodes
mass_values = [100.0, 500.0] # [kg] mass values for node 2

# Set analysis parameters
duration = 50.0 # [s] 50 Seconds
dt = 0.01

# Define ReinforcingSteel material properties
fy_m = 400.0 # [N/mm^2] Yield strength of steel
Es_m = 210.0e3 # [N/mm^2] Modulus of elasticity

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fu_m = 600.0 # [N/mm^2] Ultimate strength
Esh_m = 20.0e3 # [N/mm^2] Hardening modulus
esh_m = 0.03 # [mm/mm] Strain at start of hardening
esu_m = 0.1 # [mm/mm] Ultimate strain

# Section Fibers
numY, numZ = 1000, 1

MAX_ITERATIONS = 5000 # convergence iteration for test
TOLERANCE = 1.0e-10 # convergence tolerance for test

# Define soil-structure interaction springs at base (Node 1)
# [1] ELASTIC SOIL & [2] ELASTIC-NO TENSION SOIL:
# Spring constants for soil-structure interaction
k_horizontal = 210.0e4 # [N/mm] Horizontal spring constant
k_vertical = 150.0e4 # [N/mm] Vertical spring constant
k_rotational = 550.0e4 # [N.mm/Rad] Rotational spring constant

# [3] ELASTO-PLASTIC SOIL:
# Horizontal spring parameters
fy_h = 400.0 # [N] Yield force
Es_h = 21000.0 # [N/mm] Lateral Stiffness in Horizontal Direction
fu_h = 410.0 # [N] Ultimate force
Esh_h = 200.0 # [N/mm] Hardening modulus
esh_h = 0.4 # [mm] Displacement at start of hardening
esu_h = 1.1 # [mm] Ultimate displacement

# Vertical spring parameters
fy_v = 450.0 # [N] Yield force
Es_v = 20000.0 # [N/mm] Lateral Stiffness in Vertical Direction
fu_v = 460.0 # [N] Ultimate force
Esh_v = 180 # [N/mm] Hardening modulus
esh_v = 0.5 # [mm] Displacement at start of hardening
esu_v = 1.8 # [mm] Ultimate displacement

# Rotational spring parameters
fy_r = 50e3 # [N.mm] Yield force
Es_r = 50e5 # [N.mm/Rad] Rotational Stiffness in Horizontal Direction
fu_r = 70e3 # [N.mm] Ultimate force
Esh_r = 50e4 # [N.mm/Rad] Hardening modulus
esh_r = 0.06 # [Rad] Rotation at start of hardening
esu_r = 0.12 # [Rad] Ultimate rotation

# [4] Define the MultiLinear material properties for springs
kh = np.array([[0.05, 22.0], [0.15, 27.0], [0.25, 27.0], [0.3, 25.5], [0.5, 21.5]]) # k horizontal
kv = np.array([[0.15, 18.5], [0.3, 27.0], [0.4, 29.0], [0.5, 30.5], [1.1, 24.4]]) # k vertical
kr = np.array([[0.01, 10500.0], [0.04, 29000.0], [0.06, 30000.0], [0.09, 30500.0], [0.15, 15000.0]]) # k rotational

# Create a DataFrame
import pandas as pd
data = {
    "Displacement_X": kh[:, 0],
    "Force (kh)": kh[:, 1],
    "Displacement_Y": kv[:, 0],
    "Force (kv)": kv[:, 1],
    "Rotation_Z": kr[:, 0],
    "Force (kr)": kr[:, 1]
}

#df = pd.DataFrame(data)
PLOT_SPRING()

def INTERACTION_ANALYSIS(MASS_ONE, DAMPING, SOIL_DAMPING, periodTF, LINEAR, SPRING_KIND, CONTACT, CONTACT_DISP):
    import openseespy.opensees as ops
    import numpy as np

    KE = (3 * Ea * I) / L**3; # [N/mm] Lateral column Effective Stiffness
    GMfact = 9810 # standard acceleration of gravity or standard acceleration
    GMfact = 1;
    # Set up the model
    ops.wipe()
    ops.model('basic', '-ndm', 2, '-ndf', 3)

    # Define nodes
    ops.node(1, 0.0, 0.0)
    ops.node(2, 0.0, Le)
    ops.node(200, 0.0, 0.0) # SOIL NODE
    ops.node(400, 0.0, Le) # CONTACT NODE

    # Define boundary conditions (modified to include springs)
    ops.fix(200, 1, 1, 1) # Node 200 is Fixed in all directions initially - SOIL
    ops.fix(400, 1, 1, 1) # Node 400 is Fixed in all directions initially - CONTACT

    # Assign masses to nodes
    if MASS_ONE == True:
        ops.mass(2, mass_values[1], mass_values[1], 0.0)
        print('ONE MASS')

    if MASS_ONE == False:
        ops.mass(1, mass_values[0], mass_values[0], 0.0)
        ops.mass(2, mass_values[1], mass_values[1], 0.0)
        print('TWO MASSES')

    # Define material
    matID = 1
    if LINEAR == True:
        ops.uniaxialMaterial('Elastic', matID, Es_m)
    if LINEAR == False:
        ops.uniaxialMaterial('ReinforcingSteel', 1, fy_m, Es_m, fu_m, Esh_m, esh_m, esu_m)

    # Define fiber rectangular section
    sectionID = 1
    ops.section('Fiber', sectionID)
    #patch rect $matTag $numSubdivY $numSubdivZ $yI $zI $yJ $zJ
    ops.patch('rect', matID, numY, numZ, -0.5*H, -B, 0.5*H, B) # Core

    # Define geometric transformation
    ops.geomTransf('Linear', 1)

    # Define elements
    # element nonlinearBeamColumn $eleTag $iNode $jNode $numIntgrPts $secTag $transfTag
    eleID = 1

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ops.element('nonlinearBeamColumn', eleID, 1, 2, 5, sectionID, 1)

# OUTPUT DATA
ops.recorder('Node', '-file', f"[SALAR_DIR]DTH.DYN.txt", '-time', '-node', 2, '-dof', 1,2,3, 'disp')# Displacement Time History Node 2
ops.recorder('Node', '-file', f"[SALAR_DIR]VTH.DYN.txt", '-time', '-node', 2, '-dof', 1,2,3, 'vel') # Velocity Time History Node 2
ops.recorder('Node', '-file', f"[SALAR_DIR]ATH.DYN.txt", '-time', '-node', 2, '-dof', 1,2,3, 'accel') # Acceleration Time History Node 2
ops.recorder('Node', '-file', f"[SALAR_DIR]BTH.DYN.txt", '-time', '-node', 1, '-dof', 1,2,3, 'reaction')# Base Shear Time History Node 1
ops.recorder('Element', '-file', f"[SALAR_DIR]DEF.DYN.txt", '-time', '-ele', 1, 'section', 1, 'deformations')# Curvature Time History

# Define soil-structure interaction springs at base (Node 1)
if SPRING_KIND == 1:
    # Elastic Material:
    # uniaxialMaterial Elastic $matTag $E <$eta> <$eneg> -> (Eneg: tangent in compression (optional, default=E))
    ops.uniaxialMaterial('Elastic', 2, 0.3 * k_horizontal, 0, k_horizontal) # Horizontal spring
    ops.uniaxialMaterial('Elastic', 3, 0.3 * k_vertical, 0, k_vertical) # Vertical spring
    ops.uniaxialMaterial('Elastic', 4, 0.3 * k_rotational, 0, k_rotational) # Rotational spring
    KS = k_horizontal # Soil Lateral Stiffness
if SPRING_KIND == 2:
    # Define horizontal spring
    ops.uniaxialMaterial('ReinforcingSteel', 2, fy_h, Es_h, fu_h, Esh_h, esu_h)
    # Define vertical spring
    ops.uniaxialMaterial('ReinforcingSteel', 3, fy_v, Es_v, fu_v, Esh_v, esu_v)
    # Define rotational spring
    ops.uniaxialMaterial('ReinforcingSteel', 4, fy_r, Es_r, fu_r, Esh_r, esu_r)
    KS = Es_h # Soil Lateral Stiffness
if SPRING_KIND == 3:
    # Elastic-No Tension Material:
    # uniaxialMaterial ENT $matTag $E
    ops.uniaxialMaterial('ENT', 2, k_horizontal) # Horizontal spring
    ops.uniaxialMaterial('ENT', 3, k_vertical) # Vertical spring
    ops.uniaxialMaterial('ENT', 4, k_rotational) # Rotational spring
    KS = k_horizontal # Soil Lateral Stiffness
if SPRING_KIND == 4:
    # Define soil-structure interaction springs at base (Node 1)
    # STRAIN STRESS
    ops.uniaxialMaterial('Multilinear', 2, *kh.flatten()) # Horizontal spring
    ops.uniaxialMaterial('Multilinear', 3, *kh.flatten()) # Vertical spring
    ops.uniaxialMaterial('Multilinear', 4, *kh.flatten()) # Rotational spring
    KS = kh[0][1] / kh[0][0] # Soil Lateral Stiffness

# Soil Natural frequency (rad/s)
wns = (KS / MS) ** 0.5
# Soil Damping coefficient (Ns/m)
CS = 2 * wns * MS * soil_damping_ratio if SOIL_DAMPING else 0.0
# Define materials for Soil damper
ops.uniaxialMaterial('Elastic', 5, 0.0, CS)

if SOIL_DAMPING == False:
    # element zeroLength $eleTag $iNode $jNode -mat $matTag1 $matTag2 ... -dir $dir1 $dir2
    ops.element('zeroLength', 2, 200, 1, '-mat', 2, '-dir', 1) # Horizontal spring for soil stiffness
    ops.element('zeroLength', 3, 200, 1, '-mat', 3, '-dir', 2) # Vertical spring for soil stiffness
    ops.element('zeroLength', 4, 200, 1, '-mat', 4, '-dir', 3) # Rotational spring for soil stiffness
if SOIL_DAMPING == True:
    ops.element('zeroLength', 2, 200, 1, '-mat', 2, 5, '-dir', 1, 1) # Horizontal spring for soil stiffness and damping
    ops.element('zeroLength', 3, 200, 1, '-mat', 3, 5, '-dir', 2, 2) # Vertical spring for soil stiffness and damping
    ops.element('zeroLength', 4, 200, 1, '-mat', 4, '-dir', 3, 3) # Rotational spring for soil stiffness

# Define time series for input motion (Acceleration time history)
ops.timeSeries('Path', 1, '-dt', 0.01, '-filePath', 'OPENSEES_SPRING_SEISMIC_01.txt', '-factor', GMfact, '-startTime', st_iv0) # SEISMIC-X
ops.timeSeries('Path', 2, '-dt', 0.01, '-filePath', 'OPENSEES_SPRING_SEISMIC_02.txt', '-factor', GMfact) # SEISMIC-Y

# Define Load patterns
ops.pattern('UniformExcitation', 1, 1, '-accel', 1, '-vel0', iv0, '-fact', 1.0) # SEISMIC-X
ops.pattern('UniformExcitation', 2, 2, '-accel', 2) # SEISMIC-Y

# Perform eigenvalue analysis to determine modal periods
if periodTF == True:
    #eigenvalues01 = ops.eigen('-genBandArpack', 1) # eigenvalue mode 1
    eigenvalues01 = ops.eigen('-fullGenLapack', 1) # eigenvalue mode 1
    #eigenvalues01 = ops.eigen('-symmBandLapack', 1) # eigenvalue mode 1
    #Omega = np.power(eigenvalues01, 0.5)
    #Omega = np.power(max(min(eigenvalues01), min(eigenvalues02), min(eigenvalues03)), 0.5)
    Omega = np.power(min(eigenvalues01), 0.5)
    modal_period = 2 * np.pi / np.sqrt(Omega) # [Second]
    frequency = 1 / modal_period # [Hertz]

if periodTF == False:
    modal_period = 0.0 # [Second]
    frequency = 0.0 # [Hertz]

# Dynamic analysis
ops.constraints('Transformation')
ops.numberer('RCM')
ops.system('UmFPack')
op.test('EnergyIncr', TOLERANCE, MAX_ITERATIONS) # determine if convergence has been achieved at the end of an iteration step
#ops.integrator('CentralDifference') # Central Difference Method
ops.integrator('HHT', 0.9) # Hilber-Hughes-Taylor Method
#ops.integrator('Newmark', 0.5, 0.25) # Newmark Method
ops.algorithm('ModifiedNewton') # use Newton's solution algorithm: updates tangent stiffness at every iteration

if DAMPING == True:
    # Calculate Rayleigh damping factors
    omega1 = np.sqrt(KE / mass_values[1])
    omega2 = 2 * omega1 # Just an assumption for two modes
    a0 = damping_ratio * (2 * omega1 * omega2) / (omega1 + omega2)
    a1 = damping_ratio * 2 / (omega1 + omega2)
    # Apply Rayleigh damping
    ops.rayleigh(a0, a1, 0, 0)

ops.analysis('Transient')

if CONTACT == True:
    # CONTACT SPRING STIFFNESS PROPERTIES:
    PY_C = 114e10 # [N] Yield force capacity
    K_C = PY_C / 7 # [N/mm] Axial Rigidity Stiffness
    ops.uniaxialMaterial('Steel02', 300, PY_C, K_C, 0.1, 18.0, 0.925, 0.15) # NONLINEAR CONTACT
    #ops.uniaxialMaterial('Elastic', 300, K_C) # LINEAR CONTACT
    #CONTACT_STEPS = int(np.abs(CONTACT_DISP / DtAnalysis))

```

```

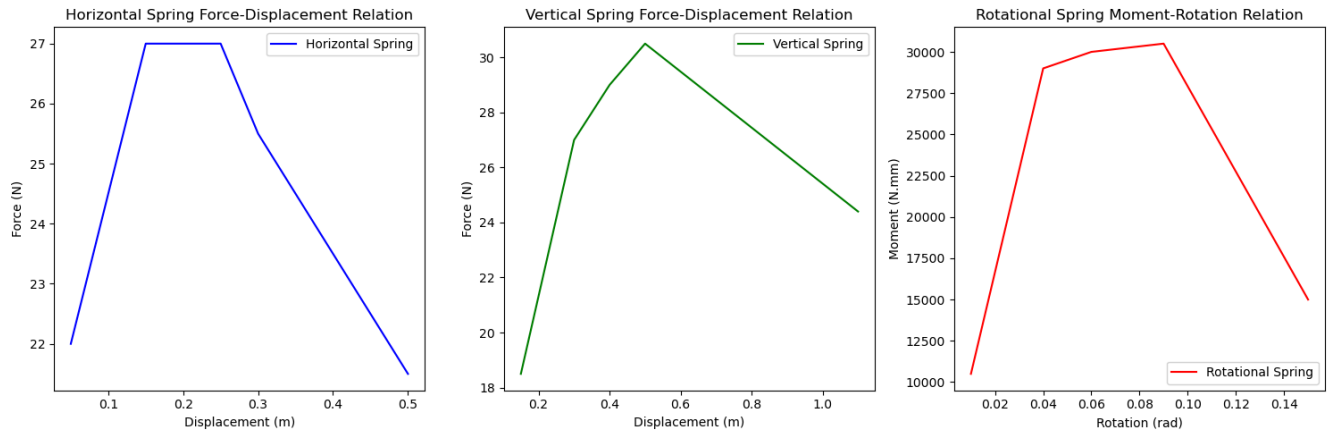
# Perform transient analysis and store results
time = []
displacement_n1x = []
displacement_n1y = []
velocity_n1 = []
acceleration_n1 = []
displacement_n2x = []
displacement_n2y = []
velocity_n2 = []
acceleration_n2 = []
spring_force_H = []
spring_force_V = []
spring_force_M = []
ele_force = []
stable = 0
current_time = 0.0
NN = 0; # [0] CHECK JUST ONE TIME THIS LATERAL SPRING -> Contact is not applied [1] Contact is applied
while stable == 0 and current_time < duration:
    stable = ops.analyze(1, dt)
    if SPRING_KIND != 3 or SPRING_KIND != 4: # if Elastic-No Tension Material
        ANALYSIS(stable, 1, TOLERANCE, MAX_ITERATIONS) # CHECK THE ANALYSIS
    current_time = ops.getTime()
    time.append(current_time)
    displacement_n1x.append(ops.nodeDisp(1, 1)) # FOUNDATION DISPLACEMENT-X
    displacement_n1y.append(ops.nodeDisp(1, 2)) # FOUNDATION DISPLACEMENT-Y
    velocity_n1.append(ops.nodeVel(1, 1)) # FOUNDATION VELOCITY-X
    acceleration_n1.append(ops.nodeAccel(1, 1)) # FOUNDATION ACCEL-X
    displacement_n2x.append(ops.nodeDisp(2, 1)) # COLUMN TOP DISPLACEMENT-X
    displacement_n2y.append(ops.nodeDisp(2, 2)) # COLUMN TOP DISPLACEMENT-Y
    velocity_n2.append(ops.nodeVel(2, 1)) # COLUMN TOP VELOCITY-X
    acceleration_n2.append(ops.nodeAccel(2, 1)) # COLUMN TOP ACCEL-X
    spring_force_H.append(-ops.eleResponse(2, 'force')[0]) # SOIL REACTION - SHEAR
    spring_force_V.append(-ops.eleResponse(3, 'force')[1]) # SOIL REACTION - AXIAL
    spring_force_M.append(-ops.eleResponse(4, 'force')[2]) # SOIL REACTION - MOMENT
    ele_force.append(-ops.eleResponse(1, 'force')[0]) # COLUMN REACTION
    KE = ele_force[-1] / displacement_n2x[-1] # Effective Lateral Stiffness
    if CONTACT == True:
        if displacement_n2x[-1] >= CONTACT_DISP: # CHECK FOR CONTACT
            if NN == 0:
                print(f'IN TIME {current_time} CONTACT DONE!')
                ops.element('zeroLength', 500, 400, 2, '-mat', 300, '-dir', 1) # DOF[4] LATERAL CONTACT SPRING
                ops.element('zeroLength', 600, 400, 2, '-mat', 300, '-dir', 2) # DOF[5] LATERAL CONTACT SPRING
                NN = 1; # WE WANT TO ADD ONE TIME THIS LATERAL SPRINGS -> Contact is applied before

ops.wipe()
print("Period: ", modal_period)
print("Frequency: ", frequency)
if DAMPING == False:
    print('Undamping Structure Dynamic Analysis Done.')
if DAMPING == True:
    print('Damping Structure Dynamic Analysis Done.')

return modal_period, time, displacement_n1x, displacement_n1y, displacement_n2x, displacement_n2y, velocity_n1, velocity_n2, acceleration_n1, acceleration_n2, spring_force_H, spring_force_V, spring_force_M, ele_force

```

Lateral column Stiffness: 1555.556 (N/mm)



```

In [21]: ### SOIL-STRUCTURE INTERACTION WITH TWO MASSES:
starttime = time.process_time()
# Perform analysis
period_undamped, time_undamped, displacement_n1x_undamped, displacement_n1y_undamped, displacement_n2x_undamped, displacement_n2y_undamped, velocity_n1_undamped, velocity_n2_undamped,
period_damped, time_damped, displacement_n1x_damped, displacement_n1y_damped, displacement_n2x_damped, displacement_n2y_damped, velocity_n1_damped, velocity_n2_damped, acceleration_n1_undamped,
acceleration_n1_damped, acceleration_n2_undamped, acceleration_n2_damped, spring_force_H_undamped, spring_force_H_damped, spring_force_V_undamped, spring_force_V_damped, ele_force_undamped, ele_force_damped

totaltime = time.process_time() - starttime
print(f'\nTotal time (s): {totaltime:.4f} \n\n')

```

```

TWO MASSES
IN TIME 32.5600000000002425 CONTACT DONE!
Period: 0.0
Frequency: 0.0
Undamping Structure Dynamic Analysis Done.
TWO MASSES
Period: 0.0
Frequency: 0.0
Damping Structure Dynamic Analysis Done.

Total time (s): 72.6250

```

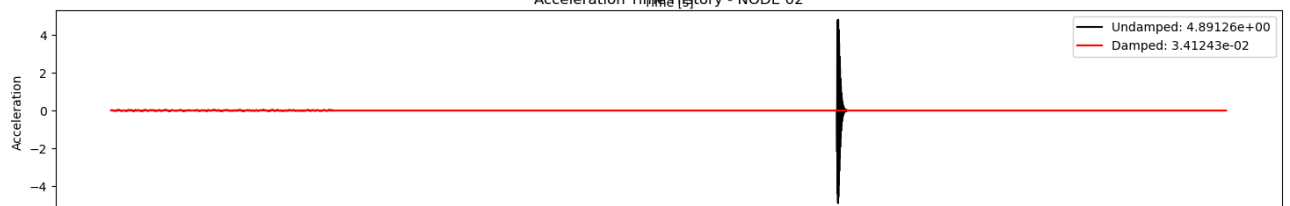
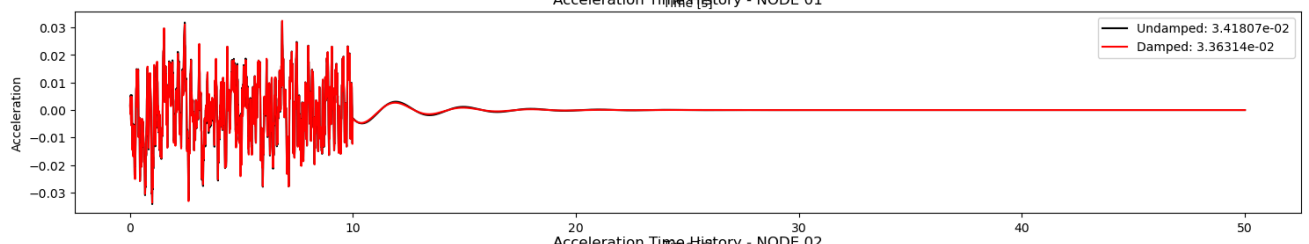
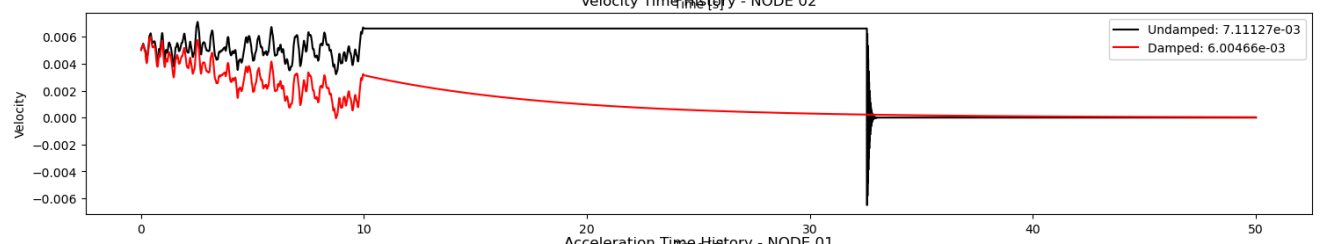
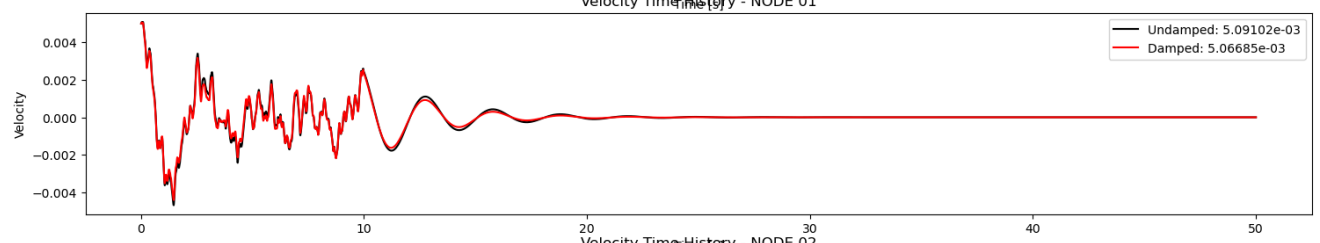
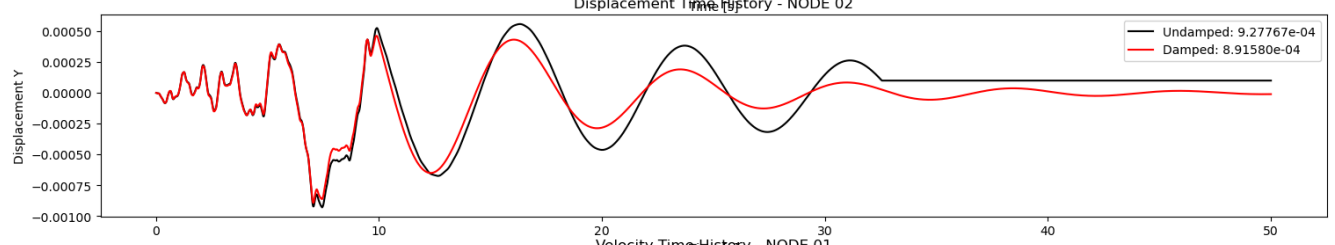
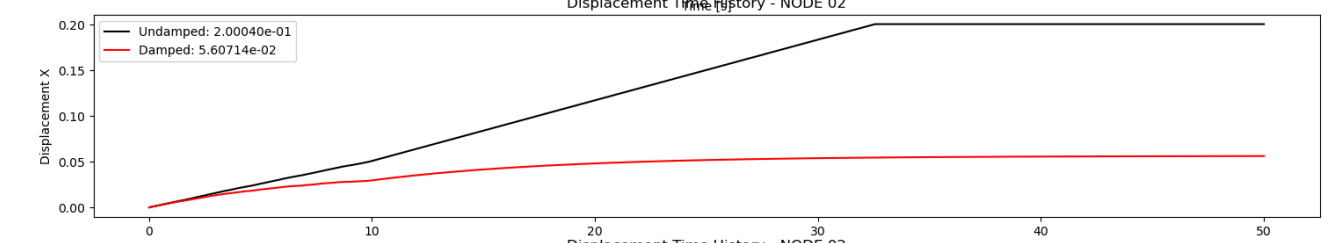
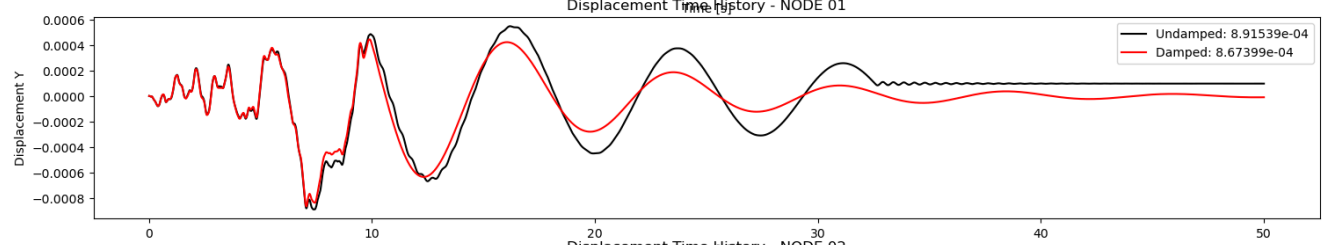
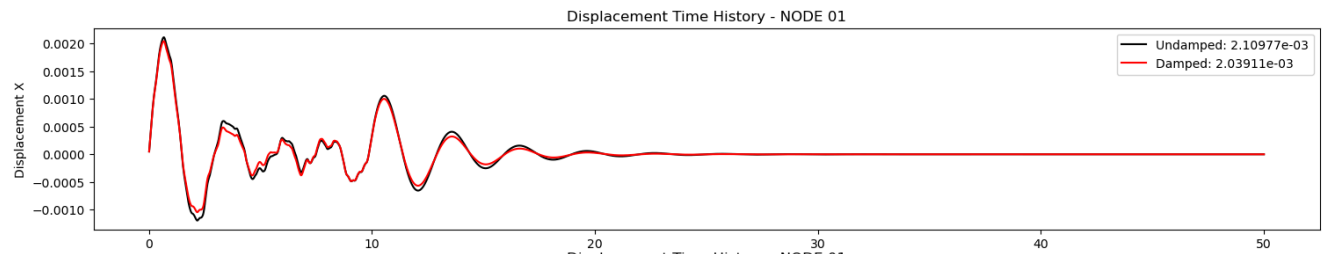
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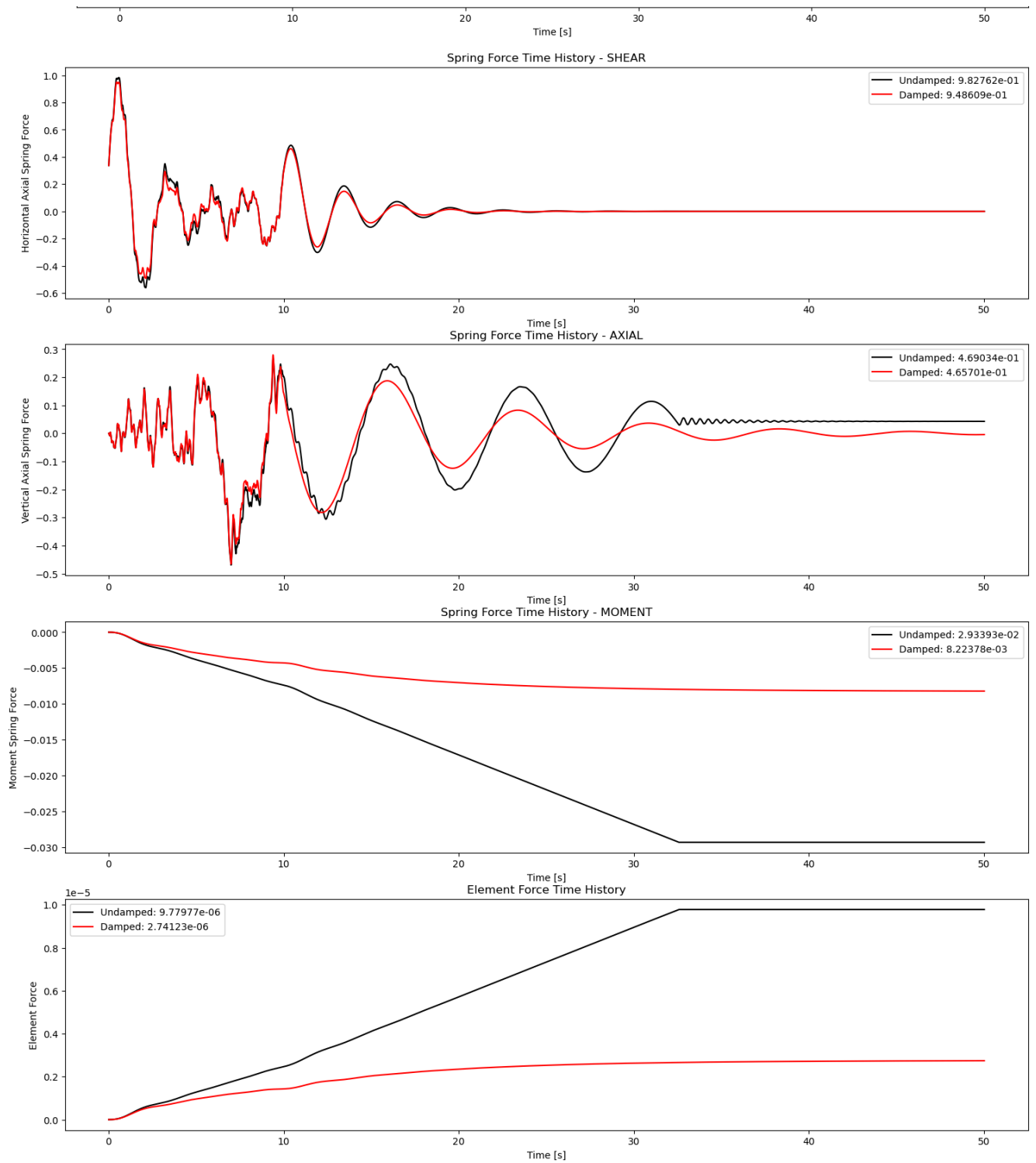
In [22]: ### TWO MASSES TIME HISTORY:
PLOT_4_CHART(time_undamped, time_damped, displacement_n1x_undamped, displacement_n1x_damped,
displacement_n2x_undamped, displacement_n2x_damped, velocity_n1_undamped,
velocity_n1_damped, velocity_n2_undamped, velocity_n2_damped, acceleration_n1_undamped,
acceleration_n1_damped, acceleration_n2_undamped, acceleration_n2_damped,
spring_force_H_undamped, spring_force_H_damped, spring_force_V_undamped, spring_force_V_damped, ele_force_undamped, ele_force_damped)

```

```
spring_force_V_damped, spring_force_M_undamped, spring_force_M_damped,  
ele_force_undamped, ele_force_damped)
```

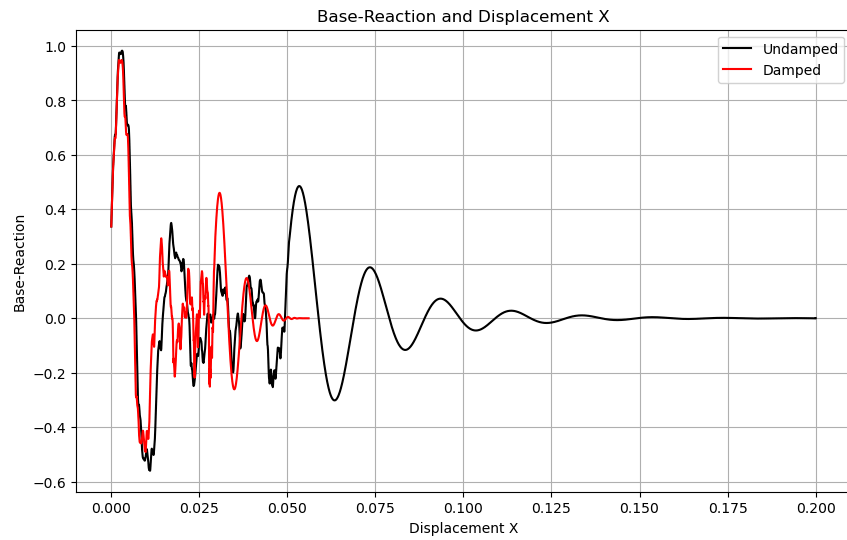






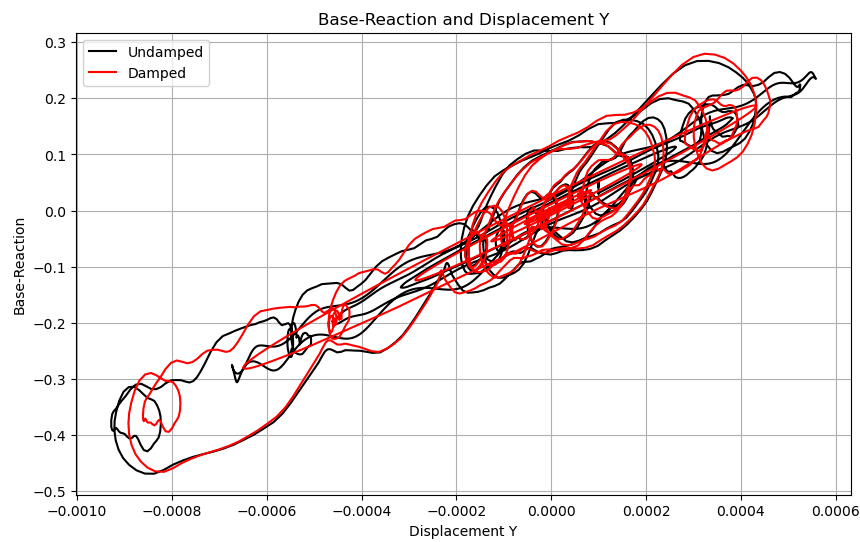
```
In [23]: ### TWO MASSES BASE REACTION & DISPALCEMENT:
X1 = displacement_n2x_undamped
X2 = displacement_n2x_damped
Y1 = spring_force_H_undamped
Y2 = spring_force_H_damped
XLABEL = 'Displacement X'
YLABEL = 'Base-Reaction'
TITLE = 'Base-Reaction and Displacement X'

PLOT_2D(X1, Y1, X2, Y2, XLABEL, YLABEL, TITLE)
```



```
In [24]: ### TWO MASSES BASE REACTION & DISPALCEMENT:
X1 = displacement_n2y_undamped
X2 = displacement_n2y_damped
Y1 = spring_force_V_undamped
Y2 = spring_force_V_damped
XLABEL = 'Displacement Y'
YLABEL = 'Base-Reaction'
TITLE = 'Base-Reaction and Displacement Y'

PLOT_2D(X1, Y1, X2, Y2, XLABEL, YLABEL, TITLE)
```



```
In [ ]:
```