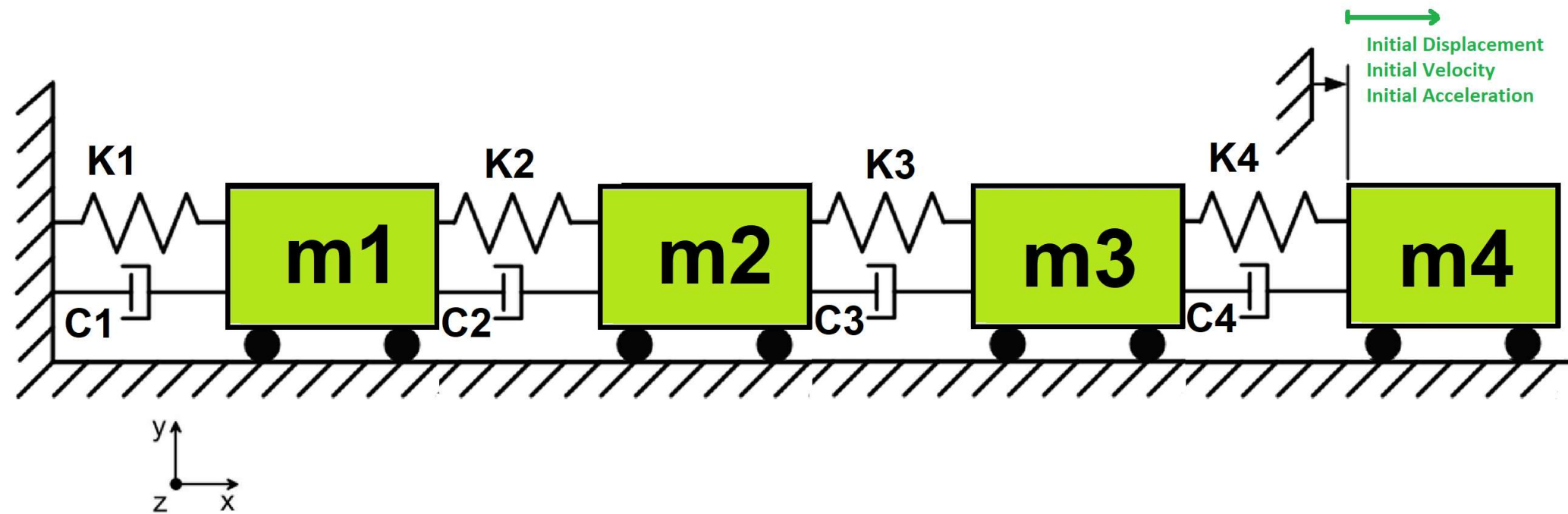


>> IN THE NAME OF ALLAH, THE MOST GRACIOUS, THE MOST MERCIFUL <<

COMPARATIVE FREE- VIBRATION ANALYSIS OF A MDOF STRUCTURE: ELASTIC VS INELASTIC RESPONSE USING OPENSEES

WRITTEN BY SALAR DELAVAR GHASHGHAEI (QASHQAI)



12345678910111213141516171819202122232425262728293031323334

```
#####
#                               >> IN THE NAME OF ALLAH, THE MOST GRACIOUS, THE MOST MERCIFUL <<
# COMPARATIVE FREE-VIBRATION ANALYSIS OF A MDOF STRUCTURE: ELASTIC VS INELASTIC RESPONSE USING OPENSEESPY
#-----
# THIS PROGRAM WRITTEN BY SALAR DELAVAR GHASHGHAEE (QASHQAI)
# EMAIL: salar.d.ghashghaei@gmail.com
#####
"""
Performs free-vibration analysis of a Multi Degree of Freedom (MDOF)
structure using OpenSeesPy, comparing elastic and inelastic spring behavior.

Key features include:
1. Implements both elastic (linear) and hysteretic (nonlinear) material models for structural springs.
2. Supports initial conditions for displacement, velocity, and acceleration.
3. Uses Newmark's method for time integration with Newton-Raphson iteration.
4. Calculates damping ratios using logarithmic decrement from response peaks.
5. Generates force-displacement backbone curves for inelastic material.
6. Tracks and plots time-history responses (displacement, velocity, acceleration, reactions).
7. Compares elastic vs inelastic system performance.
8. Includes convergence checks and analysis stability monitoring.
9. Outputs model data in JSON format for post-processing.
10. Provides theoretical validation through natural frequency calculations.

Particularly useful for earthquake engineering applications,
allowing evaluation of structural response under free vibration
with different material nonlinearities and damping characteristics.
The hysteretic material model captures energy dissipation
inelastic deformation, while the elastic case serves as a reference for linear behavior.
"""
import openseespy.opensees as ops
import numpy as np
import matplotlib.pyplot as plt
import time as TI
import ANALYSIS_FUNCTION as S01
```

...S_FILES\FREE-VIBRATION_U0_VO_A0\FREE-VIBRATION_U0_VO_A0_MDF

32 %

Displacement vs Base-reaction

Help Variable Explorer Debugger Plots Files

Console 1/A

lambda	omega	period	frequency
1.242e+02	11.1429	0.5639	1.7734
1.414e+03	37.6041	0.1671	5.9849
2.707e+03	52.0263	0.1208	8.2802
8.964e+03	94.6777	0.0664	15.0684

IPython Console History

Inline Conda: anaconda3 (Python 3.12.7) LSP: Python Line 3, Col 118 UTF-8 CRLF RW Mem 36%

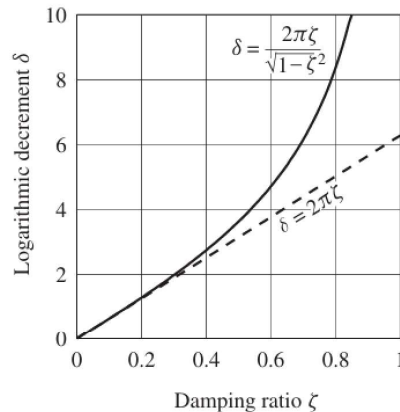
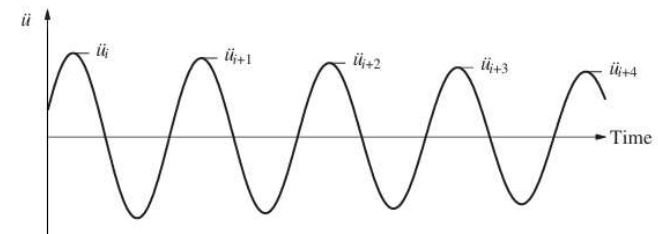
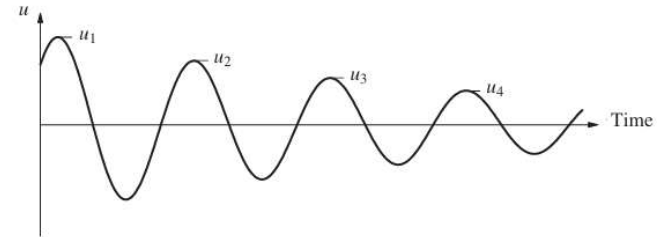
VISCOUSLY DAMPED FREE VIBRATION

$$m\ddot{u} + c\dot{u} + ku = 0$$

$$\ddot{u} + 2\zeta\omega_n\dot{u} + \omega_n^2 u = 0$$

$$\omega_n = \sqrt{k/m} \quad \zeta = \frac{c}{2m\omega_n} = \frac{c}{c_{cr}} \quad \omega_D = \omega_n \sqrt{1 - \zeta^2}$$

$$u(t) = e^{-\zeta\omega_n t} \left[u(0) \cos \omega_D t + \frac{\dot{u}(0) + \zeta\omega_n u(0)}{\omega_D} \sin \omega_D t \right]$$



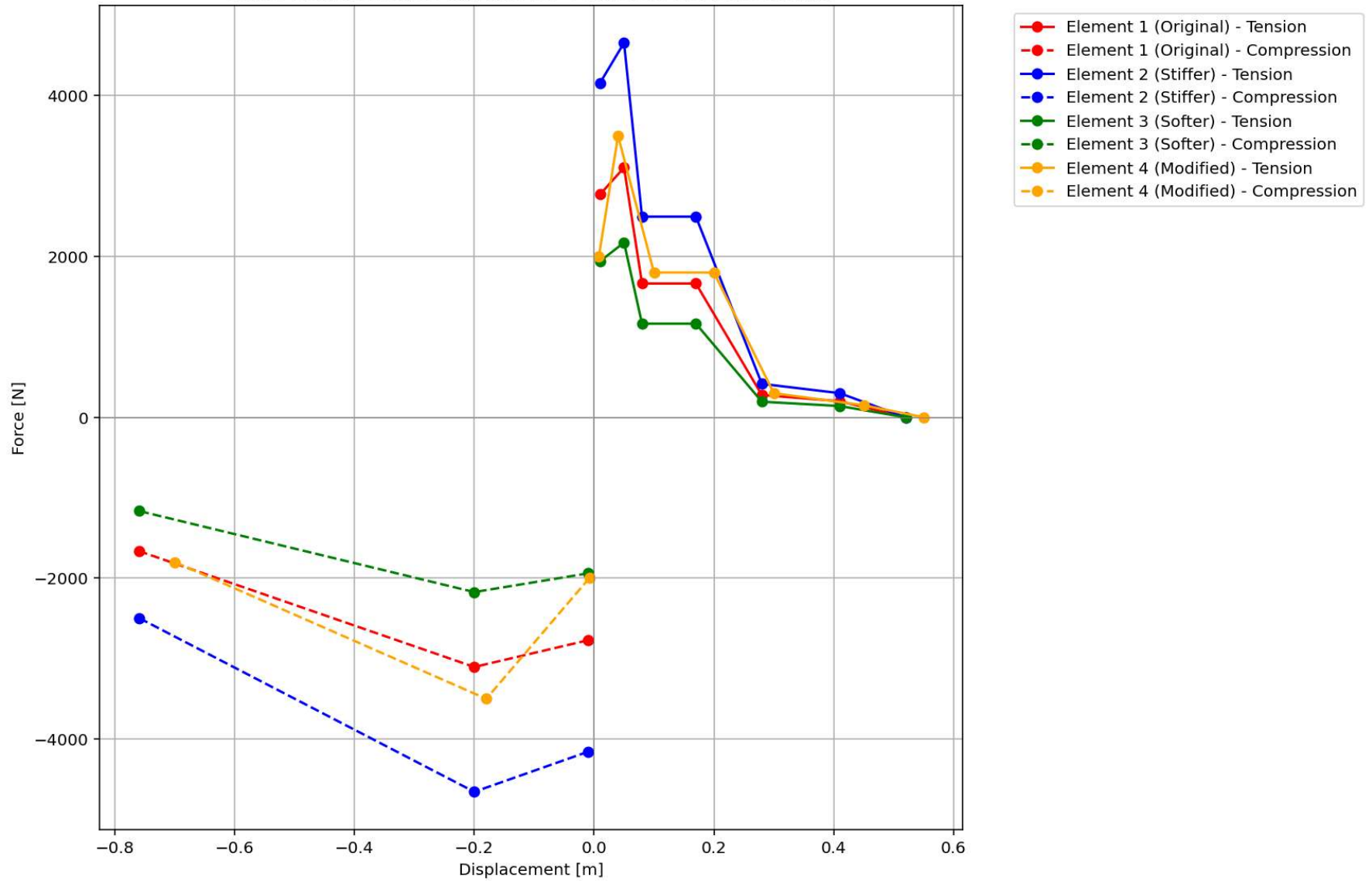
Decay of Motion

$$\delta = \ln \frac{u_i}{u_{i+1}} = 2\pi\zeta \quad (\text{APPROXIMATE RELATION})$$

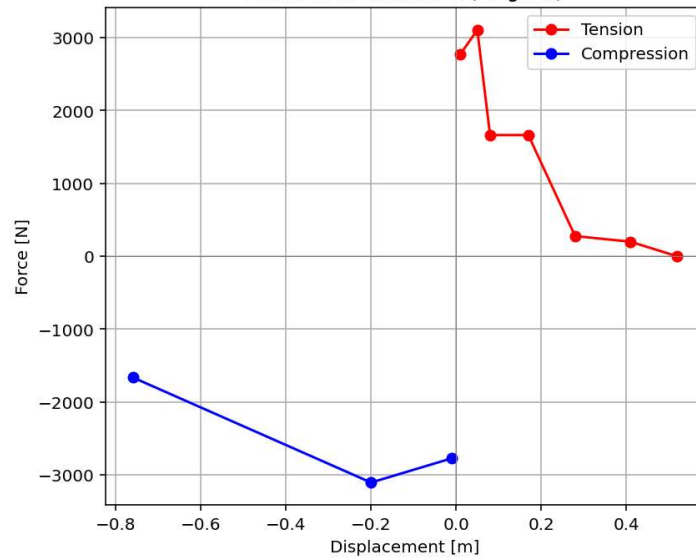
$$\delta = \ln \frac{u_i}{u_{i+1}} = \frac{2\pi\zeta}{\sqrt{1-\zeta^2}} \quad (\text{EXACT RELATION})$$

EXACT AND APPROXIMATE RELATIONS BETWEEN LOGARITHMIC DECREMENT AND DAMPING RATIO

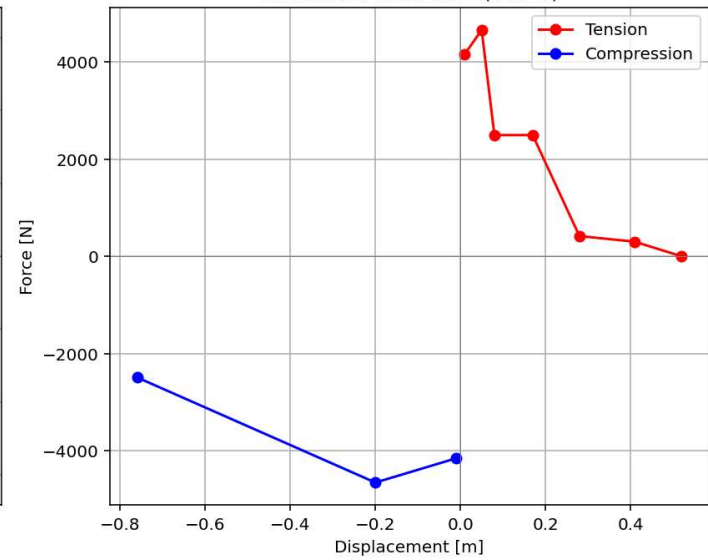
Force-Displacement Diagrams for 4 Different Inelastic Springs



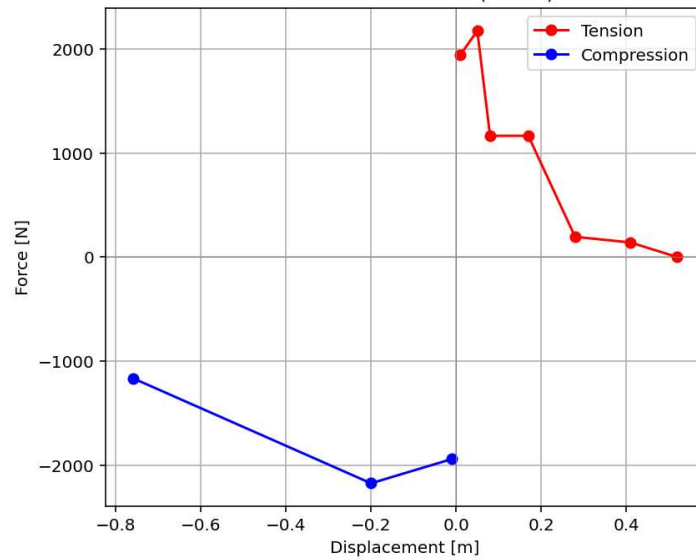
Element 1: Element 1 (Original)



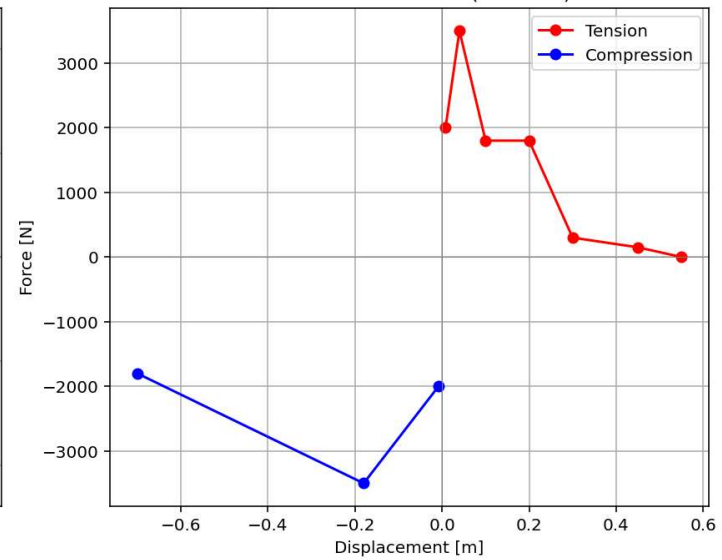
Element 2: Element 2 (Stiffer)

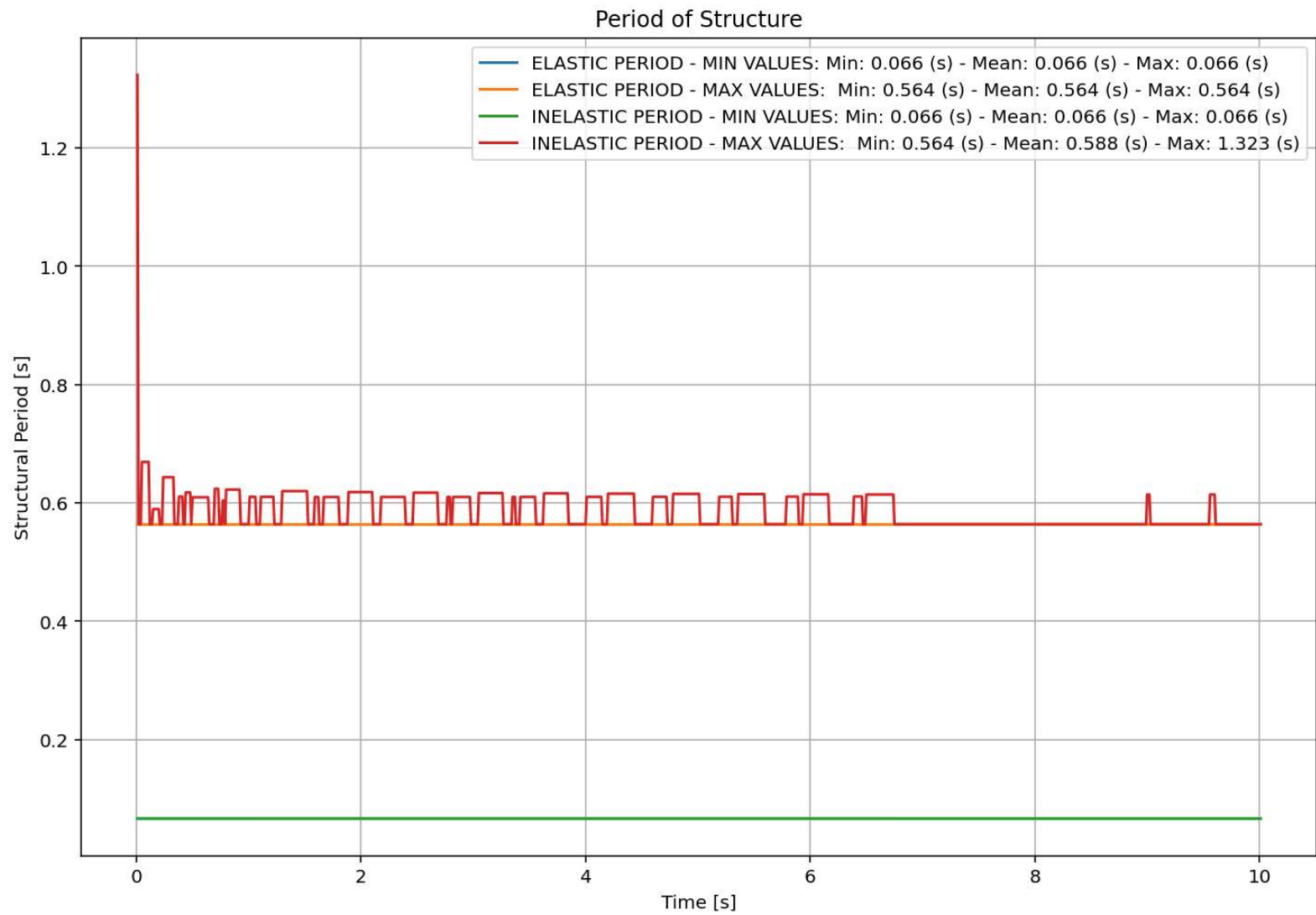


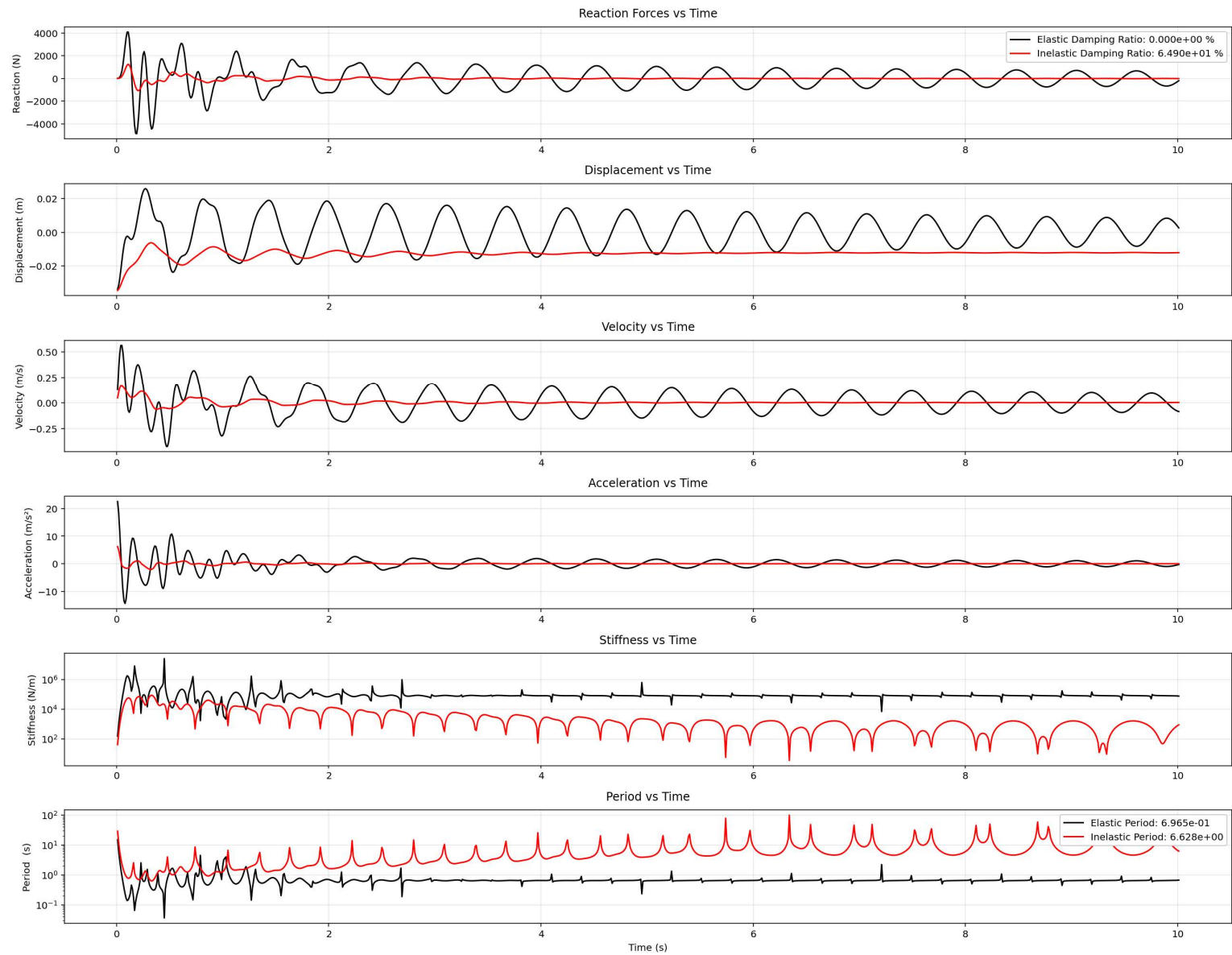
Element 3: Element 3 (Softer)



Element 4: Element 4 (Modified)







Node Displacements vs Time for Elastic Structure

