

```
In [31]: # ##### IN THE NAME OF ALLAH #####
# # CABLE FOOT SUSPENSION BRIDGE 01 #
# #-----#
# # THIS PROGRAM WRITTEN BY SALAR DELAVAR GHASHGHEI (QASHQAI) #
# # EMAIL: salar.d.ghashghe@gmail.com #
# #####
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Suspension Foot Bridge:

A Suspension Foot Bridge is a type of bridge that uses the features of a suspension bridge but is specifically designed for pedestrian use. These bridges are typically used to cross natural obstacles like valleys, rivers, or difficult terrain, whereas regular footbridges are simpler in design and construction.

Features of a Suspension Foot Bridge:

1. Suspension Structure:

- In a suspension footbridge, steel or wire cables are suspended to support the bridge deck. These cables are anchored to tall towers on each side of the bridge, transferring the load from the deck to the towers.
- Unlike regular footbridges, which are usually made of simple beams (either concrete or steel), a suspension footbridge's primary structure consists of cables and towers.

2. Design Purpose:

- These bridges are typically designed for pedestrian use, but compared to regular footbridges, they can span greater distances and are ideal for crossing deep valleys, rivers, or other natural barriers.
- Suspension footbridges are often built in natural parks, hiking trails, or areas that require crossing significant natural obstacles.

3. Load Characteristics:

- Suspension footbridges generally carry less load than full-scale suspension bridges (designed for vehicles or trains), as they are only built to support pedestrians. However, they still rely on suspended cables to carry the load.
- The load on these bridges is primarily pedestrian traffic, but in some cases, they might also support bicycles or animals.

4. Advantages:

- Longer Spans:** Suspension footbridges can cover longer distances than regular pedestrian bridges without requiring additional supports in the middle of the span.
- Crossing Natural Obstacles:** These bridges are ideal for crossing rivers, valleys, and rugged terrain where building traditional bridges is difficult.
- Aesthetic and Integration with Nature:** Suspension footbridges often have a beautiful design that blends well with the natural surroundings, especially in tourist or recreational areas.

5. Construction and Installation:

- Building a suspension footbridge requires specialized equipment for installing cables and towers. These bridges are commonly used in areas where the terrain is difficult, or access to traditional infrastructure is limited.
- Installing cables and towers in such challenging locations requires precise engineering and safety considerations.

Examples: Suspension Foot Bridges in Mountainous Areas: Suspension footbridges are commonly found in mountainous regions or nature reserves where they allow pedestrians to cross valleys and rivers. These bridges provide access for both locals and tourists to difficult-to-reach areas.

2. Suspension Footbridges in Parks:

In some natural parks or recreational areas, suspension footbridges are used to cross rivers or valleys. These bridges serve both a practical purpose and act as tourist attractions.

Differences from Regular Suspension Bridges:

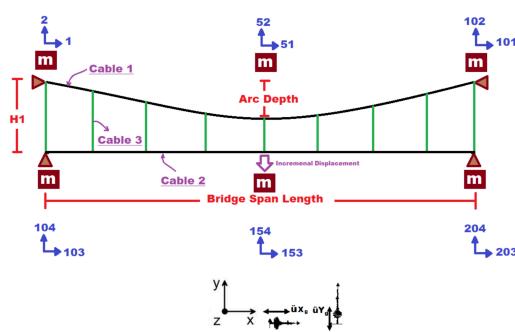
- Lower Load Capacity:** While regular suspension bridges are designed to carry vehicles, trains, or heavy loads, and can bear significant stress, suspension footbridges are designed only for pedestrian use and therefore carry much lighter loads.
- Smaller Dimensions:** Suspension footbridges are generally smaller in scale and have shorter spans than full-size suspension bridges. They are primarily intended for pedestrians and are rarely used for vehicle traffic.

Conclusion: A Suspension Foot Bridge is a type of suspension bridge specifically designed for pedestrian use. Due to the use of suspended cables, these bridges can span longer distances and are very useful in areas where crossing valleys, rivers, or difficult terrain is necessary.

```
In [32]: # Load the image
def PLOT_IMAGE(image):
    import matplotlib.pyplot as plt
    import matplotlib.image as mpimg
    image = mpimg.imread(image_path)

    # Display the image
    plt.figure(figsize=(15, 8))
    plt.imshow(image)
    plt.axis('off') # Hide axes
    plt.show()

image_path = 'OPENSEES_CABLE_SUSPENSION_BRIDGE_01.png'
PLOT_IMAGE(image_path)
```



```
In [33]: # WIKIPEDIA: Simple suspension bridge
'https://en.wikipedia.org/wiki/Simple_suspension_bridge'
# IMAGE:
'https://www.waratahbridgeconstructions.com.au/pedestrian-bridges-north-east-victoria.html'
'https://www.moodie.com.au/?product=cable-bridges'
```

```
# PAPER: Study on Suspension Bridge: Analysis and Construction criteria
'https://www.ijsdr.org/papers/IJSDR1704077.pdf'
```

```
Out[33]: 'https://www.ijsdr.org/papers/IJSDR1704077.pdf'
```

```
In [34]: #import the os module
import os
import time
import numpy as np
import openseespy.opensees as op
import matplotlib.pyplot as plt
```

```
In [35]: #to create a directory at specified path with name "Data"
os.mkdir('C:\\OPENSEESPY_SALAR')
```

```
-----
FileExistsError          Traceback (most recent call last)
Cell In[35], line 2
  1 #to create a directory at specified path with name "Data"
----> 2 os.mkdir('C:\\OPENSEESPY_SALAR')

FileExistsError: [WinError 183] Cannot create a file when that file already exists: 'C:\\OPENSEESPY_SALAR'
```

```
In [36]: FOLDER_NAME = 'OPENSEES_CABLE_SUSPENSION_BRIDGE'
dir = f"C:\\OPENSEESPY_SALAR\\{FOLDER_NAME}\\"
if not os.path.exists(dir):
    os.makedirs(dir)
```

```
In [37]: # OUTPUT DATA ADDRESS:
SALAR_DIR = f'C://OPENSEESPY_SALAR//{FOLDER_NAME}/'
```

```
In [38]: ## DELETE ALL FILES IN DIRECTORY
def DELETE_FOLDER_CONTANTS(folder_path):
    import os
    for filename in os.listdir(folder_path):
        file_path = os.path.join(folder_path, filename)
        try:
            if os.path.isfile(file_path) or os.path.islink(file_path):
                os.unlink(file_path)
        except Exception as e:
            print(f'Failed to delete {file_path}. Reason: {e}')
    print("Deletion done")

FOLDER_PATH = f'C:\\OPENSEESPY_SALAR\\{FOLDER_NAME}' # Specify the folder path
#DELETE_FOLDER_CONTANTS(FOLDER_PATH)
```

```
In [39]: def CURRENT_TIME():
    import time
    t = time.localtime()
    current_time = time.strftime("%H:%M:%S", t)
    print(f"Current time (HH:MM:SS): {current_time}\n\n")

# -----
def plot_shapes(initial_coords, displacements):
    import numpy as np
    import matplotlib.pyplot as plt
    """
    Plot the initial and deformed shapes of the arch.

    Parameters:
    - initial_coords: Initial coordinates of the nodes.
    - displacements: List of displacements for each node.
    """
    # Plot initial and deformed shape
    plt.figure(figsize=(12, 8))
    plt.plot(initial_coords[:num_nodes, 0], initial_coords[:num_nodes, 1], 'bo-', label='Initial Cable Shape')
    plt.plot(deformed_coords[:num_nodes, 0], deformed_coords[:num_nodes, 1], 'ro-', label='Deformed Cable Shape')
    plt.plot(initial_coords[num_nodes:, 0], initial_coords[num_nodes:, 1], 'go-', label='Initial Deck Shape')
    plt.plot(deformed_coords[num_nodes:, 0], deformed_coords[num_nodes:, 1], 'yo-', label='Deformed Deck Shape')

    # Plot connecting elements between the cable and the beam
    for i in range(num_nodes):
        plt.plot([initial_coords[i, 0], initial_coords[num_nodes + i, 0]],
                 [initial_coords[i, 1], initial_coords[num_nodes + i, 1]], 'c--', label='Initial Connecting Elements' if i == 0 else '')
        plt.plot([deformed_coords[i, 0], deformed_coords[num_nodes + i, 0]],
                 [deformed_coords[i, 1], deformed_coords[num_nodes + i, 1]], 'm--', label='Deformed Connecting Elements' if i == 0 else '')

    # Mark simply supported nodes with red triangles
    support_nodes = [0, num_nodes - 1, num_nodes, 2*num_nodes - 1]
    plt.plot(initial_coords[support_nodes, 0], initial_coords[support_nodes, 1], 'r^', markersize=20, label='Simply Supported Nodes')

    plt.xlabel('X [mm]')
    plt.ylabel('Y [mm]')
    plt.legend()
    plt.title('Pushover Analysis: Cable and Deck Deformed Shapes')
    plt.show()

# ----

def plot_reactions(disp_x, reaction_x, disp_y, reaction_y):
    import matplotlib.pyplot as plt
    """
    Plot the base reaction versus displacement.

    Parameters:
    - disp_x: List of x displacements of the middle node.
    - reaction_x: List of base reactions in x.
    - disp_y: List of y displacements of the middle node.
    - reaction_y: List of base reactions in y.
    """
    plt.figure(figsize=(10, 6))
    plt.plot(disp_x, reaction_x, 'b-')
    plt.xlabel('Displacement X [mm]')
    plt.ylabel('Base Reaction X [N]')
    plt.title('Base Reaction X vs. Displacement X')
    plt.show()

    plt.figure(figsize=(10, 6))
    plt.plot(disp_y, reaction_y, 'r-')
    plt.xlabel('Displacement Y [mm]')
    plt.ylabel('Base Reaction Y [N]')
    plt.title('Base Reaction Y vs. Displacement Y')
```

```

plt.title('Base Reaction Y vs. Displacement Y')
plt.show()

# -----
"""

When OK equals -1, it generally indicates that the command or operation was not executed
because it was already in progress or had already been completed. This can happen if you
try to run a command that is already running or has been completed in a previous step.

When OK equals -2, it typically indicates that the command or operation was not executed
because it was not recognized or not implemented. This could mean that the command
is either misspelled, not available in the current version of OpenSees, or not applicable to the current context.

When OK equals -3, it typically means that the command or operation failed.
This could be due to various reasons, such as incorrect input parameters,
syntax errors, or issues with the model setup.
"""

def ANALYSIS(OK, INCREMENT, TOLERANCE, MAX_ITERATIONS):
    import openseespy.opensees as op
    test = {1:'NormDispIncr', 2: 'RelativeEnergyIncr', 4: 'RelativeNormUnbalance', 5: 'RelativeNormDispIncr', 6: 'NormUnbalance'}
    algorithm = {1:'KrylovNewton', 2: 'SecantNewton', 4: 'RaphsonNewton', 5: 'PeriodicNewton', 6: 'BFGS', 7: 'Broyden', 8: 'NewtonLineSearch'}

    for i in test:
        for j in algorithm:
            if OK != 0:
                if j < 4:
                    op.algorithm(algorithm[j], '-initial')

            else:
                op.algorithm(algorithm[j])

            op.test(test[i], TOLERANCE, MAX_ITERATIONS)
            OK = op.analyze(INCREMENT)
            print(test[i], algorithm[j], OK)
            if OK == 0:
                break
            else:
                continue

```

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In [ ]: # ##### IN THE NAME OF ALLAH #####
# # PUSHOVER ANALYSIS OF CABLE SUSPENSION BRIDGE 01 #
# #-#
# # THIS PROGRAM WRITTEN BY SALAR DELAVAR GHASHGHAEI (QASHQAI) #
# # EMAIL: salar.d.ghashghaei@gmail.com #
# ##### #####

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In [40]: # -----
# # PUSHOVER ANALYSIS
# -----
def PUSHOVER_ANALYSIS(LINEAR, L, H1, arc_depth, E_cable, Cable_Dia_01, Cable_Dia_02, Cable_Dia_03, num_nodes, MAX_DISP, disp_incr, MAX_ITERATIONS, TOLERANCE):
    import openseespy.opensees as ops
    import numpy as np
    import matplotlib.pyplot as plt

    A_cable_01 = (np.pi * Cable_Dia_01 **2) / 4 # [mm^2] Horizontal Longitudinal Cable Area Bottom
    A_cable_02 = (np.pi * Cable_Dia_02 **2) / 4 # [mm^2] Horizontal Longitudinal Cable Area Top
    A_cable_03 = (np.pi * Cable_Dia_03 **2) / 4 # [mm^2] Vertical Longitudinal Cable Area Top

    # Define model builder
    ops.wipe()
    ops.model('basic', '-ndm', 2, '-ndf', 2)
    dx = L / (num_nodes - 1)

    # Create nodes
    for i in range(num_nodes):
        x = i * dx
        y = H1 - arc_depth * np.sin(np.pi * x / L)
        ops.node(i + 1, x, y)
        ops.node(num_nodes + i + 1, x * dx, 0.0)

    # Define boundary conditions (fixed at both ends)
    ops.fix(1, 1, 1) # TOP CABLE
    ops.fix(num_nodes, 1, 1) # TOP CABLE
    ops.fix(num_nodes + 1, 1, 1) # BOTTOM CABLE - DECK
    ops.fix(2 * num_nodes, 1, 1) # BOTTOM CABLE - DECK

    # Define material properties
    if LINEAR == True:
        ops.uniaxialMaterial('Elastic', 1, E_cable)
        # TENSION COMPRESSION
        #ops.uniaxialMaterial('Elastic', 1, E_cable, 0, 0.5 * E_cable)
    if LINEAR == False:
        Fy_cable = 355 # [N/mm^2] Yield strength of the cable
        b0 = 0.01
        ops.uniaxialMaterial('Steel01', 1, Fy_cable, E_cable, b0)

    # Define truss elements
    for i in range(num_nodes - 1):
        ops.element('corotTruss', i + 1, i + 1, i + 2, A_cable_02, 1) # Cable element
        ops.element('corotTruss', num_nodes + i + 1, num_nodes + i + 1, num_nodes + i + 2, A_cable_01, 1) # Deck element

    # Connect each node of the cable with the corresponding node of the deck using truss elements
    for i in range(num_nodes):
        ops.element('corotTruss', 2 * num_nodes + i + 1, i + 1, num_nodes + i + 1, A_cable_03, 1)

    #mid_node = num_nodes // 2 + 1
    mid_node = int(num_nodes + 0.5 * num_nodes)
    # Define load pattern with displacement at the middle span
    ops.timeSeries('Linear', 1)
    ops.pattern('Plain', 1, 1)
    ops.load(mid_node, 0.0, -1) # Apply a horizontal Load at node
    disp_incr = 0.01 # [mm] Incremental Displacement
    n_steps = int(np.abs(MAX_DISP / disp_incr)) # Analysis Steps

    # Define analysis parameters
    ops.system('BandGeneral')
    ops.numberer('Plain')
    ops.constraints('Plain')
    ops.integrator('DisplacementControl', mid_node, 2, -disp_incr)
    ops.test('EnergyIncr', TOLERANCE, MAX_ITERATIONS)
    ops.algorithm('ModifiedNewton')

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ops.analysis('Static')

# OUTPUT DATA
ops.recorder('Node', '-file', f'{SALAR_DIR}DTH_PUSH.txt', '-time', '-node', mid_node, '-dof', 1, 2, 'disp')# Displacement Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_PUSH_01.txt', '-time', '-node', 1, '-dof', 1, 2, 'reaction')# Base Shear Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_PUSH_02.txt', '-time', '-node', num_nodes, '-dof', 1, 2, 'reaction')# Base Shear Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_PUSH_03.txt', '-time', '-node', num_nodes + 1, '-dof', 1, 2, 'reaction')# Base Shear Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_PUSH_04.txt', '-time', '-node', 2*num_nodes, '-dof', 1, 2, 'reaction')# Base Shear Time History

DISPLACEMENTS = []
DISP_X, DISP_Y = [], []
BASEREACTION_X, BASEREACTION_Y = [], []

# Perform the analysis with increments
for i in range(n_steps):
    OK = ops.analyze(1)
    ANALYSIS(OK, 1, TOLERANCE, MAX_ITERATIONS)
    DISPLACEMENTS.append([ops.nodeDisp(j + 1) for j in range(num_nodes * 2)])
    DISP_X.append(ops.nodeDisp(mid_node, 1))
    DISP_Y.append(ops.nodeDisp(mid_node, 2))
    x1 = ops.nodeResponse(1, 1, 6)
    x2 = ops.nodeResponse(num_nodes, 1, 6)
    x3 = ops.nodeResponse(num_nodes + 1, 1, 6)
    x4 = ops.nodeResponse(2 * num_nodes, 1, 6)

    y1 = ops.nodeResponse(1, 2, 6)
    y2 = ops.nodeResponse(num_nodes, 2, 6)
    y3 = ops.nodeResponse(num_nodes + 1, 2, 6)
    y4 = ops.nodeResponse(2 * num_nodes, 2, 6)
    BASEREACTION_X.append(x1+x2+x3+x4) # CABLE REACION-X
    BASEREACTION_Y.append(y1+y2+y3+y4) # CABLE REACION-Y
    #print(f'STEP: {i+1}')

# Get initial and final node coordinates for plotting
initial_coords = np.array([ops.nodeCoord(i + 1) for i in range(num_nodes * 2)])
deformed_coords = initial_coords + np.array(DISPLACEMENTS[-1])

# Output all unconstrained node displacements
for i in range(num_nodes * 2):
    disp = ops.nodeDisp(i + 1)
    print(f'Node {i + 1}: X Displacement = {disp[0]}, Y Displacement = {disp[1]}')

return initial_coords, deformed_coords, DISPLACEMENTS, DISP_X, DISP_Y, BASEREACTION_X, BASEREACTION_Y

```

```

In [41]: # -----
#   PUSHOVER ANALYSIS
# -----
# Parameters for the analysis
L = 50000.0      # [mm] Bridge span Length
H1 = 3500.0       # [mm] Height of Top Cable
arc_depth = 1000.0 # [mm]
E_cable = 210e5   # [N/mm^2] Modulus of elasticity Cable
Cable_Dia_01 = 25 # [mm] Horizontal Longitudinal Cable Diameter Bottom
Cable_Dia_02 = 18 # [mm] Horizontal Longitudinal Cable Diameter Top
Cable_Dia_03 = 10 # [mm] Vertical Longitudinal Cable Diameter Top
num_nodes = 51    # Cable Arc Number of nodes
MAX_DISP = 500.0  # [mm] Maximum Displacement
disp_incr = -0.01 # [mm] Incremental Displacement

MAX_ITERATIONS = 50000 # Maximum number of iterations
TOLERANCE = 1.0e-10    # Tolerance for convergence

LINEAR = True # False: Cable NonLinear Materials Properties

starttime = time.process_time()
# Run the analysis
results = PUSHOVER_ANALYSIS(LINEAR, L, H1, arc_depth, E_cable, Cable_Dia_01, Cable_Dia_02, Cable_Dia_03, num_nodes, MAX_DISP, disp_incr, MAX_ITERATIONS, TOLERANCE)
initial_coords, deformed_coords, DISPLACEMENTS, DISP_X, DISP_Y, BASEREACTION_X, BASEREACTION_Y = results
totaltime = time.process_time() - starttime
print(f'\nTotal time (s): {totaltime:.4f} \n\n')

WARNING: CTestEnergyIncr::test() - failed to converge
after: 50000 iterations
current EnergyIncr: -nan(ind) (max: 1e-10)      Norm deltaX: nan, Norm deltaR: nan
ModifiedNewton::solveCurrentStep() -the ConvergenceTest object failed in test()
StaticAnalysis::analyze() - the Algorithm failed at step: 0 with domain at load factor -nan(ind)
OpenSees > analyze failed, returned: -3 error flag

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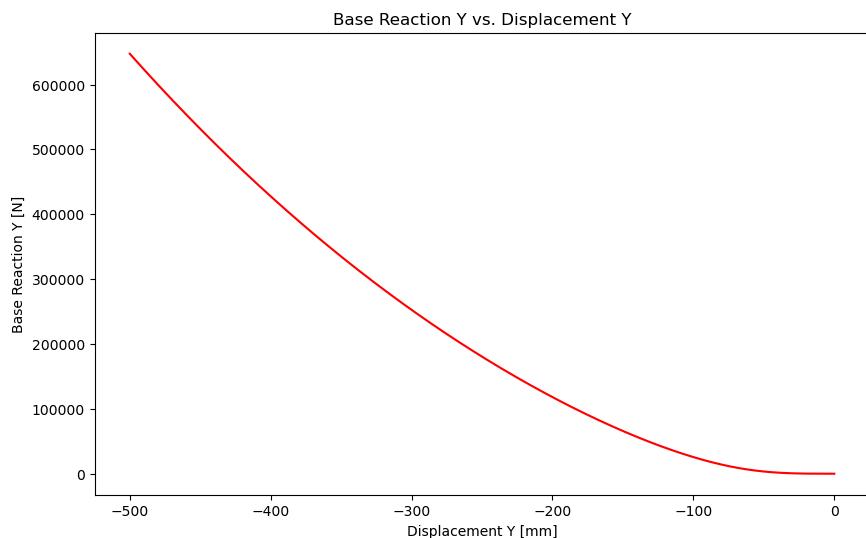
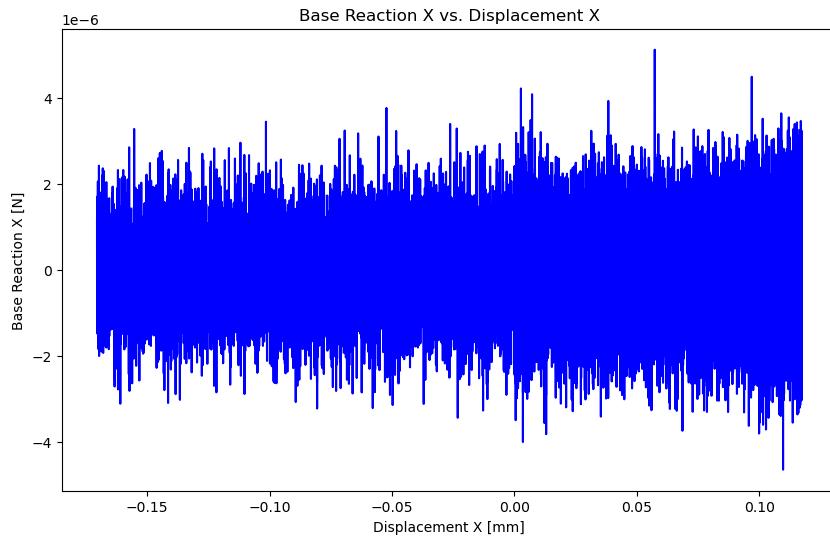
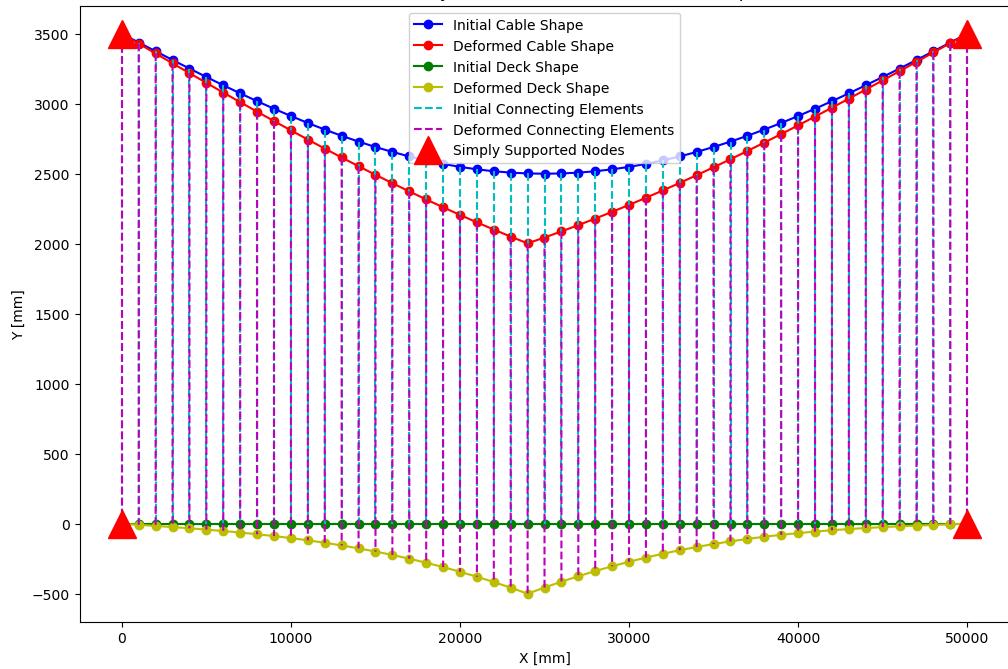
NormDispIncr KrylovNewton 0
Node 1: X Displacement = 0.0, Y Displacement = 0.0
Node 2: X Displacement = 0.3199093321564988, Y Displacement = -7.806389908584831
Node 3: X Displacement = 0.631027760133854, Y Displacement = -15.764664966975404
Node 4: X Displacement = 0.924755455256964, Y Displacement = -24.826106496111915
Node 5: X Displacement = 1.1928702307870171, Y Displacement = -32.74081689215971
Node 6: X Displacement = 1.4277073923384542, Y Displacement = -42.057135691408995
Node 7: X Displacement = 1.622332110862012, Y Displacement = -52.12106165685267
Node 8: X Displacement = 1.770698284729665, Y Displacement = -63.07568290146361
Node 9: X Displacement = 1.8677931008152302, Y Displacement = -75.06061701609693
Node 10: X Displacement = 1.9097642032318738, Y Displacement = -88.21146317507215
Node 11: X Displacement = 1.8940273651560144, Y Displacement = -102.65926820367797
Node 12: X Displacement = 1.8193527683809405, Y Displacement = -118.53008860839629
Node 13: X Displacement = 1.6859283256916666, Y Displacement = -135.94409059115955
Node 14: X Displacement = 1.4953988381728838, Y Displacement = -155.01587009100749
Node 15: X Displacement = 1.250880158343515, Y Displacement = -175.85319491640485
Node 16: X Displacement = 0.956947925340774, Y Displacement = -198.556971044306
Node 17: X Displacement = 0.6196008446537753, Y Displacement = -223.2207551615983
Node 18: X Displacement = 0.2461988961689911, Y Displacement = -249.93037508380964
Node 19: X Displacement = -0.1546227357329748, Y Displacement = -278.7635829973928
Node 20: X Displacement = -0.5730628141799169, Y Displacement = -309.78973462838667
Node 21: X Displacement = -0.9982847005899349, Y Displacement = -343.0695108359222
Node 22: X Displacement = -1.4185665547848574, Y Displacement = -378.6546685562685
Node 23: X Displacement = -1.821468717721201, Y Displacement = -416.58782978268636
Node 24: X Displacement = -2.1939860062174326, Y Displacement = -456.9017142816989
Node 25: X Displacement = -2.511670554771266, Y Displacement = -499.3906115988735
Node 26: X Displacement = -2.547112242136458, Y Displacement = -455.62603297097127
Node 27: X Displacement = -2.657280625809106, Y Displacement = -414.053153571662
Node 28: X Displacement = -2.8191839800275544, Y Displacement = -374.90342719373695
Node 29: X Displacement = -3.0193758337812246, Y Displacement = -338.1565908826925
Node 30: X Displacement = -3.244595967746345, Y Displacement = -303.79090227054826
Node 31: X Displacement = -3.4819469820560487, Y Displacement = -271.7730186968144
Node 32: X Displacement = -3.7190877816138976, Y Displacement = -242.06029416544825
Node 33: X Displacement = -3.9444152535021972, Y Displacement = -214.60095197313652
Node 34: X Displacement = -4.147231127818948, Y Displacement = -189.3342954123924
Node 35: X Displacement = -4.3178911598800236, Y Displacement = -166.19095535900598
Node 36: X Displacement = -4.447937265183557, Y Displacement = -145.0931733086479
Node 37: X Displacement = -4.530201541491634, Y Displacement = -125.95511822781721
Node 38: X Displacement = -4.558895929139094, Y Displacement = -108.68323542832194
Node 39: X Displacement = -4.529670959493224, Y Displacement = -93.17662556083904
Node 40: X Displacement = -4.439652018139383, Y Displacement = -79.32745174773434
Node 41: X Displacement = -4.287449668585613, Y Displacement = -67.02137283210837
Node 42: X Displacement = -4.0731444585121741, Y Displacement = -56.13800070193991
Node 43: X Displacement = -3.7982469701719217, Y Displacement = -46.55137964796155
Node 44: X Displacement = -3.4656329670143715, Y Displacement = -38.13048572492054
Node 45: X Displacement = -3.0794564775824953, Y Displacement = -30.739744102436582
Node 46: X Displacement = -2.6450407523323833, Y Displacement = -24.239562409355027
Node 47: X Displacement = -2.1687499699352424, Y Displacement = -18.486878891067872
Node 48: X Displacement = -1.6578435644342842, Y Displacement = -13.335717810695044
Node 49: X Displacement = -1.1203156825277303, Y Displacement = -8.637766931193015
Node 50: X Displacement = -0.5647224603638313, Y Displacement = -4.242947115834292
Node 51: X Displacement = 0.0, Y Displacement = 0.0
Node 52: X Displacement = 0.0, Y Displacement = 0.0
Node 53: X Displacement = 0.24112605465504822, Y Displacement = -7.807274869822665
Node 54: X Displacement = 0.48105567210462, Y Displacement = -15.766397713668386
Node 55: X Displacement = 0.7185279680729767, Y Displacement = -24.02864811754546
Node 56: X Displacement = 0.9521548079230172, Y Displacement = -32.744127433061436
Node 57: X Displacement = 1.1803604652083914, Y Displacement = -42.06117471138081
Node 58: X Displacement = 1.4013251633564807, Y Displacement = -52.1257881530226
Node 59: X Displacement = 1.6129337000797217, Y Displacement = -63.081054631283514
Node 60: X Displacement = 1.8127302728272177, Y Displacement = -75.06658928193129
Node 61: X Displacement = 1.9978805244068158, Y Displacement = -88.2198725348128
Node 62: X Displacement = 2.1651417115164655, Y Displacement = -102.6662897199856
Node 63: X Displacement = 2.310841766928375, Y Displacement = -118.53746625716987
Node 64: X Displacement = 2.4308678803800152, Y Displacement = -135.95191565820377
Node 65: X Displacement = 2.520665066063016, Y Displacement = -155.02398723317057
Node 66: X Displacement = 2.575245018423436, Y Displacement = -175.8615245749926
Node 67: X Displacement = 2.5892053854758474, Y Displacement = -198.56543370282027
Node 68: X Displacement = 2.5567594127425624, Y Displacement = -223.22927739674802
Node 69: X Displacement = 2.4717757340734416, Y Displacement = -249.9388974592113
Node 70: X Displacement = 2.3278279106821866, Y Displacement = -278.772066487375
Node 71: X Displacement = 2.118253149343795, Y Displacement = -309.79817069104934
Node 72: X Displacement = 1.8362194672132621, Y Displacement = -343.07792514772893
Node 73: X Displacement = 1.4748004161407329, Y Displacement = -378.6631228239429
Node 74: X Displacement = 1.027056242454488, Y Displacement = -416.59642107811
Node 75: X Displacement = -0.4860821835797728, Y Displacement = -456.9121116977464
Node 76: X Displacement = -0.17053444703073659, Y Displacement = -499.999999999690826
Node 77: X Displacement = -0.882876685547187, Y Displacement = -455.63735237292235
Node 78: X Displacement = -1.4752667052045902, Y Displacement = -414.0653383497389
Node 79: X Displacement = -1.9699453163433748, Y Displacement = -374.9135419889855
Node 80: X Displacement = -2.37334110328229, Y Displacement = -338.1667819131928
Node 81: X Displacement = -2.692066666837616, Y Displacement = -303.80094547678414
Node 82: X Displacement = -2.932865023831619, Y Displacement = -271.78294601563357
Node 83: X Displacement = -3.1025195889647424, Y Displacement = -242.07006185542613
Node 84: X Displacement = -3.207768280920555, Y Displacement = -214.61051514818658
Node 85: X Displacement = -3.255223075297706, Y Displacement = -189.34360697575613
Node 86: X Displacement = -3.2512962214099574, Y Displacement = -166.1996759809286
Node 87: X Displacement = -3.2021341003860795, Y Displacement = -145.1018406343705
Node 88: X Displacement = -3.1135596365658453, Y Displacement = -125.9633987017544
Node 89: X Displacement = -2.991023985646228, Y Displacement = -108.6910972796879
Node 90: X Displacement = -2.8395680679613786, Y Displacement = -93.18404073981579
Node 91: X Displacement = -2.6637943431529436, Y Displacement = -79.33439979813383
Node 92: X Displacement = -2.4678490466121494, Y Displacement = -67.02783774405411
Node 93: X Displacement = -2.255414930161023, Y Displacement = -56.14396812890243
Node 94: X Displacement = -2.029714372321433, Y Displacement = -46.5683379065218
Node 95: X Displacement = -1.793522550832521, Y Displacement = -38.135406366880446
Node 96: X Displacement = -1.5491901971652293, Y Displacement = -30.744104226997216
Node 97: X Displacement = -1.29867531605593, Y Displacement = -24.24332672229692
Node 98: X Displacement = -1.043583065104162, Y Displacement = -18.4900263888983
Node 99: X Displacement = -0.785212997940771, Y Displacement = -13.338150736543284
Node 100: X Displacement = -0.5246120109768905, Y Displacement = -8.639450284798523
Node 101: X Displacement = -0.2626337237928343, Y Displacement = -4.243819502893362
Node 102: X Displacement = 0.0, Y Displacement = 0.0

```

Total time (s): 89.4219

```
In [42]: # Plot results
plot_shapes(initial_coords, DISPLACEMENTS)
plot_reactions(DISP_X, BASEREACTION_X, DISP_Y, BASEREACTION_Y)
```

Pushover Analysis: Cable and Deck Deformed Shapes



```
In [24]: # #####
# #####
# IN THE NAME OF ALLAH #####
# FREE VIBRATION ANALYSIS OF CABLE SUSPENSION BRIDGE 01 #####
# - - - - - #####
# THIS PROGRAM WRITTEN BY SALAR DELAVAR GHASHGHAEI (QASHQAI) #####
#
```

```

#           #           EMAIL: solar.d.ghashghaei@gmail.com           #
#           #####                                                 #####
In [25]: # -----
#   FREE VIBRATION ANALYSIS
# -----
def FREE_VIBRATION_ANALYSIS(damping, damping_ratio, LINEAR, duration, dt, TOTAL_MASS, u0, L, H1, arc_depth, E_cable, Cable_Dia_01, Cable_Dia_02, Cable_Dia_03, num_nodes, MAX_ITERATIONS):
    import openseespy.opensees as ops
    import numpy as np
    import matplotlib.pyplot as plt

    A_cable_01 = (np.pi * Cable_Dia_01 **2) / 4 # [mm^2] Horizontal Longitudinal Cable Area Bottom
    A_cable_02 = (np.pi * Cable_Dia_02 **2) / 4 # [mm^2] Horizontal Longitudinal Cable Area Top
    A_cable_03 = (np.pi * Cable_Dia_03 **2) / 4 # [mm^2] Vertical Longitudinal Cable Area Top

    # Define model builder
    ops.wipe()
    ops.model('basic', '-ndm', 2, '-ndf', 2)
    KE = 1;
    dx = L / (num_nodes - 1)
    MASS = TOTAL_MASS / num_nodes
    # Create nodes
    for i in range(num_nodes):
        X = i * dx
        Y1 = H1 - arc_depth * np.sin(np.pi * X / L)
        ops.node(i + 1, X, Y1)
        ops.node(num_nodes + i + 1, i * dx, 0.0)
    # Define mass
    ops.mass(num_nodes + i + 1, MASS, MASS, 0)

    # Define boundary conditions (fixed at both ends)
    ops.fix(1, 1, 1) # TOP CABLE
    ops.fix(num_nodes, 1, 1) # TOP CABLE
    ops.fix(num_nodes + 1, 1, 1) # BOTTOM CABLE
    ops.fix(2 * num_nodes, 1, 1) # BOTTOM CABLE

    # Define material properties
    if LINEAR == True:
        ops.uniaxialMaterial('Elastic', 1, E_cable)
        #                                         TENSION      COMPRESSION
        #ops.uniaxialMaterial('Elastic', 1, E_cable, 0, 0.5 * E_cable)
    if LINEAR == False:
        Fy_cable = 355 # [N/mm^2] Yield strength of the cable
        b0 = 0.01
        ops.uniaxialMaterial('Steel01', 1, Fy_cable, E_cable, b0)

    # Define truss elements
    for i in range(num_nodes - 1):
        ops.element('corotTruss', i + 1, i + 1, i + 2, A_cable_02, 1) # Top Cable element
        ops.element('corotTruss', num_nodes + i + 1, num_nodes + i + 1, num_nodes + i + 2, A_cable_01, 1) # Bottom Cable Element

    # Connect each node of the cable with the corresponding node of the deck using truss elements
    for i in range(num_nodes):
        ops.element('corotTruss', 2 * num_nodes + i + 1, i + 1, num_nodes + i + 1, A_cable_03, 1) # Vertical Cable Element

    #mid_node = num_nodes // 2 + 1
    mid_node = int(num_nodes * 0.5 * num_nodes)
    # Define load pattern with displacement at the middle span
    ops.timeSeries('Linear', 1)
    ops.pattern('Plain', 1, 1)
    ops.load(mid_node, 0.0, -1) # Apply a horizontal load at node
    ops.constraints('Transformation')
    ops.numberer('RCM')
    ops.system('BandGeneral')
    ops.algorithm('Newton')
    ops.test('EnergyIncr', TOLERANCE, MAX_ITERATIONS)
    ops.integrator('DisplacementControl', mid_node, 2, u0)
    ops.analysis('Static')
    ops.analyze(1)

    ops.setTime(0.0)

    # Wipe analysis and reset time
    ops.wipeAnalysis()
    ops.remove('loadpattern', 1)
    ops.system('UmfPack')

    # Dynamic analysis
    ops.constraints('Transformation')
    ops.numberer('RCM')
    ops.system('UmfPack')
    ops.test('EnergyIncr', TOLERANCE, MAX_ITERATIONS)
    ops.integrator('Newmark', 0.5, 0.25)
    ops.algorithm('Newton')

    if damping == True:
        # Calculate Rayleigh damping factors
        omega1 = np.sqrt(KE / TOTAL_MASS)
        omega2 = 2 * omega1 # Just an assumption for two modes
        a0 = damping_ratio * (2 * omega1 * omega2) / (omega1 + omega2)
        a1 = damping_ratio * 2 / (omega1 + omega2)
        # Apply Rayleigh damping
        ops.rayleigh(a0, a1, 0, 0)

    ops.analysis('Transient')

    # OUTPUT DATA
    ops.recorder('Node', '-file', f'{SALAR_DIR}DTH_DYN.txt', '-time', '-node', mid_node, '-dof', 1, 2, 'disp')# Displacement Time History
    ops.recorder('Node', '-file', f'{SALAR_DIR}VTH_DYN.txt', '-time', '-node', mid_node, '-dof', 1, 2, 3, 'vel') # Velocity Time History
    ops.recorder('Node', '-file', f'{SALAR_DIR}ATH_DYN.txt', '-time', '-node', mid_node, '-dof', 1, 2, 3, 'accel') # Acceleration Time History
    ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_DYN_01.txt', '-time', '-node', 1, '-dof', 1, 2, 'reaction')# Base Shear Time History
    ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_DYN_02.txt', '-time', '-node', num_nodes, '-dof', 1, 2, 'reaction')# Base Shear Time History
    ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_DYN_03.txt', '-time', '-node', num_nodes + 1, '-dof', 1, 2, 'reaction')# Base Shear Time History
    ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_DYN_04.txt', '-time', '-node', 2 * num_nodes, '-dof', 1, 2, 'reaction')# Base Shear

    DISPLACEMENTS = []
    DISP_X, DISP_Y, VELOCITY, ACCELERATION = [], [], [], []
    BASEREACTION_X, BASEREACTION_Y = [], []

    stable = 0
    current_time = 0.0
    # Perform the analysis with increments

```

```

while stable == 0 and current_time < duration:
    OK = ops.analyze(1, dt)
    ANALYSIS(OK, 1, TOLERANCE, MAX_ITERATIONS)
    current_time = ops.getTime()
    DISPLACEMENTS.append([ops.nodeDisp(j + 1) for j in range(num_nodes * 2)])
    DISP_X.append(ops.nodeDisp(mid_node, 1))
    DISP_Y.append(ops.nodeDisp(mid_node, 2))
    VELOCITY.append(ops.nodeVel(mid_node, 2))
    ACCELERATION.append(ops.nodeAccel(mid_node, 2))
    x1 = ops.nodeResponse(1, 1, 6)
    x2 = ops.nodeResponse(num_nodes, 1, 6)
    x3 = ops.nodeResponse(num_nodes + 1, 1, 6)
    x4 = ops.nodeResponse(2 * num_nodes, 1, 6)
    y1 = ops.nodeResponse(1, 2, 6)
    y2 = ops.nodeResponse(num_nodes, 2, 6)
    y3 = ops.nodeResponse(num_nodes + 1, 2, 6)
    y4 = ops.nodeResponse(2 * num_nodes, 2, 6)
    BASE_REACTION_X.append(x1 + x2 + x3 + x4) # CABLE REACTION-X
    BASE_REACTION_Y.append(y1 + y2 + y3 + y4) # CABLE REACTION-Y
    KE = BASE_REACTION_Y[-1] / DISP_Y[-1] # Effective Lateral Stiffness
    #print(f'STEP: {i+1}')

# Get initial and final node coordinates for plotting
initial_coords = np.array([ops.nodeCoord(i + 1) for i in range(num_nodes * 2)])
deformed_coords = initial_coords + np.array(DISPLACEMENTS[-1])

# Output all unconstrained node displacements
for i in range(num_nodes * 2):
    disp = ops.nodeDisp(i + 1)
    print(f'Node {i + 1}: X Displacement = {disp[0]}, Y Displacement = {disp[1]}')

return initial_coords, deformed_coords, DISPLACEMENTS, VELOCITY, ACCELERATION, DISP_X, DISP_Y, BASE_REACTION_X, BASE_REACTION_Y

```

```
In [26]: # Define the plotting function
def plot_time_history(TIME, DISP_X_undamped, DISP_X_damped, DISP_Y_undamped, DISP_Y_damped,
                      VELOCITY_undamped, VELOCITY_damped, ACCELERATION_undamped, ACCELERATION_damped,
                      BASE_REACTION_X_undamped, BASE_REACTION_X_damped, BASE_REACTION_Y_undamped, BASE_REACTION_Y_damped):
    import matplotlib.pyplot as plt
    import numpy as np

    plt.figure(figsize=(14, 26))

    # Displacement X
    plt.subplot(6, 1, 1)
    P1 = np.max(np.abs(np.array(DISP_X_undamped)))
    P2 = np.max(np.abs(np.array(DISP_X_damped)))
    plt.plot(TIME, DISP_X_undamped, color='black', label=f'Undamped: {P1:.4e}')
    plt.plot(TIME, DISP_X_damped, color='red', label=f'Damped: {P2:.4e}')
    plt.xlabel('Time [s]')
    plt.ylabel('Displacement X [mm]')
    plt.legend()

    # Displacement Y
    plt.subplot(6, 1, 2)
    P1 = np.max(np.abs(np.array(DISP_Y_undamped)))
    P2 = np.max(np.abs(np.array(DISP_Y_damped)))
    plt.plot(TIME, DISP_Y_undamped, color='black', label=f'Undamped: {P1:.4e}')
    plt.plot(TIME, DISP_Y_damped, color='red', label=f'Damped: {P2:.4e}')
    plt.xlabel('Time [s]')
    plt.ylabel('Displacement Y [mm]')
    plt.legend()

    # Velocity
    plt.subplot(6, 1, 3)
    P1 = np.max(np.abs(np.array(VELOCITY_damped)))
    P2 = np.max(np.abs(np.array(VELOCITY_undamped)))
    plt.plot(TIME, VELOCITY_undamped, color='black', label=f'Undamped: {P1:.4e}')
    plt.plot(TIME, VELOCITY_damped, color='red', label=f'Damped: {P2:.4e}')
    plt.xlabel('Time [s]')
    plt.ylabel('Velocity [mm/s]')
    plt.legend()

    # Acceleration
    plt.subplot(6, 1, 4)
    P1 = np.max(np.abs(np.array(ACCELERATION_undamped)))
    P2 = np.max(np.abs(np.array(ACCELERATION_damped)))
    plt.plot(TIME, ACCELERATION_undamped, color='black', label=f'Undamped: {P1:.4e}')
    plt.plot(TIME, ACCELERATION_damped, color='red', label=f'Damped: {P2:.4e}')
    plt.xlabel('Time [s]')
    plt.ylabel('Acceleration [mm/s^2]')
    plt.legend()

    # Base Reaction X
    plt.subplot(6, 1, 5)
    P1 = np.max(np.abs(np.array(BASE_REACTION_X_undamped)))
    P2 = np.max(np.abs(np.array(BASE_REACTION_X_damped)))
    plt.plot(TIME, BASE_REACTION_X_undamped, color='black', label=f'Undamped: {P1:.4e}')
    plt.plot(TIME, BASE_REACTION_X_damped, color='red', label=f'Damped: {P2:.4e}')
    plt.xlabel('Time [s]')
    plt.ylabel('Base Reaction X [N]')
    plt.legend()

    # Base Reaction Y
    plt.subplot(6, 1, 6)
    P1 = np.max(np.abs(np.array(BASE_REACTION_Y_undamped)))
    P2 = np.max(np.abs(np.array(BASE_REACTION_Y_damped)))
    plt.plot(TIME, BASE_REACTION_Y_undamped, color='black', label=f'Undamped: {P1:.4e}')
    plt.plot(TIME, BASE_REACTION_Y_damped, color='red', label=f'Damped: {P2:.4e}')
    plt.xlabel('Time [s]')
    plt.ylabel('Base Reaction Y [N]')
    plt.legend()

    plt.suptitle('Time History of Free Vibration Analysis: Damped vs Undamped')
    plt.tight_layout(rect=[0, 0.03, 1, 0.95])
    plt.show()
```

```
In [27]: # -----
# FREE VIBRATION ANALYSIS
# -----
# Parameters for the analysis
L = 5000.0          # [mm] Bridge span length
H1 = 3500.0          # [mm] Height of Top Cable
```

```

arc_depth = 1000.0 # [mm]
E_cable = 210e5 # [N/mm2] Modulus of elasticity Cable
Cable Dia_01 = 25 # [mm] Horizontal Longitudinal Cable Diameter Bottom
Cable Dia_02 = 18 # [mm] Horizontal Longitudinal Cable Diameter Top
Cable Dia_03 = 10 # [mm] Vertical Longitudinal Cable Diameter Top
num_nodes = 51 # Cable Arc Number of nodes

TOTAL_MASS = 500000.0 # [kg] Total Mass of Structure
u0 = 2.0 # [mm] Initial displacement
damping_ratio = 0.05 # Damping ratio
duration = 50.0 # [s] Duration of the analysis in seconds
dt = 0.01 # Time step in seconds

MAX_ITERATIONS = 10000 # Maximum number of iterations
TOLERANCE = 1.0e-14 # Tolerance for convergence

import time
starttime = time.process_time()

# Run the undamped analysis
damping = False
LINEAR = True # False: Cable Nonlinear Materials Properties
results_undamped = FREE_VIBRATION_ANALYSIS(damping, damping_ratio, LINEAR, duration, dt, TOTAL_MASS, u0, L, H1, arc_depth, E_cable, Cable Dia_01, Cable Dia_02, Cable Dia_03, num_nodes, initial_coords, deformed_coords, DISPLACEMENTS_undamped, VELOCITY_undamped, ACCELERATION_undamped, DISP_X_undamped, DISP_Y_undamped, BASEREACTION_X_undamped, BASEREACTION_Y_undamped)

# Run the damped analysis
damping = True
LINEAR = True # False: Cable Nonlinear Materials Properties
results_damped = FREE_VIBRATION_ANALYSIS(damping, damping_ratio, LINEAR, duration, dt, TOTAL_MASS, u0, L, H1, arc_depth, E_cable, Cable Dia_01, Cable Dia_02, Cable Dia_03, num_nodes, initial_coords, deformed_coords, DISPLACEMENTS_damped, VELOCITY_damped, ACCELERATION_damped, DISP_X_damped, DISP_Y_damped, BASEREACTION_X_damped, BASEREACTION_Y_damped = results_undamped

totaltime = time.process_time() - starttime
print(f'\nTotal time (s): {totaltime:.4f} \n\n')

```

Node 1: X Displacement = 0.0, Y Displacement = 0.0
 Node 2: X Displacement = -0.00600880285929951325, Y Displacement = -0.09561323463466348
 Node 3: X Displacement = -0.011861368101459054, Y Displacement = -0.18913514782977003
 Node 4: X Displacement = -0.01740905180190132, Y Displacement = -0.2784266326808296
 Node 5: X Displacement = -0.02250749531421594, Y Displacement = -0.3615891846792667
 Node 6: X Displacement = -0.027024029350335637, Y Displacement = -0.43641275510935623
 Node 7: X Displacement = -0.03385509486112691, Y Displacement = -0.5009441186472807
 Node 8: X Displacement = -0.03385509486112691, Y Displacement = -0.5532146526426193
 Node 9: X Displacement = -0.03598769314098611, Y Displacement = -0.5913041389681727
 Node 10: X Displacement = -0.037179779387947, Y Displacement = -0.613348343546968
 Node 11: X Displacement = -0.03739773819661379, Y Displacement = -0.617546374152317
 Node 12: X Displacement = -0.036634318384885435, Y Displacement = -0.602167787123886
 Node 13: X Displacement = -0.03490972476677465, Y Displacement = -0.5655594156872907
 Node 14: X Displacement = -0.03227235567483926, Y Displacement = -0.5061518916333355
 Node 15: X Displacement = -0.028798993862640412, Y Displacement = -0.4224658359676727
 Node 16: X Displacement = -0.024594505032209153, Y Displacement = -0.31311769341571466
 Node 17: X Displacement = -0.01979102270580173, Y Displacement = -0.17682518815795872
 Node 18: X Displacement = -0.014546671033091976, Y Displacement = -0.012412379425652426
 Node 19: X Displacement = -0.009843772574362046, Y Displacement = 0.18118570276721332
 Node 20: X Displacement = -0.0034866365155758693, Y Displacement = 0.4049188597906146
 Node 21: X Displacement = 0.0019010824034236624, Y Displacement = 0.659617980749508
 Node 22: X Displacement = 0.006879400867173785, Y Displacement = 0.9459917674420825
 Node 23: X Displacement = 0.011195355064763816, Y Displacement = 1.2646238465932016
 Node 24: X Displacement = 0.014586019139360326, Y Displacement = 1.6159057115413942
 Node 25: X Displacement = 0.016858525845849043, Y Displacement = 1.982360267064577
 Node 26: X Displacement = 0.015817346337994476, Y Displacement = 1.5827670787783283
 Node 27: X Displacement = 0.016561769131742814, Y Displacement = 1.1986093641851365
 Node 28: X Displacement = 0.018515361022481722, Y Displacement = 0.847625766066709
 Node 29: X Displacement = 0.0215895131410905898, Y Displacement = 0.5296842656425084
 Node 30: X Displacement = 0.02545778250083776, Y Displacement = 0.24445761755015744
 Node 31: X Displacement = 0.02985962465461811, Y Displacement = -0.008510601089632145
 Node 32: X Displacement = 0.0345442480553363, Y Displacement = -0.22980409667014448
 Node 33: X Displacement = 0.03927502792603657, Y Displacement = -0.4201315579629968
 Node 34: X Displacement = 0.04383109525805321, Y Displacement = -0.5803238628298513
 Node 35: X Displacement = 0.0488012108263183036, Y Displacement = -0.7113088035047032
 Node 36: X Displacement = 0.0516339992787406504, Y Displacement = -0.8142173431104059
 Node 37: X Displacement = 0.05456132417414063, Y Displacement = -0.8901594179074599
 Node 38: X Displacement = 0.056649080917386734, Y Displacement = -0.9404393815632028
 Node 39: X Displacement = 0.0578039321801709776, Y Displacement = -0.9664045494880513
 Node 40: X Displacement = 0.05795495922488085, Y Displacement = -0.9694625429304212
 Node 41: X Displacement = 0.05706008944037034, Y Displacement = -0.9516146543227563
 Node 42: X Displacement = 0.055105668321802935, Y Displacement = -0.9140100565182431
 Node 43: X Displacement = 0.052105894018439865, Y Displacement = -0.8585592005267121
 Node 44: X Displacement = 0.04810153502671609, Y Displacement = -0.7870629872968078
 Node 45: X Displacement = 0.043158313529581986, Y Displacement = -0.7013856605462575
 Node 46: X Displacement = 0.03736477720163467, Y Displacement = -0.6034474487364657
 Node 47: X Displacement = 0.030829833163543596, Y Displacement = -0.49521698529723207
 Node 48: X Displacement = 0.02367991714978625, Y Displacement = -0.3787035712987703
 Node 49: X Displacement = 0.016055834424870626, Y Displacement = -0.255949072182558
 Node 50: X Displacement = 0.008109436143016848, Y Displacement = -0.1290201975417752
 Node 51: X Displacement = 0.0, Y Displacement = 0.0
 Node 52: X Displacement = 0.0, Y Displacement = 0.0
 Node 53: X Displacement = 1.1778779369701396e-05, Y Displacement = -0.0956132298201296
 Node 54: X Displacement = 2.3755334300911662e-05, Y Displacement = -0.18913512779589122
 Node 55: X Displacement = 3.6113567558451427e-05, Y Displacement = -0.27848261864644013
 Node 56: X Displacement = 4.900994626475564e-05, Y Displacement = -0.36158910814575607
 Node 57: X Displacement = 6.256038322755048e-05, Y Displacement = -0.43641264223408976
 Node 58: X Displacement = 7.682795370523368e-05, Y Displacement = -0.5009439684845365
 Node 59: X Displacement = 9.181156255459337e-05, Y Displacement = -0.5532144679840715
 Node 60: X Displacement = 0.00010743586743282566, Y Displacement = -0.591303926215029
 Node 61: X Displacement = 0.00012354259944439641, Y Displacement = -0.6133481121819705
 Node 62: X Displacement = 0.00013988348886551048, Y Displacement = -0.6175461358125576
 Node 63: X Displacement = 0.00015611493688544051, Y Displacement = -0.6021675545656181
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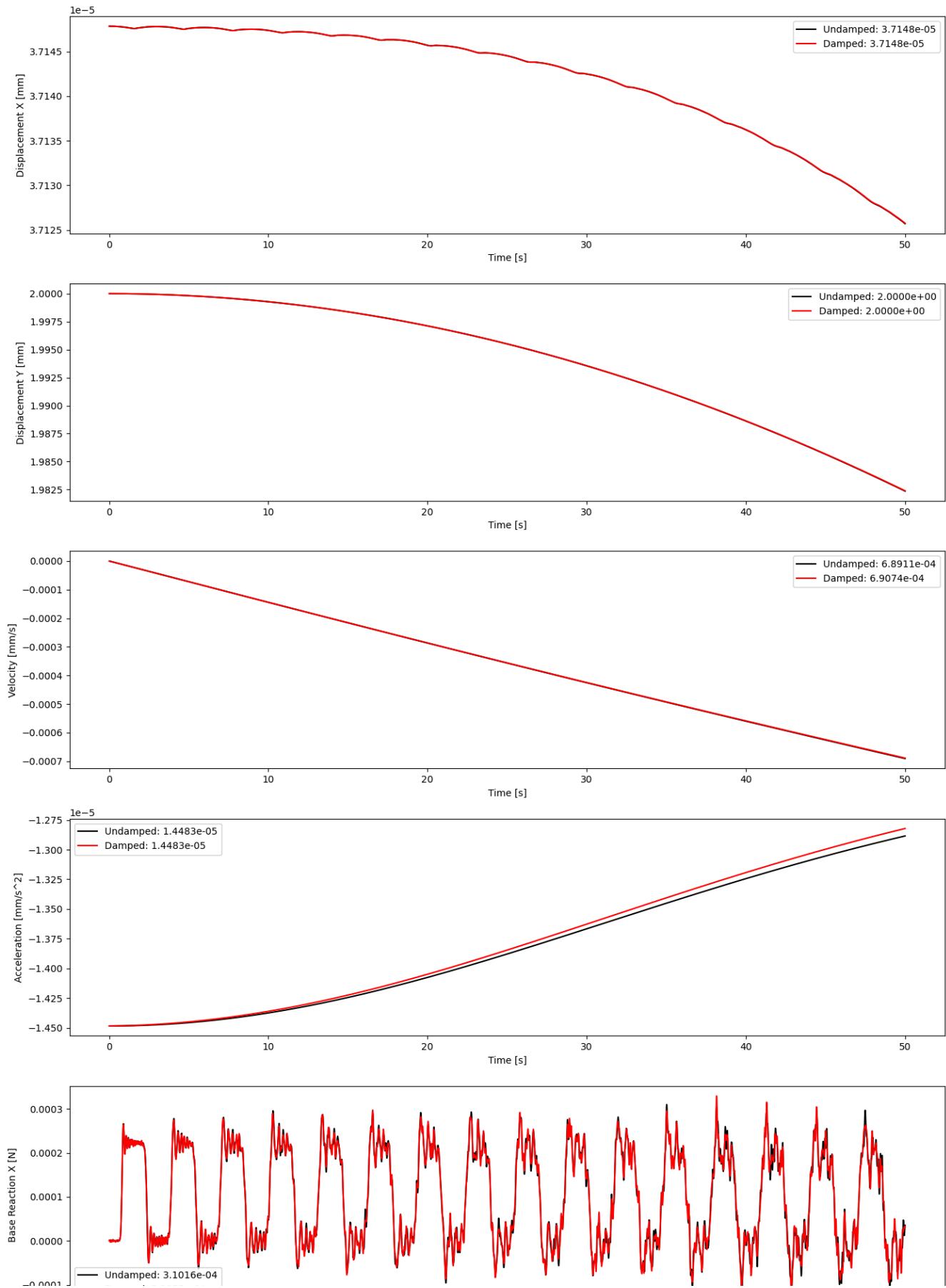
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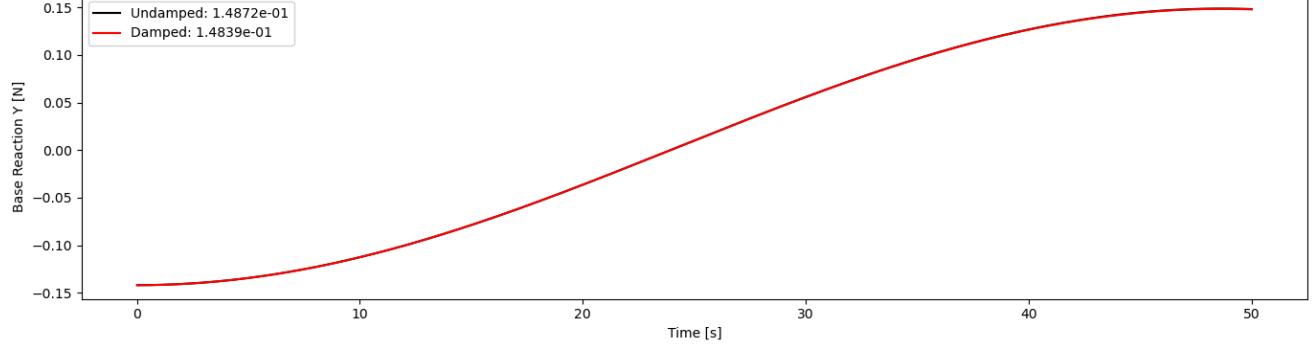
Total time (s): 7.9844

```
# Plotting the time history
TIME = np.arange(dt, duration+2*dt, dt)

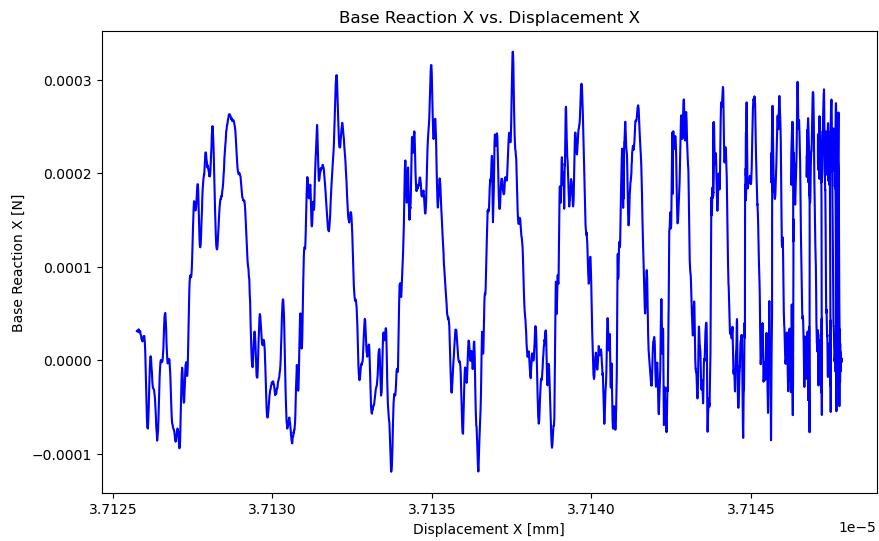
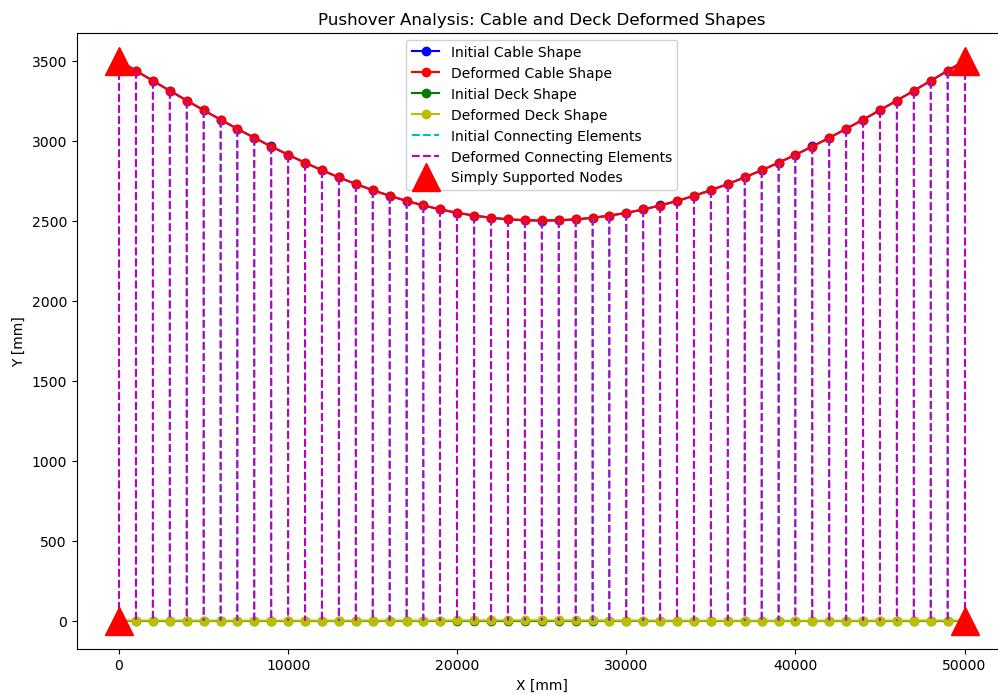
plot_time_history(TIME, DISP_X_undamped, DISP_X_damped, DISP_Y_undamped, DISP_Y_damped,
                  VELOCITY_undamped, VELOCITY_damped, ACCELERATION_undamped, ACCELERATION_damped,
                  BASEREACTION_X_undamped, BASEREACTION_X_damped, BASEREACTION_Y_undamped, BASEREACTION_Y_damped)
```

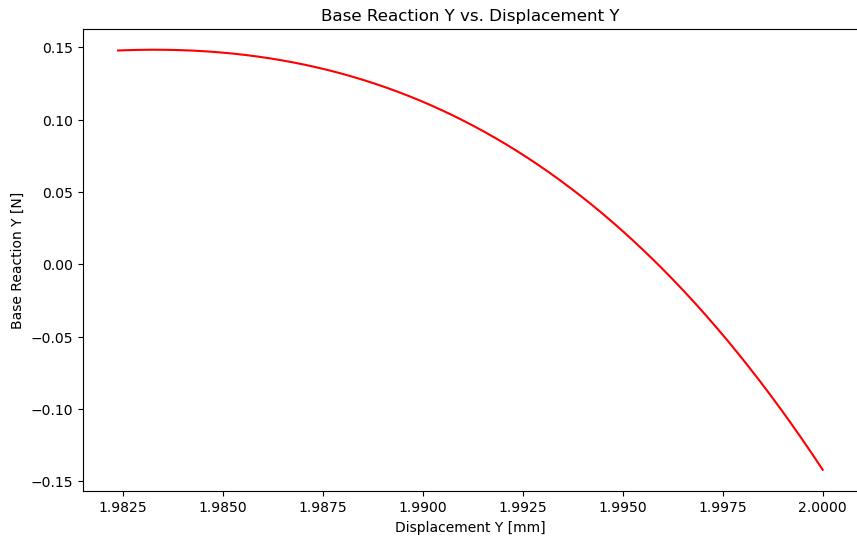
Time History of Free Vibration Analysis: Damped vs Undamped



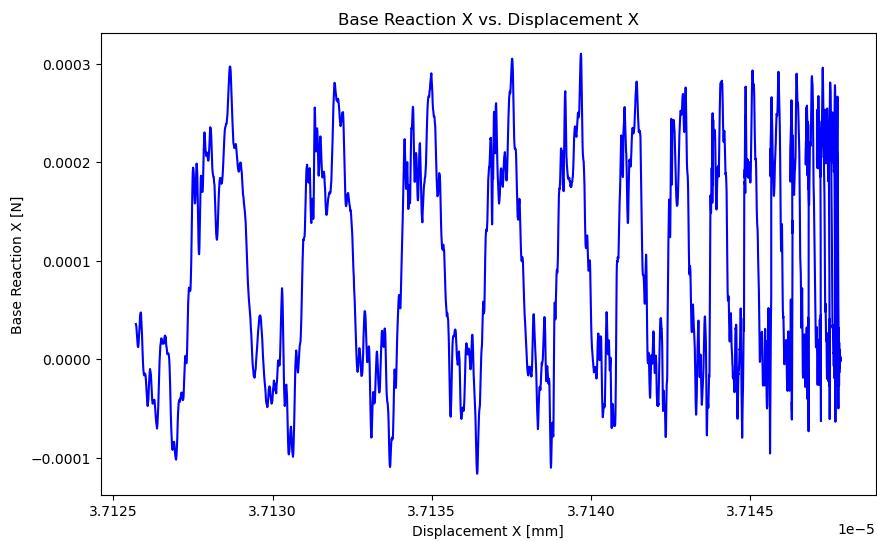
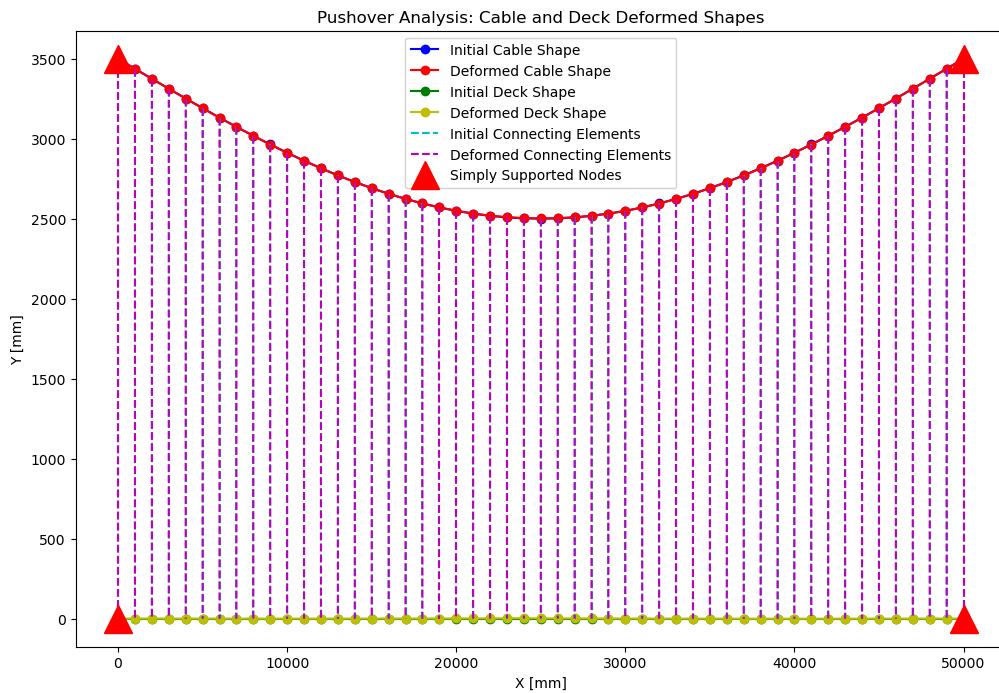


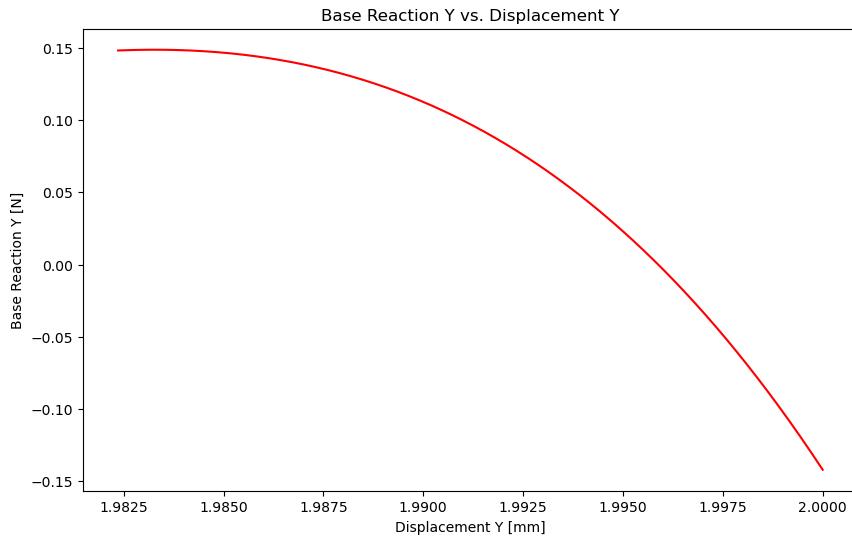
```
In [29]: # Plot damped results
plot_shapes(initial_coords, DISPLACEMENTS_damped)
plot_reactions(DISP_X_damped, BASEREACTION_X_damped, DISP_Y_damped, BASEREACTION_Y_damped)
```





```
In [30]: # Plot undamped results
plot_shapes(initial_coords, DISPLACEMENTS_undamped)
plot_reactions(DISPLACEMENT_X_undamped, BASE_REACTION_X_undamped, DISPLACEMENT_Y_undamped, BASE_REACTION_Y_undamped)
```





```
In [23]: def HISTOGRAM_BOXPLOT(X, HISTO_COLOR, LABEL):
    import numpy as np
    import matplotlib.pyplot as plt
    X = np.array(X)
    print("-----")
    from scipy.stats import skew, kurtosis
    MINIMUM = np.min(X)
    MAXIMUM = np.max(X)
    #MODE = max(set(X), key=list(X).count)
    MEDIAN = np.quantile(X, .50)#q2
    MEAN = np.mean(X)
    STD = np.std(X)
    q1 = np.quantile(X, .25)
    q3 = np.quantile(X, .75)
    SKEW = skew(X)
    KURT = kurtosis(X)
    #SKEW = (MEAN - MODE) / STD
    #KURT = (np.mean((X - MEAN)**4) / STD**4)
    # Estimate confidence intervals of the output variable
    lower_bound = np.quantile(X, .05)
    upper_bound = np.quantile(X, .95)
    print("Box-Chart Datas:")
    print(f' Minimum: {MINIMUM:.4e}')
    print(f' First quartile: {q1:.4e}')
    #print(f' Mode: {MODE:.4e}')
    print(f' Median: {MEDIAN:.4e}')
    print(f' Mean: {MEAN:.4e}')
    print(f' Std: {STD:.4e}')
    print(f' Third quartile: {q3:.4e}')
    print(f' Maximum: {MAXIMUM:.4e}')
    print(f' Skewness: {skew(X):.4e}')
    print(f' Kurtosis: {kurtosis(X):.4e}')
    print(f' 99% Confidence Interval: ({lower_bound:.4e}, {upper_bound:.4e})')
    print("-----")

    plt.figure(figsize=(10,6))
    # Plot histogram of data
    count, bins, ignored = plt.hist(X, bins=100, color=HISTO_COLOR, density=True, align='mid')#, edgecolor="black"

    # Plot Lognormal PDF
    x = np.linspace(min(bins), max(bins), 10000)
    pdf = (np.exp(-(x - MEAN)**2 / (2 * STD**2)) / (STD * np.sqrt(2 * np.pi)))
    plt.plot(x, pdf, linewidth=2, color='r', label="Normal PDF")

    # Plot vertical lines for risk measures
    plt.axvline(q1, color="black", linestyle="--", label=f"Quantile 0.25: {q1:.4e}")
    plt.axvline(MEDIAN, color="green", linestyle="--", label=f"Median: {MEDIAN:.4e}")
    plt.axvline(q3, color="black", linestyle="--", label=f"Quantile 0.75: {q3:.4e}")
    #plt.axvline(MODE, color="purple", linestyle="--", label=f"Mode: {MODE:.4e}")
    plt.axvline(MEAN, color="red", linestyle="--", label=f"Mean: {MEAN:.4e}")
    plt.axvline(MEAN-STD, color="blue", linestyle="--", label=f"Mean-Std: {MEAN-STD:.4e}")
    plt.axvline(MEAN+STD, color="blue", linestyle="--", label=f"Mean+Std: {MEAN+STD:.4e}")
    plt.xlabel(LABEL)
    plt.ylabel("Frequency")
    prob = np.sum(X > 0) / len(X)
    plt.title(f"Histogram - Probability of Positive {LABEL} is {100*prob:.2f} %")
    plt.legend()
    #plt.grid()
    plt.show()

#Plot boxplot with outliers
plt.figure(figsize=(10,6))
plt.boxplot(X, vert=0)
# Write the quartile data on the chart
plt.text(q1, 1.05, f" Q1: {q1:.4e}")
plt.text(MEDIAN, 1.1, f" Q2: {MEDIAN:.4e}")
plt.text(q3, 1.05, f" Q3: {q3:.4e}")
plt.text(MODE, 1.15, f" Mode: {MODE:.4e}")

plt.text(MEAN, 0.9, f" Mean: {MEAN:.4e}")
plt.text(MEAN-STD, 0.9, f" Mean-Std: {MEAN-STD:.4e}")
plt.text(MEAN+STD, 0.9, f" Mean+Std: {MEAN+STD:.4e}")
plt.scatter(MEAN, 1, color="red", marker="+", s=200, label=f"Mean: {MEAN:.4e}")
plt.scatter(MEAN-STD, 1, color="green", marker="x", s=200, label=f"Mean-Std: {MEAN-STD:.4e}")
plt.scatter(MEAN+STD, 1, color="blue", marker="*", s=200, label=f"Mean+Std: {MEAN+STD:.4e}")
plt.xlabel(LABEL)
plt.ylabel("Data")
plt.title(f"Boxplot of {LABEL}")
plt.legend()
```

```

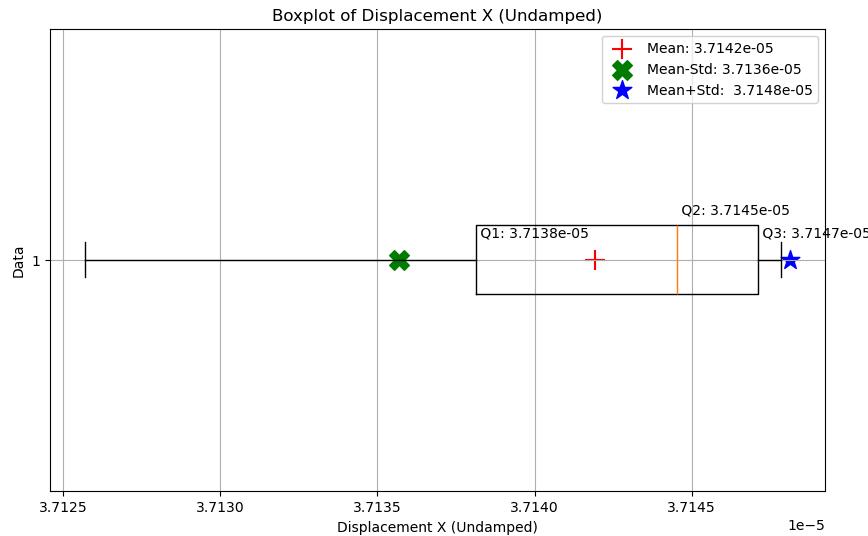
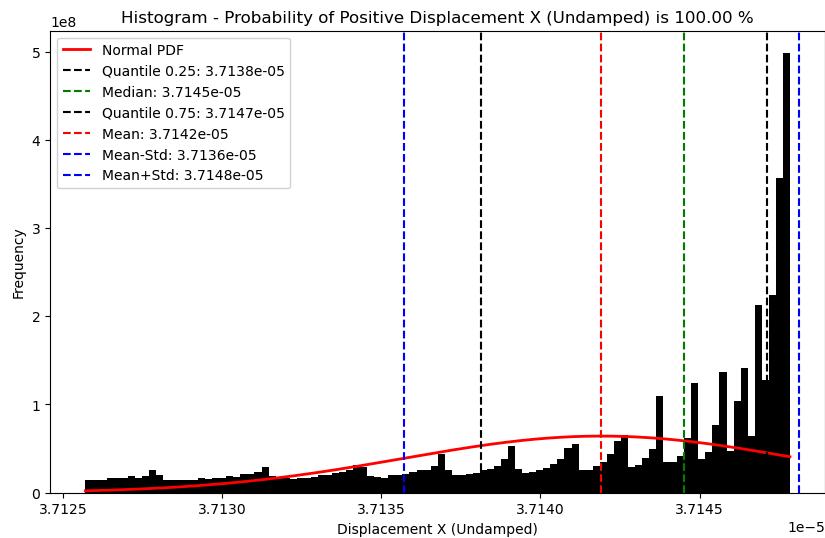
plt.grid()
plt.show()

# -----
def HISTOGRAM_BOXPLOT_PLOTLY( DATA, XLABEL='X', TITLE='A', COLOR='cyan'):
    # Plotting histogram and boxplot
    import plotly.express as px
    fig = px.histogram(x=DATA, marginal="box", color_discrete_sequence=[COLOR])
    fig.update_layout(title=TITLE, xaxis_title=XLABEL, yaxis_title="Frequency")
    fig.show()

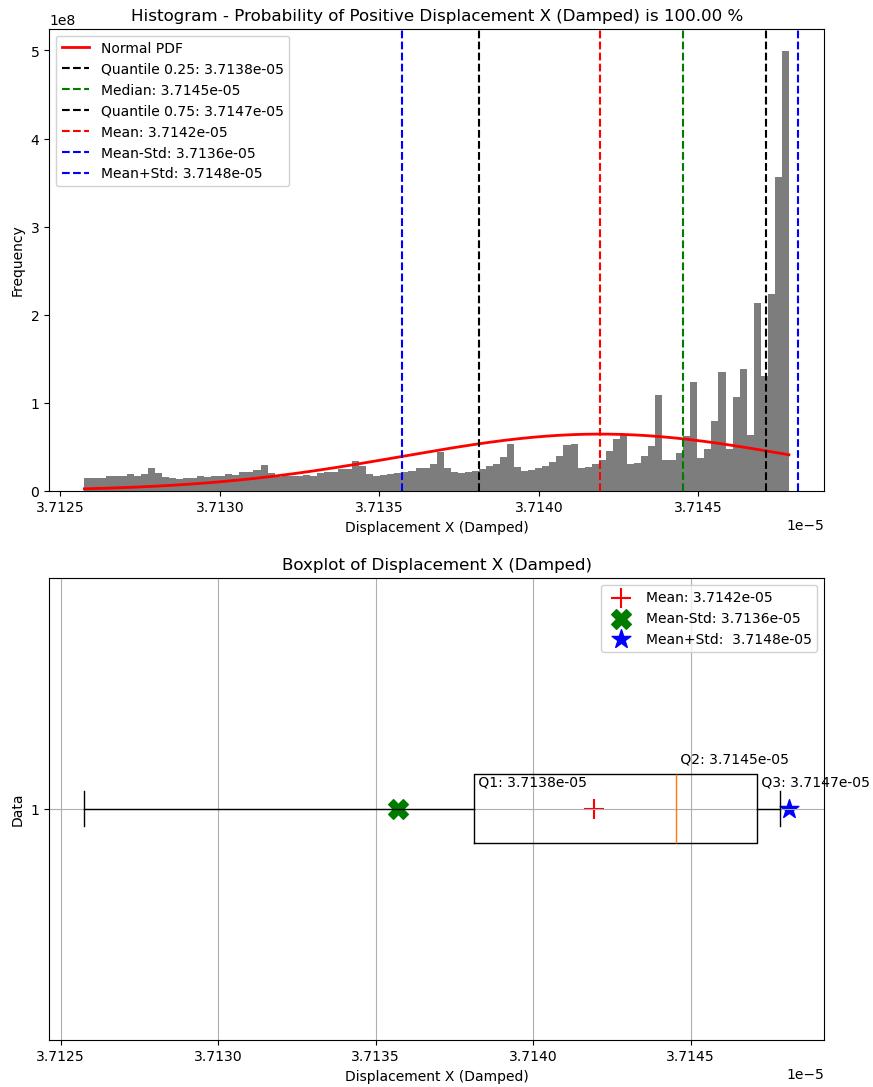
In [32]: HISTOGRAM_BOXPLOT(DISP_X_undamped, HISTO_COLOR='black', LABEL='Displacement X (Undamped)')
HISTOGRAM_BOXPLOT(DISP_X_damped, HISTO_COLOR='grey', LABEL='Displacement X (Damped)')
#HISTOGRAM_BOXPLOT_PLOTLY(DISP_X_undamped, XLABEL='Displacement X (Undamped)', TITLE='Displacement X (Undamped)', COLOR='black')
#HISTOGRAM_BOXPLOT_PLOTLY(DISP_X_damped, XLABEL='Displacement X (Damped)', TITLE='Displacement X (Damped)', COLOR='grey')

```

Box-Chart Datas:
Minimum: 3.7126e-05
First quartile: 3.7138e-05
Median: 3.7145e-05
Mean: 3.7142e-05
Std: 6.2137e-09
Third quartile: 3.7147e-05
Maximum: 3.7148e-05
Skewness: -1.0058e+00
kurtosis: -1.8075e-01
90% Confidence Interval: (3.7129e-05, 3.7148e-05)

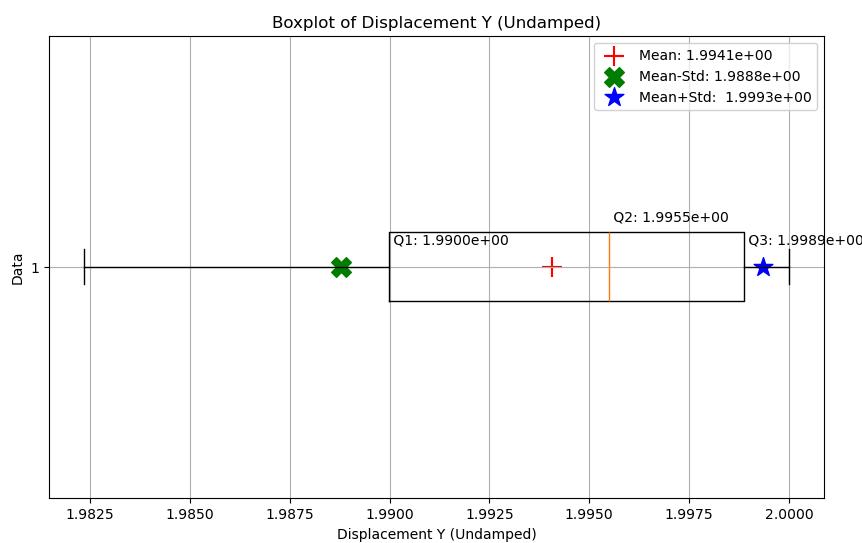
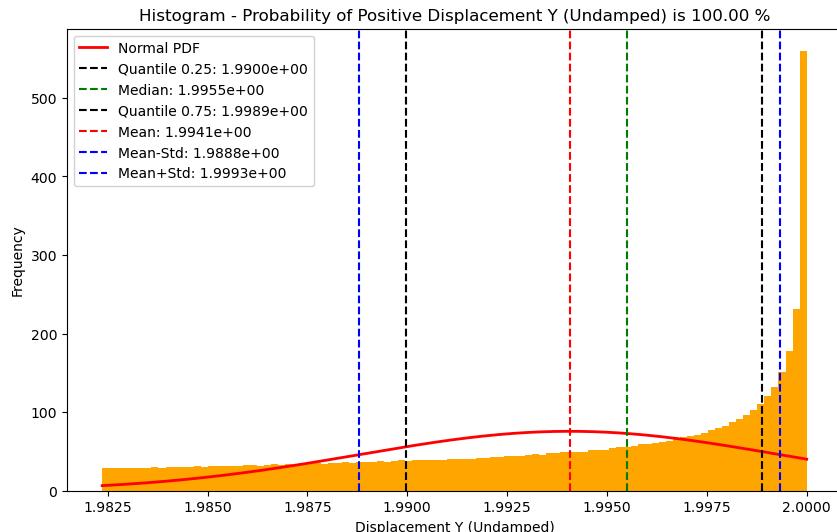


Box-Chart Datas:
Minimum: 3.7126e-05
First quartile: 3.7138e-05
Median: 3.7145e-05
Mean: 3.7142e-05
Std: 6.2016e-09
Third quartile: 3.7147e-05
Maximum: 3.7148e-05
Skewness: -1.0058e+00
kurtosis: -1.8262e-01
90% Confidence Interval: (3.7129e-05, 3.7148e-05)

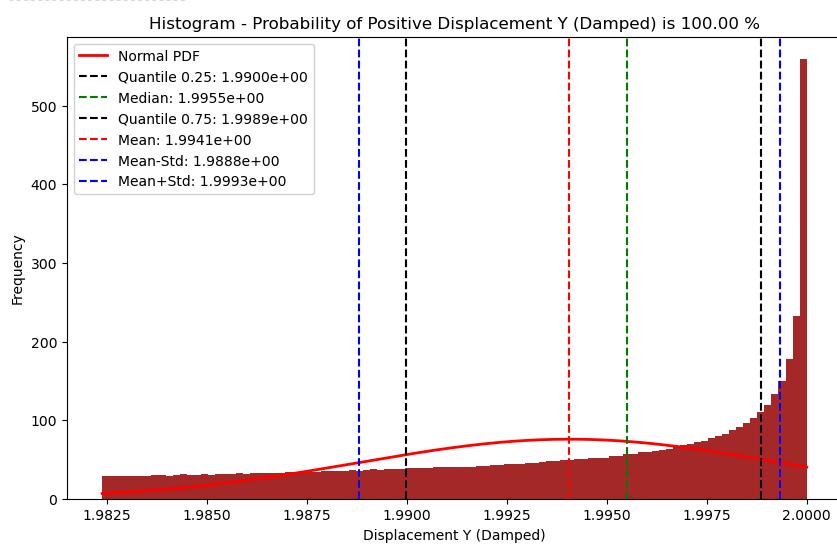


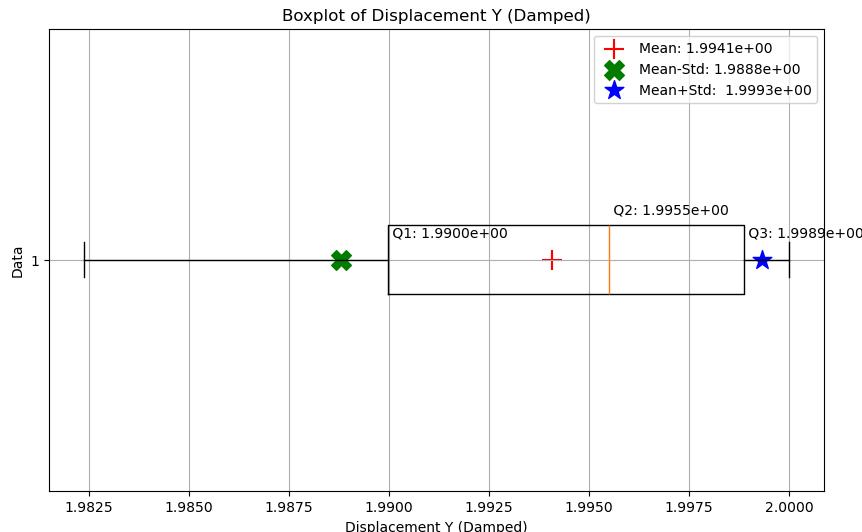
```
In [33]: HISTOGRAM_BOXPLOT(DISP_Y_undamped, HISTO_COLOR='orange', LABEL='Displacement Y (Undamped)')
HISTOGRAM_BOXPLOT(DISP_Y_damped, HISTO_COLOR='brown', LABEL='Displacement Y (Damped)')
#HISTOGRAM_BOXPLOT_PLOTLY(DISP_Y_undamped, XLABEL='Displacement Y (Undamped)', TITLE='Displacement Y (Undamped)', COLOR='orange')
#HISTOGRAM_BOXPLOT_PLOTLY(DISP_Y_damped, XLABEL='Displacement Y (Damped)', TITLE='Displacement Y (Damped)', COLOR='brown')
```

Box-Chart Data:
Minimum: 1.9824e+00
First quartile: 1.9900e+00
Median: 1.9955e+00
Mean: 1.9941e+00
Std: 5.2758e-03
Third quartile: 1.9989e+00
Maximum: 2.0000e+00
Skewness: -6.2305e-01
kurtosis: -8.8164e-01
90% Confidence Interval: (1.9840e+00, 2.0000e+00)



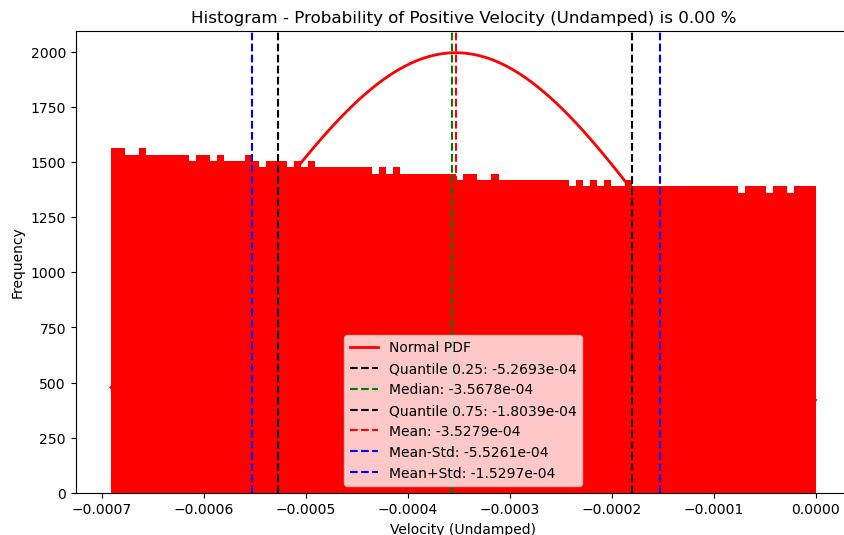
```
-----  
Box-Chart Datas:  
Minimum: 1.9824e+00  
First quartile: 1.9900e+00  
Median: 1.9955e+00  
Mean: 1.9941e+00  
Std: 5.2681e-03  
Third quartile: 1.9989e+00  
Maximum: 2.0000e+00  
Skewness: -6.2241e-01  
kurtosis: -8.8257e-01  
90% Confidence Interval: (1.9841e+00, 2.0000e+00)  
-----
```

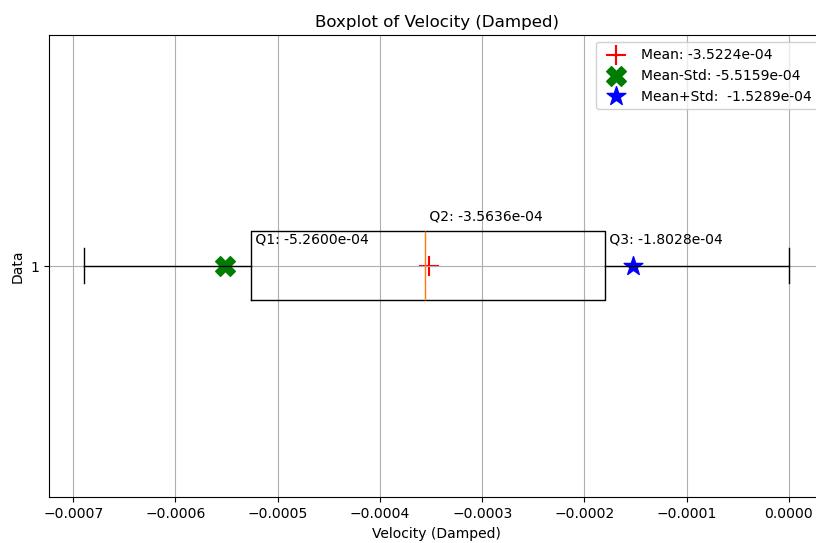
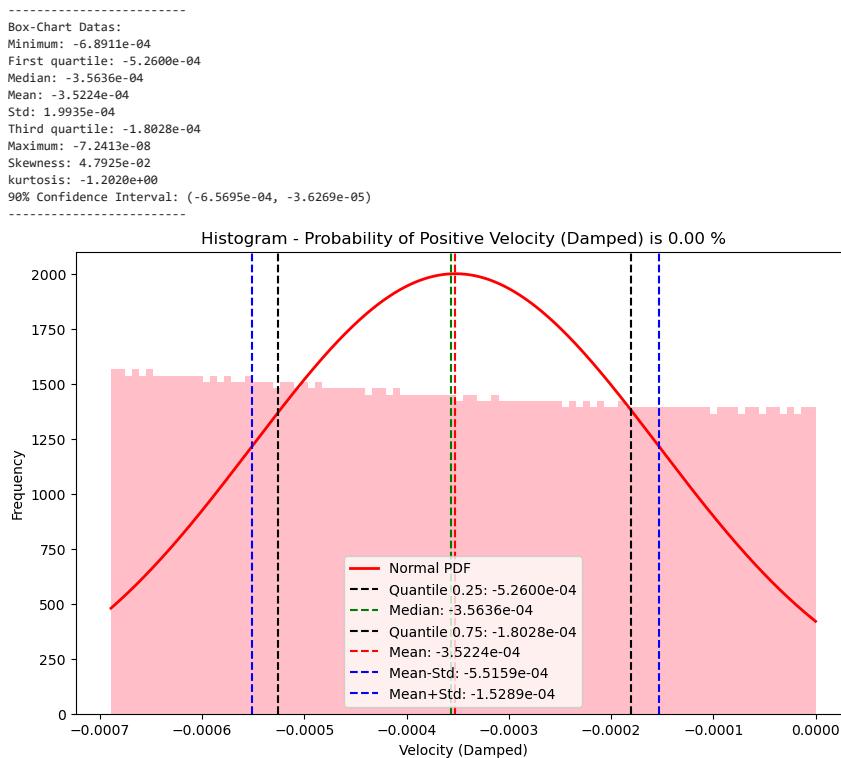
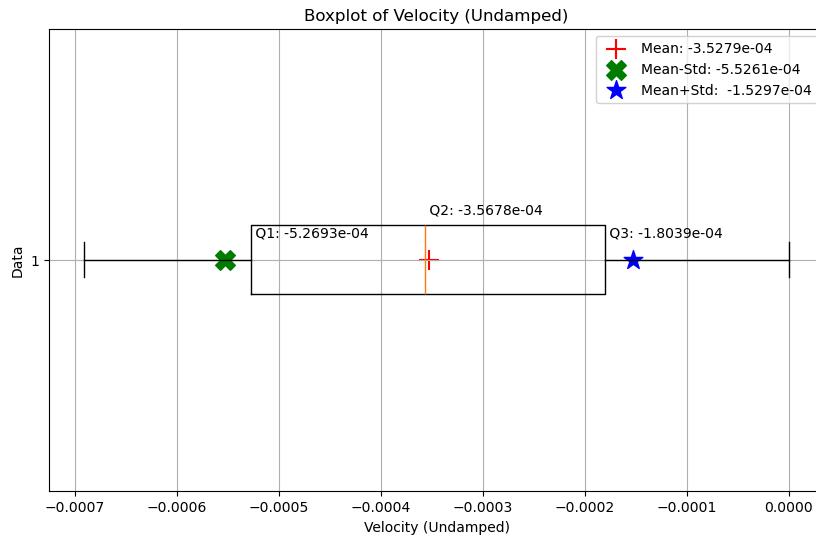




```
In [34]: HISTOGRAM_BOXPLOT(VELOCITY_undamped, HISTO_COLOR='red', LABEL='Velocity (Undamped)')
HISTOGRAM_BOXPLOT(VELOCITY_damped, HISTO_COLOR='pink', LABEL='Velocity (Damped)')
#HISTOGRAM_BOXPLOT_PLOTLY(VELOCITY_undamped, XLABEL='Velocity (Undamped)', TITLE='Velocity (Undamped)', COLOR='red')
#HISTOGRAM_BOXPLOT_PLOTLY(VELOCITY_damped, XLABEL='Velocity (Damped)', TITLE='Velocity (Damped)', COLOR='pink')
```

 Box-Chart Datas:
 Minimum: -6.9074e-04
 First quartile: -5.2693e-04
 Median: -3.5678e-04
 Mean: -3.5279e-04
 Std: 1.9982e-04
 Third quartile: -1.8039e-04
 Maximum: -7.2413e-08
 Skewness: 4.6243e-02
 kurtosis: -1.2022e+00
 90% Confidence Interval: (-6.5842e-04, -3.6273e-05)

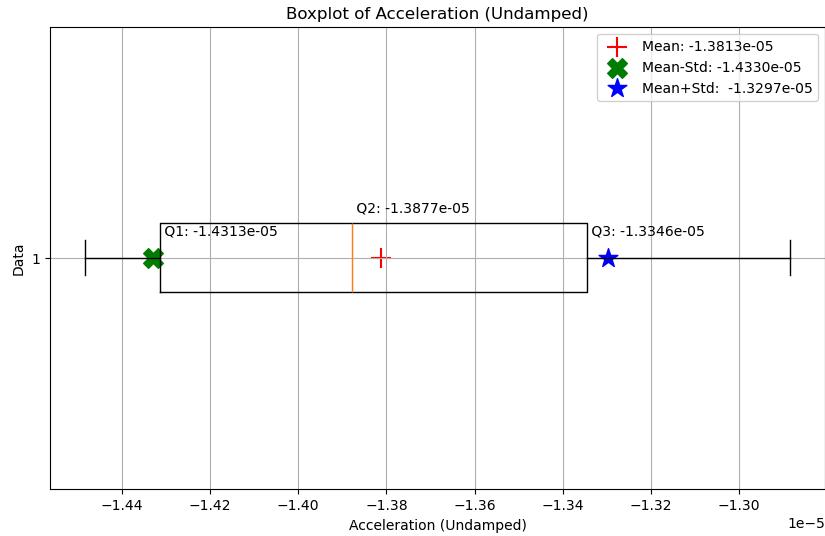
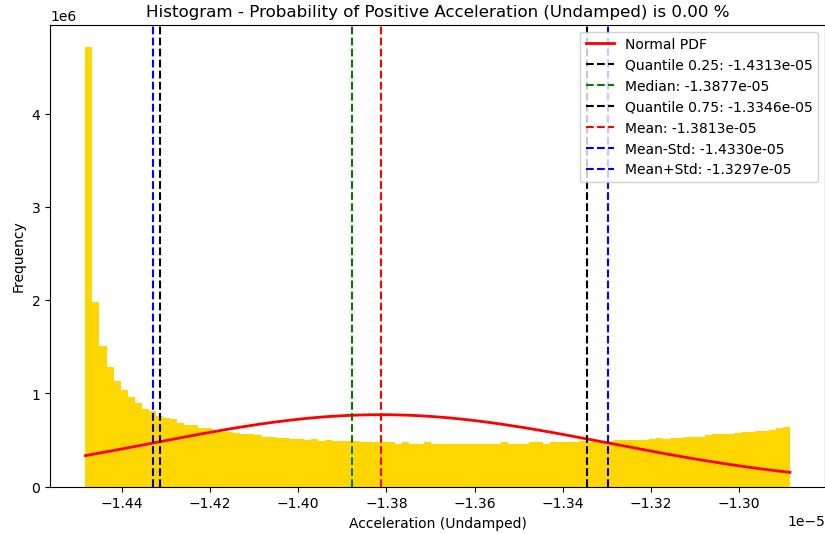




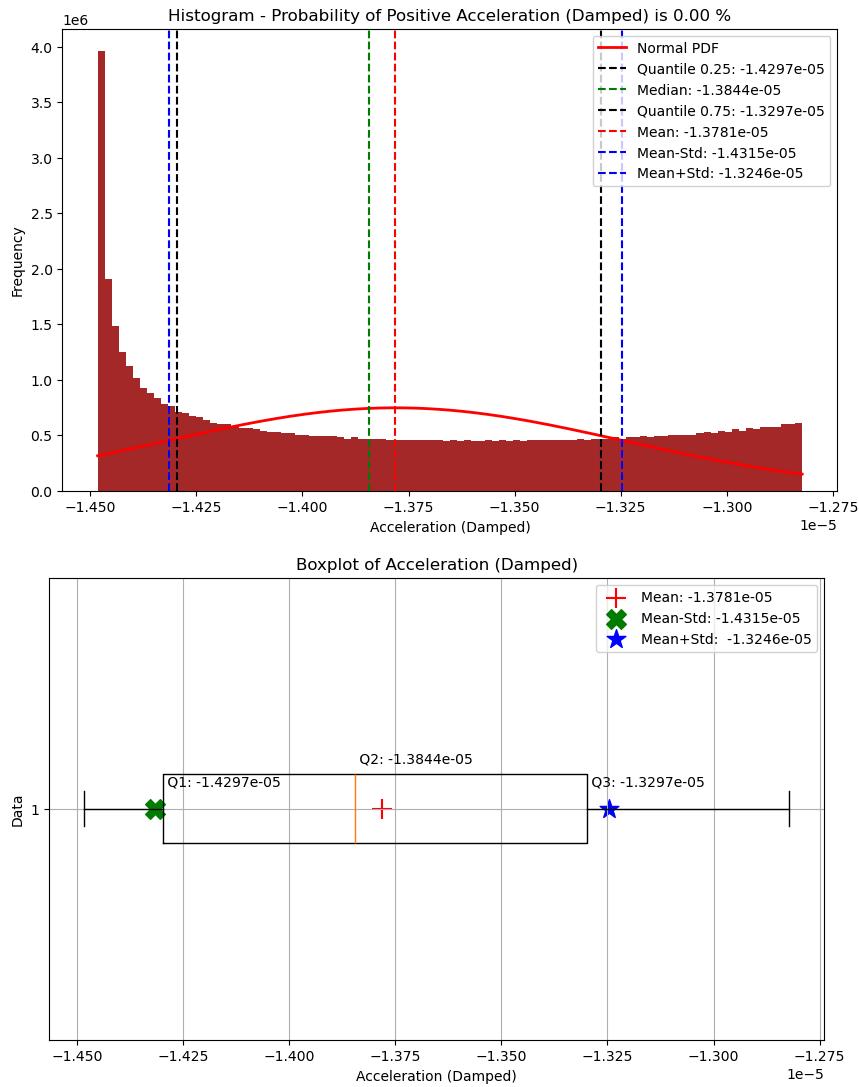
```
In [35]: HISTOGRAM_BOXPLOT(ACCELERATION_undamped, HISTO_COLOR='gold', LABEL='Acceleration (Undamped)')
HISTOGRAM_BOXPLOT(ACCELERATION_damped, HISTO_COLOR='brown', LABEL='Acceleration (Damped)')
```

```
#HISTOGRAM_BOXPLOT_PLOTLY(ACCELERATIONUndamped, XLABEL='Acceleration (Undamped)', TITLE='Acceleration (Undamped)', COLOR='gold')
#HISTOGRAM_BOXPLOT_PLOTLY(ACCELERATIONDamped, XLABEL='Acceleration (Damped)', TITLE='Acceleration (Damped)', COLOR='brown')
```

Box-Chart Datas:
Minimum: -1.4483e-05
First quartile: -1.4313e-05
Median: -1.3877e-05
Mean: -1.3813e-05
Std: 5.1642e-07
Third quartile: -1.3346e-05
Maximum: -1.2885e-05
Skewness: 2.5677e-01
kurtosis: -1.3241e+00
90% Confidence Interval: (-1.4476e-05, -1.2967e-05)

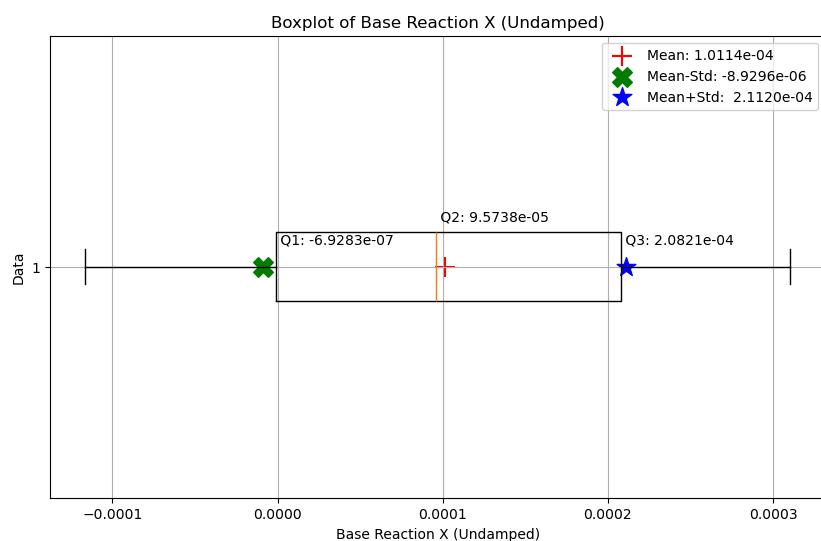
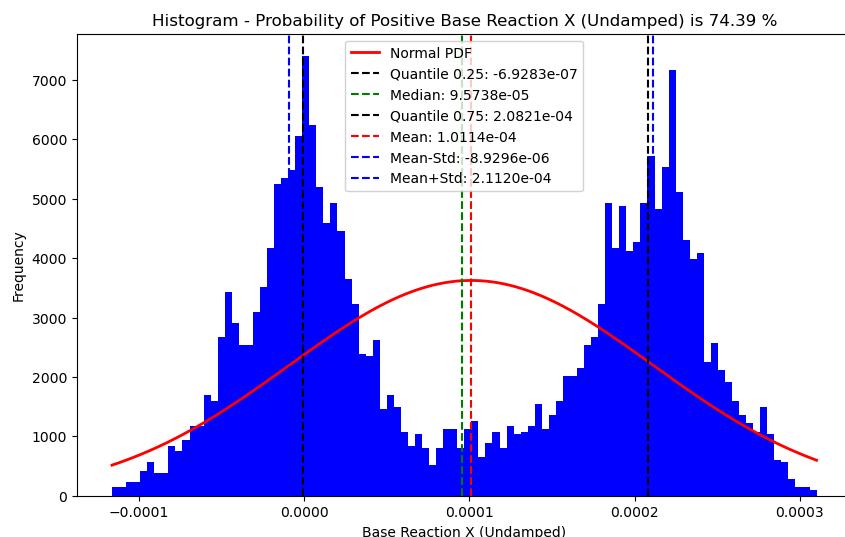


Box-Chart Datas:
Minimum: -1.4483e-05
First quartile: -1.4297e-05
Median: -1.3844e-05
Mean: -1.3781e-05
Std: 5.3434e-07
Third quartile: -1.3297e-05
Maximum: -1.2822e-05
Skewness: 2.4734e-01
kurtosis: -1.3235e+00
90% Confidence Interval: (-1.4472e-05, -1.2907e-05)

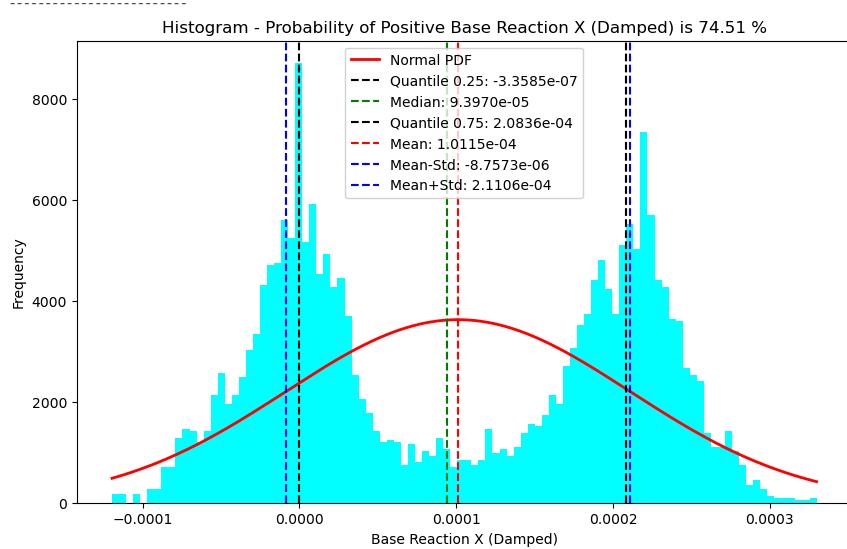


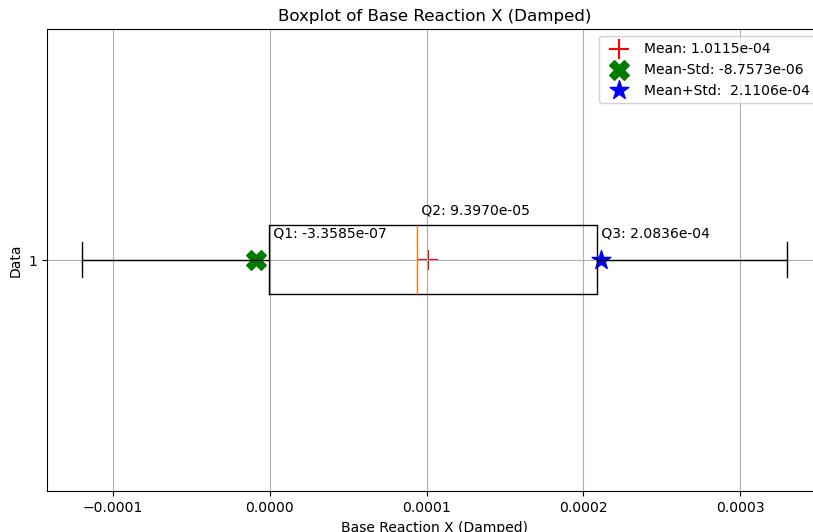
```
In [36]: HISTOGRAM_BOXPLOT(BASE_REACTION_X_undamped, HISTO_COLOR='blue', LABEL='Base Reaction X (Undamped)')
HISTOGRAM_BOXPLOT(BASE_REACTION_X_damped, HISTO_COLOR='cyan', LABEL='Base Reaction X (Damped)')
#HISTOGRAM_BOXPLOT_PLOTLY(BASE_REACTION_X_undamped, XLABEL='Base Reaction X (Undamped)', TITLE='Base Reaction X (Undamped)', COLOR='blue')
#HISTOGRAM_BOXPLOT_PLOTLY(BASE_REACTION_X_damped, XLABEL='Base Reaction X (Damped)', TITLE='Base Reaction X (Damped)', COLOR='cyan')
```

Box-Chart Data:
Minimum: -1.1630e-04
First quartile: -6.9283e-07
Median: 9.5738e-05
Mean: 1.0114e-04
Std: 1.1007e-04
Third quartile: 2.0821e-04
Maximum: 3.1016e-04
Skewness: 1.9720e-02
kurtosis: -1.5392e+00
90% Confidence Interval: (-5.0007e-05, 2.5391e-04)



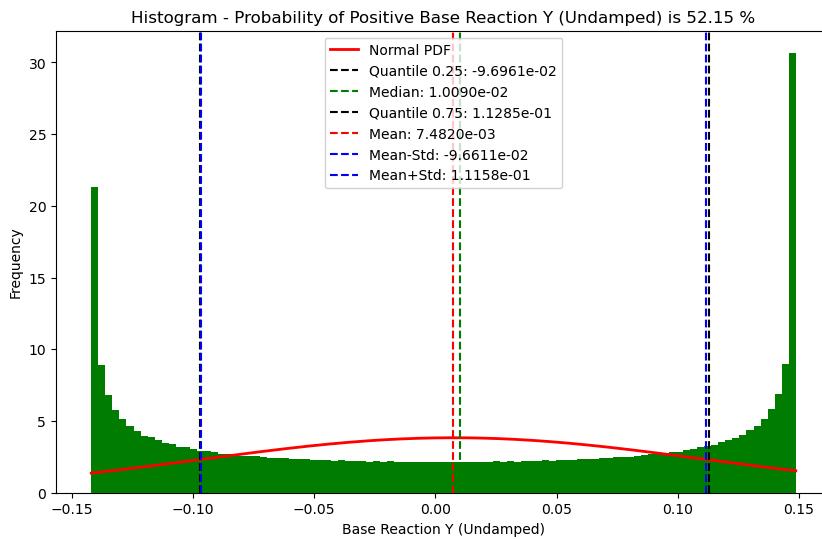
```
-----  
Box-Chart Datas:  
Minimum: -1.1947e-04  
First quartile: -3.3585e-07  
Median: 9.3970e-05  
Mean: 1.0115e-04  
Std: 1.0991e-04  
Third quartile: 2.0836e-04  
Maximum: 3.2975e-04  
Skewness: 1.8111e-02  
kurtosis: -1.5364e+00  
90% Confidence Interval: (-5.2773e-05, 2.5308e-04)
```

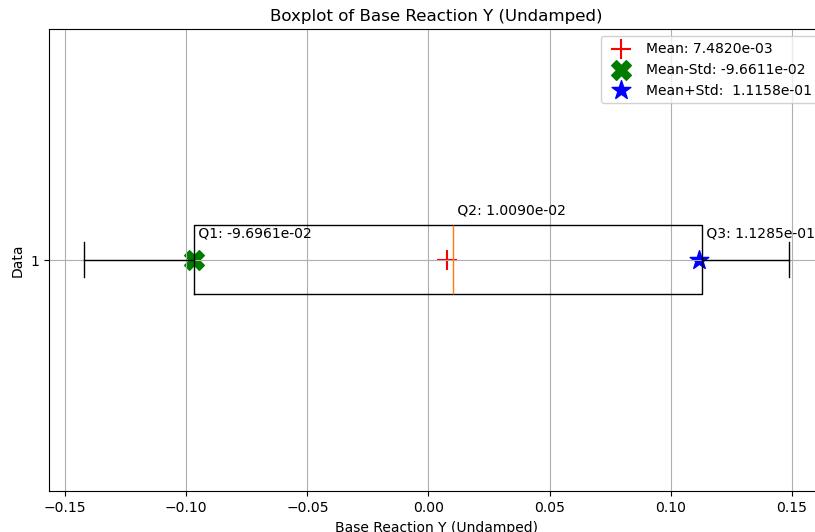




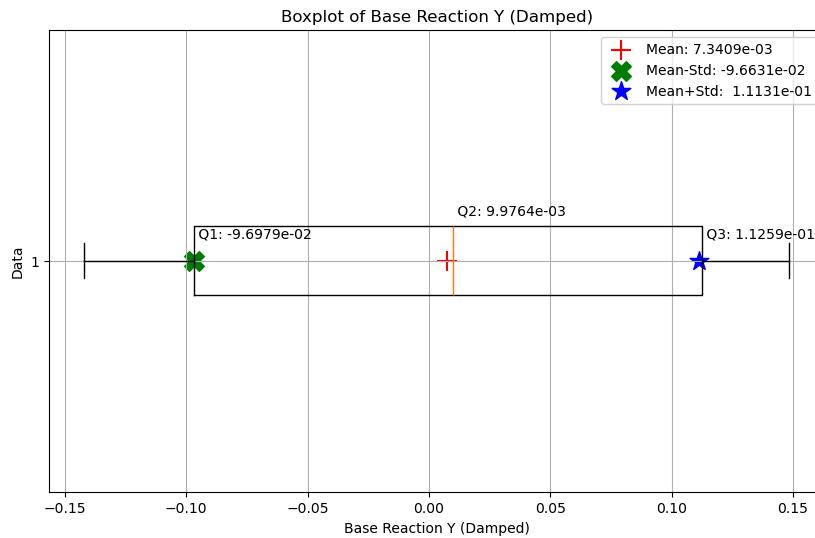
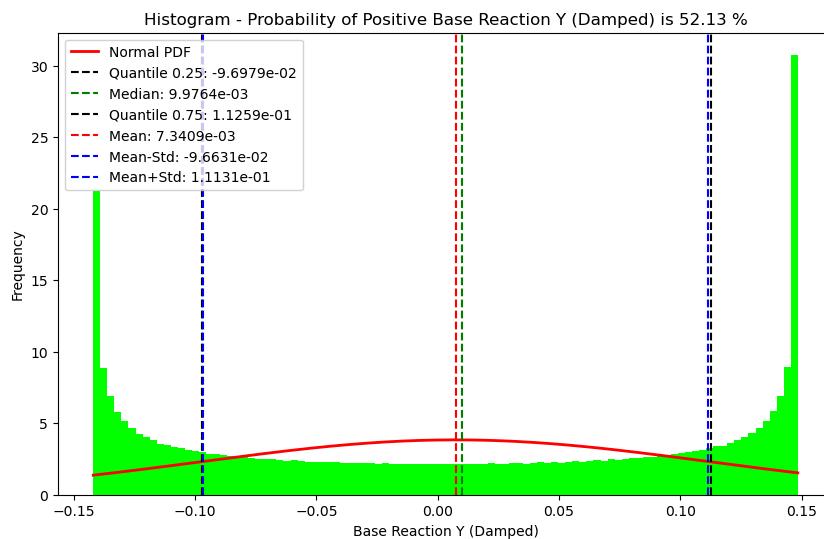
```
In [37]: HISTOGRAM_BOXPLOT(BASE_REACTION_Y_undamped, HISTO_COLOR='green', LABEL='Base Reaction Y (Undamped)')
HISTOGRAM_BOXPLOT(BASE_REACTION_Y_damped, HISTO_COLOR='lime', LABEL='Base Reaction Y (Damped)')
#HISTOGRAM_BOXPLOT_PLOTLY(BASE_REACTION_Y_undamped, XLABEL='Base Reaction Y (Undamped)', TITLE='Base Reaction Y (Undamped)', COLOR='green')
#HISTOGRAM_BOXPLOT_PLOTLY(BASE_REACTION_Y_damped, XLABEL='Base Reaction Y (Damped)', TITLE='Base Reaction Y (Damped)', COLOR='lime')
```

 Box-Chart Data:
 Minimum: $-1.4199e-01$
 First quartile: $-9.6961e-02$
 Median: $1.0090e-02$
 Mean: $7.4820e-03$
 Std: $1.0409e-01$
 Third quartile: $1.1285e-01$
 Maximum: $1.4872e-01$
 Skewness: $-4.2545e-02$
 kurtosis: $-1.5124e+00$
 90% Confidence Interval: $(-1.4008e-01, 1.4825e-01)$





```
-----
Box-Chart Datas:
Minimum: -1.4199e-01
First quartile: -9.6979e-02
Median: 9.9764e-03
Mean: 7.3409e-03
Std: 1.0397e-01
Third quartile: 1.1259e-01
Maximum: 1.4839e-01
Skewness: -4.2987e-02
kurtosis: -1.5123e+00
90% Confidence Interval: (-1.4008e-01, 1.4792e-01)
-----
```



```
In [15]: # #####
# #####
# IN THE NAME OF ALLAH
#
```

```

#           # DYNAMIC ANALYSIS OF CABLE SUSPENSION BRIDGE 01 #
#           #-#
#           # THIS PROGRAM WRITTEN BY SALAR DELAVAR GHASHGHAEI (QASHQAI) #
#           # EMAIL: salar.d.ghashghaei@gmail.com #
#           ######
# In [47]: # -----
#   DYNAMIC ANALYSIS
# -----
def DYNAMIC_ANALYSIS(damping, damping_ratio, LINEAR, periodTF, duration, dt, TOTAL_MASS, L, H1, arc_depth, E_cable, Cable_Dia_01, Cable_Dia_02, Cable_Dia_03, num_nodes, MAX_ITERATIONS,
                     import openseespy.opensees as ops
                     import numpy as np
                     import matplotlib.pyplot as plt

                     #GMfact = 9810          # standard acceleration of gravity or standard acceleration
                     GMfact = 1;
                     iv0 = 0.0               # [mm/s] Initial velocity applied to the node
                     st_iv0 = 0.0             # [s] Initial velocity applied starting time
                     KE = 1000;              # [N/mm] Lateral column Effective Stiffness
                     A_cable_01 = (np.pi * Cable_Dia_01 **2) / 4 # [mm^2] Horizontal Longitudinal Cable Area Bottom
                     A_cable_02 = (np.pi * Cable_Dia_02 **2) / 4 # [mm^2] Horizontal Longitudinal Cable Area Top
                     A_cable_03 = (np.pi * Cable_Dia_03 **2) / 4 # [mm^2] Vertical Longitudinal Cable Area Top

                     # Define model builder
                     ops.wipe()
                     ops.model('basic', '-ndm', 2, '-ndf', 2)
                     dx = L / (num_nodes - 1)
                     MASS = TOTAL_MASS / num_nodes
                     # Create nodes
                     for i in range(num_nodes):
                         X = i * dx
                         Y1 = H1 - arc_depth * np.sin(np.pi * X / L)
                         ops.node(i + 1, X, Y1)
                         ops.node(num_nodes + i + 1, i * dx, 0.0)
                         # Define mass
                         ops.mass(num_nodes + i + 1, MASS, MASS, 0)

                     # Define boundary conditions (fixed at both ends)
                     ops.fix(1, 1, 1)          # TOP CABLE
                     ops.fix(num_nodes, 1, 1)    # TOP CABLE
                     ops.fix(num_nodes + 1, 1, 1) # BOTTOM CABLE
                     ops.fix(2 * num_nodes, 1, 1) # BOTTOM CABLE

                     # Define material properties
                     if LINEAR == True:
                         ops.uniaxialMaterial('Elastic', 1, E_cable)
                         #                                         TENSION      COMPRESSION
                         #ops.uniaxialMaterial('Elastic', 1, E_cable, 0, 0.5 * E_cable)
                     if LINEAR == False:
                         Fy_cable = 355 # [N/mm^2] Yield strength of the cable
                         b0 = 0.01
                         ops.uniaxialMaterial('Steel01', 1, Fy_cable, E_cable, b0)

                     # Define truss elements
                     for i in range(num_nodes - 1):
                         ops.element('corotTruss', i + 1, i + 1, i + 2, A_cable_02, 1) # Top Cable element
                         ops.element('corotTruss', num_nodes + i + 1, num_nodes + i + 2, A_cable_01, 1) # Bottom Cable Element

                     # Connect each node of the cable with the corresponding node of the deck using truss elements
                     for i in range(num_nodes):
                         ops.element('corotTruss', 2 * num_nodes + i + 1, i + 1, num_nodes + i + 1, A_cable_03, 1) # Vertical Cable Element

                     #mid_node = num_nodes // 2 + 1
                     mid_node = int(num_nodes + 0.5 * num_nodes)

                     # Dynamic analysis setup
                     ops.constraints('Transformation')
                     ops.numberer('RCM')
                     ops.system('UmfPack')
                     ops.test('EnergyIncr', TOLERANCE, MAX_ITERATIONS)
                     #ops.integrator('CentralDifference')
                     ops.integrator('HHT', 0.9)
                     #ops.integrator('Newmark', 0.5, 0.25)
                     ops.algorithm('ModifiedNewton')

                     # Define analysis type
                     ops.analysis('Transient')

                     # Define time series for input motion (Acceleration time history)
                     ops.timeSeries('Path', 1, '-dt', 0.01, '-filePath', 'OPENSEES_SPRING_SEISMIC_01.txt', '-factor', GMfact, '-startTime', st_iv0) # SEISMIC-X
                     ops.timeSeries('Path', 2, '-dt', 0.01, '-filePath', 'OPENSEES_SPRING_SEISMIC_02.txt', '-factor', GMfact) # SEISMIC-Y

                     # Define load patterns
                     # pattern UniformExcitation $patternTag $dof -accel $tsTag <-vel0 $vel0> <-fact $cFact>
                     ops.pattern('UniformExcitation', 1, 1, '-accel', 1, '-vel0', iv0, '-fact', 1.0) # SEISMIC-X
                     ops.pattern('UniformExcitation', 2, 2, '-accel', 2)                                # SEISMIC-Y

                     # Perform eigenvalue analysis to determine modal periods
                     if periodTF == True:
                         eigenvalues01 = ops.eigen('-genBandArpack', 1) # eigenvalue mode 1
                         #eigenvalues02 = ops.eigen('-fullGenLapack', 1) # eigenvalue mode 1
                         #eigenvalues03 = ops.eigen('-symmBandLapack', 1) # eigenvalue mode 1
                         #Omega = np.power(eigenvalues01, 0.5)
                         #Omega = np.power(max(min(eigenvalues01), min(eigenvalues02), min(eigenvalues03)), 0.5)
                         Omega = np.power(min(eigenvalues01), 0.5)
                         Omega = np.power(min(eigenvalues01), 0.5)
                         modal_period = 2 * np.pi / np.sqrt(Omega) # [Second]
                         frequency = 1 / modal_period                # [Hertz]

                     if periodTF == False:
                         modal_period = 0.0                      # [Second]
                         frequency = 0.0                        # [Hertz]

                     if damping == True:
                         # Calculate Rayleigh damping factors
                         omega1 = np.sqrt(KE / TOTAL_MASS)
                         omega2 = 2 * omega1 # Just an assumption for two modes
                         a0 = damping_ratio * (2 * omega1 * omega2) / (omega1 + omega2)
                         a1 = damping_ratio * 2 / (omega1 + omega2)
                         # Apply Rayleigh damping
                         ops.rayleigh(a0, a1, 0, 0)

                     # OUTPUT DATA

```

```

ops.recorder('Node', '-file', f'{SALAR_DIR}DTH_DYN.txt', '-time', '-node', mid_node, '-dof', 1, 2, 'disp')# Displacement Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}DYN.DVN.txt', '-time', '-node', mid_node, '-dof', 1, 2, 3, 'vel') # Velocity Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}ATH.DVN.txt', '-time', '-node', mid_node, '-dof', 1, 2, 3, 'accel') # Acceleration Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_DYN_01.txt', '-time', '-node', 1, '-dof', 1, 2, 'reaction')# Base Shear Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_DYN_02.txt', '-time', num_nodes, '-dof', 1, 2, 'reaction')# Base Shear Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_DYN_03.txt', '-time', '-node', num_nodes + 1, '-dof', 1, 2, 'reaction')# Base Shear Time History
ops.recorder('Node', '-file', f'{SALAR_DIR}BTH_DYN_04.txt', '-time', '-node', 2*num_nodes, '-dof', 1, 2, 'reaction')# Base Shear

DISPLACEMENTS = []
DISP_X, DISP_Y, VELOCITY, ACCELERATION = [], [], [], []
BASEREACTION_X, BASEREACTION_Y = [], []

stable = 0
current_time = 0.0
# Perform the analysis with increments
while stable == 0 and current_time < duration:
    OK = ops.analyze(1, dt)
    ANALYSIS(OK, 1, TOLERANCE, MAX_ITERATIONS)
    current_time = ops.getTime()
    DISPLACEMENTS.append([ops.nodeDisp(j + 1) for j in range(num_nodes * 2)])
    DISP_X.append(ops.nodeDisp(mid_node, 1))
    DISP_Y.append(ops.nodeDisp(mid_node, 2))
    VELOCITY.append(ops.nodeVel(mid_node, 2))
    ACCELERATION.append(ops.nodeAccel(mid_node, 2))
    x1 = ops.nodeResponse(1, 1, 6)
    x2 = ops.nodeResponse(num_nodes, 1, 6)
    x3 = ops.nodeResponse(num_nodes + 1, 1, 6)
    x4 = ops.nodeResponse(2 * num_nodes, 1, 6)
    y1 = ops.nodeResponse(1, 2, 6)
    y2 = ops.nodeResponse(num_nodes, 2, 6)
    y3 = ops.nodeResponse(num_nodes + 1, 2, 6)
    y4 = ops.nodeResponse(2 * num_nodes, 2, 6)
    BASEREACTION_X.append(x1 + x2 + x3 + x4) # CABLE REACION-X
    BASEREACTION_Y.append(y1 + y2 + y3 + y4) # CABLE REACION-Y
    KE = BASEREACTION_Y[-1] / DISP_Y[-1] # Effective Lateral Stiffness
    #print(f'STEP: {i+1}')

# Get initial and final node coordinates for plotting
initial_coords = np.array([ops.nodeCoord(i + 1) for i in range(num_nodes * 2)])
deformed_coords = initial_coords + np.array(DISPLACEMENTS[-1])

# Output all unconstrained node displacements
for i in range(num_nodes * 2):
    disp = ops.nodeDisp(i + 1)
    print(f'Node {i + 1}: X Displacement = {disp[0]}, Y Displacement = {disp[1]}')

ops.wipe()
print("Period: ", modal_period)
print("Frequency: ", frequency)
if damping == False:
    print(' Undamping Structure Dynamic Analysis Done.')
if damping == True:
    print(' Damping Structure Dynamic Analysis Done.')

return initial_coords, deformed_coords, DISPLACEMENTS, VELOCITY, ACCELERATION, DISP_X, DISP_Y, BASEREACTION_X, BASEREACTION_Y

```

```

In [48]: def plot_time_history(DISP_X_undamped, DISP_X_damped, DISP_Y_undamped, DISP_Y_damped,
                           VELOCITY_undamped, VELOCITY_damped, ACCELERATION_undamped, ACCELERATION_damped,
                           BASEREACTION_X_undamped, BASEREACTION_X_damped, BASEREACTION_Y_undamped, BASEREACTION_Y_damped):
    import matplotlib.pyplot as plt
    import numpy as np
    # Assuming you have 'time' array representing the time steps
    time = [i * dt for i in range(len(DISPLACEMENTS_undamped))]
    plt.figure(figsize=(14, 20))

    # Plot Displacements in X direction
    plt.subplot(6, 1, 1)
    P1 = DISP_X_undamped
    P2 = DISP_X_damped
    P11 = np.max(np.abs(P1))
    P22 = np.max(np.abs(P2))
    P3 = P11 / P22
    plt.plot(time, P1, color='black', label=f'Undamped X: {P11:.5e}')
    plt.plot(time, P2, color='red', label=f'Damped X: {P22:.5e}')
    plt.title(f'Displacement Time History (X direction) - Amplification Factor: {P3:.3f}')
    plt.xlabel('Time [s]')
    plt.ylabel('Displacement [m] (X direction)')
    plt.legend()

    # Plot Displacements in Y direction
    plt.subplot(6, 1, 2)
    P1 = DISP_Y_undamped
    P2 = DISP_Y_damped
    P11 = np.max(np.abs(P1))
    P22 = np.max(np.abs(P2))
    P3 = P11 / P22
    plt.plot(time, P1, color='black', label=f'Undamped Y: {P11:.5e}')
    plt.plot(time, P2, color='red', label=f'Damped Y: {P22:.5e}')
    plt.title(f'Displacement Time History (Y direction) - Amplification Factor: {P3:.3f}')
    plt.xlabel('Time [s]')
    plt.ylabel('Displacement [m] (Y direction)')
    plt.legend()

    # Plot Velocities
    plt.subplot(6, 1, 3)
    P1 = VELOCITY_undamped
    P2 = VELOCITY_damped
    P11 = np.max(np.abs(P1))
    P22 = np.max(np.abs(P2))
    P3 = P11 / P22
    plt.plot(time, P1, color='black', label=f'Undamped: {P11:.5e}')
    plt.plot(time, P2, color='red', label=f'Damped: {P22:.5e}')
    plt.title(f'Velocity Time History - Amplification Factor: {P3:.3f}')
    plt.xlabel('Time [s]')
    plt.ylabel('Velocity [m/s] (Y direction)')
    plt.legend()

    # Plot Accelerations
    plt.subplot(6, 1, 4)
    P1 = ACCELERATION_undamped
    P2 = ACCELERATION_damped
    P11 = np.max(np.abs(P1))

```

```

P22 = np.max(np.abs(P2))
P3 = P11 / P22
plt.plot(time, P1, color='black', label=f'Undamped: {P11:.5e}')
plt.plot(time, P2, color='red', label=f'Damped: {P22:.5e}')
plt.title(f'Acceleration Time History - Amplification Factor: {P3:.3f}')
plt.xlabel('Time [s]')
plt.ylabel('Acceleration [m/s2] (Y direction)')
plt.legend()

# Plot Base Reactions in X direction
plt.subplot(6, 1, 5)
P1 = BASE_REACTION_X_undamped
P2 = BASE_REACTION_X_damped
P11 = np.max(np.abs(P1))
P22 = np.max(np.abs(P2))
P3 = P11 / P22
plt.plot(time, P1, color='black', label=f'Undamped: {P11:.5e}')
plt.plot(time, P2, color='red', label=f'Damped: {P22:.5e}')
plt.title(f'Base Reaction Time History (X direction) - Amplification Factor: {P3:.3f}')
plt.xlabel('Time [s]')
plt.ylabel('Base Reaction [N] (X direction)')
plt.legend()

# Plot Base Reactions in Y direction
plt.subplot(6, 1, 6)
P1 = BASE_REACTION_Y_undamped
P2 = BASE_REACTION_Y_damped
P11 = np.max(np.abs(P1))
P22 = np.max(np.abs(P2))
P3 = P11 / P22
plt.plot(time, P1, color='black', label=f'Undamped: {P11:.5e}')
plt.plot(time, P2, color='red', label=f'Damped: {P22:.5e}')
plt.title(f'Base Reaction Time History (Y direction) - Amplification Factor: {P3:.3f}')
plt.xlabel('Time [s]')
plt.ylabel('Base Reaction [N] (Y direction)')
plt.legend()

plt.tight_layout()
plt.show()

```

```

In [49]: # -----
# DYNAMIC ANALYSIS
# -----
# Parameters for the analysis
L = 50000.0          # [mm] Bridge span length
H1 = 3500.0           # [mm] Height of Top Cable
arc_depth = 1000.0    # [mm]
E_cable = 210e5        # [N/mm2] Modulus of elasticity Cable
Cable_Dia_01 = 25     # [mm] Horizontal Longitudinal Cable Diameter Bottom
Cable_Dia_02 = 18     # [mm] Horizontal Longitudinal Cable Diameter Top
Cable_Dia_03 = 10     # [mm] Vertical Longitudinal Cable Diameter Top
num_nodes = 51         # Cable Arc Number of nodes

TOTAL_MASS = 500000.0   # [kg] Total Mass of Structure
damping_ratio = 0.05    # Damping ratio
duration = 50.0          # [s] Duration of the analysis in seconds
dt = 0.01               # Time step in seconds

MAX_ITERATIONS = 10000  # Maximum number of iterations
TOLERANCE = 1.0e-14      # Tolerance for convergence

import time
starttime = time.process_time()

# Run the undamped analysis
damping = False
LINEAR = True # False: Cable Nonlinear Materials Properties
periodTF = False
results_undamped = DYNAMIC_ANALYSIS(damping, damping_ratio, LINEAR, periodTF, duration, dt, TOTAL_MASS, L, H1, arc_depth, E_cable, Cable_Dia_01, Cable_Dia_02, Cable_Dia_03, num_nodes, initial_coords, deformed_coords, DISPLACEMENTS_undamped, VELOCITY_undamped, ACCELERATION_undamped, DISP_X_undamped, DISP_Y_undamped, BASE_REACTION_X_undamped, BASE_REACTION_Y_undamped)

# Run the damped analysis
damping = True
LINEAR = True # False: Cable Nonlinear Materials Properties
periodTF = False
results_damped = DYNAMIC_ANALYSIS(damping, damping_ratio, LINEAR, periodTF, duration, dt, TOTAL_MASS, L, H1, arc_depth, E_cable, Cable_Dia_01, Cable_Dia_02, Cable_Dia_03, num_nodes, M, initial_coords, deformed_coords, DISPLACEMENTS_damped, VELOCITY_damped, ACCELERATION_damped, DISP_X_damped, DISP_Y_damped, BASE_REACTION_X_damped, BASE_REACTION_Y_damped = results_damp)

totaltime = time.process_time() - starttime
print(f'\nTotal time (s): {totaltime:.4f} \n\n')

```

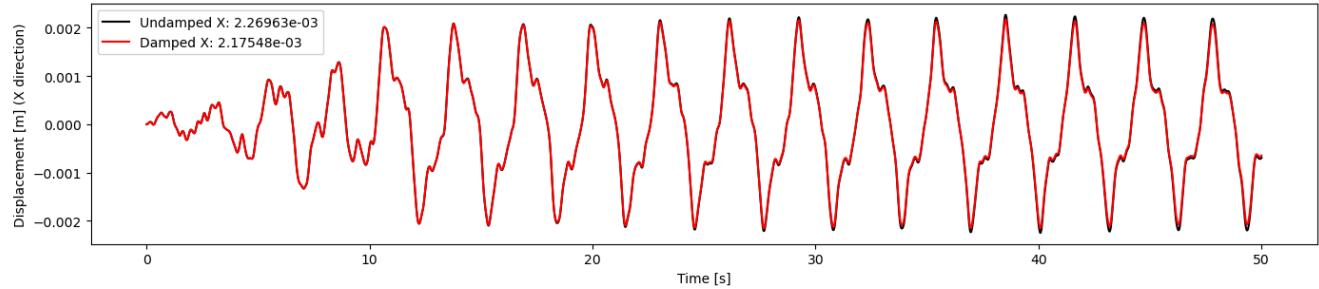
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 Node 93: X Displacement = -0.000519110402934358, Y Displacement = -0.01938965342085049
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 Node 95: X Displacement = -0.0004970276653674196, Y Displacement = -0.024476608425267964
 Node 96: X Displacement = -0.000494047208633195, Y Displacement = -0.027141698251925972
 Node 97: X Displacement = -0.0004956820580765575, Y Displacement = -0.02987394462243987
 Node 98: X Displacement = -0.00048141910620336773, Y Displacement = -0.03266256466874812
 Node 99: X Displacement = -0.0004300975569750489, Y Displacement = -0.03549655288674025
 Node 100: X Displacement = -0.0003296702556367218, Y Displacement = -0.03836441333199171
 Node 101: X Displacement = -0.00018033073851959487, Y Displacement = -0.04102533962523656
 Node 102: X Displacement = 0.0, Y Displacement = 0.0
 Period: 0.0
 Frequency: 0.0
 Damping Structure Dynamic Analysis Done.

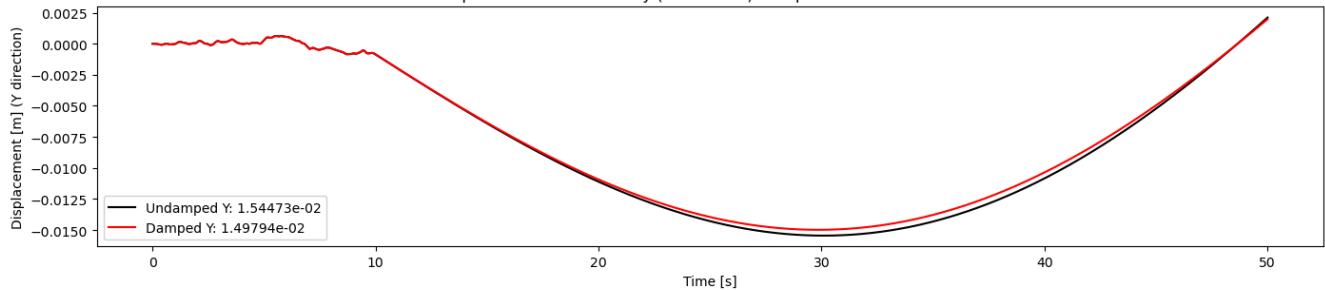
Total time (s): 10.8594

```
In [50]: plot_time_history(DISP_X_undamped, DISP_X_damped, DISP_Y_undamped, DISP_Y_damped, VELOCITY_undamped, VELOCITY_damped, ACCELERATION_undamped, ACCELERATION_damped, BASEREACTION_X_undamped)
```

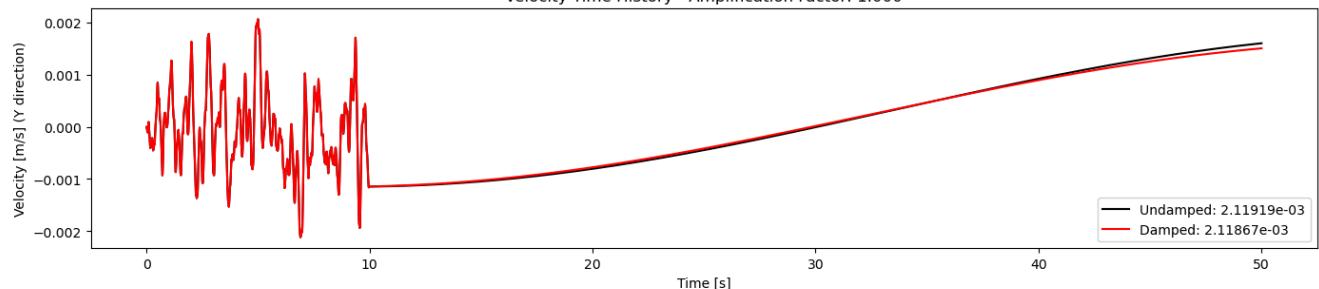
Displacement Time History (X direction) - Amplification Factor: 1.043



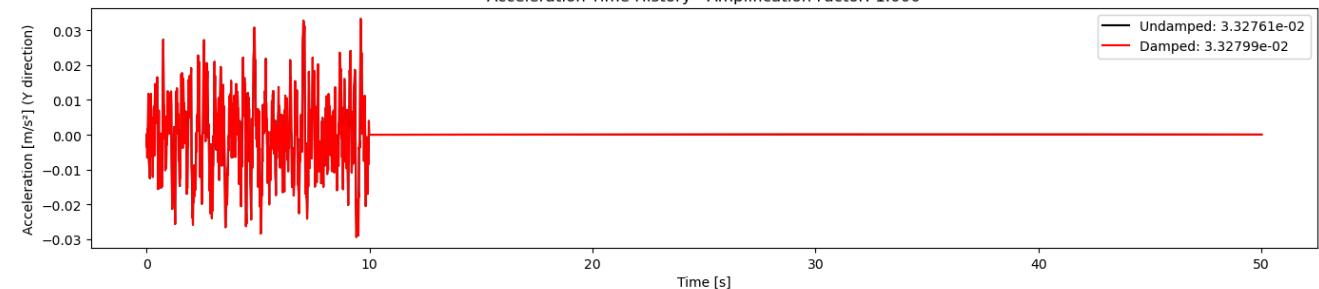
Displacement Time History (Y direction) - Amplification Factor: 1.031



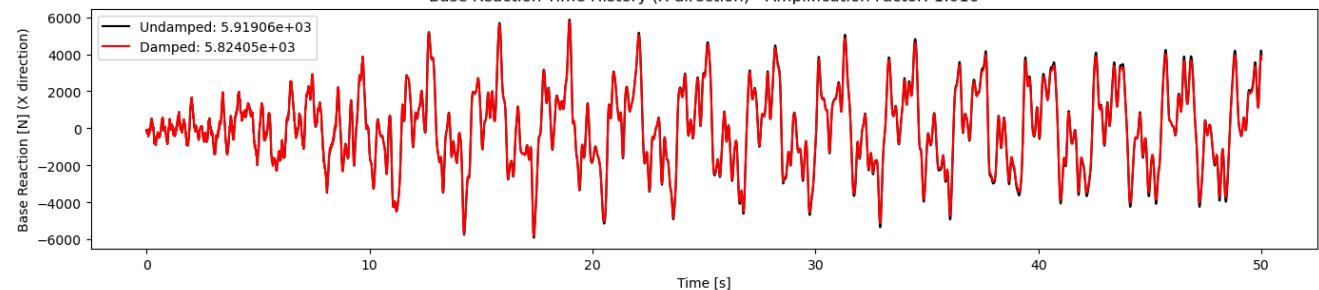
Velocity Time History - Amplification Factor: 1.000



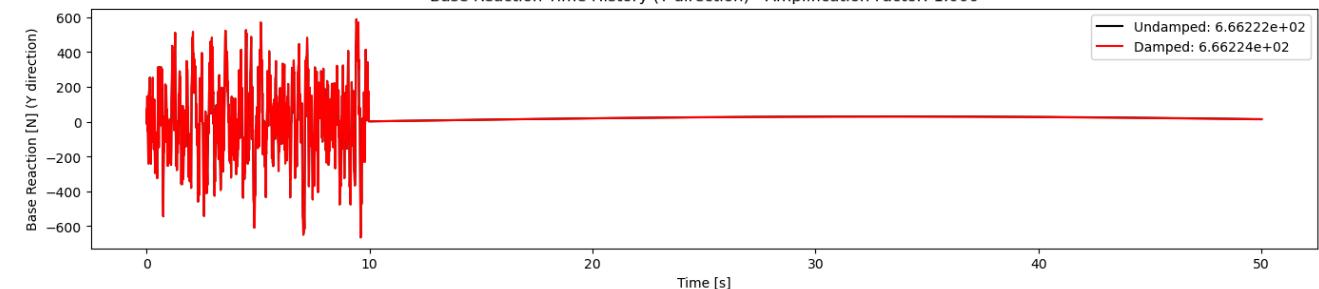
Acceleration Time History - Amplification Factor: 1.000



Base Reaction Time History (X direction) - Amplification Factor: 1.016



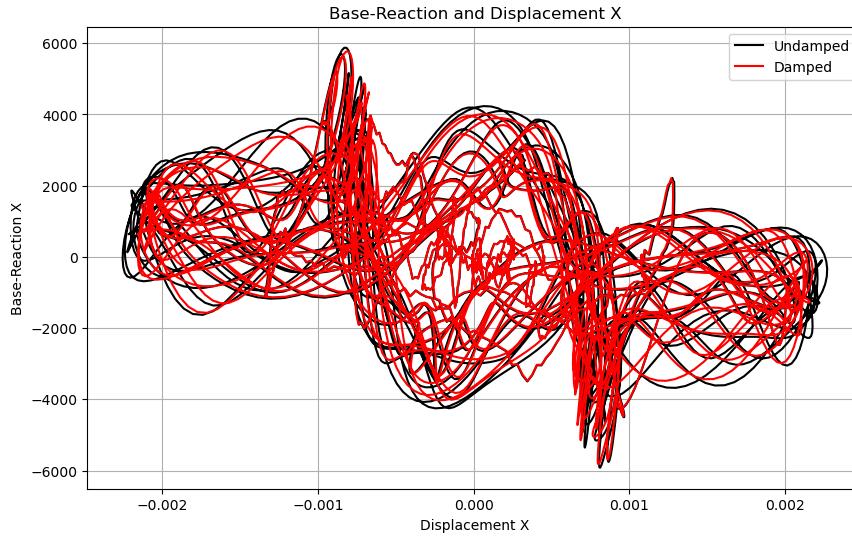
Base Reaction Time History (Y direction) - Amplification Factor: 1.000



```
In [51]: def PLOT_2D(X1, Y1, X2, Y2, XLABEL, YLABEL, TITLE):
    plt.figure(figsize=(10, 6))
    plt.plot(X1, Y1, label='Undamped', color='black')
    plt.plot(X2, Y2, label='Damped', color='red')
    plt.xlabel(XLABEL)
    plt.ylabel(YLABEL)
    plt.title(TITLE)
    plt.grid(True)
    #plt.semilogy()
    plt.legend()
    plt.show()
```

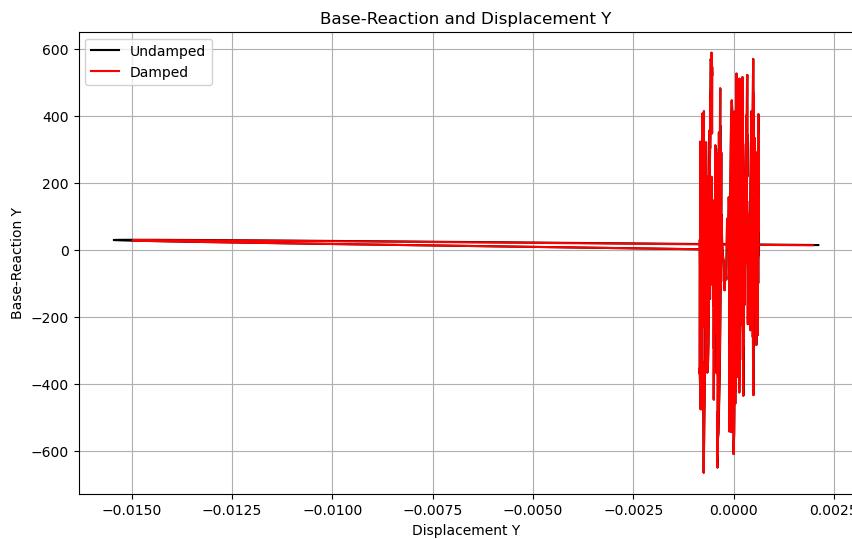
```
In [53]: ### BASE REACTION & DISPLACEMENT IN X:
X1 = DISP_X_undamped
X2 = DISP_X_damped
Y1 = BASE_REACTION_X_undamped
Y2 = BASE_REACTION_X_damped
XLABEL = 'Displacement X'
YLABEL = 'Base-Reaction X'
TITLE = 'Base-Reaction and Displacement X'

PLOT_2D(X1, Y1, X2, Y2, XLABEL, YLABEL, TITLE)
```



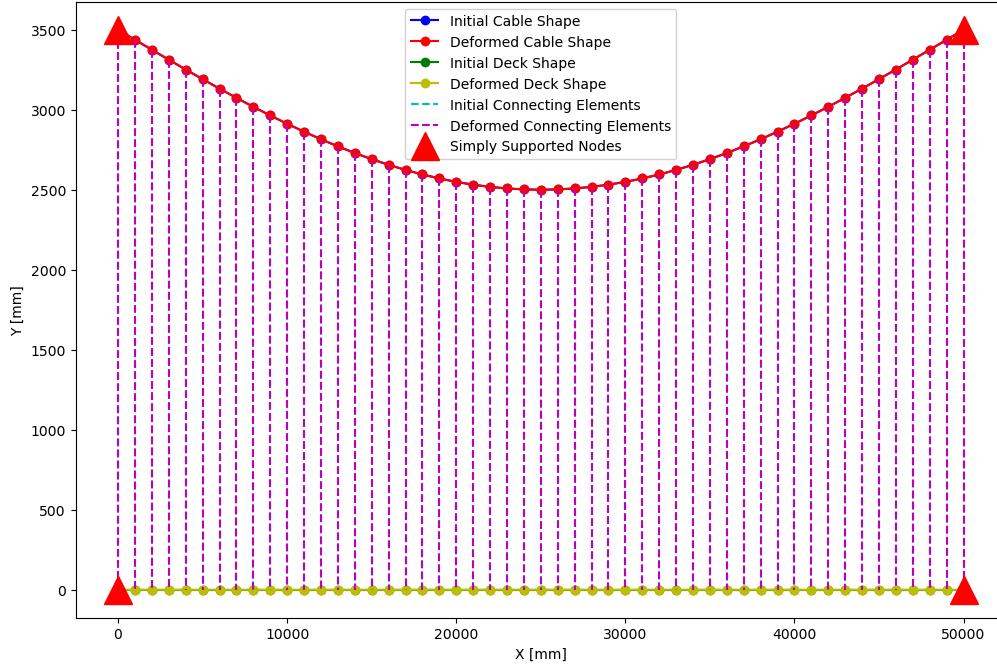
```
In [54]: ### BASE REACTION & DISPLACEMENT IN Y:
X1 = DISP_Y_undamped
X2 = DISP_Y_damped
Y1 = BASE_REACTION_Y_undamped
Y2 = BASE_REACTION_Y_damped
XLABEL = 'Displacement Y'
YLABEL = 'Base-Reaction Y'
TITLE = 'Base-Reaction and Displacement Y'

PLOT_2D(X1, Y1, X2, Y2, XLABEL, YLABEL, TITLE)
```



```
In [55]: # Plot damped results
plot_shapes(initial_coords, DISPLACEMENTS_damped)
```

Pushover Analysis: Cable and Deck Deformed Shapes



Plotting a histogram for the results of dynamic analysis of a structure, such as displacements, accelerations, and support reactions, helps analyze the statistical distribution and trends in the data. The following insights can be drawn from these histograms:

1. Range Identification

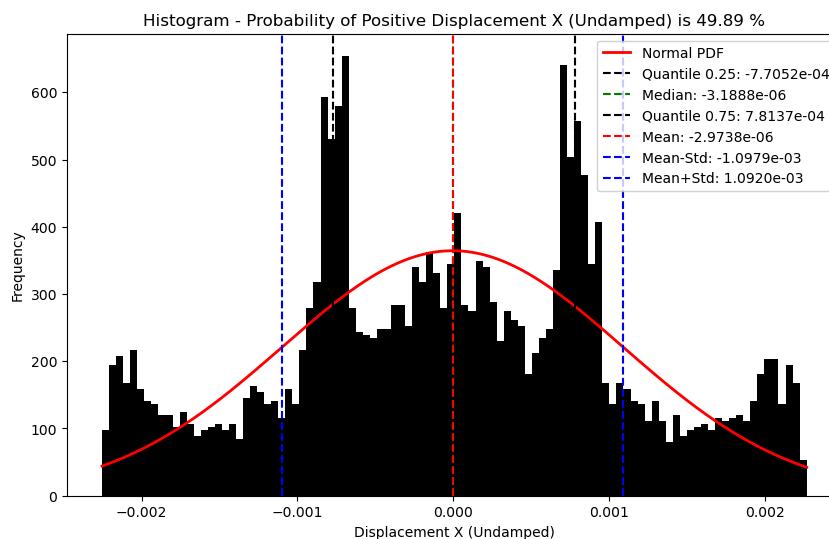
The histogram shows the range within which displacements, accelerations, or support reactions vary. This is crucial for assessing the stability of the structure and determining the allowable design limits. 2. Central Tendency From the histogram, measures like mean, median, or mode can be extracted. This helps identify the most likely values and provides an overview of the dominant behavior of the structure. 3. Data Dispersion The histogram illustrates the spread of the data (e.g., through its width and shape). A wide histogram may indicate nonlinear behavior or unusual effects in the structure. 4. Anomaly Detection Unexpected peaks or asymmetric distributions may indicate abnormal structural behavior (e.g., weaknesses or inelastic behavior). It can also reveal specific phenomena like resonances or amplification effects. 5. Probability of Specific Values The histogram's shape can indicate the probability distribution of the data (e.g., normal, uniform, or Poisson). This is useful for simulations and predicting future structural behavior. 6. Behavior Under Specific Conditions Analyzing histograms of acceleration or displacement can reveal dominant modes of the structure and their impact on overall response. These insights are helpful in designing vibration control systems (e.g., dampers). 7. Impact of External Forces or Loading An unusual histogram shape might suggest the influence of extraordinary dynamic loads (e.g., severe earthquakes or impact forces). Conclusion: Histograms are a simple yet effective tool for analyzing dynamic data. They help engineers and designers:

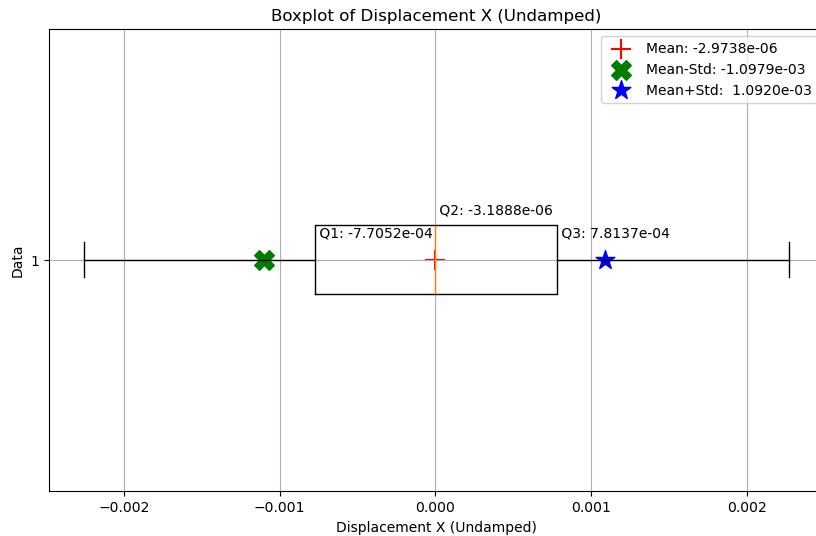
Understand the overall behavior of a structure. Assess the likelihood of extreme or unexpected responses. Define appropriate design or retrofitting criteria. For deeper analysis, histograms can be combined with other statistical tools, such as standard deviation or spectral analysis.

```
In [56]: HISTOGRAM_BOXPLOT(DISP_X_undamped, HISTO_COLOR='black', LABEL='Displacement X (Undamped)')
HISTOGRAM_BOXPLOT(DISP_X_damped, HISTO_COLOR='grey', LABEL='Displacement X (Damped)')
#HISTOGRAM_BOXPLOT_PLOTLY(DISP_X_undamped, XLABEL='Displacement X (Undamped)', TITLE='Displacement X (Undamped)', COLOR='black')
#HISTOGRAM_BOXPLOT_PLOTLY(DISP_X_damped, XLABEL='Displacement X (Damped)', TITLE='Displacement X (Damped)', COLOR='grey')
```

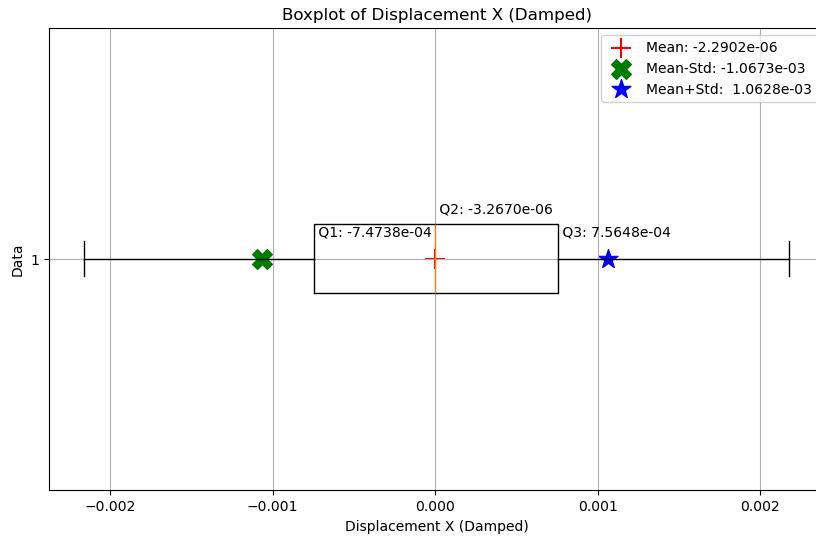
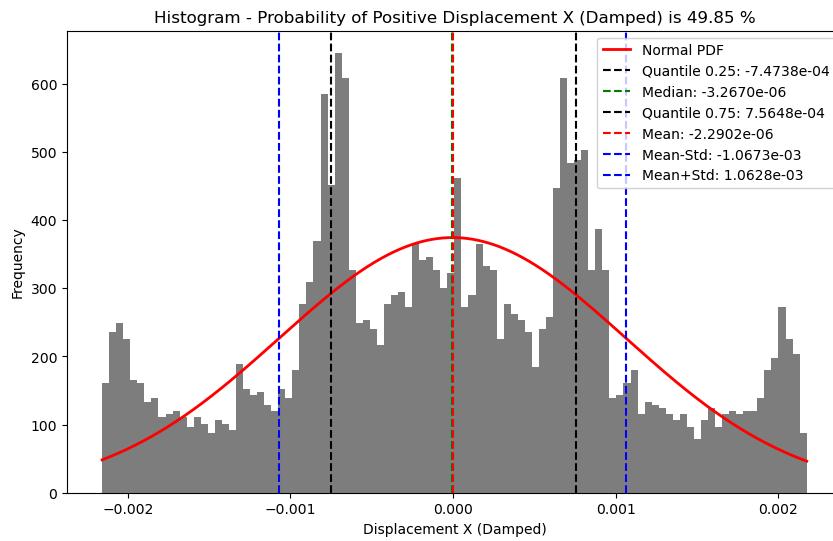
Box-Chart Data:

```
Minimum: -2.2531e-03
First quartile: -7.7052e-04
Median: -3.1888e-06
Mean: -2.9738e-06
Std: 1.0949e-03
Third quartile: 7.8137e-04
Maximum: 2.2696e-03
Skewness: 4.0488e-03
Kurtosis: -6.1986e-01
90% Confidence Interval: (-1.9638e-03, 1.9613e-03)
```





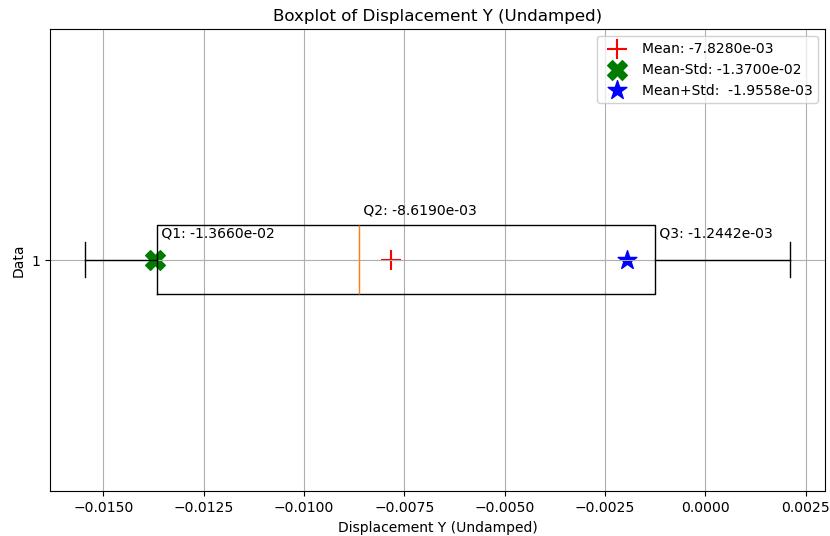
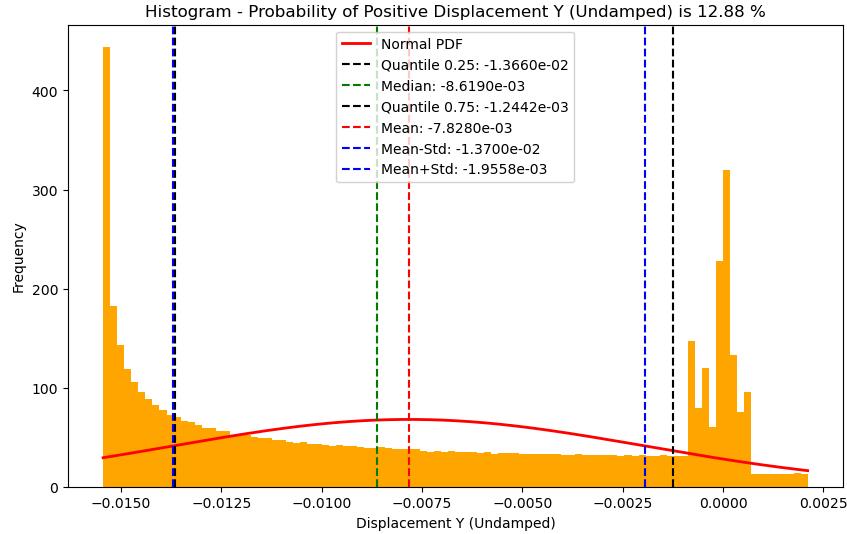
```
-----
Box-Chart Datas:
Minimum: -2.1582e-03
First quartile: -7.4738e-04
Median: -3.2670e-06
Mean: -2.2902e-06
Std: 1.0650e-03
Third quartile: 7.5648e-04
Maximum: 2.1755e-03
Skewness: 6.7665e-03
kurtosis: -6.2866e-01
90% Confidence Interval: (-1.9084e-03, 1.9175e-03)
-----
```



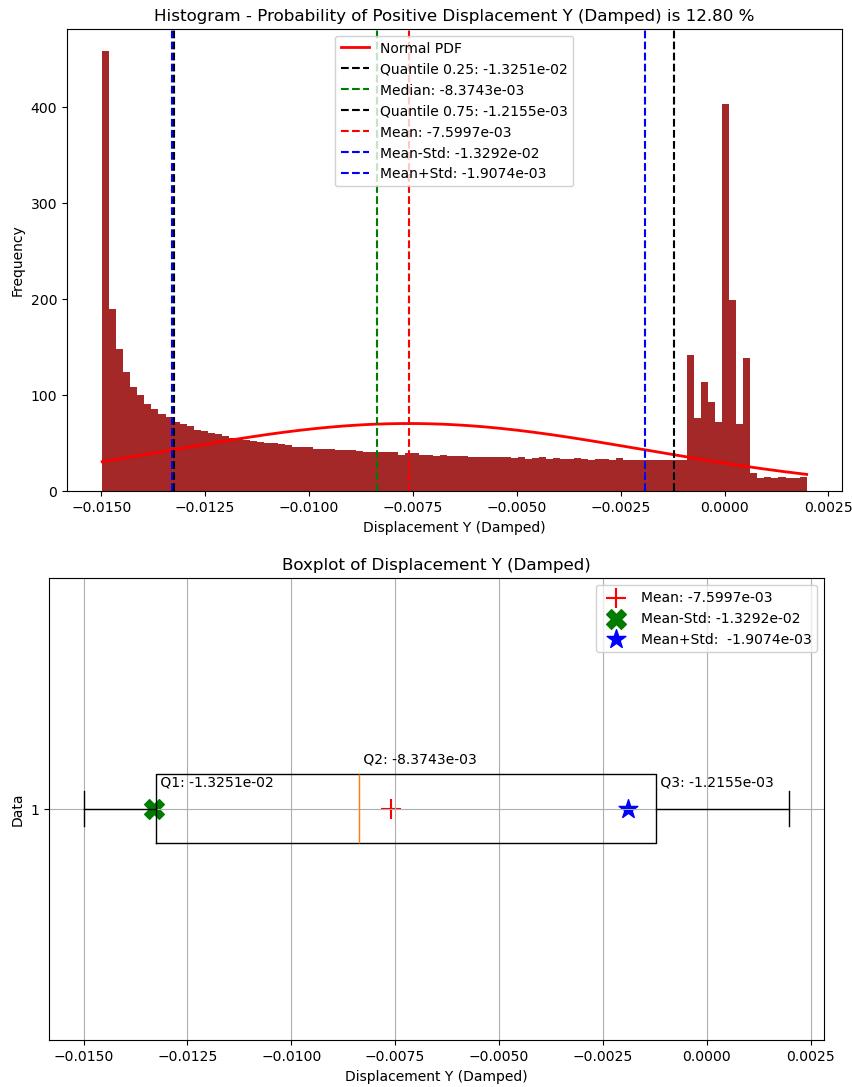
```
In [57]: HISROGRAM_BOXPLOT(DISP_Y_undamped, HISTO_COLOR='orange', LABEL='Displacement Y (Undamped)')
HISROGRAM_BOXPLOT(DISP_Y_damped, HISTO_COLOR='brown', LABEL='Displacement Y (Damped)')
```

```
#HISTOGRAM_BOXPLOT_PLOTLY(DISP_Y_undamped, XLABEL='Displacement Y (Undamped)', TITLE='Displacement Y (Undamped)', COLOR='orange')
#HISTOGRAM_BOXPLOT_PLOTLY(DISP_Y_damped, XLABEL='Displacement Y (Damped)', TITLE='Displacement Y (Damped)', COLOR='brown')
```

Box-Chart Datas:
Minimum: -1.5447e-02
First quartile: -1.3660e-02
Median: -8.6190e-03
Mean: -7.8280e-03
Std: 5.8722e-03
Third quartile: -1.2442e-03
Maximum: 2.1095e-03
Skewness: 1.5596e-01
kurtosis: -1.5264e+00
90% Confidence Interval: (-1.5375e-02, 3.3226e-04)

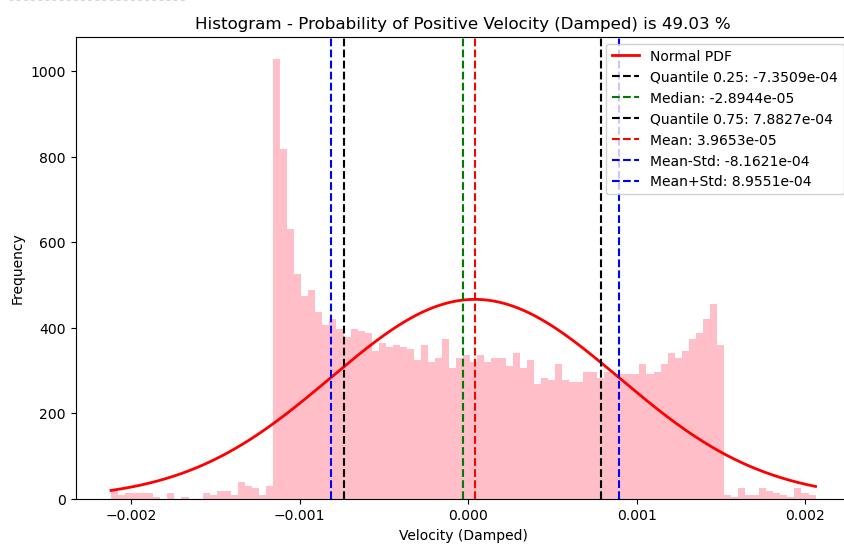
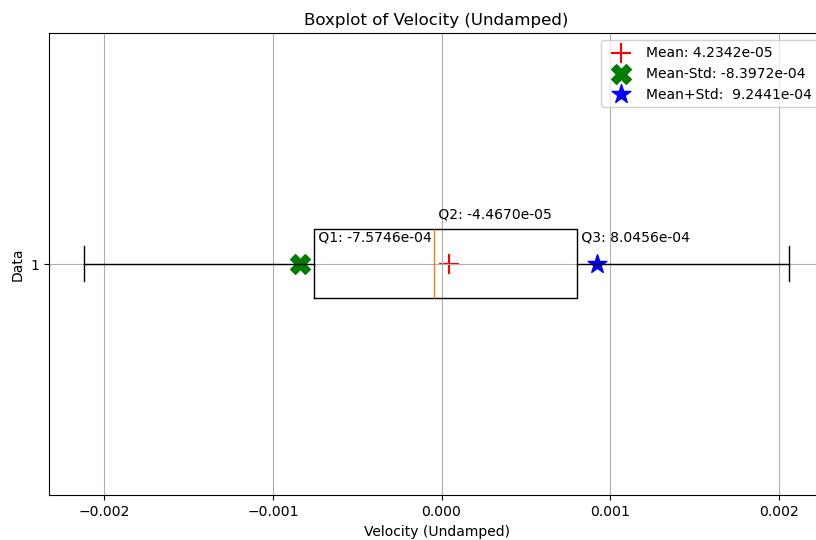
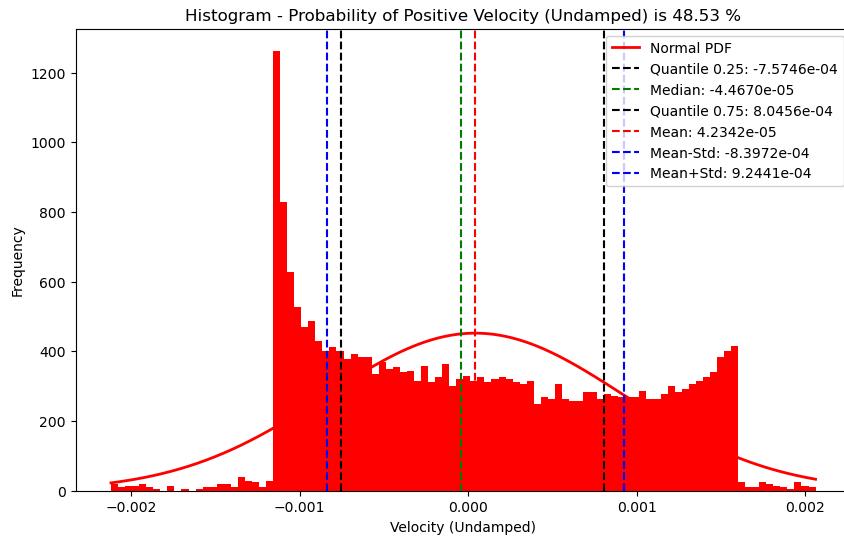


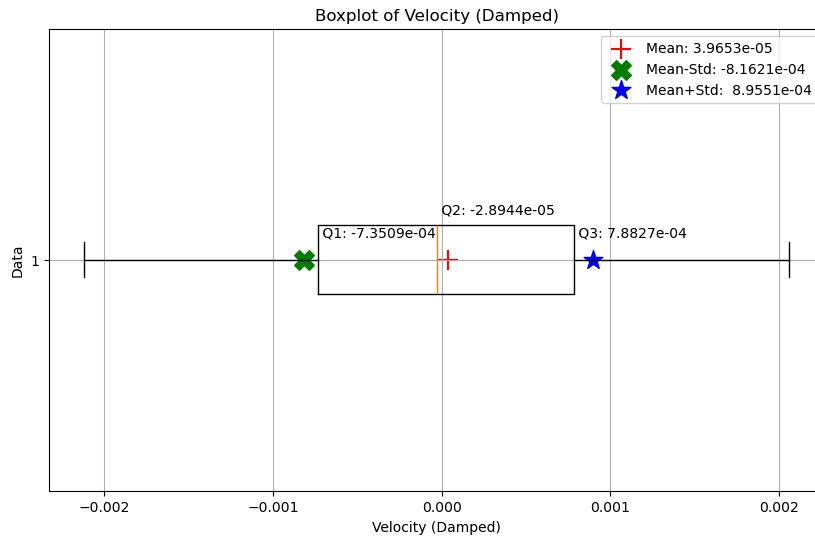
Box-Chart Datas:
Minimum: -1.4979e-02
First quartile: -1.3251e-02
Median: -8.3743e-03
Mean: -7.5997e-03
Std: 5.6923e-03
Third quartile: -1.2155e-03
Maximum: 1.9755e-03
Skewness: 1.5760e-01
kurtosis: -1.5264e+00
90% Confidence Interval: (-1.4909e-02, 3.2601e-04)



```
In [58]: HISTOGRAM_BOXPLOT(VELOCITY_undamped, HISTO_COLOR='red', LABEL='Velocity (Undamped)')
HISTOGRAM_BOXPLOT(VELOCITY_damped, HISTO_COLOR='pink', LABEL='Velocity (Damped)')
#HISTOGRAM_BOXPLOT_PLOTLY(VELOCITY_undamped, XLABEL='Velocity (Undamped)', TITLE='Velocity (Undamped)', COLOR='red')
#HISTOGRAM_BOXPLOT_PLOTLY(VELOCITY_damped, XLABEL='Velocity (Damped)', TITLE='Velocity (Damped)', COLOR='pink')
```

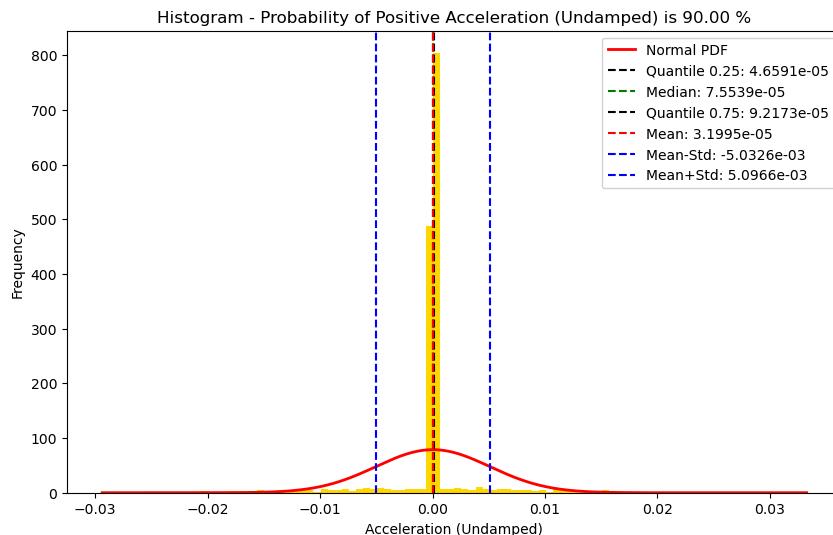
```
-----
Box-Chart Datas:
Minimum: -2.1192e-03
First quartile: -7.5746e-04
Median: -4.4670e-05
Mean: 4.2342e-05
Std: 8.8206e-04
Third quartile: 8.0456e-03
Maximum: 2.0610e-03
Skewness: 2.0954e-01
kurtosis: -1.1562e+00
90% Confidence Interval: (-1.1298e-03, 1.4930e-03)
-----
```

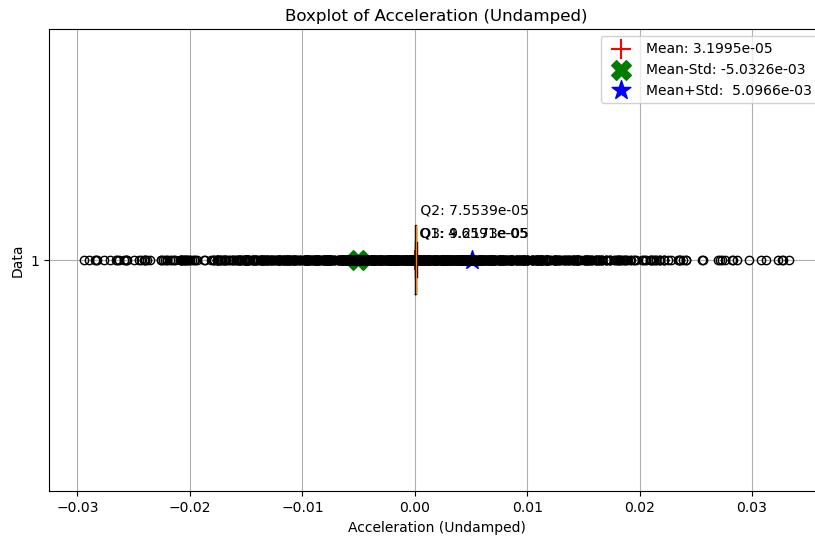




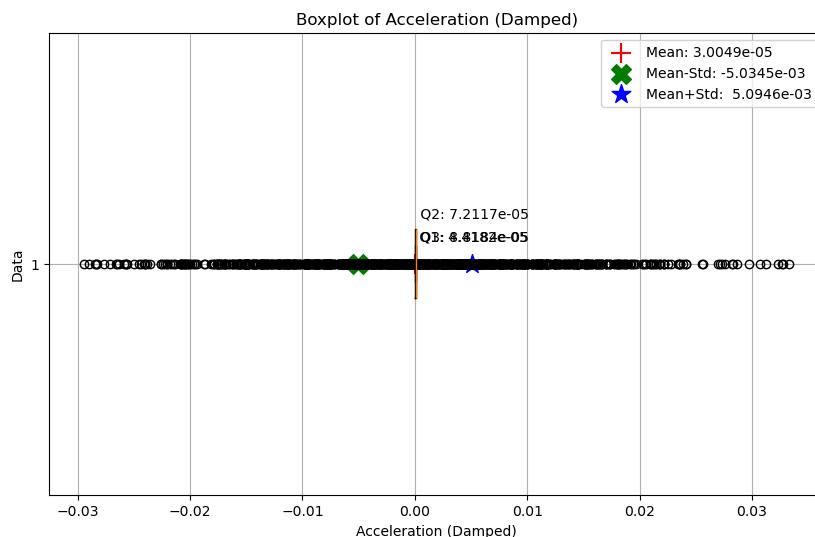
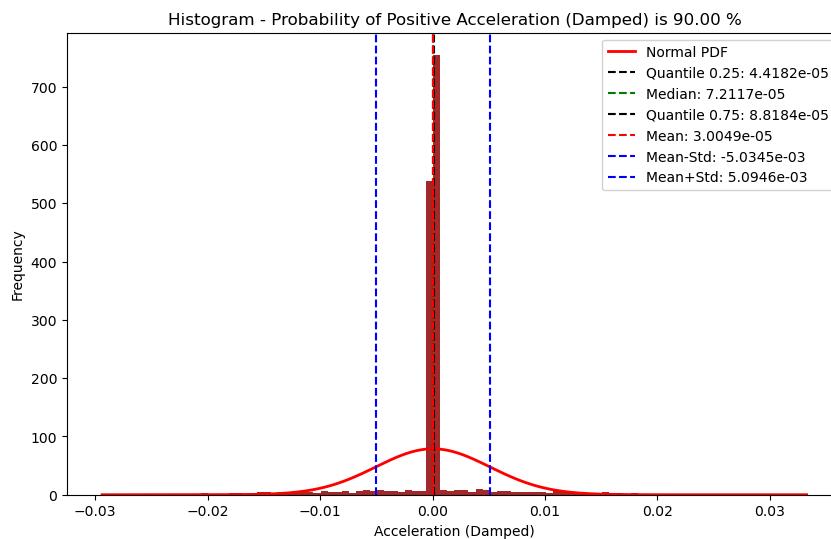
```
In [59]: HISTOGRAM_BOXPLOT(ACCELERATION_undamped, HISTO_COLOR='gold', LABEL='Acceleration (Undamped)')
HISTOGRAM_BOXPLOT(ACCELERATION_damped, HISTO_COLOR='brown', LABEL='Acceleration (Damped)')
#HISTOGRAM_BOXPLOT_PLOTLY(ACCELERATION_undamped, XLABEL='Acceleration (Undamped)', TITLE='Acceleration (Undamped)', COLOR='gold')
#HISTOGRAM_BOXPLOT_PLOTLY(ACCELERATION_damped, XLABEL='Acceleration (Damped)', TITLE='Acceleration (Damped)', COLOR='brown')
```

Box-Chart Data:
Minimum: -2.9386e-02
First quartile: 4.6591e-05
Median: 7.5539e-05
Mean: 3.1995e-05
Std: 5.0646e-03
Third quartile: 9.2173e-05
Maximum: 3.3276e-02
Skewness: 7.3052e-02
kurtosis: 1.1642e+01
90% Confidence Interval: (-7.6494e-03, 6.9992e-03)





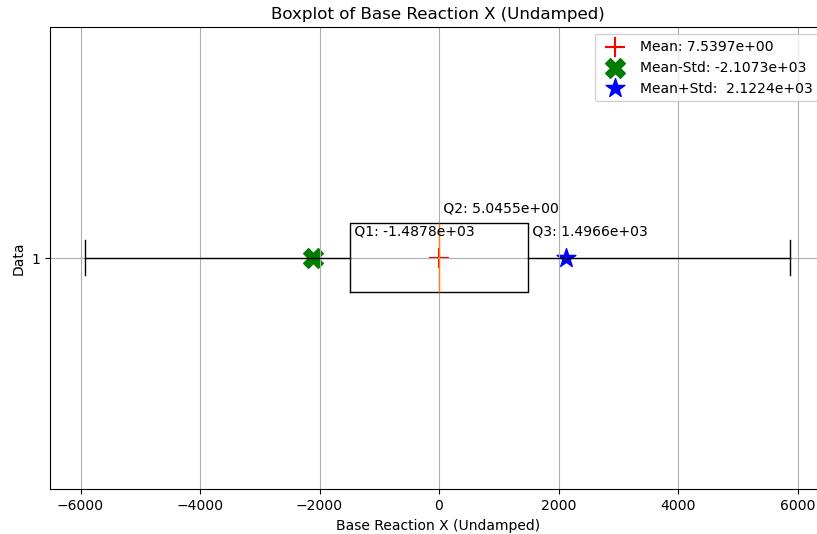
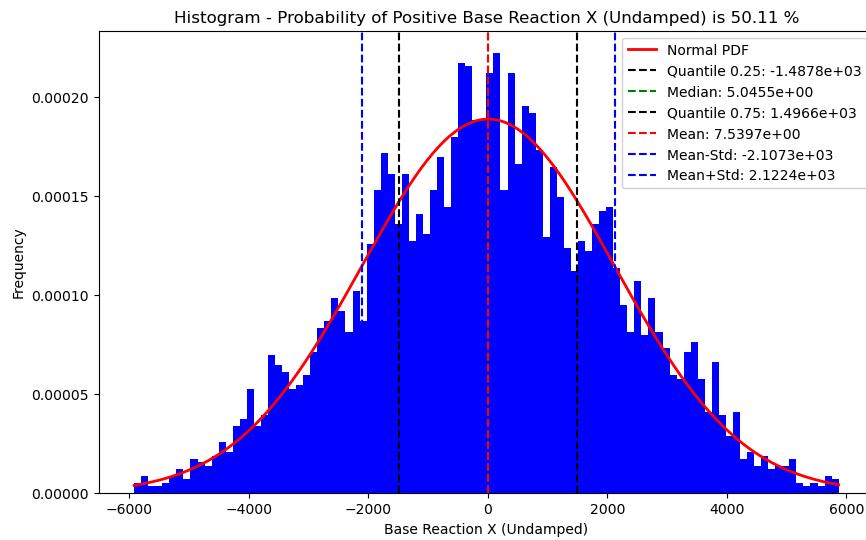
```
-----
Box-Chart Data:
Minimum: -2.9389e-02
First quartile: 4.4182e-05
Median: 7.2117e-05
Mean: 3.0049e-05
Std: 5.0646e-03
Third quartile: 8.8184e-05
Maximum: 3.3280e-02
Skewness: 7.4283e-02
kurtosis: 1.1643e+01
90% Confidence Interval: (-7.6465e-03, 6.9998e-03)
-----
```



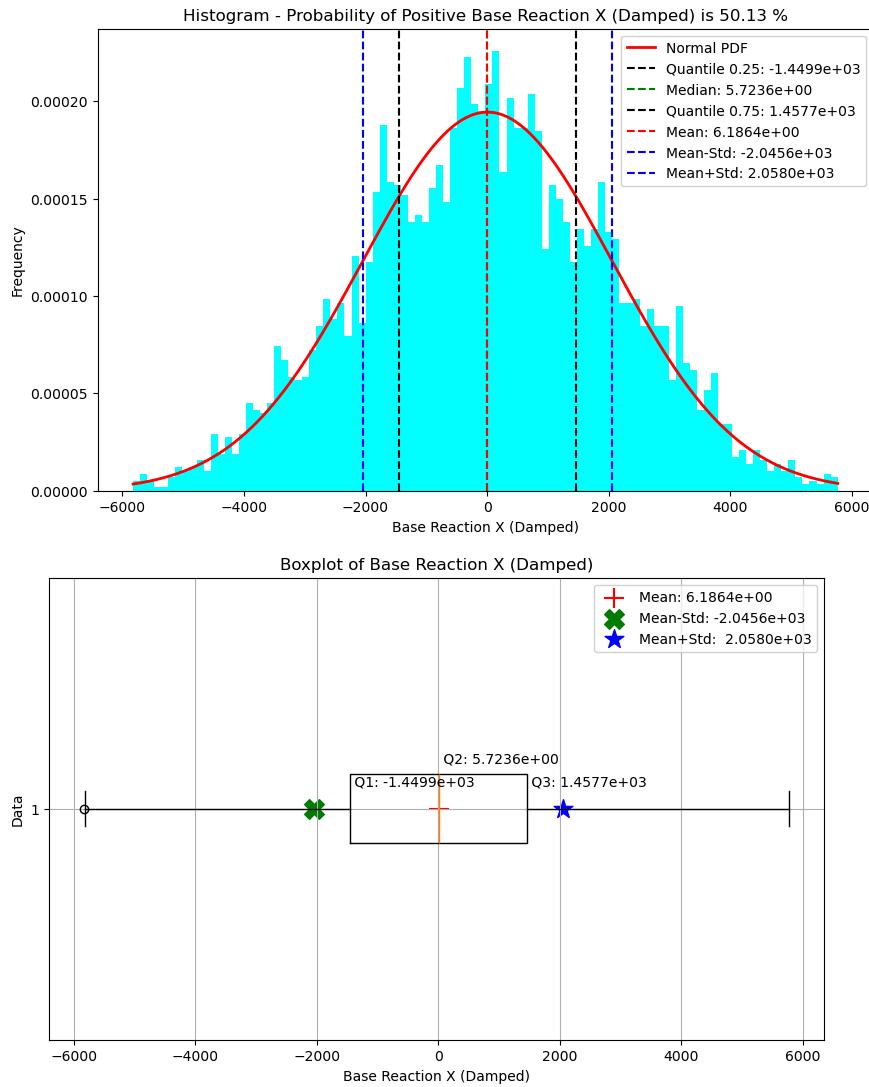
```
In [60]: HISROGRAM_BOXPLOT(BASE_REACTION_X_undamped, HISTO_COLOR='blue', LABEL='Base Reaction X (Undamped)')
HISROGRAM_BOXPLOT(BASE_REACTION_X_damped, HISTO_COLOR='cyan', LABEL='Base Reaction X (Damped)')
```

```
#HISTOGRAM_BOXPLOT_PLOTLY(BASE_REACTION_X_undamped, XLABEL='Base Reaction X (Undamped)', TITLE='Base Reaction X (Undamped)', COLOR='blue')
#HISTOGRAM_BOXPLOT_PLOTLY(BASE_REACTION_X_damped, XLABEL='Base Reaction X (Damped)', TITLE='Base Reaction X (Damped)', COLOR='cyan')
```

Box-Chart Datas:
Minimum: -5.9191e+03
First quartile: -1.4878e+03
Median: 5.0455e+00
Mean: 7.5397e+00
Std: 2.1149e+03
Third quartile: 1.4966e+03
Maximum: 5.8726e+03
Skewness: -1.0914e-02
kurtosis: -3.3292e-01
90% Confidence Interval: (-3.5753e+03, 3.5248e+03)

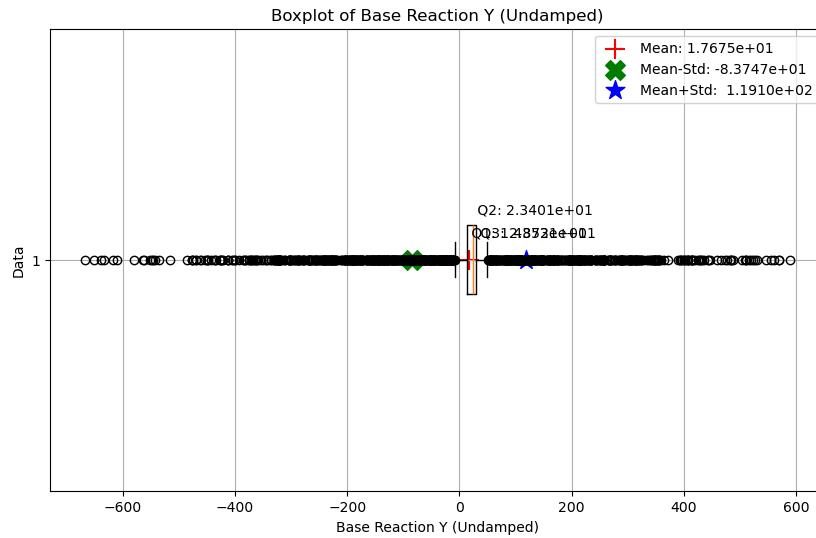
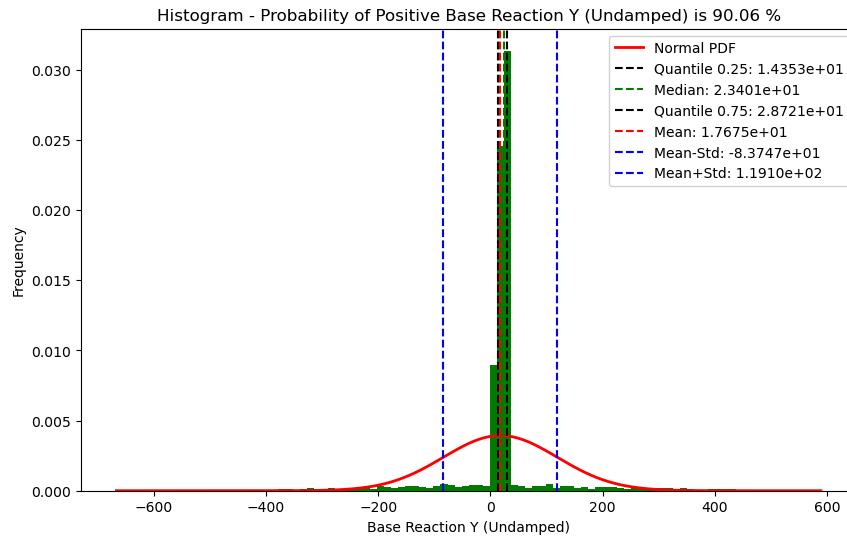


Box-Chart Datas:
Minimum: -5.8241e+03
First quartile: -1.4499e+03
Median: 5.7236e+00
Mean: 6.1864e+00
Std: 2.0518e+03
Third quartile: 1.4577e+03
Maximum: 5.7685e+03
Skewness: -1.2907e-02
kurtosis: -3.2647e-01
99% Confidence Interval: (-3.4306e+03, 3.3849e+03)

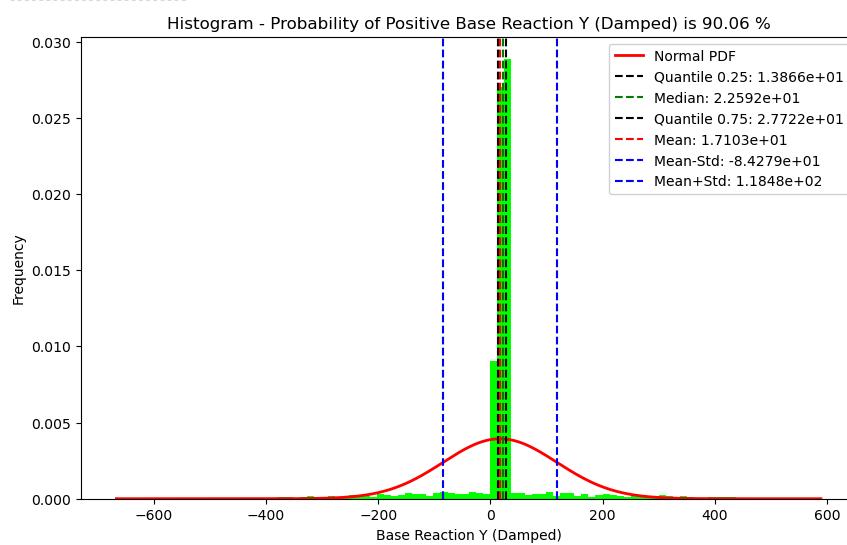


```
In [61]: HISTOGRAM_BOXPLOT(BASE_REACTION_Y_undamped, HISTO_COLOR='green', LABEL='Base Reaction Y (Undamped)')
HISTOGRAM_BOXPLOT(BASE_REACTION_Y_damped, HISTO_COLOR='lime', LABEL='Base Reaction Y (Damped)')
#HISTOGRAM_BOXPLOT_PLOTLY(BASE_REACTION_Y_undamped, XLABEL='Base Reaction Y (Undamped)', TITLE='Base Reaction Y (Undamped)', COLOR='green')
#HISTOGRAM_BOXPLOT_PLOTLY(BASE_REACTION_Y_damped, XLABEL='Base Reaction Y (Damped)', TITLE='Base Reaction Y (Damped)', COLOR='lime')
```

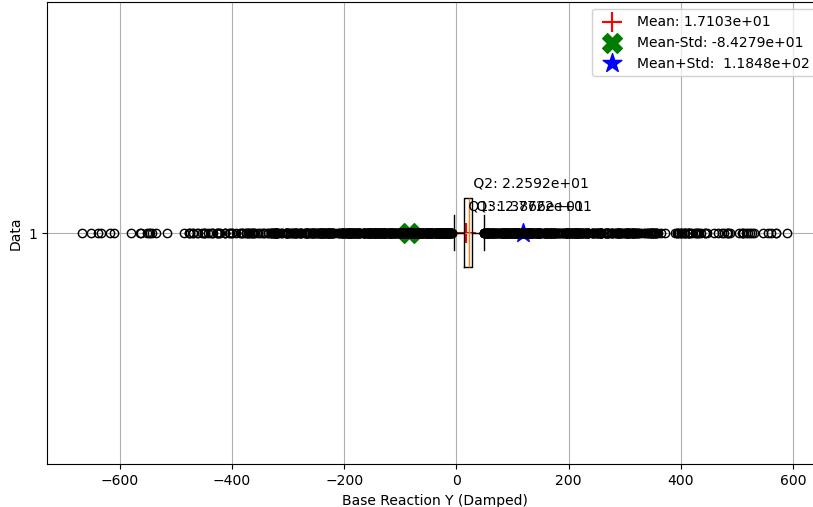
 Box-Chart Data:
 Minimum: -6.6622e+02
 First quartile: 1.4353e+01
 Median: 2.3401e+01
 Mean: 1.7675e+01
 Std: 1.0142e+02
 Third quartile: 2.8721e+01
 Maximum: 5.8865e+02
 Skewness: -5.9676e-01
 kurtosis: 1.1418e+01
 90% Confidence Interval: (-1.4462e+02, 1.5239e+02)



```
-----  
Box-Chart Datas:  
Minimum: -6.662e+02  
First quartile: 1.3866e+01  
Median: 2.2592e+01  
Mean: 1.7103e+01  
Std: 1.0138e+02  
Third quartile: 2.7722e+01  
Maximum: 5.8865e+02  
Skewness: -5.8863e-01  
kurtosis: 1.1428e+01  
90% Confidence Interval: (-1.4462e+02, 1.5239e+02)  
-----
```



Boxplot of Base Reaction Y (Damped)



```
In [ ]:
In [ ]:
In [ ]: ##### BEAM #####
def PUSHOVER_ANALYSIS_BEAM(span, height, A_cable, num_nodes, MAX_DISP, MAX_ITERATIONS, TOLERANCE):
    import openseespy.opensees as ops
    import numpy as np
    import matplotlib.pyplot as plt

    # Define model builder
    ops.wipe()
    ops.model('basic', '-ndm', 2, '-ndf', 3)
    dx = span / (num_nodes - 1)

    # Create nodes
    for i in range(num_nodes):
        x = i * dx
        y = -height * np.sin(np.pi * x / span)
        ops.node(i + 1, x, y)
    for j in range(num_nodes):
        ops.node(num_nodes + j + 1, j * dx, -7000)

    # Define boundary conditions (fixed at both ends)
    ops.fix(1, 1, 1) # TOP CABLE
    ops.fix(num_nodes, 1, 1) # TOP CABLE
    ops.fix(num_nodes + 1, 1, 1) # BOTTOM CABLE - DECK
    ops.fix(2 * num_nodes, 1, 1) # BOTTOM CABLE - DECK

    # Define material properties
    Fy_Cable = 355 # [N/mm^2] Yield strength of cable
    E_cable = 210e5 # [N/mm^2] Modulus of elasticity Cable
    E_beam = 210e5 # [N/mm^2] Modulus of elasticity Beam
    I_beam = 100000 # [mm^4] Moment of inertia of the beam
    A_beam = 2000 # [mm^2] Cross-sectional area of the beam
    b0 = 0.01
    ops.uniaxialMaterial('Elastic', 1, E_cable)
    ops.uniaxialMaterial('Elastic', 2, E_beam)

    # Define truss elements for the cable
    for i in range(num_nodes - 1):
        ops.element('corotTruss', i + 1, i + 1, i + 2, A_cable, 1) # Cable element

    # Define geometric transformation
    ops.geomTransf('Linear', 1)
    # Define beam elements for the beam
    for i in range(num_nodes - 1):
        # element elasticBeamColumn $eleTag $iNode $jNode $A $E $Iz $transfTag <-mass $massDens> <-cMass>
        ops.element('elasticBeamColumn', num_nodes + i + 1, num_nodes + i + 1, num_nodes + i + 2, A_beam, E_beam, I_beam, 1) # Beam element

    # Connect each node of the cable with the corresponding node of the beam using truss elements
    for i in range(num_nodes):
        ops.element('corotTruss', 2 * num_nodes + i + 1, i + 1, num_nodes + i + 1, A_cable, 1)

    mid_node = num_nodes // 2 + 1
    # Define load pattern with displacement at the middle span
    ops.timeSeries('Linear', 1)
    ops.pattern('Plain', 1, 1)
    ops.load(mid_node, 0.0, 1) # Apply a horizontal load at node
    disp_incr = 0.01 # [mm] Incremental Displacement
    n_steps = int(np.abs(MAX_DISP / disp_incr)) # Analysis Steps

    # Define analysis parameters
    ops.system('BandGeneral')
    ops.numberer('Plain')
    ops.constraints('Plain')
    ops.integrator('DisplacementControl', mid_node, 2, disp_incr)
    ops.test('EnergyIncr', TOLERANCE, MAX_ITERATIONS)
    ops.algorithm('ModifiedNewton')
    ops.analysis('Static')

    DISPLACEMENTS = []
    DISP_X, DISP_Y = [], []
    BASEREACTION_X, BASEREACTION_Y = [], []

    # Perform the analysis with increments
    for i in range(n_steps):
        OK = ops.analyze(1)
```

```

ANALYSIS(OK, 1, TOLERANCE, MAX_ITERATIONS)
displacements = [ops.nodeDisp(j + 1) for j in range(num_nodes * 2)]
DISPLACEMENTS.append([disp[:2] for disp in displacements]) # Remove the rotational component
DISP_X.append(ops.nodeDisp(mid_node, 1))
DISP_Y.append(ops.nodeDisp(mid_node, 2))
BASE_REACTION_X.append(ops.eleResponse(1, 'force')[0])
BASE_REACTION_Y.append(ops.eleResponse(1, 'force')[1])
print(f"STEP: {i+1}")

# Get initial and final node coordinates for plotting
initial_coords = np.array([ops.nodeCoord(i + 1) for i in range(num_nodes * 2)])
deformed_coords = initial_coords + np.array(DISPLACEMENTS[-1])

# Output all unconstrained node displacements
for i in range(num_nodes * 2):
    disp = ops.nodeDisp(i + 1)
    print(f"Node {i + 1}: X Displacement = {disp[0]}, Y Displacement = {disp[1]}")

# Plot initial and deformed shape
plt.figure(figsize=(12, 8))
plt.plot(initial_coords[:num_nodes, 0], initial_coords[:num_nodes, 1], 'bo-', label='Initial Cable Shape')
plt.plot(deformed_coords[:num_nodes, 0], deformed_coords[:num_nodes, 1], 'ro-', label='Deformed Cable Shape')
plt.plot(initial_coords[num_nodes:, 0], initial_coords[num_nodes:, 1], 'go-', label='Initial Beam Shape')
plt.plot(deformed_coords[num_nodes:, 0], deformed_coords[num_nodes:, 1], 'yo-', label='Deformed Beam Shape')

# Plot connecting elements between the cable and the beam
for i in range(num_nodes):
    plt.plot([initial_coords[i, 0], initial_coords[num_nodes + i, 0]],
            [initial_coords[i, 1], initial_coords[num_nodes + i, 1]], 'c--', label='Initial Connecting Elements' if i == 0 else "")
    plt.plot([deformed_coords[i, 0], deformed_coords[num_nodes + i, 0]],
            [deformed_coords[i, 1], deformed_coords[num_nodes + i, 1]], 'm--', label='Deformed Connecting Elements' if i == 0 else "")

# Mark simply supported nodes with red triangles
support_nodes = [0, num_nodes - 1, num_nodes, BEAM_NODE - 1]
plt.plot(initial_coords[support_nodes, 0], initial_coords[support_nodes, 1], 'r^', markersize=20, label='Simply Supported Nodes')

plt.xlabel('X [mm]')
plt.ylabel('Y [mm]')
plt.legend()
plt.title('Pushover Analysis: Cable and Beam Deformed Shapes')
plt.show()

return initial_coords, deformed_coords, DISPLACEMENTS, DISP_X, DISP_Y, BASE_REACTION_X, BASE_REACTION_Y

```

```

In [ ]: # Define geometry and nodes
span = 10000.0 # [mm]
height = 5000.0 # [mm]
A_cable = 200 # [mm^2]
num_nodes = 51
MAX_DISP = -10.0 # [mm]
MAX_ITERATIONS = 5000
TOLERANCE = 1.0e-10

# Run the analysis and plot results
initial_coords, deformed_coords, DISPLACEMENTS, DISP_X, DISP_Y, BASE_REACTION_X, BASE_REACTION_Y = PUSHOVER_ANALYSIS_BEAM(span, height, A_cable, num_nodes, MAX_DISP, MAX_ITERATIONS, TOLERANCE)
plot_reactions(DISP_X, BASE_REACTION_X, DISP_Y, BASE_REACTION_Y)

```

In []:

In []: