

# Tutorial-7

Tutorial 7: Concept of Stress and Strain - II

## Problem-1

An 80-m-long wire of 5-mm diameter is made of a steel with  $E = 200$  GPa and an ultimate tensile strength of 400 MPa. If a factor of safety of 3.2 is desired, determine (a) the largest allowable tension in the wire, (b) the corresponding elongation of the wire.

### Solution

$$\sigma_U = 400 \times 10^6 \text{ Pa}$$

$$A = \frac{\pi}{4} d^2 = \frac{\pi}{4} (5)^2 = 19.635 \text{ mm}^2 = 19.635 \times 10^{-6} \text{ m}^2$$

$$P_U = \sigma_U A = (400 \times 10^6)(19.635 \times 10^{-6}) = 7854 \text{ N} \quad \Rightarrow \quad P_{\text{all}} = \frac{P_U}{F.S} = \frac{7854}{3.2} = 2454 \text{ N} = 2.45 \text{ kN} \quad \text{Ans.}$$

$$\delta = \frac{PL}{AE} = \frac{(2454)(80)}{(19.635 \times 10^{-6})(200 \times 10^9)} = 50.0 \times 10^{-3} \text{ m} \quad \text{Ans.}$$

## Problem-2

Two gauge marks are placed exactly 250 mm apart on a 12 mm diameter aluminum rod. Knowing that, with an axial load of 6000 N acting on the rod, the distance between the gage marks is 250.18 mm; determine the modulus of elasticity of the aluminum used in the rod.

### Solution

$$\delta = \Delta L = L - L_0 = 250.18 - 250.00 = 0.18 \text{ mm}$$

$$\delta = \frac{PL}{AE} = \frac{(6000)(250)}{\frac{\pi}{4}(12)^2 E} = \frac{13262.912}{E} = 0.18 \text{ mm}$$

$$\Rightarrow \text{Modulus of elasticity, } E = \frac{13262.912}{0.18} \text{ N/mm}^2 = 73.682 \text{ N/mm}^2 = 73.68 \text{ GPa}$$

Ans.

## Problem-4

A square yellow-brass bar must not stretch more than 2.5 mm when it is subjected to a tensile load. Knowing that  $E = 105$  GPa and that the allowable tensile strength is 180 MPa, determine (a) the maximum allowable length of the bar, (b) the required dimensions of the cross section if the tensile load is 40 kN.

### Solution

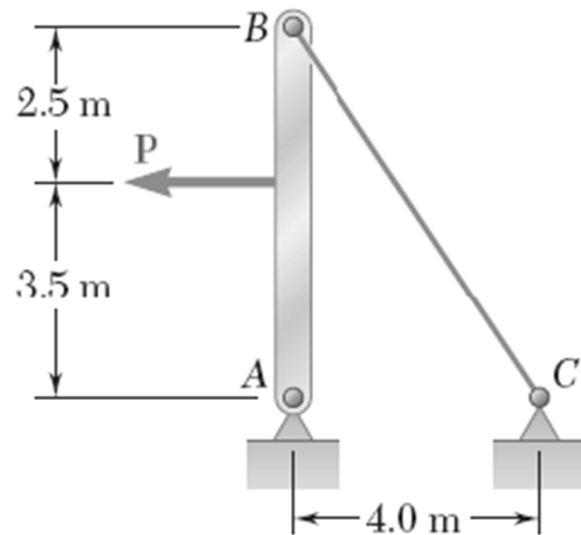
$$\text{We know } \delta = \frac{PL}{AE} = \frac{\sigma L}{E} \quad \Rightarrow L_{allow} = \frac{\delta E}{\sigma_{allow}} = \frac{(2.5)(105 \times 10^3)}{180} = 1458 \text{ mm} = 1.458 \text{ m}$$

Ans.

$$\text{Required cross section area} = \frac{\text{Tensile load}}{\text{Allowable tensile strength}} = \frac{40000}{180} \text{ mm}^2 = 222.22 \text{ mm}^2.$$

$$\therefore \text{Side of the square section} = \sqrt{222.22} \text{ mm} = 14.91 \text{ mm} \quad \text{Ans.}$$

## Problem-5



The 4-mm-diameter cable BC is made of a steel with  $E = 200$  GPa. Knowing that the maximum stress in the cable must not exceed 190 MPa and that the elongation of the cable must not exceed 6 mm, find the maximum load  $P$  that can be applied as shown.

### Solution

$$L_{BC} = \sqrt{6^2 + 4^2} = 7.2111 \text{ m}$$

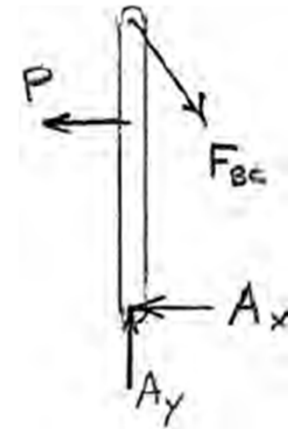
$$\text{At equilibrium of the bar AB: } \Sigma M_A = 0: \quad 3.5P - (6) \left( \frac{4}{7.2111} F_{BC} \right) = 0$$

$$\Rightarrow P = 0.9509 F_{BC}$$

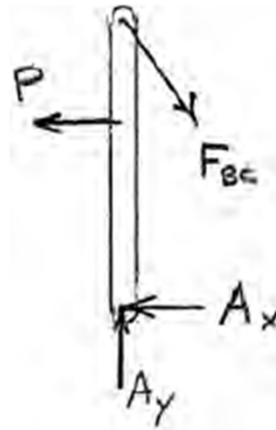
$$A = \frac{\pi}{4} d^2 = \frac{\pi}{4} (0.004)^2 = 12.566 \times 10^{-6} \text{ m}^2$$

$$\text{Since, } \sigma = \frac{F_{BC}}{A} \quad \therefore F_{BC} = \sigma A = (190 \times 10^6)(12.566 \times 10^{-6}) = 2.388 \times 10^3 \text{ N}$$

$$\text{Since } \delta = \frac{F_{BC} L_{BC}}{AE} \quad \therefore F_{BC} = \frac{AE \delta}{L_{BC}} = \frac{(12.566 \times 10^{-6})(200 \times 10^9)(6 \times 10^{-3})}{7.2111} = 2.091 \times 10^3 \text{ N}$$



FBD of bar AB



FBD of bar AB

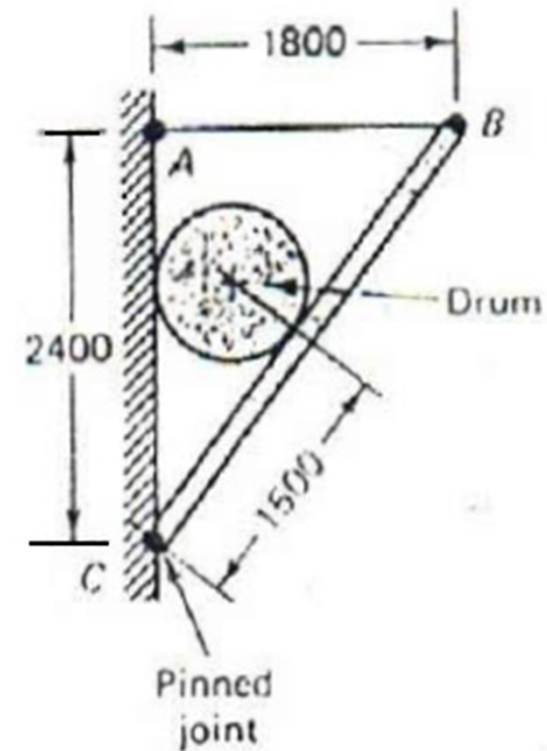
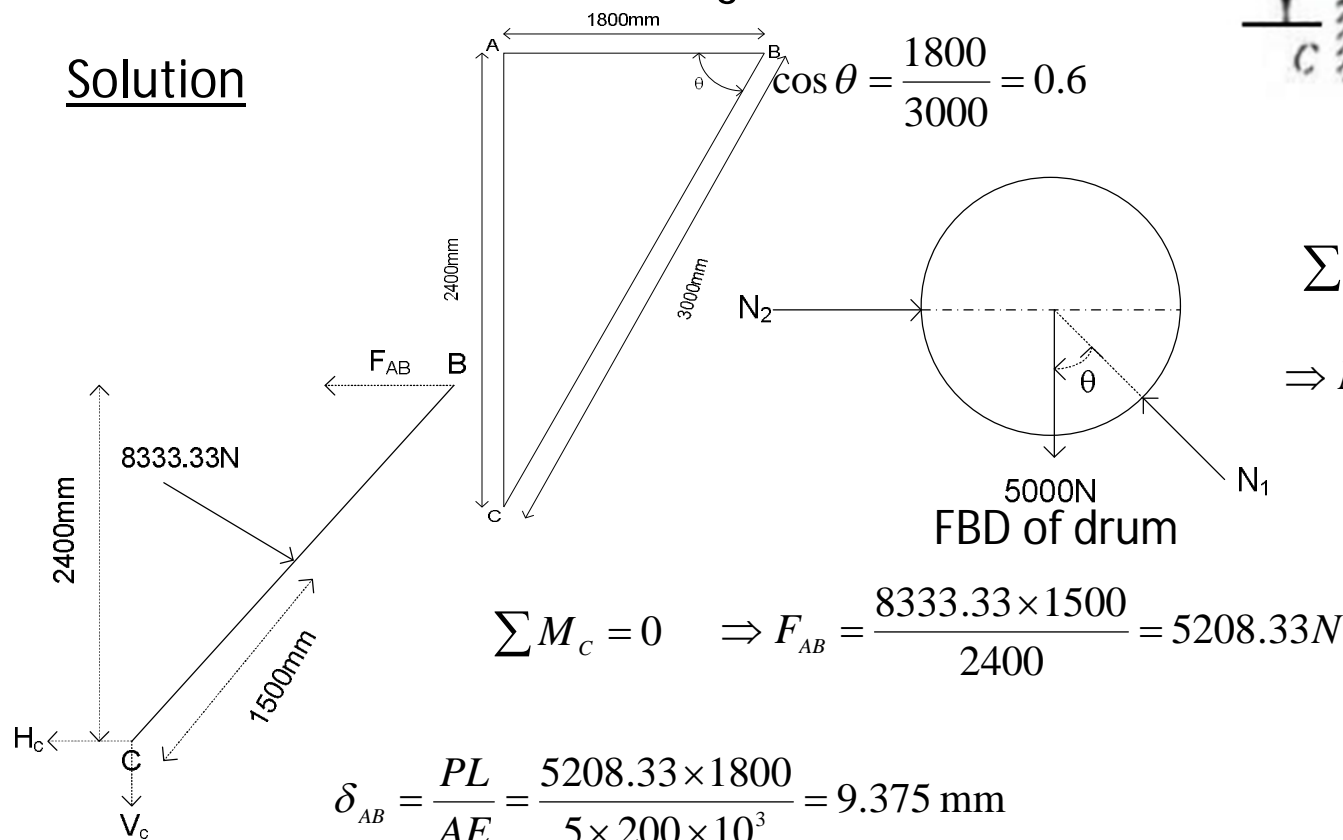
Therefore, maximum tensile load which the cable BC can sustain is :  $F_{BC} = 2.091 \times 10^3 \text{ N}$

$$\therefore P = 0.9509 F_{BC} = (0.9509)(2.091 \times 10^3) = 1.988 \times 10^3 \text{ N} \quad \text{Ans.}$$

## Problem-6

All the joints of the wall bracket may be considered as pin connected. Steel rod AB ( $E = 200 \text{ GPa}$ ) has a cross sectional area of  $5 \text{ mm}^2$  and the member BC is a rigid beam. If a  $1000 \text{ mm}$  diameter frictionless drum of weight  $5000 \text{ N}$  is placed in the position shown, what will be the elongation of rod AB?

### Solution



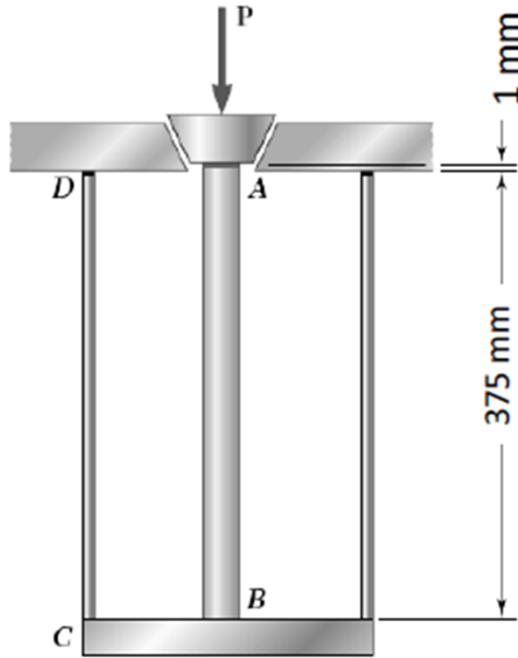
$$\sum F_y = 0$$

$$\Rightarrow N_1 = \frac{5000}{\cos \theta} = \frac{5000}{0.6} = 8333.33 \text{ N}$$

$$\sum M_C = 0 \Rightarrow F_{AB} = \frac{8333.33 \times 1500}{2400} = 5208.33 \text{ N}$$

$$\delta_{AB} = \frac{PL}{AE} = \frac{5208.33 \times 1800}{5 \times 200 \times 10^3} = 9.375 \text{ mm}$$

## Problem-7



The brass tube AB ( $E = 105 \text{ GPa}$ ) has a cross-sectional area of  $140 \text{ mm}^2$  and is fitted with a plug at A. The tube is attached at B to a rigid plate that is itself attached at C to the bottom of an aluminum cylinder ( $E = 72 \text{ GPa}$ ) with a cross-sectional area of  $250 \text{ mm}^2$ . The cylinder is then hung from a support at D. In order to close the cylinder, the plug must move down through  $1 \text{ mm}$ . Determine the force  $P$  that must be applied to the tube.

### Solution

Shortening of brass tube AB

$$L_{AB} = 375 + 1 = 376 \text{ mm} = 0.376 \text{ m} \quad A_{AB} = 140 \text{ mm}^2 = 140 \times 10^{-6} \text{ m}^2 \quad E_{AB} = 105 \times 10^9 \text{ Pa}$$

$$\delta_{AB} = \frac{PL_{AB}}{E_{AB}A_{AB}} = \frac{P(0.376)}{(105 \times 10^9)(140 \times 10^{-6})} = 25.578 \times 10^{-9} P$$

Elongation of Aluminum cylinder CD

$$L_{CD} = 0.375 \text{ m} \quad A_{CD} = 250 \text{ mm}^2 = 250 \times 10^{-6} \text{ m}^2 \quad E_{CD} = 72 \times 10^9 \text{ Pa}$$

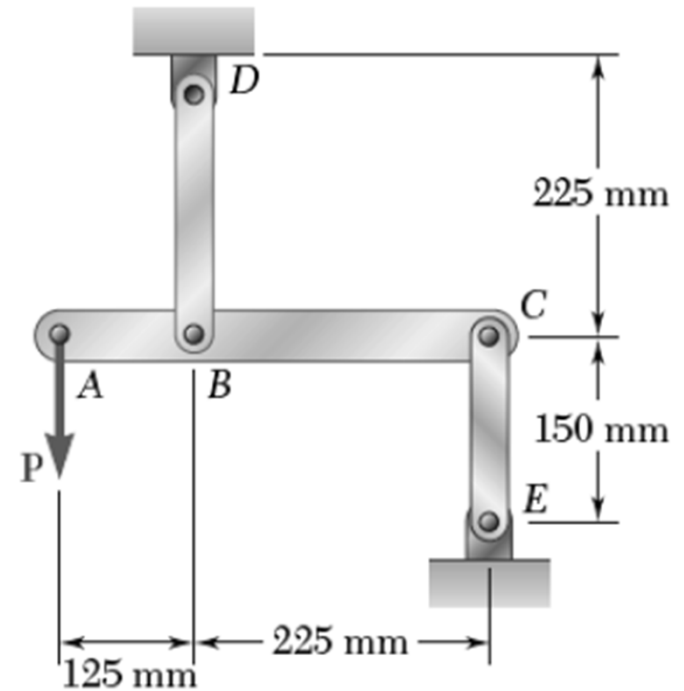
$$\delta_{CD} = \frac{PL_{CD}}{E_{CD}A_{CD}} = \frac{P(0.375)}{(72 \times 10^9)(250 \times 10^{-6})} = 20.833 \times 10^{-9} P$$

$$\text{Total deflection:} \quad \delta_A = \delta_{AB} + \delta_{CD} \text{ where } \delta_A = 0.001 \text{ m} \quad \Rightarrow P = 21.547 \times 10^3 \text{ N}$$

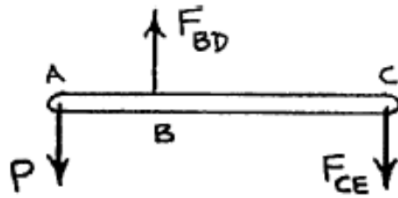


## Problem-10

Link BD is made of brass ( $E = 105 \text{ GPa}$ ) and has a cross-sectional area of  $240 \text{ mm}^2$ . Link CE is made of aluminum ( $E = 72 \text{ GPa}$ ) and has a cross-sectional area of  $300 \text{ mm}^2$ . Knowing that they support rigid member ABC, determine the maximum force  $P$  that can be applied vertically at point A if the deflection of A is not to exceed  $0.35 \text{ mm}$ .



## Solution



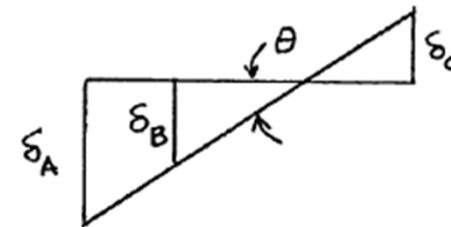
FBD of member ABC

$$\begin{aligned}\sum M_C &= 0 & \Rightarrow 0.350P - 0.225F_{BD} &= 0 \\ \Rightarrow F_{BD} &= 1.55556P\end{aligned}$$

$$\begin{aligned}\text{Again, } \sum M_B &= 0 & \Rightarrow 0.125P - 0.225F_{CE} &= 0 \\ \Rightarrow F_{CE} &= 0.55556P\end{aligned}$$

$$\delta_B = \delta_{BD} = \frac{F_{BD} L_{BD}}{E_{BD} A_{BD}} = \frac{(1.55556P)(0.225)}{(105 \times 10^9)(240 \times 10^{-6})} = 13.8889 \times 10^{-9} P$$

$$\delta_C = \delta_{CE} = \frac{F_{CE} L_{CE}}{E_{CE} A_{CE}} = \frac{(0.55556P)(0.150)}{(72 \times 10^9)(300 \times 10^{-6})} = 3.8581 \times 10^{-9} P$$



Deformation Diagram

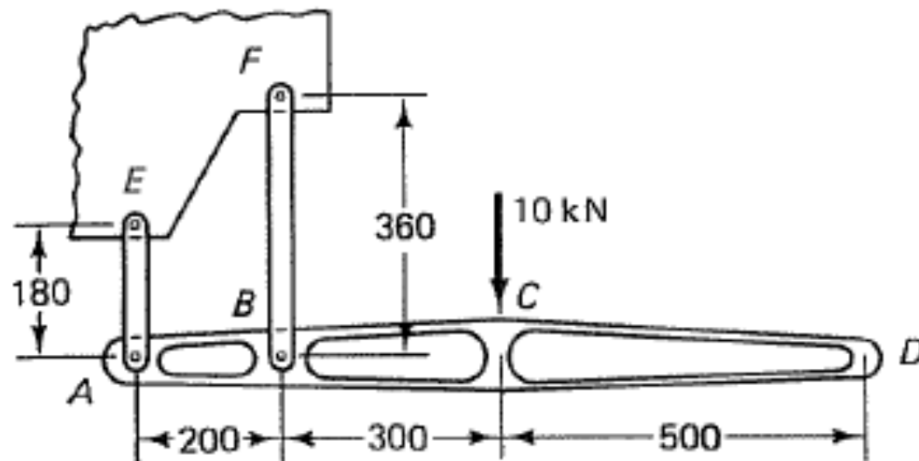
$$\text{Slope, } \theta = \frac{\delta_B + \delta_C}{L_{BC}} = \frac{17.7470 \times 10^{-9} P}{0.225} = 78.876 \times 10^{-9} P$$

$$\begin{aligned}\delta_A &= \delta_B + L_{AB}\theta \\ &= 13.8889 \times 10^{-9} P + (0.125)(78.876 \times 10^{-9} P) \\ &= 23.748 \times 10^{-9} P\end{aligned}$$

$$\text{Since, } \delta_A = 0.35 \text{ mm} = 0.35 \times 10^{-3} \text{ m} \quad \delta_A = 0.35 \times 10^{-3} \text{ m} = 23.748 \times 10^{-9} P \quad \Rightarrow P = 14.7381 \times 10^3 \text{ N}$$

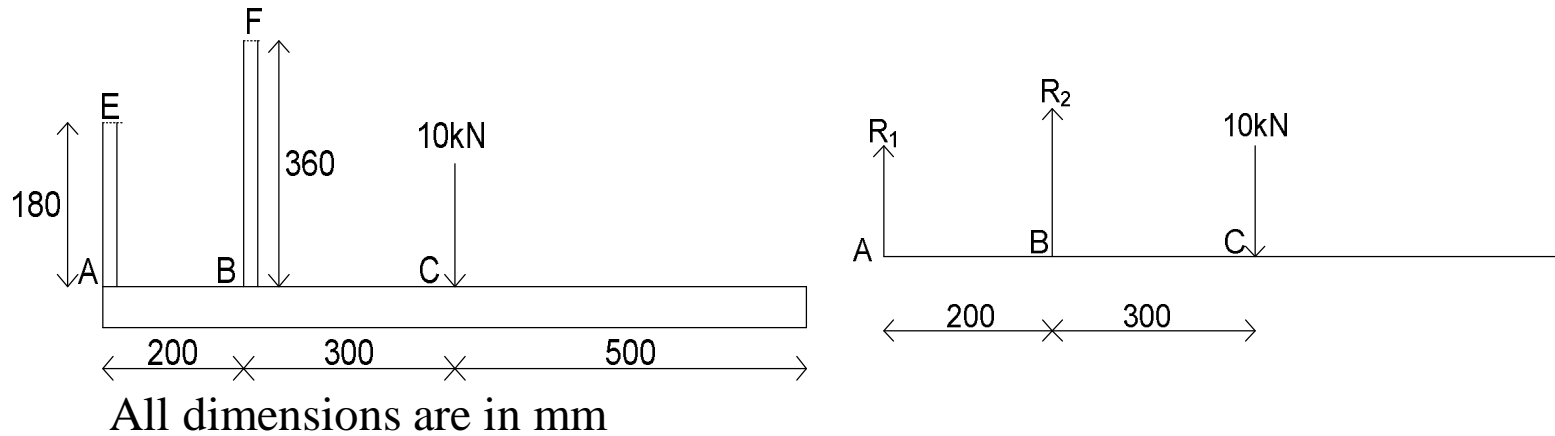
Ans.

## Problem-11



A rigid machine part AD is suspended by double hangers AE of cross sectional area of  $50 \text{ mm}^2$  each and BF of cross sectional area of  $100 \text{ mm}^2$  each respectively. The elastic modulus of hanger material is  $180 \text{ GPa}$  and yield stress is  $600 \text{ MPa}$ . Determine the deflection that would occur at D by applying a downward force of  $10 \text{ kN}$  at C. Check hanger stress to assure that an elastic solution is applicable.

## Solution



$$\sum F_y = 0 \quad R_1 + R_2 = 10 \times 10^3 \text{ N}$$

$$\sum M_A = 0 \quad R_2 \times 200 = 10 \times 10^3 \times 500 \quad \Rightarrow \quad R_2 = \frac{10 \times 10^3 \times 500}{200} = 25 \times 10^3 \text{ N}$$

$$R_1 = (10 - 25) \times 10^3 = -15 \times 10^3 \text{ N}$$

$$\delta_1 = \frac{PL}{AE} = \frac{-15 \times 10^3 \times 180}{(50/2) \times 180 \times 10^3} = -0.15 \text{ mm} \quad \text{Therefore} \quad \varepsilon_1 = \frac{\delta_1}{L} = \frac{0.15}{180} = 8.33 \times 10^{-4}$$

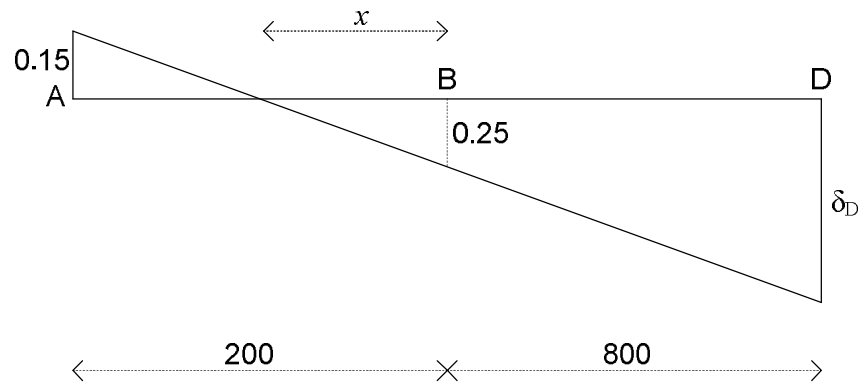
$$\text{Hence, } \sigma_1 = \varepsilon_1 E = 8.33 \times 10^{-4} \times 180 \times 10^3 = 150 \text{ MPa} < 600 \text{ MPa}$$

$$\delta_2 = \frac{PL}{AE} = \frac{25 \times 10^3 \times 360}{(100/2) \times 180 \times 10^3} = 0.25 \text{ mm} \quad \varepsilon_2 = \frac{\delta_2}{L} = \frac{0.25}{360} = 6.94 \times 10^{-4}$$

$$\text{Hence, } \sigma_2 = \varepsilon_2 E = 6.94 \times 10^{-4} \times 180 \times 10^3 = 125 \text{ MPa} < 600 \text{ MPa}$$

$\Rightarrow$  Elastic condition is assured

## Solution Cont...



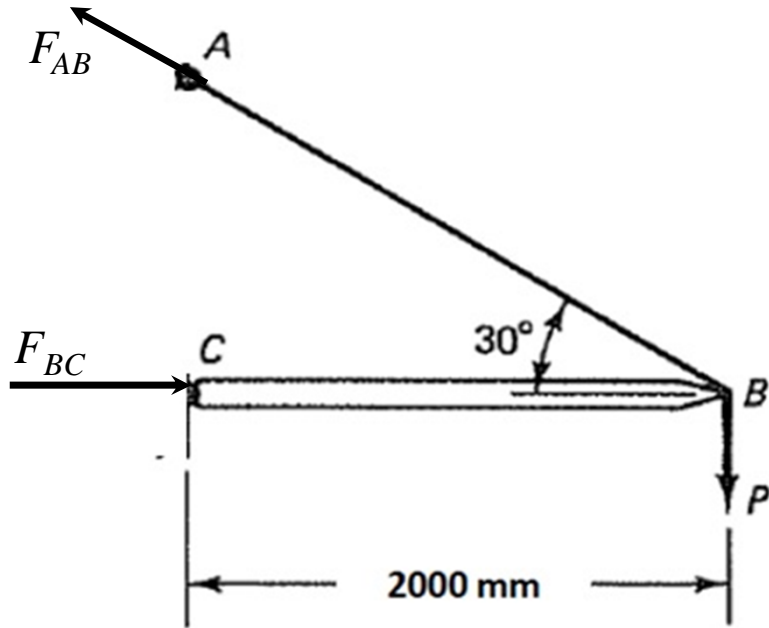
All dimensions are in mm

$$\frac{x}{0.25} = \frac{200 - x}{0.15}$$

$$\Rightarrow x = \frac{50}{0.4} = 125 \text{ mm}$$

$$\frac{125}{0.25} = \frac{800 + 125}{\delta_D}$$

$$\Rightarrow \delta_D = \frac{231.25}{125} = 1.85 \text{ mm Ans.}$$



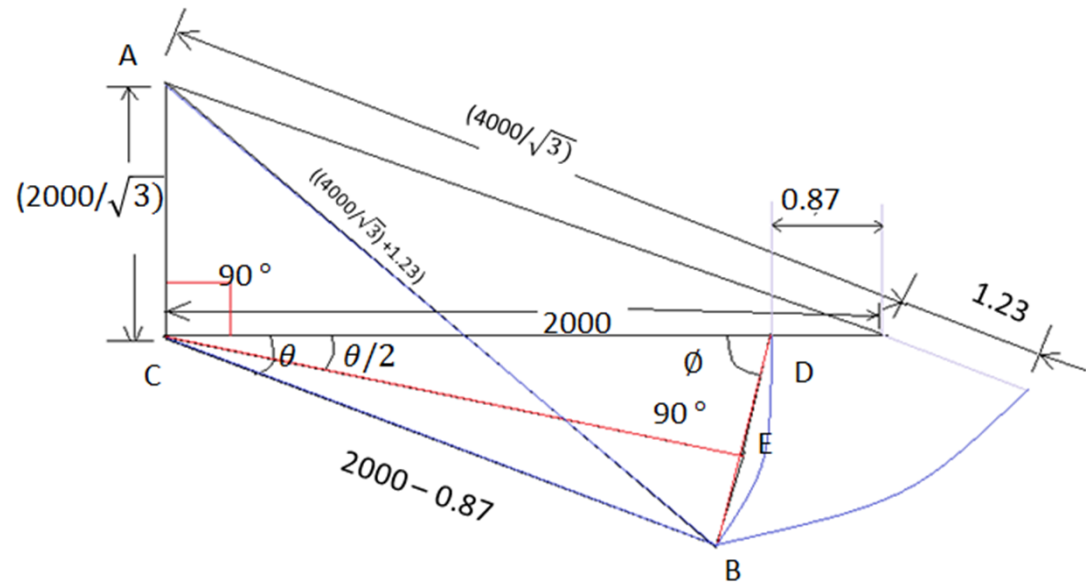
FBD of member ABC

Solution

$$F_{AB} = 32 \text{ kN} \quad F_{BC} = 32 \text{ kN}$$

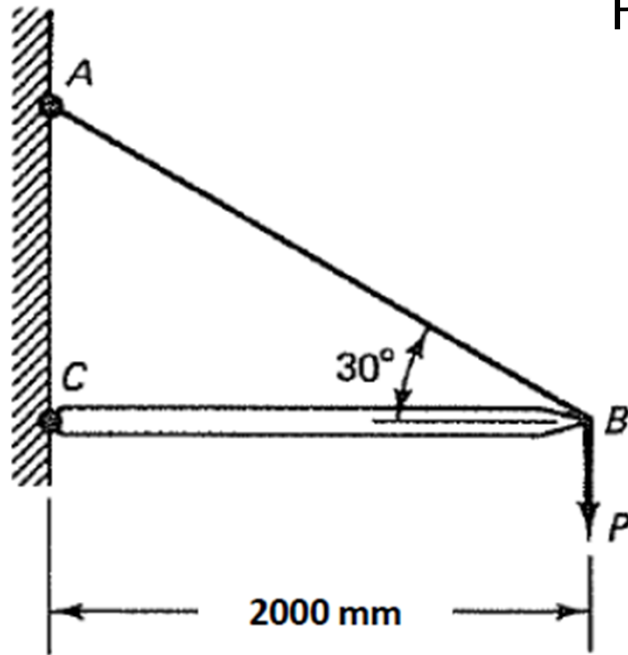
$$\Delta_{AB} = 1.23 \text{ mm}$$

$$\Delta_{BC} = -0.87 \text{ mm}$$

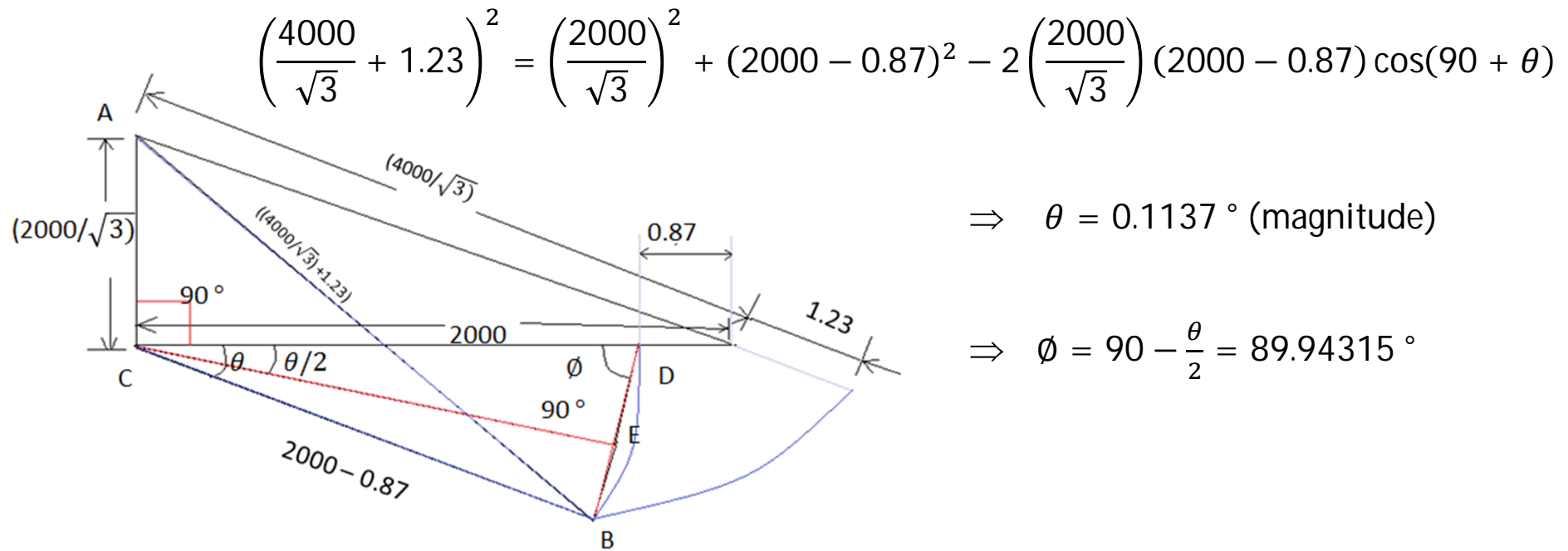


Deformed shape of member ABC

## Problem-12



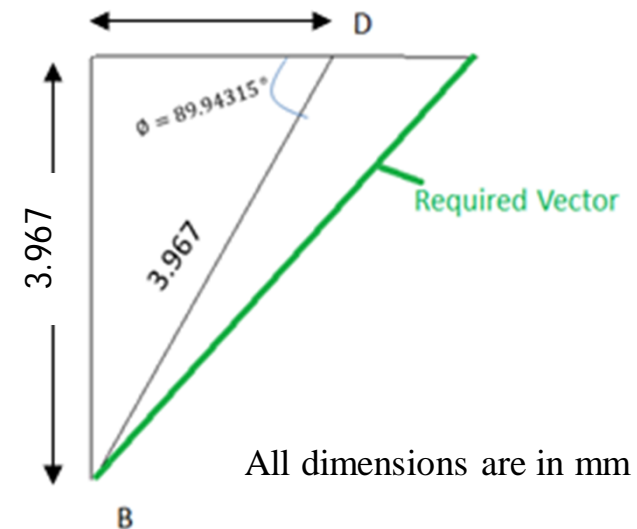
The jib crane shown in the figure has the cable AB of cross-sectional area of  $300 \text{ mm}^2$  and the bar BC of cross-sectional area of  $320 \text{ mm}^2$ . (a) Determine the deflection vector at B caused by the application of a force  $P = 16 \text{ kN}$ . (b) Hence, estimate the vertical stiffness of the crane at point B. Take  $E = 200 \text{ GPa}$ .



$\therefore DE = (2000 - 0.87) \cos \phi = 1.9836 \text{ mm}$  (E is the middle point of DB)

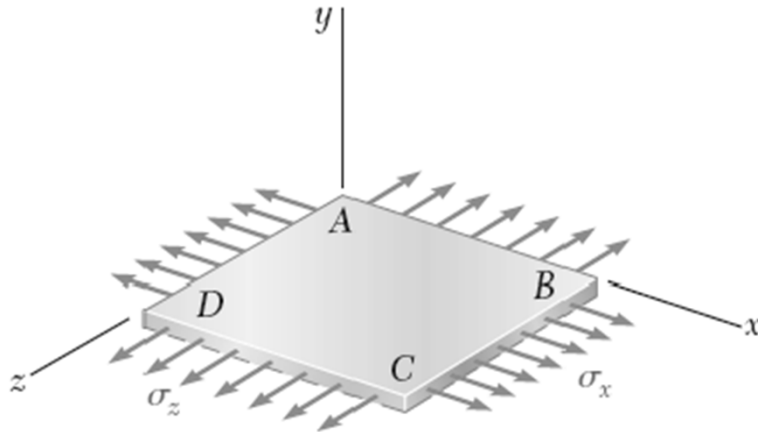
$$DB = (2)(1.9836) = 3.967 \text{ mm}$$

Hence, displacement vector =  $(-0.8739i - 3.967j) \text{ mm}$





## Problem-15



The homogeneous plate ABCD is subjected to a biaxial loading as shown. It is known that  $\sigma_z = k$  and that the change in length of the plate in the  $x$  direction must be zero, that is,  $\epsilon_x = 0$ . If the modulus of elasticity is  $E$  and Poisson's ratio is  $\nu$ , determine (a) the required magnitude of  $\sigma_x$ , (b) the ratio  $k/\epsilon_z$ .

### Solution

$$\sigma_z = \sigma_0, \quad \sigma_y = 0, \quad \epsilon_x = 0$$

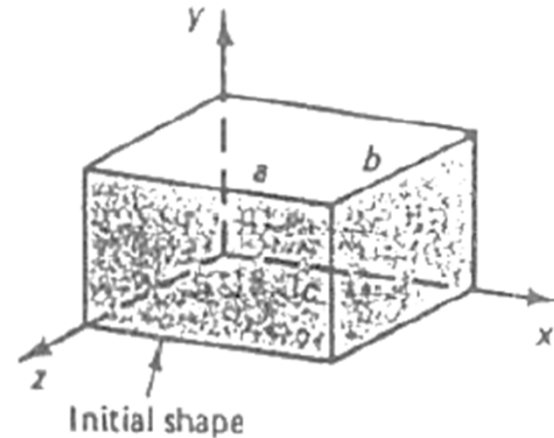
$$\text{since } \epsilon_x = \frac{1}{E}(\sigma_x - \nu\sigma_y - \nu\sigma_z) = \frac{1}{E}(\sigma_x - \nu\sigma_0) = 0 \quad \Rightarrow \quad \sigma_x = \nu\sigma_0$$

$$\text{since } \epsilon_z = \frac{1}{E}(-\nu\sigma_x - \nu\sigma_y + \sigma_z) \quad \therefore \quad \epsilon_z = \frac{1}{E}(-\nu^2\sigma_0 - 0 + \sigma_0) = \frac{1-\nu^2}{E}\sigma_0$$

$$\Rightarrow \quad \frac{k}{\epsilon_z} = \frac{\sigma_0}{\epsilon_z} = \frac{E}{1-\nu^2}$$

## Problem-16

A rectangular steel block has the following dimensions:  $a = 50$  mm,  $b = 75$  mm and  $c = 100$  mm. The faces of this block are subjected to uniformly distributed forces of 180 kN (tension) in the  $x$ -direction, 200 kN (tension) in the  $y$ -direction and 240 kN (compression) in the  $z$ -direction. Determine the magnitude of a single system of forces acting only in the  $y$ -direction that would cause the same deformation in the  $y$ -direction as the initial forces. Consider  $\nu = 0.25$ .



### Solution

$F_x = +180$  kN (tension); Area ( $A_x$ ) =  $b \times c = 75 \times 100$  mm

$F_y = +200$  kN (tension); Area ( $A_y$ ) =  $a \times b = 50 \times 75$  mm

$F_z = -240$  kN (compression); Area ( $A_z$ ) =  $a \times c = 50 \times 100$  mm

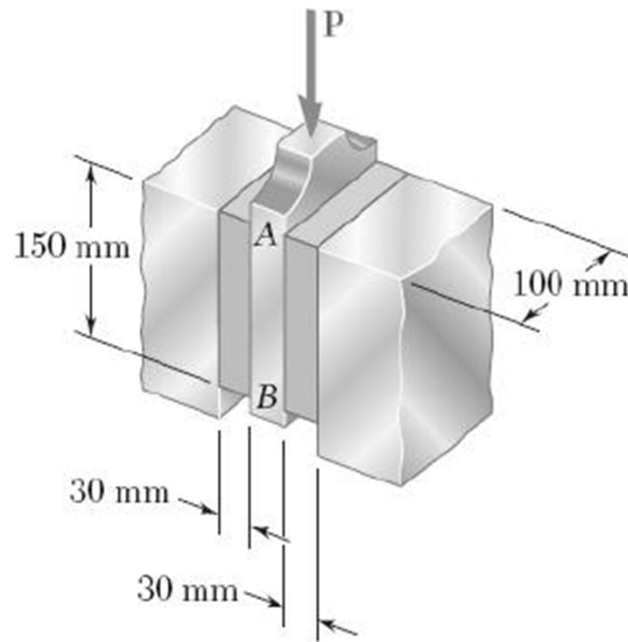
$$\sigma_x = \frac{F_x}{A_x}; \sigma_y = \frac{F_y}{A_y}; \sigma_z = \frac{F_z}{A_z}$$

$$\varepsilon_y = \frac{1}{E} [\sigma_y - \nu(\sigma_x + \sigma_z)] = \frac{1}{E} \left[ \frac{200 \times 10^3}{50 \times 75} - 0.25 \left( \frac{180 \times 10^3}{75 \times 100} - \frac{240 \times 10^3}{50 \times 100} \right) \right] = \frac{1}{E} [53.33 + 6] = \frac{59.33}{E}$$

For Equivalent force system,  $\varepsilon'_y = \frac{1}{E} \frac{F'_y}{A_y} = \varepsilon_y = \frac{1}{E} \frac{F'_y}{50 \times 75} = \frac{59.33}{E}$  Therefore,  $\frac{1}{E} \frac{F'_y}{50 \times 75} = \frac{59.33}{E}$

$$\Rightarrow F'_y = 222.49 \times 10^3 \text{ N} = 222.49 \text{ kN}$$

## Problem-17



A vibration isolation unit consists of two blocks of rubber bonded to a rigid metal plate AB and to rigid supports as shown. Knowing that a force of magnitude  $P = 25$  kN causes a deflection  $d = 1.5$  mm of plate AB in the downward direction, determine the modulus of rigidity of the rubber used.

### Solution

Given:  $P = 25 \text{ kN} = 25 \times 10^3 \text{ N}$       $\delta = 1.5 \text{ mm}$

Area =  $(150)(100) \text{ mm}^2$

Shear strain =  $\gamma = \frac{\delta}{h}$      Shear stress =  $\tau = G\gamma = G \frac{\delta}{h}$

Total force =  $\frac{P}{2} = \text{Area} \times \tau = \text{Area} \times G\gamma = \text{Area} \times G \frac{\delta}{h}$

Or,  $\frac{(25)(10^3)}{2} = (150)(100)G \frac{1.5}{30}$

Therefore,  $G = \frac{25 \times 10^3 \times 30}{2 \times 150 \times 100 \times 1.5} \text{ N/mm}^2 = 16.67 \text{ MPa}$

