

TOS

Design a Cantilever retaining Wall retain on soil upto 4m above ground level the soil density is 16 kN/m^3 angle of repose is 30° coefficient of friction is 0.6 Soil bearing Capacity is 150 kN/m^2 if grade of concrete as m25 and grade of steel as Fe415 draw detail diagram showing reinforcement details.

Data m25 $\Rightarrow f_{ck} = 8.5$, Fe415 $\Rightarrow f_y = 230 \text{ N/mm}^2$ $\phi = 30$
 Solution: Step 1

$$\rightarrow K_a = \frac{1 - \sin \phi}{1 + \sin \phi} = \frac{1 - \sin 30}{1 + \sin 30} = 0.33 \text{ or } 1/3$$

$$\rightarrow DF = \frac{SBC}{\gamma_s} \times K_a^2 = \frac{150}{16} \times \left(\frac{1}{3}\right)^2 = 1.04 \text{ or } 1.5 \text{ m}$$

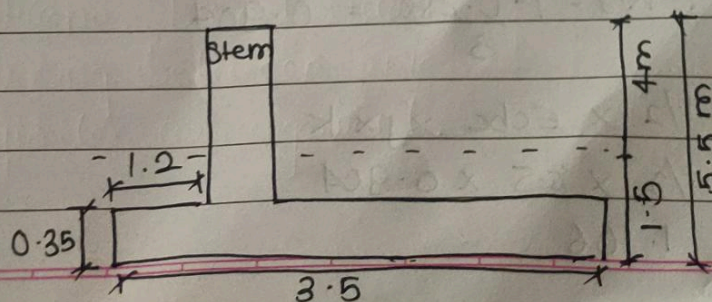
$$\rightarrow \text{Total } h_t = H_{\text{soil}} + DF = 4 + 1.5 = 5.5 \text{ m}$$

Step 2 preliminary Sizing

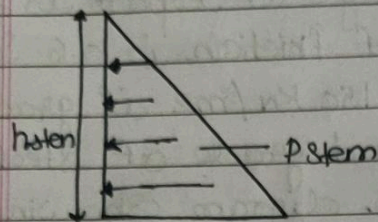
$$\begin{aligned} \text{Width of Base Slab (B)} &= 0.6(H) \text{ to } 0.7(H) \\ &= 5.5 \times \frac{0.6}{100} \text{ to } 5.5 \times \frac{0.7}{100} \\ &= 3.85 \text{ to } 3.85 \text{ to } 3.85 = 3.5 \end{aligned}$$

$$\rightarrow \text{THK of Base Slab} = B/10 = 0.35$$

$$\rightarrow \text{Tae width} = 1/3 \times B = 1/3 \times 3.5 = 1.2 \text{ m}$$



→ Calculation of lateral pressure and Bending moment



$$\begin{aligned}
 h_{\text{stem}} &= H - \text{thk of base slab} \\
 &= 6.8 - 0.35 \\
 &= 5.15
 \end{aligned}$$

$$\begin{aligned}
 \rightarrow p &= \frac{1}{2} \times \text{Base} \times \text{ht} - \frac{1}{2} \times (K_a \times \gamma_s \times h_{\text{stem}}) \times h_{\text{stem}} \\
 &= \frac{1}{2} \times (27.46) \times 5.15 \\
 &= 70.725 \text{ KN}
 \end{aligned}$$

$$\begin{aligned}
 \rightarrow \text{Bm of Stem} &= \text{Force} \times \text{1/2 distance} = p \times \frac{h_{\text{stem}}}{3} \\
 &= 70.725 \times \frac{5.15}{3} = 121.41
 \end{aligned}$$

$$\rightarrow \sigma_{cbc} = 8.5 \text{ N/mm}^2, \sigma_{st} = 230 \text{ N/mm}^2$$

Step 3 Finding the Constants.

$$\rightarrow m = \frac{280}{3\sigma_{cbc}} = \frac{280}{3 \times 8.5} = 10.98$$

$$\begin{aligned}
 k &= \frac{1}{1 + \frac{\sigma_{ck}}{m \cdot \sigma_{cbc}}} = \frac{1}{1 + \frac{230}{10.98 \times 8.5}} = 0.285
 \end{aligned}$$

$$j = 1 - \frac{k}{3} = 1 - \frac{0.285}{3} = 0.904$$

$$\begin{aligned}
 Q &= \frac{1}{2} \times \sigma_{cbc} \times j \times k \\
 &= \frac{1}{2} \times 8.5 \times 0.904 \\
 &= 1.106
 \end{aligned}$$

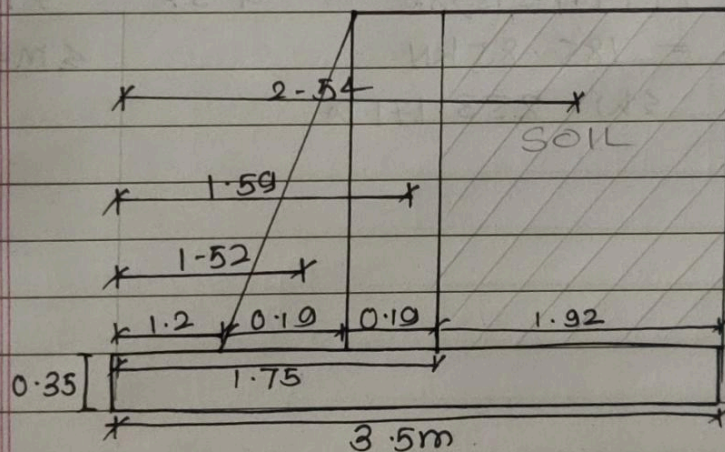
Step 4 Calculation of Stem Depth (thickness)

$$d = \sqrt{\frac{Bm}{\phi \times b}} = \sqrt{\frac{121.4 \times 10^3}{1.106 \times 1.06}} = 331.4 \text{ mm}$$

$$D = d + \text{clear cover (50mm)} = 381.4 \approx 380 \text{ mm}$$

$$B = 3.5 \text{ m, thk of base slab} = 0.35$$

$$\text{Thk of top width} = 1.2 \text{ m}$$



$$\text{Step 5. Centroids (w3)} = 1.2 + 2\left(\frac{0.19}{3}\right) = 1.2 + 2\left(\frac{0.19}{3}\right) = 0.132$$

$$\text{Centroids for other side} = 1.2 + 0.19 + \frac{0.19}{2} = 1.485 \approx 1.59$$

$$\text{Centroids of Soil} = 1.2 + 0.19 + 0.19 = \frac{1.9^2}{2} = 2.54$$

Step 6 Stability check

Components Weight (KN) CF Bricks moments

- Base slab $\rightarrow (3.5 \times 0.35) \times 25 \quad 1.58 \quad 48.3$
 $= 30.625 \text{ KN}$

- Stem Foot $\rightarrow (0.19 \times 5.16) \times 25 \quad 1.59 \quad 38.8$
 $= 24.465 \text{ KN}$

- Stem Root A $\rightarrow (1/2 \times 0.19 \times 5.15) \times 25 \quad 1.32 \quad 18.58$
 $= 12.23 \text{ KN}$

- Soil $\rightarrow (1.92 \times 5.15) \times 16^{(30)} \quad 2.54 \quad 472.059$
 $= 185.85 \text{ KN}$
 $\leq m = 575$
 $\leq W = 253.171 \text{ W}$