

Linear stability analysis for *monami* in a submerged sea grass bed

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(Received xx; revised xx; accepted xx)

The onset of *monami* – the synchronous waving of sea grass beds driven by a steady flow – is modeled as a linear instability of the flow. Unlike previous works, our model considers the drag exerted by the grass in establishing the steady flow profile, and in damping out perturbations to it. We find two distinct modes of instability, which we label Mode 1 and Mode 2. Mode 1 is closely related to Kelvin-Helmholtz instability modified by vegetation drag, whereas Mode 2 is unrelated to Kelvin-Helmholtz and arises from an interaction between the flow in the vegetated and unvegetated layers. The vegetation damping, according to our model, leads to a finite threshold flow for both these modes. Experimental observations for the onset and frequency of waving compare well with model predictions for the instability onset criteria and the imaginary part of the complex growth rate respectively, but experiments lie in a parameter regime where the two modes can not be distinguished.

1. Introduction

Sea grasses exhibit a rich set of dynamical behaviors due to their collective interaction with fluid flows. The hydrodynamic processes resulting from this behavior influence many environmental processes such as sedimentation, transport of dissolved oxygen, plant growth, and biomass production (Fonseca & Kenworthy 1987; Grizzle *et al.* 1996; Nepf 1999, 2012). One response of the submerged grass beds to steady currents is the formation of coherent large amplitude oscillations, known as *monami* (Ackerman & Okubo 1993). In this article, we present a linear hydrodynamic instability underlying the onset of these coherent oscillations.

Current explanations of *monami* (Ikeda & Kanazawa 1996; Ghisalberti & Nepf 2002; Raupach *et al.* 1996) invoke the existence of a shear layer at the top of the grass bed (henceforth called grass top) due to vegetation drag. Its instability, through a mechanism similar to the Kelvin-Helmholtz (KH) instability, is thought to lead to coherent eddies over the grass bed. The grass responds to these eddies by deforming, which leads to large amplitude synchronous oscillations of the grass blades. This picture can be used to derive a simple scaling dependence of the *monami* frequency on the flow speed and the shear layer thickness, and understand transport in the seagrass bed (Nepf & Vivoni 2000; Ghisalberti & Nepf 2002, 2004; Okamoto *et al.* 2012).

However, several aspects of this explanation remain unsatisfactory. The instability is modeled using the inviscid Rayleigh equation and despite the role of drag in producing the shear layer, its role in damping the perturbations is ignored (Raupach *et al.* 1996).

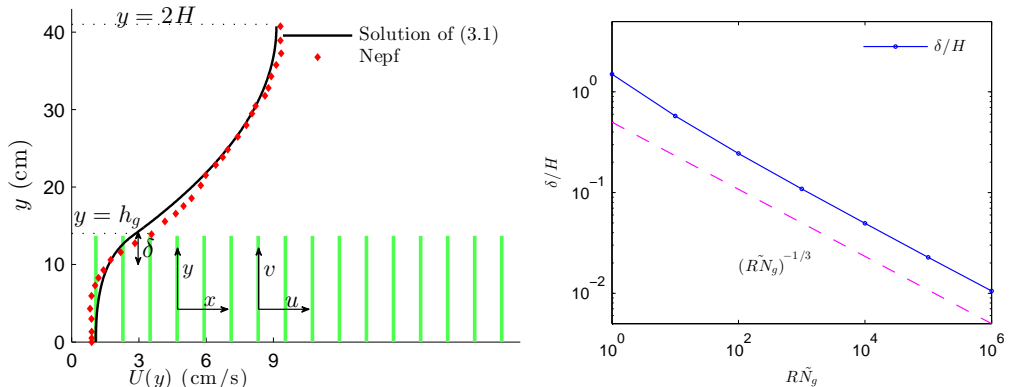


FIGURE 1. Schematic setup and comparison of our steady flow profile with that from the experiments (left) in ref. Ghisalberti & Nepf (2004) (Case I from Table 1) and its approximation with $U_0 = 7.28$ cm/s and $\delta = 5.02$ cm in our model. The grass extends up to $y = h_g$ in the water column of depth $2H$. The steady velocity profile can be decomposed into a parabolic profile in the unvegetated region, a uniform profile deep within the vegetation, and a boundary layer of thickness δ near the grass top. The dependence of the boundary layer thickness (estimated as $|U/U_y|$ at $y = h_g$ from the numerical solution of (3.1)) on the vegetation density parameter $R\tilde{N}_g$ is shown in the figure on right.

Furthermore, the shear layer is assumed not to be influenced by the top and bottom boundary of the domain, although the experimentally measured thickness of the shear layer is in many cases comparable to the unvegetated water depth. The velocity profile of the free shear layer is assumed *ad hoc* to be piecewise linear (Py *et al.* 2006) or hyperbolic tangent (Ghisalberti & Nepf 2002; Raupach *et al.* 1996), with parameters fitted using the experimental observations. The origin of these profiles, the values of the fitted parameters, and their effect on *monami* remains unexplained. And finally, no existing theory explains the threshold flow speed, observed in the lab (Ghisalberti & Nepf 2002) and the field (Grizzle *et al.* 1996), below which *monami* is not observed.

Here we present a mathematical model for the linear instability that accounts for these effects. Although *monami* is manifested in the motion of the grass, the drag exerted by the vegetation on the flow is central to the hypothesized instability. The instability and the resulting flow structures persist in lab experiments even when flexible grass mimics are replaced by rigid dowels (Ghisalberti & Nepf 2002, 2006). Therefore, to develop the essential mathematical model, we assume the grass blades to be rigid, and oriented vertically (along y direction) on average. The vegetation is modeled as a continuum drag on the fluid acting perpendicular to the blade orientation and proportional to the grass density. In the limit of dense vegetation, the steady profile established in the presence of the drag exhibits a localized region of enhanced shear gradient near the grass top and drives a flow instability. While some features of instability are similar to KH, we also find significant differences. This comparison is the focus of this article.

2. Mathematical model

The vegetation is assumed sufficiently dense so that the drag exerted by it may be modeled by a continuous body force \mathbf{f} entering the equation governing the flow as.

$$\nabla \cdot \mathbf{u} = 0, \quad \rho(\mathbf{u}_t + \mathbf{u} \cdot \nabla \mathbf{u}) = -\nabla p + \mu \nabla^2 \mathbf{u} + \mathbf{f}, \quad (2.1)$$

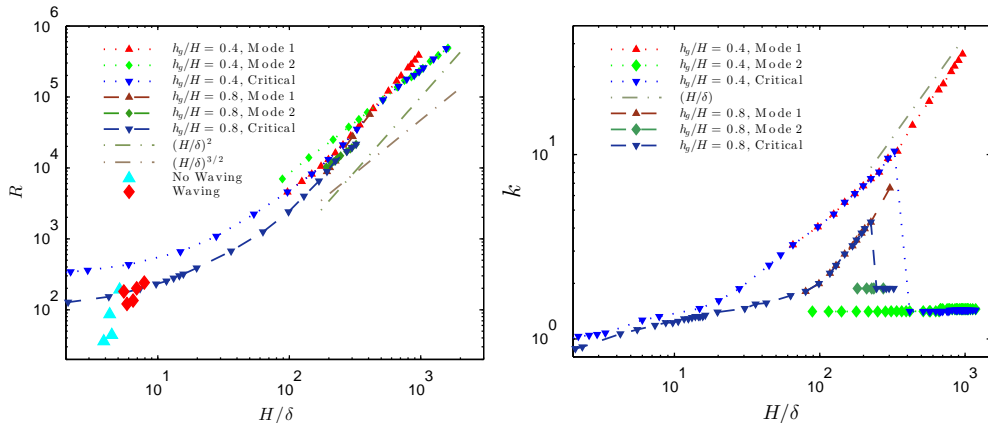


FIGURE 2. Critical Reynolds number, threshold Reynolds number for Mode 1 and Mode 2 (left) and the corresponding marginally stable wave number (right) for different submergence ratio as a function of vegetation density parametrized by the boundary layer thickness. Parameters from experiments reported by Ghisalberti & Nepf (2002) to exhibit or suppress synchronous waving are also included in the left panel.

where ρ , $\mathbf{u} = (u, v)$, and p are the fluid density, velocity, and the dynamic pressure respectively, and μ is the (dynamic) eddy viscosity. The Reynolds number of the flow based on the scale of the grass blade is $O(10^2 - 10^3)$; therefore, neglecting skin friction, we model the form drag on the vegetation as $\mathbf{f} = -N_g C_N \rho \mathbf{u} |u| d \hat{\mathbf{x}}$ (Nepf 1999; Nepf & Vivoni 2000; Ghisalberti & Nepf 2004), where N_g is the number of grass blades per unit horizontal area, C_N the drag coefficient for the flow normal to the grass, $\hat{\mathbf{x}}$ is unit vector in the horizontal direction and d the average blade width projected perpendicular to the flow. In the interest of simplicity, we model the turbulence using an eddy viscosity; see §5.3 for a detailed discussion. In the field, C_N , N_g and μ vary with position, but we do not expect these variations to be central to the instability mechanism, and therefore take them to be constants.

3. Linear stability analysis

We first calculate the fully developed steady solution $\mathbf{u} = U(y)\hat{\mathbf{x}}$ of (2.1) driven by constant pressure gradient dP/dx in a water column of depth $2H$ and vegetated depth h_g , and use it to non-dimensionalize the mathematical model. The momentum balance (2.1) for $U(y)$ simplifies to

$$-\frac{dP}{dx} + \mu U''(y) - S(y)\rho C_N d N_g U|U| = 0, \quad (3.1)$$

where $S(y) = 1$ for $0 < y < h_g$ and $S(y) = 0$ for $h_g < y < 2H$. Eq. (3.1) is solved subject to no shear at the boundaries, i.e., $U'(0) = U'(2H) = 0$. The lower boundary conditions is appropriate for dense vegetation because the shear stress exerted by the bottom surface is expected to be negligible compared to the vegetation drag (Nepf & Vivoni 2000), whereas the upper boundary condition models the free interface. A comparison of the steady flow profile from the solution of (3.1) with experimental measurements by Ghisalberti & Nepf (2004) is shown in Fig. 1. The profile $U(y)$ has three distinct regions, as determined by a matched asymptotic analysis (Hinch 1991). Within the vegetation, it is approximately uniform with $U(y) \approx U_g = \sqrt{\frac{-dP/dx}{\rho C_N d N_g}}$, arising from a balance of the drag with the pressure

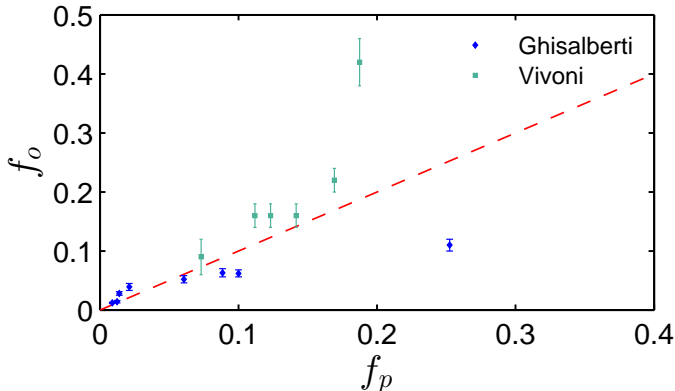


FIGURE 3. Comparison of experimental observations of the experimentally measured dominant frequency f_o (in Hz) with the predictions $f_p = \text{Im}(\sigma)$ from the solution of (3.2). The experimental data in the inset is obtained from Ghisalberti & Nepf (2002) and Vivoni (1998).

gradient. Outside the vegetation, the velocity has a simple parabolic profile. At the grass top, continuity of shear stresses results in a boundary layer of thickness δ . Denoting U_{bl} to be the velocity scale in the boundary layer, and $U_0 = (-dP/dx) H^2/\mu$ the velocity scale in the unvegetated region, the balance between viscous forces and vegetation drag implies $\mu U_{\text{bl}}/\delta^2 \sim \rho C_N d N_g U_{\text{bl}}^2$, and the continuity of shear stress across the grass top implies $U_{\text{bl}}/\delta \sim U_0/H$. Solving for δ and U_{bl} yields $\delta/H \sim U_{\text{bl}}/U_0 \sim (R\tilde{N}_g)^{-1/3}$, where $\tilde{N}_g = (C_N d H N_g)$ is the vegetation frontal area per bed area, and $R = \rho U_0 H/\mu$ is the Reynolds number of the flow. A numerical estimate of δ (estimated as U/U_y at $y = h_g$) is compared with this prediction in Fig. 1 (inset). We identify the boundary layer to be analogous to the shear layer invoked by Ghisalberti and Nepf (Ghisalberti & Nepf 2002, 2004) in the previous explanation of *monami*. This dependence of δ on N_g gives us a way to systematically investigate the effect of the shear layer thickness on the instability mechanism. Fig. 1 also shows that the asymptotic regime of a thin boundary layer is expected to hold for $R\tilde{N}_g \gtrsim 100$. In this notation, $U_g/U_0 = (R\tilde{N}_g)^{-1/2}$ (used later in deriving (5.3)).

Next we substitute $\mathbf{u} = (U + \tilde{u}, \tilde{v})$, $p = P + \tilde{p}$ in (2.1) and expand to linear order to investigate the evolution of small perturbations (\tilde{u}, \tilde{v}) , which obey

$$\begin{aligned} \rho(u_t + U u_x + v U_y) &= -p_x + \mu \nabla^2 u - 2S\rho C_N d N_g U u, \\ \rho(v_t + U v_x) &= -p_y + \mu \nabla^2 v, \quad \nabla \cdot \mathbf{u} = 0, \end{aligned}$$

where the tilde are dropped. These equations are non-dimensionalized using half channel height H , velocity U_0 , and time H/U_0 , leading to three non-dimensional parameters, *viz.* R , \tilde{N}_g , and the vegetation submergence ratio h_g/H . We also use δ/H in lieu of \tilde{N}_g to parametrize the vegetation density and help elucidate the instability mechanism. Using a stream function ψ with $u = \psi_y, v = -\psi_x$ to satisfy mass balance, we seek a solution of the form $(u, v, \psi) = (\hat{u}(y), \hat{v}(y), \phi(y)) e^{ikx + \sigma t}$ to obtain a modified Orr-Sommerfeld equation (Drazin & Reid 1981; Chen & Jirka 1997; Chu *et al.* 1991)

$$R^{-1} (D^2 - k^2)^2 \phi = [(\sigma + ikU) (D^2 - k^2) - ikU_{yy}] \phi + \tilde{N}_g D (2SUD\phi), \quad (3.2)$$

where $D = d/dy$, and subject to the boundary conditions $\phi = D^2\phi = 0$ at $y = 0$ and $y = 2$. The growth rate σ for a given wave number k appears as an eigenvalue that allows a non-trivial solution ϕ of (3.2). We solve (3.2) numerically for σ and ϕ .

4. Results - unstable modes and critical parameters

A threshold in R , above which the flow is unstable ($\text{Re}(\sigma) > 0$) for at least one k , emerges from the solution of (3.2). The dependence of this threshold R , and the corresponding marginally stable wavenumber k , on δ/H and h_g/H is shown in Fig. 2, and is found to compare well with experimental observations of Ghisalberti & Nepf (2002). The threshold R increases with the vegetation density, indicating a competition between the destabilizing shear in the flow, and the stabilizing effect of damping due to vegetation drag. The frequency ($\text{Im}(\sigma)$) of the fastest growing mode also agrees well with observed behaviour – frequency of *monami*, maxima in the velocity spectra, and frequency of vortex passage in lab scale experiments (Ghisalberti & Nepf 2002) – for cases where the vegetation was sufficiently dense to be modeled by a continuum drag field as shown in Fig. 3. The measured values of eddy viscosity vary with depth in the water column (Ghisalberti & Nepf 2004); we use $\mu = 0.1 \text{ Pa s}$ as a representative value from this range for comparison with experiments. Figure 2 also shows that the predicted dimensionless wavenumber of the dominant mode for parameters corresponding to the lab scale experiments ($4 < H/\delta < 10$) is very close to unity, and therefore the dimensional wavelength is approximately $2\pi H$.

To better understand the instability mechanism, we consider the dependence of the fastest growing wavenumber on δ . The fastest growing wavenumber first increases proportional to H/δ , but at a critical δ discontinuously jumps and remains $O(1)$ (see Fig. 2). To aid in explaining this behavior, we show heat maps of $\text{Re}(\sigma)$ as a function of R and k , for different h_g/H and \tilde{N}_g in Fig. 4. The smallest R on the neutral curve ($\text{Re}(\sigma) = 0$) sets the threshold. We observe that as \tilde{N}_g increases, the unstable region splits into two; we refer to the region with the higher k as “Mode 1”, and the one with the lower k as “Mode 2”. For $h_g/H \lesssim 0.9$, the unstable region for Mode 1 recedes to higher R , and for $h_g/H \gtrsim 0.9$, the region shrinks to zero size. In either case, due to such behaviour the most unstable mode transitions discontinuously from Mode 1 to Mode 2. All experimental data we have found corresponds to a vegetation density for which the unstable region in the $R - k$ space has not split into two, so we are unable to determine if flow instability in the lab scale experiments (Ghisalberti & Nepf 2002) are due to Mode 1 or Mode 2.

5. Discussion – comparison of unstable modes with Kelvin Helmholtz

The distinct asymptotic behavior of the two modes as $\tilde{N}_g \gg 1$ distinguish them from each other and facilitate comparison with KH instability mechanism.

5.1. Mode 1

We numerically observe that the threshold Reynolds number for Mode 1 instability scales as $R \sim (H/\delta)^2$ (or $R \propto \tilde{N}_g^2$). Our calculations also show that Mode 1 asymptotically localizes to the boundary layer near the grass top, and exhibits highest growth for a perturbation of $k \sim H/\delta$ (see Fig. 2). The behavior of this critical Reynolds number can be understood by considering the limit $R \gg 1$ and $\tilde{N}_g \gg 1$. Estimating the size of various terms of (3.2) within the boundary layer in this limit help us understand the behavior of the critical R . Using $D \sim H/\delta$, $\sigma \sim O(1)$, and $U = U_{bl} \sim \delta/H$ in the boundary layer; the magnitude of the advection term is $(H/\delta)^2$ (or $R^{2/3}\tilde{N}_g^{2/3}$), and the viscous and vegetation drag terms are $(1/R)(\delta/H)^{-4}$ (or $(R^{1/3}\tilde{N}_g)^{4/3}$). The advection term, viscous term and vegetation drag terms balance when $R \sim (H/\delta)^2$ (or $R \sim \tilde{N}_g^2$).

To further understand the mechanism of Mode 1, we rescale (3.2) near the grass-top using the boundary layer scalings $\eta = y/(\delta/H)$, $U(\eta) = (\delta/H)\tilde{U}(\eta)$ and $k = (H/\delta)\tilde{k}$.

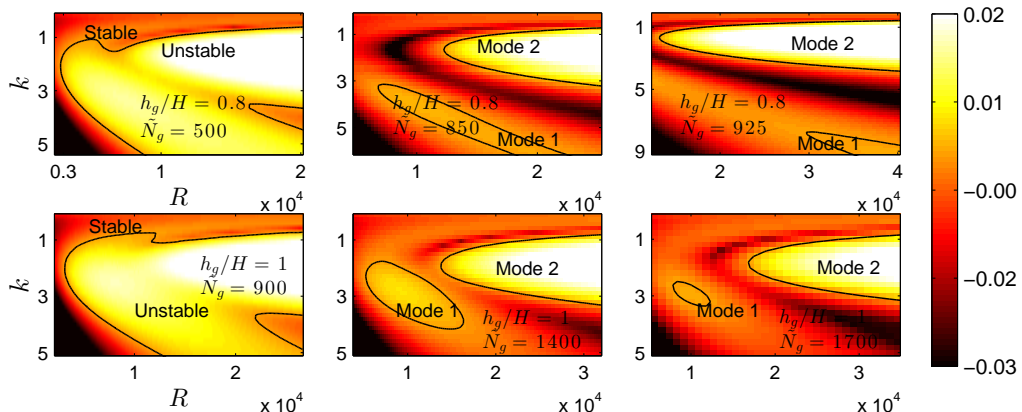


FIGURE 4. $\text{Re}(\sigma)$ and the neutral curve ($\text{Re}(\sigma)=0$) as a function of wavenumber and R for parameters shown in the corresponding panel. As \tilde{N}_g increases, the unstable region splits into two labeled as “Mode 1” and “Mode 2”. For \tilde{N}_g below (above) a critical value, Mode 1 (Mode 2) sets the threshold R .

With these scalings (3.2) simplifies to

$$(\bar{D}^2 - \bar{k}^2)^2 \phi = (R/\tilde{N}_g^2)^{1/3} [(\sigma + i\bar{k}\bar{U})(\bar{D}^2 - \bar{k}^2) - i\bar{k}\bar{U}_{\eta\eta}] \phi + \bar{D}(2S\bar{U}\bar{D}\phi), \quad (5.1)$$

in a region of thickness $O(\delta)$ near $y = h_g$, where $\bar{D} = d/d\eta$. Since (R/\tilde{N}_g^2) is the only remaining parameter in (5.1), the mode shape and solution are expected to converge in the limit $R \gg 1$, $\tilde{N}_g \gg 1$, but R/\tilde{N}_g^2 fixed. Our numerical findings confirm this expectation; the critical R scales as $(H/\delta)^2$ as shown in Fig. 2 and the mode shapes are self-similar with length scale δ , as shown in Fig 5.

Mode 1 shares many characteristics with the KH instability (see Table 1). The fastest growing wavenumber at the critical R scales as $k \propto (H/\delta)$, similar to KH instability. The extent of the unstable mode is also localized to the boundary layer region. The porous nature of the vegetation implies that a weak flow of magnitude $U_{bl} = U_0\delta/H$ penetrates a thin boundary layer region δ , and therefore the shear gradient $U_{yy} \sim U_0/\delta H$ is largest in this region. The strong shear gradient U_{yy} in the boundary layer plays a central role in destabilizing the flow and localizing the instability to that region.

Our detailed description of Mode 1, given by (5.1) also highlights key differences with formulations of KH. KH is usually described using the inviscid Rayleigh’s equation,

$$(\sigma + ikU)(D^2 - k^2)\phi = ikU_{yy}\phi, \quad (5.2)$$

and is therefore not parametrized by the Reynolds number. Describing the instability using the Orr-Sommerfeld equation introduces the Reynolds number as a parameter, but shear flows with tanh-profiles are unstable for all values of the parameter (Drazin & Reid 1981). Therefore, based on the inviscid formulations of KH instability, the origin of the threshold flow conditions observed in experiments and the field is unclear.

In our model, (turbulent eddy) viscosity sets the scale of the boundary layer, and therefore for Mode 1. However, the boundary layer is established only in the vegetated region; the velocity profile does not saturate on the scale of δ in the unvegetated region. The threshold flow condition arises from a competition between the destabilizing role of fluid inertia, which is very similar to the one played in KH, and the vegetation drag. The vegetation drag may not be neglected within this boundary layer, and therefore plays a central role in the Mode 1 instability mechanism.

5.2. Mode 2

The threshold condition for Mode 2 is numerically observed to be $R \propto (\delta/H)^{-3/2}$ (or $R \propto \tilde{N}_g$) for $k \sim O(1)$, shown in Fig. 2, which can be understood by assuming $R \gg 1$ but fixed $R/\tilde{N}_g \sim O(1)$. In this limit, the non-dimensional flow in the grass bed is $U_g/U_0 \sim (R\tilde{N}_g)^{-1/2} \ll 1$, and therefore $ikU \ll \sigma$ may be neglected in comparison to σ . Furthermore, U_{yy} decays to zero within the grass outside the boundary layer. Outside the grass, the turbulent viscous stress term is negligible compared to the inertial term because $R \gg 1$. Thus, (3.2) simplifies to

$$\sigma (D^2 - k^2) \phi = -2(\tilde{N}_g/R)^{1/2} D^2 \phi, \quad \text{for } y < h_g \quad (5.3a)$$

$$(\sigma + ikU) (D^2 - k^2) \phi = ikU_{yy} \phi, \quad \text{for } y > h_g. \quad (5.3b)$$

The only remaining parameter in (5.3) is R/\tilde{N}_g . For fixed R/\tilde{N}_g , the mode shape converges in the aforementioned limit, in agreement with our numerical results shown in Fig. 5. This convergence indicates that we have identified the correct asymptotic limit to investigate Mode 2. The parameter R/\tilde{N}_g therefore sets the threshold, justifying the numerically observed asymptotic behavior $R \propto \tilde{N}_g$ (or $R \sim (\delta/H)^{-3/2}$; see Fig. 2 for comparison with numerical results).

The structure of this mode in the aforementioned limit is such that ϕ is continuous at $y = h_g$, but $D\phi$ undergoes a rapid transition there, on the scale of boundary layer thickness δ . The eigenvalues and the mode shape are otherwise independent of δ . Therefore we conclude that the boundary layer only plays a secondary role of regularizing the discontinuity in tangential velocity arising at $y = h_g$ in this instability mechanism. The enhanced shear in the boundary layer plays no role for this mode of instability.

Mode 2 has characteristics distinct from KH. Outside the grass, the unstable mode shape is governed by the inviscid Rayleigh's equation (5.2). An inflection point in $U(y)$ is a necessary condition for instability arising from (5.2) according to Rayleigh's criteria (Rayleigh 1879). However, for our $U(y)$ profiles, $U_{yy}(y) = -1$ above the grass and therefore does not change sign for $y > h_g$. Instead, the dynamics are coupled with the flow in the grass bed described by (5.3a) in $y < h_g$. The absence of U_{yy} in (5.3a) indicates that U_{yy} is approximated to be zero in $y < h_g$, and therefore the positive values of U_{yy} that occurs in the boundary layer do not affect this mode of instability to leading order. Furthermore, the presence of the critical parameter R/\tilde{N}_g in (5.3a) indicates the presence of alternative destabilizing dynamics, involving the interaction of flow in the unvegetated region governed by (5.2) with the flow in the vegetated region incorporating the drag. Therefore, we conclude that Mode 2 is distinct from the KH instability, and owes its existence to vegetation drag.

5.3. Role of turbulence model

We have modeled turbulence using a constant eddy viscosity. The simplicity of this model allows us to make progress and capture the essential features of the instability. However, this simplicity in some cases only provides a qualitatively accurate description of the flow. In this subsection, we present an account of the advantages and shortcomings of assuming a constant eddy viscosity to model the turbulence.

As a consequence of the constant eddy viscosity, the boundary layer thickness δ scales as $H(R\tilde{N}_g)^{-1/3}$. Experimental observations show that the boundary layer thickness scales instead as \tilde{N}_g^{-1} (Nepf *et al.* 2007). Whereas the precise boundary layer thickness is governed by the details of the turbulence model, the existence of this boundary layer for dense vegetation is independent of the turbulence model. We have captured one possible

	KH	Mode 1	Mode 2
Base velocity profile	$U(y) = U_0 \tanh(y/\delta)$	Equation (3.1)	
Domain	$-\infty < y < \infty$	$-1 < y < 1$	
Inflection point	exists at $y = 0$	$U''(y)$ discontinuous at $y = h_g$	
Shear layer thickness	δ	$\delta \sim H \left(R \tilde{N}_g \right)^{-1/3}$	
Linearized dynamics	Equation (5.2)	Equation (3.2)	
Dense grass limit	no grass included	Equation (5.1)	Equation (5.3)
Critical parameters	none	$R \propto \tilde{N}_g^2$	$R \propto \tilde{N}_g$
Most unstable k as $\delta \rightarrow 0$	$\propto H/\delta$	$\propto H/\delta$	$O(1)$
Mode localized?	yes, near $y = 0$	yes, near $y = h_g$	no, spans water column

TABLE 1. Comparison between KH instability and the two unstable modes resulting from solution of 3.2.

realization of this feature using constant eddy viscosity. Experiments have also shown that a model based on mixing length l better approximates the turbulent characteristics of the flow with $l \sim \delta$; *i.e.*, the boundary layer itself establishes eddies to transport momentum. The eddy viscosity corresponding to this model is $\mu \sim \rho U \delta$, and the leading order balance between turbulent momentum transport and vegetation drag is $\mu U / \delta^2 \sim \rho C_N d N_g U^2$. Substituting μ yields $\delta / H \sim \tilde{N}_g^{-1}$, in agreement with the experimental observations.

Within our framework, the mixing length model implies a scale for the eddy viscosity $\mu \sim \rho U_{bl} \delta$ at grass top, which corresponds to an effective $R \sim U_0 H / U_{bl} \delta$. Furthermore, matching the slope of the velocity profile from the boundary layer to the unvegetated flow implies $U_{bl} / U_0 \sim \delta / H$, and therefore $R \sim (H / \delta)^2$. Substituting this relation in $\delta / H \sim (R \tilde{N}_g)^{-1/3}$ and solving for δ yields $\delta / H \sim \tilde{N}_g^{-1}$. This simple scaling analysis shows that the boundary layer thickness depends on the turbulence model, and indicates that turbulence models based on mixing lengths will yield more realistic scalings for boundary layer thickness. At the same time, the qualitative features of the instability are represented by our analysis.

The Mode 1 instability is driven by the intense shear on the scale of the boundary layer. The driving mechanism for this instability is similar to that of KH, and relies only on the presence of this shear as presented in the $\bar{U}_{\eta\eta}$ term in equation (5.1). Therefore, we expect Mode 1 instability to be exhibited independent of the turbulence model. We further expect the fastest growing wavenumber to be proportional to $1/\delta$, and the mode to be localized to the boundary layer because these results have a basis in dimensional analysis. The threshold parameters for Mode 1, however, may depend on the precise turbulence model used.

For the Mode 2 instability, the turbulent momentum transport is found to be irrelevant to leading order. In the asymptotic limit of dense grass, (5.3) shows that the instability is driven by the interaction of the unvegetated flow with the vegetation drag. The influence of the turbulence model is limited to the regularization of the sharp transition in tangential velocity across the grass top. Therefore we expect Mode 2 and its features to be preserved even if a different turbulence model is used.

5.4. Comparison with previous models

A modified version of the Orr-Sommerfeld equation was analyzed previously by Chu *et al.* (1991), Chen & Jirka (1997), and White & Nepf (2007) in the context of instabilities in depth-averaged shallow water flows, where bottom friction replaces or augments vegetation drag. They assumed the steady profile to be a hyperbolic tangent, the drag to be isotropic, and the flow domain to be infinite in y . White & Nepf (2007) also

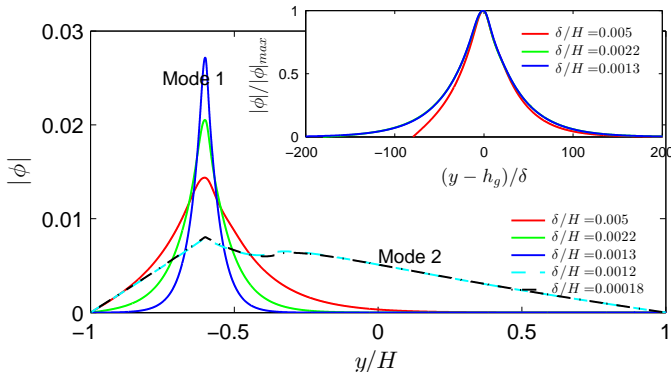


FIGURE 5. Plot of the neutral Mode 1 (solid) and Mode 2 (dashed) shape $|\phi|$ in the limit of small δ/H for $h_g/H = 0.2$. The approach of mode shapes to each other for these small values of δ/H indicates that the dense vegetation asymptote is reached. Mode 1 shapes appear self-similar in shape as $\delta \rightarrow 0$. Inset shows rescaled $|\phi|$ for Mode 1 as a function of $(y - h_g)/\delta$ approach a universal shape, indicating that an asymptotic limit has been reached.

neglected the eddy viscosity in their stability analysis. While a detailed investigation needed to compare the consequence of the different assumptions is outside the scope of this paper, we discuss similarities and differences between their results and ours. These investigations only found one unstable mode. It is most likely so because the calculations were restricted to a parameter regime where the two modes have not yet been separated from each other, as is the case shown in Fig. 4 for the lowest \tilde{N}_g . These investigations also found that increasing the drag could further destabilize the flow, which is consistent with our interpretation of the Mode 2 instability mechanism.

The analogous oscillation of terrestrial canopies in wind, known as *honami* (Inoue 1956; Raupach *et al.* 1996), is different because the atmospheric boundary layer is much larger than the vegetation height. In the framework of our model, the limit of $h_g/H \ll 1$ while $\delta/h_g = \text{constant}$ can be used to represent the hydrodynamic instability for the terrestrial case. We find that in this case, the transition from Mode 1 to Mode 2 happens at such a large vegetation density, that Mode 2 is irrelevant. Hence, only the KH-like characteristics are observed in the terrestrial case.

While predictions of the threshold R for the onset of the instability and the frequency of oscillations are comparable to experimental observations, the deviation of our model predictions from the observed may be attributed to the various simplifications in our model. In real meadows, the grass is flexible, and the drag coefficients are known to vary from bottom to tip of the grass blades due to variation in vegetation characteristics (Vivoni 1998; Nepf & Vivoni 2000). The turbulence model for the flow through the meadow can also be improved from one with constant eddy viscosity (Ghisalberti & Nepf 2002, 2004). Although these model improvements might lead to a better agreement between the observed and the predicted quantities, the insight furnished by (5.1) and (5.3), and therefore our main conclusions, remain useful.

We now test the assumption of an undeformable grass bed due to the dominant restoring force of buoyancy, using the criteria that the buoyancy time scale be much shorter than the hydrodynamic time scale H/U_0 . For a common seagrass, *Zostera Marina*, the relative density difference is $\Delta\rho/\rho \approx 0.25$, the volume fraction is $V_f \approx 0.1$ and $H = 1$ m (Fonseca 1998), yielding the buoyancy time scale to be $\sqrt{\rho H/V_f \Delta\rho g} \approx 2$ s. The hydrodynamic time scale assuming $U_0 \approx 0.1$ m/s is 10 s, and therefore longer than the buoyancy time scale. We have neither accounted for the pre-factors appearing in the

scaling argument, or considered cases when the time-scale separation is not so evident. Accounting for these factors can lead to further interesting behavior (Py *et al.* 2006; Gosselin & De Langre 2009), and motivates further investigation.

6. Conclusion

In conclusion, we show that the hydrodynamic instability underlying *monami* differs from the traditional KH due to the presence of the vegetation drag. The threshold flow condition observed in the field and in lab experiments arises due to the presence of this drag. Furthermore, our linear stability analysis reveals two modes, namely Mode 1 with a mechanism similar to KH and Mode 2 with characteristics distinctly different from KH. We are unable to determine based on observations, and therefore have refrained from identifying, which mode is observed in experiments and it still remains a subtle question and subject of future investigation. Since the two modes merge for the experimental parameters, KH may not be assumed to underlie *monami*. The spatial structure of the instability modes has direct implications for transport in the grass bed. Our analysis also informs flow structure formation in many other related scenarios, such as flow over coral reefs, permeable sediments, and flow through urban environments, and therefore is expected to have a wider impact.

MMB was supported by the Collective Interactions Unit, OIST Graduate University, while visiting Brown University. We thank Heidi Nepf, Marco Ghisalberti, Mitul Luhar, and L. Mahadevan for helpful discussions.

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