

Calculations

Relative humidity of the ambient environment	$RH$	75.00		
Depth of the section	$h_{section}$	400.00	$mm$	
Width of the section	$b_{section}$	1000.00	$mm$	Consider 1000mm when working with slab
Cross-sectional area	$A_c$	400000	$mm^2$	
$h_{section} \cdot b_{section} = 400 \cdot 1000 = 400000$				
Characteristic compressive cylinder strength of concrete at 28 days	$f_{ck}$	35.00	$MPa$	
Mean value of concrete cylinder compressive strength at 28 days	$f_{cm}$	43.00	$MPa$	BS EN 1992-1-1 Table 3.1
$f_{ck} + 8 = 35 + 8 = 43$				
Perimeter of the member in contact with the atmosphere	$u$	1000.00	$mm$	The values is for a 1m slab
Age of concrete in days at the moment considered	$t$	2557.00	$days$	7 years is considered
Age of concrete at loading in days	$t_0$	28.00	$days$	Typically 28 days but may be 7 days for rapid construction
Coefficient to consider the influence of the concrete strength	$\alpha_1$	0.87		B.8c
$\left(\frac{35}{f_{cm}}\right)^{0.7} = \left(\frac{35}{43}\right)^{0.7}$				
Coefficient to consider the influence of the concrete strength	$\alpha_2$	0.96		B.8c
$\left(\frac{35}{f_{cm}}\right)^{0.2} = \left(\frac{35}{43}\right)^{0.2}$				
Coefficient to consider the influence of the concrete strength	$\alpha_3$	0.90		B.8c
$\left(\frac{35}{f_{cm}}\right)^{0.5} = \left(\frac{35}{43}\right)^{0.5}$				
Notional size of the member	$h_0$	800.00	$mm$	B.6
$\frac{2 \cdot A_c}{u} = \frac{2 \cdot 400000}{1000}$				
Interim value for calculation below	$\beta_{H,interim1}$	1630.11		B.8a
$1.5 \cdot \left(1 + (0.012 \cdot RH)^{18}\right) \cdot h_0 + 250 = 1.5 \cdot \left(1 + (0.012 \cdot 75)^{18}\right) \cdot 800 + 250$				
Interim value for calculation below	$\beta_{H,interim2}$	1605.66		B.8b
$1.5 \cdot \left(1 + (0.012 \cdot RH)^{18}\right) \cdot h_0 + 250 \cdot \alpha_3 = 1.5 \cdot \left(1 + (0.012 \cdot 75)^{18}\right) \cdot 800 + 250 \cdot 0.90$				
Coefficient depending on the relative humidity (RH in %) and the notional member size	$\beta_H$	1353.29		B.8a & B.8b
$\begin{cases} \min(\beta_{H,interim1}, 1500), & \text{if } f_{cm} \leq 35 \\ \min(\beta_{H,interim2}, 1500 \cdot \alpha_3), & \text{otherwise} \end{cases} = \begin{cases} \min(1630.11, 1500), & \text{if } 43 \leq 35 \\ \min(1605.66, 1500 \cdot 0.90), & \text{otherwise} \end{cases}$				
Coefficient to describe the development of creep with time after loading	$\beta_c(t, t_0)$	0.88		B.7
$\left(\frac{t - t_0}{\beta_H + t - t_0}\right)^{0.3} = \left(\frac{2557 - 28}{1353.29 + 2557 - 28}\right)^{0.3}$				
Factor to allow for the effect of concrete age at loading on the notional creep coefficient	$\beta(t_0)$	0.49		B.5
$\frac{1}{0.1 + t_0^{0.2}} = \frac{1}{0.1 + 28^{0.2}}$				
Factor to allow for the effect of concrete strength on the notional creep coefficient	$\beta(f_{cm})$	2.56		B.4
$\frac{16.8}{\sqrt{f_{cm}}} = \frac{16.8}{\sqrt{43}}$				

Interim value for calculation below	$\varphi_{RH,interim1}$	1.27	B.3a
$1 + \frac{1 - \frac{RH}{100}}{0.1 \cdot h_0^{\frac{1}{3}}} = 1 + \frac{1 - \frac{75}{100}}{0.1 \cdot 800^{\frac{1}{3}}}$			
Interim value for calculation below	$\varphi_{RH,interim2}$	1.18	B.3b
$\left(1 + \frac{1 - \frac{RH}{100}}{0.1 \cdot h_0^{\frac{1}{3}}} \cdot \alpha_1\right) \cdot \alpha_2 = \left(1 + \frac{1 - \frac{75}{100}}{0.1 \cdot 800^{\frac{1}{3}}} \cdot 0.87\right) \cdot 0.96$			
Factor to allow for the effect of relative humidity on the notional creep coefficient	$\varphi_{RH}$	1.18	B.3a & B.3b
$\begin{cases} \varphi_{RH,interim1}, & \text{if } f_{cm} \leq 35 \\ \varphi_{RH,interim2}, & \text{otherwise} \end{cases} = \begin{cases} 1.27, & \text{if } 43 \leq 35 \\ 1.18, & \text{otherwise} \end{cases}$			
Notional creep coefficient	$\varphi_0$	1.48	B.2
$\varphi_{RH} \cdot \beta(f_{cm}) \cdot \beta(t_0) = 1.18 \cdot 2.56 \cdot 0.49$			
The creep coefficient	$\varphi(t, t_0)$	1.30	BS EN 1992-1-1 Annex B (B.1)
$\varphi_0 \cdot \beta_c(t, t_0) = 1.48 \cdot 0.88$			