

# Homework 4

*Due Date: November 21, 2025*

## 2D Heat Equation with Finite Element Method

Consider the time-dependent heat equation as follows:

$$\frac{\partial u}{\partial t} - \nu \Delta u = f(x, y, t) \quad \text{in } \Omega = (-2, 2) \times (-2, 2), \quad t \in (0, 1]$$

with diffusion coefficient  $\nu = 0.05$  and corresponding homogeneous Dirichlet boundary conditions:

$$u(x, y, \cdot) = 0 \quad \text{for } (x, y) \in \partial\Omega$$

We assume the exact solution is given by:

$$u_{exact}(x, y, t) = e^{-8\pi^2\nu t} \sin(2\pi x) \sin(2\pi y)$$

We will be using rectangular elements. You will use the bilinear  $Q_1$  element as your basis function to solve the heat equation. The corresponding four shape functions defined in the reference element  $(\xi, \eta) \in (-1, 1) \times (-1, 1)$  are:

$$\begin{aligned} \Phi_1(\xi, \eta) &= \frac{1}{4}(1 - \xi)(1 - \eta), & \Phi_2(\xi, \eta) &= \frac{1}{4}(1 + \xi)(1 - \eta), \\ \Phi_3(\xi, \eta) &= \frac{1}{4}(1 + \xi)(1 + \eta), & \Phi_4(\xi, \eta) &= \frac{1}{4}(1 - \xi)(1 + \eta) \end{aligned}$$

## Preliminary Setup

This is a general outline of the finite element method for solving the 2D heat equation. The specifics for our problem will be addressed in the subsequent questions.

## Weak Formulation

Let  $v$  be a test function belonging to the function space:

$$V = \{v \in H_0^1(\Omega) \mid v, v' \in L^2(\Omega)\}$$

Note that  $v = 0$  on  $\partial\Omega$ . Multiplying the PDE by  $v$  and integrating over the domain  $\Omega$ , we have:

$$\int_{\Omega} u_t v - \nu \nabla u \cdot \nabla v \, dx = \int_{\Omega} f v \, dx$$

Integrating by parts on the second term of the left-hand side:

$$\int_{\Omega} u_t v \, dx + \nu \left[ \int_{\Omega} \nabla u \cdot \nabla v \, dx - \int_{\partial\Omega} v (\nabla u \cdot n) \, ds \right] = \int_{\Omega} f v \, dx$$

Since  $v = 0$  on  $\partial\Omega$ , the boundary integral vanishes. Therefore, the weak formulation is given by:

$$\int_{\Omega} u_t v \, dx + \nu \int_{\Omega} \nabla u \cdot \nabla v \, dx = \int_{\Omega} f v \, dx$$

Where  $u, v \in V$ . Denote the left hand side as  $a(u, v)$  and the right hand side as  $L(v)$ .

## Discretization and Global System

We discretize the spatial domain  $\Omega$  into rectangular elements. Let  $V_h \subset V$  be the finite-dimensional subspace spanned by the basis functions  $\{\phi_i\}_{i=1}^N$ , where  $N$  is the total number of nodes in the mesh (will be reduced later on based on BC). Each bilinear  $\phi_n$  corresponds to some node  $(x_i, y_j)$  satisfying  $\phi_{i,j} = \delta_{i,j}$ . We assume  $u_h \in V_h$  satisfies the weak formulation  $a(u_h, v_h) = L(v_h)$  for all  $v_h \in V_h$ .

Approximate the solution of  $u$  as:

$$u_h = \sum_{j=1}^N U_j(t) \phi_j(x, y)$$

where  $U_j$  are time-dependent coefficients to be determined. The test function  $v$  is also chosen from the same space and discretized similarly:

$$v_h = \sum_{i=1}^N V_i(t) \phi_i(x, y)$$

Substituting these approximations into the weak formulation, we obtain the system:

$$\sum_{i,j=1}^N V_j \left( \int_{\Omega} \phi_i \phi_j \, dx \right) \frac{dU_i}{dt} + \nu \sum_{i,j=1}^N V_j \left( \int_{\Omega} \nabla \phi_i \cdot \nabla \phi_j \, dx \right) U_i = \sum_{j=1}^N V_j \int_{\Omega} f \phi_j \, dx$$

We may factor out the  $V_j$  to give us:

$$\sum_{i,j=1}^N \left( \int_{\Omega} \phi_i \phi_j \, dx \right) \frac{dU_i}{dt} + \nu \sum_{i,j=1}^N \left( \int_{\Omega} \nabla \phi_i \cdot \nabla \phi_j \, dx \right) U_i = \sum_{j=1}^N \int_{\Omega} f \phi_j \, dx$$

More compactly, let:

$$U = [U_1, U_2, \dots, U_N]^T, \quad M_{ij} = \int_{\Omega} \phi_i \phi_j \, dx, \quad K_{ij} = \int_{\Omega} \nabla \phi_i \cdot \nabla \phi_j \, dx, \quad F_j = \int_{\Omega} f \phi_j \, dx$$

such that  $M = (M_{ij})$ ,  $K = (K_{ij})$ , and  $F = [F_1, F_2, \dots, F_N]^T$ . We can rewrite the system as:

$$M \frac{dU}{dt} + \nu K U = F$$

where  $M$  is the mass matrix,  $K$  is the stiffness matrix, and  $F$  is the force vector.

## Elemental-Level Systems and Assembly

We suppose element-wise, each  $u^{(e)}$  satisfies the weak formulation over its own domain  $\Omega^{(e)}$ :

$$\int_{\Omega^{(e)}} u_t^{(e)} v^{(e)} \, dx + \nu \int_{\Omega^{(e)}} \nabla u^{(e)} \cdot \nabla v^{(e)} \, dx = \int_{\Omega^{(e)}} f v^{(e)} \, dx$$

We suppose the local approximations are given by:

$$u_h^{(e)} = \sum_{j=1}^4 U_j^{(e)} \phi_j^{(e)}(x, y), \quad v_h^{(e)} = \sum_{i=1}^4 V_i^{(e)} \phi_i^{(e)}(x, y)$$

since we have rectangular elements with four nodes each. Using the same process as before, we can derive the elemental system:

$$M^{(e)} \frac{dU^{(e)}}{dt} + \nu K^{(e)} U^{(e)} = F^{(e)}$$

We can express the global matrices and vector as sums over all elements:

$$M = \sum_{e=1}^E M^{(e)}, \quad K = \sum_{e=1}^E K^{(e)}, \quad F = \sum_{e=1}^E F^{(e)}$$

where  $E$  is the total number of elements, and the elemental matrices and vector are defined as:

$$M_{ij}^{(e)} = \int_{\Omega^{(e)}} \phi_i^{(e)} \phi_j^{(e)} \, dx, \quad K_{ij}^{(e)} = \int_{\Omega^{(e)}} \nabla \phi_i^{(e)} \cdot \nabla \phi_j^{(e)} \, dx, \quad F_j^{(e)} = \int_{\Omega^{(e)}} f \phi_j^{(e)} \, dx$$

Here,  $\Omega^{(e)}$  is the domain of element  $e$ , and  $\phi_i^{(e)}$  are the local shape functions associated with element  $e$ .

We can use quadrature to numerically compute the integrals for  $M^{(e)}$ ,  $K^{(e)}$ , and  $F^{(e)}$  on each element, then assemble them into the global system.

## Question 1

By substituting  $u_{exact}$  into the PDE, determine the forcing term  $f(x, y, t)$  such that:

$$\frac{\partial u_{exact}}{\partial t} - \nu \Delta u_{exact} = f(x, y, t)$$

*Solution.* For the purposes of this question, denote  $u = u_{exact}$ . Where  $u$  is the exact solution given by:

$$u(x, y, t) = e^{-8\pi^2 \nu t} \sin(2\pi x) \sin(2\pi y)$$

Let us first compute the time derivative:

$$\frac{\partial u}{\partial t} = -8\pi^2 \nu e^{-8\pi^2 \nu t} \sin(2\pi x) \sin(2\pi y)$$

Next, we want to find the Laplacian. Computing the first and second derivative with respect to  $x$ :

$$\frac{\partial u}{\partial x} = 2\pi e^{-8\pi^2 \nu t} \cos(2\pi x) \sin(2\pi y)$$

$$\frac{\partial^2 u}{\partial x^2} = -4\pi^2 e^{-8\pi^2 \nu t} \sin(2\pi x) \sin(2\pi y)$$

The 2nd derivative with respect to  $y$  is the same:

$$\frac{\partial^2 u}{\partial y^2} = -4\pi^2 e^{-8\pi^2 \nu t} \sin(2\pi x) \sin(2\pi y)$$

Therefore, the Laplacian is:

$$\Delta u = \frac{\partial^2 u}{\partial x^2} + \frac{\partial^2 u}{\partial y^2} = -8\pi^2 e^{-8\pi^2 \nu t} \sin(2\pi x) \sin(2\pi y)$$

Substituting these results into the heat equation, we have:

$$\frac{\partial u}{\partial t} - \nu \Delta u = -8\pi^2 \nu e^{-8\pi^2 \nu t} \sin(2\pi x) \sin(2\pi y) - \nu(-8\pi^2 e^{-8\pi^2 \nu t} \sin(2\pi x) \sin(2\pi y)) = 0$$

So our forcing term is:

$$f(x, y, t) = 0$$

and we are working with the homogeneous heat equation.

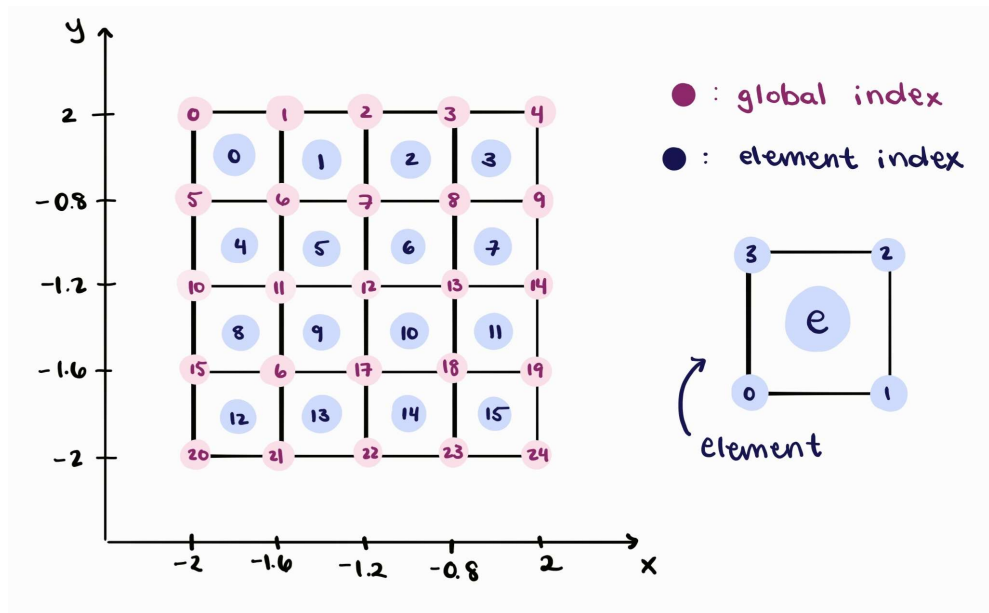
## Question 2

Discretize the spacial domain  $\Omega$  into 16 square elements arranged in a  $4 \times 4$  grid, with the node coordinates:

$$(x, y) \in \{-2, -1.6, -1.2, -0.8, 2\} \times \{-2, -1.6, -1.2, -0.8, 2\}$$

Draw this mesh, define your own global numbering, label all global node numbers, and generate corresponding elemental connectivities.

*Solution.* All indexing used will start from 0. The mesh is as follows:



Globally, we have 25 nodes numbered from 0 to 24. Numbering starts from the top-left corner and goes row-wise. The elements are numbered from 0 to 15, also row-wise in the same fashion. For each element, its indices (0, 1, 2, 3) start from the bottom-left and go counter-clockwise to match the definition of the bilinear  $Q_1$  element. The following code was used to generate the connectivity matrix:

Listing 1: Question 2 Code

```

1 def global_indexing(width, height=None):
2     if height is None:
3         height = width
4     return np.arange(width * height).reshape((height, width))
5
6 def generate_connectivity_matrix(global_indices):
7     total_elements = (global_indices.shape[0] - 1) * (global_indices.shape[1] -
8     1)
9     connectivity_matrix = np.zeros((total_elements, 4), dtype=int)
10    element = 0
11    for i in range(global_indices.shape[0] - 1):

```

```

11     for j in range(global_indices.shape[1] - 1):
12         # Element node ordering: bottom-left, bottom-right,
13         # top-right, top-left
14         connectivity_matrix[element, 0] = global_indices[i+1, j]
15         connectivity_matrix[element, 1] = global_indices[i+1, j+1]
16         connectivity_matrix[element, 2] = global_indices[i, j+1]
17         connectivity_matrix[element, 3] = global_indices[i, j]
18         element += 1
19     return connectivity_matrix
20
21 if __name__ == "__main__":
22     width = 5 # Number of nodes along one dimension
23     global_indices = global_indexing(width)
24     connectivity_matrix = generate_connectivity_matrix(global_indices)
25
26     with open("./outputs_4/matrices.txt", "w") as f:
27         # print(connectivity_matrix)
28         latex_matrix = sp.latex(sp.Matrix(connectivity_matrix))
29         f.write("Connectivity Matrix:\n")
30         f.write(latex_matrix + "\n\n")

```

The elemental connectivities are as follows:

Element	Node 0	Node 1	Node 2	Node 3
0	5	6	1	0
1	6	7	2	1
2	7	8	3	2
3	8	9	4	3
4	10	11	6	5
5	11	12	7	6
6	12	13	8	7
7	13	14	9	8
8	15	16	11	10
9	16	17	12	11
10	17	18	13	12
11	18	19	14	13
12	20	21	16	15
13	21	22	17	16
14	22	23	18	17
15	23	24	19	18

### Question 3

For one physical element  $u^{(e)}$ , write the mapping from the reference element  $(\xi, \eta) \in (-1, 1) \times (-1, 1)$  to the physical coordinates  $(x, y)$  in terms of the nodal coordinates  $(x_n, y_n)$  and the shape functions  $\Phi_n(\xi, \eta)$ . Also, derive the Jacobian matrix  $J(\xi, \eta)$  of this mapping.

*Solution.* Reindexing the four shape functions to fit our indexing scheme for the element nodes, we have:

$$\begin{aligned}\Phi_0(\xi, \eta) &= \frac{1}{4}(1 - \xi)(1 - \eta), & \Phi_1(\xi, \eta) &= \frac{1}{4}(1 + \xi)(1 - \eta), \\ \Phi_2(\xi, \eta) &= \frac{1}{4}(1 + \xi)(1 + \eta), & \Phi_3(\xi, \eta) &= \frac{1}{4}(1 - \xi)(1 + \eta)\end{aligned}$$

On a physical element  $e$ , it is a rectangle of the region  $[x_0, x_1] \times [y_0, y_1]$  where  $(x_0, y_0)$  is the bottom-left corner and  $(x_1, y_1)$  is the top-right corner. The mapping from  $(\xi, \eta) \mapsto (x, y)$  should be given by the standard change of variables:

$$\begin{bmatrix} x(\xi, \eta) \\ y(\xi, \eta) \end{bmatrix} = \begin{bmatrix} \frac{1}{2}(x_1 - x_0)\xi + \frac{1}{2}(x_1 + x_0) \\ \frac{1}{2}(y_1 - y_0)\eta + \frac{1}{2}(y_1 + y_0) \end{bmatrix} \quad (1)$$

We will verify this using the shape functions. We assume that:

$$x(\xi, \eta) = \sum_{n=0}^3 x_n^{(e)} \Phi_n^{(e)}(\xi, \eta), \quad y(\xi, \eta) = \sum_{n=0}^3 y_n^{(e)} \Phi_n^{(e)}(\xi, \eta)$$

where  $(x_n^{(e)}, y_n^{(e)})$  are the nodal coordinates of element  $e$ . Note that by our indexing scheme:

$$\begin{aligned}(x_0^{(e)}, y_0^{(e)}) &= (x_0, y_0), & (x_1^{(e)}, y_1^{(e)}) &= (x_1, y_0), \\ (x_2^{(e)}, y_2^{(e)}) &= (x_1, y_1), & (x_3^{(e)}, y_3^{(e)}) &= (x_0, y_1)\end{aligned}$$

Expanding  $x(\xi, \eta)$ :

$$\begin{aligned}x(\xi, \eta) &= x_0^{(e)} \Phi_0^{(e)} + x_1^{(e)} \Phi_1^{(e)} + x_2^{(e)} \Phi_2^{(e)} + x_3^{(e)} \Phi_3^{(e)} \\ &= x_0 \frac{1}{4}(1 - \xi)(1 - \eta) + x_1 \frac{1}{4}(1 + \xi)(1 - \eta) + x_1 \frac{1}{4}(1 + \xi)(1 + \eta) + x_0 \frac{1}{4}(1 - \xi)(1 + \eta) \\ &= \frac{1}{4} [x_0(1 - \xi)(1 - \eta + 1 + \eta) + x_1(1 + \xi)(1 - \eta + 1 + \eta)] \\ &= \frac{1}{4} [2x_0(1 - \xi) + 2x_1(1 + \xi)] \\ &= \frac{x_1 - x_0}{2} \xi + \frac{x_1 + x_0}{2}\end{aligned}$$

Similarly for  $y(\xi, \eta)$ , we get:

$$y(\xi, \eta) = \frac{y_1 - y_0}{2} \eta + \frac{y_1 + y_0}{2}$$

Therefore, the mapping from reference to physical coordinates is given by equation 1

Now we can derive the Jacobian matrix,  $J(\xi, \eta)$ , of this mapping, which is defined as:

$$J(\xi, \eta) = \begin{bmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial x}{\partial \eta} \\ \frac{\partial y}{\partial \xi} & \frac{\partial y}{\partial \eta} \end{bmatrix}$$

Let us compute each partial derivative:

$$\frac{\partial x}{\partial \xi} = \frac{x_1 - x_0}{2}, \quad \frac{\partial x}{\partial \eta} = 0$$

$$\frac{\partial y}{\partial \xi} = 0, \quad \frac{\partial y}{\partial \eta} = \frac{y_1 - y_0}{2}$$

Therefore, our Jacobian is:

$$J(\xi, \eta) = \begin{bmatrix} \frac{x_1 - x_0}{2} & 0 \\ 0 & \frac{y_1 - y_0}{2} \end{bmatrix}$$

Note that if our elements were all equally sized, the Jacobian would be identical for all elements.



## Question 4

Using the basis functions from the reference element, compute the following derivatives:

$$\frac{\partial \Phi_i}{\partial \xi}, \frac{\partial \Phi_i}{\partial \eta} \quad \text{for } i = 0, 1, 2, 3$$

Then express the physical gradients  $\nabla \Phi_i = \left[ \frac{\partial \Phi_i}{\partial x}, \frac{\partial \Phi_i}{\partial y} \right]^T$  using the Jacobian.

*Solution.* Note that:

$$\Phi_i(x, y) = \Phi_i(\xi(x, y), \eta(x, y))$$

We know that:

$$\nabla \Phi_i(\xi, \eta) = J(\xi, \eta) \nabla \Phi_i(x, y)$$

where  $J$  is the Jacobian matrix derived in the previous question. Therefore, we can express the physical gradients as:

$$\nabla \Phi_i(x, y) = J^{-1}(\xi, \eta) \nabla \Phi_i(\xi, \eta)$$

Given the Jacobian from before:

$$J(\xi, \eta) = \begin{bmatrix} \frac{x_1 - x_0}{2} & 0 \\ 0 & \frac{y_1 - y_0}{2} \end{bmatrix}$$

Its inverse is given by:

$$J^{-1}(x, y) = \begin{bmatrix} \frac{2}{x_1 - x_0} & 0 \\ 0 & \frac{2}{y_1 - y_0} \end{bmatrix}$$

Then for each  $\Phi_i(x, y)$ , we have:

$$\begin{aligned} \nabla \Phi_i(x, y) &= J^{-1}(\xi, \eta) \nabla \Phi_i(\xi, \eta) \\ &= \begin{bmatrix} \frac{2}{x_1 - x_0} & 0 \\ 0 & \frac{2}{y_1 - y_0} \end{bmatrix} \begin{bmatrix} \frac{\partial \Phi_i}{\partial \xi} \\ \frac{\partial \Phi_i}{\partial \eta} \end{bmatrix} \\ &= \begin{bmatrix} \frac{2}{x_1 - x_0} & 0 \\ 0 & \frac{2}{y_1 - y_0} \end{bmatrix} \begin{bmatrix} \frac{\partial \Phi_i}{\partial \xi} \\ \frac{\partial \Phi_i}{\partial \eta} \end{bmatrix} \\ &= \begin{bmatrix} \frac{2}{x_1 - x_0} \frac{\partial \Phi_i}{\partial \xi} \\ \frac{2}{y_1 - y_0} \frac{\partial \Phi_i}{\partial \eta} \end{bmatrix} \\ &= 2 \begin{bmatrix} \frac{1}{x_1 - x_0} \frac{\partial \Phi_i}{\partial \xi} \\ \frac{1}{y_1 - y_0} \frac{\partial \Phi_i}{\partial \eta} \end{bmatrix} \end{aligned}$$

Let us first calculate  $\nabla \Phi_i(\xi, \eta)$ . For reference, the shape functions are:

$$\Phi_0(\xi, \eta) = \frac{1}{4}(1 - \xi)(1 - \eta), \quad \Phi_1(\xi, \eta) = \frac{1}{4}(1 + \xi)(1 - \eta),$$

$$\Phi_2(\xi, \eta) = \frac{1}{4}(1 + \xi)(1 + \eta), \quad \Phi_3(\xi, \eta) = \frac{1}{4}(1 - \xi)(1 + \eta)$$

Starting with derivatives with respect to  $\xi$ :

$$\begin{aligned} \frac{\partial \Phi_0}{\partial \xi} &= -\frac{1}{4}(1 - \eta), & \frac{\partial \Phi_1}{\partial \xi} &= \frac{1}{4}(1 - \eta), \\ \frac{\partial \Phi_2}{\partial \xi} &= \frac{1}{4}(1 + \eta), & \frac{\partial \Phi_3}{\partial \xi} &= -\frac{1}{4}(1 + \eta) \end{aligned}$$

Then for derivatives with respect to  $\eta$ :

$$\begin{aligned} \frac{\partial \Phi_0}{\partial \eta} &= -\frac{1}{4}(1 - \xi), & \frac{\partial \Phi_1}{\partial \eta} &= -\frac{1}{4}(1 + \xi), \\ \frac{\partial \Phi_2}{\partial \eta} &= \frac{1}{4}(1 + \xi), & \frac{\partial \Phi_3}{\partial \eta} &= \frac{1}{4}(1 - \xi) \end{aligned}$$

Therefore, the gradients in reference coordinates are:

$$\begin{aligned} \nabla \Phi_0(\xi, \eta) &= \frac{1}{4} \begin{bmatrix} -(1 - \eta) \\ -(1 - \xi) \end{bmatrix}, & \nabla \Phi_1(\xi, \eta) &= \frac{1}{4} \begin{bmatrix} (1 - \eta) \\ -(1 + \xi) \end{bmatrix} \\ \nabla \Phi_2(\xi, \eta) &= \frac{1}{4} \begin{bmatrix} (1 + \eta) \\ (1 + \xi) \end{bmatrix}, & \nabla \Phi_3(\xi, \eta) &= \frac{1}{4} \begin{bmatrix} -(1 + \eta) \\ (1 - \xi) \end{bmatrix} \end{aligned}$$

So we have the physical gradients:

$$\begin{aligned} \nabla \Phi_0(x, y) &= \frac{1}{2} \begin{bmatrix} -\frac{1}{x_1 - x_0}(1 - \eta) \\ -\frac{1}{y_1 - y_0}(1 - \xi) \end{bmatrix}, & \nabla \Phi_1(x, y) &= \frac{1}{2} \begin{bmatrix} \frac{1}{x_1 - x_0}(1 - \eta) \\ -\frac{1}{y_1 - y_0}(1 + \xi) \end{bmatrix} \\ \nabla \Phi_2(x, y) &= \frac{1}{2} \begin{bmatrix} \frac{1}{x_1 - x_0}(1 + \eta) \\ \frac{1}{y_1 - y_0}(1 + \xi) \end{bmatrix}, & \nabla \Phi_3(x, y) &= \frac{1}{2} \begin{bmatrix} -\frac{1}{x_1 - x_0}(1 + \eta) \\ \frac{1}{y_1 - y_0}(1 - \xi) \end{bmatrix} \end{aligned}$$

## Question 5

Use the following formulas for the elemental mass matrix  $M^{(e)}$ , and stiffness matrix  $K^{(e)}$ :

$$M_{ij}^{(e)} = \int_{\Omega^{(e)}} \Phi_i^{(e)} \Phi_j^{(e)} dx, \quad K_{ij}^{(e)} = \int_{\Omega^{(e)}} \nabla \Phi_i^{(e)} \cdot \nabla \Phi_j^{(e)} dx$$

to evaluate these integrals explicitly for an arbitrary square element in this mesh. Your  $M^{(e)}$  and  $K^{(e)}$  should be  $4 \times 4$  matrices.

*Solution.* For ease of notation let  $\Phi_i^{(e)} = \Phi_i$ .

Let us denote  $\Phi = [\Phi_0, \Phi_1, \Phi_2, \Phi_3]^T$  as the vector of shape functions for element  $e$ . Note that  $M^{(e)}$  can also be expressed as  $M^{(e)} = \int_{\Omega_e} \Phi \cdot \Phi^T dx$ . Using the change of variables from physical to reference coordinates, we have:

$$M^{(e)} = \int_{-1}^1 \int_{-1}^1 \Phi \cdot \Phi^T |\det J(\xi, \eta)| d\xi d\eta$$

where  $|\det J(\xi, \eta)|$  is the absolute value of the determinant of our Jacobian. Let the width of the node be  $w = (x_1 - x_0)$  and the height be  $h = (y_2 - y_1)$ . From question 5, we have:

$$|\det J(\xi, \eta)| = \left| \frac{w}{2} \cdot \frac{h}{2} \right| = \frac{|w| \cdot |h|}{4}$$

When corrected for our local indexing scheme.

Since this constant, we can factor it out of the integral:

$$M^{(e)} = \frac{|w| \cdot |h|}{4} \int_{-1}^1 \int_{-1}^1 \Phi \cdot \Phi^T d\xi d\eta$$

Note that:

$$\begin{aligned} \Phi \cdot \Phi^T &= \begin{bmatrix} \Phi_0 \\ \Phi_1 \\ \Phi_2 \\ \Phi_3 \end{bmatrix} \begin{bmatrix} \Phi_0 & \Phi_1 & \Phi_2 & \Phi_3 \end{bmatrix} \\ &= \begin{bmatrix} \Phi_0\Phi_0 & \Phi_0\Phi_1 & \Phi_0\Phi_2 & \Phi_0\Phi_3 \\ \Phi_1\Phi_0 & \Phi_1\Phi_1 & \Phi_1\Phi_2 & \Phi_1\Phi_3 \\ \Phi_2\Phi_0 & \Phi_2\Phi_1 & \Phi_2\Phi_2 & \Phi_2\Phi_3 \\ \Phi_3\Phi_0 & \Phi_3\Phi_1 & \Phi_3\Phi_2 & \Phi_3\Phi_3 \end{bmatrix} \end{aligned}$$

We will compute some auxiliary integrals with dummy variables first:

$$\int_{-1}^1 (1 \pm z)^2 dz = \pm \frac{(1 \pm z)^3}{3} \Big|_{-1}^1 = \frac{8}{3}$$

$$\int_{-1}^1 (1+z)(1-z) dz = \int_{-1}^1 (1-z^2) dz = z - \frac{z^3}{3} \Big|_{-1}^1 = \frac{4}{3}$$

By the nature of the calculations, the integral matrix will be symmetric. We will show one sample calculation for each main-diagonal and off-diagonal entries. For reference, the shape functions are:

$$\begin{aligned}\Phi_0(\xi, \eta) &= \frac{1}{4}(1 - \xi)(1 - \eta), & \Phi_1(\xi, \eta) &= \frac{1}{4}(1 + \xi)(1 - \eta), \\ \Phi_2(\xi, \eta) &= \frac{1}{4}(1 + \xi)(1 + \eta), & \Phi_3(\xi, \eta) &= \frac{1}{4}(1 - \xi)(1 + \eta)\end{aligned}$$

First note that the product of any pair of shape functions will always have a factor of  $\frac{1}{16}$ . Also note that each product pair will contain two factors in some combination of the forms shown in the auxiliary integrals above.

Case: Main-diagonal entry ( $i = j = 0$ ):

$$\begin{aligned}\int_{-1}^1 \int_{-1}^1 \Phi_0 \Phi_0 \, d\xi d\eta &= \int_{-1}^1 \int_{-1}^1 \frac{1}{16} (1 - \xi)^2 (1 - \eta)^2 \, d\xi d\eta \\ &= \frac{1}{16} \int_{-1}^1 (1 - \eta)^2 \int_{-1}^1 (1 - \xi)^2 \, d\xi d\eta \\ &= \frac{1}{16} \cdot \frac{8}{3} \int_{-1}^1 (1 - \eta)^2 \, d\eta \\ &= \frac{1}{16} \cdot \frac{8}{3} \cdot \frac{8}{3} \\ &= \frac{4}{9}\end{aligned}$$

Case: 1st Off-diagonal entry ( $i = 0, j = 1$ ):

$$\begin{aligned}\int_{-1}^1 \int_{-1}^1 \Phi_0 \Phi_1 \, d\xi d\eta &= \int_{-1}^1 \int_{-1}^1 \frac{1}{16} (1 - \xi)(1 - \eta)(1 + \xi)(1 - \eta) \, d\xi d\eta \\ &= \frac{1}{16} \int_{-1}^1 (1 - \eta)^2 \int_{-1}^1 (1 - \xi^2) \, d\xi d\eta \\ &= \frac{1}{16} \cdot \frac{4}{3} \int_{-1}^1 (1 - \eta)^2 \, d\eta \\ &= \frac{1}{16} \cdot \frac{4}{3} \cdot \frac{8}{3} \\ &= \frac{2}{9}\end{aligned}$$

Case: 2nd Off-diagonal entry ( $i = 0, j = 2$ ):

$$\begin{aligned}\int_{-1}^1 \int_{-1}^1 \Phi_0 \Phi_2 \, d\xi d\eta &= \int_{-1}^1 \int_{-1}^1 \frac{1}{16} (1 - \xi)(1 - \eta)(1 + \xi)(1 + \eta) \, d\xi d\eta \\ &= \frac{1}{16} \int_{-1}^1 (1 - \eta^2) \int_{-1}^1 (1 - \xi^2) \, d\xi d\eta \\ &= \frac{1}{16} \cdot \frac{4}{3} \int_{-1}^1 (1 - \eta^2) \, d\eta \\ &= \frac{1}{16} \cdot \frac{4}{3} \cdot \frac{4}{3} \\ &= \frac{1}{9}\end{aligned}$$

Case: Skew-Diagonal entry ( $i = 0, j = 3$ ):

Same as 1st off-diagonal by symmetry, so the result is  $\frac{2}{9}$ .

Substituting all these results back into the integral matrix, we have:

$$\int_{-1}^1 \int_{-1}^1 \Phi \cdot \Phi^T d\xi d\eta = \frac{1}{9} \begin{bmatrix} 4 & 2 & 1 & 2 \\ 2 & 4 & 2 & 1 \\ 1 & 2 & 4 & 2 \\ 2 & 1 & 2 & 4 \end{bmatrix}$$

Therefore, the elemental mass matrix is:

$$M^{(e)} = \frac{|w| \cdot |h|}{4} \cdot \frac{1}{9} \begin{bmatrix} 4 & 2 & 1 & 2 \\ 2 & 4 & 2 & 1 \\ 1 & 2 & 4 & 2 \\ 2 & 1 & 2 & 4 \end{bmatrix}$$

Next, we compute the elemental stiffness matrix  $K^{(e)}$ . Denote  $\nabla\Phi = [\nabla\Phi_0, \nabla\Phi_1, \nabla\Phi_2, \nabla\Phi_3]$ . Note that  $K^{(e)}$  can also be expressed as:

$$K^{(e)} = \int_{\Omega_e} \nabla\Phi(x, y) \cdot \nabla\Phi^T(x, y) dx$$

Using the change of variables, we have:

$$K^{(e)} = \int_{-1}^1 \int_{-1}^1 (J^{-1}\nabla\Phi(\xi, \eta)) \cdot (J^{-1}\nabla\Phi(\xi, \eta))^T |\det J(\xi, \eta)| d\xi d\eta$$

With the Jacobian determinant factored out, we have:

$$K^{(e)} = \frac{|w| \cdot |h|}{4} \int_{-1}^1 \int_{-1}^1 (J^{-1}\nabla\Phi(\xi, \eta)) \cdot (J^{-1}\nabla\Phi(\xi, \eta))^T d\xi d\eta$$

As reference from question 4, when corrected for our indexing scheme, we have the physical gradients:

$$\begin{aligned} \nabla\Phi_0(x, y) &= \frac{1}{2} \begin{bmatrix} -\frac{1}{w}(1-\eta) \\ -\frac{1}{h}(1-\xi) \end{bmatrix}, & \nabla\Phi_1(x, y) &= \frac{1}{2} \begin{bmatrix} \frac{1}{w}(1-\eta) \\ -\frac{1}{h}(1+\xi) \end{bmatrix} \\ \nabla\Phi_2(x, y) &= \frac{1}{2} \begin{bmatrix} \frac{1}{w}(1+\eta) \\ \frac{1}{h}(1+\xi) \end{bmatrix}, & \nabla\Phi_3(x, y) &= \frac{1}{2} \begin{bmatrix} -\frac{1}{w}(1+\eta) \\ \frac{1}{h}(1-\xi) \end{bmatrix} \end{aligned}$$

so note that  $J^{-1}\nabla\Phi(\xi, \eta) = \frac{1}{2} \cdot 2\nabla\Phi(\xi, \eta)$ . In other words we rewrite each physical gradient above in terms of reference coordinates without the  $\frac{1}{2}$  factor. Until further notice we denote  $\nabla\Phi$  instead of  $2\nabla\Phi$  for ease of notation. Therefore:

$$K^{(e)} = \frac{|w| \cdot |h|}{16} \int_{-1}^1 \int_{-1}^1 \nabla\Phi(\xi, \eta) \cdot \nabla\Phi^T(\xi, \eta) d\xi d\eta$$

Also note that :

$$\nabla\Phi \cdot \nabla\Phi^T = \begin{bmatrix} \nabla\Phi_0 \cdot \nabla\Phi_0 & \nabla\Phi_0 \cdot \nabla\Phi_1 & \nabla\Phi_0 \cdot \nabla\Phi_2 & \nabla\Phi_0 \cdot \nabla\Phi_3 \\ \nabla\Phi_1 \cdot \nabla\Phi_0 & \nabla\Phi_1 \cdot \nabla\Phi_1 & \nabla\Phi_1 \cdot \nabla\Phi_2 & \nabla\Phi_1 \cdot \nabla\Phi_3 \\ \nabla\Phi_2 \cdot \nabla\Phi_0 & \nabla\Phi_2 \cdot \nabla\Phi_1 & \nabla\Phi_2 \cdot \nabla\Phi_2 & \nabla\Phi_2 \cdot \nabla\Phi_3 \\ \nabla\Phi_3 \cdot \nabla\Phi_0 & \nabla\Phi_3 \cdot \nabla\Phi_1 & \nabla\Phi_3 \cdot \nabla\Phi_2 & \nabla\Phi_3 \cdot \nabla\Phi_3 \end{bmatrix}$$

The stiffness matrix is also symmetric, so we will only show one sample calculation for each unique entry.

Case: Main-diagonal entry ( $i = j = 0$ ):

$$\begin{aligned} \int_{-1}^1 \int_{-1}^1 \nabla\Phi_0 \cdot \nabla\Phi_0 \, d\xi d\eta &= \int_{-1}^1 \int_{-1}^1 \left( -\frac{1}{w}(1-\eta) \right)^2 + \left( -\frac{1}{h}(1-\xi) \right)^2 \, d\xi d\eta \\ &= \frac{1}{w^2} \int_{-1}^1 \int_{-1}^1 (1-\eta)^2 \, d\xi d\eta + \frac{1}{h^2} \int_{-1}^1 \int_{-1}^1 (1-\xi)^2 \, d\xi d\eta \\ &= \frac{2}{w^2} \int_{-1}^1 (1-\eta)^2 \, d\eta + \frac{2}{h^2} \int_{-1}^1 (1-\xi)^2 \, d\xi \\ &= \frac{2}{w^2} \cdot \frac{8}{3} + \frac{2}{h^2} \cdot \frac{8}{3} \\ &= \frac{16}{3w^2} + \frac{16}{3h^2} \\ &= \frac{16}{3} \left( \frac{1}{w^2} + \frac{1}{h^2} \right) \end{aligned}$$

Case: Skew-Diagonal entry ( $i = 0, j = 3$ ):

$$\begin{aligned} \int_{-1}^1 \int_{-1}^1 \nabla\Phi_0 \cdot \nabla\Phi_3 \, d\xi d\eta &= \int_{-1}^1 \int_{-1}^1 \frac{1}{w^2}(1-\eta)(1+\eta) - \frac{1}{h^2}(1-\xi)(1-\xi) \, d\xi d\eta \\ &= \frac{1}{w^2} \int_{-1}^1 \int_{-1}^1 (1-\eta^2) \, d\xi d\eta - \frac{1}{h^2} \int_{-1}^1 \int_{-1}^1 (1-\xi)^2 \, d\xi d\eta \\ &= \frac{2}{w^2} \int_{-1}^1 (1-\eta^2) \, d\eta - \frac{2}{h^2} \int_{-1}^1 (1-\xi)^2 \, d\xi \\ &= \frac{2}{w^2} \cdot \frac{4}{3} - \frac{2}{h^2} \cdot \frac{8}{3} \\ &= \frac{8}{3w^2} - \frac{16}{3h^2} \\ &= \frac{8}{3} \left( \frac{1}{w^2} - \frac{2}{h^2} \right) \end{aligned}$$

Case: First Off-Diagonal entry ( $i = 0, j = 1$ ):

$$\begin{aligned}
 \int_{-1}^1 \int_{-1}^1 \nabla \Phi_0 \cdot \nabla \Phi_1 \, d\xi d\eta &= \int_{-1}^1 \int_{-1}^1 -\frac{1}{w^2}(1-\eta)^2 + \frac{1}{h^2}(1-\xi)(1+\xi) \, d\xi d\eta \\
 &= -\frac{1}{w^2} \int_{-1}^1 \int_{-1}^1 (1-\eta)^2 \, d\xi d\eta + \frac{1}{h^2} \int_{-1}^1 \int_{-1}^1 (1-\xi^2) \, d\xi d\eta \\
 &= -\frac{2}{w^2} \int_{-1}^1 (1-\eta)^2 \, d\eta + \frac{2}{h^2} \int_{-1}^1 (1-\xi^2) \, d\xi \\
 &= -\frac{2}{w^2} \cdot \frac{8}{3} + \frac{2}{h^2} \cdot \frac{4}{3} \\
 &= -\frac{16}{3w^2} + \frac{8}{3h^2} \\
 &= -\frac{8}{3} \left( \frac{2}{w^2} - \frac{1}{h^2} \right)
 \end{aligned}$$

Case: Second Off-Diagonal entry ( $i = 0, j = 2$ ):

$$\begin{aligned}
 \int_{-1}^1 \int_{-1}^1 \nabla \Phi_0 \cdot \nabla \Phi_2 \, d\xi d\eta &= \int_{-1}^1 \int_{-1}^1 -\frac{1}{w^2}(1-\eta)(1+\eta) - \frac{1}{h^2}(1-\xi)(1+\xi) \, d\xi d\eta \\
 &= -\frac{1}{w^2} \int_{-1}^1 \int_{-1}^1 (1-\eta^2) \, d\xi d\eta - \frac{1}{h^2} \int_{-1}^1 \int_{-1}^1 (1-\xi^2) \, d\xi d\eta \\
 &= -\frac{2}{w^2} \int_{-1}^1 (1-\eta^2) \, d\eta - \frac{2}{h^2} \int_{-1}^1 (1-\xi^2) \, d\xi \\
 &= -\frac{2}{w^2} \cdot \frac{4}{3} - \frac{2}{h^2} \cdot \frac{4}{3} \\
 &= -\frac{8}{3w^2} - \frac{8}{3h^2} \\
 &= -\frac{8}{3} \left( \frac{1}{w^2} + \frac{1}{h^2} \right)
 \end{aligned}$$

Now let us compute the unique elemental and stiffness matrices for our mesh using our formulas above.

There will be 3 unique element mass matrices for the following combinations of width  $w = (x_1 - x_0)$  and height  $h = (y_2 - y_1)$ :

1. Element with width 0.4 and height 0.4
2. Element with width 0.4 and height 2.8 or vice versa
3. Element with width 2.8 and height 2.8

Case 1: Element with width 0.4 and height 0.4

$$M^{(e)} = \frac{\left(\frac{2}{5}\right)^2}{4} \cdot \frac{1}{9} \begin{bmatrix} 4 & 2 & 1 & 2 \\ 2 & 4 & 2 & 1 \\ 1 & 2 & 4 & 2 \\ 2 & 1 & 2 & 4 \end{bmatrix} = \frac{1}{225} \begin{bmatrix} 4 & 2 & 1 & 2 \\ 2 & 4 & 2 & 1 \\ 1 & 2 & 4 & 2 \\ 2 & 1 & 2 & 4 \end{bmatrix}$$

Case 2: Element with width 0.4 and height 2.8

$$M^{(e)} = \frac{\left(\frac{2}{5}\right) \cdot \left(\frac{14}{5}\right)}{4} \cdot \frac{1}{9} \begin{bmatrix} 4 & 2 & 1 & 2 \\ 2 & 4 & 2 & 1 \\ 1 & 2 & 4 & 2 \\ 2 & 1 & 2 & 4 \end{bmatrix} = \frac{7}{225} \begin{bmatrix} 4 & 2 & 1 & 2 \\ 2 & 4 & 2 & 1 \\ 1 & 2 & 4 & 2 \\ 2 & 1 & 2 & 4 \end{bmatrix}$$

Case 3: Element with width 2.8 and height 2.8

$$M^{(e)} = \frac{\left(\frac{14}{5}\right)^2}{4} \cdot \frac{1}{9} \begin{bmatrix} 4 & 2 & 1 & 2 \\ 2 & 4 & 2 & 1 \\ 1 & 2 & 4 & 2 \\ 2 & 1 & 2 & 4 \end{bmatrix} = \frac{49}{225} \begin{bmatrix} 4 & 2 & 1 & 2 \\ 2 & 4 & 2 & 1 \\ 1 & 2 & 4 & 2 \\ 2 & 1 & 2 & 4 \end{bmatrix}$$

There will be 3 unique element stiffness matrices for the following combinations of width  $w = (x_1 - x_0)$  and height  $h = (y_2 - y_1)$ :

1. Square element with width 0.4 and height 0.4 or width 2.8 and height 2.8
2. Element with width 0.4 and height 2.8
3. Element with width 2.8 and height 0.4

Case 1: Square element (ex. width 0.4 and height 0.4)

Main diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot \frac{16}{3} \left( \frac{1}{w^2} + \frac{1}{h^2} \right) = \frac{\left(\frac{2}{5}\right)^2}{16} \cdot \frac{16}{3} \left( \frac{1}{\left(\frac{2}{5}\right)^2} + \frac{1}{\left(\frac{2}{5}\right)^2} \right) = \frac{2}{3} = \frac{4}{6}$$

Skew diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot \frac{8}{3} \left( \frac{1}{w^2} - \frac{2}{h^2} \right) = \frac{\left(\frac{2}{5}\right)^2}{16} \cdot \frac{8}{3} \left( \frac{1}{\left(\frac{2}{5}\right)^2} - \frac{2}{\left(\frac{2}{5}\right)^2} \right) = -\frac{1}{6}$$

First off-diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot -\frac{8}{3} \left( \frac{2}{w^2} - \frac{1}{h^2} \right) = \frac{\left(\frac{2}{5}\right)^2}{16} \cdot -\frac{8}{3} \left( \frac{2}{\left(\frac{2}{5}\right)^2} - \frac{1}{\left(\frac{2}{5}\right)^2} \right) = -\frac{1}{6}$$

Second off-diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot -\frac{8}{3} \left( \frac{1}{w^2} + \frac{1}{h^2} \right) = \frac{\left(\frac{2}{5}\right)^2}{16} \cdot -\frac{8}{3} \left( \frac{1}{\left(\frac{2}{5}\right)^2} + \frac{1}{\left(\frac{2}{5}\right)^2} \right) = -\frac{1}{3} = -\frac{2}{6}$$

Therefore the elemental stiffness matrix is:

$$K^{(e)} = \frac{1}{6} \begin{bmatrix} 4 & -1 & -2 & -1 \\ -1 & 4 & -1 & -2 \\ -2 & -1 & 4 & -1 \\ -1 & -2 & -1 & 4 \end{bmatrix}$$



Case 2: Element with width 0.4 and height 2.8

Main diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot \frac{16}{3} \left( \frac{1}{w^2} + \frac{1}{h^2} \right) = \frac{(\frac{2}{5}) \cdot (\frac{14}{5})}{16} \cdot \frac{16}{3} \left( \frac{1}{(\frac{2}{5})^2} + \frac{1}{(\frac{14}{5})^2} \right) = \frac{50}{21} = \frac{100}{42}$$

Skew-diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot \frac{8}{3} \left( \frac{1}{w^2} - \frac{2}{h^2} \right) = \frac{(\frac{2}{5}) \cdot (\frac{14}{5})}{16} \cdot \frac{8}{3} \left( \frac{1}{(\frac{2}{5})^2} - \frac{2}{(\frac{14}{5})^2} \right) = \frac{47}{42}$$

First off-diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot -\frac{8}{3} \left( \frac{2}{w^2} - \frac{1}{h^2} \right) = \frac{(\frac{2}{5}) \cdot (\frac{3}{5})}{16} \cdot -\frac{8}{3} \left( \frac{2}{(\frac{2}{5})^2} - \frac{1}{(\frac{14}{5})^2} \right) = -\frac{97}{42}$$

Second off-diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot -\frac{8}{3} \left( \frac{1}{w^2} + \frac{1}{h^2} \right) = \frac{(\frac{2}{5}) \cdot (\frac{3}{5})}{16} \cdot -\frac{8}{3} \left( \frac{1}{(\frac{2}{5})^2} + \frac{1}{(\frac{14}{5})^2} \right) = -\frac{25}{21} = -\frac{50}{42}$$

Therefore the elemental stiffness matrix is:

$$K^{(e)} = \frac{1}{42} \begin{bmatrix} 100 & -97 & -50 & 47 \\ -97 & 100 & 47 & -50 \\ -50 & 47 & 100 & -97 \\ 47 & -50 & -97 & 100 \end{bmatrix}$$

Case 3: Element with width 2.8 and height 0.4

Main diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot \frac{16}{3} \left( \frac{1}{w^2} + \frac{1}{h^2} \right) = \frac{(\frac{14}{5}) \cdot (\frac{2}{5})}{16} \cdot \frac{16}{3} \left( \frac{1}{(\frac{14}{5})^2} + \frac{1}{(\frac{2}{5})^2} \right) = \frac{50}{21} = \frac{100}{42}$$

Skew-diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot \frac{8}{3} \left( \frac{1}{w^2} - \frac{2}{h^2} \right) = \frac{(\frac{14}{5}) \cdot (\frac{2}{5})}{16} \cdot \frac{8}{3} \left( \frac{1}{(\frac{14}{5})^2} - \frac{2}{(\frac{2}{5})^2} \right) = -\frac{97}{42}$$

First off-diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot -\frac{8}{3} \left( \frac{2}{w^2} - \frac{1}{h^2} \right) = \frac{(\frac{14}{5}) \cdot (\frac{2}{5})}{16} \cdot -\frac{8}{3} \left( \frac{2}{(\frac{14}{5})^2} - \frac{1}{(\frac{2}{5})^2} \right) = \frac{47}{42}$$

Second off-diagonal entry:

$$\frac{|w| \cdot |h|}{16} \cdot -\frac{8}{3} \left( \frac{1}{w^2} + \frac{1}{h^2} \right) = \frac{(\frac{14}{5}) \cdot (\frac{2}{5})}{16} \cdot -\frac{8}{3} \left( \frac{1}{(\frac{14}{5})^2} + \frac{1}{(\frac{2}{5})^2} \right) = -\frac{25}{21} = -\frac{50}{42}$$

Therefore the elemental stiffness matrix is:

$$K^{(e)} = \frac{1}{42} \begin{bmatrix} 100 & 47 & -50 & -97 \\ 47 & 100 & -97 & -50 \\ -50 & -97 & 100 & 47 \\ -97 & -50 & 47 & 100 \end{bmatrix}$$

## Question 6

Assemble the global mass matrix  $M$  and global stiffness matrix  $K$  for the entire  $2 \times 2$  mesh using the connectivity determined in question 2. Impose homogeneous Dirichlet boundary conditions on all boundary nodes. Write the resulting discretized ODE system in the following format:

$$M \frac{dU}{dt} + KU = F(t)$$

*Solution.*

We will adjust the indexing in the preliminary setup to match this scenario. From before we had:

$$\sum_{i,j=1}^N \left( \int_{\Omega} \Phi_i \Phi_j \, dx \right) \frac{dU_i}{dt} + \nu \sum_{i,j=1}^N \left( \int_{\Omega} \nabla \Phi_i \cdot \nabla \Phi_j \, dx \right) U_i = \sum_{j=1}^N \int_{\Omega} f \Phi_j \, dx$$

Was our global discretized system, where  $N$  is the total number of nodes. Rewriting this terms of the elemental components, we have:

$$\left( \sum_{e=1}^E M^{(e)} \right) \frac{dU}{dt} + \nu \left( \sum_{e=1}^E K^{(e)} \right) U = \sum_{e=1}^E F^{(e)}(t)$$

Where  $E$  is the total number of elements.

Correcting for our indexing scheme, we have:

$$\left( \sum_{e=0}^{15} M^{(e)} \right) \frac{dU}{dt} + \nu \left( \sum_{e=0}^{15} K^{(e)} \right) U = \sum_{e=0}^{15} F^{(e)}(t)$$

Such that that  $M = \sum_{e=0}^{15} M^{(e)}$  and  $K = \sum_{e=0}^{15} K^{(e)}$ . We could lump the  $\nu$  into  $K$  to get the form:

$$M \frac{dU}{dt} + KU = F(t)$$

Also, since we have the homogeneous heat equation,  $F^{(e)}(t) = 0$  for all  $e$ .

In addition to methods defined in Question 2, the following code was used to assemble our global matrices:

Listing 2: Question 6 Python

```

1 import numpy as np
2 import sympy as sp
3
4 def element_mass_matrix(w, h):
5     area = w * h
6     Me = (area/36) * np.array([[4, 2, 1, 2],
7                                [2, 4, 2, 1],

```

```
8             [1, 2, 4, 2],
9             [2, 1, 2, 4]])
10
11     return Me
12
13 def element_stiffness_matrix(w, h):
14     # When multiplying each entry formula by w * h /16, we get the following
15     reduced formulas:
16     wh = w/h
17     hw = h/w
18     # Main diagonal
19     a = (1/3) * (hw + wh)
20     # Skew diagonal
21     b = (1/6) * (hw - 2*wh)
22     # First off-diagonal
23     c = -(1/6) * (2*hw - wh)
24     # Second off-diagonal
25     d = -(1/6) * (hw + wh)
26     # Construct element stiffness matrix
27     Ke = np.array([[a, c, d, b],
28                   [c, a, b, d],
29                   [d, b, a, c],
30                   [b, d, c, a]])
31
32     return Ke
33
34 def generate_global_coordinates(x_nodes, y_nodes=None):
35     if y_nodes is None:
36         y_nodes = x_nodes
37
38     y_nodes = y_nodes[::-1]
39     global_coordinates = []
40     for y in y_nodes:
41         for x in x_nodes:
42             global_coordinates.append([x, y])
43     global_coordinates = np.array(global_coordinates)
44     return global_coordinates
45
46 def global_assembly(x_nodes, y_nodes=None):
47     if y_nodes is None:
48         y_nodes = x_nodes
49
50     global_coordinates = generate_global_coordinates(x_nodes, y_nodes)
51
52     width = len(x_nodes)
53     height = len(y_nodes)
54
55     global_indices = global_indexing(width, height)
```

```

54     connectivity_matrix = generate_connectivity_matrix(global_indices)
55
56     num_nodes = width * height
57     M_global = np.zeros((num_nodes, num_nodes))
58     K_global = np.zeros((num_nodes, num_nodes))
59
60     for element in connectivity_matrix:
61
62         # Bottom-left (node 0)
63         x0 = global_coordinates[element[0], 0]
64         y0 = global_coordinates[element[0], 1]
65
66         # Bottom-right (node 1)
67         x1 = global_coordinates[element[1], 0]
68
69         # Top-left (node 3)
70         y3 = global_coordinates[element[3], 1]
71
72         # Element width and height
73         w = abs(x1 - x0)
74         h = abs(y3 - y0)
75
76         # Get element matrices
77         Me = element_mass_matrix(w, h)
78         Ke = element_stiffness_matrix(w, h)
79
80         # Add element contributions to global matrices
81         for i_local in range(4):
82             i_global = element[i_local]
83             for j_local in range(4):
84                 j_global = element[j_local]
85                 M_global[i_global, j_global] += Me[i_local, j_local]
86                 K_global[i_global, j_global] += Ke[i_local, j_local]
87
88     return global_coordinates, M_global, K_global
89
90 if __name__ == "__main__":
91     x_nodes = np.array([-2, -1.6, -1.2, -0.8, 2])
92     _, M, K = global_assembly(x_nodes)
93
94     with open("./outputs_4/matrices.txt", "w") as f:
95         M = np.round(M, 4)
96         M_split = np.array_split(M, 3, axis=1)
97         latex_1_matrix = sp.latex(sp.Matrix(M_split[0]))
98         latex_2_matrix = sp.latex(sp.Matrix(M_split[1]))
99         latex_3_matrix = sp.latex(sp.Matrix(M_split[2]))
100        f.write("Mass Matrix (Part 1):\n")

```

```
101     f.write(latex_1_matrix + "\n\n")
102     f.write("Mass Matrix (Part 2):\n")
103     f.write(latex_2_matrix + "\n\n")
104     f.write("Mass Matrix (Part 3):\n")
105     f.write(latex_3_matrix + "\n\n")
106
107     K = np.round(K, 4)
108     K_split = np.array_split(K, 3, axis=1)
109     latex_1_matrix = sp.latex(sp.Matrix(K_split[0]))
110     latex_2_matrix = sp.latex(sp.Matrix(K_split[1]))
111     latex_3_matrix = sp.latex(sp.Matrix(K_split[2]))
112     f.write("Stiffness Matrix (Part 1):\n")
113     f.write(latex_1_matrix + "\n\n")
114     f.write("Stiffness Matrix (Part 2):\n")
115     f.write(latex_2_matrix + "\n\n")
116     f.write("Stiffness Matrix (Part 3):\n")
117     f.write(latex_3_matrix + "\n\n")
```

The code was constructed such that non-uniform meshes of arbitrary widths and heights could be used.

Our mass matrix  $M$  will be of size  $25 \times 25$  and was found to be:

$$\begin{bmatrix}
 0.1244 & 0.0622 & 0.0 & 0.0 & 0.0 & 0.0622 & 0.0311 & 0.0 & 0.0 \\
 0.0622 & 0.2489 & 0.0622 & 0.0 & 0.0 & 0.0311 & 0.1244 & 0.0311 & 0.0 \\
 0.0 & 0.0622 & 0.2489 & 0.0622 & 0.0 & 0.0 & 0.0311 & 0.1244 & 0.0311 \\
 0.0 & 0.0 & 0.0622 & 0.9956 & 0.4356 & 0.0 & 0.0 & 0.0311 & 0.4978 \\
 0.0 & 0.0 & 0.0 & 0.4356 & 0.8711 & 0.0 & 0.0 & 0.0 & 0.2178 \\
 0.0622 & 0.0311 & 0.0 & 0.0 & 0.0 & 0.1422 & 0.0711 & 0.0 & 0.0 \\
 0.0311 & 0.1244 & 0.0311 & 0.0 & 0.0 & 0.0711 & 0.2844 & 0.0711 & 0.0 \\
 0.0 & 0.0311 & 0.1244 & 0.0311 & 0.0 & 0.0 & 0.0711 & 0.2844 & 0.0711 \\
 0.0 & 0.0 & 0.0311 & 0.4978 & 0.2178 & 0.0 & 0.0 & 0.0711 & 1.1378 \\
 0.0 & 0.0 & 0.0 & 0.2178 & 0.4356 & 0.0 & 0.0 & 0.0 & 0.4978 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0089 & 0.0044 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0044 & 0.0178 & 0.0044 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0044 & 0.0178 & 0.0044 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0044 & 0.0711 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0311 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0
 \end{bmatrix} \dots$$

	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.2178	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.4356	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0089	0.0044	0.0	0.0	0.0	0.0	0.0
	0.0	0.0044	0.0178	0.0044	0.0	0.0	0.0	0.0
	0.0	0.0	0.0044	0.0178	0.0044	0.0	0.0	0.0
	0.4978	0.0	0.0	0.0044	0.0711	0.0311	0.0	0.0
	0.9956	0.0	0.0	0.0	0.0311	0.0622	0.0	0.0
	0.0	0.0356	0.0178	0.0	0.0	0.0	0.0089	0.0044
	0.0	0.0178	0.0711	0.0178	0.0	0.0	0.0044	0.0178
...	0.0	0.0	0.0178	0.0711	0.0178	0.0	0.0	0.0044
	0.0311	0.0	0.0	0.0178	0.2844	0.1244	0.0	0.0
	0.0622	0.0	0.0	0.0	0.1244	0.2489	0.0	0.0
	0.0	0.0089	0.0044	0.0	0.0	0.0	0.0356	0.0178
	0.0	0.0044	0.0178	0.0044	0.0	0.0	0.0178	0.0711
	0.0	0.0	0.0044	0.0178	0.0044	0.0	0.0	0.0178
	0.0	0.0	0.0	0.0044	0.0711	0.0311	0.0	0.0
	0.0	0.0	0.0	0.0	0.0311	0.0622	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0089	0.0044
	0.0	0.0	0.0	0.0	0.0	0.0	0.0044	0.0178
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0044
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

$$\begin{array}{c}
 \dots \\
 \left[ \begin{array}{cccccccc}
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0044 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0178 & 0.0044 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0044 & 0.0711 & 0.0311 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0311 & 0.0622 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0089 & 0.0044 & 0.0 & 0.0 & 0.0 \\
 0.0178 & 0.0 & 0.0 & 0.0044 & 0.0178 & 0.0044 & 0.0 & 0.0 \\
 0.0711 & 0.0178 & 0.0 & 0.0 & 0.0044 & 0.0178 & 0.0044 & 0.0 \\
 0.0178 & 0.2844 & 0.1244 & 0.0 & 0.0 & 0.0044 & 0.0711 & 0.0311 \\
 0.0 & 0.1244 & 0.2489 & 0.0 & 0.0 & 0.0 & 0.0311 & 0.0622 \\
 0.0 & 0.0 & 0.0 & 0.0178 & 0.0089 & 0.0 & 0.0 & 0.0 \\
 0.0044 & 0.0 & 0.0 & 0.0089 & 0.0356 & 0.0089 & 0.0 & 0.0 \\
 0.0178 & 0.0044 & 0.0 & 0.0 & 0.0089 & 0.0356 & 0.0089 & 0.0 \\
 0.0044 & 0.0711 & 0.0311 & 0.0 & 0.0 & 0.0089 & 0.1422 & 0.0622 \\
 0.0 & 0.0311 & 0.0622 & 0.0 & 0.0 & 0.0 & 0.0622 & 0.1244
 \end{array} \right]
 \end{array}$$



Our stiffness matrix  $K$  will also be of size  $25 \times 25$  and was found to be (without  $\nu$ ):

$$\begin{bmatrix}
 2.381 & -2.3095 & 0.0 & 0.0 & 0.0 & 1.119 & -1.1905 & 0.0 & 0.0 \\
 -2.3095 & 4.7619 & -2.3095 & 0.0 & 0.0 & -1.1905 & 2.2381 & -1.1905 & 0.0 \\
 0.0 & -2.3095 & 4.7619 & -2.3095 & 0.0 & 0.0 & -1.1905 & 2.2381 & -1.1905 \\
 0.0 & 0.0 & -2.3095 & 3.0476 & -0.1667 & 0.0 & 0.0 & -1.1905 & 0.9524 \\
 0.0 & 0.0 & 0.0 & -0.1667 & 0.6667 & 0.0 & 0.0 & 0.0 & -0.3333 \\
 1.119 & -1.1905 & 0.0 & 0.0 & 0.0 & 3.0476 & -2.4762 & 0.0 & 0.0 \\
 -1.1905 & 2.2381 & -1.1905 & 0.0 & 0.0 & -2.4762 & 6.0952 & -2.4762 & 0.0 \\
 0.0 & -1.1905 & 2.2381 & -1.1905 & 0.0 & 0.0 & -2.4762 & 6.0952 & -2.4762 \\
 0.0 & 0.0 & -1.1905 & 0.9524 & -0.3333 & 0.0 & 0.0 & -2.4762 & 6.0952 \\
 0.0 & 0.0 & 0.0 & -0.3333 & -0.1667 & 0.0 & 0.0 & 0.0 & 0.9524 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -0.1667 & -0.3333 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -0.3333 & -0.3333 & -0.3333 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -0.3333 & -0.3333 & -0.3333 \dots \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -0.3333 & -2.4762 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -1.1905 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0
 \end{bmatrix}$$

$$\begin{array}{cccccccc}
 & \left[ \begin{array}{cccccccc}
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 -0.3333 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 -0.1667 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & -0.1667 & -0.3333 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & -0.3333 & -0.3333 & -0.3333 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & -0.3333 & -0.3333 & -0.3333 & 0.0 & 0.0 & 0.0 \\
 0.9524 & 0.0 & 0.0 & -0.3333 & -2.4762 & -1.1905 & 0.0 & 0.0 \\
 3.0476 & 0.0 & 0.0 & 0.0 & -1.1905 & -2.3095 & 0.0 & 0.0 \\
 0.0 & 1.3333 & -0.3333 & 0.0 & 0.0 & 0.0 & -0.1667 & -0.3333 \\
 0.0 & -0.3333 & 2.6667 & -0.3333 & 0.0 & 0.0 & -0.3333 & -0.3333 \\
 \dots & 0.0 & 0.0 & -0.3333 & 2.6667 & -0.3333 & 0.0 & 0.0 & -0.3333 & \dots \\
 -1.1905 & 0.0 & 0.0 & -0.3333 & 6.0952 & 2.2381 & 0.0 & 0.0 \\
 -2.3095 & 0.0 & 0.0 & 0.0 & 2.2381 & 4.7619 & 0.0 & 0.0 \\
 0.0 & -0.1667 & -0.3333 & 0.0 & 0.0 & 0.0 & 1.3333 & -0.3333 \\
 0.0 & -0.3333 & -0.3333 & -0.3333 & 0.0 & 0.0 & -0.3333 & 2.6667 \\
 0.0 & 0.0 & -0.3333 & -0.3333 & -0.3333 & 0.0 & 0.0 & -0.3333 \\
 0.0 & 0.0 & 0.0 & -0.3333 & -2.4762 & -1.1905 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & -1.1905 & -2.3095 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -0.1667 & -0.3333 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -0.3333 & -0.3333 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & -0.3333 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0
 \end{array} \right]
 \end{array}$$

$$\begin{bmatrix}
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 -0.3333 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 -0.3333 & -0.3333 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 -0.3333 & -2.4762 & -1.1905 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & -1.1905 & -2.3095 & 0.0 & 0.0 & 0.0 & 0.0 & 0.0 \\
 0.0 & 0.0 & 0.0 & -0.1667 & -0.3333 & 0.0 & 0.0 & 0.0 \\
 -0.3333 & 0.0 & 0.0 & -0.3333 & -0.3333 & -0.3333 & 0.0 & 0.0 \\
 2.6667 & -0.3333 & 0.0 & 0.0 & -0.3333 & -0.3333 & -0.3333 & 0.0 \\
 -0.3333 & 6.0952 & 2.2381 & 0.0 & 0.0 & -0.3333 & -2.4762 & -1.1905 \\
 0.0 & 2.2381 & 4.7619 & 0.0 & 0.0 & 0.0 & -1.1905 & -2.3095 \\
 0.0 & 0.0 & 0.0 & 0.6667 & -0.1667 & 0.0 & 0.0 & 0.0 \\
 -0.3333 & 0.0 & 0.0 & -0.1667 & 1.3333 & -0.1667 & 0.0 & 0.0 \\
 -0.3333 & -0.3333 & 0.0 & 0.0 & -0.1667 & 1.3333 & -0.1667 & 0.0 \\
 -0.3333 & -2.4762 & -1.1905 & 0.0 & 0.0 & -0.1667 & 3.0476 & 1.119 \\
 0.0 & -1.1905 & -2.3095 & 0.0 & 0.0 & 0.0 & 1.119 & 2.381
 \end{bmatrix} \dots$$

As expected, both  $M$  and  $K$  are symmetric, banded matrices.

## Question 7

Using the following initial condition

$$u(x, y, 0) = \sin(2\pi x) \sin(2\pi y)$$

to solve the reduced ODE system from  $t = 0$  to  $t = 1$ . (Show Code)

*Solution.* Given our system:

$$M \frac{dU}{dt} + \nu KU = 0$$

We can rearrange this to get:

$$\frac{dU}{dt} = -\nu M^{-1} KU$$

### Implicit: Backward Euler

Let us find  $U^{n+1}$  implicitly using backward difference in time:

$$\frac{U^{n+1} - U^n}{\Delta t} = -\nu M^{-1} KU^{n+1}$$

Rearranging for  $U^{n+1}$ , we have:

$$(I + \nu \Delta t M^{-1} K) U^{n+1} = U^n$$

The following code was used to implement the implicit backward Euler method:

Listing 3: implicit\_euler.py

```

1 import numpy as np
2 def classify_boundary_nodes(global_indexing):
3     # Create a boundary indicator array where 1 indicates a boundary node
4     boundary_indicator = np.zeros_like(global_indexing)
5     boundary_indicator[0, :] = 1 # Top boundary
6     boundary_indicator[-1, :] = 1 # Bottom boundary
7     boundary_indicator[:, 0] = 1 # Left boundary
8     boundary_indicator[:, -1] = 1 # Right boundary
9     return boundary_indicator.flatten()
10
11 def implement_dirichlet_bc(M, K, boundary_indicator):
12     num_nodes = M.shape[0]
13     for i in range(num_nodes):
14         if boundary_indicator[i] == 1:
15             M[i, :] = 0
16             M[:, i] = 0
17             M[i, i] = 1
18             K[i, :] = 0
19             K[:, i] = 0

```

```
20         K[i, i] = 1
21     return M, K
22
23 def u_0(coordinates, boundary_indicator):
24     x = coordinates[:, 0]
25     y = coordinates[:, 1]
26     u = np.sin(2*np.pi*x) * np.sin(2*np.pi*y)
27     u[boundary_indicator == 1] = 0.0 # Apply Dirichlet BCs
28     num_nodes = coordinates.shape[0]
29     return u.reshape((num_nodes,1))
30
31 def implicit_heat_solver(x_nodes, dt, t_final, nu=0.05):
32     global_coordinates, M, K = global_assembly(x_nodes)
33     width = len(x_nodes)
34     global_indexing_array = global_indexing(width)
35     boundary_indicator = classify_boundary_nodes(global_indexing_array)
36
37     M, K = implement_dirichlet_bc(M, K, boundary_indicator)
38
39     A = M + nu * dt * K
40
41     num_time_steps = math.ceil(t_final / dt)
42     dt = t_final / num_time_steps
43     prev_U = u_0(global_coordinates, boundary_indicator)
44     U = np.array([prev_U])
45
46     for n in range(num_time_steps):
47         b = np.matmul(M, U[n])
48         next_U = np.linalg.solve(A, b)
49         U = np.append(U, [next_U], axis=0)
50
51     return boundary_indicator, U.T, global_coordinates
```

## Question 8

Write your own solver for solving linear systems. You can freely choose the methods from Gaussian, Jacobi, or Gauss-Seidel.

*Solution.*

Jacobi is defined iteratively as:

$$x_{n+1} = D^{-1}(-R \cdot x_n + b)$$

Where  $D$  is the diagonal of  $A$  and  $R = A - D$  is the remainder matrix. We will use an initial guess of  $x_0 = 0$  and iterate until tolerance is met (with  $l_\infty$  norm) or maximum iterations are reached.

The following code was used to implement the Jacobi method:

Listing 4: Jacobi Method

```

1 import numpy as np
2
3 def jacobi(A, b, x0=None, tol=1e-12, max_iterations=10000):
4     x_prev = np.zeros_like(b) if x0 is None else x0.copy()
5     diagonal_list = np.diag(A)
6     D = np.diagflat(diagonal_list)
7     R = A - D
8     D_inverse = np.linalg.inv(D)
9
10    for _ in range(max_iterations):
11        rx_b = -np.dot(R, x_prev) + b
12        x_next = np.dot(D_inverse, rx_b)
13        if np.linalg.norm(x_next - x_prev, ord=np.inf) < tol:
14            return x_next
15        x_prev = x_next
16
17    raise ValueError("Jacobi method did not converge within the maximum number
    of iterations")

```

*Extra:* See Appendix for past implementations of Gaussian Elimination, Jacobi, and Gauss-Seidel in Maple.

## Question 9

Perform a convergence study with different refinements on time steps. Plot the log-log plot of error vs time step size.

*Solution.* The following code was used to perform the convergence study:

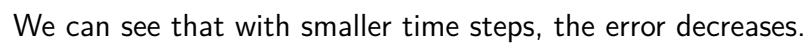
Listing 5: Convergence Study

```

1 import numpy as np
2 import matplotlib.pyplot as plt
3 def convergence_study_timesteps(x_nodes, t_final, dt_values, y_nodes=None):
4     if y_nodes is None:
5         y_nodes = x_nodes
6     errors_implicit = []
7     global_coordinates, _, _ = global_assembly(x_nodes, y_nodes)
8     width = len(x_nodes)
9     global_indexing_array = global_indexing(width)
10    boundary = classify_boundary_nodes(global_indexing_array)
11    x_coords = global_coordinates[:, 0].reshape(width**2, 1)
12    y_coords = global_coordinates[:, 1].reshape(width**2, 1)
13    u_exact_values = u_exact(x_coords, y_coords, t_final, boundary)
14
15    for dt in dt_values:
16        _, U, _ = implicit_heat_solver(x_nodes, dt, t_final)
17        u_numerical_values = U[:, :, -1]
18        error = np.linalg.norm(u_numerical_values - u_exact_values, ord=2)
19        errors_implicit.append(error)
20
21    return errors_implicit
22
23 if __name__ == "__main__":
24     # <some matplotlib styling and enable latex>
25     x_nodes = np.array([-2, -1.6, -1.2, -0.8, 2])
26     t_final = 1
27     dt_values = np.logspace(-1, -10, 10, base=2)
28     errors_implicit = convergence_study_timesteps(x_nodes, t_final, dt_values)
29     fig, ax = plt.subplots(figsize=(9,6))
30     ax.loglog(dt_values, errors_implicit, marker='x', label='Implicit')
31     ax.set_xlabel(r'$\log(\Delta t)$')
32     ax.set_ylabel(r'$\log(\|u - u_{\text{exact}}\|_2)$')
33     ax.set_title('Convergence Study for Implicit Solver')
34     ax.legend()
35     fig.savefig("./outputs_4/implicit_convergence_study.png", dpi=300)

```

For efficiency, we use numpy's built in linear algebra. We get the resulting error plot below:

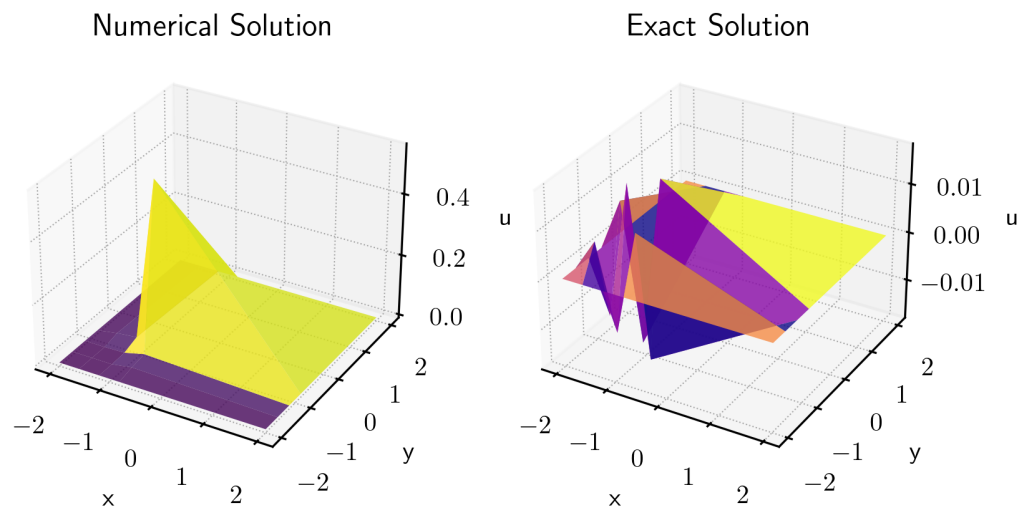




## Question 10

Plot  $U(t)$  at  $T = 1$ .

*Solution.* The following plot was generated using the implicit solver at  $t = 1$ :



## Appendix

### 1 Maple Implementations of Linear Solvers

These are implementations of Gaussian Elimination, Jacobi Method, and Gauss-Seidel Method in Maple I have done in the past.

These were the questions being answered:

3. Use  $LU$  factorization to solve the system:

$$\begin{aligned} 6x_1 - 2x_2 + 2x_3 + 4x_4 &= 16 \\ 12x_1 - 8x_2 + 6x_3 + 10x_4 &= 26 \\ 3x_1 - 13x_2 + 9x_3 + 3x_4 &= -19 \\ -6x_1 + 4x_2 + x_3 - 18x_4 &= -34 \end{aligned}$$

Be sure to state the matrices  $L$  and  $U$ .

4. Use the Jacobi iterative method and the Gauss-Seidel iterative method to find the solution to the following set of equations within  $10^{-4}$  in the  $\ell_\infty$  norm using  $\mathbf{x}^{(0)} = \mathbf{0}$  as your initial condition. Show theoretically whether or not both methods will converge in this case.

$$\begin{aligned} 4x_1 + x_2 + x_3 + x_4 &= -5 \\ x_1 + 8x_2 + 2x_3 + 3x_4 &= 23 \\ x_1 + 2x_2 - 5x_3 &= 9 \\ -x_1 + 2x_3 + 4x_4 &= 4 \end{aligned}$$

The following pages contain the Maple implementations and sample outputs.

Note: the Matrix Solver is Gaussian Elimination with addition of LU decomposition. No row exchanges were implemented.

## **Matrix Solver**

*with*(LinearAlgebra) :

*MatrixSolve* := **proc**(*A*, *b*)

**local** *i*, *j*, *n*, *L*, *U*, *v*, *const*, *det*;

$n := \text{RowDimension}(A)$ ;

$L := \text{Matrix}(n)$ ;

$U := A$ ;

$v := b$ ;

$det := 1$ ;

$\text{print}(A, b)$ ;

**for** *i* **from** 1 **to**  $n - 1$  **do**

**for** *j* **from**  $i + 1$  **to**  $n$  **do**

**if** ( $U[i, i] = 0$ ) **then**

**error** "zero along main diagonal";

**end if**;

$const := \frac{U[j, i]}{U[i, i]}$ ;

**if** ( $const \neq 0$ ) **then**

$L[j, i] := const$ ;

$U := \text{RowOperation}(U, [j, i], -const)$ ;

$v[j] := v[j] - const \cdot v[i]$ ;

$\text{print}(R(j) - const \cdot R(i), U, v)$ ;

**end if**;

**end do**;

**end do**;

**for** *i* **from** 1 **to**  $n$  **do**

$det := det \cdot U[i, i]$ ;

$L[i, i] := 1$ ;

**end do**;

$\text{print}(L, U, v, det)$ ;

**end proc**:

③ **Problem #3**

$A := \text{Matrix}([ [6, -2, 2, 4], [12, -8, 6, 10], [3, -13, 9, 3], [-6, 4, 1, -18] ])$ ;

$b := \text{Vector}([16, 26, -19, -34])$ ;

$$A := \begin{bmatrix} 6 & -2 & 2 & 4 \\ 12 & -8 & 6 & 10 \\ 3 & -13 & 9 & 3 \\ -6 & 4 & 1 & -18 \end{bmatrix}$$

$$b := \begin{bmatrix} 16 \\ 26 \\ -19 \\ -34 \end{bmatrix}$$

(6)

$\text{MatrixSolve}(A, b)$ ;

$$\begin{bmatrix} 6 & -2 & 2 & 4 \\ 12 & -8 & 6 & 10 \\ 3 & -13 & 9 & 3 \\ -6 & 4 & 1 & -18 \end{bmatrix}, \begin{bmatrix} 16 \\ 26 \\ -19 \\ -34 \end{bmatrix}$$

$$R(2) - 2R(1), \begin{bmatrix} 6 & -2 & 2 & 4 \\ 0 & -4 & 2 & 2 \\ 3 & -13 & 9 & 3 \\ -6 & 4 & 1 & -18 \end{bmatrix}, \begin{bmatrix} 16 \\ -6 \\ -19 \\ -34 \end{bmatrix}$$

$$R(3) - \frac{R(1)}{2}, \begin{bmatrix} 6 & -2 & 2 & 4 \\ 0 & -4 & 2 & 2 \\ 0 & -12 & 8 & 1 \\ -6 & 4 & 1 & -18 \end{bmatrix}, \begin{bmatrix} 16 \\ -6 \\ -27 \\ -34 \end{bmatrix}$$

$$R(4) + R(1), \begin{bmatrix} 6 & -2 & 2 & 4 \\ 0 & -4 & 2 & 2 \\ 0 & -12 & 8 & 1 \\ 0 & 2 & 3 & -14 \end{bmatrix}, \begin{bmatrix} 16 \\ -6 \\ -27 \\ -18 \end{bmatrix}$$

$$R(3) - 3R(2), \begin{bmatrix} 6 & -2 & 2 & 4 \\ 0 & -4 & 2 & 2 \\ 0 & 0 & 2 & -5 \\ 0 & 2 & 3 & -14 \end{bmatrix}, \begin{bmatrix} 16 \\ -6 \\ -9 \\ -18 \end{bmatrix}$$

$$R(4) + \frac{R(2)}{2}, \begin{bmatrix} 6 & -2 & 2 & 4 \\ 0 & -4 & 2 & 2 \\ 0 & 0 & 2 & -5 \\ 0 & 0 & 4 & -13 \end{bmatrix}, \begin{bmatrix} 16 \\ -6 \\ -9 \\ -21 \end{bmatrix}$$

$$R(4) - 2R(3), \begin{bmatrix} 6 & -2 & 2 & 4 \\ 0 & -4 & 2 & 2 \\ 0 & 0 & 2 & -5 \\ 0 & 0 & 0 & -3 \end{bmatrix}, \begin{bmatrix} 16 \\ -6 \\ -9 \\ -3 \end{bmatrix}$$

$$\boxed{\begin{bmatrix} 1 & 0 & 0 & 0 \\ 2 & 1 & 0 & 0 \\ \frac{1}{2} & 3 & 1 & 0 \\ -1 & -\frac{1}{2} & 2 & 1 \end{bmatrix}, \begin{bmatrix} 6 & -2 & 2 & 4 \\ 0 & -4 & 2 & 2 \\ 0 & 0 & 2 & -5 \\ 0 & 0 & 0 & -3 \end{bmatrix}, \begin{bmatrix} 16 \\ -6 \\ -9 \\ -3 \end{bmatrix}, 144}$$

(7)

Ly=b

$y1 := 16 :$

$y2 := -2 \cdot y1 + 26 :$

$y3 := -\frac{1}{2} \cdot y1 - 3 \cdot y2 - 19 :$

$y4 := y1 + \frac{1}{2} \cdot y2 - 2 \cdot y3 - 34 :$

$y := \text{Vector}([y1, y2, y3, y4]);$

$$y := \begin{bmatrix} 16 \\ -6 \\ -9 \\ -3 \end{bmatrix}$$

(8)

Ux=y

$x4 := -\frac{1}{3}(-3) :$

$x3 := \frac{1}{2}(5 \cdot x4 - 9) :$

$x2 := -\frac{1}{4}(-2 \cdot x3 - 2 \cdot x4 - 6) :$

$x1 := \frac{1}{6}(2 \cdot x2 - 2 \cdot x3 - 4 \cdot x4 + 16) :$

$x := \text{Vector}([x1, x2, x3, x4]);$

$$x := \begin{bmatrix} 3 \\ 1 \\ -2 \\ 1 \end{bmatrix}$$

**(9)**

## **Jacobi**

*with(Student[LinearAlgebra]) :*

*Jacobi :=***proc**( $T, c, \alpha, \epsilon, N$ )

**local**  $i, x, err, temp$ ;

$x := \alpha$ ;

**for**  $i$  **from** 1 **to**  $N$  **do**

$temp := T \cdot x + c$ ;

$err := Norm(temp - x, infinity)$ ;

$x := temp$ ;

**if** ( $err < \epsilon$ ) **then**

**break**;

**end if**;

**end do**;

*print(evalf( $x$ ),  $i$ );*

**end proc**:

$$\begin{aligned}
 T &:= \text{Matrix}\left(\left[\left[0, -\frac{1}{4}, -\frac{1}{4}, -\frac{1}{4}\right], \left[-\frac{1}{8}, 0, -\frac{1}{4}, -\frac{3}{8}\right], \left[\frac{1}{5}, \frac{2}{5}, 0, 0\right], \left[\frac{1}{4}, 0, -\frac{1}{2}, 0\right]\right]\right); \\
 T &:= \begin{bmatrix} 0 & -\frac{1}{4} & -\frac{1}{4} & -\frac{1}{4} \\ -\frac{1}{8} & 0 & -\frac{1}{4} & -\frac{3}{8} \\ \frac{1}{5} & \frac{2}{5} & 0 & 0 \\ \frac{1}{4} & 0 & -\frac{1}{2} & 0 \end{bmatrix}
 \end{aligned}
 \tag{1}$$

$$\begin{aligned}
 c &:= \text{Vector}\left(\left[-\frac{5}{4}, \frac{23}{8}, -\frac{9}{5}, 1\right]\right); \\
 c &:= \begin{bmatrix} -\frac{5}{4} \\ \frac{23}{8} \\ -\frac{9}{5} \\ 1 \end{bmatrix}
 \end{aligned}
 \tag{2}$$

$$\begin{aligned}
 \alpha &:= \text{Vector}([0, 0, 0, 0]); \\
 \alpha &:= \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \end{bmatrix}
 \end{aligned}
 \tag{3}$$

$$\begin{aligned}
 &\text{Jacobi}(T, c, \alpha, 10^{-4}, 100); \\
 &\boxed{\begin{bmatrix} -1.999988563 \\ 3.000005627 \\ -1.000036062 \\ 0.9999687134 \end{bmatrix}}^{20}
 \end{aligned}
 \tag{4}$$



## Gauss-Seidel

*with*(LinearAlgebra) :

*GaussSeidel* := **proc**( $A, b, \alpha, \varepsilon, N$ )

**local**  $i, j, n, L, D, U, T, c, x, err, temp$ ;

$n := \text{RowDimension}(A)$ ;

$L := \text{Matrix}(n)$ ;

$D := \text{Matrix}(n)$ ;

$U := \text{Matrix}(n)$ ;

$x := \alpha$ ;

  # Create  $L$

**for**  $j$  **from** 1 **to**  $n - 1$  **do**

**for**  $i$  **from**  $j + 1$  **to**  $n$  **do**

$L[i, j] := A[i, j]$ ;

**end do**;

**end do**;

  # Create  $D$

**for**  $i$  **from** 1 **to**  $n$  **do**

$D[i, i] := A[i, i]$ ;

**end do**;

  # Create  $U$

**for**  $i$  **from** 1 **to**  $n - 1$  **do**

**for**  $j$  **from**  $i + 1$  **to**  $n$  **do**

$U[i, j] := A[i, j]$ ;

**end do**;

**end do**;

$T := -(L + D)^{-1} \cdot U$ ;

$c := (L + D)^{-1} \cdot b$ ;

**for**  $i$  **from** 1 **to**  $N$  **do**

$temp := T \cdot x + c$ ;

$err := \text{Norm}(temp - x, \text{infinity})$ ;

$x := temp$ ;

**if** ( $err < \varepsilon$ ) **then**

**break**;

**end if**;

**end do**;

$\text{print}(\text{evalf}(x), i)$ ;

**return**  $T$ ;

**end proc**;

$A := \text{Matrix}([\begin{bmatrix} 4, 1, 1, 1 \end{bmatrix}, \begin{bmatrix} 1, 8, 2, 3 \end{bmatrix}, \begin{bmatrix} 1, 2, -5, 0 \end{bmatrix}, \begin{bmatrix} -1, 0, 2, 4 \end{bmatrix}]);$

$$A := \begin{bmatrix} 4 & 1 & 1 & 1 \\ 1 & 8 & 2 & 3 \\ 1 & 2 & -5 & 0 \\ -1 & 0 & 2 & 4 \end{bmatrix} \quad (5)$$

$b := \text{Vector}(\begin{bmatrix} -5, 23, 9, 4 \end{bmatrix});$

$$b := \begin{bmatrix} -5 \\ 23 \\ 9 \\ 4 \end{bmatrix} \quad (6)$$

$T2 := \text{GaussSeidel}(A, b, \alpha, 10^{-4}, 100);$

$$T2 := \begin{bmatrix} 0 & -\frac{1}{4} & -\frac{1}{4} & -\frac{1}{4} \\ 0 & \frac{1}{32} & -\frac{7}{32} & -\frac{11}{32} \\ 0 & -\frac{3}{80} & -\frac{11}{80} & -\frac{3}{16} \\ 0 & -\frac{7}{160} & \frac{1}{160} & \frac{1}{32} \end{bmatrix} \begin{bmatrix} -2.000010143 \\ 2.999995446 \\ -1.000003850 \\ 0.9999993894 \end{bmatrix}^8 \quad (7)$$

### Jacobi

*evalf(Eigenvalues(T));*

$$\begin{bmatrix} 0.3637795831 \\ 0.0960484587 \\ -0.2299140209 + 0.5521692700 I \\ -0.2299140209 - 0.5521692700 I \end{bmatrix} \quad (8)$$

*eval3 := sqrt(0.2299140209<sup>2</sup> + 0.552169270<sup>2</sup>);*

*eval4 := sqrt(0.2299140209<sup>2</sup> + 0.552169270<sup>2</sup>);*

*eval3 := 0.5981231978*

*eval4 := 0.5981231978* (9)

$\rho(T) = 0.5981231978 < 1$ , therefore the sequence will converge.

### Gauss-Seidel

*evalf(Eigenvalues(T2));*

$$\begin{bmatrix} 0. \\ 0. \\ 0.1388341998 \\ -0.2138341998 \end{bmatrix} \quad (10)$$

*eval3 := 0.2053959591*

*eval4 := 0.2053959591* (11)

$\rho(T) = 0.2138341998 < 1$ , therefore the sequence will converge.

**Actual**

*with(Student[NumericalAnalysis]) :*

*evalf( IterativeApproximate(A, b, method = jacobi, initialapprox =  $\alpha$ , tolerance =  $10^{-4}$ , maxiterations = 100) );*

*evalf( IterativeApproximate(A, b, method = gaussseidel, initialapprox =  $\alpha$ , tolerance =  $10^{-4}$ , maxiterations = 100) );*

$$\begin{bmatrix} -1.999988563 \\ 3.000005627 \\ -1.000036062 \\ 0.9999687134 \end{bmatrix}$$

$$\begin{bmatrix} -2.000010143 \\ 2.999995446 \\ -1.000003850 \\ 0.9999993894 \end{bmatrix}$$

**(12)**

## 2 Assignment Code

The complete code used for this assignment is provided in the appendix for reference. Files can be accessed directly at this [GitHub repository](#).

Listing 6: Assembly

```

1 from copy import deepcopy
2 import numpy as np
3 import sympy as sp
4
5 def global_indexing(width, height=None):
6     if height is None:
7         height = width
8     return np.arange(width * height).reshape((height, width))
9
10 def generate_connectivity_matrix(global_indices):
11     total_elements = (global_indices.shape[0] - 1) * (global_indices.shape[1] -
12     1)
13     connectivity_matrix = np.zeros((total_elements, 4), dtype=int)
14     element = 0
15     for i in range(global_indices.shape[0] - 1):
16         for j in range(global_indices.shape[1] - 1):
17             connectivity_matrix[element, 0] = global_indices[i+1, j]
18             connectivity_matrix[element, 1] = global_indices[i+1, j+1]
19             connectivity_matrix[element, 2] = global_indices[i, j+1]
20             connectivity_matrix[element, 3] = global_indices[i, j]
21             element += 1
22     return connectivity_matrix
23
24 def element_mass_matrix(w, h):
25     area = w * h
26     Me = (area/36) * np.array([[4, 2, 1, 2],
27     [2, 4, 2, 1],
28     [1, 2, 4, 2],
29     [2, 1, 2, 4]])
30     return Me
31
32 def element_stiffness_matrix(w, h):
33     # When multiplying each entry formula by w * h /16, we get the following
34     reduced formulas:
35     wh = w/h
36     hw = h/w
37     # Main diagonal
38     a = (1/3) * (hw + wh)
39     # Skew diagonal
40     b = (1/6) * (hw - 2*wh)
41     # First off-diagonal

```

```
40     c = -(1/6) * (2*hw - wh)
41     # Second off-diagonal
42     d = -(1/6) * (hw + wh)
43     # Construct element stiffness matrix
44     Ke = np.array([[a, c, d, b],
45                   [c, a, b, d],
46                   [d, b, a, c],
47                   [b, d, c, a]])
48     return Ke
49
50 def generate_global_coordinates(x_nodes, y_nodes=None):
51     if y_nodes is None:
52         y_nodes = x_nodes
53
54     y_nodes = y_nodes[::-1]
55     global_coordinates = []
56     for y in y_nodes:
57         for x in x_nodes:
58             global_coordinates.append([x, y])
59     global_coordinates = np.array(global_coordinates)
60     return global_coordinates
61
62
63 def global_assembly(x_nodes, y_nodes=None):
64     if y_nodes is None:
65         y_nodes = x_nodes
66
67     global_coordinates = generate_global_coordinates(x_nodes, y_nodes)
68
69     width = len(x_nodes)
70     height = len(y_nodes)
71
72     global_indices = global_indexing(width, height)
73     connectivity_matrix = generate_connectivity_matrix(global_indices)
74
75     num_nodes = width * height
76     M_global = np.zeros((num_nodes, num_nodes))
77     K_global = np.zeros((num_nodes, num_nodes))
78
79     for element in connectivity_matrix:
80
81         # Bottom-left (node 0)
82         x0 = global_coordinates[element[0], 0]
83         y0 = global_coordinates[element[0], 1]
84
85         # Bottom-right (node 1)
86         x1 = global_coordinates[element[1], 0]
```

```

87
88     # Top-left (node 3)
89     y3 = global_coordinates[element[3], 1]
90
91     # Element width and height
92     w = abs(x1 - x0)
93     h = abs(y3 - y0)
94
95     # Get element matrices
96     Me = element_mass_matrix(w, h)
97     Ke = element_stiffness_matrix(w, h)
98
99     # Add element contributions to global matrices
100    for i_local in range(4):
101        i_global = element[i_local]
102        for j_local in range(4):
103            j_global = element[j_local]
104            M_global[i_global, j_global] += Me[i_local, j_local]
105            K_global[i_global, j_global] += Ke[i_local, j_local]
106
107    return global_coordinates, M_global, K_global
108
109 def classify_boundary_nodes(global_indexing):
110     # Create a boundary indicator array where 1 indicates a boundary node
111     boundary_indicator = np.zeros_like(global_indexing)
112     boundary_indicator[0, :] = 1 # Top boundary
113     boundary_indicator[-1, :] = 1 # Bottom boundary
114     boundary_indicator[:, 0] = 1 # Left boundary
115     boundary_indicator[:, -1] = 1 # Right boundary
116     return boundary_indicator.flatten()
117
118 def implement_dirichlet_bc(M, K, boundary_indicator):
119     num_nodes = M.shape[0]
120     for i in range(num_nodes):
121         if boundary_indicator[i] == 1:
122             M[i, :] = 0
123             M[:, i] = 0
124             M[i, i] = 1
125             K[i, :] = 0
126             K[:, i] = 0
127             K[i, i] = 1
128     return M, K
129
130 def u_0(coordinates, boundary_indicator):
131     x = coordinates[:, 0]
132     y = coordinates[:, 1]
133     u = np.sin(2*np.pi*x) * np.sin(2*np.pi*y)

```

```

134     u[boundary_indicator == 1] = 0.0 # Apply Dirichlet BCs
135     num_nodes = coordinates.shape[0]
136     return u.reshape((num_nodes,1))
137
138 if __name__ == "__main__":
139     width = 5 # Number of nodes along one dimension
140     global_indices = global_indexing(width)
141     connectivity_matrix = generate_connectivity_matrix(global_indices)
142
143     x_nodes = np.array([-2, -1.6, -1.2, -0.8, 2])
144     _, M, K = global_assembly(x_nodes)
145
146     coordinates = generate_global_coordinates(x_nodes)
147     boundary_indicator = classify_boundary_nodes(global_indices)
148     u = u_0(coordinates, boundary_indicator).reshape(5,5)
149     print(u)
150
151     # avoid forming explicit inverse; use solve for  $M^{-1} K$ 
152     M_inv_K = np.linalg.solve(M, K)
153     evals = np.linalg.eigvals(M_inv_K)
154     max_eigenvalue = np.max(np.abs(evals))
155     M_evals = np.linalg.eigvals(M)
156     K_evals = np.linalg.eigvals(K)
157
158     with open("./outputs_4/matrices.txt", "w") as f:
159         latex_matrix = sp.latex(sp.Matrix(coordinates))
160         f.write("Global Coordinates:\n")
161         f.write(latex_matrix + "\n\n")
162
163         # print(global_indices)
164         latex_matrix = sp.latex(sp.Matrix(global_indices))
165         f.write("Global Indices Matrix:\n")
166         f.write(latex_matrix + "\n\n")
167
168         # print(connectivity_matrix)
169         latex_matrix = sp.latex(sp.Matrix(connectivity_matrix))
170         f.write("Connectivity Matrix:\n")
171         f.write(latex_matrix + "\n\n")
172
173         M = np.round(M, 4)
174         M_split = np.array_split(M, 3, axis=1)
175         latex_1_matrix = sp.latex(sp.Matrix(M_split[0]))
176         latex_2_matrix = sp.latex(sp.Matrix(M_split[1]))
177         latex_3_matrix = sp.latex(sp.Matrix(M_split[2]))
178         f.write("Mass Matrix (Part 1):\n")
179         f.write(latex_1_matrix + "\n\n")
180         f.write("Mass Matrix (Part 2):\n")

```



```

181     f.write(latex_2_matrix + "\n\n")
182     f.write("Mass Matrix (Part 3):\n")
183     f.write(latex_3_matrix + "\n\n")
184
185     K = np.round(K, 4)
186     K_split = np.array_split(K, 3, axis=1)
187     latex_1_matrix = sp.latex(sp.Matrix(K_split[0]))
188     latex_2_matrix = sp.latex(sp.Matrix(K_split[1]))
189     latex_3_matrix = sp.latex(sp.Matrix(K_split[2]))
190     f.write("Stiffness Matrix (Part 1):\n")
191     f.write(latex_1_matrix + "\n\n")
192     f.write("Stiffness Matrix (Part 2):\n")
193     f.write(latex_2_matrix + "\n\n")
194     f.write("Stiffness Matrix (Part 3):\n")
195     f.write(latex_3_matrix + "\n\n")
196
197     latex_matrix = sp.latex(sp.Matrix(M_evals))
198     f.write("Mass Matrix Evals:\n")
199     f.write(latex_matrix + "\n\n")
200
201     latex_matrix = sp.latex(sp.Matrix(K_evals))
202     f.write("Stiffness Matrix Evals:\n")
203     f.write(latex_matrix + "\n\n")
204
205     latex_matrix = sp.latex(sp.Matrix(evals))
206     f.write("M-1 * K Evals:\n")
207     f.write(latex_matrix + "\n\n")
208
209     f.write(f"Maximum Eigenvalue of M-1 * K: {max_eigenvalue}\n")

```

Listing 7: Implicit Euler Method

```

1  from assembly import global_assembly, global_indexing, classify_boundary_nodes,
    u_0, implement_dirichlet_bc
2  import numpy as np
3  import math
4  import matplotlib.pyplot as plt
5
6  def u_exact(x, y, t, boundary_indicator, nu=0.05):
7      u = np.exp(-8 * (np.pi**2) * nu * t) * np.sin(2*np.pi*x) * np.sin(2*np.pi*y)
8      boundary_indicator = boundary_indicator.reshape(u.shape)
9      u[boundary_indicator == 1] = 0.0 # Apply Dirichlet BCs
10     return u
11
12  def implicit_heat_solver(x_nodes, dt, t_final, nu=0.05):
13      global_coordinates, M, K = global_assembly(x_nodes)
14      width = len(x_nodes)

```

```

15     global_indexing_array = global_indexing(width)
16     boundary_indicator = classify_boundary_nodes(global_indexing_array)
17
18     M, K = implement_dirichlet_bc(M, K, boundary_indicator)
19
20     A = M + nu * dt * K
21
22     num_time_steps = math.ceil(t_final / dt)
23     dt = t_final / num_time_steps
24     prev_U = u_0(global_coordinates, boundary_indicator)
25     U = np.array([prev_U])
26
27     for n in range(num_time_steps):
28         b = np.matmul(M, U[n])
29         next_U = np.linalg.solve(A, b)
30         U = np.append(U, [next_U], axis=0)
31
32     return boundary_indicator, U.T, global_coordinates
33
34 def convergence_study_timesteps(x_nodes, t_final, dt_values, y_nodes=None):
35     if y_nodes is None:
36         y_nodes = x_nodes
37     errors_implicit = []
38     global_coordinates, _, _ = global_assembly(x_nodes, y_nodes)
39     width = len(x_nodes)
40     global_indexing_array = global_indexing(width)
41     boundary = classify_boundary_nodes(global_indexing_array)
42     x_coords = global_coordinates[:, 0].reshape(width**2, 1)
43     y_coords = global_coordinates[:, 1].reshape(width**2, 1)
44     u_exact_values = u_exact(x_coords, y_coords, t_final, boundary)
45
46     for dt in dt_values:
47         _, U, _ = implicit_heat_solver(x_nodes, dt, t_final)
48         u_numerical_values = U[:, :, -1]
49         error = np.linalg.norm(u_numerical_values - u_exact_values, ord=2)
50         errors_implicit.append(error)
51
52     return errors_implicit
53
54 if __name__ == "__main__":
55     plt.rcParams["text.usetex"] = True
56     plt.rcParams["axes.grid"] = True
57     plt.rc("grid", color="#a6a6a6", linestyle="dotted", linewidth=0.5)
58     plt.style.use("seaborn-v0_8-deep")
59
60     x_nodes = np.array([-2, -1.6, -1.2, -0.8, 2])
61     # x_nodes = chebyshev_nodes(7) * 2 # Scale to [-2, 2]

```

```

62     width = len(x_nodes)
63     dt = 0.01
64     t_final = 1
65     boundary, U, global_coordinates = implicit_heat_solver(x_nodes, dt, t_final
66 )
67     u_approx = U[:, :, -1].reshape((width, width))
68
69     # create plotting mesh consistent with assembly (y reversed in
70 generate_global_coordinates)
71     x_mesh = global_coordinates[:, 0].reshape((width, width))
72     y_mesh = global_coordinates[:, 1].reshape((width, width))
73     u_exact_values = u_exact(x_nodes, y_mesh, 1, boundary)
74
75     fig = plt.figure()
76     ax1 = fig.add_subplot(121, projection='3d')
77     ax1.plot_surface(x_mesh, y_mesh, u_approx, cmap='viridis', alpha=0.8)
78     ax1.set_xlabel('x')
79     ax1.set_ylabel('y')
80     ax1.set_zlabel('u')
81     ax1.set_title('Numerical Solution')
82
83     ax2 = fig.add_subplot(122, projection='3d')
84     ax2.plot_surface(x_mesh, y_mesh, u_exact_values, cmap='plasma', alpha=0.8)
85     ax2.set_xlabel('x')
86     ax2.set_ylabel('y')
87     ax2.set_zlabel('u')
88     ax2.set_title('Exact Solution')
89
90     fig.savefig("./outputs_4/implicit_solution.png", dpi=300)
91     plt.show()
92
93     dt_values = np.logspace(-1, -10, 10, base=2)
94     errors_implicit = convergence_study_timesteps(x_nodes, t_final, dt_values)
95     fig, ax = plt.subplots(figsize=(9, 6))
96     ax.loglog(dt_values, errors_implicit, marker='x', label='Implicit')
97     ax.set_xlabel(r'$\log(\Delta t)$')
98     ax.set_ylabel(r'$\log(||u - u_{exact}||_2)$')
99     ax.set_title('Convergence Study for Implicit Solver')
100    ax.legend()
101    fig.savefig("./outputs_4/implicit_convergence_study.png", dpi=300,
102                bbox_inches='tight')

```

Listing 8: Jacobi Method

```

1  import numpy as np
2
3  def jacobi(A, b, x0=None, tol=1e-12, max_iterations=10000):
4      x_prev = np.zeros_like(b) if x0 is None else x0.copy()

```

```
5     diagonal_list = np.diag(A)
6     D = np.diagflat(diagonal_list)
7     R = A - D
8     D_inverse = np.linalg.inv(D)
9
10    for _ in range(max_iterations):
11        rx_b = -np.dot(R, x_prev) + b
12        x_next = np.dot(D_inverse, rx_b)
13        if np.linalg.norm(x_next - x_prev, ord=np.inf) < tol:
14            return x_next
15        x_prev = x_next
16
17    raise ValueError("Jacobi method did not converge within the maximum number
of iterations")
```