

Applications of PISA3D

Ming-Chieh Chuang (莊明介), Ph.D.

Assistant Researcher

(mcchuang@ncree.narl.org.tw)

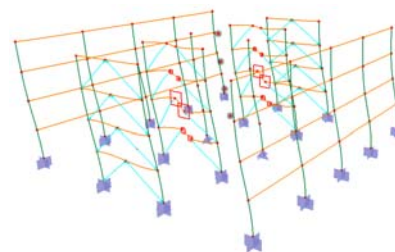
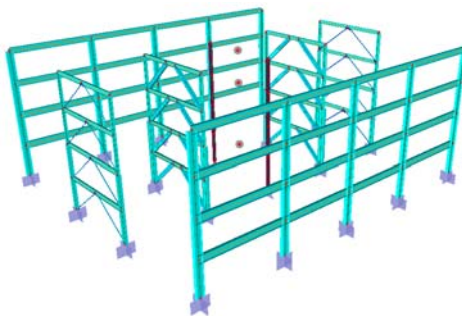
National Center for Research on Earthquake Engineering (NCREE), Taiwan

www.narlabs.org.tw

June 4, 2018

The Example for Demonstration

1. Import the model exported from ETABS/SAP2000



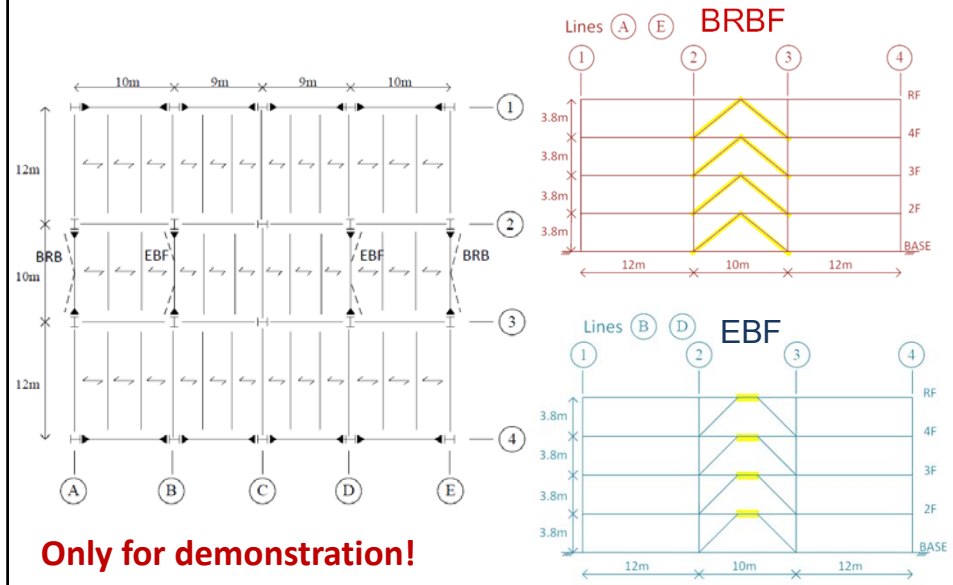
2. Perform nonlinear analyses

4-story Structure system:

Longitudinal direction: MRF in Lines 1 and 4 only.

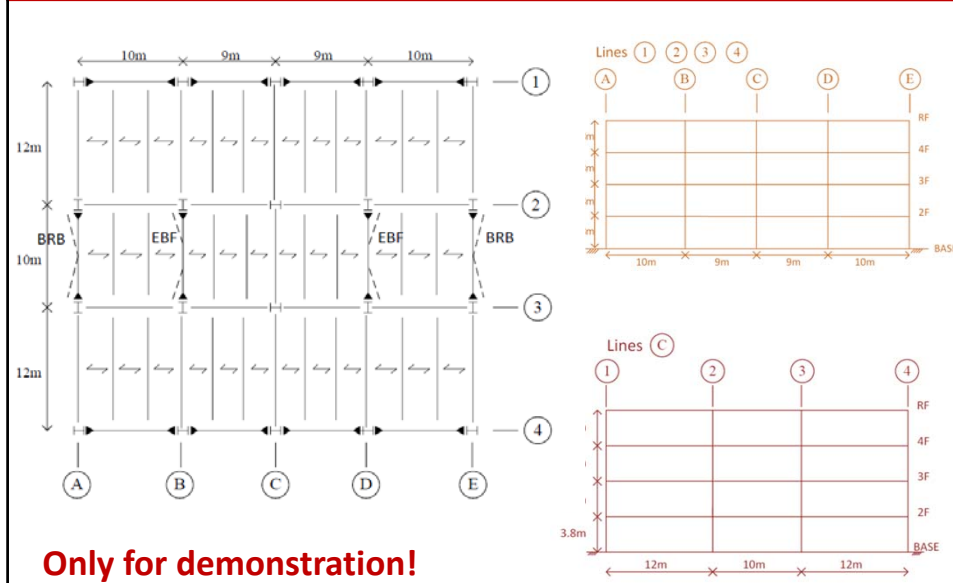
Transverse direction: BRBF in Lines A and E, EBF in Lines B and D.

Floor Framing Plan & Elevations (1/2)



Only for demonstration!

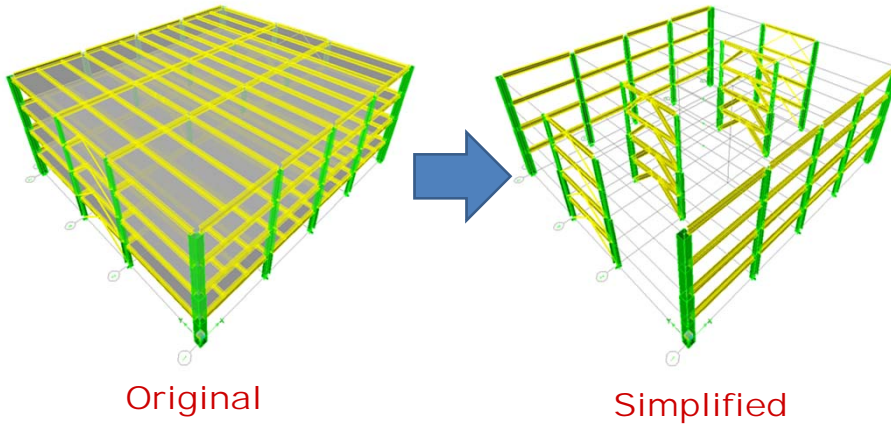
Floor Framing Plan & Elevations (2/2)



Only for demonstration!

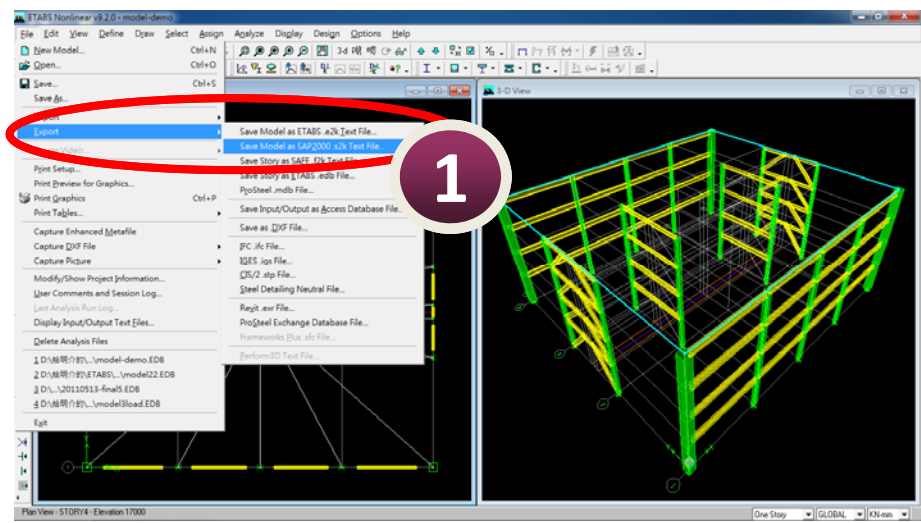
Before you export the ETABS Model

Simplify the model and simulate the seismic resisting system only .



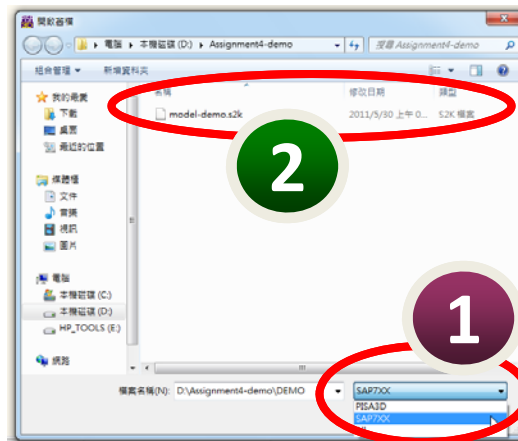
Export the ETABS Model (v9.7.3)

1.Click the **File>Export>Save Model as SAP2000 .s2k Text File** to export your model.



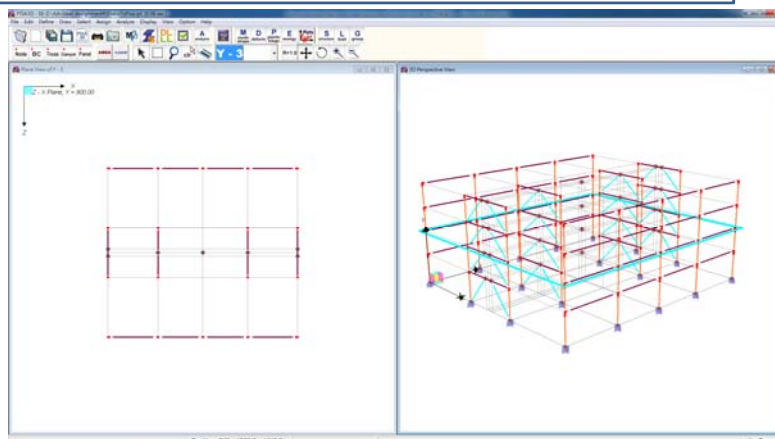
Import the *.s2k file **NARLabs** exported from ETABS Model

1. Open File and Select file format of **SAP7XX** from the drop-down list.
2. Double clicks on the ***.s2k file** to open the project.



Migrate Your Model from ETABS to PISA3D **NARLabs**

1. Save the Model as the *.ipt of file format.
2. **Close All.**
3. **Re-open** the *.ipt to check your model.



What You Have to Check and Modify

1.Nodes and Elements

Check Nodes.

Check BeamColumn elements for beams and columns.

Use Truss elements to simulate braces in BRBF and EBF.

2.Nonlinear Materials

Define the nonlinear materials for simulation of SMRF, EBF and BRBF.

3.Section

Check the properties and the specified materials.

4.Rigid End Zone

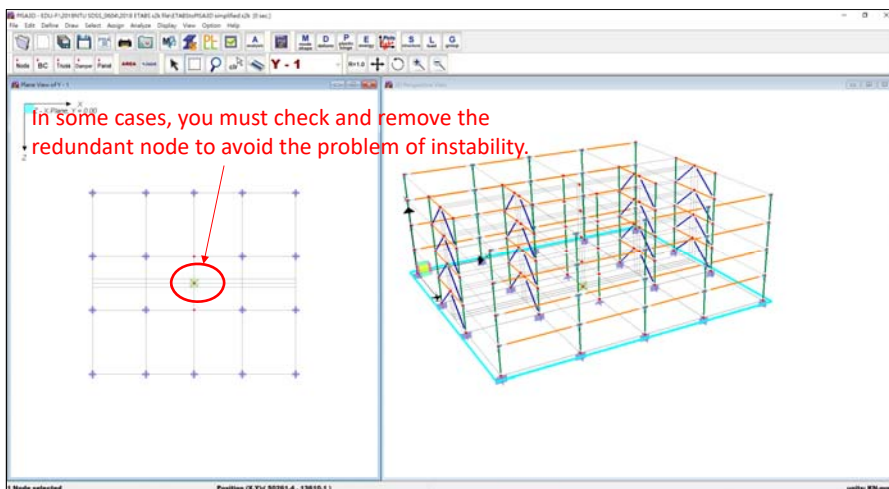
Check/remove the settings of the rigid end offset.

5.Lumped Masses and Constraints

Specify the lumped masses and constraints.

Check Nodes

In some cases, you must check and remove the redundant node to avoid the problem of instability.



What You Have to Check and Modify

1. Nodes and Elements

Check Nodes.

Check BeamColumn elements for beams and columns.

Use Truss elements to simulate braces in BRBF and EBF.

2. Nonlinear Materials

Define the nonlinear materials for simulation of SMRF, EBF and BRBF.

3. Section

Check the properties and the specified materials.

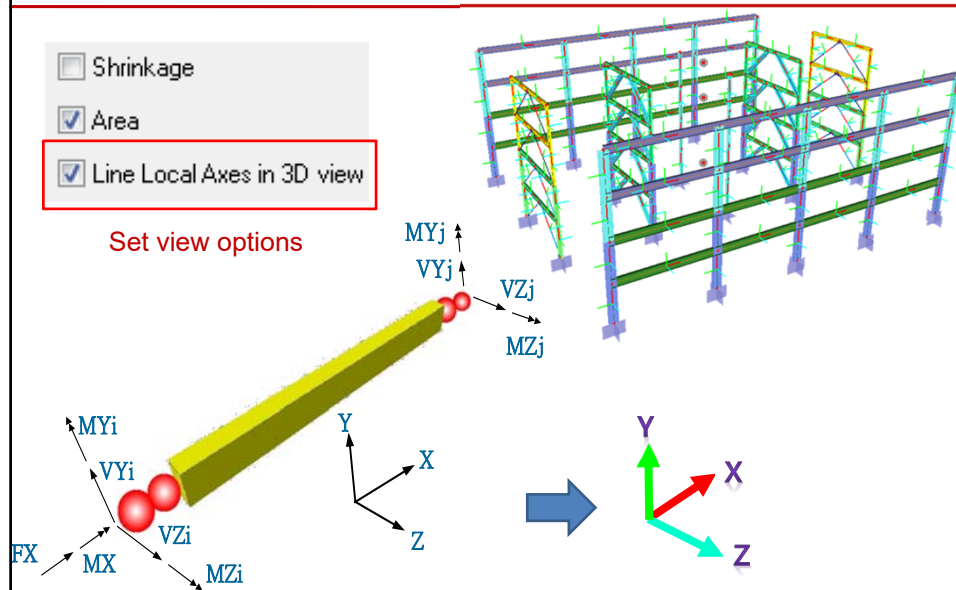
4. Rigid End Zone

Check/remove the settings of the rigid end offset.

5. Lumped Masses and Constraints

Specify the lumped masses and constraints.

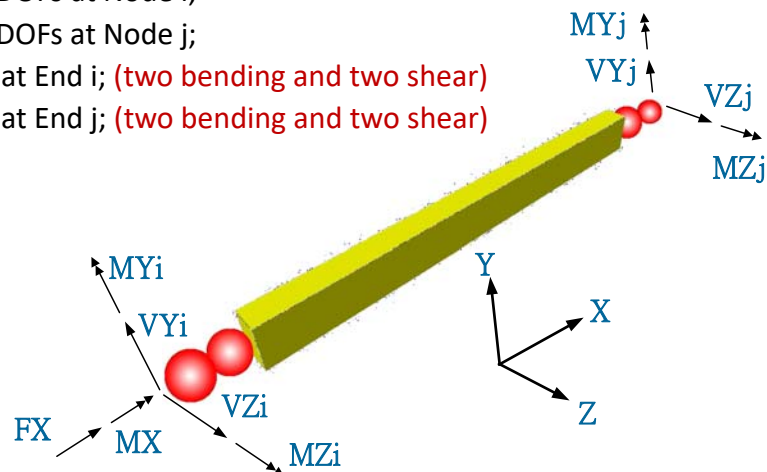
Display the Local Coordinate Systems



BeamColumn Element

- The zero-length plastic hinge (lumped plasticity)

- (1) 6 global DOFs at Node i;
- (2) 6 global DOFs at Node j;
- (3) 4 hinges at End i; (two bending and two shear)
- (4) 4 hinges at End j; (two bending and two shear)



What You Have to Check and Modify

1.Nodes and Elements

Check Nodes.

Check BeamColumn elements for beams and columns.

Use Truss elements to simulate braces in BRBF and EBF.

2.Nonlinear Materials

Define the nonlinear materials for simulation of SMRF, EBF and BRBF.

3.Section

Check the properties and the specified materials.

4.Rigid End Zone

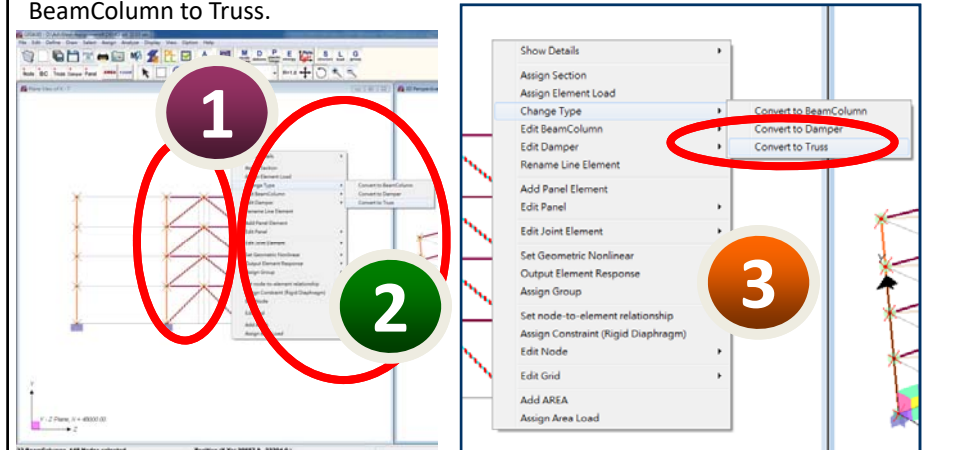
Check/remove the settings of the rigid end offset.

5.Lumped Masses and Constraints

Specify the lumped masses and constraints.

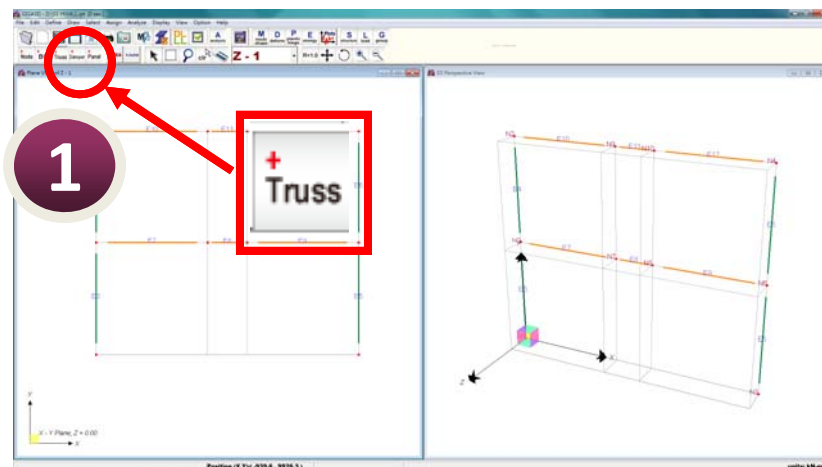
Change the Type of Element

1. Select the Braces of BRBF and EBF.
2. Click right mouse button to activate the **pop-up menu**.
3. Select and Click **Change Type>Convert to Truss** to change the type from BeamColumn to Truss.



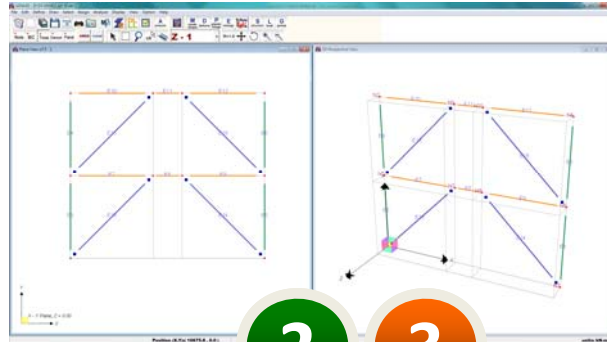
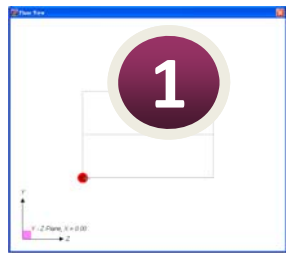
Activate Draw Mode for Drawing Truss Element

1. Click the button to start to draw a Truss element.



Draw a Truss Element

1. When the mouse position is close to the grid intersection, red point means **snap to grid function** is available.
2. Click the left mouse button and drag mouse to define a Truss element.
3. Repeat step 2, construct the braces.



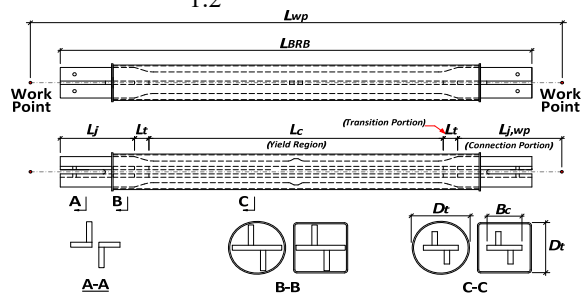
BRB Model

Command: **Truss, Material (Hardening)**

$$K_{eff} = \frac{1}{\frac{1}{K_c} + \frac{1}{K_t} + \frac{1}{1.2K_j}} = \frac{EA_c A_t A_j}{L_c A_t A_j + 2L_t A_c A_j + \frac{2L_{j,wp} A_c A_t}{1.2}} = Q \frac{EA_c}{L_{wp}}$$

$$P_{y_core} = A_c F_y$$

K_{eff} & P_y are both decided.
 L (length) is fixed, how to
 define E , A , F_y in PISA3D?



Method A:

Fix E ; modify A & F_y as

$$A_{eff} = K_{eff} \times L \div E \text{ (Section)}$$

$$F_{y_{eff}} = P_y \div A_{eff} \text{ (Material)}$$

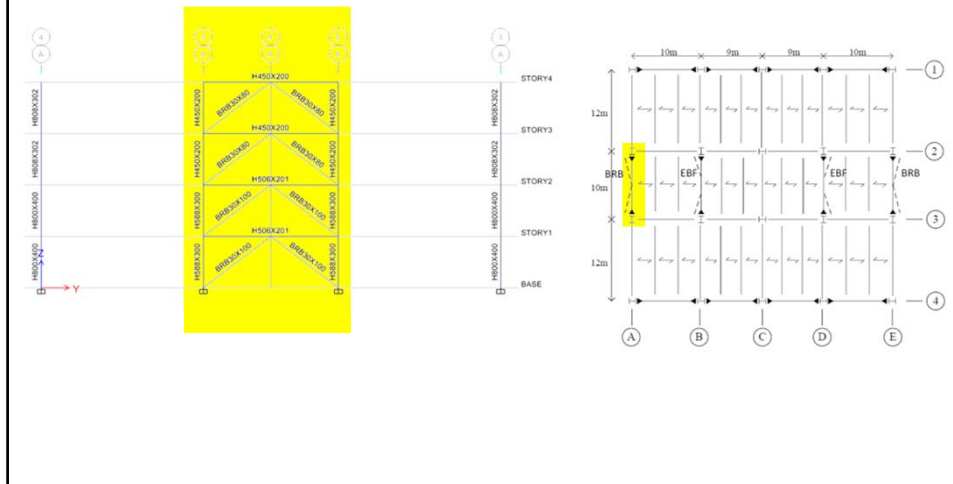
Method B:

Fix A ; modify E as

$$E_{eff} = K_{eff} \times L \div A \text{ (Material)}$$

$$F_y = P_y \div A$$

Details of the BRBs in BRBF



What You Have to Check and Modify

1.Nodes and Elements

Check Nodes.

Check BeamColumn elements for beams and columns.

Use Truss elements to simulate braces in BRBF and EBF.

2.Nonlinear Materials

Define the nonlinear materials for simulation of SMRF, EBF and BRBF.

3.Section

Check the properties and the specified materials.

4.Rigid End Zone

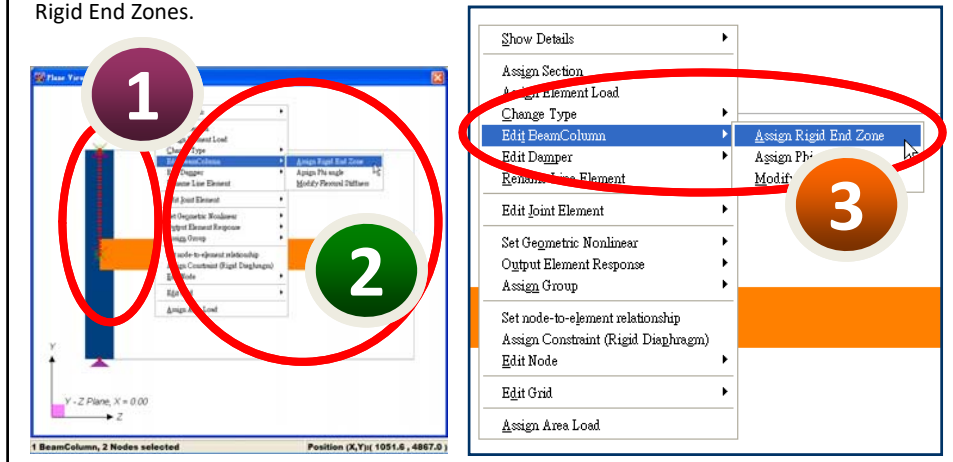
Check/remove the settings of the rigid end offset.

5.Lumped Masses and Constraints

Specify the lumped masses and constraints.

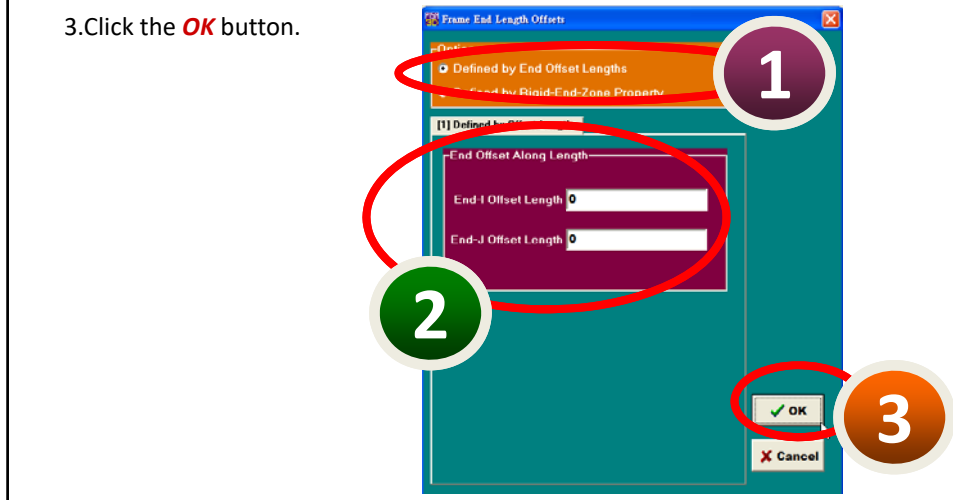
Erase the Rigid End Offset

1. Select All of Beams and Columns.
2. Click right mouse button to activate the **pop-up menu**.
3. Select and Click **Edit BeamColumn>Assign Rigid End Zone** to activate the form of Assign Rigid End Zones.



Erase the Rigid End Offset

1. Select the option of **Defined by End Offset Lengths**.
2. Input the End Offset Lengths according to **BeamColumn Local axis**.
3. Click the **OK** button.



What You Have to Check and Modify

1.Nodes and Elements

Check Nodes

Check BeamColumn elements for beams and columns.

Use Truss elements to simulate braces in BRBF and EBF.

2.Nonlinear Materials

Define the nonlinear materials for simulation of SMRF, EBF and BRBF.

3.Section

Check the properties and the specified materials.

4.Rigid End Zone

Check/remove the settings of the rigid end offset.

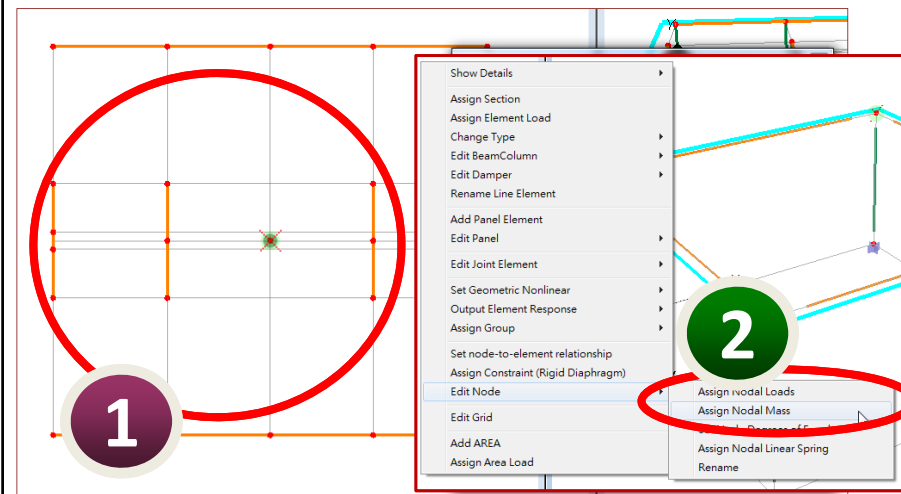
5.Lumped Masses and Constraints

Specify the lumped masses and constraints.

Assign Nodal Masses

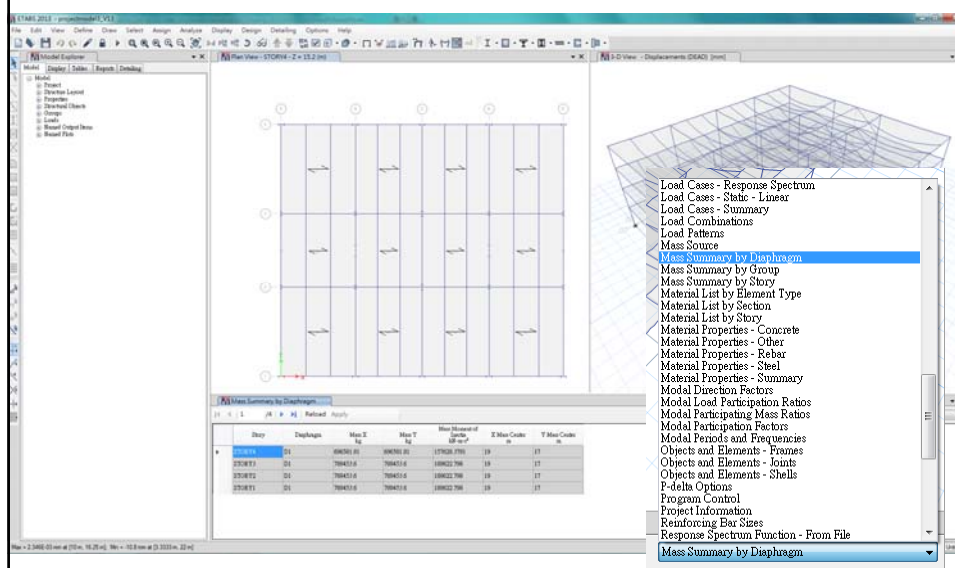
1.Select the **master node**, then click right mouse button to show the popup menu.

2.Select and Click the **Edit Node>Assign Nodal Masses**.



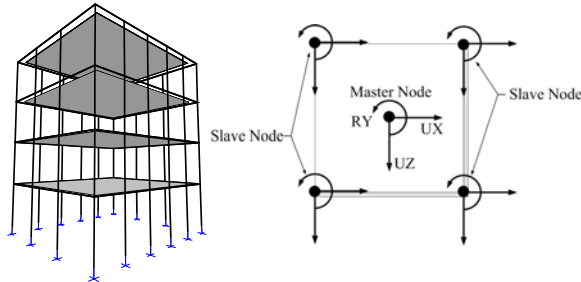
-

Nodal Masses calculated by ETABS



Setting of Nodal Degrees of Freedom (DOFs)

Diaphragm setting



(See manual: D02, D05)

Command: **Constraint, DOF**

A) The Master Node is a virtual one:

//DOF Node UX? UY? UZ? RX? RY? RZ?

DOF N20 0 -1 0 -1 0 -1

B) The Master Node is an existing one:

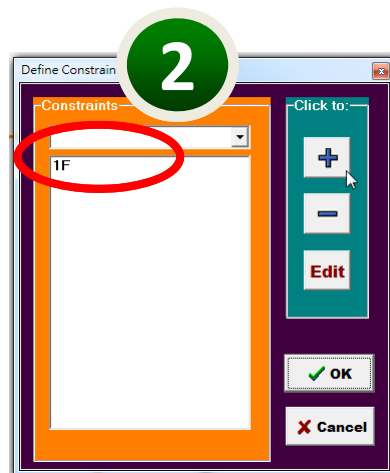
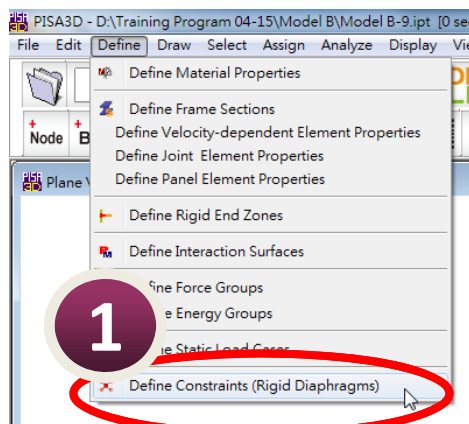
DOF N20 0 0 0 0 0 0



27

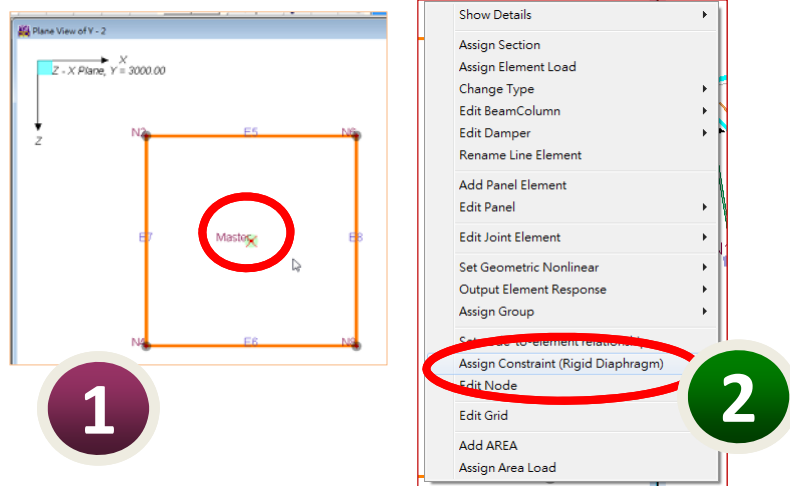
Define the Constraint

1. Select and Click the **Define>Define Constraints (Rigid Diaphragm)** from the main menu.
2. Add a new **constraint 1F**.



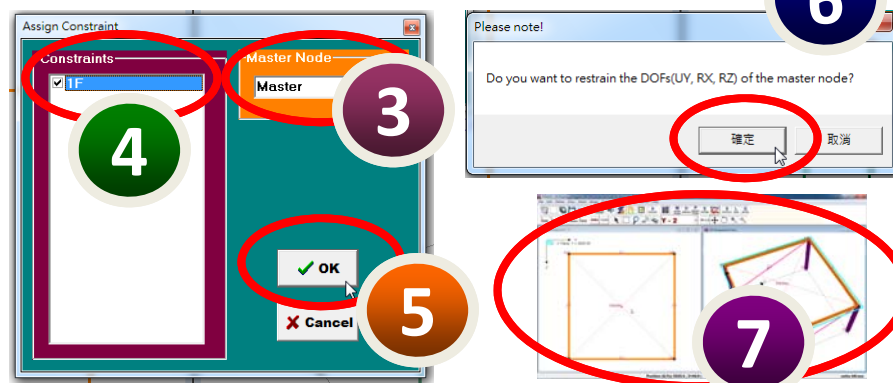
Assign the Constraint

1. Select Node **Master**, then click right mouse button to show the popup menu.
2. Select and Click the **Assign Constraints (Rigid Diaphragm)**.



Assign the Constraint

3. Specify the Node **Master** as Master Node.
4. Assign the Constraint **1F**.
5. Click the **OK** button.
6. Click to modify the DOFs of the master node.
7. Immediate representation of the Diaphragm Extent.



Modal Analysis

Set Analysis Options

Click to:

- Add New Analysis
- Add Copy of Analysis
- Edt/Show Analysis
- Delete Analysis

Analysis Queue

Analysis Title: Setory

Geometric Nonlinear Effect

- Without Geometric Matrix
- Consider only after every Analysis
- Update before each analysis step

Load Combination (Static Load)

1.0 * DL

Enable All Disable All

Define Analysis Case

1.Modal

Define Modal Analysis

Tag: modal

Modes: 3

ID_M1: 1 C1: 0.02

ID_M2: 2 C2: 0.02

Enabled

OK Cancel

.Eigen file

EX4 Eigen: 记事本

檔案(F) 編輯(E) 格式(O) 檢視(V) 說明(H)

P15830 Mode Shape analysis Results File

1st Mode Shape Analysis Method

Mode1	Mode2	DampingRatio1	DampingRatio2
1	2	0.020	0.020

Natural Period

Period of Mode 1 = 2.8195E+000 Sec.

Period of Mode 2 = 1.6377E+000 Sec.

Period of Mode 3 = 1.1981E+000 Sec.

	alpha	Beta
	5.63994494E-002	6.59435401E-003

Node Shape

Node 1, Period = 2.8195E+000 Sec.

Node	UX	UY	UZ	RX	RY
N1	0.000E+000	0.000E+000	0.000E+000	0.000E+000	0.000E+000
N2	1.547E-001	-0.005E-004	1.711E-004	-4.412E-000	2.004E-000
N3	0.000E+000	0.000E+000	0.000E+000	0.000E+000	0.000E+000

■ Check mode shapes & periods
Input required modes, damping ratios

(See manual: B04, Modal Analysis)

Modal Analysis

Define Analysis Case

1.Modal

Define Modal Analysis

Tag: Modal_Analysis

Modes: 6

ID_M1: 1 C1: 0.02

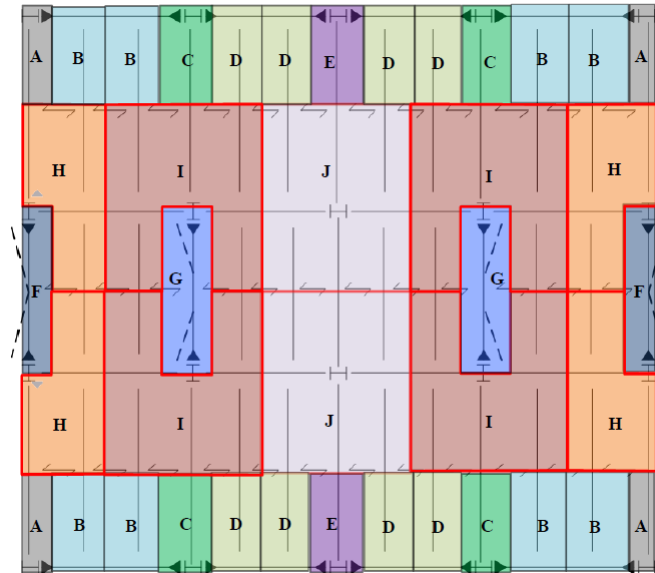
ID_M2: 2 C2: 0.02

Enabled

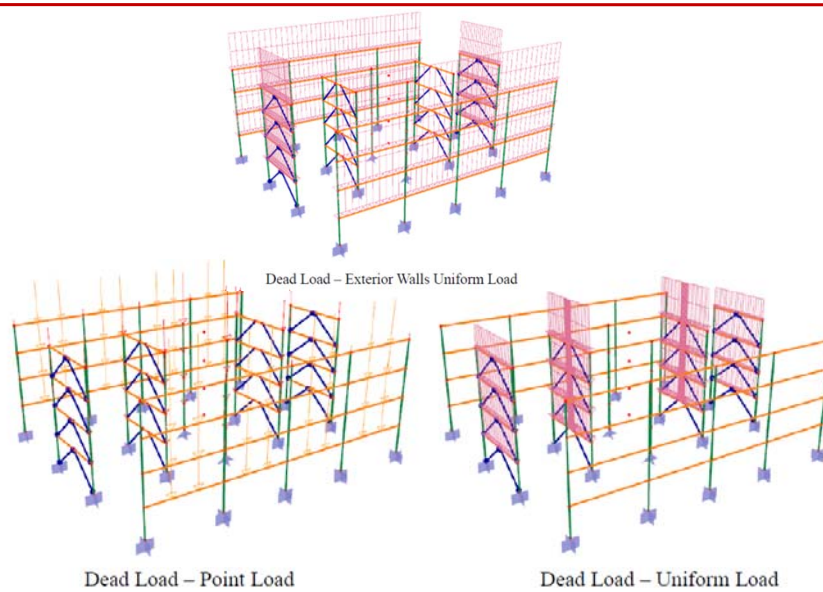
PARAMETER	DESCRIPTION
Analysis	Control Word.
ModeShape	Perform modal analysis.
reqdModes	Number of modes required to be calculated, must > 0 If "reqdModes" is greater than the number of free DOFs, only the number of free DOFs would be calculated.
ID_M1	The ID number of the 1st mode specified to get α and β . (Must > 0 and <= reqdModes).
ID_M2	The ID number of the 2nd mode specified to get α and β . (Must > 0 and <= reqdModes).
C1	The damping ratio of the 1st mode specified to get α and β .
C2	The damping ratio of the 2nd mode specified to get α and β .

(See manual: B04, Modal Analysis)

Tributary Area



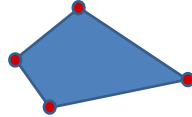
Loads



The Area

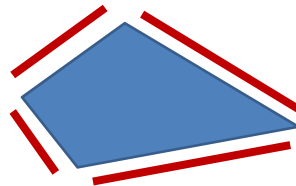
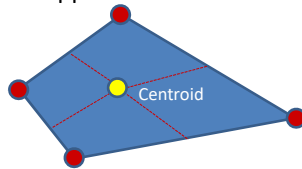
Definition

The area is restricted to be defined by a convex quadrilateral.



Intention

The area is an object defined only for assigning the uniform surface load. The uniform surface load can be distributed by Nodal Loads/Element Loads, then be applied on the model.

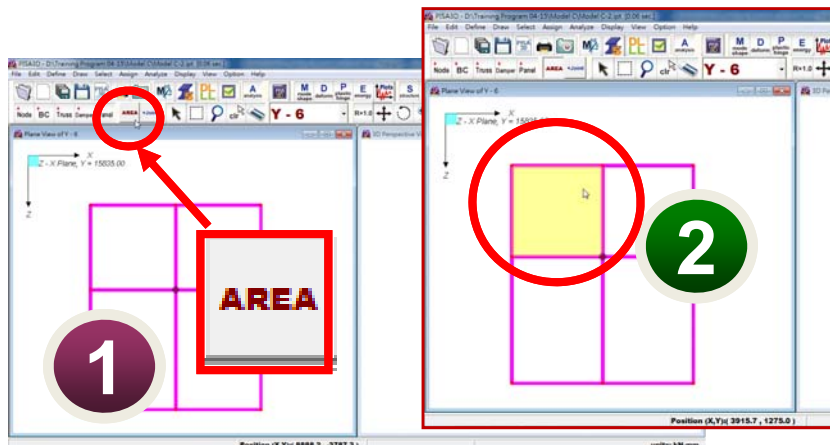


Create

Create Area at Click. Create Area using four nodes.

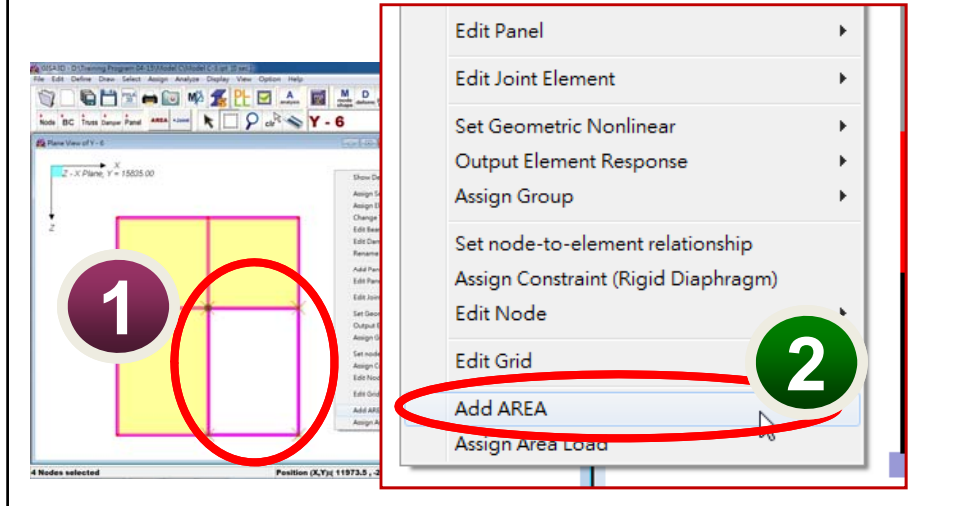
Activate Draw Mode for Creating Areas at Click

1. Click the button to start to Create Areas at Click.
2. Click left mouse button in the region to create the area.



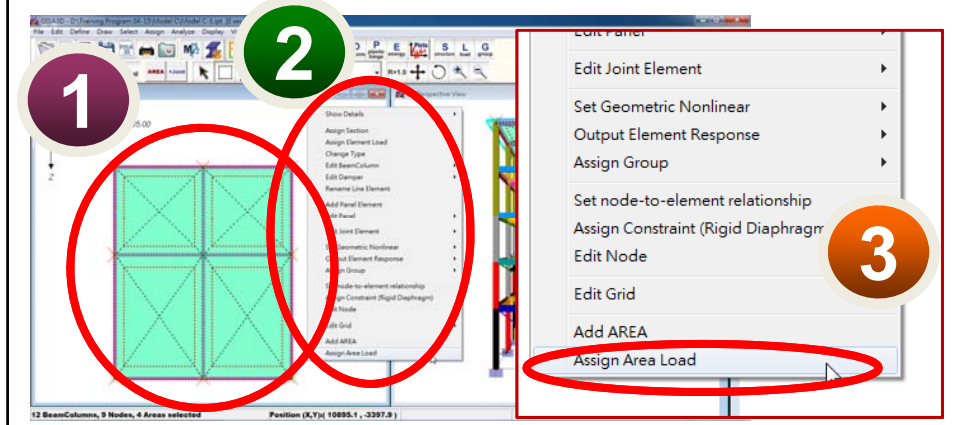
Create a Area using 4 nodes

1. Select the **4 nodes**, then click right mouse button to show the popup menu.
2. Select and Click the **Add AREA**.



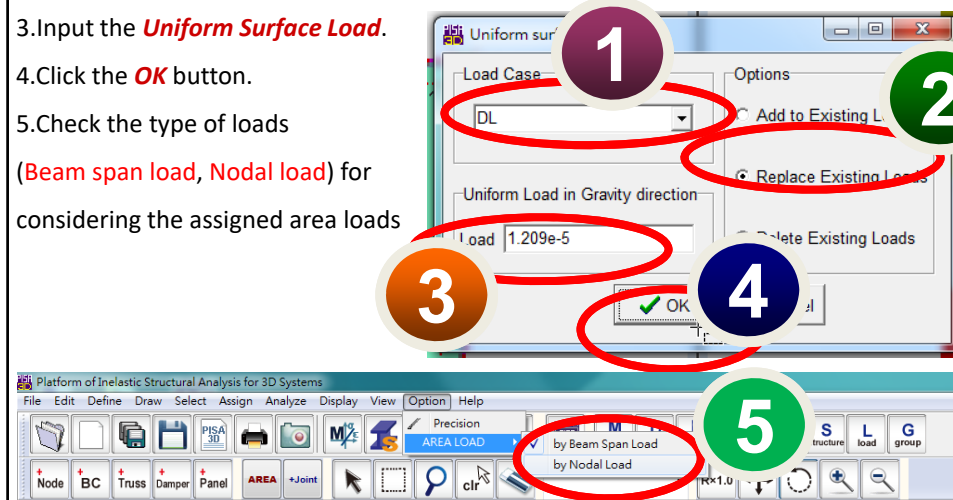
Assign Area Loads

1. Select **Areas**.
2. Click right mouse button to activate the **pop-up menu**.
3. Select and Click **Assign Area Load** to activate the form of Assign Uniform Surface Loads.

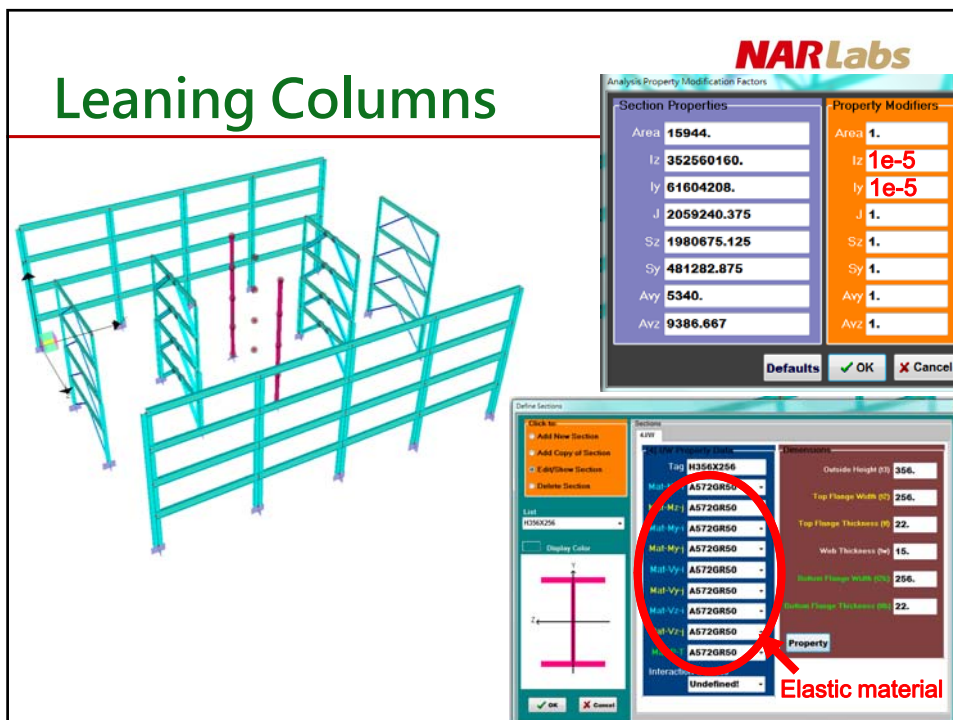


Specify Area Loads using the Static Load Case

1. Select **Load Case DL**.
2. Make sure the selected option is **Replace Existing Loads**.
3. Input the **Uniform Surface Load**.
4. Click the **OK** button.
5. Check the type of loads
(**Beam span load**, **Nodal load**) for considering the assigned area loads

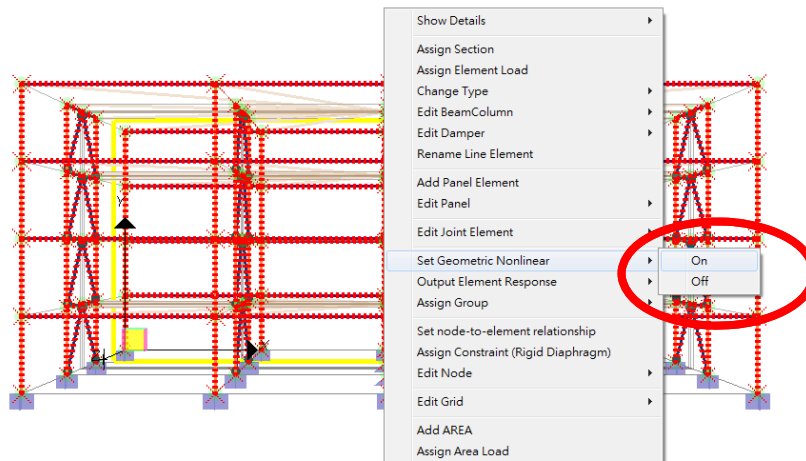


Leaning Columns



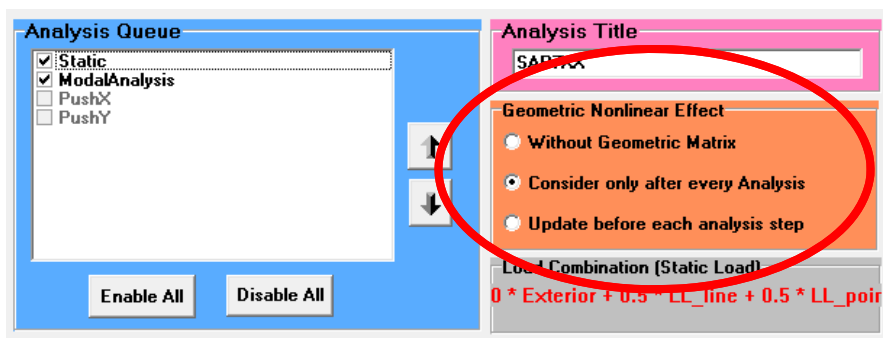
Include P-Delta effect

1. Select All of Beams Braces and Columns.
2. Click right mouse button to activate the **pop-up menu**.
3. Select and Click **Set Geometric Nonlinear>On** to include P-Delta effect.



Include P-Delta effect

1. Set the Geometric Matrix Effect in the form of Analysis Setting.
2. Make sure that the gravity loads has been included before modal analysis.



Quick Search

設定Element搜尋範圍 設定Node搜尋範圍

***Select>Search**
Ctrl+Alt+S

可選定項目，以**交集**方式過濾，進行條件式搜尋Element

可選定項目，以**交集**方式過濾，進行條件式搜尋Node

Define the Force Group

1. Select and Click the **Define>Define Force Group** from the main menu.
2. Add the new **Force Group 1f**.

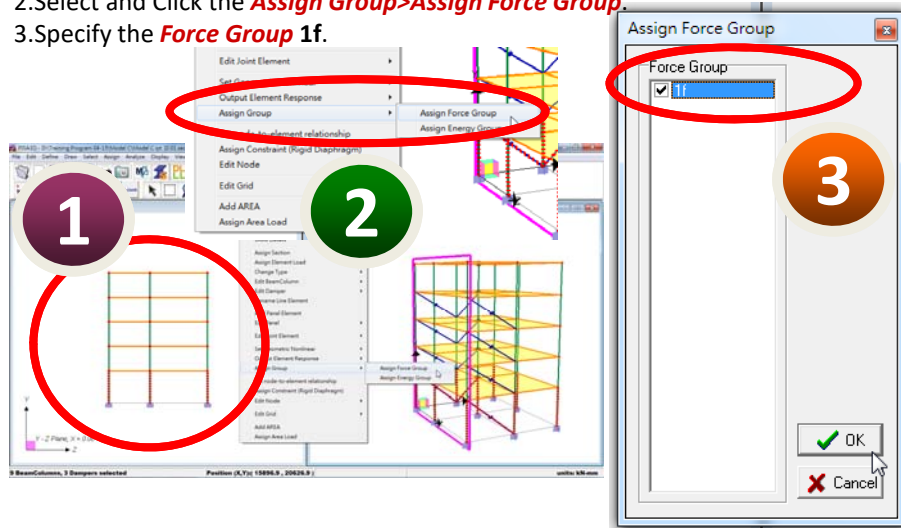
Define Force Groups

Click to : 1f

OK

Assign the Force Group

1. Select elements, then click right mouse button to show the popup menu.
2. Select and Click the **Assign Group>Assign Force Group**.
3. Specify the **Force Group 1f**.

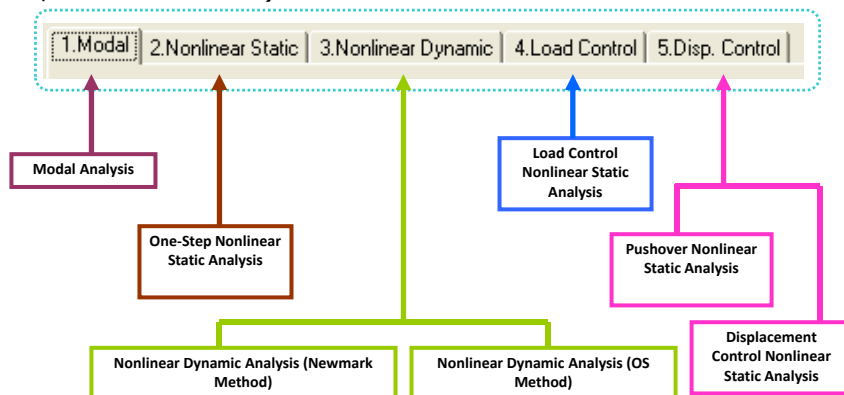


Analysis Setting

***Analyze->Analysis Setting**

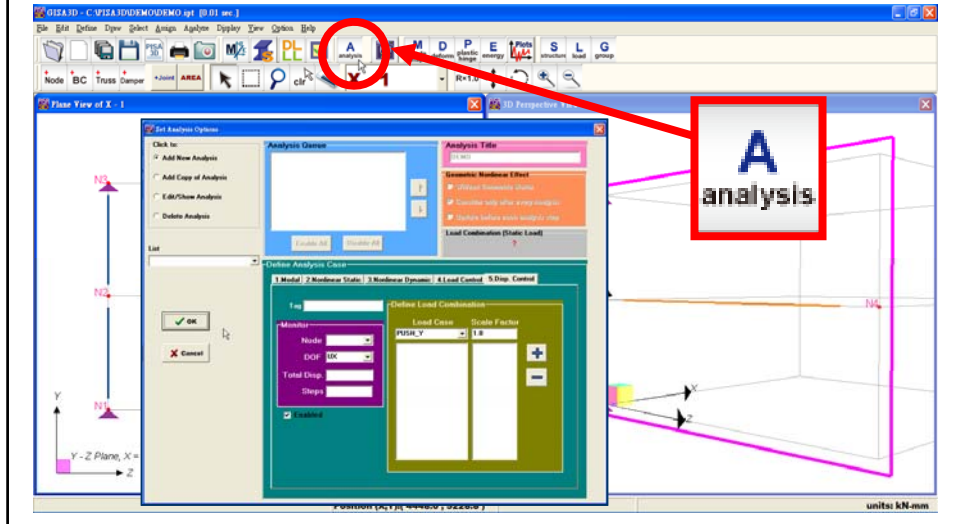


(See PISA3D Manual: [PART B.](#))



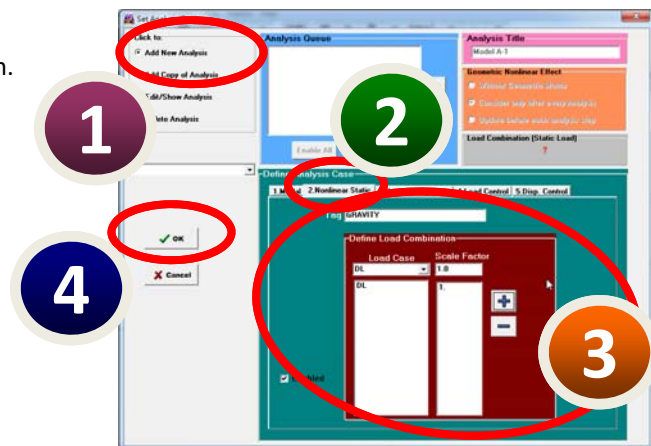
Define the Analysis Cases for Analyzing the Model

1. Click the button to activate the form of **Set Analysis Options**.



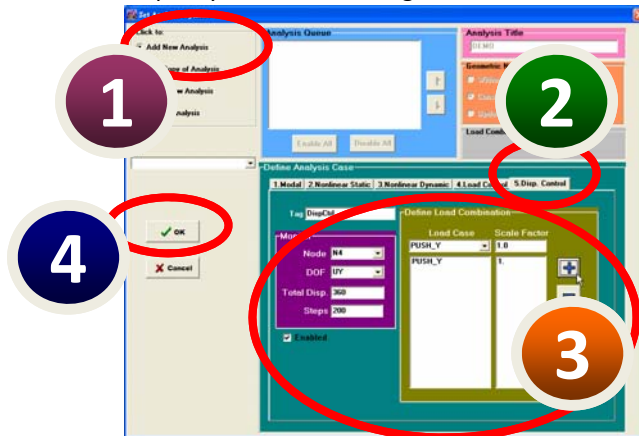
Define the Nonlinear Static Analysis for Gravity Analysis

1. Make sure the selected option is **Add New Analysis**.
2. Select the **Nonlinear Static** page.
3. Input the Tag, the parameters and specify gravity loads using the **static load case DL**.
4. Click the **OK** button.



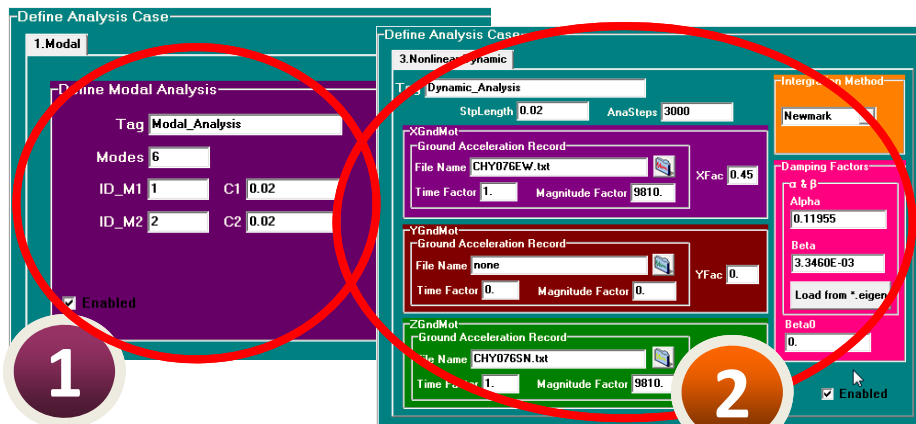
Define the Displacement Control Nonlinear Static Analysis

1. Make sure the selected option is **Add New Analysis**.
2. Select the **Disp. Control** page.
3. Input the Tag, the parameters and specify nodal loads using the **static load case**.
4. Click the **OK** button.



Define the Nonlinear Dynamic Analysis Cases

1. Perform the modal analysis to analyze your model.
2. The Damping Factors (α , β) can be determined from the modal analysis, then add and perform the nonlinear dynamic analysis. $[C] = \alpha[M] + \beta[K_t]$ or $[C] = \alpha[M] + \beta_0[K_0]$



(See manual: B05 or B07)

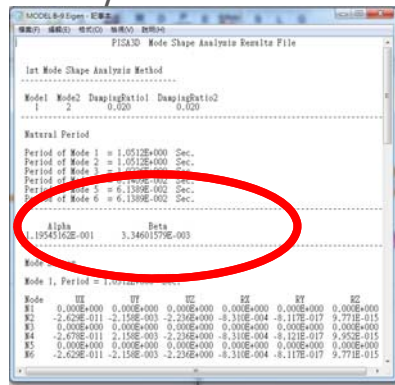
Define the Nonlinear Dynamic Analysis Cases

Damping Factors (α , β) can be determined in two ways:

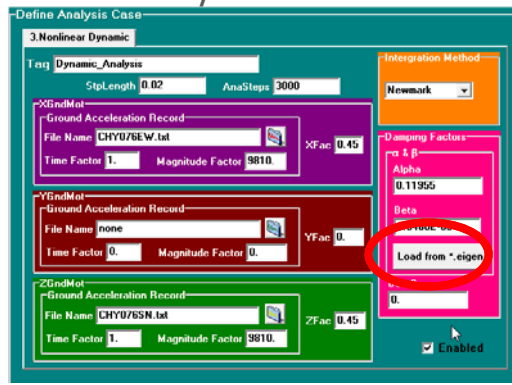
[Option A] Get from EIGEN file.

[Option B] Click the Load from *.eigen Button.

Option A



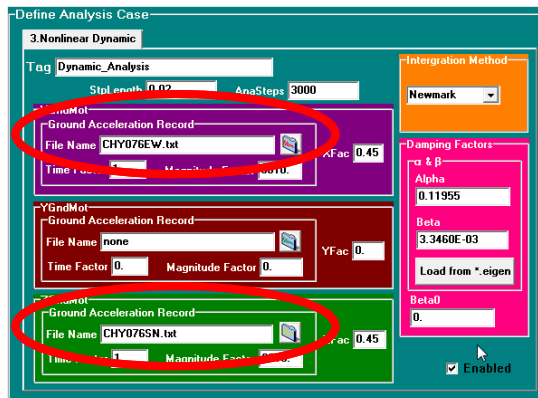
Option B



Define the Nonlinear Dynamic Analysis Cases

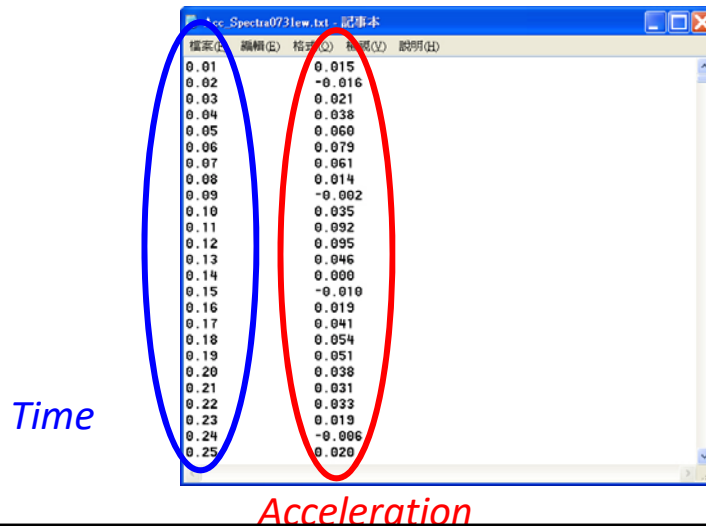
1.The file path of Ground Acc. Record can be defined using both of the absolute path and the relative path.

2.The default directory of the relative path is identical to the location of your PISA3D input file (*.ipt).



Nonlinear Dynamic Analysis

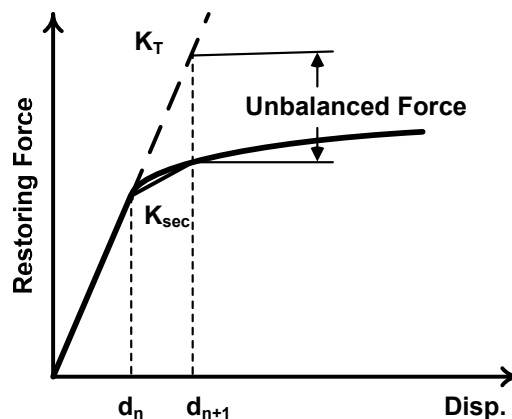
Ground Acceleration Records



Nonlinear Dynamic Analysis

Non-iterative Newmark Scheme

- Newmark scheme for transient response analysis.
- Constant average acceleration method
- In each time step, the updated element stiffness at the beginning of each time step is used to calculate the responses.
- The approach is non-iterative and any unbalanced forces resulting from the element stiffness change in any time step are incorporated into the external loads in the next time step.



Check the Unbalanced Energy

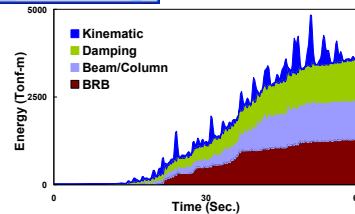
【.Energy file】

Analysis Time/Step	Unbalanced Energy_Ratio	Input Energy	Kinetic Energy	Damping Energy	Strain Energy	Element Group-BC	Element Group-LINK
0.100	0.00%	3.237E+001	3.458E+000	6.459E+000	2.245E+001	2.176E+001	6.141E+001
0.200	-0.00%	1.050E+002	7.191E+001	1.630E+001	9.738E+001	8.779E+001	3.037E+000
0.300	0.00%	2.260E+002	9.238E+001	4.583E+001	8.775E+001	4.578E+001	8.067E+001
0.400	0.00%	2.152E+002	7.704E+001	5.065E+001	8.753E+001	4.581E+001	6.143E+001
0.500	0.00%	2.391E+002	1.437E+002	6.659E+001	2.884E+001	2.268E+001	9.477E+001
0.600	0.00%	2.473E+002	1.240E+002	7.873E+001	4.469E+001	4.107E+001	9.285E+002
0.700	0.00%	2.364E+002	1.124E+002	9.205E+001	3.201E+001	2.101E+001	9.491E+001
0.800	0.00%	2.822E+002	5.714E+001	1.030E+002	1.220E+002	1.019E+002	4.171E+000
0.900	0.00%	4.618E+002	6.164E+001	1.132E+002	2.870E+002	2.358E+002	1.091E+001
1.000	0.00%	5.804E+002	1.385E+002	1.278E+002	3.142E+002	1.892E+002	1.128E+001
1.100	0.00%	5.642E+002	1.253E+002	1.491E+002	2.892E+002	1.324E+002	5.250E+000
1.200	0.00%	8.339E+002	2.991E+002	1.903E+002	3.445E+002	2.630E+002	1.223E+001
1.300	0.00%	1.120E+003	6.396E+002	2.051E+002	2.753E+002	2.406E+002	8.186E+000
1.400	0.00%	1.149E+003	7.764E+002	2.755E+002	9.716E+001	8.054E+001	1.779E+001
1.500	0.00%	1.315E+003	6.668E+002	3.283E+002	3.188E+002	3.137E+002	2.425E+000
1.600	0.00%	1.400E+003	8.057E+002	3.632E+002	2.310E+002	1.667E+002	4.276E+000
1.700	-0.00%	1.510E+003	6.114E+002	4.099E+002	4.889E+002	3.164E+002	1.120E+001
1.800	-0.00%	1.434E+003	1.587E+002	4.325E+002	8.432E+002	6.233E+002	1.638E+001

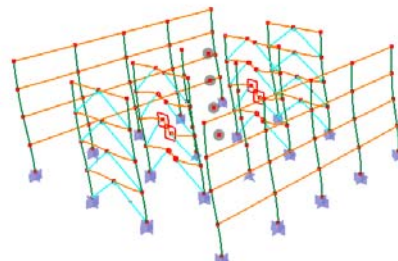
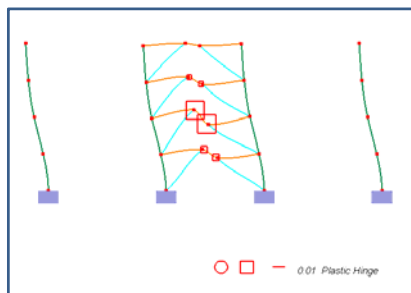
Element Group

< 10~20% is better !

Energy Distribution



Display the Plastic Hinge Distributions with the Structural Deformation



About the representation of hinges in BeamColumn elements, circle and square indicate the nonlinear behavior occurred with flexural and shear yielding respectively. The red line show on middle point of truss element indicates the axial yielding.

Additionally, the size of the shapes and line mentioned means the magnitudes of the nonlinear behavior, plastic hinge rotation, axial strain level. And the sizes are all normalized with respect to the given or default circle, square and line.

User can find the default standard shapes (0.01 for axial strain and 0.01 radians for rotation) on the bottom of the 2D view windows, and visually make a comparison between the standard one and the hinges showed in the model.

Output Setting

***Analyze->Output Setting**

(See PISA3D Manual: [I01](#), [I02](#), [I03](#))

1. Output Interval

2. Nodal Absolute Response

3. Nodal Relative Response

Contents of Output Files (1)

1. **Echo**
包含分析模型的資料與分析過程的訊息，可供使用者檢核模型正確性，錯誤訊息亦會輸出於此檔。
2. **Element**
在每個指定要輸出的時間點上，輸出使用者指定要輸出的所有元素反應。
3. **ElemRecord**
印出上述.Element檔案的所有內容，但是會將每個元素的反應依時間順序排列在一起，方便作圖。此檔只有在分析完成後才會產生。
4. **ElemEnvelope**
在每個指定要輸出的時間點上，輸出使用者指定要輸出的所有元素，到此時間點為止的最大反應。
5. **SectionX, SectionY, SectionZ**
輸出X/Y/Z向的元素群組斷面力。
6. **Energy**
印出系統應變能、阻尼能、動能、外力輸入能量、元素群組應變能量，與不平衡能量。

Contents of Output Files (2)

7. NodeAbsDisp

在每個指定要輸出的時間點上，輸出使用者指定要輸出的所有節點的位移反應。

8. NodeDisRecord

印出上述.NodeAbsDisp檔案的所有內容，但是會將每個節點的反應依時間順序排列在一起，方便作圖。此檔只有在分析完成後才會產生。

9. NodeVelRecord, NodeAccRecord

如同.NodeDisRecord，但記載的是速度與相對加速度的反應

10. NodeRelDisp, NodeRelVel, NodeRelAcc

根據使用者的指定，輸出節點間的相對位移、速度與加速度反應，例如可用來觀測層間變位。

11. NodeEnvelope

在每個指定要輸出的時間點上，輸出使用者指定要輸出的所有節點，到此時間點為止的最大反應。

12. Eigen

振態分析結果的輸出檔，記載自然週期、振態，與比例阻尼的 α 、 β 值。

Contents of Output Files (3)

【.Energy file】

ADST-1.Energy - 記事本

檔案(F) 編輯(E) 格式(O) 檢視(V) 說明(H)

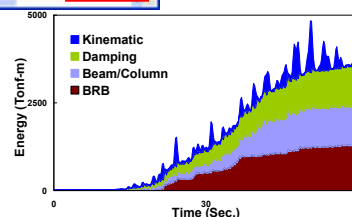
PISA3D Energy Distribution File

Analysis Time/Step	Unbalanced Energy_Ratio	Input Energy	Kinetic Energy	Damping Energy	Strain Energy	Element Group-BC	Element Group-LINK
0.100	0.00%	3.237E+001	3.458E+000	6.459E+000	2.245E+001	2.176E+001	6.141E+001
0.200	-0.00%	1.856E+002	7.191E+001	1.630E+001	9.738E+001	8.779E+001	3.697E+000
0.300	0.00%	2.260E+002	9.238E+001	4.583E+001	8.775E+001	4.578E+001	8.067E+001
0.400	0.00%	2.152E+002	7.704E+001	5.065E+001	8.753E+001	4.581E+001	6.143E+001
0.500	0.00%	2.391E+002	1.437E+002	6.659E+001	2.884E+001	2.268E+001	9.477E+001
0.600	0.00%	2.473E+002	1.240E+002	7.873E+001	4.460E+001	4.107E+001	9.285E+002
0.700	0.00%	2.364E+002	1.124E+002	9.205E+001	3.201E+001	2.101E+001	9.491E+001
0.800	0.00%	2.822E+002	5.714E+001	1.030E+002	1.220E+002	1.018E+002	4.171E+000
0.900	0.00%	4.618E+002	6.164E+001	1.132E+002	2.870E+002	2.358E+002	1.091E+001
1.000	0.00%	5.804E+002	1.385E+002	1.278E+002	3.142E+002	1.892E+002	1.128E+001
1.100	0.00%	5.642E+002	1.253E+002	1.497E+002	2.892E+002	1.324E+002	5.250E+000
1.200	0.00%	6.358E+002	2.991E+002	1.903E+002	3.445E+002	2.630E+002	1.223E+001
1.300	0.00%	1.120E+003	6.396E+002	2.051E+002	2.753E+002	2.406E+002	8.186E+000
1.400	0.00%	1.149E+003	7.764E+002	2.755E+002	9.716E+001	8.054E+001	1.779E+001
1.500	0.00%	1.315E+003	6.668E+002	3.283E+002	3.198E+002	3.137E+002	2.425E+000
1.600	0.00%	1.400E+003	8.057E+002	3.632E+002	2.310E+002	1.667E+002	4.276E+000
1.700	-0.00%	1.510E+003	6.114E+002	4.099E+002	4.889E+002	3.164E+002	1.120E+001
1.800	-0.00%	1.434E+003	1.587E+002	4.325E+002	8.432E+002	6.233E+002	1.638E+001

Element Group

< 10~20% is better !

Energy Distribution



Contents of Output Files (4)

【.Element file】

ADST_1.Element - 記事本

Elem. Tag	Yield Code	Axial Force	Total Extension	Now_Plas. Extension	Accum. Plastic Extension Positive	Accum. Plastic Extension Negative	Total StrainEng	Hysteresis StrainEng
E25	1	-1.234E+003	-2.444E+001	-3.744E+000	5.807E+002	-5.845E+002	1.362E+006	1.585E+000

Joint Element

Elem. Tag	Yield Code	Internal Moment	Total Rotation	Now_Plas. Rotation	Accum. Plastic Rotation Positive	Accum. Plastic Rotation Negative	Total StrainEng	Hysteresis StrainEng
J2	0	1.159E+006	4.841E+003	3.428E+003	1.057E+002	-7.139E+003	2.221E+004	2.245E+004
J3	0	1.025E+006	4.671E+003	3.441E+003	1.534E+002	-1.190E+002	3.336E+004	3.452E+004
J10	0	1.159E+006	4.839E+003	3.425E+003	1.056E+002	-7.139E+003	2.221E+004	2.244E+004
J11	0	1.025E+006	4.672E+003	3.441E+003	1.534E+002	-1.190E+002	3.336E+004	3.451E+004

Element Response at Time: 30.0000

BeamColumn Element

Elem. Tag	MzI	MzJ	MyI	MyJ	UyI	UyJ	UzI	UzJ	BendingMom. Z_I_Now	BendingMom. Z_J_Now	BendingMom. V_I_Now	BendingMom. V_J_Now	ShearForce V_I_Now
E13	0	1	0	0	0	0	0	0	-6.887E+005	1.238E+006	-1.404E+000	1.712E+000	7.851E+002

Truss Element

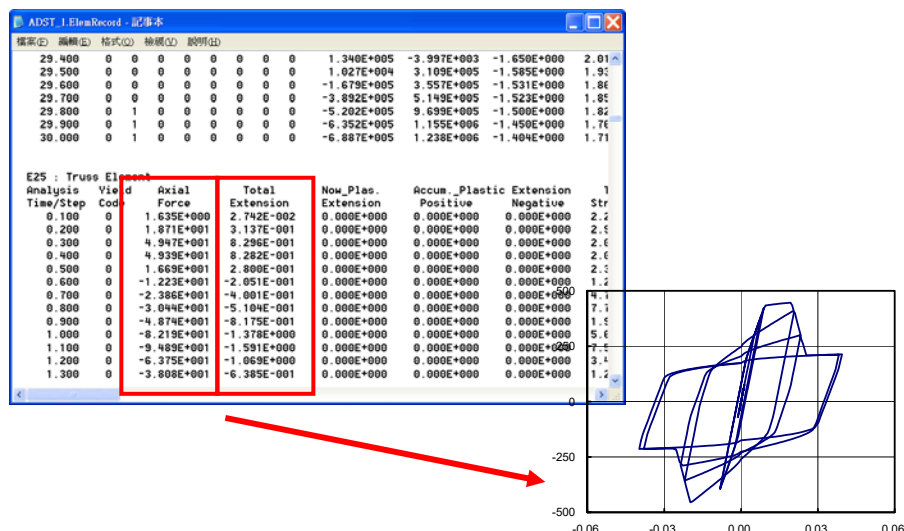
Elem. Tag	Yield Code	Axial Force	Total Extension	Now_Plas. Extension	Accum. Plastic Extension Positive	Accum. Plastic Extension Negative	Total StrainEng	Hysteresis StrainEng
E25	1	-1.265E+003	-3.039E+001	-9.174E+000	5.807E+002	-5.899E+002	1.370E+006	1.593E+000

Joint Element

Elem. Tag	Yield Code	Internal Moment	Total Rotation	Now_Plas. Rotation	Accum. Plastic Rotation Positive	Accum. Plastic Rotation Negative	Total StrainEng	Hysteresis StrainEng
J2	0	1.020E+006	4.652E+003	3.428E+003	1.057E+002	-7.139E+003	2.200E+004	2.245E+004
J3	0	6.930E+005	4.216E+003	3.441E+003	1.534E+002	-1.190E+002	3.297E+004	3.452E+004
J10	0	1.020E+006	4.649E+003	3.425E+003	1.056E+002	-7.139E+003	2.200E+004	2.244E+004
J11	0	6.925E+005	4.216E+003	3.441E+003	1.534E+002	-1.190E+002	3.297E+004	3.451E+004

Contents of Output Files (5)

【.ElementRecord file】



Contents of Output Files (6)

【.ElemEnvelope file】

ADST_1.ElemEnvelope

Tag	Positive	Time	Negative	Time	Positive	Time	Negative	Time
J2	1.455E+006	26.600	-1.275E+006	23.690	7.219E-003	26.600	-2.300E-003	
J3	1.545E+006	26.590	-1.326E+006	23.350	9.686E-003	26.590	-3.686E-003	
J10	1.455E+006	26.600	-1.275E+006	23.690	7.216E-003	26.600	-2.301E-003	
J11	1.545E+006	26.590	-1.326E+006	23.350	9.684E-003	26.590	-3.687E-003	

Element Response Envelopes at Time: 26.9000

BeamColumn Element

Elem. --- Max. Bending_Moment Z_axis_I_end----- --- Max. Bending_Rotation Z_axis_I_en

Tag	Positive	Time	Negative	Time	Positive	Time	Negative	Time
E13	8.705E+005	15.230	-1.043E+006	25.130	1.865E-003	15.070	-2.423E-003	

Truss Element

Elem. --- Maximum Axial Force----- --- Maximum Extension -----

Tag	Positive	Time	Negative	Time	Positive	Time	Negative	Time
E25	1.330E+003	26.830	-1.364E+003	25.100	7.696E+001	16.050	-1.198E+002	

Joint Element

Elem. --- Maximum Internal Moment----- --- Maximum Rotation -----

Tag	Positive	Time	Negative	Time	Positive	Time	Negative	Time
J2	1.455E+006	26.600	-1.275E+006	23.690	7.219E-003	26.600	-2.300E-003	
J3	1.545E+006	26.590	-1.326E+006	23.350	9.686E-003	26.590	-3.686E-003	
J10	1.455E+006	26.600	-1.275E+006	23.690	7.216E-003	26.600	-2.301E-003	
J11	1.545E+006	26.590	-1.326E+006	23.350	9.684E-003	26.590	-3.687E-003	

Contents of Output Files (7)

【.NodeAbsDisp file】

ADST_1.NodeAbsDisp - 記事本

PISA3D Nodal Absolute Displacement File

Nodal Absolute Displacement at Time: 0.1000

Tag	UX	UY	UZ	RX	RY	RV
N14	-1.541E-001	0.000E+000	3.366E-001	0.000E+000	-7.246E-011	0.0
N20	-1.523E-001	0.000E+000	3.700E-001	0.000E+000	-4.887E-011	0.0

Nodal Absolute Displacement at Time: 0.2000

Tag	UX	UY	UZ	RX	RY	RV
N14	1.292E-001	0.000E+000	7.517E-001	0.000E+000	1.403E-009	0.0
N20	1.015E-001	0.000E+000	1.113E+000	0.000E+000	1.738E-010	0.0

Nodal Absolute Displacement at Time: 0.3000

Tag	UX	UY	UZ	RX	RY	RV
N14	-3.352E-001	0.000E+000	1.110E-001	0.000E+000	1.329E-010	0.0
N20	-3.520E-001	0.000E+000	1.054E+000	0.000E+000	2.870E-009	0.0

Nodal Absolute Displacement at Time: 0.4000

Tag	UX	UY	UZ	RX	RY	RV
N14	-3.420E-001	0.000E+000	5.409E-002	0.000E+000	1.126E-008	0.0
N20	-4.911E-001	0.000E+000	9.954E-001	0.000E+000	4.929E-009	0.0

Nodal Absolute Displacement at Time: 0.5000

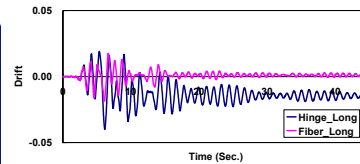
Contents of Output Files (8)

【.NodeDisRecord file】

ADST_1.NodeDisRecord - 記事本

要素(E)	時刻(T)	変位(D)	速度(V)	加速度(A)	変位(D)	速度(V)	加速度(A)
N14	29.300	4.516E+000	0.000E+000	7.664E+000	0.000E+000	3.342E+000	
N14	29.400	-8.328E+000	0.000E+000	6.349E+001	0.000E+000	4.035E+000	
N14	29.500	-1.046E+001	0.000E+000	-1.219E+001	0.000E+000	1.187E+000	
N14	29.600	-2.046E+001	0.000E+000	-6.958E+000	0.000E+000	1.035E+000	
N14	29.700	-3.679E+001	0.000E+000	-2.019E+000	0.000E+000	1.018E+000	
N14	29.800	-4.727E+001	0.000E+000	-1.801E+001	0.000E+000	1.608E+000	
N14	29.900	-5.880E+001	0.000E+000	-4.425E+001	0.000E+000	1.915E+000	
N14	30.000	-6.079E+001	0.000E+000	-6.183E+001	0.000E+000	1.870E+000	

Tag	Time/Step	UX	UY	UZ	RX	RY
N20	0.100	-1.523E-001	0.000E+000	3.700E-001	0.000E+000	-4.887E-001
N20	0.200	1.015E-001	0.000E+000	1.113E+000	0.000E+000	1.738E-001
N20	0.300	-3.520E-001	0.000E+000	1.054E+000	0.000E+000	2.870E-001
N20	0.400	-4.911E-001	0.000E+000	9.954E-001	0.000E+000	4.929E-001
N20	0.500	-5.780E-001	0.000E+000	6.630E-001	0.000E+000	2.330E-001
N20	0.600	-3.723E-001	0.000E+000	-4.512E-002	0.000E+000	7.420E-001
N20	0.700	-6.367E-001	0.000E+000	-7.598E-001	0.000E+000	1.398E-001
N20	0.800	-6.620E-001	0.000E+000	-1.328E+000	0.000E+000	1.871E-001
N20	0.900	-6.991E-001	0.000E+000	-2.141E+000	0.000E+000	1.988E-001
N20	1.000	-4.888E-001	0.000E+000	-2.618E+000	0.000E+000	1.697E-001
N20	1.100	-4.576E-001	0.000E+000	-2.295E+000	0.000E+000	1.184E-001
N20	1.200	-1.127E+000	0.000E+000	-2.432E+000	0.000E+000	6.295E-001
N20	1.300	-1.600E+000	0.000E+000	-1.797E+000	0.000E+000	2.544E-001
N20	1.400	-9.430E-001	0.000E+000	-5.538E-001	0.000E+000	4.732E-001
N20	1.500	1.768E-001	0.000E+000	7.490E-001	0.000E+000	1.428E-001
N20	1.600	1.434E+000	0.000E+000	1.723E+000	0.000E+000	2.371E-001
N20	1.700	3.030E+000	0.000E+000	2.770E+000	0.000E+000	2.071E-001
N20	1.800	4.242E+000	0.000E+000	3.183E+000	0.000E+000	-5.679E-001
N20	1.900	4.440E+000	0.000E+000	2.419E+000	0.000E+000	-5.750E-001
N20	2.000	3.836E+000	0.000E+000	7.770E-001	0.000E+000	-1.196E-001
N20	2.100	3.302E+000	0.000E+000	-4.415E-001	0.000E+000	-1.639E-001
N20	2.200	3.361E+000	0.000E+000	-1.155E+000	0.000E+000	-1.676E-001



Contents of Output Files (9)

【.NodeEnvelope file】

ADST_1.NodeEnvelope - 記事本

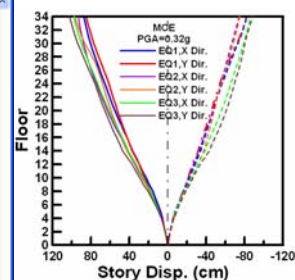
要素(E)	時刻(T)	変位(D)	速度(V)	加速度(A)	変位(D)	速度(V)	加速度(A)
N14	22.6000	1.716E+002	-3.750E+002	19.600	0.000E+000	0.000E+000	0.000E+000
N20	22.550	-8.097E+002	19.540	0.000E+000	0.000E+000	0.000E+000	0.000E+000

Tag	Time	UX	UY	UZ	RX	RY
N14	3.958E+002	22.170	-3.888E+002	16.930	0.000E+000	0.000E+000
N20	6.123E+002	22.180	-6.751E+002	18.830	0.000E+000	0.000E+000

Tag	Time	UX	UY	UZ	RX	RY
N14	3.122E+003	16.440	-3.222E+003	18.380	0.000E+000	0.000E+000
N20	2.831E+003	15.960	-3.046E+003	22.440	0.000E+000	0.000E+000

Tag	Time	UX	UY	UZ	RX	RY
N14	1.716E+002	22.400	-3.758E+002	19.600	0.000E+000	0.000E+000
N20	3.682E+002	22.550	-8.097E+002	19.540	0.000E+000	0.000E+000

Tag	Time	UX	UY	UZ	RX	RY
N14	3.958E+002	22.170	-3.888E+002	16.930	0.000E+000	0.000E+000
N20	6.123E+002	22.180	-6.751E+002	18.830	0.000E+000	0.000E+000



Contents of Output Files (10)

【.SectionX/Y/Z file】

FIBERCONTEST4 - SectionX - 記事本

檔案(F) 編輯(E) 格式(O) 抽籤(O) 說明(H)

PISA3D X-Direction Group Force File

Time/Step	Group-1F	Group-2F	Group-3F	Group-4F
0	1.017E+001	5.821E+001	7.214E+002	1.445E+002
0.010	2.484E+000	-4.306E+001	7.003E+002	-2.573E+003
0.020	1.704E+002	-3.125E+001	2.319E+000	-4.843E+001
0.030	3.737E+002	-4.611E+001	-2.400E+000	1.491E+001
0.040	3.383E+000	7.325E+001	-2.375E+001	3.105E+000
0.050	-1.039E+003	2.394E+002	-3.674E+001	3.028E+000
0.060	-2.306E+003	4.143E+002	3.061E+000	-6.668E+000
0.070	-3.005E+003	1.067E+002	1.107E+002	-2.390E+001
0.080	-2.649E+003	-4.443E+002	2.689E+002	-3.146E+001
0.090	-1.304E+003	-1.299E+003	3.523E+002	-3.368E+000
0.100	6.849E+002	-2.072E+003	2.579E+002	7.620E+001
0.110	3.173E+003	-2.506E+003	-7.506E+001	1.957E+002
0.120	6.107E+003	-2.412E+003	-6.316E+002	3.117E+002
0.130	9.090E+003	-1.616E+003	-1.323E+003	3.580E+002
0.140	1.144E+004	-8.582E+000	-1.990E+003	2.590E+002
0.150	1.252E+004	2.343E+003	-2.414E+003	-4.443E+001
0.160	1.228E+004	5.103E+003	-2.357E+003	-5.775E+002
0.170	1.131E+004	7.735E+003	-1.634E+003	-1.296E+003
0.180	1.052E+004	9.733E+003	-2.054E+002	-2.074E+003
0.190	1.039E+004	1.086E+004	1.701E+003	-2.711E+003
0.200	1.069E+004	1.125E+004	4.015E+003	-2.970E+003
0.210	1.091E+004	1.121E+004	6.134E+003	-2.642E+003
0.220	1.089E+004	1.098E+004	7.844E+003	-1.610E+003
0.230	1.087E+004	1.067E+004	8.991E+003	1.217E+002
0.240	1.108E+004	1.035E+004	9.568E+003	2.416E+003
0.250	1.152E+004	1.009E+004	9.696E+003	5.032E+003
0.260	1.222E+004	9.910E+003	9.585E+003	7.663E+003
0.270	1.327E+004	9.819E+003	9.470E+003	9.997E+003
0.280	1.429E+004	9.929E+003	9.538E+003	1.177E+004

