

Appendix C

Documentation of Structural Design and Modeling for Each Archetype Building

This Appendix provides the structural design details (element sizes, reinforcement, etc.) for each of the archetype buildings. In addition, this Appendix also provides the documentation of the modeling parameters used for the structural models of each archetype frame. This Appendix is organized sequentially in order of building ID number.

The body of this Appendix was created by graduate summer interns Brian S. Dean and Jason Chou; their work is greatly appreciated. Collaboratively with myself, they completed all the archetype building designs, created the OpenSees models, managed the OpenSees analyses, and organized the analysis results.

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1001

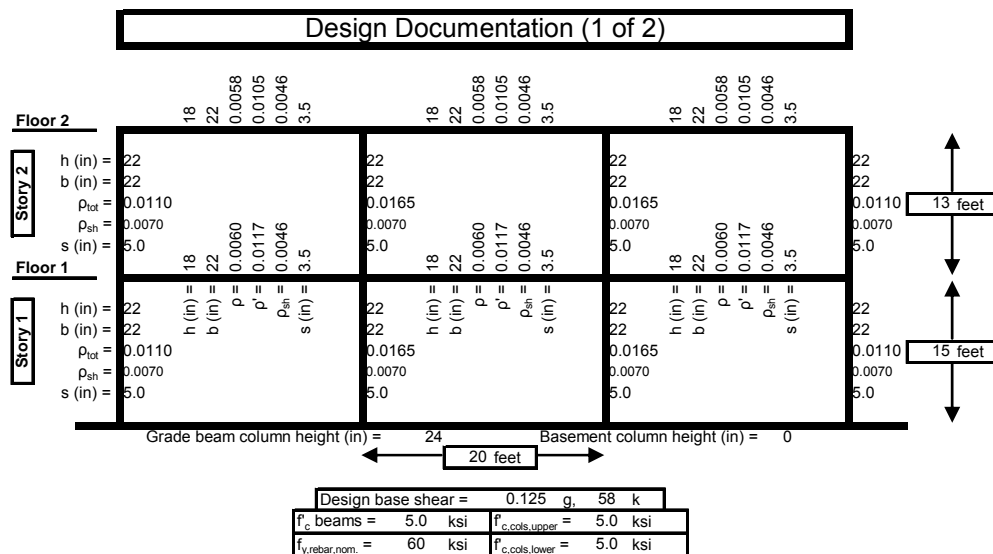
Number of Stories: 2

Fundamental Period (sec): 0.63

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined mostly by minimum size requirements and column-beam compatibility, in addition to joint shear requirements. Beam strengths were controlled by force demands, particularly lateral forces. Column strengths were determined by strong-column weak-beam (SCWB) ratios. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)										
Story 2	SCWB =	0.76	1.17	0.71	1.26	0.69	1.17	0.67	Design Drifts:	
	Joint $\Phi V_n/V_u$ =	1.52	1.17	1.31	1.33	1.31	1.17	1.52		
Story 1	$\Phi M_r/M_u$ =	1.21	1.17	2.94	1.23	2.89	1.17	1.08	1.0%	
	$\Phi V_n/V_{npr}$ =	2.77	1.22	1.88	1.35	2.02	1.22	2.94		
Floor 1	$P/A_g f'_c$ =	0.01	0.52	0.01	0.52	0.00	0.52	0.00	1.1%	
	$\Phi M_r/M_u$ =	1.20	1.17	1.83	1.23	1.82	1.17	1.20		
Story 2	$\Phi M_r/M_u$ =	1.20	1.17	1.88	1.35	2.02	1.17	2.78	1.1%	
	$\Phi V_n/V_{npr}$ =	2.67	1.22	1.88	1.35	2.02	1.22	2.78		
Story 1	$P/A_g f'_c$ =	0.01	0.52	0.01	0.52	0.00	0.52	0.01	1.1%	
	$\Phi M_r/M_u$ =	1.20	1.17	1.83	1.23	1.82	1.17	1.20		
Floor 1	$\Phi M_r/M_u$ =	1.20	1.17	1.88	1.35	2.02	1.17	2.78	1.1%	
	$\Phi V_n/V_{npr}$ =	2.67	1.22	1.88	1.35	2.02	1.22	2.78		
Modeling Documentation (1 of 1)										
Floor 2	$M_{y,exp}$ (k-in) =	3163	1865	4831	1865	4831	1865	3163	1.0%	
	$E I_{eff}/E I_g$ =	0.35	-4276	0.35	-4276	0.35	-4276	0.35		
Story 2	M_c/M_y =	1.21	0.35	1.21	0.35	1.21	0.35	1.21	1.0%	
	$\Theta_{cap,pl}$ (rad) =	0.067	0.054	0.068	0.054	0.068	0.054	0.067		
Story 1	Θ_{pc} (rad) =	0.100	0.055	0.100	0.055	0.100	0.055	0.100	1.0%	
	λ =	93	101	91	101	91	101	93		
Floor 1	$(P/A_g f'_c)_{exp}$ =	0.02	0.03	0.03	0.03	0.03	0.03	0.02	1.0%	
	$M_{y,exp}$ (k-in) =	3494	5465	5465	5465	5465	5465	3494		
Story 2	$E I_{eff}/E I_g$ =	0.35	1.20	0.35	1.20	0.35	1.20	0.35	1.0%	
	M_c/M_y =	1.21	0.35	1.21	0.35	1.21	0.35	1.21		
Story 1	$\Theta_{cap,pl}$ (rad) =	0.065	0.065	0.065	0.065	0.065	0.065	0.065	1.0%	
	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100		
Floor 1	λ =	91	87	87	87	87	87	91	1.0%	
	$(P/A_g f'_c)_{exp}$ =	0.03	0.06	0.06	0.06	0.06	0.06	0.03		
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60										
Model periods (sec): $T_1 = 0.63$ $T_2 = 0.18$ $T_3 = 0.03$										
$f_{v, \text{rebar, expected}} = 67$ ksi										

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1001a

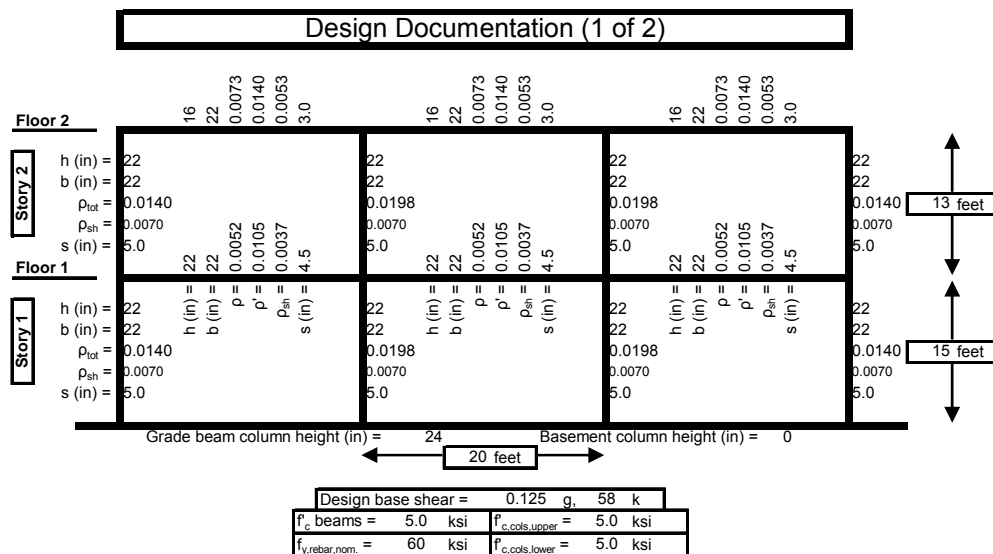
Number of Stories: 2

Fundamental Period (sec): 0.56

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined by joint shear requirements and beam-column dimensional compatibility. Beam strengths were controlled by force demands, particularly lateral forces. Column strengths were determined by strong-column weak-beam (SCWB) ratios. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Modeling Documentation (1 of 1)

Mass tributary to one frame for lateral load (each floor) (k-s/in):	0.60
---	------

Model periods (sec): $T_1 = 0.56$ $T_2 = 0.18$ $T_3 = 0.03$

$$f_{y, \text{rebar, expected}} = 67 \text{ ksi}$$

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1002

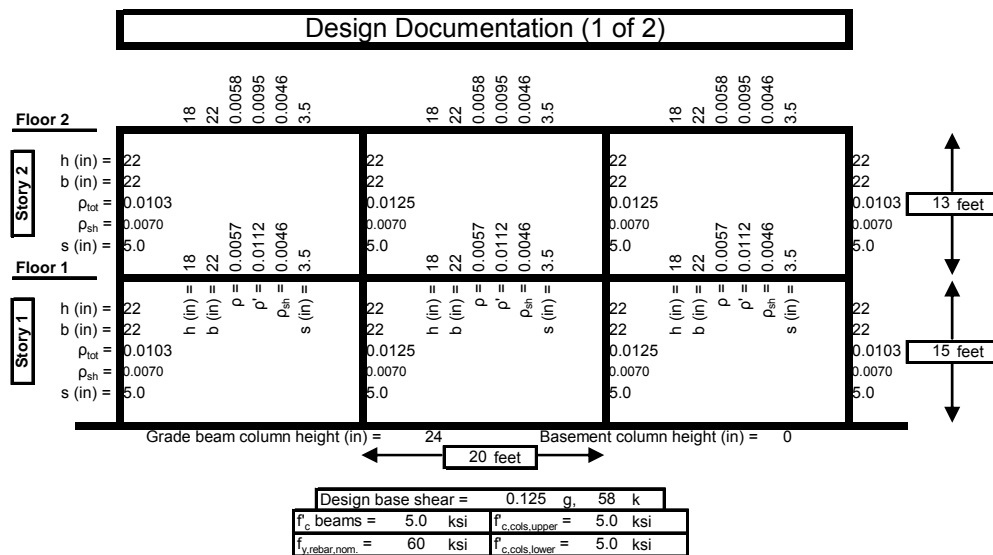
Number of Stories: 2

Fundamental Period (sec): 0.63

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined mostly by minimum size requirements and column-beam compatibility. Beam strengths were controlled by force demands, particularly lateral forces. Column strengths were determined by flexural demands in the ground floor together with strong-column weak-beam (SCWB) ratios. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)										
Story 2	SCWB =	0.78	1.16	0.66	1.24	0.66	1.16	0.78	Design Drifts:	
	Joint $\Phi V_n/V_u$ =	1.68	1.17	1.39	1.33	1.39	1.17	1.68		
Story 1	$\Phi M_r/M_u$ =	1.26	2.79	2.81	2.70	2.81	2.70	1.26	0.9%	
	$\Phi V_n/V_{npr}$ =	2.70	2.03	2.03	0.05	2.03	0.05	2.70		
Floor 1	$P/A_g f'_c$ =	0.02	0.05	0.05	0.02	0.05	0.02	0.02	1.1%	
	$\Phi M_r/M_u$ =	1.41	1.30	1.30	1.16	1.30	1.16	1.41		
Story 1	$\Phi M_r/M_u$ =	1.13	1.70	1.71	2.40	1.71	2.40	1.13	1.1%	
	$\Phi V_n/V_{npr}$ =	2.40	2.45	2.45	0.09	2.45	0.09	2.40		
Story 1	$P/A_g f'_c$ =	0.04	0.09	0.09	0.04	0.09	0.04	0.04		
	$\Phi M_r/M_u$ =	1.28	1.13	1.13	1.19	1.13	1.19	1.28		
Story 1	$\Phi M_r/M_u$ =	1.16	1.22	1.22	0.52	1.22	0.52	1.16	1.1%	
	$\Phi V_n/V_{npr}$ =	2.24	2.24	2.24	0.05	2.24	0.05	2.24		
Story 1	$\Phi M_r/M_u$ =	1.16	1.19	1.19	0.52	1.19	0.52	1.16	1.1%	
	$\Phi V_n/V_{npr}$ =	2.24	2.24	2.24	0.05	2.24	0.05	2.24		
Modeling Documentation (1 of 1)										
Floor 2	$M_{y,exp}$ (k-in) =	2978	3860	3860	2978	3860	3860	2978	0.60	
	$E I_{eff}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35		
Story 2	M_c/M_y =	1.21	1.21	1.21	1.21	1.21	1.21	1.21	0.63	
	$\Theta_{cap,pl}$ (rad) =	0.067	0.066	0.066	0.067	0.066	0.066	0.067		
Story 1	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.18	
	λ =	93	91	91	93	91	91	93		
Floor 1	$(P/A_g f'_c)_{exp}$ =	0.02	0.03	0.03	0.02	0.03	0.03	0.02	0.03	
	$M_{y,exp}$ (k-in) =	3311	4503	4503	3311	4503	4503	3311		
Story 1	$E I_{eff}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.63	
	M_c/M_y =	1.21	1.20	1.20	1.21	1.20	1.20	1.21		
Story 1	$\Theta_{cap,pl}$ (rad) =	0.065	0.063	0.063	0.065	0.063	0.063	0.065	0.18	
	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100		
Story 1	λ =	91	87	87	91	87	87	91	0.03	
	$(P/A_g f'_c)_{exp}$ =	0.03	0.06	0.06	0.03	0.06	0.06	0.03		
Mass tributary to one frame for lateral load (each floor) (k-s/s/in):										
Model periods (sec): $T_1 = 0.63$ $T_2 = 0.18$ $T_3 = 0.03$										
f_y rebar expected = 67 ksi										

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1003

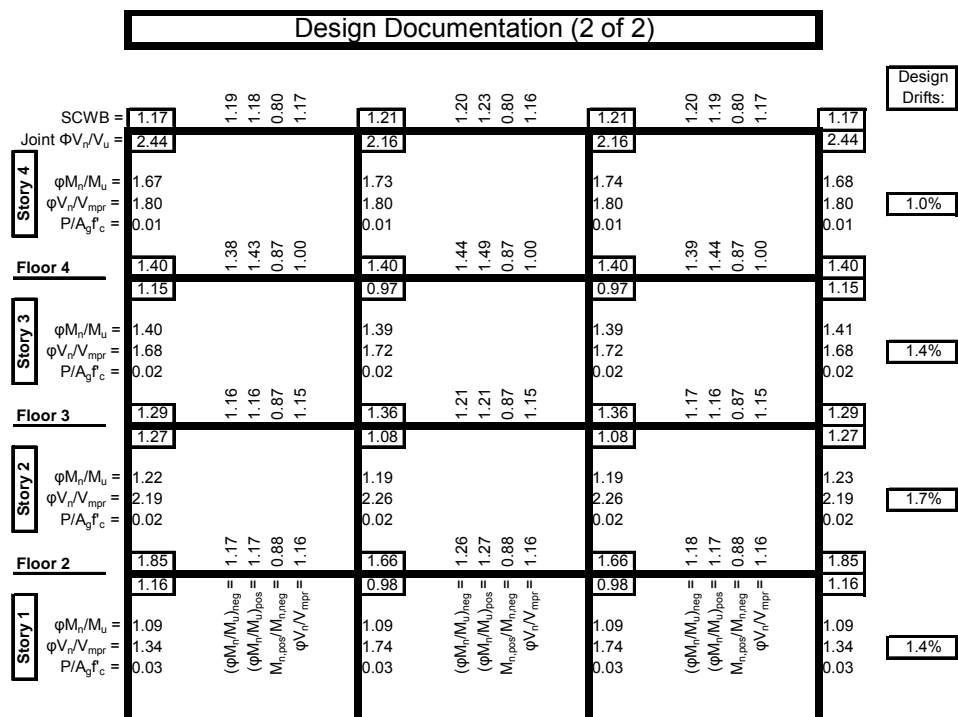
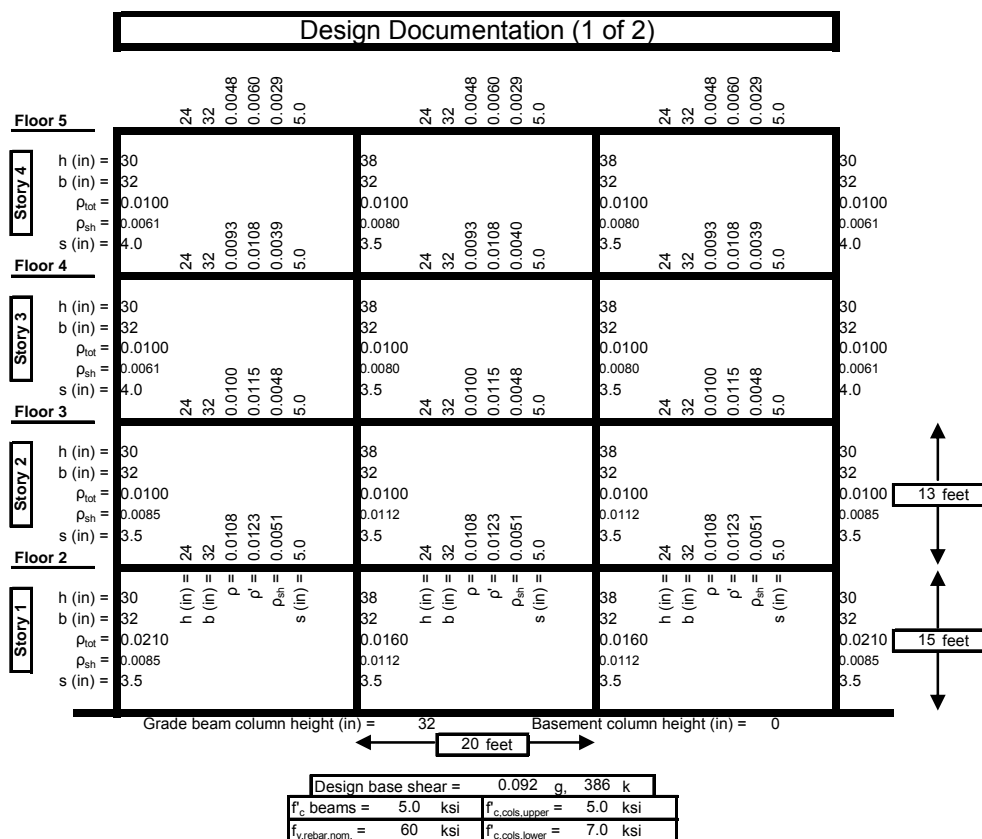
Number of Stories: 4

Fundamental Period (sec): 1.12

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined mostly by joint shear and minimum size requirements, in addition to column-beam compatibility considerations. The depth of the grade beams was increased to help alleviate joint shear concerns. Beam strengths were controlled by force demands, particularly lateral forces. Fourth floor beam strength was increased slightly to help reduce SCWB ratio at affected joints. Column strengths were determined by strong-column weak-beam (SCWB) ratios except in the first story, where flexural demands controlled. Concrete strength was increased to 7.0 ksi in the lower columns to help meet joint shear requirements. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Modeling Documentation (1 of 1)													
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	7731	13144	13144	7731							
		EI_{stf}/EI_g =	0.35	0.35	0.35	0.35							
		M_u/M_y =	1.21	1.21	1.21	1.21							
		$\Theta_{cap,pl}$ (rad) =	0.069	0.080	0.080	0.069							
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100							
Floor 4	Story 3	λ =	121	135	135	121							
		$(P/A_g f_c)_{exp}$ =	0.00	0.01	0.01	0.00							
		$M_{y,exp}$ (k-in) =	7965	13737	13737	7965							
		EI_{stf}/EI_g =	0.35	0.35	0.35	0.35							
		M_u/M_y =	1.21	1.21	1.21	1.21							
Floor 3	Story 2	$\Theta_{cap,pl}$ (rad) =	0.069	0.079	0.079	0.069							
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100							
		λ =	121	134	134	121							
		$(P/A_g f_c)_{exp}$ =	0.01	0.01	0.01	0.01							
		$M_{y,exp}$ (k-in) =	8240	14395	14395	8240							
Floor 2	Story 1	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35							
		M_u/M_y =	1.19	1.19	1.19	1.19							
		$\Theta_{cap,pl}$ (rad) =	0.075	0.083	0.083	0.075							
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100							
		λ =	126	134	134	126							
Floor 1	Story 0	$(P/A_g f_c)_{exp}$ =	0.01	0.01	0.01	0.01							
		$M_{y,exp}$ (k-in) =	16404	22262	22262	16404							
		EI_{stf}/EI_g =	0.35	0.35	0.35	0.35							
		M_u/M_y =	1.19	1.19	1.19	1.19							
		$\Theta_{cap,pl}$ (rad) =	0.082	0.086	0.086	0.082							
Floor 0	Story -1	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100							
		λ =	125	133	133	125							
		$(P/A_g f_c)_{exp}$ =	0.01	0.02	0.02	0.01							
		Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 2.69											
		Model periods (sec): T ₁ = 1.12 T ₂ = 0.33 T ₃ = 0.17											
f _{y, rebar, expected} = 67 ksi													

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

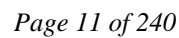
Building Design ID: 1004

Number of Stories: 4

Fundamental Period (sec): 1.11

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

All floors designed according to ground floor requirements of index 1003. Column strengths re-adjusted to have both uniform reinforcement ratios and attempt to conform to SCWB requirements, which was difficult in a perimeter frame.



Design Documentation (2 of 2)

												Design Drifts:					
Story 4	SCWB =	1.25	2.37	2.66	0.90	1.15	0.89	2.39	2.77	0.90	1.15	0.89	2.39	2.68	0.90	1.15	1.25
	Joint $\Phi V_r/V_u$ =	1.42					1.18					1.18					1.42
	$\Phi M_r/M_u$ =	3.30					2.55					2.56					3.32
	$\Phi V_r/V_{mpr}$ =	1.31					1.72					1.72					1.31
Floor 4	$P/A_g f'_c$ =	0.01					0.01					0.01					0.01
		2.49	1.56	1.69	0.90	1.15	1.79	1.64	1.76	0.90	1.15	1.79	1.57	1.70	0.90	1.15	2.49
		1.19					0.99					0.99					1.19
Story 3	$\Phi M_r/M_u$ =	2.81					2.02					2.03					2.81
	$\Phi V_r/V_{mpr}$ =	1.27					1.67					1.67					1.27
	$P/A_g f'_c$ =	0.01					0.01					0.01					0.01
		2.45	1.23	1.28	0.90	1.15	1.83	1.29	1.34	0.90	1.15	1.83	1.24	1.28	0.90	1.15	2.45
Floor 3		1.19					0.99					0.99					1.19
Story 2	$\Phi M_r/M_u$ =	2.48					1.71					1.71					2.49
	$\Phi V_r/V_{mpr}$ =	1.22					1.61					1.61					1.22
	$P/A_g f'_c$ =	0.02					0.02					0.02					0.02
		2.41	1.17	1.19	0.90	1.15	1.87	1.26	1.30	0.90	1.15	1.87	1.18	1.20	0.90	1.15	2.41
Floor 2		1.16					0.97					0.97					1.16
Story 1	$\Phi M_r/M_u$ =	1.03					1.03					1.03					1.04
	$\Phi V_r/V_{mpr}$ =	1.39					1.83					1.83					1.39
	$P/A_g f'_c$ =	0.03					0.03					0.03					0.03

Modeling Documentation (1 of 1)

Floor 5	$M_{y,exp}$ (k-in) =	15010	9797	-11790	0.35	1.098E+08	0.062	-0.068	0.100	100
	$E I_{eff}/E I_g$ =	0.35								
	M_c/M_y =	1.19								
	$\Theta_{cap,pl}$ (rad) =	0.082								
Floor 4	Θ_{pc} (rad) =	0.100								
	λ =	127								
	$(P/A_g f'_c)_{exp}$ =	0.00								
Floor 3	$M_{y,exp}$ (k-in) =	15237	9797	-11790	0.35	1.098E+08	0.062	-0.068	0.100	100
	$E I_{eff}/E I_g$ =	0.35								
	M_c/M_y =	1.19								
	$\Theta_{cap,pl}$ (rad) =	0.082								
Floor 2	Θ_{pc} (rad) =	0.100								
	λ =	126								
	$(P/A_g f'_c)_{exp}$ =	0.01								
Floor 1	$M_{y,exp}$ (k-in) =	15687	9797	-11790	0.35	1.098E+08	0.062	-0.068	0.100	100
	$E I_{eff}/E I_g$ =	0.35								
	M_c/M_y =	1.19								
	$\Theta_{cap,pl}$ (rad) =	0.081								
Story 1	Θ_{pc} (rad) =	0.100								
	λ =	125								
	$(P/A_g f'_c)_{exp}$ =	0.01								

Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 2.69

Model periods (sec): $T_1 = 1.11$ $T_2 = 0.33$ $T_3 = 0.17$ $f_{y, rebar, expected} = 67$ ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1008

Number of Stories: 4

Fundamental Period (sec): 0.94

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined mostly by minimum size requirements and column-beam compatibility. Joint shear requirements dictated beam depths. The beams' negative strengths were controlled by force demands, particularly lateral forces, while their positive strengths were dictated by the minimum positive to negative strength ratio. Column strengths were determined by strong-column weak-beam (SCWB) ratios. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Modeling Documentation (1 of 1)																										
Floor 5	Story 4	$M_{y,exp}$ (k-in) = EI_{stf}/EI_g = M_c/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	2014	-4264	0.35	9.353E+07	0.039	-0.064	0.100	100	2014	-4264	0.35	9.353E+07	0.039	-0.064	0.100	100	2014	-4264	0.35	9.353E+07	0.039	-0.064	0.100	100
			4346	0.35	1.21	0.067	0.100	91	0.03	2012	-5156	0.35	9.353E+07	0.038	-0.067	0.100	100	91	0.03	2012	-5156	0.35	9.353E+07	0.038	-0.067	0.100
Floor 4	Story 3	$M_{y,exp}$ (k-in) = EI_{stf}/EI_g = M_c/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	2012	-5156	0.35	9.353E+07	0.038	-0.067	0.100	100	2012	-5156	0.35	9.353E+07	0.038	-0.067	0.100	100	2012	-5156	0.35	9.353E+07	0.038	-0.067	0.100	100
			4984	0.35	1.20	0.064	0.100	87	0.06	2351	-6041	0.35	9.353E+07	0.039	-0.068	0.100	100	87	0.06	2351	-6041	0.35	9.353E+07	0.039	-0.068	0.100
Floor 3	Story 2	$M_{y,exp}$ (k-in) = EI_{stf}/EI_g = M_c/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	2351	-6041	0.35	9.353E+07	0.039	-0.068	0.100	100	2351	-6041	0.35	9.353E+07	0.039	-0.068	0.100	100	2351	-6041	0.35	9.353E+07	0.039	-0.068	0.100	100
			3024	0.35	1.20	0.061	0.100	83	0.10	2681	-6483	0.35	9.353E+07	0.040	-0.067	0.100	100	83	0.10	2681	-6483	0.35	9.353E+07	0.040	-0.067	0.100
Floor 2	Story 1	$M_{y,exp}$ (k-in) = EI_{stf}/EI_g = M_c/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	2681	-6483	0.35	9.353E+07	0.040	-0.067	0.100	100	2681	-6483	0.35	9.353E+07	0.040	-0.067	0.100	100	2681	-6483	0.35	9.353E+07	0.040	-0.067	0.100	100
			4623	0.35	1.20	0.063	0.100	87	0.06	2681	-6483	0.35	9.353E+07	0.040	-0.067	0.100	100	87	0.06	2681	-6483	0.35	9.353E+07	0.040	-0.067	0.100
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60																										
Model periods (sec): T ₁ = 0.94 T ₂ = 0.30 T ₃ = 0.17																										
f _{y, rebar, expected} = 67 ksi																										

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1009

Number of Stories: 4

Fundamental Period (sec): 1.16

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined mostly by joint shear requirements, in addition to column-beam compatibility considerations. The depth of the grade beams was increased to help alleviate joint shear concerns. Beam strengths were

Design Documentation (1 of 2)

Floor 5

Story 4

Floor 4

Story 3

Floor 3

Story 2

Floor 2

Story 1

Grade beam column height (in) = 36

Basement column height (in) = 0

30 feet

13 feet

15 feet

Design base shear = 0.092 g, 580 k

f'_c beams = 6.0 ksi

f'_c cols upper = 5.0 ksi

$f'_{y, \text{rebar, nom}}$ = 60 ksi

f'_c cols lower = 8.0 ksi

Design Documentation (2 of 2)																		
Story 4	SCWB =	0.78	1.18	1.17	0.55	-2.29	0.79	1.21	1.31	0.55	-2.29	0.79	1.18	1.19	0.55	-2.29	0.78	Design Drifts:
	Joint $\Phi V_r/V_u =$	2.04					2.11					2.11					2.04	
	$\Phi M_r/M_u =$	1.36					1.42					1.42					1.37	
	$\Phi V_r/V_{mpr} =$	1.32					1.51					1.51					1.32	1.0%
Floor 4	$P/A_g f_c =$	0.01					0.02					0.02					0.01	
		1.35	1.18	1.20	0.67	1.15	1.23	1.20	1.26	0.67	1.16	1.23	1.19	1.21	0.67	1.15	1.35	
		1.07					1.02					1.02					1.07	
Story 3	$\Phi M_r/M_u =$	1.77					1.48					1.48					1.77	
	$\Phi V_r/V_{mpr} =$	1.09					1.23					1.23					1.09	1.5%
	$P/A_g f_c =$	0.03					0.04					0.04					0.03	
		1.38	1.08	1.09	0.75	1.21	1.33	1.10	1.15	0.75	1.21	1.33	1.08	1.10	0.75	1.21	1.38	
Floor 3		1.18					1.07					1.07					1.18	
Story 2	$\Phi M_r/M_u =$	1.35					1.40					1.40					1.35	
	$\Phi V_r/V_{mpr} =$	1.23					1.23					1.23					1.23	1.8%
	$P/A_g f_c =$	0.03					0.04					0.04					0.03	
		1.72	1.05	1.05	0.77	1.23	1.58	1.09	1.16	0.77	1.24	1.58	1.06	1.06	0.77	1.23	1.73	
Floor 2		1.08					0.97					0.97					1.08	
Story 1	$\Phi M_r/M_u =$	1.11					1.09					1.09					1.12	
	$\Phi V_r/V_{mpr} =$	0.89					1.16					1.16					0.89	1.6%
	$P/A_g f_c =$	0.04					0.05					0.05					0.04	

Design Drifts:

1.0%

1.5%

1.8%

1.6%

Modeling Documentation (1 of 1)																					
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	10031	5703	-12303	0.35	2.977E+08	0.028	-0.040	0.063	99	14669	5703	-12303	0.35	2.977E+08	0.028	-0.040	0.063	99	10031
		$E I_{eff}/E I_g$ =	0.35										0.35								0.35
		M_c/M_y =	1.21										1.21								1.21
		$\Theta_{cap,pl}$ (rad) =	0.068										0.076								0.068
		Θ_{pc} (rad) =	0.100										0.100								0.100
Floor 4	Story 3	λ =	121	10165	-17013	0.35	2.977E+08	0.045	-0.058	0.100	108	134	10165	-17013	0.35	2.977E+08	0.046	-0.058	0.100	108	121
		$(P/A_g f_c)_{exp}$ =	0.01									0.01									0.01
		$M_{y,exp}$ (k-in) =	14364	10165	-17013	0.35	2.977E+08	0.045	-0.058	0.100	108	19841	10165	-17013	0.35	2.977E+08	0.046	-0.058	0.100	108	14364
		$E I_{eff}/E I_g$ =	0.35									0.35									0.35
		M_c/M_y =	1.21									1.21									1.21
Floor 3	Story 2	$\Theta_{cap,pl}$ (rad) =	0.075	13020	-19294	0.35	2.977E+08	0.049	-0.059	0.100	103	0.079	13020	-19294	0.35	2.977E+08	0.049	-0.059	0.100	103	0.075
		Θ_{pc} (rad) =	0.100									0.100									0.100
		λ =	119	13020	-19294	0.35	2.977E+08	0.049	-0.059	0.100	103	131	13020	-19294	0.35	2.977E+08	0.049	-0.059	0.100	103	119
		$(P/A_g f_c)_{exp}$ =	0.02									0.03									0.02
		$M_{y,exp}$ (k-in) =	14974	14095	-20130	0.35	2.977E+08	0.053	-0.063	0.100	108	25132	14095	-20130	0.35	2.977E+08	0.054	-0.063	0.100	108	14974
Floor 2	Story 1	$E I_{eff}/E I_g$ =	0.35									0.35									0.35
		M_c/M_y =	1.18									1.18									1.18
		$\Theta_{cap,pl}$ (rad) =	0.071									0.079									0.071
		Θ_{pc} (rad) =	0.100									0.100									0.100
		λ =	120	13020	-19294	0.35	2.977E+08	0.053	-0.063	0.100	108	132	13020	-19294	0.35	2.977E+08	0.054	-0.063	0.100	108	120
<div>Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 4.03</div> <div>Model periods (sec): T₁ = 1.16 T₂ = 0.35 T₃ = 0.18</div> <div>I_{v, rehab. expected} = 67 ksi</div>																					

Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 4.03

Model periods (sec): $T_1 = 1.16$ $T_2 = 0.35$ $T_3 = 0.18$

$f_{y, rebar, expected} = 67$ ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1010

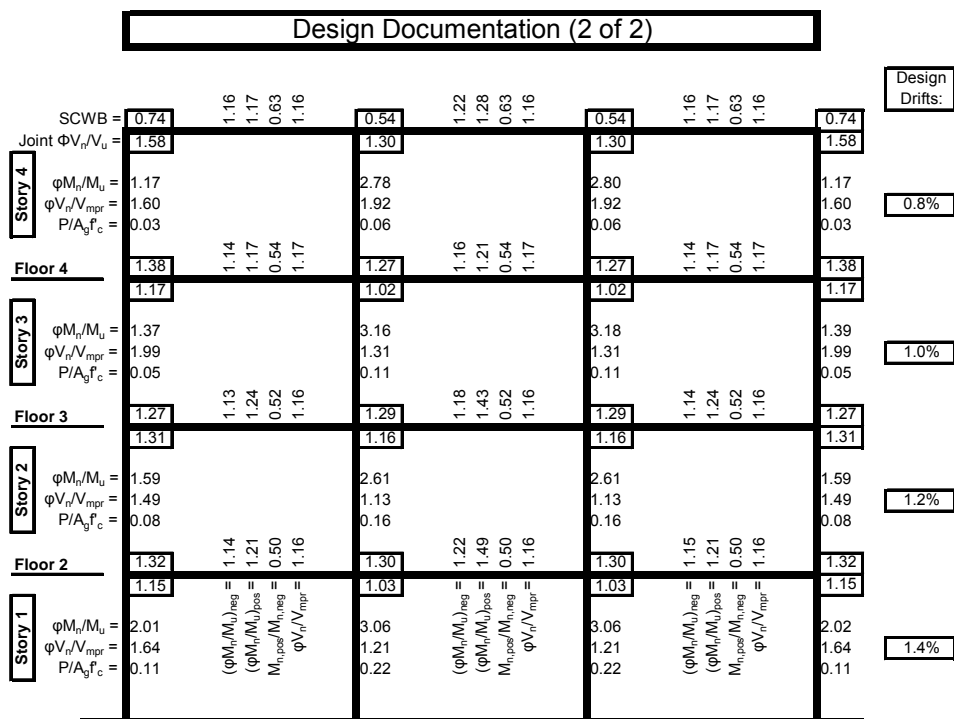
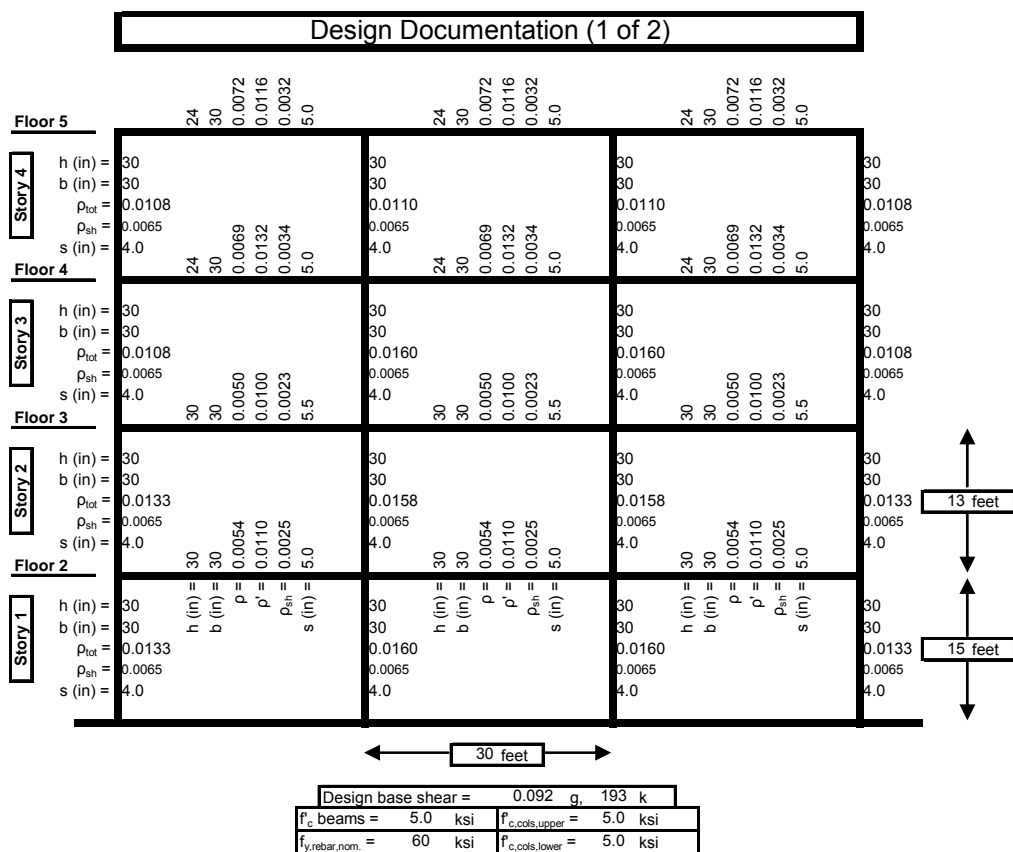
Number of Stories: 4

Fundamental Period (sec): 0.86

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined by joint shear requirements. Beam strengths were controlled by force demands, particularly lateral forces. Column strengths were determined by strong-column weak-beam (SCWB) ratios. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Modeling Documentation (1 of 1)											
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	3583	9770	9770	3583					
		EI_{stf}/EI_g =	0.35	0.35	0.35	0.35					
Floor 4	Story 3	M_c/M_y =	1.21	1.20	1.20	1.21					
		$\Theta_{cap,pl}$ (rad) =	0.069	0.067	0.067	0.069					
Floor 3	Story 2	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
		λ =	119	116	116	119					
Floor 2	Story 1	$(P/A_g f_c)_{exp}$ =	0.02	0.04	0.04	0.02					
		$M_{y,exp}$ (k-in) =	3602	15072	15072	3602					
Floor 1	Story 0	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35					
		M_c/M_y =	1.20	1.20	1.20	1.20					
Floor 0	Story -1	$\Theta_{cap,pl}$ (rad) =	0.066	0.065	0.065	0.066					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor -1	Story -2	λ =	116	110	110	116					
		$(P/A_g f_c)_{exp}$ =	0.04	0.08	0.08	0.04					
Floor -2	Story -3	$M_{y,exp}$ (k-in) =	12270	16800	16800	12270					
		EI_{stf}/EI_g =	0.35	0.35	0.35	0.35					
Floor -3	Story -4	M_c/M_y =	1.20	1.19	1.19	1.20					
		$\Theta_{cap,pl}$ (rad) =	0.066	0.060	0.060	0.066					
Floor -4	Story -5	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
		λ =	113	105	105	113					
Floor -5	Story -6	$(P/A_g f_c)_{exp}$ =	0.06	0.12	0.12	0.06					
		$M_{y,exp}$ (k-in) =	13243	18812	18812	13243					
Floor -6	Story -7	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35					
		M_c/M_y =	1.20	1.19	1.19	1.20					
Floor -7	Story -8	$\Theta_{cap,pl}$ (rad) =	0.063	0.056	0.056	0.063					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor -8	Story -9	λ =	110	100	100	110					
		$(P/A_g f_c)_{exp}$ =	0.08	0.15	0.15	0.08					
Mass tributary to one frame for lateral load (each floor) (k-s/in): 1.34											
Model periods (sec): $T_1 = 0.86$ $T_2 = 0.27$ $T_3 = 0.15$											
f_y rebar expected = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1011

Number of Stories: 8

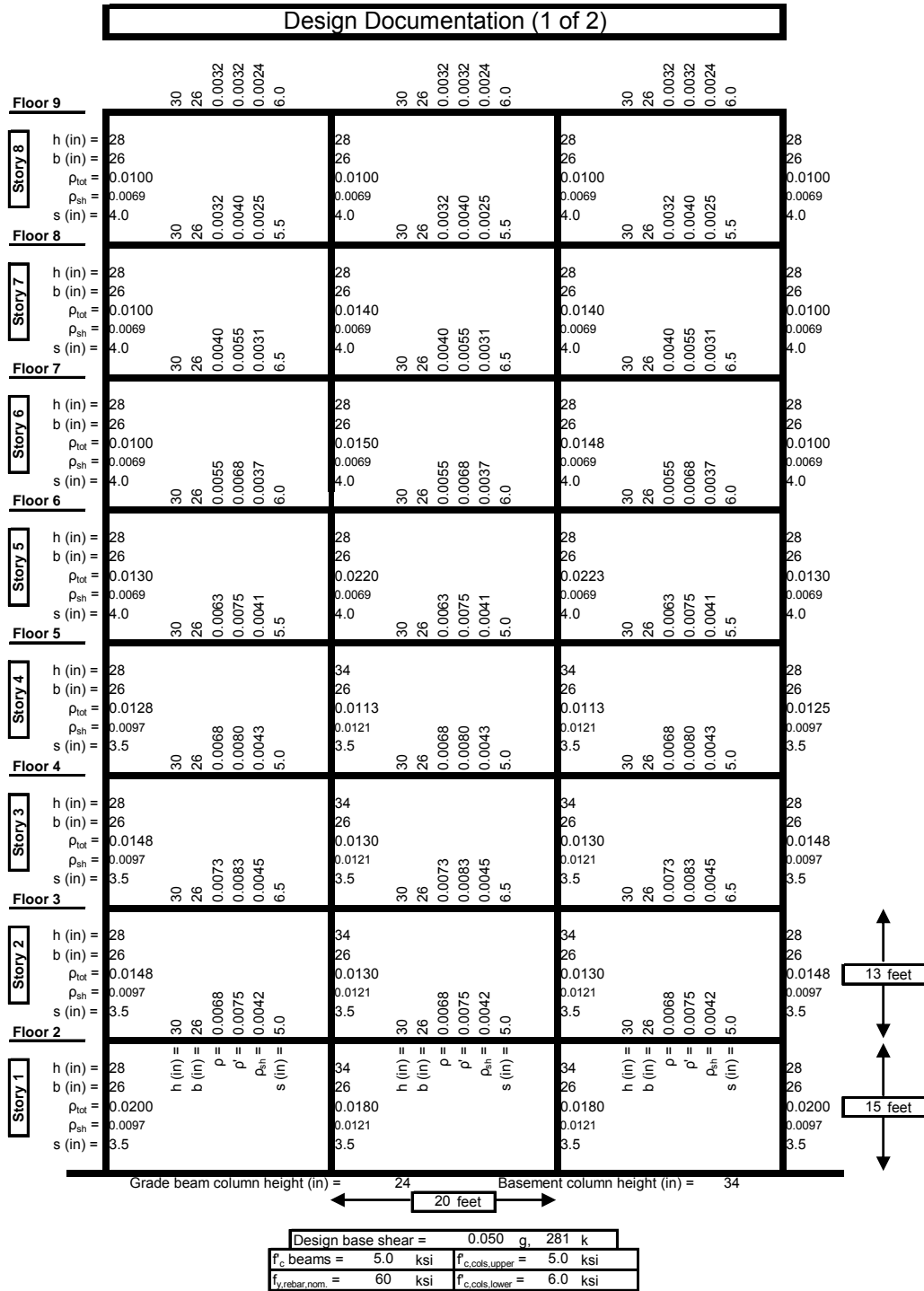
Fundamental Period (sec): 1.71

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined by joint shear requirements, particularly in the lower stories. Negative beam strengths were controlled by force demands, especially lateral forces, while positive beam strengths were dictated by the

minimum positive-to-negative strength ratio. Column strengths were determined by strong-column weak-beam (SCWB) ratios except in the bottom floor columns, where strength was determined by flexural demand. Column concrete was stepped up to 6.0 ksi in the lower floors to meet joint shear requirements. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Page 23 of 240

Page 24 of 240

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1012

Number of Stories: 8

Fundamental Period (sec): 1.80

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined by joint shear requirements. Beam strengths were controlled by force demands, particularly lateral forces. Column strengths were determined by strong-column weak-beam (SCWB) ratios. Column concrete was stepped up to 6.0 ksi in all stories to meet joint shear requirements. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Page 27 of 240

Page 28 of 240

Modeling Documentation (1 of 1)									
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	3421	1770	3765E+07	0.0542	0.086	0.100	101
		$E I_{eff}/E I_g$ =	0.35	-3664	0.35	0.0542	0.086	0.100	101
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	3753	1920	3765E+07	0.054	0.092	0.100	101
		$E I_{eff}/E I_g$ =	0.35	-4856	0.35	0.054	0.092	0.100	101
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	4080	2081	3765E+07	0.056	0.092	0.100	101
		$E I_{eff}/E I_g$ =	0.35	-5078	0.35	0.056	0.092	0.100	101
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	4402	2819	7.178E+07	0.046	0.075	0.100	100
		$E I_{eff}/E I_g$ =	0.36	-6744	0.35	0.046	0.075	0.100	100
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	4721	2819	7.178E+07	0.046	0.075	0.100	100
		$E I_{eff}/E I_g$ =	0.36	-6744	0.35	0.046	0.075	0.100	100
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	5034	2819	7.178E+07	0.046	0.075	0.100	100
		$E I_{eff}/E I_g$ =	0.36	-6744	0.35	0.046	0.075	0.100	100
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	5225	2819	7.178E+07	0.046	0.075	0.100	100
		$E I_{eff}/E I_g$ =	0.36	-6744	0.35	0.046	0.075	0.100	100
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	5532	2819	7.178E+07	0.046	0.075	0.100	100
		$E I_{eff}/E I_g$ =	0.36	-6744	0.35	0.046	0.075	0.100	100
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60 Model periods (sec): $T_1 = 1.80$ $T_2 = 0.60$ $T_3 = 0.34$ $f_y, \text{rebar, expected} = 67$ ksi									

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1013

Number of Stories: 12

Fundamental Period (sec): 2.01

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined by both drift limits and joint shear requirements. Negative beam strengths were controlled by force demands, particularly lateral forces, while positive beam strengths were determined by the minimum positive-to-negative strength ratio. Column strengths were determined by strong-column weak-beam (SCWB) ratios except in the lower story, where flexural demand controlled the design. Column concrete was stepped up to 7.0 ksi in the lower floors to meet joint shear requirements. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)											
Floor 13											
Story 12	h (in) =	32	32	28	0.0033	32	32	28	0.0033	32	32
	b (in) =	28	28	28	0.0033	28	28	28	0.0033	28	28
	ρ_{tot} =	0.0100	0.0100	0.0096	0.0100	0.0100	0.0096	0.0100	0.0100	0.0096	0.0100
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 12											
Story 11	h (in) =	32	32	28	0.0035	32	32	28	0.0035	32	32
	b (in) =	28	28	28	0.0043	28	28	28	0.0043	28	28
	ρ_{tot} =	0.0100	0.0100	0.0099	0.0100	0.0100	0.0099	0.0100	0.0100	0.0099	0.0100
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 11											
Story 10	h (in) =	32	32	28	0.0053	32	32	28	0.0053	32	32
	b (in) =	28	28	28	0.0065	28	28	28	0.0065	28	28
	ρ_{tot} =	0.0100	0.0100	0.0129	0.0100	0.0100	0.0129	0.0100	0.0100	0.0129	0.0100
	ρ_{sh} =	0.0084	0.0084	0.0084	0.0084	0.0084	0.0084	0.0084	0.0084	0.0084	0.0084
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 10											
Story 9	h (in) =	32	32	28	0.0037	32	32	28	0.0037	32	32
	b (in) =	28	28	28	0.0048	28	28	28	0.0048	28	28
	ρ_{tot} =	0.0100	0.0100	0.0111	0.0100	0.0100	0.0111	0.0100	0.0100	0.0111	0.0100
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 9											
Story 8	h (in) =	32	32	28	0.0040	32	32	28	0.0040	32	32
	b (in) =	28	28	28	0.0050	28	28	28	0.0050	28	28
	ρ_{tot} =	0.0105	0.0105	0.0075	0.0105	0.0105	0.0075	0.0105	0.0105	0.0075	0.0105
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 8											
Story 7	h (in) =	32	32	28	0.0052	32	32	28	0.0052	32	32
	b (in) =	28	28	28	0.0063	28	28	28	0.0063	28	28
	ρ_{tot} =	0.0130	0.0130	0.0075	0.0130	0.0130	0.0075	0.0130	0.0130	0.0075	0.0130
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 7											
Story 6	h (in) =	32	32	28	0.0052	32	32	28	0.0052	32	32
	b (in) =	28	28	28	0.0063	28	28	28	0.0063	28	28
	ρ_{tot} =	0.0140	0.0140	0.0075	0.0140	0.0140	0.0075	0.0140	0.0140	0.0075	0.0140
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 6											
Story 5	h (in) =	32	32	28	0.0057	32	32	28	0.0057	32	32
	b (in) =	28	28	28	0.0066	28	28	28	0.0066	28	28
	ρ_{tot} =	0.0150	0.0150	0.0075	0.0150	0.0150	0.0075	0.0150	0.0150	0.0075	0.0150
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 5											
Story 4	h (in) =	32	32	28	0.0060	32	32	28	0.0060	32	32
	b (in) =	28	28	28	0.0068	28	28	28	0.0068	28	28
	ρ_{tot} =	0.0155	0.0155	0.0075	0.0155	0.0155	0.0075	0.0155	0.0155	0.0075	0.0155
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 4											
Story 3	h (in) =	32	32	28	0.0060	32	32	28	0.0060	32	32
	b (in) =	28	28	28	0.0068	28	28	28	0.0068	28	28
	ρ_{tot} =	0.0165	0.0165	0.0075	0.0165	0.0165	0.0075	0.0165	0.0165	0.0075	0.0165
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 3											
Story 2	h (in) =	32	32	28	0.0061	32	32	28	0.0061	32	32
	b (in) =	28	28	28	0.0066	28	28	28	0.0066	28	28
	ρ_{tot} =	0.0170	0.0170	0.0075	0.0170	0.0170	0.0075	0.0170	0.0170	0.0075	0.0170
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 2											
Story 1	h (in) =	32	32	28	0.0061	32	32	28	0.0061	32	32
	b (in) =	28	28	28	0.0066	28	28	28	0.0066	28	28
	ρ_{tot} =	0.0230	0.0230	0.0075	0.0230	0.0230	0.0075	0.0230	0.0230	0.0075	0.0230
	ρ_{sh} =	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075	0.0075
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Grade beam column height (in) = 24											
Basement column height (in) = 32											
20 feet											
Design base shear = 0.044 g, 368 k											
f'_c beams = 5.0 ksi f'_c cols, upper = 5.0 ksi f_y rebar, nom = 60 ksi f'_c cols, lower = 7.0 ksi											
<div>13 feet</div> <div>15 feet</div>											

Page 33 of 240

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) = 7808 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.21 $\Theta_{cap,p}$ (rad) = 0.078 Θ_{pc} (rad) = 0.100 λ = 129 $(P/A_g^f c)_{exp}$ = 0.00	4981 -6358 0.35 2.379E+08 0.0389 -0.044 0.086 111	8058 0.35 1.21 0.086 0.100 128 0.01	4981 -6358 0.35 2.379E+08 0.0389 -0.044 0.086 111	8058 0.35 1.21 0.086 0.100 128 0.01	4981 -6358 0.35 2.379E+08 0.0389 -0.044 0.086 111	8058 0.35 1.21 0.086 0.100 128 0.01	4981 -6358 0.35 2.379E+08 0.0389 -0.044 0.086 111	7808 0.35 1.21 0.078 0.100 129 0.00	
Floor 12	Story 11	$M_{y,exp}$ (k-in) = 8058 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.21 $\Theta_{cap,p}$ (rad) = 0.077 Θ_{pc} (rad) = 0.100 λ = 128 $(P/A_g^f c)_{exp}$ = 0.01	5339 -7812 0.35 2.379E+08 0.041 -0.048 0.090 116	9282 0.35 1.21 0.086 0.100 127 0.02	5339 -7812 0.35 2.379E+08 0.041 -0.048 0.090 116	9282 0.35 1.21 0.086 0.100 127 0.02	5339 -7812 0.35 2.379E+08 0.041 -0.048 0.090 116	9282 0.35 1.21 0.086 0.100 127 0.02	5339 -7812 0.35 2.379E+08 0.041 -0.048 0.090 116	8058 0.35 1.21 0.077 0.100 128 0.01	
Floor 11	Story 10	$M_{y,exp}$ (k-in) = 8306 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.21 $\Theta_{cap,p}$ (rad) = 0.078 Θ_{pc} (rad) = 0.100 λ = 123 $(P/A_g^f c)_{exp}$ = 0.01	6453 -9273 0.35 2.379E+08 0.039 -0.046 0.100 103	11221 0.35 1.21 0.096 0.100 126 0.03	6453 -9273 0.35 2.379E+08 0.039 -0.046 0.100 103	11221 0.35 1.21 0.096 0.100 126 0.03	6453 -9273 0.35 2.379E+08 0.039 -0.046 0.100 103	11221 0.35 1.21 0.096 0.100 126 0.03	6453 -9273 0.35 2.379E+08 0.039 -0.046 0.100 103	8306 0.35 1.21 0.078 0.100 123 0.01	
Floor 10	Story 9	$M_{y,exp}$ (k-in) = 8553 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.21 $\Theta_{cap,p}$ (rad) = 0.076 Θ_{pc} (rad) = 0.100 λ = 127 $(P/A_g^f c)_{exp}$ = 0.02	7930 -11091 0.35 2.379E+08 0.046 -0.054 0.100 111	13867 0.35 1.21 0.091 0.100 124 0.03	7930 -11091 0.35 2.379E+08 0.046 -0.054 0.100 111	13867 0.35 1.21 0.091 0.100 124 0.03	7930 -11091 0.35 2.379E+08 0.046 -0.054 0.100 111	13867 0.35 1.21 0.091 0.100 124 0.03	7930 -11091 0.35 2.379E+08 0.046 -0.054 0.100 111	8553 0.35 1.21 0.076 0.100 127 0.02	
Floor 9	Story 8	$M_{y,exp}$ (k-in) = 9163 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.21 $\Theta_{cap,p}$ (rad) = 0.076 Θ_{pc} (rad) = 0.100 λ = 126 $(P/A_g^f c)_{exp}$ = 0.02	8230 -12091 0.35 3.838E+08 0.042 -0.047 0.100 112	14338 0.35 1.20 0.076 0.100 123 0.04	8230 -12091 0.35 3.838E+08 0.042 -0.047 0.100 112	14338 0.35 1.20 0.076 0.100 123 0.04	8230 -12091 0.35 3.838E+08 0.042 -0.047 0.100 112	14338 0.35 1.20 0.076 0.100 123 0.04	8230 -12091 0.35 3.838E+08 0.042 -0.047 0.100 112	9163 0.35 1.21 0.076 0.100 126 0.02	
Floor 8	Story 7	$M_{y,exp}$ (k-in) = 11221 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.21 $\Theta_{cap,p}$ (rad) = 0.077 Θ_{pc} (rad) = 0.100 λ = 126 $(P/A_g^f c)_{exp}$ = 0.03	8771 -12624 0.35 3.838E+08 0.051 -0.058 0.100 123	17681 0.35 1.20 0.078 0.100 121 0.05	8771 -12624 0.35 3.838E+08 0.0507 -0.058 0.100 123	17681 0.35 1.20 0.078 0.100 121 0.05	8771 -12624 0.35 3.838E+08 0.0507 -0.058 0.100 123	17681 0.35 1.20 0.078 0.100 121 0.05	8771 -12624 0.35 3.838E+08 0.0507 -0.058 0.100 123	11221 0.35 1.21 0.077 0.100 126 0.03	
Floor 7	Story 6	$M_{y,exp}$ (k-in) = 12251 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.19 $\Theta_{cap,p}$ (rad) = 0.072 Θ_{pc} (rad) = 0.100 λ = 126 $(P/A_g^f c)_{exp}$ = 0.02	11506 -15282 0.35 3.838E+08 0.051 -0.058 0.100 123	19678 0.35 1.19 0.074 0.100 123 0.04	11506 -15282 0.35 3.838E+08 0.0507 -0.058 0.100 123	19678 0.35 1.19 0.074 0.100 123 0.04	11506 -15282 0.35 3.838E+08 0.0507 -0.058 0.100 123	19678 0.35 1.19 0.074 0.100 123 0.04	11506 -15282 0.35 3.838E+08 0.0507 -0.058 0.100 123	12251 0.35 1.19 0.072 0.100 126 0.02	
Floor 6	Story 5	$M_{y,exp}$ (k-in) = 13217 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.19 $\Theta_{cap,p}$ (rad) = 0.072 Θ_{pc} (rad) = 0.100 λ = 126 $(P/A_g^f c)_{exp}$ = 0.02	12531 -15995 0.35 3.838E+08 0.049 -0.055 0.100 116	20140 0.35 1.19 0.073 0.100 122 0.05	12531 -15995 0.35 3.838E+08 0.0486 -0.055 0.100 116	20140 0.35 1.19 0.073 0.100 122 0.05	12531 -15995 0.35 3.838E+08 0.0486 -0.055 0.100 116	20140 0.35 1.19 0.073 0.100 122 0.05	12531 -15995 0.35 3.838E+08 0.0486 -0.055 0.100 116	13217 0.35 1.19 0.072 0.100 126 0.02	
Floor 5	Story 4	$M_{y,exp}$ (k-in) = 13818 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.19 $\Theta_{cap,p}$ (rad) = 0.072 Θ_{pc} (rad) = 0.100 λ = 125 $(P/A_g^f c)_{exp}$ = 0.03	13066 -16525 0.35 3.838E+08 0.050 -0.055 0.100 116	20600 0.35 1.19 0.072 0.100 121 0.06	13066 -16525 0.35 3.838E+08 0.050 -0.055 0.100 116	20600 0.35 1.19 0.072 0.100 121 0.06	13066 -16525 0.35 3.838E+08 0.050 -0.055 0.100 116	20600 0.35 1.19 0.072 0.100 121 0.06	13066 -16525 0.35 3.838E+08 0.050 -0.055 0.100 116	13818 0.35 1.19 0.072 0.100 125 0.03	
Floor 4	Story 3	$M_{y,exp}$ (k-in) = 14779 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.19 $\Theta_{cap,p}$ (rad) = 0.072 Θ_{pc} (rad) = 0.100 λ = 125 $(P/A_g^f c)_{exp}$ = 0.03	13111 -16526 0.35 3.838E+08 0.050 -0.055 0.100 116	21057 0.35 1.19 0.071 0.100 120 0.06	13111 -16526 0.35 3.838E+08 0.050 -0.055 0.100 116	21057 0.35 1.19 0.071 0.100 120 0.06	13111 -16526 0.35 3.838E+08 0.050 -0.055 0.100 116	21057 0.35 1.19 0.071 0.100 120 0.06	13111 -16526 0.35 3.838E+08 0.050 -0.055 0.100 116	14779 0.35 1.19 0.072 0.100 125 0.03	
Floor 3	Story 2	$M_{y,exp}$ (k-in) = 15376 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.19 $\Theta_{cap,p}$ (rad) = 0.072 Θ_{pc} (rad) = 0.100 λ = 124 $(P/A_g^f c)_{exp}$ = 0.03	13306 -16004 0.35 3.838E+08 0.050 -0.054 0.100 116	21512 0.35 1.18 0.070 0.100 119 0.07	13306 -16004 0.35 3.838E+08 0.050 -0.054 0.100 116	21512 0.35 1.18 0.070 0.100 119 0.07	13306 -16004 0.35 3.838E+08 0.050 -0.054 0.100 116	21512 0.35 1.18 0.070 0.100 119 0.07	13306 -16004 0.35 3.838E+08 0.050 -0.054 0.100 116	15376 0.35 1.19 0.072 0.100 124 0.03	
Floor 2	Story 1	$M_{y,exp}$ (k-in) = 19933 E_{stiff}/E_{lg} = 0.35 M_o/M_y = 1.19 $\Theta_{cap,p}$ (rad) = 0.075 Θ_{pc} (rad) = 0.100 λ = 124 $(P/A_g^f c)_{exp}$ = 0.04	13306 $M_{y,post,exp}$ (k-in) = 13306 $M_{y,sub,exp}$ (k-in) = -16004 E_{stiff}/E_{lg} = 0.35 $E_{stiff} w/ SAs$ (k-in/rad) = 3.838E+08 $(\Theta_{cap,p})_{exp}$ (rad) = 0.050 $(\Theta_{cap,p})_{sub}$ (rad) = -0.054 Θ_{pc} (rad) = 0.100 λ = 118	21964 0.35 1.18 0.070 0.100 118 0.07	13306 $M_{y,post,exp}$ (k-in) = 13306 $M_{y,sub,exp}$ (k-in) = -16004 E_{stiff}/E_{lg} = 0.35 $E_{stiff} w/ SAs$ (k-in/rad) = 3.838E+08 $(\Theta_{cap,p})_{exp}$ (rad) = 0.050 $(\Theta_{cap,p})_{sub}$ (rad) = -0.054 Θ_{pc} (rad) = 0.100 λ = 118	21964 0.35 1.18 0.070 0.100 118 0.07	13306 $M_{y,post,exp}$ (k-in) = 13306 $M_{y,sub,exp}$ (k-in) = -16004 E_{stiff}/E_{lg} = 0.35 $E_{stiff} w/ SAs$ (k-in/rad) = 3.838E+08 $(\Theta_{cap,p})_{exp}$ (rad) = 0.050 $(\Theta_{cap,p})_{sub}$ (rad) = -0.054 Θ_{pc} (rad) = 0.100 λ = 118	21964 0.35 1.18 0.070 0.100 118 0.07	13306 $M_{y,post,exp}$ (k-in) = 13306 $M_{y,sub,exp}$ (k-in) = -16004 E_{stiff}/E_{lg} = 0.35 $E_{stiff} w/ SAs$ (k-in/rad) = 3.838E+08 $(\Theta_{cap,p})_{exp}$ (rad) = 0.050 $(\Theta_{cap,p})_{sub}$ (rad) = -0.054 Θ_{pc} (rad) = 0.100 λ = 118	19933 0.35 1.19 0.075 0.100 124 0.04	
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.79											
Model periods (sec): $T_1 = 2.01$ $T_2 = 0.68$ $T_3 = 0.39$											
$f_{lateral, expected} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1014

Number of Stories: 12

Fundamental Period (sec): 2.14

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined by drift limits and joint shear requirements. Beam strengths were controlled by force demands, particularly lateral forces. Column strengths were determined by strong-column weak-beam (SCWB) ratios. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)												
Floor 13												
Story 12	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0120	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0120
	ρ_{br} =	0.0070	0.0041	0.0083	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0070
Floor 12												
Story 11	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0120	0.0041	0.0083	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0120
	ρ_{br} =	0.0070	0.0041	0.0083	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0070
Floor 11												
Story 10	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0120	0.0041	0.0083	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0120
	ρ_{br} =	0.0070	0.0041	0.0083	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0070
Floor 10												
Story 9	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0135	0.0053	0.0103	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0135
	ρ_{br} =	0.0070	0.0053	0.0103	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0070
Floor 9												
Story 8	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0135	0.0053	0.0103	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0135
	ρ_{br} =	0.0070	0.0053	0.0103	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0037	0.0070
Floor 8												
Story 7	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0155	0.0040	0.0078	0.0048	0.0048	0.0048	0.0048	0.0048	0.0048	0.0048	0.0155
	ρ_{br} =	0.0070	0.0040	0.0078	0.0048	0.0048	0.0048	0.0048	0.0048	0.0048	0.0048	0.0070
Floor 7												
Story 6	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0155	0.0040	0.0080	0.0048	0.0048	0.0048	0.0048	0.0048	0.0048	0.0048	0.0155
	ρ_{br} =	0.0070	0.0040	0.0080	0.0048	0.0048	0.0048	0.0048	0.0048	0.0048	0.0048	0.0070
Floor 6												
Story 5	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0155	0.0040	0.0081	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0155
	ρ_{br} =	0.0070	0.0040	0.0081	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0070
Floor 5												
Story 4	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0145	0.0040	0.0081	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0145
	ρ_{br} =	0.0070	0.0040	0.0081	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0070
Floor 4												
Story 3	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0145	0.0042	0.0078	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0145
	ρ_{br} =	0.0070	0.0042	0.0078	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0070
Floor 3												
Story 2	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0145	0.0048	0.0076	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0145
	ρ_{br} =	0.0070	0.0048	0.0076	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0070
Floor 2												
Story 1	h (in) =	22	22	22	22	22	22	22	22	22	22	22
	b (in) =	22	22	22	22	22	22	22	22	22	22	22
	ρ_{br} =	0.0145	0.0048	0.0076	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0145
	ρ_{br} =	0.0070	0.0048	0.0076	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0049	0.0070
Basement												
Grade beam column height (in) = 24												
Basement column height (in) = 22												
Design base shear = 0.044 g, 123 k												
f'_c beams = 5.0 ksi f'_c cols upper = 5.0 ksi f'_c rebars nom. = 60 ksi f'_c cols lower = 5.0 ksi												

Design Documentation (2 of 2)															
	SCWB =	0.93	1.13	1.13	4.59	0.77	1.14	1.29	4.59	0.77	1.13	1.13	4.59	0.93	Design Drifts:
Joint	$\Phi V_r/V_u$	2.68				2.09				2.09				2.68	
Story 12	$\Phi M_r/M_u$	1.39				3.27				3.27				1.39	0.8%
	$\Phi V_r/V_{mpr}$	2.28				1.72				1.72				2.28	
	$P/A_g f_c$	0.02				0.05				0.05				0.02	
Floor 12		1.36	1.14	1.35	2.94	1.25	1.33	1.40	2.94	1.25	1.14	1.35	2.94	1.36	1.2%
		1.47				1.30				1.30				1.47	
Story 11	$\Phi M_r/M_u$	1.66				2.71				2.71				1.66	1.2%
	$\Phi V_r/V_{mpr}$	2.04				2.14				2.14				2.04	
	$P/A_g f_c$	0.05				0.08				0.08				0.05	
Floor 11		1.38	1.12	1.41	2.52	1.30	1.29	1.51	2.52	1.30	1.12	1.41	2.52	1.38	1.5%
		1.31				1.15				1.15				1.31	
Story 10	$\Phi M_r/M_u$	1.57				2.46				2.46				1.57	1.6%
	$\Phi V_r/V_{mpr}$	2.36				1.75				1.75				2.36	
	$P/A_g f_c$	0.07				0.13				0.13				0.07	
Floor 10		1.40	1.14	1.45	2.22	1.41	1.30	1.59	2.22	1.41	1.14	1.45	2.22	1.40	1.5%
		1.16				1.02				1.02				1.16	
Story 9	$\Phi M_r/M_u$	1.27				1.93				1.93				1.28	1.6%
	$\Phi V_r/V_{mpr}$	2.57				1.8				1.8				2.57	
	$P/A_g f_c$	0.10				0.17				0.17				0.10	
Floor 9		1.21	1.14	1.41	3.00	1.20	1.43	1.52	3.00	1.20	1.14	1.41	3.00	1.21	1.5%
		1.26				1.13				1.13				1.26	
Story 8	$\Phi M_r/M_u$	1.51				1.90				1.90				1.51	1.6%
	$\Phi V_r/V_{mpr}$	2.47				1.98				1.98				2.47	
	$P/A_g f_c$	0.13				0.22				0.22				0.13	
Floor 8		1.31	1.12	1.37	3.00	1.24	1.37	1.48	3.00	1.24	1.12	1.37	3.00	1.31	1.7%
		1.26				1.13				1.13				1.26	
Story 7	$\Phi M_r/M_u$	1.46				1.88				1.88				1.46	1.8%
	$\Phi V_r/V_{mpr}$	2.37				1.88				1.88				2.37	
	$P/A_g f_c$	0.16				0.26				0.26				0.16	
Floor 7		1.30	1.12	1.40	1.15	1.24	1.35	1.47	1.15	1.24	1.12	1.40	1.15	1.30	1.9%
		1.22				1.08				1.08				1.22	
Story 6	$\Phi M_r/M_u$	1.46				1.83				1.83				1.46	1.9%
	$\Phi V_r/V_{mpr}$	2.25				1.78				1.78				2.25	
	$P/A_g f_c$	0.19				0.31				0.31				0.19	
Floor 6		1.30	1.13	1.31	1.15	1.26	1.35	1.39	1.15	1.26	1.13	1.31	1.15	1.30	1.9%
		1.18				1.05				1.05				1.18	
Story 5	$\Phi M_r/M_u$	1.47				1.88				1.88				1.47	1.9%
	$\Phi V_r/V_{mpr}$	2.14				1.82				1.82				2.14	
	$P/A_g f_c$	0.22				0.35				0.35				0.22	
Floor 5		1.29	1.14	1.18	1.16	1.30	1.33	1.35	1.16	1.30	1.14	1.18	1.16	1.29	1.9%
		1.17				1.05				1.05				1.17	
Story 4	$\Phi M_r/M_u$	1.43				1.87				1.87				1.43	1.8%
	$\Phi V_r/V_{mpr}$	2.11				1.93				1.93				2.11	
	$P/A_g f_c$	0.25				0.40				0.40				0.25	
Floor 4		1.28	1.14	1.10	1.16	1.33	1.32	1.34	1.16	1.33	1.14	1.10	1.16	1.28	1.9%
		1.17				1.05				1.05				1.17	
Story 3	$\Phi M_r/M_u$	1.47				1.97				1.97				1.47	1.8%
	$\Phi V_r/V_{mpr}$	2.03				1.98				1.98				2.03	
	$P/A_g f_c$	0.28				0.45				0.45				0.28	
Floor 3		1.34	1.12	1.10	1.16	1.40	1.27	1.43	1.16	1.40	1.12	1.10	1.16	1.34	1.8%
		1.21				1.05				1.05				1.21	
Story 2	$\Phi M_r/M_u$	1.51				2.00				2.00				1.51	1.8%
	$\Phi V_r/V_{mpr}$	1.88				2.02				2.02				1.88	
	$P/A_g f_c$	0.31				0.49				0.49				0.31	
Floor 2		1.39	1.12	1.12	1.15	1.43	1.26	1.77	1.15	1.43	1.12	1.12	1.15	1.39	1.8%
		1.25				1.02				1.02				1.25	
Story 1	$\Phi M_r/M_u$	1.28				1.85				1.85				1.28	1.8%
	$\Phi V_r/V_{mpr}$	1.91				2.07				2.07				1.91	
	$P/A_g f_c$	0.33				0.54				0.54				0.33	

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in)	=	3409							
		E_{eff}/E_g	=	0.35	1939	-3976	7.178E+07	4468	1939	-3976	7.178E+07
Floor 12	Story 11	$M_{y,exp}$ (k-in)	=	3738							
		E_{eff}/E_g	=	0.35	2139	-5428	7.178E+07	5105	2139	-5428	7.178E+07
Floor 11	Story 10	$M_{y,exp}$ (k-in)	=	4426							
		E_{eff}/E_g	=	0.35	2447	-5908	7.178E+07	6323	2447	-5908	7.178E+07
Floor 10	Story 9	$M_{y,exp}$ (k-in)	=	4744							
		E_{eff}/E_g	=	0.35	2695	-6386	7.178E+07	7344	2695	-6386	7.178E+07
Floor 9	Story 8	$M_{y,exp}$ (k-in)	=	5657							
		E_{eff}/E_g	=	0.38	3318	-8239	1.471E+08	7699	3318	-8239	1.471E+08
Floor 8	Story 7	$M_{y,exp}$ (k-in)	=	5845							
		E_{eff}/E_g	=	0.39	3553	-8449	1.471E+08	8223	3553	-8449	1.471E+08
Floor 7	Story 6	$M_{y,exp}$ (k-in)	=	6150							
		E_{eff}/E_g	=	0.41	3553	-8656	1.471E+08	8561	3553	-8656	1.471E+08
Floor 6	Story 5	$M_{y,exp}$ (k-in)	=	6452							
		E_{eff}/E_g	=	0.42	3553	-8725	1.471E+08	9126	3553	-8725	1.471E+08
Floor 5	Story 4	$M_{y,exp}$ (k-in)	=	6514							
		E_{eff}/E_g	=	0.44	3553	-8725	1.471E+08	9329	3553	-8725	1.471E+08
Floor 4	Story 3	$M_{y,exp}$ (k-in)	=	6810							
		E_{eff}/E_g	=	0.46	3553	-8725	1.471E+08	10570	3553	-8725	1.471E+08
Floor 3	Story 2	$M_{y,exp}$ (k-in)	=	7104							
		E_{eff}/E_g	=	0.47	3770	-8521	1.471E+08	10826	3770	-8521	1.471E+08
Floor 2	Story 1	$M_{y,exp}$ (k-in)	=	7395							
		E_{eff}/E_g	=	0.49	4284	-8320	1.471E+08	10687	4284	-8320	1.471E+08
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60											
Model periods (sec): T ₁ = 2.14 T ₂ = 0.72 T ₃ = 0.42											
f _{rebar,exp} = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1015

Number of Stories: 12

Fundamental Period (sec): 2.13

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Beam and column sizes from ground floor of index 1014 were imposed throughout the building. Beam reinforcement was designed for flexure in the critical (third) floor, while column reinforcement was increased from that in index 1014 to meet SCWB requirements in the upper stories. Member size and reinforcement ratios were kept the same throughout all stories.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)															
Floor 13															
Story 12	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 12															
Story 11	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 11															
Story 10	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 10															
Story 9	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 9															
Story 8	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 8															
Story 7	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 7															
Story 6	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 6															
Story 5	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 5															
Story 4	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 4															
Story 3	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 3															
Story 2	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Floor 2															
Story 1	h (in) =	22	28	22	0.0045	0.0075	0.0049	5.0	28	22	0.0045				
	b (in) =	22	28	22	0.0075	0.0049	5.0	28	22	0.0045					
	ρ_{tot} =	0.0170	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070	0.0070					
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0					
Basement															
Grade beam column height (in) = 24															
Basement column height (in) = 22															
20 feet															
13 feet															
15 feet															
Design base shear = 0.044 g, 123 k															
<table><tr><td>f'_c beams = 5.0 ksi</td><td>f'_c cols upper = 5.0 ksi</td></tr><tr><td>f_y rebar nom. = 60 ksi</td><td>f'_c cols lower = 5.0 ksi</td></tr></table>												f'_c beams = 5.0 ksi	f'_c cols upper = 5.0 ksi	f_y rebar nom. = 60 ksi	f'_c cols lower = 5.0 ksi
f'_c beams = 5.0 ksi	f'_c cols upper = 5.0 ksi														
f_y rebar nom. = 60 ksi	f'_c cols lower = 5.0 ksi														

Design Documentation (2 of 2)																						
	SCWB =	0.59		2.83	2.07	1.17		0.58		2.91	2.41	0.61	1.17		0.58	2.83	2.07	1.17		0.59		Design Drifts:
Joint $\Phi V_r/V_u$		1.68						1.40							1.40					1.68		
Story 12	$\Phi M_r/M_u$	2.03						4.72							4.73					2.04		
	$\Phi V_r/V_{mpc}$	1.62						1.07							1.07					1.62		0.5%
	$P/A_g f_c$	0.02						0.05							0.05					0.02		
Floor 12		1.21	1.64	2.26	0.61	1.17		1.19		2.12	2.32	0.61	1.17		1.19	1.64	2.26	0.61	1.17	1.21		
		1.34						1.11							1.11					1.34		
Story 11	$\Phi M_r/M_u$	1.98						3.55							3.55					1.98		
	$\Phi V_r/V_{mpc}$	1.48						1.39							1.39					1.48		0.8%
	$P/A_g f_c$	0.05						0.08							0.08					0.05		
Floor 11		1.27	1.45	2.07	0.61	1.17		1.26		1.82	2.21	0.61	1.17		1.26	1.45	2.07	0.61	1.17	1.27		
		1.34						1.11							1.11					1.34		
Story 10	$\Phi M_r/M_u$	1.79						2.93							2.93					1.79		
	$\Phi V_r/V_{mpc}$	1.90						1.30							1.30					1.90		1.1%
	$P/A_g f_c$	0.08						0.12							0.12					0.08		
Floor 10		1.32	1.29	1.92	0.61	1.17		1.33		1.62	2.04	0.61	1.17		1.33	1.29	1.92	0.61	1.17	1.32		
		1.30						1.08							1.08					1.30		
Story 9	$\Phi M_r/M_u$	1.68						2.61							2.61					1.68		
	$\Phi V_r/V_{mpc}$	2.11						1.46							1.46					2.11		1.3%
	$P/A_g f_c$	0.11						0.17							0.17					0.11		
Floor 9		1.36	1.19	1.78	0.61	1.17		1.39		1.48	1.91	0.61	1.17		1.39	1.19	1.78	0.61	1.17	1.36		
		1.28						1.05							1.05					1.28		
Story 8	$\Phi M_r/M_u$	1.62						2.44							2.44					1.62		
	$\Phi V_r/V_{mpc}$	2.36						1.65							1.65					2.36		1.5%
	$P/A_g f_c$	0.13						0.21							0.21					0.13		
Floor 8		1.40	1.13	1.67	0.61	1.17		1.46		1.38	1.83	0.61	1.17		1.46	1.13	1.67	0.61	1.17	1.40		
		1.28						1.05							1.05					1.28		
Story 7	$\Phi M_r/M_u$	1.59						2.37							2.37					1.59		
	$\Phi V_r/V_{mpc}$	2.24						1.58							1.58					2.24		1.6%
	$P/A_g f_c$	0.16						0.26							0.26					0.16		
Floor 7		1.44	1.09	1.59	0.61	1.17		1.53		1.32	1.68	0.61	1.17		1.53	1.09	1.59	0.61	1.17	1.44		
		1.26						1.05							1.05					1.26		
Story 6	$\Phi M_r/M_u$	1.58						2.35							2.35					1.58		
	$\Phi V_r/V_{mpc}$	2.13						1.50							1.50					2.13		1.7%
	$P/A_g f_c$	0.19						0.30							0.30					0.19		
Floor 6		1.47	1.07	1.45	0.61	1.17		1.60		1.27	1.59	0.61	1.17		1.60	1.07	1.46	0.61	1.17	1.47		
		1.26						1.05							1.05					1.26		
Story 5	$\Phi M_r/M_u$	1.59						2.38							2.38					1.59		
	$\Phi V_r/V_{mpc}$	2.04						1.54							1.54					2.04		1.8%
	$P/A_g f_c$	0.22						0.35							0.35					0.22		
Floor 5		1.50	1.06	1.33	0.61	1.17		1.67		1.24	1.54	0.61	1.17		1.67	1.06	1.33	0.61	1.17	1.50		
		1.26						1.05							1.05					1.26		
Story 4	$\Phi M_r/M_u$	1.61						2.45							2.45					1.61		
	$\Phi V_r/V_{mpc}$	1.96						1.57							1.57					1.96		1.9%
	$P/A_g f_c$	0.25						0.39							0.39					0.25		
Floor 4		1.53	1.07	1.23	0.61	1.17		1.73		1.23	1.53	0.61	1.17		1.73	1.07	1.23	0.61	1.17	1.53		
		1.26						1.05							1.05					1.26		
Story 3	$\Phi M_r/M_u$	1.65						2.56							2.56					1.65		
	$\Phi V_r/V_{mpc}$	1.75						1.61							1.61					1.75		1.9%
	$P/A_g f_c$	0.28						0.44							0.44					0.28		
Floor 3		1.55	1.08	1.17	0.61	1.17		1.80		1.22	1.53	0.61	1.17		1.80	1.08	1.17	0.61	1.17	1.55		
		1.26						1.05							1.05					1.26		
Story 2	$\Phi M_r/M_u$	1.69						2.58							2.58					1.69		
	$\Phi V_r/V_{mpc}$	1.78						1.65							1.65					1.78		1.9%
	$P/A_g f_c$	0.31						0.49							0.49					0.31		
Floor 2		1.57	1.11	1.05	0.61	1.17		1.87		1.24	1.66	0.61	1.17		1.87	1.11	1.05	0.61	1.17	1.57		
		1.26						1.05							1.05					1.26		
Story 1	$\Phi M_r/M_u$	1.43						2.35							2.35					1.44		1.8%
	$\Phi V_r/V_{mpc}$	1.81						1.69							1.69					1.81		
	$P/A_g f_c$	0.34						0.54							0.54					0.34		

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) = 4629 E_{eff}/E_g = 0.35 M_d/M_y = 1.21 $\Theta_{cap,pl}$ (rad) = 0.071 Θ_{pc} (rad) = 0.100 λ = 93	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	7000 0.35 1.21 0.074 0.100 91 0.03	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	7000 0.35 1.21 0.074 0.100 91 0.03	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	4629 0.35 1.21 0.07 0.100 93 0.02			
Floor 12	Story 11	$M_{y,exp}$ (k-in) = 4952 E_{eff}/E_g = 0.35 M_d/M_y = 1.21 $\Theta_{cap,pl}$ (rad) = 0.069 Θ_{pc} (rad) = 0.100 λ = 91	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	7621 0.35 1.20 0.070 0.100 87 0.06	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	7621 0.35 1.20 0.070 0.100 87 0.06	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	4952 0.35 1.21 0.069 0.100 91 0.02			
Floor 11	Story 10	$M_{y,exp}$ (k-in) = 5270 E_{eff}/E_g = 0.35 M_d/M_y = 1.20 $\Theta_{cap,pl}$ (rad) = 0.067 Θ_{pc} (rad) = 0.100 λ = 89	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	8230 0.35 1.20 0.066 0.100 83 0.10	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	8230 0.35 1.20 0.066 0.100 83 0.10	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	5270 0.35 1.20 0.067 0.100 89 0.03			
Floor 10	Story 9	$M_{y,exp}$ (k-in) = 5585 E_{eff}/E_g = 0.35 M_d/M_y = 1.20 $\Theta_{cap,pl}$ (rad) = 0.065 Θ_{pc} (rad) = 0.100 λ = 87	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	8829 0.37 1.19 0.062 0.100 80 0.13	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	8829 0.37 1.19 0.062 0.100 80 0.13	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	5585 0.35 1.20 0.065 0.100 87 0.03			
Floor 9	Story 8	$M_{y,exp}$ (k-in) = 5896 E_{eff}/E_g = 0.38 M_d/M_y = 1.20 $\Theta_{cap,pl}$ (rad) = 0.063 Θ_{pc} (rad) = 0.100 λ = 85	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	9419 0.46 1.19 0.058 0.100 77 0.16	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	9419 0.46 1.19 0.058 0.100 77 0.16	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	5896 0.38 1.20 0.063 0.100 85 0.03			
Floor 8	Story 7	$M_{y,exp}$ (k-in) = 6203 E_{eff}/E_g = 0.39 M_d/M_y = 1.20 $\Theta_{cap,pl}$ (rad) = 0.061 Θ_{pc} (rad) = 0.100 λ = 83	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	10001 0.49 1.18 0.055 0.100 74 0.19	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	10001 0.49 1.18 0.055 0.100 74 0.19	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	6203 0.39 1.20 0.061 0.100 83 0.03			
Floor 7	Story 6	$M_{y,exp}$ (k-in) = 6508 E_{eff}/E_g = 0.41 M_d/M_y = 1.19 $\Theta_{cap,pl}$ (rad) = 0.059 Θ_{pc} (rad) = 0.100 λ = 82	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	10574 0.52 1.18 0.052 0.100 71 0.22	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	10574 0.52 1.18 0.052 0.100 71 0.22	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	6508 0.41 1.19 0.059 0.100 82 0.11			
Floor 6	Story 5	$M_{y,exp}$ (k-in) = 6809 E_{eff}/E_g = 0.42 M_d/M_y = 1.19 $\Theta_{cap,pl}$ (rad) = 0.058 Θ_{pc} (rad) = 0.100 λ = 80	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	11141 0.55 1.17 0.049 0.091 68 0.26	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	11141 0.55 1.17 0.049 0.091 68 0.26	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	6809 0.42 1.19 0.058 0.100 80 0.13			
Floor 5	Story 4	$M_{y,exp}$ (k-in) = 7107 E_{eff}/E_g = 0.44 M_d/M_y = 1.19 $\Theta_{cap,pl}$ (rad) = 0.056 Θ_{pc} (rad) = 0.100 λ = 78	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	11701 0.58 1.17 0.046 0.081 65 0.29	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	11701 0.58 1.17 0.046 0.081 65 0.29	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	7107 0.44 1.19 0.056 0.100 78 0.14			
Floor 4	Story 3	$M_{y,exp}$ (k-in) = 7403 E_{eff}/E_g = 0.46 M_d/M_y = 1.19 $\Theta_{cap,pl}$ (rad) = 0.054 Θ_{pc} (rad) = 0.100 λ = 77	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	12787 0.61 1.17 0.043 0.073 62 0.32	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	12787 0.61 1.17 0.043 0.073 62 0.32	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	7403 0.46 1.19 0.054 0.100 77 0.16			
Floor 3	Story 2	$M_{y,exp}$ (k-in) = 7696 E_{eff}/E_g = 0.47 M_d/M_y = 1.19 $\Theta_{cap,pl}$ (rad) = 0.053 Θ_{pc} (rad) = 0.100 λ = 75	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	12742 0.64 1.16 0.041 0.065 60 0.35	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	12742 0.64 1.16 0.041 0.065 60 0.35	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109	7696 0.47 1.19 0.053 0.100 75 0.18			
Floor 2	Story 1	$M_{y,exp}$ (k-in) = 7986 E_{eff}/E_g = 0.49 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.051 Θ_{pc} (rad) = 0.100 λ = 74 $M_{y,pos,exc}$ (k-in) = 4629 $M_{y,neg,exc}$ (k-in) = -8246 E_{eff}/E_g = 0.35 E_{eff}/E_g = 0.35 E_{eff}/E_g = 1.471E+08 $(\Theta_{cap,pl})_{pos}$ (rad) = 0.048 $(\Theta_{cap,pl})_{neg}$ (rad) = -0.075 Θ_{pc} (rad) = 0.100 λ = 109	3995 -8246 0.35 1.471E+08 0.0483 -0.075 0.100 109 3995 -8246 0.35 1.471E+08 0.0483 -0.0								

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1017

Number of Stories: 12

Fundamental Period (sec): 1.92

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Column sizes were increased from 22 inches square in index 1014 to 24 inches square in this design in order to meet SCWB and joint shear requirements in the upper floors while maintaining 65% weak story (actually the two lower stories) and not exceeding maximum reinforcement ratios. Column concrete strength was increased to 7.0 ksi in lower stories and 6.0 ksi in upper stories to meet joint shear requirements. Beam reinforcement was determined by demand in the bottom floor and then stepped up to meet 65% weak story requirement. Column strengths were determined by SCWB considerations.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)											
Floor 13											
Story 12	h (in) =	24	28	24	0.0033	28	24	0.0033	24	28	0.0033
	b (in) =	24	28	24	0.0045	28	24	0.0045	24	28	0.0045
	ρ_{ext} =	0.0205	0.0077	0.0038	0.0075	0.0086	0.0075	0.0086	0.0075	0.0086	0.0075
	s (in) =	4.0	5.5	5.5	6.0	5.5	5.5	6.0	5.5	5.5	6.0
Floor 12											
Story 11	h (in) =	24	28	24	0.0043	28	24	0.0043	24	28	0.0043
	b (in) =	24	28	24	0.0085	28	24	0.0085	24	28	0.0085
	ρ_{ext} =	0.0205	0.0077	0.0047	0.0047	0.0077	0.0047	0.0077	0.0047	0.0077	0.0047
	s (in) =	4.0	5.0	5.0	5.0	4.0	5.0	4.0	5.0	4.0	5.0
Floor 11											
Story 10	h (in) =	24	28	24	0.0048	28	24	0.0048	24	28	0.0048
	b (in) =	24	28	24	0.0095	28	24	0.0095	24	28	0.0095
	ρ_{ext} =	0.0205	0.0077	0.0049	0.0049	0.0077	0.0049	0.0077	0.0049	0.0077	0.0049
	s (in) =	4.0	6.0	6.0	6.0	4.0	6.0	4.0	6.0	4.0	6.0
Floor 10											
Story 9	h (in) =	24	28	24	0.0050	28	24	0.0050	24	28	0.0050
	b (in) =	24	28	24	0.0100	28	24	0.0100	24	28	0.0100
	ρ_{ext} =	0.0205	0.0077	0.0051	0.0051	0.0077	0.0051	0.0077	0.0051	0.0077	0.0051
	s (in) =	4.0	6.0	6.0	6.0	4.0	6.0	4.0	6.0	4.0	6.0
Floor 9											
Story 8	h (in) =	24	28	24	0.0053	28	24	0.0053	24	28	0.0053
	b (in) =	24	28	24	0.0105	28	24	0.0105	24	28	0.0105
	ρ_{ext} =	0.0205	0.0077	0.0052	0.0052	0.0077	0.0052	0.0077	0.0052	0.0077	0.0052
	s (in) =	4.0	6.0	6.0	6.0	4.0	6.0	4.0	6.0	4.0	6.0
Floor 8											
Story 7	h (in) =	24	28	24	0.0054	28	24	0.0054	24	28	0.0054
	b (in) =	24	28	24	0.0108	28	24	0.0108	24	28	0.0108
	ρ_{ext} =	0.0205	0.0077	0.0053	0.0053	0.0077	0.0053	0.0077	0.0053	0.0077	0.0053
	s (in) =	4.0	6.0	6.0	6.0	4.0	6.0	4.0	6.0	4.0	6.0
Floor 7											
Story 6	h (in) =	24	28	24	0.0055	28	24	0.0055	24	28	0.0055
	b (in) =	24	28	24	0.0110	28	24	0.0110	24	28	0.0110
	ρ_{ext} =	0.0205	0.0089	0.0054	0.0054	0.0089	0.0054	0.0089	0.0054	0.0089	0.0054
	s (in) =	3.5	6.0	6.0	6.0	3.5	6.0	3.5	6.0	3.5	6.0
Floor 6											
Story 5	h (in) =	24	28	24	0.0066	28	24	0.0066	24	28	0.0066
	b (in) =	24	28	24	0.0113	28	24	0.0113	24	28	0.0113
	ρ_{ext} =	0.0205	0.0089	0.0056	0.0056	0.0089	0.0056	0.0089	0.0056	0.0089	0.0056
	s (in) =	3.5	5.5	5.5	5.5	3.5	5.5	3.5	5.5	3.5	5.5
Floor 5											
Story 4	h (in) =	24	28	24	0.0055	28	24	0.0055	24	28	0.0055
	b (in) =	24	28	24	0.0113	28	24	0.0113	24	28	0.0113
	ρ_{ext} =	0.0205	0.0089	0.0056	0.0056	0.0089	0.0056	0.0089	0.0056	0.0089	0.0056
	s (in) =	3.5	5.5	5.5	5.5	3.5	5.5	3.5	5.5	3.5	5.5
Floor 4											
Story 3	h (in) =	24	28	24	0.0040	28	24	0.0040	24	28	0.0040
	b (in) =	24	28	24	0.0070	28	24	0.0070	24	28	0.0070
	ρ_{ext} =	0.0205	0.0089	0.0045	0.0045	0.0089	0.0045	0.0089	0.0045	0.0089	0.0045
	s (in) =	3.5	5.0	5.0	5.0	3.5	5.0	3.5	5.0	3.5	5.0
Floor 3											
Story 2	h (in) =	24	28	24	0.0040	28	24	0.0040	24	28	0.0040
	b (in) =	24	28	24	0.0070	28	24	0.0070	24	28	0.0070
	ρ_{ext} =	0.0120	0.0089	0.0045	0.0045	0.0089	0.0045	0.0089	0.0045	0.0089	0.0045
	s (in) =	3.5	5.0	5.0	5.0	3.5	5.0	3.5	5.0	3.5	5.0
Floor 2											
Story 1	h (in) =	24	28	24	0.0040	28	24	0.0040	24	28	0.0040
	b (in) =	24	28	24	0.0070	28	24	0.0070	24	28	0.0070
	ρ_{ext} =	0.0120	0.0089	0.0045	0.0045	0.0089	0.0045	0.0089	0.0045	0.0089	0.0045
	s (in) =	3.5	5.0	5.0	5.0	3.5	5.0	3.5	5.0	3.5	5.0
Grade beam column height (in) = 24											
Basement column height (in) = 24											
Design base shear = 0.044 g, 123 k											
f_c beams = 5.0 ksi f_c cols upper = 6.0 ksi f_y rebar nom = 60 ksi f_c cols lower = 7.0 ksi											

Design Documentation (2 of 2)																																																																												
SCWB =			0.89					1.82					1.72					0.73					16.63					0.84			1.96					2.00					0.73					16.63					0.84			1.82					1.72					0.73					16.63					0.89		
Story 12	Joint $\Phi V_r/V_u$ =		3.35																	2.59																	2.59																	2.59																	3.35					
	$\Phi M_r/M_u$ =		3.20																	6.63																	6.64																	3.20																						
	$\Phi V_r/V_{mpr}$ =		1.38																	1.16																	1.16																	1.38																						
$P/A_g f_c$ =			0.02																	0.03																	0.03																	0.02																						
Floor 12			1.35		1.81					2.09					0.51					1.13					1.34			2.35					2.23					0.51					1.13					1.34			1.81					2.09					0.51					1.13					1.35					
			1.60																	1.42																	1.42																	1.60																						
Story 11	$\Phi M_r/M_u$ =		3.09																	4.95																	4.95																	3.10																						
	$\Phi V_r/V_{mpr}$ =		1.29																	1.34																	1.34																	1.29																						
	$P/A_g f_c$ =		0.03																	0.06																	0.06																	0.03																						
Floor 11			1.29		1.81					2.17					0.51					1.12					1.28			2.27					2.32					0.51					1.12					1.28			1.81					2.17					0.51					1.12					1.29					
			1.41																	1.25																	1.25																	1.41																						
Story 10	$\Phi M_r/M_u$ =		2.76																	4.01																	4.01																	2.76																						
	$\Phi V_r/V_{mpr}$ =		1.69																	1.28																	1.28																	1.69																						
	$P/A_g f_c$ =		0.05																	0.09																	0.09																	0.05																						
Floor 10			1.23		1.80					2.21					0.51					1.11					1.23			2.24					2.39					0.51					1.11					1.23			1.80					2.21					0.51					1.11					1.23					
			1.22																	1.09																	1.09																	1.22																						
Story 9	$\Phi M_r/M_u$ =		2.57																	3.53																	3.53																	2.57																						
	$\Phi V_r/V_{mpr}$ =		1.91																	1.44																	1.44																	1.91																						
	$P/A_g f_c$ =		0.07																	0.12																	0.12																	0.07																						
Floor 9			1.22		1.75					2.14					0.51					1.11					1.23			2.15					2.38					0.51					1.11					1.23			1.75					2.14					0.51					1.11					1.22					
			1.13																	1.01																	1.01																	1.13																						
Story 8	$\Phi M_r/M_u$ =		2.45																	3.27																	3.27																	2.46																						
	$\Phi V_r/V_{mpr}$ =		2.16																	1.46																	1.64																	2.16																						
	$P/A_g f_c$ =		0.09																	0.15																	0.15																	0.09																						
Floor 8			1.20		1.74					2.12					0.51					1.11					1.23			2.11					2.35					0.51					1.11					1.23			1.74					2.13					0.51					1.11					1.20					
			1.08																	0.96																	0.96																	1.08																						
Story 7	$\Phi M_r/M_u$ =		2.39																	3.14																	3.14																	2.39																						
	$\Phi V_r/V_{mpr}$ =		2.07																	1.58																	1.58																	2.07																						
	$P/A_g f_c$ =		0.11																	0.18																	0.18																	0.11																						
Floor 7			1.21		1.72					2.06					0.51					1.12					1.26			2.06					2.19					0.51					1.12					1.26			1.72					2.06					0.51					1.12					1.21					
			1.14																	1.01																	1.01																	1.14																						
Story 6	$\Phi M_r/M_u$ =		2.36																	3.11																	3.11																	2.37																						
	$\Phi V_r/V_{mpr}$ =		2.26																	1.72																	1.72																	2.26																						
	$P/A_g f_c$ =		0.12																	0.18																	0.18																	0.12																						
Floor 6			1.21		1.72					1.99					0.51					1.12					1.28			2.03					2.11					0.51					1.12					1.28			1.72					1.99					0.51					1.12					1.21					
			1.11																	0.99																	0.99																	1.11																						
Story 5	$\Phi M_r/M_u$ =		2.36																	3.12																	3.12																	2.37																						
	$\Phi V_r/V_{mpr}$ =		2.18																	1.67																	1.67																	2.18																						
	$P/A_g f_c$ =		0.13																	0.21																	0.21																	0.13																						
Floor 5			1.21		1.75					1.90					0.51					1.14					1.31			2.03					2.09					0.51					1.14					1.31			1.75					1.90					0.51					1.14					1.21					
			1.09																	0.97																	0.97																	1.09																						
Story 4	$\Phi M_r/M_u$ =		2.39																	3.19																	3.19																	2.39																						
	$\Phi V_r/V_{mpr}$ =		2.11																	1.61																	1.61																	2.11																						
	$P/A_g f_c$ =		0.15																	0.24																	0.24																	0.15																						
Floor 4			1.23		1.76					1.73					0.50					1.15					1.36			2.01					2.03					0.50					1.15					1.36			1.76					1.73					0.50					1.15					1.23					
			1.09																	0.97																	0.97																	1.09																						
Story 3	$\Phi M_r/M_u$ =		2.43																	3.31																	3.31																	2.43																						
	$\Phi V_r/V_{mpr}$ =		2.05																	1.57																	1.57																	2.05																						
	$P/A_g f_c$ =		0.17																	0.26																	0.26																	0.17																						
Floor 3			1.44		1.14					1.21					0.58					1.15					1.59			1.27					1.50					0.58					1.15					1.59			1.14					1.21					0.58					1.15					1.44					
			1.75																	1.48																	1.48																	1.75																						
Story 2	$\Phi M_r/M_u$ =		1.67																	2.23																	2.23																	1.67																						
	$\Phi V_r/V_{mpr}$ =		2.53																	1.81																	1.81																	2.53																						
	$P/A_g f_c$ =		0.19																	0.29																	0.29																	0.19																						
Floor 2			1.19		1.22					1.18					0.58					1.15					1.32			1.33					1.73					0.58					1.15					1.32			1.22					1.18					0.58					1.15					1.19					
			1.75																	1.48																	1.48																	1.75																						
Story 1	$\Phi M_r/M_u$ =		1.31																	1.99																	1.99																	1.31																						
	$\Phi V_r/V_{mpr}$ =		2.44																	1.85																	1.85																	2.44																						
	$P/A_g f_c$ =		0.20																	0.32																	0.32																	0.20																						

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	7234	3175	3175	3175	3175	3175	3175	3175	7234
		$E_{l,eff}/E_{l,g}$ =	0.35	-8021	-8021	-8021	-8021	-8021	-8021	-8021	-8021
Floor 12	Story 11	$M_{y,exp}$ (k-in) =	7587	3648	3648	3648	3648	3648	3648	3648	7587
		$E_{l,eff}/E_{l,g}$ =	0.35	-10732	-10732	-10732	-10732	-10732	-10732	-10732	-10732
Floor 11	Story 10	$M_{y,exp}$ (k-in) =	7936	4121	4121	4121	4121	4121	4121	4121	7936
		$E_{l,eff}/E_{l,g}$ =	0.35	-11633	-11633	-11633	-11633	-11633	-11633	-11633	-11633
Floor 10	Story 9	$M_{y,exp}$ (k-in) =	8282	4594	4594	4594	4594	4594	4594	4594	8282
		$E_{l,eff}/E_{l,g}$ =	0.35	-12532	-12532	-12532	-12532	-12532	-12532	-12532	-12532
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	8625	5065	5065	5065	5065	5065	5065	5065	8625
		$E_{l,eff}/E_{l,g}$ =	0.35	-13429	-13429	-13429	-13429	-13429	-13429	-13429	-13429
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	8965	5531	5531	5531	5531	5531	5531	5531	8965
		$E_{l,eff}/E_{l,g}$ =	0.35	-14329	-14329	-14329	-14329	-14329	-14329	-14329	-14329
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	9334	6005	6005	6005	6005	6005	6005	6005	9334
		$E_{l,eff}/E_{l,g}$ =	0.35	-15329	-15329	-15329	-15329	-15329	-15329	-15329	-15329
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	9671	6478	6478	6478	6478	6478	6478	6478	9671
		$E_{l,eff}/E_{l,g}$ =	0.35	-16329	-16329	-16329	-16329	-16329	-16329	-16329	-16329
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	10005	6951	6951	6951	6951	6951	6951	6951	10005
		$E_{l,eff}/E_{l,g}$ =	0.35	-17329	-17329	-17329	-17329	-17329	-17329	-17329	-17329
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	10337	7424	7424	7424	7424	7424	7424	7424	10337
		$E_{l,eff}/E_{l,g}$ =	0.35	-18329	-18329	-18329	-18329	-18329	-18329	-18329	-18329
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	10670	7897	7897	7897	7897	7897	7897	7897	10670
		$E_{l,eff}/E_{l,g}$ =	0.35	-19329	-19329	-19329	-19329	-19329	-19329	-19329	-19329
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	11003	8370	8370	8370	8370	8370	8370	8370	11003
		$E_{l,eff}/E_{l,g}$ =	0.35	-20329	-20329	-20329	-20329	-20329	-20329	-20329	-20329
Mass tributary to one frame for lateral load (each floor) (k-s/s/in): 0.60											
Model periods (sec): T ₁ = 1.92 T ₂ = 0.63 T ₃ = 0.37											
f _{y,steel,exp,des} = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1018

Number of Stories: 12

Fundamental Period (sec): 2.09

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Column sizes were returned to 22 inches square to meet drift requirements while still enabling SCWB and joint shear requirements to be met in the upper floors. Column concrete strength was increased to 7.0 ksi in the lower stories and 6.0 ksi in the upper stories to meet joint shear requirements. Beam reinforcement was determined by demand in the bottom floor and then stepped up to meet 80% weak story requirement. In some cases, positive bending strength was determined by the minimum positive-to-negative strength ratio. Column strengths were determined by SCWB considerations.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)											
Floor 13											
Story 12	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0225	0.0225	0.0225	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	6.0	6.0	6.0	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 12											
Story 11	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0225	0.0225	0.0225	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	6.0	6.0	6.0	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 11											
Story 10	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0250	0.0250	0.0250	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	6.0	6.0	6.0	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 10											
Story 9	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0250	0.0250	0.0250	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	6.0	6.0	6.0	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 9											
Story 8	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0260	0.0260	0.0260	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	5.0	5.0	5.0	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 8											
Story 7	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0260	0.0260	0.0260	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	5.0	5.0	5.0	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 7											
Story 6	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0260	0.0260	0.0260	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	5.0	5.0	5.0	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 6											
Story 5	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0260	0.0260	0.0260	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	5.0	5.0	5.0	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 5											
Story 4	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0260	0.0260	0.0260	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	5.0	5.0	5.0	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 4											
Story 3	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0260	0.0260	0.0260	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0087	0.0087	0.0087	5.5	5.5	5.5	28	22	0.0087	0.0087
	s (in) =	4.0	4.0	4.0				28	22	4.0	4.0
Floor 3											
Story 2	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0168	0.0168	0.0168	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0070	0.0070	0.0070	5.5	5.5	5.5	28	22	0.0070	0.0070
	s (in) =	5.0	5.0	5.0				28	22	5.0	5.0
Floor 2											
Story 1	h (in) =	22	28	22	0.0032	0.0032	0.0032	28	22	0.0032	0.0032
	b (in) =	22	22	22	0.0035	0.0035	0.0035	28	22	0.0035	0.0035
	ρ_{tot} =	0.0168	0.0168	0.0168	0.0028	0.0028	0.0028	28	22	0.0028	0.0028
	ρ_{sh} =	0.0070	0.0070	0.0070	5.5	5.5	5.5	28	22	0.0070	0.0070
	s (in) =	5.0	5.0	5.0				28	22	5.0	5.0
Basement											
Grade beam column height (in) = 24											
Basement column height (in) = 22											
20 feet											
Design base shear = 0.044 g, 123 k											
f'_c beams = 5.0 ksi f'_c cols, upper = 6.0 ksi f_y rebar, nom = 60 ksi f'_c cols, lower = 7.0 ksi											
<div>13 feet</div> <div>15 feet</div>											

Design Documentation (2 of 2)																		
	SCWB =	0.86	1.35	1.49	0.92	18.65	0.71	1.39	1.74	0.92	18.65	0.71	1.35	1.49	0.92	18.65	0.86	Design Drifts:
Joint $\Phi V_r/V_u$ =		3.94					2.74					2.74					3.94	
Story 12	$\Phi M_r/M_u$ =	2.64					4.82					4.82					2.64	0.5%
	$\Phi V_r/V_{mpr}$ =	1.58					1.31					1.31					1.58	
	$P/A_g f_c$ =	0.02					0.04					0.04					0.02	
Floor 12		1.35	1.32	1.63	0.55	4.77	1.21	1.71	1.67	0.55	4.77	1.21	1.32	1.63	0.55	4.77	1.35	0.8%
		1.83					1.59					1.59					1.83	
Story 11	$\Phi M_r/M_u$ =	2.54					3.62					3.62					2.54	
	$\Phi V_r/V_{mpr}$ =	1.47					1.64					1.64					1.47	
	$P/A_g f_c$ =	0.04					0.07					0.07					0.04	
Floor 11		1.37	1.31	1.56	0.51	3.77	1.27	1.64	1.67	0.51	3.77	1.27	1.31	1.56	0.51	3.77	1.37	1.1%
		1.63					1.45					1.45					1.63	
Story 10	$\Phi M_r/M_u$ =	2.49					3.35					3.35					2.49	
	$\Phi V_r/V_{mpr}$ =	1.71					1.39					1.39					1.71	
	$P/A_g f_c$ =	0.06					0.10					0.10					0.06	
Floor 10		1.36	1.33	1.66	0.51	0.68	1.28	1.67	1.76	0.51	0.68	1.28	1.33	1.66	0.51	0.68	1.36	1.3%
		1.37					1.22					1.22					1.37	
Story 9	$\Phi M_r/M_u$ =	2.32					2.97					2.97					2.32	
	$\Phi V_r/V_{mpr}$ =	1.94					1.57					1.57					1.94	
	$P/A_g f_c$ =	0.09					0.14					0.14					0.09	
Floor 9		1.33	1.35	1.68	0.51	1.13	1.26	1.67	1.81	0.51	1.13	1.26	1.35	1.68	0.51	1.13	1.33	1.5%
		1.22					1.09					1.09					1.22	
Story 8	$\Phi M_r/M_u$ =	2.29					2.77					2.77					2.29	
	$\Phi V_r/V_{mpr}$ =	2.14					1.77					1.77					2.14	
	$P/A_g f_c$ =	0.11					0.18					0.18					0.11	
Floor 8		1.36	1.31	1.63	0.51	1.12	1.28	1.61	1.78	0.51	1.12	1.28	1.31	1.63	0.51	1.12	1.36	1.6%
		1.19					1.05					1.05					1.19	
Story 7	$\Phi M_r/M_u$ =	2.22					2.67					2.67					2.22	
	$\Phi V_r/V_{mpr}$ =	2.06					1.7					1.7					2.06	
	$P/A_g f_c$ =	0.14					0.21					0.21					0.14	
Floor 7		1.33	1.34	1.63	0.51	1.14	1.28	1.61	1.72	0.51	1.14	1.28	1.34	1.63	0.51	1.14	1.33	1.7%
		1.21					1.08					1.08					1.21	
Story 6	$\Phi M_r/M_u$ =	2.20					2.64					2.64					2.20	
	$\Phi V_r/V_{mpr}$ =	2.03					1.68					1.68					2.03	
	$P/A_g f_c$ =	0.14					0.22					0.22					0.14	
Floor 6		1.36	1.31	1.49	0.51	1.14	1.34	1.56	1.63	0.51	1.14	1.34	1.31	1.49	0.51	1.14	1.36	1.8%
		1.21					1.08					1.08					1.21	
Story 5	$\Phi M_r/M_u$ =	2.20					2.67					2.67					2.20	
	$\Phi V_r/V_{mpr}$ =	1.96					1.63					1.63					1.96	
	$P/A_g f_c$ =	0.16					0.25					0.25					0.16	
Floor 5		1.35	1.33	1.40	0.51	1.14	1.36	1.56	1.62	0.51	1.14	1.36	1.33	1.40	0.51	1.14	1.35	1.9%
		1.18					1.05					1.05					1.18	
Story 4	$\Phi M_r/M_u$ =	2.22					2.73					2.73					2.22	
	$\Phi V_r/V_{mpr}$ =	1.91					1.38					1.38					1.91	
	$P/A_g f_c$ =	0.18					0.28					0.28					0.18	
Floor 4		1.37	1.34	1.37	0.54	1.15	1.39	1.54	1.69	0.54	1.15	1.39	1.34	1.37	0.54	1.15	1.37	1.9%
		1.18					1.03					1.03					1.18	
Story 3	$\Phi M_r/M_u$ =	2.26					2.84					2.84					2.27	
	$\Phi V_r/V_{mpr}$ =	1.86					1.41					1.41					1.86	
	$P/A_g f_c$ =	0.20					0.32					0.32					0.20	
Floor 3		1.43	1.04	1.14	0.62	1.16	1.43	1.17	1.49	0.62	1.16	1.43	1.04	1.14	0.62	1.16	1.43	1.9%
		1.56					1.29					1.29					1.56	
Story 2	$\Phi M_r/M_u$ =	1.69					2.03					2.03					1.69	
	$\Phi V_r/V_{mpr}$ =	1.94					1.69					1.69					1.94	
	$P/A_g f_c$ =	0.22					0.35					0.35					0.22	
Floor 2		1.22	1.07	1.03	0.62	1.16	1.22	1.20	1.62	0.62	1.16	1.22	1.07	1.03	0.62	1.16	1.22	1.8%
		1.56					1.29					1.29					1.56	
Story 1	$\Phi M_r/M_u$ =	1.43					1.98					1.98					1.43	
	$\Phi V_r/V_{mpr}$ =	1.89					1.74					1.74					1.89	
	$P/A_g f_c$ =	0.24					0.38					0.38					0.24	

Modeling Documentation (1 of 1)									
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	5981	7143	7143	5981			
		E_{eff}/E_g =	0.35	0.35	0.35	0.35			
		M_d/M_y =	1.20	1.20	1.20	1.20			
		$\Theta_{cap,pl}$ (rad) =	0.084	0.084	0.084	0.084			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
		λ =	105	103	103	105			
Floor 12	Story 11	$(P/A_g)_{f=0}^{exp}$ =	0.01	0.03	0.03	0.01			
		$M_{y,exp}$ (k-in) =	3300	7768	7768	3300			
		E_{eff}/E_g =	0.35	0.35	0.35	0.35			
		M_d/M_y =	1.20	1.19	1.19	1.20			
		$\Theta_{cap,pl}$ (rad) =	0.082	0.080	0.080	0.082			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 11	Story 10	$(P/A_g)_{f=0}^{exp}$ =	0.03	0.05	0.05	0.03			
		$M_{y,exp}$ (k-in) =	3017	9335	9335	3017			
		E_{eff}/E_g =	0.35	0.35	0.35	0.35			
		M_d/M_y =	1.20	1.19	1.19	1.20			
		$\Theta_{cap,pl}$ (rad) =	0.081	0.079	0.079	0.081			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 10	Story 9	$(P/A_g)_{f=0}^{exp}$ =	0.04	0.08	0.08	0.04			
		$M_{y,exp}$ (k-in) =	3452	9936	9936	3452			
		E_{eff}/E_g =	0.35	0.35	0.35	0.35			
		M_d/M_y =	1.20	1.19	1.19	1.20			
		$\Theta_{cap,pl}$ (rad) =	0.081	0.079	0.079	0.081			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 9	Story 8	$(P/A_g)_{f=0}^{exp}$ =	0.05	0.11	0.11	0.05			
		$M_{y,exp}$ (k-in) =	3778	10529	10529	3778			
		E_{eff}/E_g =	0.36	0.43	0.43	0.36			
		M_d/M_y =	1.19	1.18	1.18	1.19			
		$\Theta_{cap,pl}$ (rad) =	0.078	0.071	0.071	0.078			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 8	Story 7	$(P/A_g)_{f=0}^{exp}$ =	0.07	0.13	0.13	0.07			
		$M_{y,exp}$ (k-in) =	3886	11114	11114	3886			
		E_{eff}/E_g =	0.38	0.46	0.46	0.38			
		M_d/M_y =	1.19	1.18	1.18	1.19			
		$\Theta_{cap,pl}$ (rad) =	0.076	0.068	0.068	0.076			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 7	Story 6	$(P/A_g)_{f=0}^{exp}$ =	0.08	0.16	0.16	0.08			
		$M_{y,exp}$ (k-in) =	4103	11735	11735	4103			
		E_{eff}/E_g =	0.38	0.46	0.46	0.38			
		M_d/M_y =	1.18	1.17	1.17	1.18			
		$\Theta_{cap,pl}$ (rad) =	0.073	0.065	0.065	0.073			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 6	Story 5	$(P/A_g)_{f=0}^{exp}$ =	0.08	0.16	0.16	0.08			
		$M_{y,exp}$ (k-in) =	4103	12310	12310	4103			
		E_{eff}/E_g =	0.39	0.48	0.48	0.39			
		M_d/M_y =	1.18	1.17	1.17	1.18			
		$\Theta_{cap,pl}$ (rad) =	0.071	0.062	0.062	0.071			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 5	Story 4	$(P/A_g)_{f=0}^{exp}$ =	0.09	0.18	0.18	0.09			
		$M_{y,exp}$ (k-in) =	4211	12878	12878	4211			
		E_{eff}/E_g =	0.40	0.50	0.50	0.40			
		M_d/M_y =	1.18	1.17	1.17	1.18			
		$\Theta_{cap,pl}$ (rad) =	0.070	0.060	0.060	0.070			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 4	Story 3	$(P/A_g)_{f=0}^{exp}$ =	0.10	0.21	0.21	0.10			
		$M_{y,exp}$ (k-in) =	4427	13441	13441	4427			
		E_{eff}/E_g =	0.41	0.52	0.52	0.41			
		M_d/M_y =	1.18	1.16	1.16	1.18			
		$\Theta_{cap,pl}$ (rad) =	0.069	0.057	0.057	0.069			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 3	Story 2	$(P/A_g)_{f=0}^{exp}$ =	0.11	0.23	0.23	0.11			
		$M_{y,exp}$ (k-in) =	3908	10323	10323	3908			
		E_{eff}/E_g =	0.42	0.55	0.55	0.42			
		M_d/M_y =	1.18	1.16	1.16	1.18			
		$\Theta_{cap,pl}$ (rad) =	0.053	0.041	0.041	0.053			
		Θ_{pc} (rad) =	0.100	0.093	0.093	0.100			
Floor 2	Story 1	$(P/A_g)_{f=0}^{exp}$ =	0.13	0.25	0.25	0.13			
		$M_{y,exp}$ (k-in) =	3011	10870	10870	3011			
		E_{eff}/E_g =	0.43	0.57	0.57	0.43			
		M_d/M_y =	1.18	1.16	1.16	1.18			
		$\Theta_{cap,pl}$ (rad) =	0.052	0.040	0.040	0.052			
		Θ_{pc} (rad) =	0.100	0.085	0.085	0.100			
Floor 1	Story 0	$(P/A_g)_{f=0}^{exp}$ =	0.14	0.27	0.27	0.14			
		$M_{y,exp}$ (k-in) =	3908	10870	10870	3908			
		E_{eff}/E_g =	0.35	0.35	0.35	0.35			
		M_d/M_y =	1.471E+08	1.471E+08	1.471E+08	1.471E+08			
		$(\Theta_{cap,pl})_{pos}$ (rad) =	0.046	0.046	0.046	0.046			
		$(\Theta_{cap,pl})_{neg}$ (rad) =	-0.071	-0.071	-0.071	-0.071			
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60									
Model periods (sec): T ₁ = 2.09 T ₂ = 0.69 T ₃ = 0.40									
f _{fundamental} = 67 ksi									

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1019

Number of Stories: 12

Fundamental Period (sec): 2.00

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined by both drift limits and minimum joint shear requirements. Beam strengths were controlled by force demands, particularly lateral forces. Column strengths were determined by strong-column weak-beam (SCWB) ratios. Column concrete strength was increased to 6.0 ksi in order to meet joint shear requirements. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

Floor 13																
Story 12	h (in) =	30	28	30	0.0050	0.0075	0.0021	6.0	28	30	0.0050	0.0075	0.0021	6.0		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0150	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0052	0.0103	0.0025	5.0	0.0052	0.0103	0.0025	5.0	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 12																
Story 11	h (in) =	30	28	30	0.0055	0.0113	0.0026	5.0	28	30	0.0055	0.0113	0.0026	5.0		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0150	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0055	0.0113	0.0026	5.0	0.0055	0.0113	0.0026	5.0	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 11																
Story 10	h (in) =	30	28	30	0.0060	0.0120	0.0028	6.0	28	30	0.0060	0.0120	0.0028	6.0		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0150	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0060	0.0120	0.0028	6.0	0.0060	0.0120	0.0028	6.0	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 10																
Story 9	h (in) =	30	34	30	0.0045	0.0093	0.0019	6.5	34	30	0.0045	0.0093	0.0019	6.5		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0150	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0045	0.0093	0.0019	6.5	0.0045	0.0093	0.0019	6.5	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 9																
Story 8	h (in) =	30	34	30	0.0048	0.0095	0.0020	6.5	34	30	0.0048	0.0095	0.0020	6.5		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0158	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0048	0.0095	0.0020	6.5	0.0048	0.0095	0.0020	6.5	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 8																
Story 7	h (in) =	30	34	30	0.0048	0.0098	0.0021	6.0	34	30	0.0048	0.0098	0.0021	6.0		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0158	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0048	0.0098	0.0021	6.0	0.0048	0.0098	0.0021	6.0	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 7																
Story 6	h (in) =	30	34	30	0.0048	0.0098	0.0021	6.0	34	30	0.0048	0.0098	0.0021	6.0		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0158	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0048	0.0098	0.0021	6.0	0.0048	0.0098	0.0021	6.0	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 6																
Story 5	h (in) =	30	34	30	0.0050	0.0100	0.0021	6.0	34	30	0.0050	0.0100	0.0021	6.0		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0158	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0050	0.0100	0.0021	6.0	0.0050	0.0100	0.0021	6.0	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 5																
Story 4	h (in) =	30	34	30	0.0050	0.0100	0.0021	6.0	34	30	0.0050	0.0100	0.0021	6.0		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0158	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0050	0.0100	0.0021	6.0	0.0050	0.0100	0.0021	6.0	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 4																
Story 3	h (in) =	30	34	30	0.0048	0.0098	0.0021	6.0	34	30	0.0048	0.0098	0.0021	6.0		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0158	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0048	0.0098	0.0021	6.0	0.0048	0.0098	0.0021	6.0	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 3																
Story 2	h (in) =	30	34	30	0.0048	0.0095	0.0020	6.5	34	30	0.0048	0.0095	0.0020	6.5		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0100	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0048	0.0095	0.0020	6.5	0.0048	0.0095	0.0020	6.5	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Floor 2																
Story 1	h (in) =	30	34	30	0.0048	0.0095	0.0020	6.5	34	30	0.0048	0.0095	0.0020	6.5		
	b (in) =	30	30	30	0.0130	0.0078	4.0	30	30	30	0.0100	0.0078	4.0	30		
	ρ_{br} =	0.0130	0.0078	4.0	0.0048	0.0095	0.0020	6.5	0.0048	0.0095	0.0020	6.5	0.0130	0.0078	4.0	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Grade beam column height (in) = 24																
Basement column height (in) = 30																
Design base shear = 0.044 g, 276 k																
<table><tr><td>f'_c beams = 5.0 ksi</td><td>f'_c cols upper = 6.0 ksi</td></tr><tr><td>f_y rebar nom. = 60 ksi</td><td>f'_c cols lower = 6.0 ksi</td></tr></table>													f'_c beams = 5.0 ksi	f'_c cols upper = 6.0 ksi	f_y rebar nom. = 60 ksi	f'_c cols lower = 6.0 ksi
f'_c beams = 5.0 ksi	f'_c cols upper = 6.0 ksi															
f_y rebar nom. = 60 ksi	f'_c cols lower = 6.0 ksi															

13 feet

15 feet

Design Documentation (2 of 2)																
	SCWB =	0.92	1.19	1.10	1.16	0.71	1.18	1.23	1.16	0.71	1.19	1.10	0.67	1.16	0.92	Design Drifts:
	Joint $\Phi V_r/V_u$	2.51				2.01				2.01					2.51	
Story 12	$\Phi M_r/M_u$	1.42				4.16				4.17					1.42	
	$\Phi V_r/V_{mpr}$	1.58				1.26				1.26					1.58	0.8%
	$P/A_g f_c$	0.02				0.05				0.05					0.02	
Floor 12		1.47	1.17	1.24	1.16	1.25	1.28	1.28	1.16	1.25	1.17	1.24	1.16	1.47		
		1.46				1.29				1.29				1.46		
Story 11	$\Phi M_r/M_u$	1.81				3.68				3.68					1.81	
	$\Phi V_r/V_{mpr}$	1.41				1.55				1.55					1.41	1.2%
	$P/A_g f_c$	0.05				0.09				0.09					0.05	
Floor 11		1.44	1.16	1.30	1.15	1.29	1.26	1.35	1.15	1.29	1.16	1.30	1.15	1.44		
		1.33				1.19				1.19				1.33		
Story 10	$\Phi M_r/M_u$	1.58				3.05				3.05					1.58	
	$\Phi V_r/V_{mpr}$	1.77				1.38				1.38					1.77	1.5%
	$P/A_g f_c$	0.07				0.13				0.13					0.07	
Floor 10		1.43	1.16	1.38	1.17	1.33	1.26	1.45	1.17	1.32	1.16	1.38	1.17	1.43		
		1.21				1.08				1.08				1.21		
Story 9	$\Phi M_r/M_u$	1.33				2.14				2.14					1.33	
	$\Phi V_r/V_{mpr}$	1.93				1.49				1.49					1.93	1.6%
	$P/A_g f_c$	0.10				0.18				0.18					0.10	
Floor 9		1.24	1.14	1.42	1.16	1.23	1.30	1.51	1.16	1.23	1.14	1.42	1.16	1.24		
		1.30				1.16				1.16				1.30		
Story 8	$\Phi M_r/M_u$	1.40				2.30				2.30					1.40	
	$\Phi V_r/V_{mpr}$	2.12				1.59				1.59					2.12	1.6%
	$P/A_g f_c$	0.13				0.23				0.23					0.13	
Floor 8		1.26	1.15	1.46	1.17	1.30	1.29	1.56	1.17	1.30	1.15	1.46	1.17	1.26		
		1.27				1.13				1.13				1.27		
Story 7	$\Phi M_r/M_u$	1.40				2.32				2.32					1.41	
	$\Phi V_r/V_{mpr}$	1.98				1.49				1.49					1.98	1.7%
	$P/A_g f_c$	0.15				0.27				0.27					0.15	
Floor 7		1.27	1.14	1.43	1.18	1.37	1.27	1.53	1.18	1.37	1.14	1.43	1.18	1.26		
		1.24				1.11				1.11				1.24		
Story 6	$\Phi M_r/M_u$	1.41				2.34				2.34					1.39	
	$\Phi V_r/V_{mpr}$	1.87				1.29				1.29					1.88	1.8%
	$P/A_g f_c$	0.18				0.32				0.32					0.18	
Floor 6		1.31	1.12	1.38	1.18	1.45	1.24	1.50	1.18	1.45	1.12	1.38	1.18	1.30		
		1.24				1.11				1.11				1.24		
Story 5	$\Phi M_r/M_u$	1.42				2.39				2.39					1.42	
	$\Phi V_r/V_{mpr}$	1.77				1.33				1.33					1.77	1.9%
	$P/A_g f_c$	0.21				0.37				0.37					0.21	
Floor 5		1.31	1.14	1.40	1.16	1.49	1.24	1.57	1.16	1.49	1.14	1.41	1.16	1.31		
		1.20				1.07				1.07				1.20		
Story 4	$\Phi M_r/M_u$	1.44				2.48				2.48					1.44	
	$\Phi V_r/V_{mpr}$	1.69				1.36				1.36					1.69	1.9%
	$P/A_g f_c$	0.24				0.41				0.41					0.24	
Floor 4		1.34	1.15	1.36	1.16	1.57	1.23	1.57	1.16	1.57	1.15	1.36	1.16	1.34		
		1.20				1.07				1.07				1.20		
Story 3	$\Phi M_r/M_u$	1.48				2.59				2.59					1.48	
	$\Phi V_r/V_{mpr}$	1.62				1.4				1.4					1.62	1.9%
	$P/A_g f_c$	0.27				0.46				0.46					0.27	
Floor 3		1.40	1.13	1.28	1.17	1.57	1.19	1.51	1.17	1.57	1.13	1.28	1.17	1.40		
		1.24				1.10				1.10				1.24		
Story 2	$\Phi M_r/M_u$	1.45				2.11				2.11					1.45	
	$\Phi V_r/V_{mpr}$	1.37				1.68				1.68					1.37	1.9%
	$P/A_g f_c$	0.30				0.51				0.51					0.30	
Floor 2		1.46	1.14	1.20	1.16	1.54	1.20	1.57	1.16	1.54	1.14	1.20	1.16	1.46		
		1.27				1.12				1.12				1.27		
Story 1	$\Phi M_r/M_u$	1.33				1.89				1.89					1.33	
	$\Phi V_r/V_{mpr}$	1.39				1.73				1.73					1.39	1.7%
	$P/A_g f_c$	0.32				0.56				0.56					0.32	

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	10131 0.35 1.20 0.074 0.100 120 0.02	6012 -10636 0.35 2.128E+08 0.0335 -0.047 0.074 99							10131 0.35 1.20 0.074 0.100 120 0.02
Floor 12	Story 11	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	11148 0.35 1.20 0.071 0.100 117 0.03	6307 -13745 0.35 2.128E+08 0.0335 -0.058 0.086 109							11148 0.35 1.20 0.071 0.100 117 0.03
Floor 11	Story 10	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	12147 0.35 1.20 0.069 0.100 115 0.05	6637 -14869 0.35 2.128E+08 0.0335 -0.060 0.090 109							12147 0.35 1.20 0.069 0.100 115 0.05
Floor 10	Story 9	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	13129 0.35 1.19 0.067 0.100 112 0.06	7214 -15717 0.35 3.675E+08 0.030 -0.047 0.070 106							13129 0.35 1.19 0.067 0.100 112 0.06
Floor 9	Story 8	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	14095 0.35 1.19 0.065 0.100 110 0.08	8432 -18855 0.35 3.675E+08 0.031 -0.047 0.073 106							14095 0.35 1.19 0.065 0.100 110 0.08
Floor 8	Story 7	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	15048 0.35 1.19 0.063 0.100 108 0.10	8813 -19283 0.35 3.675E+08 0.033 -0.050 0.074 110							15048 0.35 1.19 0.063 0.100 108 0.10
Floor 7	Story 6	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	15987 0.35 1.19 0.062 0.100 105 0.11	8814 -19724 0.35 3.675E+08 0.033 -0.050 0.074 110							15987 0.35 1.19 0.062 0.100 105 0.11
Floor 6	Story 5	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	16914 0.35 1.18 0.060 0.100 103 0.13	9268 -20163 0.35 3.675E+08 0.033 -0.050 0.076 110							16914 0.35 1.18 0.060 0.100 103 0.13
Floor 5	Story 4	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	17830 0.36 1.18 0.058 0.100 101 0.15	9268 -20163 0.35 3.675E+08 0.033 -0.050 0.076 110							17830 0.36 1.18 0.058 0.100 101 0.15
Floor 4	Story 3	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	18736 0.37 1.18 0.056 0.100 99 0.16	9268 -20163 0.35 3.675E+08 0.033 -0.050 0.074 110							18736 0.37 1.18 0.056 0.100 99 0.16
Floor 3	Story 2	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	19631 0.39 1.18 0.055 0.100 97 0.18	9268 -20163 0.35 3.675E+08 0.033 -0.047 0.073 106							19631 0.39 1.18 0.055 0.100 97 0.18
Floor 2	Story 1	$M_{y,exp}$ (k-in) = E_{eff}/E_g = M_d/M_y = $\Theta_{cap,pl}$ (rad) = Θ_{pc} (rad) = λ = $(P/A_g f_c)_{exp}$ =	20517 0.40 1.18 0.053 0.100 95 0.19	9268 -20163 0.35 3.675E+08 0.033 -0.047 0.073 106							20517 0.40 1.18 0.053 0.100 95 0.19
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.34											
Model periods (sec): $T_1 = 2.00$ $T_2 = 0.67$ $T_3 = 0.39$											
$f_{rebar,exp} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1020

Number of Stories: 20

Fundamental Period (sec): 2.63

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined by drift limits and column-beam compatibility considerations. Beam strengths were controlled by force demands, particularly lateral forces. Column strengths were determined by strong-column weak-beam (SCWB) ratios except in the bottom story where flexural considerations controlled and in the upper stories where minimum reinforcement ratios controlled. Column concrete was increased to 6.0 ksi in the lower stories to meet joint shear requirements. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Page 62 of 240

Design Documentation (2 of 2)									
	SCWB	0.92	0.92	0.92	0.92	0.92	0.92	0.92	Design Drifts
	Joint $\phi V_u/V_u$	0.32	0.32	0.32	0.32	0.32	0.32	0.32	
Story 20	$\phi M_u/M_u$	5.89	3.76	3.76	3.76	3.76	3.76	3.76	1.0%
	$\phi V_u/V_{up}$	1.59	1.31	1.31	1.31	1.31	1.31	1.31	
	$P/A_f \epsilon_c$	3.01	3.01	3.01	3.01	3.01	3.01	3.01	
Floor 20		1.85	1.24	1.24	1.24	1.24	1.24	1.24	
Story 19	$\phi M_u/M_u$	3.10	3.82	3.82	3.82	3.82	3.82	3.82	1.2%
	$\phi V_u/V_{up}$	1.51	1.17	1.17	1.17	1.17	1.17	1.17	
	$P/A_f \epsilon_c$	3.01	3.02	3.02	3.02	3.02	3.02	3.02	
Floor 19		1.85	1.40	1.40	1.40	1.40	1.40	1.40	
Story 18	$\phi M_u/M_u$	3.10	1.87	1.87	1.87	1.87	1.87	1.87	1.4%
	$\phi V_u/V_{up}$	1.43	1.12	1.12	1.12	1.12	1.12	1.12	
	$P/A_f \epsilon_c$	3.02	3.03	3.03	3.03	3.03	3.03	3.03	
Floor 18		1.85	1.40	1.40	1.40	1.40	1.40	1.40	
Story 17	$\phi M_u/M_u$	2.88	1.72	1.72	1.72	1.72	1.72	1.72	1.6%
	$\phi V_u/V_{up}$	1.63	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.03	3.04	3.04	3.04	3.04	3.04	3.04	
Floor 17		1.48	1.29	1.29	1.29	1.29	1.29	1.29	
Story 16	$\phi M_u/M_u$	2.07	1.56	1.56	1.56	1.56	1.56	1.56	1.7%
	$\phi V_u/V_{up}$	1.68	1.38	1.38	1.38	1.38	1.38	1.38	
	$P/A_f \epsilon_c$	3.05	3.05	3.05	3.05	3.05	3.05	3.05	
Floor 16		1.29	1.22	1.22	1.22	1.22	1.22	1.22	
Story 15	$\phi M_u/M_u$	1.85	1.47	1.47	1.47	1.47	1.47	1.47	1.8%
	$\phi V_u/V_{up}$	1.42	1.11	1.11	1.11	1.11	1.11	1.11	
	$P/A_f \epsilon_c$	3.06	3.06	3.06	3.06	3.06	3.06	3.06	
Floor 15		1.33	1.22	1.22	1.22	1.22	1.22	1.22	
Story 14	$\phi M_u/M_u$	1.79	1.41	1.41	1.41	1.41	1.41	1.41	2.0%
	$\phi V_u/V_{up}$	1.49	1.15	1.15	1.15	1.15	1.15	1.15	
	$P/A_f \epsilon_c$	3.07	3.07	3.07	3.07	3.07	3.07	3.07	
Floor 14		1.24	1.20	1.20	1.20	1.20	1.20	1.20	
Story 13	$\phi M_u/M_u$	1.55	1.41	1.41	1.41	1.41	1.41	1.41	2.0%
	$\phi V_u/V_{up}$	1.25	1.15	1.15	1.15	1.15	1.15	1.15	
	$P/A_f \epsilon_c$	3.09	3.07	3.07	3.07	3.07	3.07	3.07	
Floor 13		1.28	1.16	1.16	1.16	1.16	1.16	1.16	
Story 12	$\phi M_u/M_u$	1.81	1.49	1.49	1.49	1.49	1.49	1.49	1.9%
	$\phi V_u/V_{up}$	1.40	1.11	1.11	1.11	1.11	1.11	1.11	
	$P/A_f \epsilon_c$	3.09	3.07	3.07	3.07	3.07	3.07	3.07	
Floor 12		1.33	1.11	1.11	1.11	1.11	1.11	1.11	
Story 11	$\phi M_u/M_u$	1.74	1.44	1.44	1.44	1.44	1.44	1.44	2.0%
	$\phi V_u/V_{up}$	1.39	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.10	3.08	3.08	3.08	3.08	3.08	3.08	
Floor 11		1.36	1.12	1.12	1.12	1.12	1.12	1.12	
Story 10	$\phi M_u/M_u$	1.83	1.40	1.40	1.40	1.40	1.40	1.40	2.0%
	$\phi V_u/V_{up}$	1.87	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.10	3.07	3.07	3.07	3.07	3.07	3.07	
Floor 10		1.34	1.13	1.13	1.13	1.13	1.13	1.13	
Story 9	$\phi M_u/M_u$	1.76	1.36	1.36	1.36	1.36	1.36	1.36	2.0%
	$\phi V_u/V_{up}$	1.77	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.11	3.08	3.08	3.08	3.08	3.08	3.08	
Floor 9		1.33	1.12	1.12	1.12	1.12	1.12	1.12	
Story 8	$\phi M_u/M_u$	1.76	1.37	1.37	1.37	1.37	1.37	1.37	2.0%
	$\phi V_u/V_{up}$	1.67	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.12	3.09	3.09	3.09	3.09	3.09	3.09	
Floor 8		1.30	1.14	1.14	1.14	1.14	1.14	1.14	
Story 7	$\phi M_u/M_u$	1.69	1.36	1.36	1.36	1.36	1.36	1.36	2.0%
	$\phi V_u/V_{up}$	1.56	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.13	3.09	3.09	3.09	3.09	3.09	3.09	
Floor 7		1.30	1.13	1.13	1.13	1.13	1.13	1.13	
Story 6	$\phi M_u/M_u$	1.89	1.39	1.39	1.39	1.39	1.39	1.39	1.9%
	$\phi V_u/V_{up}$	1.25	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.14	3.10	3.10	3.10	3.10	3.10	3.10	
Floor 6		1.25	1.14	1.14	1.14	1.14	1.14	1.14	
Story 5	$\phi M_u/M_u$	1.63	1.39	1.39	1.39	1.39	1.39	1.39	1.9%
	$\phi V_u/V_{up}$	1.42	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.16	3.11	3.11	3.11	3.11	3.11	3.11	
Floor 5		1.25	1.13	1.13	1.13	1.13	1.13	1.13	
Story 4	$\phi M_u/M_u$	1.63	1.40	1.40	1.40	1.40	1.40	1.40	1.8%
	$\phi V_u/V_{up}$	1.21	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.17	3.11	3.11	3.11	3.11	3.11	3.11	
Floor 4		1.20	1.12	1.12	1.12	1.12	1.12	1.12	
Story 3	$\phi M_u/M_u$	1.63	1.42	1.42	1.42	1.42	1.42	1.42	1.8%
	$\phi V_u/V_{up}$	1.27	1.15	1.15	1.15	1.15	1.15	1.15	
	$P/A_f \epsilon_c$	3.18	3.12	3.12	3.12	3.12	3.12	3.12	
Floor 3		1.33	1.25	1.25	1.25	1.25	1.25	1.25	
Story 2	$\phi M_u/M_u$	1.63	1.38	1.38	1.38	1.38	1.38	1.38	1.6%
	$\phi V_u/V_{up}$	1.21	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.19	3.13	3.13	3.13	3.13	3.13	3.13	
Floor 2		1.98	1.54	1.54	1.54	1.54	1.54	1.54	
Story 1	$\phi M_u/M_u$	1.02	1.11	1.11	1.11	1.11	1.11	1.11	1.1%
	$\phi V_u/V_{up}$	1.09	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f \epsilon_c$	3.20	3.13	3.13	3.13	3.13	3.13	3.13	

[illegible]

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1021

Number of Stories: 20

Fundamental Period (sec): 2.36

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Initial member sizes (beam depths, column dimensions) were determined by both drift limits and minimum joint shear requirements. Beam strengths were controlled by force demands, particularly lateral forces. Column strengths were determined by strong-column weak-beam (SCWB) ratios except in the lower stories, where they were controlled by minimum reinforcement requirements. Beam stirrups were controlled by shear capacity design. The column stirrups were controlled by both the shear capacity design and the confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Page 67 of 240

Design Documentation (2 of 2)									
	SCWB =	1.03	1.28	1.44	1.60	1.76	1.92	2.08	Design Drifts
Joint $\Phi V_u/V_u$	1.82	1.82	1.82	1.82	1.82	1.82	1.82	1.82	
Story 25	$\phi M_u/M_u =$	1.88	1.88	1.88	1.88	1.88	1.88	1.88	
	$\Phi V_u/V_{up} =$	1.51	1.51	1.51	1.51	1.51	1.51	1.51	0.8%
	$P/A_f =$	3.03	3.03	3.03	3.03	3.03	3.03	3.03	
Floor 20	$\phi M_u/M_u =$	1.49	1.49	1.49	1.49	1.49	1.49	1.49	
	$\Phi V_u/V_{up} =$	1.51	1.51	1.51	1.51	1.51	1.51	1.51	
	$P/A_f =$	3.03	3.03	3.03	3.03	3.03	3.03	3.03	
Story 19	$\phi M_u/M_u =$	1.81	1.81	1.81	1.81	1.81	1.81	1.81	
	$\Phi V_u/V_{up} =$	1.91	1.91	1.91	1.91	1.91	1.91	1.91	1.1%
	$P/A_f =$	3.05	3.05	3.05	3.05	3.05	3.05	3.05	
Floor 19	$\phi M_u/M_u =$	1.40	1.40	1.40	1.40	1.40	1.40	1.40	
	$\Phi V_u/V_{up} =$	1.66	1.66	1.66	1.66	1.66	1.66	1.66	
	$P/A_f =$	3.08	3.08	3.08	3.08	3.08	3.08	3.08	
Story 18	$\phi M_u/M_u =$	1.72	1.72	1.72	1.72	1.72	1.72	1.72	
	$\Phi V_u/V_{up} =$	1.67	1.67	1.67	1.67	1.67	1.67	1.67	1.4%
	$P/A_f =$	3.08	3.08	3.08	3.08	3.08	3.08	3.08	
Floor 18	$\phi M_u/M_u =$	1.40	1.40	1.40	1.40	1.40	1.40	1.40	
	$\Phi V_u/V_{up} =$	1.66	1.66	1.66	1.66	1.66	1.66	1.66	
	$P/A_f =$	3.08	3.08	3.08	3.08	3.08	3.08	3.08	
Story 17	$\phi M_u/M_u =$	1.59	1.59	1.59	1.59	1.59	1.59	1.59	
	$\Phi V_u/V_{up} =$	1.87	1.87	1.87	1.87	1.87	1.87	1.87	1.7%
	$P/A_f =$	3.11	3.11	3.11	3.11	3.11	3.11	3.11	
Floor 17	$\phi M_u/M_u =$	1.37	1.37	1.37	1.37	1.37	1.37	1.37	
	$\Phi V_u/V_{up} =$	1.19	1.19	1.19	1.19	1.19	1.19	1.19	
	$P/A_f =$	3.15	3.15	3.15	3.15	3.15	3.15	3.15	
Story 16	$\phi M_u/M_u =$	1.59	1.59	1.59	1.59	1.59	1.59	1.59	
	$\Phi V_u/V_{up} =$	2.01	2.01	2.01	2.01	2.01	2.01	2.01	1.9%
	$P/A_f =$	3.15	3.15	3.15	3.15	3.15	3.15	3.15	
Floor 16	$\phi M_u/M_u =$	1.37	1.37	1.37	1.37	1.37	1.37	1.37	
	$\Phi V_u/V_{up} =$	1.19	1.19	1.19	1.19	1.19	1.19	1.19	
	$P/A_f =$	3.15	3.15	3.15	3.15	3.15	3.15	3.15	
Story 15	$\phi M_u/M_u =$	1.51	1.51	1.51	1.51	1.51	1.51	1.51	
	$\Phi V_u/V_{up} =$	1.91	1.91	1.91	1.91	1.91	1.91	1.91	2.0%
	$P/A_f =$	3.18	3.18	3.18	3.18	3.18	3.18	3.18	
Floor 15	$\phi M_u/M_u =$	1.35	1.35	1.35	1.35	1.35	1.35	1.35	
	$\Phi V_u/V_{up} =$	1.41	1.41	1.41	1.41	1.41	1.41	1.41	
	$P/A_f =$	3.21	3.21	3.21	3.21	3.21	3.21	3.21	
Story 14	$\phi M_u/M_u =$	1.53	1.53	1.53	1.53	1.53	1.53	1.53	
	$\Phi V_u/V_{up} =$	2.06	2.06	2.06	2.06	2.06	2.06	2.06	1.7%
	$P/A_f =$	3.15	3.15	3.15	3.15	3.15	3.15	3.15	
Floor 14	$\phi M_u/M_u =$	1.35	1.35	1.35	1.35	1.35	1.35	1.35	
	$\Phi V_u/V_{up} =$	1.41	1.41	1.41	1.41	1.41	1.41	1.41	
	$P/A_f =$	3.21	3.21	3.21	3.21	3.21	3.21	3.21	
Story 13	$\phi M_u/M_u =$	1.52	1.52	1.52	1.52	1.52	1.52	1.52	
	$\Phi V_u/V_{up} =$	1.95	1.95	1.95	1.95	1.95	1.95	1.95	1.8%
	$P/A_f =$	3.18	3.18	3.18	3.18	3.18	3.18	3.18	
Floor 13	$\phi M_u/M_u =$	1.41	1.41	1.41	1.41	1.41	1.41	1.41	
	$\Phi V_u/V_{up} =$	1.32	1.32	1.32	1.32	1.32	1.32	1.32	
	$P/A_f =$	3.21	3.21	3.21	3.21	3.21	3.21	3.21	
Story 12	$\phi M_u/M_u =$	1.50	1.50	1.50	1.50	1.50	1.50	1.50	
	$\Phi V_u/V_{up} =$	1.85	1.85	1.85	1.85	1.85	1.85	1.85	1.9%
	$P/A_f =$	3.21	3.21	3.21	3.21	3.21	3.21	3.21	
Floor 12	$\phi M_u/M_u =$	1.40	1.40	1.40	1.40	1.40	1.40	1.40	
	$\Phi V_u/V_{up} =$	1.28	1.28	1.28	1.28	1.28	1.28	1.28	
	$P/A_f =$	3.21	3.21	3.21	3.21	3.21	3.21	3.21	
Story 11	$\phi M_u/M_u =$	1.36	1.36	1.36	1.36	1.36	1.36	1.36	
	$\Phi V_u/V_{up} =$	1.77	1.77	1.77	1.77	1.77	1.77	1.77	1.8%
	$P/A_f =$	3.23	3.23	3.23	3.23	3.23	3.23	3.23	
Floor 11	$\phi M_u/M_u =$	1.24	1.24	1.24	1.24	1.24	1.24	1.24	
	$\Phi V_u/V_{up} =$	1.21	1.21	1.21	1.21	1.21	1.21	1.21	
	$P/A_f =$	3.23	3.23	3.23	3.23	3.23	3.23	3.23	
Story 10	$\phi M_u/M_u =$	1.45	1.45	1.45	1.45	1.45	1.45	1.45	
	$\Phi V_u/V_{up} =$	1.68	1.68	1.68	1.68	1.68	1.68	1.68	1.7%
	$P/A_f =$	3.26	3.26	3.26	3.26	3.26	3.26	3.26	
Floor 10	$\phi M_u/M_u =$	1.30	1.30	1.30	1.30	1.30	1.30	1.30	
	$\Phi V_u/V_{up} =$	1.35	1.35	1.35	1.35	1.35	1.35	1.35	
	$P/A_f =$	3.26	3.26	3.26	3.26	3.26	3.26	3.26	
Story 9	$\phi M_u/M_u =$	1.46	1.46	1.46	1.46	1.46	1.46	1.46	
	$\Phi V_u/V_{up} =$	1.48	1.48	1.48	1.48	1.48	1.48	1.48	1.7%
	$P/A_f =$	3.29	3.29	3.29	3.29	3.29	3.29	3.29	
Floor 9	$\phi M_u/M_u =$	1.31	1.31	1.31	1.31	1.31	1.31	1.31	
	$\Phi V_u/V_{up} =$	1.16	1.16	1.16	1.16	1.16	1.16	1.16	
	$P/A_f =$	3.31	3.31	3.31	3.31	3.31	3.31	3.31	
Story 8	$\phi M_u/M_u =$	1.45	1.45	1.45	1.45	1.45	1.45	1.45	
	$\Phi V_u/V_{up} =$	1.51	1.51	1.51	1.51	1.51	1.51	1.51	1.7%
	$P/A_f =$	3.32	3.32	3.32	3.32	3.32	3.32	3.32	
Floor 8	$\phi M_u/M_u =$	1.31	1.31	1.31	1.31	1.31	1.31	1.31	
	$\Phi V_u/V_{up} =$	1.30	1.30	1.30	1.30	1.30	1.30	1.30	
	$P/A_f =$	3.32	3.32	3.32	3.32	3.32	3.32	3.32	
Story 7	$\phi M_u/M_u =$	1.46	1.46	1.46	1.46	1.46	1.46	1.46	
	$\Phi V_u/V_{up} =$	1.54	1.54	1.54	1.54	1.54	1.54	1.54	1.7%
	$P/A_f =$	3.34	3.34	3.34	3.34	3.34	3.34	3.34	
Floor 7	$\phi M_u/M_u =$	1.29	1.29	1.29	1.29	1.29	1.29	1.29	
	$\Phi V_u/V_{up} =$	1.38	1.38	1.38	1.38	1.38	1.38	1.38	
	$P/A_f =$	3.37	3.37	3.37	3.37	3.37	3.37	3.37	
Story 6	$\phi M_u/M_u =$	1.42	1.42	1.42	1.42	1.42	1.42	1.42	
	$\Phi V_u/V_{up} =$	1.59	1.59	1.59	1.59	1.59	1.59	1.59	1.7%
	$P/A_f =$	3.37	3.37	3.37	3.37	3.37	3.37	3.37	
Floor 6	$\phi M_u/M_u =$	1.30	1.30	1.30	1.30	1.30	1.30	1.30	
	$\Phi V_u/V_{up} =$	1.41	1.41	1.41	1.41	1.41	1.41	1.41	
	$P/A_f =$	3.41	3.41	3.41	3.41	3.41	3.41	3.41	
Story 5	$\phi M_u/M_u =$	1.39	1.39	1.39	1.39	1.39	1.39	1.39	
	$\Phi V_u/V_{up} =$	1.61	1.61	1.61	1.61	1.61	1.61	1.61	1.6%
	$P/A_f =$	3.40	3.40	3.40	3.40	3.40	3.40	3.40	
Floor 5	$\phi M_u/M_u =$	1.46	1.46	1.46	1.46	1.46	1.46	1.46	
	$\Phi V_u/V_{up} =$	1.91	1.91	1.91	1.91	1.91	1.91	1.91	
	$P/A_f =$	3.43	3.43	3.43	3.43	3.43	3.43	3.43	
Story 4	$\phi M_u/M_u =$	1.74	1.74	1.74	1.74	1.74	1.74	1.74	
	$\Phi V_u/V_{up} =$	1.47	1.47	1.47	1.47	1.47	1.47	1.47	1.3%
	$P/A_f =$	3.32	3.32	3.32	3.32	3.32	3.32	3.32	
Floor 4	$\phi M_u/M_u =$	1.60	1.60	1.60	1.60	1.60	1.60	1.60	
	$\Phi V_u/V_{up} =$	1.91	1.91	1.91	1.91	1.91	1.91	1.91	
	$P/A_f =$	3.43	3.43	3.43	3.43	3.43	3.43	3.43	
Story 3	$\phi M_u/M_u =$	1.77	1.77	1.77	1.77	1.77	1.77	1.77	
	$\Phi V_u/V_{up} =$	1.48	1.48	1.48	1.48	1.48	1.48	1.48	1.2%
	$P/A_f =$	3.34	3.34	3.34	3.34	3.34	3.34	3.34	
Floor 3	$\phi M_u/M_u =$	1.66	1.66	1.66	1.66	1.66	1.66	1.66	
	$\Phi V_u/V_{up} =$	2.00	2.00	2.00	2.00	2.00	2.00	2.00	
	$P/A_f =$	3.43	3.43	3.43	3.43	3.43	3.43	3.43	
Story 2	$\phi M_u/M_u =$	1.86	1.86	1.86	1.86	1.86	1.86	1.86	
	$\Phi V_u/V_{up} =$	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.2%
	$P/A_f =$	3.36	3.36	3.36	3.36	3.36	3.36	3.36	
Floor 2	$\phi M_u/M_u =$	1.71	1.71	1.71	1.71	1.71	1.71	1.71	
	$\Phi V_u/V_{up} =$	2.00	2.00	2.00	2.00	2.00	2.00	2.00	
	$P/A_f =$	3.43	3.43	3.43	3.43	3.43	3.43	3.43	
Story 1	$\phi M_u/M_u =$	1.42	1.42	1.42	1.42	1.42	1.42	1.42	
	$\Phi V_u/V_{up} =$	1.51	1.51	1.51	1.51	1.51	1.51	1.51	1.2%
	$P/A_f =$	3.37	3.37	3.37	3.37	3.37	3.37	3.37	

Modeling Documentation (1 of 1)

Floor 9	Story 9	M _{base} (k-in)	1672	3279	3379	1.79E+08	3580	3279	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	3379	
---------	---------	--------------------------	------	------	------	----------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	------	--

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1022

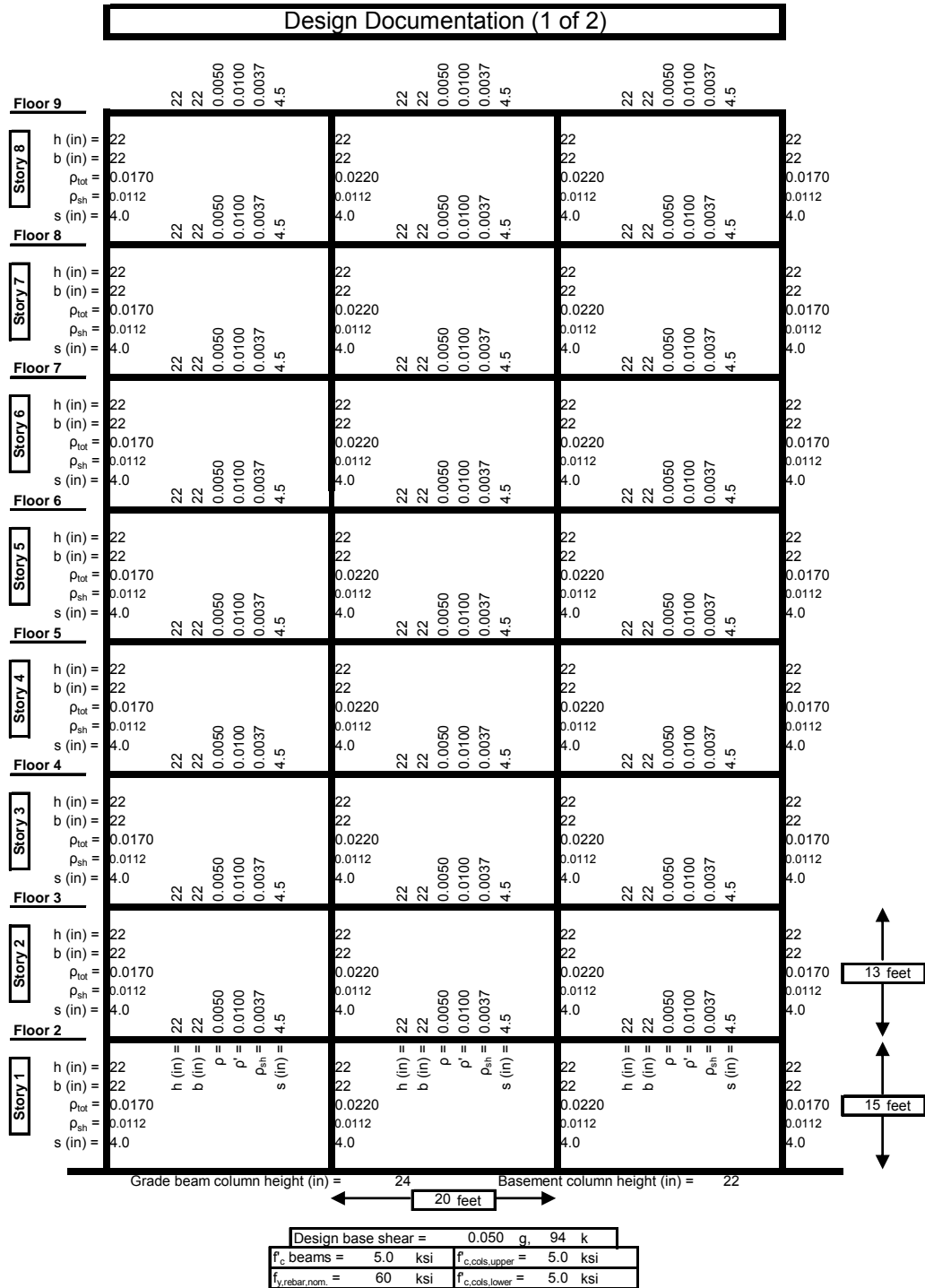
Number of Stories: 8

Fundamental Period (sec): 1.80

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

All members sized according to ground floor requirements for index 1012. Columns reinforced to be uniform and meet SCWB requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)																	
Story 8	SCWB =	0.59	2.42	1.50	0.51	2.31	0.58	2.20	1.78	0.51	2.31	0.58	2.43	1.50	0.51	2.31	Design Drifts:
	Joint $\Phi V_r/V_u =$	1.41					1.25					1.25					
	$\Phi M_r/M_u =$	2.13					6.05					6.06					
Floor 8	$\Phi V_r/V_{mpr} =$	2.73					2.02					2.02					0.6%
	$P/A_g f'_c =$	0.02					0.05					0.05					
		1.21	1.56	1.66	0.51	2.31	1.21	1.68	1.75	0.51	2.31	1.21	1.56	1.66	0.51	2.31	
Story 7		1.21					1.08					1.08					0.9%
	$\Phi M_r/M_u =$	2.50					4.55					4.55					
	$\Phi V_r/V_{mpr} =$	2.52					2.31					2.31					
Floor 7	$P/A_g f'_c =$	0.05					0.09					0.09					0.9%
		1.25	1.35	1.53	0.51	2.31	1.29	1.44	1.67	0.51	2.31	1.29	1.36	1.53	0.51	2.31	
		1.21					1.08					1.08					
Story 6	$\Phi M_r/M_u =$	2.17					3.51					3.51					1.2%
	$\Phi V_r/V_{mpr} =$	2.89					2.11					2.11					
	$P/A_g f'_c =$	0.07					0.13					0.13					
Floor 6		1.30	1.21	1.43	0.51	2.31	1.38	1.29	1.59	0.51	2.31	1.38	1.21	1.43	0.51	2.31	1.2%
		1.18					1.05					1.05					
		1.34	1.12	1.31	0.51	2.31	1.46	1.20	1.49	0.51	2.31	1.46	1.12	1.31	0.51	2.31	
Story 5	$\Phi M_r/M_u =$	2.02					3.06					3.06					1.5%
	$\Phi V_r/V_{mpr} =$	3.19					2.31					2.31					
	$P/A_g f'_c =$	0.10					0.18					0.18					
Floor 5		1.34	1.12	1.31	0.51	2.31	1.46	1.20	1.49	0.51	2.31	1.46	1.12	1.31	0.51	2.31	1.5%
		1.17					1.04					1.04					
		1.37	1.08	1.22	0.51	2.31	1.54	1.14	1.43	0.51	2.31	1.54	1.08	1.22	0.51	2.31	
Story 4	$\Phi M_r/M_u =$	1.93					2.86					2.86					1.7%
	$\Phi V_r/V_{mpr} =$	3.42					2.46					2.46					
	$P/A_g f'_c =$	0.12					0.23					0.23					
Floor 4		1.37	1.08	1.22	0.51	2.31	1.54	1.14	1.43	0.51	2.31	1.54	1.08	1.22	0.51	2.31	1.7%
		1.17					1.04					1.04					
		1.41	1.06	1.18	0.51	2.31	1.62	1.11	1.40	0.51	2.31	1.62	1.06	1.18	0.51	2.31	
Story 3	$\Phi M_r/M_u =$	1.88					2.81					2.81					1.8%
	$\Phi V_r/V_{mpr} =$	3.24					2.32					2.32					
	$P/A_g f'_c =$	0.15					0.27					0.27					
Floor 3		1.41	1.06	1.18	0.51	2.31	1.62	1.11	1.40	0.51	2.31	1.62	1.06	1.18	0.51	2.31	1.8%
		1.17					1.04					1.04					
		1.44	1.10	1.12	0.51	2.31	1.70	1.17	1.50	0.51	2.31	1.70	1.10	1.12	0.51	2.31	
Story 2	$\Phi M_r/M_u =$	1.83					2.61					2.61					1.8%
	$\Phi V_r/V_{mpr} =$	3.08					2.22					2.22					
	$P/A_g f'_c =$	0.17					0.32					0.32					
Floor 2		1.44	1.10	1.12	0.51	2.31	1.70	1.17	1.50	0.51	2.31	1.70	1.10	1.12	0.51	2.31	1.4%
		1.17					1.04					1.04					
		1.27	$(\Phi M_r/M_u)_{neg} = 1.10$	$(\Phi M_r/M_u)_{pos} = 1.12$	$M_{n, pos}/M_{n, neg} = 0.51$	$\Phi V_r/V_{mpr} = 2.31$	2.59	$(\Phi M_r/M_u)_{neg} = 1.17$	$(\Phi M_r/M_u)_{pos} = 1.50$	$M_{n, pos}/M_{n, neg} = 0.51$	$\Phi V_r/V_{mpr} = 2.31$	2.59	$(\Phi M_r/M_u)_{neg} = 1.10$	$(\Phi M_r/M_u)_{pos} = 1.12$	$M_{n, pos}/M_{n, neg} = 0.51$	$\Phi V_r/V_{mpr} = 2.31$	
Story 1	$\Phi M_r/M_u =$	1.27					2.26					2.26					1.4%
	$\Phi V_r/V_{mpr} =$	2.94					0.37					2.26					
	$P/A_g f'_c =$	0.20										0.37					

Modeling Documentation (1 of 1)											
Floor 9	Story 8	$M_{y,exp}$ (k-in) = 4629 $EI_{stf}/EI_g = 0.35$ $M_c/M_y = 1.21$ $\Theta_{cap,pl}$ (rad) = 0.091 Θ_{pc} (rad) = 0.100 $\lambda = 104$ $(P/A_g f_c)_{exp} = 0.02$	2571 -7650 0.35 7.178E+07 0.0446 -0.074 0.100 100	6158 0.35 1.21 0.092 0.100 102 0.03	2571 -7650 0.35 7.178E+07 0.0446 -0.074 0.100 100	6158 0.35 1.21 0.092 0.100 102 0.03	2571 -7650 0.35 7.178E+07 0.0446 -0.074 0.100 100	6158 0.35 1.21 0.092 0.100 102 0.03	2571 -7650 0.35 7.178E+07 0.0446 -0.074 0.100 100	4629 0.35 1.21 0.091 0.100 104 0.02	
Floor 8	Story 7	$M_{y,exp}$ (k-in) = 4952 $EI_{stf}/EI_g = 0.35$ $M_c/M_y = 1.21$ $\Theta_{cap,pl}$ (rad) = 0.088 Θ_{pc} (rad) = 0.100 $\lambda = 102$ $(P/A_g f_c)_{exp} = 0.03$	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	6783 0.35 1.20 0.087 0.100 98 0.06	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	6783 0.35 1.20 0.087 0.100 98 0.06	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	6783 0.35 1.20 0.087 0.100 98 0.06	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	4952 0.35 1.21 0.088 0.100 102 0.03	
Floor 7	Story 6	$M_{y,exp}$ (k-in) = 5270 $EI_{stf}/EI_g = 0.35$ $M_c/M_y = 1.20$ $\Theta_{cap,pl}$ (rad) = 0.086 Θ_{pc} (rad) = 0.100 $\lambda = 100$ $(P/A_g f_c)_{exp} = 0.05$	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	7396 0.35 1.20 0.082 0.100 94 0.10	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	7396 0.35 1.20 0.082 0.100 94 0.10	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	7396 0.35 1.20 0.082 0.100 94 0.10	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	5270 0.35 1.20 0.086 0.100 100 0.05	
Floor 6	Story 5	$M_{y,exp}$ (k-in) = 5585 $EI_{stf}/EI_g = 0.35$ $M_c/M_y = 1.20$ $\Theta_{cap,pl}$ (rad) = 0.083 Θ_{pc} (rad) = 0.100 $\lambda = 98$ $(P/A_g f_c)_{exp} = 0.06$	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	7997 0.38 1.19 0.077 0.100 90 0.13	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	7997 0.38 1.19 0.077 0.100 90 0.13	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	7997 0.38 1.19 0.077 0.100 90 0.13	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	5585 0.35 1.20 0.083 0.100 98 0.06	
Floor 5	Story 4	$M_{y,exp}$ (k-in) = 5896 $EI_{stf}/EI_g = 0.38$ $M_c/M_y = 1.20$ $\Theta_{cap,pl}$ (rad) = 0.081 Θ_{pc} (rad) = 0.100 $\lambda = 96$ $(P/A_g f_c)_{exp} = 0.08$	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	8589 0.46 1.19 0.073 0.100 86 0.16	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	8589 0.46 1.19 0.073 0.100 86 0.16	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	8589 0.46 1.19 0.073 0.100 86 0.16	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	5896 0.38 1.20 0.081 0.100 96 0.08	
Floor 4	Story 3	$M_{y,exp}$ (k-in) = 6203 $EI_{stf}/EI_g = 0.39$ $M_c/M_y = 1.20$ $\Theta_{cap,pl}$ (rad) = 0.078 Θ_{pc} (rad) = 0.100 $\lambda = 94$ $(P/A_g f_c)_{exp} = 0.10$	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	9171 0.49 1.18 0.069 0.100 83 0.19	2571 -7650 0.35 7.178E+07 0.0446 -0.074 0.100 100	9171 0.49 1.18 0.069 0.100 83 0.19	2571 -7650 0.35 7.178E+07 0.0446 -0.074 0.100 100	9171 0.49 1.18 0.069 0.100 83 0.19	2571 -7650 0.35 7.178E+07 0.0446 -0.074 0.100 100	6203 0.39 1.20 0.078 0.100 94 0.10	
Floor 3	Story 2	$M_{y,exp}$ (k-in) = 6508 $EI_{stf}/EI_g = 0.41$ $M_c/M_y = 1.19$ $\Theta_{cap,pl}$ (rad) = 0.076 Θ_{pc} (rad) = 0.100 $\lambda = 92$ $(P/A_g f_c)_{exp} = 0.11$	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	9745 0.52 1.18 0.065 0.100 79 0.22	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	9745 0.52 1.18 0.065 0.100 79 0.22	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	9745 0.52 1.18 0.065 0.100 79 0.22	2571 -7650 0.35 7.178E+07 0.045 -0.074 0.100 100	6508 0.41 1.19 0.076 0.100 92 0.11	
Floor 2	Story 1	$M_{y,exp}$ (k-in) = 6809 $EI_{stf}/EI_g = 0.42$ $M_c/M_y = 1.19$ $\Theta_{cap,pl}$ (rad) = 0.074 Θ_{pc} (rad) = 0.100 $\lambda = 90$ $(P/A_g f_c)_{exp} = 0.13$	$M_{y,pos,exp}$ (k-in) = 2571 $M_{y,n,slab,exp}$ (k-in) = -7650 $EI_{stf}/EI_g = 0.35$ $(\Theta_{cap,pl})_{pos}$ (rad) = 0.045 $(\Theta_{cap,pl})_{neg}$ (rad) = -0.074 Θ_{pc} (rad) = 0.100 $\lambda = 90$ $(P/A_g f_c)_{exp} = 0.13$	10311 0.55 1.17 0.061 0.100 76 0.26	$M_{y,pos,exp}$ (k-in) = 2571 $M_{y,n,slab,exp}$ (k-in) = -7650 $EI_{stf}/EI_g = 0.35$ $(\Theta_{cap,pl})_{pos}$ (rad) = 0.045 $(\Theta_{cap,pl})_{neg}$ (rad) = -0.074 Θ_{pc} (rad) = 0.100 $\lambda = 90$ $(P/A_g f_c)_{exp} = 0.13$	10311 0.55 1.17 0.061 0.100 76 0.26	$M_{y,pos,exp}$ (k-in) = 2571 $M_{y,n,slab,exp}$ (k-in) = -7650 $EI_{stf}/EI_g = 0.35$ $(\Theta_{cap,pl})_{pos}$ (rad) = 0.045 $(\Theta_{cap,pl})_{neg}$ (rad) = -0.074 Θ_{pc} (rad) = 0.100 $\lambda = 90$ $(P/A_g f_c)_{exp} = 0.13$	10311 0.55 1.17 0.061 0.100 76 0.26	$M_{y,pos,exp}$ (k-in) = 2571 $M_{y,n,slab,exp}$ (k-in) = -7650 $EI_{stf}/EI_g = 0.35$ $(\Theta_{cap,pl})_{pos}$ (rad) = 0.045 $(\Theta_{cap,pl})_{neg}$ (rad) = -0.074 Θ_{pc} (rad) = 0.100 $\lambda = 90$ $(P/A_g f_c)_{exp} = 0.13$	6809 0.42 1.19 0.074 0.100 90 0.13	
Mass tributary to one frame for lateral load (each floor) (k-s-s/in):										0.60	
Model periods (sec):										$T_1 = 1.80$ $T_2 = 0.58$ $T_3 = 0.34$	
f_y rebar expected =										67 ksi	

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1023

Number of Stories: 8

Fundamental Period (sec): 1.57

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

First floor beams initially reinforced for flexure, then above beams sized to meet 65% weak story requirement. Columns initially sized to 24 inches square to alleviate joint shear problems. Columns reinforced to meet SCWB provisions while not overstrengthening the first story columns. To do this, first story column reinforcement ratios were allowed below the minimum allowed by code. Column strengths increased to 6.0 ksi in upper stories and 8.0 ksi in lower stories to meet joint shear requirements. Beam strength increased to 6.0 ksi to facilitate joint concrete placement by contractor.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

Floor 9

Story 8

h (in) = 24

b (in) = 24

ρ_{tot} = 0.0170

ρ_{sh} = 0.0084

s (in) = 3.5

Story 7

h (in) = 24

b (in) = 24

ρ_{tot} = 0.0170

ρ_{sh} = 0.0084

s (in) = 3.5

Story 6

h (in) = 24

b (in) = 24

ρ_{tot} = 0.0170

ρ_{sh} = 0.0084

s (in) = 3.5

Story 5

h (in) = 24

b (in) = 24

ρ_{tot} = 0.0170

ρ_{sh} = 0.0084

s (in) = 3.5

Story 4

h (in) = 24

b (in) = 24

ρ_{tot} = 0.0170

ρ_{sh} = 0.0084

s (in) = 3.5

Story 3

h (in) = 24

b (in) = 24

ρ_{tot} = 0.0200

ρ_{sh} = 0.0084

s (in) = 3.5

Story 2

h (in) = 24

b (in) = 24

ρ_{tot} = 0.0120

ρ_{sh} = 0.0084

s (in) = 3.5

Story 1

h (in) = 24

b (in) = 24

ρ_{tot} = 0.0090

ρ_{sh} = 0.0084

s (in) = 3.5

22

24

0.0050

0.0068

0.0037

4.5

22

24

0.0050

0.0068

0.0037

4.5

22

24

0.0050

0.0068

0.0037

4.5

Grade beam column height (in) = 24

Basement column height (in) = 24

20 feet

13 feet

15 feet

Design base shear = 0.050 g, 94 k

f'_c beams = 6.0 ksi

f_y rebar nom. = 60 ksi

$f'_{c,cols,upper}$ = 6.0 ksi

$f'_{c,cols,lower}$ = 8.0 ksi

Design Documentation (2 of 2)																	
Story 8	SCWB =	0.93	1.77	1.73	0.75	5.75	0.78	1.67	2.01	0.75	5.75	0.78	1.77	1.73	0.75	5.75	Design Drifts:
	Joint $\Phi V_r/V_u$ =	2.50					1.91					1.91				2.50	
Floor 8	$\phi M_r/M_u$ =	2.68					6.77					6.78				2.68	0.6%
	$\phi V_r/V_{mpr}$ =	1.87					1.52					1.52				1.87	
Story 7	$P/A_g f'_c$ =	0.02					0.03					0.03				0.02	0.9%
Floor 7	$\phi M_r/M_u$ =	1.49	1.73	1.86	0.51	3.45	1.36	1.87	1.95	0.51	3.45	1.36	1.73	1.86	0.51	3.45	1.3%
	$\phi V_r/V_{mpr}$ =	1.45					1.29					1.29				1.45	
Story 6	$P/A_g f'_c$ =	0.03					0.06					0.06				0.03	1.5%
Floor 6	$\phi M_r/M_u$ =	1.41	1.73	2.00	0.51	2.70	1.31	1.85	2.14	0.51	2.70	1.31	1.73	2.00	0.51	2.70	1.7%
	$\phi V_r/V_{mpr}$ =	1.26					1.12					1.12				1.26	
Story 5	$P/A_g f'_c$ =	0.05					0.09					0.09				0.05	1.8%
Floor 5	$\phi M_r/M_u$ =	1.34	1.74	2.06	0.51	0.72	1.26	1.86	2.21	0.72	1.26	1.26	1.74	2.06	0.51	0.72	1.8%
	$\phi V_r/V_{mpr}$ =	1.09					0.97				0.97	0.97				1.09	
Story 4	$P/A_g f'_c$ =	0.07					0.13					0.13				0.07	1.7%
Floor 4	$\phi M_r/M_u$ =	1.31	1.74	2.04	0.51	0.69	1.26	1.85	2.25	0.69	1.26	1.26	1.74	2.04	0.51	0.69	1.8%
	$\phi V_r/V_{mpr}$ =	1.15					1.02				1.02	1.02				1.15	
Story 3	$P/A_g f'_c$ =	0.06					0.12					0.12				0.06	1.8%
Floor 3	$\phi M_r/M_u$ =	1.40	1.72	1.99	0.51	0.68	1.36	1.82	2.25	0.68	1.36	1.36	1.72	1.99	0.51	0.68	1.8%
	$\phi V_r/V_{mpr}$ =	1.11					0.99				0.99	0.99				1.11	
Story 2	$P/A_g f'_c$ =	0.08					0.14					0.14				0.08	1.8%
Floor 2	$\phi M_r/M_u$ =	1.26	1.74	1.98	0.51	0.67	1.19	1.81	2.25	0.67	1.19	1.19	1.74	1.98	0.51	0.67	1.4%
	$\phi V_r/V_{mpr}$ =	1.09					0.97				0.97	0.97				1.09	
Story 1	$P/A_g f'_c$ =	0.09					0.17					0.17				0.09	1.4%
Floor 1	$\phi M_r/M_u$ =	1.30	1.17	1.25	0.52	3.93	1.28	1.24	1.57	3.93	1.28	1.28	1.17	1.25	0.52	3.93	1.4%
	$\phi V_r/V_{mpr}$ =	1.74					1.54				1.54	1.54				1.74	
Ground	$P/A_g f'_c$ =	0.11					0.19				0.19	0.19				0.11	1.4%

Page 79 of 240

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 1024

Number of Stories: 8

Fundamental Period (sec): 1.71

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

First floor beams initially reinforced for flexure, then above beams sized to meet 85% weak story requirement. Columns reinforced to meet SCWB provisions while not overstrengthening the first story columns. Column strengths increased to 6.0 ksi in upper stories and 8.0 ksi in lower stories to meet joint shear requirements. Beam strength increased to 6.0 ksi to facilitate joint concrete placement by contractor.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

Floor 9	22	22	0.0035	0.0055	0.0037	4.5	22	22	0.0035	0.0055	0.0037	4.5	22	22	0.0035	0.0055	0.0037	4.5		
Story 8	h (in) = 22	22	0.0180	0.0084	4.0	22	22	0.0215	0.0084	4.0	22	22	0.0215	0.0084	4.0	22	22	0.0180	0.0084	4.0
Floor 8	22	22	0.0045	0.0090	0.0037	4.5	22	22	0.0045	0.0090	0.0037	4.5	22	22	0.0045	0.0090	0.0037	4.5		
Story 7	h (in) = 22	22	0.0180	0.0084	4.0	22	22	0.0215	0.0084	4.0	22	22	0.0215	0.0084	4.0	22	22	0.0180	0.0084	4.0
Floor 7	22	22	0.0051	0.0103	0.0037	4.5	22	22	0.0051	0.0103	0.0037	4.5	22	22	0.0051	0.0103	0.0037	4.5		
Story 6	h (in) = 22	22	0.0180	0.0084	4.0	22	22	0.0215	0.0084	4.0	22	22	0.0215	0.0084	4.0	22	22	0.0180	0.0084	4.0
Floor 6	22	22	0.0059	0.0118	0.0037	4.5	22	22	0.0059	0.0118	0.0037	4.5	22	22	0.0059	0.0118	0.0037	4.5		
Story 5	h (in) = 22	22	0.0180	0.0084	4.0	22	22	0.0215	0.0084	4.0	22	22	0.0215	0.0084	4.0	22	22	0.0180	0.0084	4.0
Floor 5	22	22	0.0063	0.0125	0.0037	4.5	22	22	0.0063	0.0125	0.0037	4.5	22	22	0.0063	0.0125	0.0037	4.5		
Story 4	h (in) = 22	22	0.0180	0.0112	4.0	22	22	0.0215	0.0112	4.0	22	22	0.0215	0.0112	4.0	22	22	0.0180	0.0112	4.0
Floor 4	22	22	0.0065	0.0130	0.0037	4.5	22	22	0.0065	0.0130	0.0037	4.5	22	22	0.0065	0.0130	0.0037	4.5		
Story 3	h (in) = 22	22	0.0200	0.0112	4.0	22	22	0.0250	0.0112	4.0	22	22	0.0250	0.0112	4.0	22	22	0.0200	0.0112	4.0
Floor 3	22	22	0.0066	0.0133	0.0037	4.5	22	22	0.0066	0.0133	0.0037	4.5	22	22	0.0066	0.0133	0.0037	4.5		
Story 2	h (in) = 22	22	0.0150	0.0112	4.0	22	22	0.0150	0.0112	4.0	22	22	0.0150	0.0112	4.0	22	22	0.0150	0.0112	4.0
Floor 2	22	22	0.0050	0.0100	0.0037	4.5	22	22	0.0050	0.0100	0.0037	4.5	22	22	0.0050	0.0100	0.0037	4.5		
Story 1	h (in) = 22	22	0.0120	0.0112	4.0	22	22	0.0120	0.0112	4.0	22	22	0.0120	0.0112	4.0	22	22	0.0120	0.0112	4.0
Grade beam column height (in) = 24						Basement column height (in) = 22														
20 feet																				
13 feet																				
15 feet																				
Design base shear = 0.050 g, 94 k																				
f'c beams = 6.0 ksi										f'c cols, upper = 6.0 ksi										
fy rebar, nom. = 60 ksi										f'c cols, lower = 8.0 ksi										

Design Documentation (2 of 2)																		
	SCWB =	0.86	1.37	1.07	0.64	7.26	0.80	1.24	1.27	0.64	7.26	0.80	1.37	1.07	0.64	7.26	0.86	Design Drifts:
Joint	$\Phi V_r/V_u =$	2.81					2.29					2.29					2.81	
Story 8	$\Phi M_r/M_u =$	2.25					5.95					5.96					2.25	0.6%
	$\Phi V_r/V_{mpr} =$	1.94					1.54					1.54					1.94	
	$P/A_g f_c =$	0.02					0.04					0.04					0.02	
Floor 8		1.35	1.41	1.51	0.51	3.34	1.28	1.52	1.59	0.51	3.34	1.28	1.41	1.51	0.51	3.34	1.35	
		1.47					1.31					1.31					1.47	
Story 7	$\Phi M_r/M_u =$	2.63					4.48					4.48					2.64	0.9%
	$\Phi V_r/V_{mpr} =$	1.8					1.92					1.92					1.8	
	$P/A_g f_c =$	0.04					0.07					0.07					0.04	
Floor 7		1.30	1.39	1.57	0.51	2.73	1.25	1.48	1.72	0.51	2.73	1.25	1.39	1.57	0.51	2.73	1.30	
		1.29					1.15					1.15					1.29	
Story 6	$\Phi M_r/M_u =$	2.29					3.46					3.46					2.29	1.2%
	$\Phi V_r/V_{mpr} =$	2.26					1.77					1.77					2.26	
	$P/A_g f_c =$	0.06					0.11					0.11					0.06	
Floor 6		1.23	1.42	1.68	0.51	2.24	1.21	1.52	1.87	0.51	2.24	1.21	1.42	1.68	0.51	2.24	1.23	
		1.10					0.98					0.98					1.10	
Story 5	$\Phi M_r/M_u =$	2.12					3.02					3.02					2.12	1.5%
	$\Phi V_r/V_{mpr} =$	2.51					1.94					1.94					2.51	
	$P/A_g f_c =$	0.08					0.15					0.15					0.08	
Floor 5		1.22	1.40	1.63	0.51	2.06	1.23	1.49	1.85	0.51	2.06	1.23	1.40	1.63	0.51	2.06	1.22	
		1.18					1.05					1.05					1.18	
Story 4	$\Phi M_r/M_u =$	2.04					2.84					2.84					2.04	1.7%
	$\Phi V_r/V_{mpr} =$	3.45					2.62					2.62					3.45	
	$P/A_g f_c =$	0.08					0.14					0.14					0.08	
Floor 4		1.27	1.39	1.58	0.51	1.95	1.33	1.48	1.85	0.51	1.95	1.33	1.39	1.58	0.51	1.95	1.27	
		1.14					1.01					1.01					1.14	
Story 3	$\Phi M_r/M_u =$	2.16					3.09					3.10					2.16	1.8%
	$\Phi V_r/V_{mpr} =$	3.07					2.29					2.29					3.07	
	$P/A_g f_c =$	0.09					0.17					0.17					0.09	
Floor 3		1.21	1.40	1.55	0.51	1.90	1.25	1.46	1.84	0.51	1.90	1.25	1.40	1.55	0.51	1.90	1.21	
		1.12					0.99					0.99					1.12	
Story 2	$\Phi M_r/M_u =$	1.69					2.13					2.13					1.69	1.8%
	$\Phi V_r/V_{mpr} =$	3.44					2.74					2.74					3.44	
	$P/A_g f_c =$	0.11					0.20					0.20					0.11	
Floor 2		1.23	1.11	1.12	0.51	2.83	1.32	1.18	1.50	0.51	2.83	1.32	1.11	1.12	0.51	2.83	1.23	
		1.48					1.31					1.31					1.48	
Story 1	$\Phi M_r/M_u =$	1.00					1.93					1.93					1.00	1.4%
	$\Phi V_r/V_{mpr} =$	3.63					2.79					2.79					3.63	
	$P/A_g f_c =$	0.13					0.23					0.23					0.13	

Modeling Documentation (1 of 1)												
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	4889	1828	-5532	0.35	7.863E+07	0.0406	-0.068	0.100	100	
		$E I_{stf}/E I_g$	0.35									
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	5213	2329	-7205	0.35	7.863E+07	0.042	-0.071	0.100	100	
		$E I_{stf}/E I_g$	0.35									
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	5533	2641	-7799	0.35	7.863E+07	0.043	-0.071	0.100	100	
		$E I_{stf}/E I_g$	0.35									
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	5849	3200	-8864	0.35	7.863E+07	0.044	-0.072	0.100	100	
		$E I_{stf}/E I_g$	0.35									
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	6204	3324	-9101	0.35	7.863E+07	0.045	-0.072	0.100	100	
		$E I_{stf}/E I_g$	0.35									
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	6997	3385	-9219	0.35	7.863E+07	0.045	-0.073	0.100	100	
		$E I_{stf}/E I_g$	0.35									
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	8110	3829	-10681	0.35	7.863E+07	0.043	-0.071	0.100	100	
		$E I_{stf}/E I_g$	0.35									
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	5702	2578	-7681	0.35	7.863E+07	0.043	-0.071	0.100	100	
		$E I_{stf}/E I_g$	0.35									
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60												
Model periods (sec): $T_1 = 1.71$ $T_2 = 0.56$ $T_3 = 0.32$												
f_y rebar expected = 67 ksi												

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2001

Number of Stories: 4

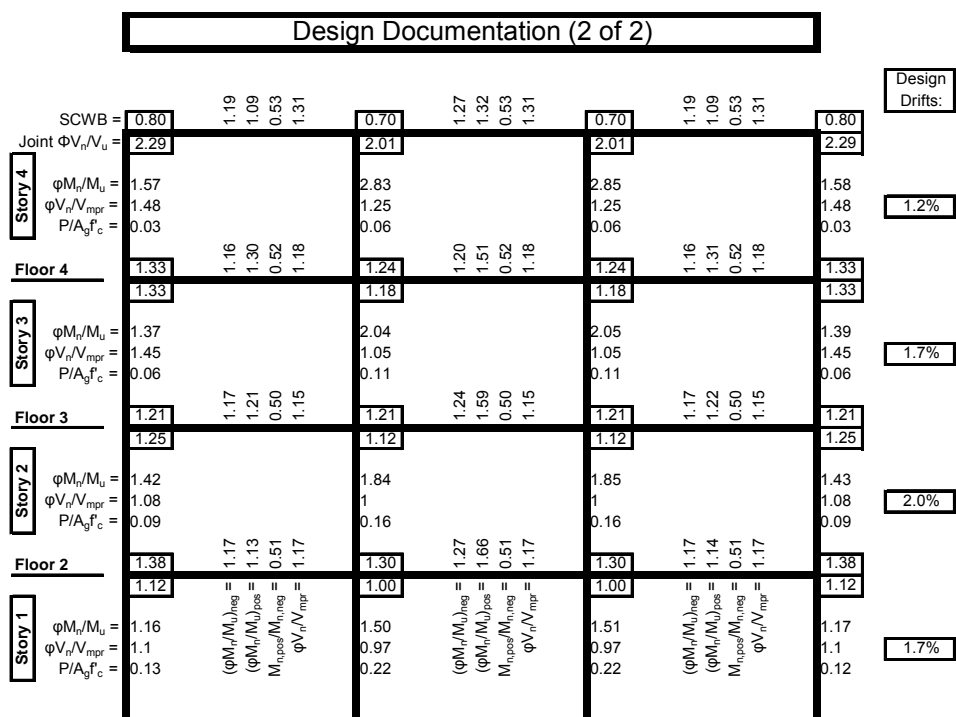
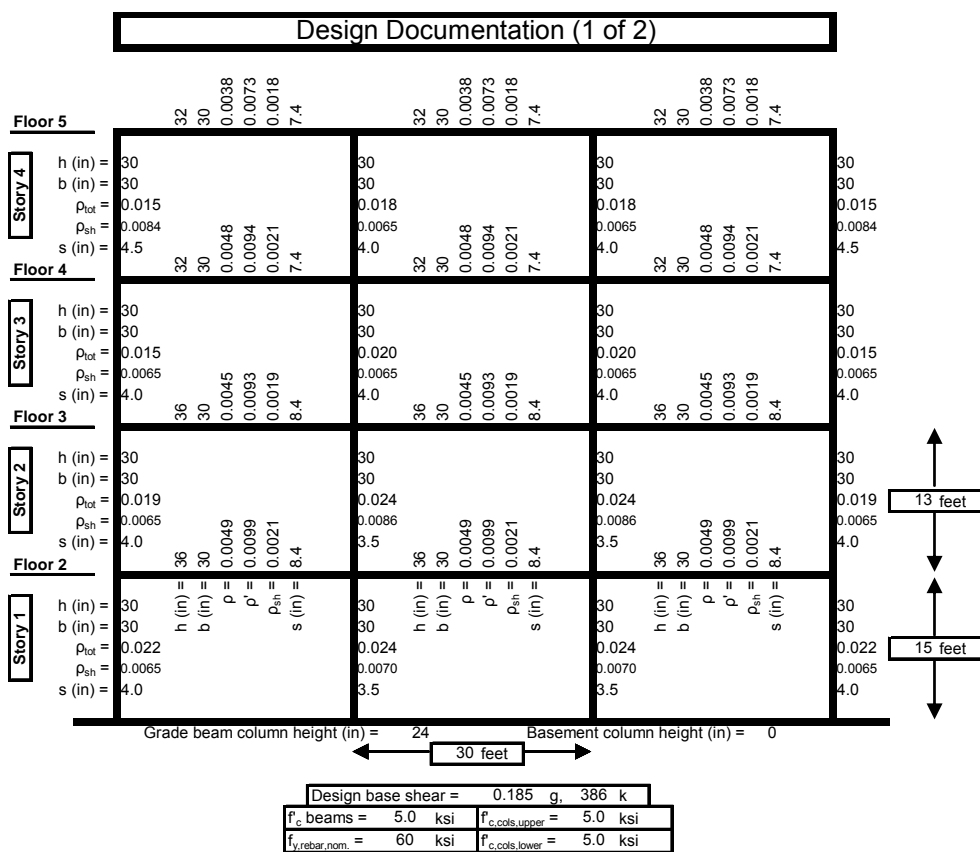
Fundamental Period (sec): 0.74

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2001 R=4

The beam depths were controlled by drift limits, caused by large lateral force demand due to the low R value. Both negative and positive flexural beam strength was controlled by strength demands, but then additional steel was added to 5 bays to meet the minimum reinforcement requirements (this was needed due to large beam sizes). Column flexural strengths were controlled by the strong-column weak-beam ratio, except 1 by strength demand. Beam stirrups were controlled by both minimum requirements and by the capacity shear design, more by capacity demand. Most of column stirrups were controlled by both minimum confinement requirements and only two columns had added stirrups for capacity shear demand. Joint shear did not control design because elements were already large due to drift demands.

DESIGN AND MODELING DOCUMENTATION FIGURES



Modeling Documentation (1 of 1)																				
Floor 5	Story 4											6186 -13642 0.35 3.106E+08 0.0277 -0.043 0.066 99			6186 -13642 0.35 3.106E+08 0.0277 -0.043 0.066 99			6186 -13642 0.35 3.106E+08 0.0277 -0.043 0.066 99		
		$M_{y,exp}$ (k-in) = 11449	E_{lsf}/E_l = 0.35	M_c/M_y = 1.21	$\Theta_{cap,pl}$ (rad) = 0.077	Θ_{pc} (rad) = 0.100	λ = 114	$(P/A_g f_c)_{exp}$ = 0.02	$M_{y,exp}$ (k-in) = 14458	E_{lsf}/E_l = 0.35	M_c/M_y = 1.20	$\Theta_{cap,pl}$ (rad) = 0.071	Θ_{pc} (rad) = 0.100	λ = 116	$(P/A_g f_c)_{exp}$ = 0.04	$M_{y,exp}$ (k-in) = 17731	E_{lsf}/E_l = 0.35	M_c/M_y = 1.20	$\Theta_{cap,pl}$ (rad) = 0.067	Θ_{pc} (rad) = 0.100
Floor 4	Story 3											7779 -16793 0.35 3.106E+08 0.033 -0.050 0.075 107			7779 -16793 0.35 3.106E+08 0.033 -0.050 0.075 107			7779 -16793 0.35 3.106E+08 0.033 -0.050 0.075 107		
		$M_{y,exp}$ (k-in) = 12452	E_{lsf}/E_l = 0.35	M_c/M_y = 1.20	$\Theta_{cap,pl}$ (rad) = 0.069	Θ_{pc} (rad) = 0.100	λ = 116	$(P/A_g f_c)_{exp}$ = 0.04	$M_{y,exp}$ (k-in) = 17731	E_{lsf}/E_l = 0.35	M_c/M_y = 1.20	$\Theta_{cap,pl}$ (rad) = 0.067	Θ_{pc} (rad) = 0.100	λ = 110	$(P/A_g f_c)_{exp}$ = 0.08	$M_{y,exp}$ (k-in) = 22271	E_{lsf}/E_l = 0.35	M_c/M_y = 1.19	$\Theta_{cap,pl}$ (rad) = 0.075	Θ_{pc} (rad) = 0.100
Floor 3	Story 2											9552 -21211 0.35 4.300E+08 0.029 -0.044 0.069 106			9552 -21211 0.35 4.300E+08 0.029 -0.044 0.069 106			9552 -21211 0.35 4.300E+08 0.029 -0.044 0.069 106		
		$M_{y,exp}$ (k-in) = 16103	E_{lsf}/E_l = 0.35	M_c/M_y = 1.20	$\Theta_{cap,pl}$ (rad) = 0.069	Θ_{pc} (rad) = 0.100	λ = 113	$(P/A_g f_c)_{exp}$ = 0.06	$M_{y,exp}$ (k-in) = 22271	E_{lsf}/E_l = 0.35	M_c/M_y = 1.19	$\Theta_{cap,pl}$ (rad) = 0.075	Θ_{pc} (rad) = 0.100	λ = 113	$(P/A_g f_c)_{exp}$ = 0.15	$M_{y,exp}$ (k-in) = 24113	E_{lsf}/E_l = 0.35	M_c/M_y = 1.19	$\Theta_{cap,pl}$ (rad) = 0.064	Θ_{pc} (rad) = 0.100
Floor 2	Story 1											10210 -22413 0.35 4.300E+08 0.033 -0.050 0.074 113			10210 -22413 0.35 4.300E+08 0.033 -0.050 0.074 113			10210 -22413 0.35 4.300E+08 0.033 -0.050 0.074 113		
		$M_{y,exp}$ (k-in) = 19226	E_{lsf}/E_l = 0.35	M_c/M_y = 1.20	$\Theta_{cap,pl}$ (rad) = 0.068	Θ_{pc} (rad) = 0.100	λ = 110	$(P/A_g f_c)_{exp}$ = 0.08	$M_{y,exp}$ (k-in) = 24113	E_{lsf}/E_l = 0.35	M_c/M_y = 1.19	$\Theta_{cap,pl}$ (rad) = 0.064	Θ_{pc} (rad) = 0.100	λ = 104	$(P/A_g f_c)_{exp}$ = 0.15	$M_{y,exp}$ (k-in) = 24113	E_{lsf}/E_l = 0.35	M_c/M_y = 1.19	$\Theta_{cap,pl}$ (rad) = 0.064	Θ_{pc} (rad) = 0.100
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.34																				
Model periods (sec): $T_1 = 0.74$ $T_2 = 0.24$ $T_3 = 0.13$																				
f_y rebar, expected = 67 ksi																				

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2003

Number of Stories: 4

Fundamental Period (sec): 0.97

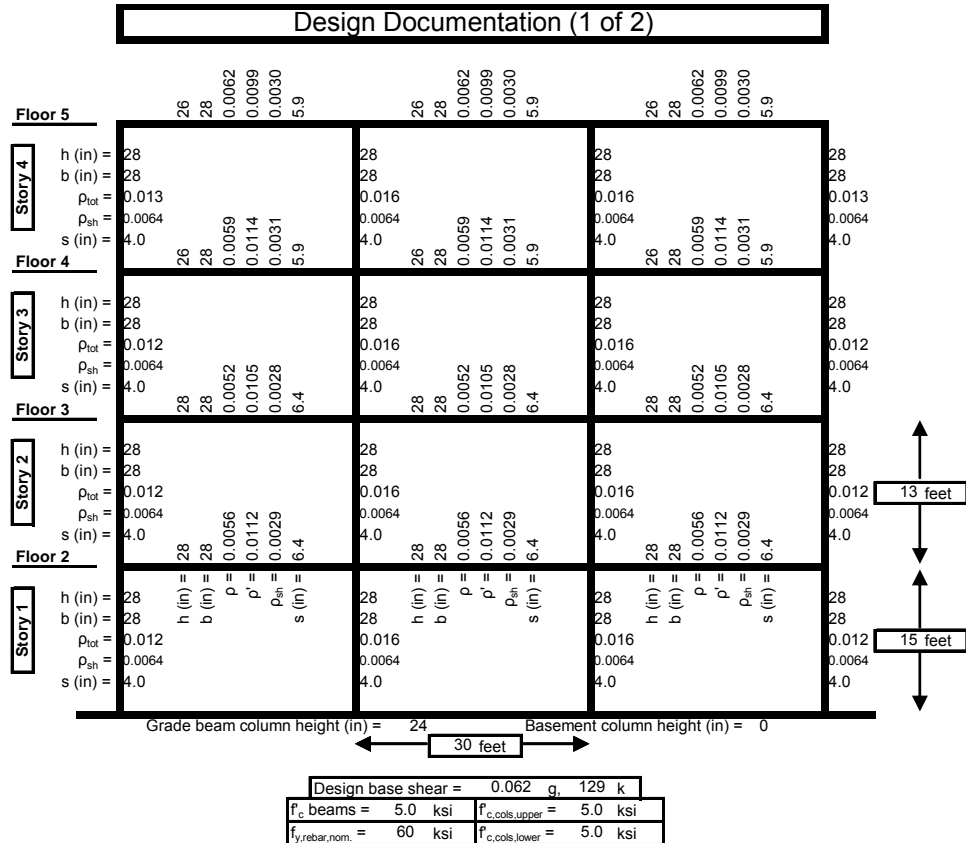
SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2003 R=12

The column size and beam depth were controlled by joint shear demand. Both negative and positive flexural beam strength was controlled by strength demands. More reinforcements were added to beams in four bays in order make consistent reinforcement for each floor. Two

story beams had additional reinforcement added to meet the minimum positive and negative ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 1 by strength demand. All beam stirrups were controlled by the capacity shear design. All of column stirrups were controlled by minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)																	
Story 4	SCWB =	0.74	1.14	1.13	0.64	1.16	0.61	1.20	1.23	0.64	1.16	0.61	1.15	1.13	0.64	1.16	0.74
	Joint $\Phi V_r/V_u$ =	1.67					1.37					1.37					1.67
	$\phi M_r/M_u$ =	1.16					3.75					3.76					1.17
	$\phi V_r/V_{mgr}$ =	1.39					1.5					1.5					1.39
Floor 4	$P/A_g f_c$ =	0.03					0.07					0.07					0.03
		1.31	1.14	1.13	0.53	1.15	1.23	1.16	1.16	0.53	1.15	1.23	1.14	1.13	0.53	1.15	1.32
		1.19					1.04					1.04					1.19
	$\phi M_r/M_u$ =	1.61					3.74					3.74					1.61
Story 3	$\phi V_r/V_{mgr}$ =	1.89					1.33					1.33					1.89
	$P/A_g f_c$ =	0.06					0.12					0.12					0.06
		1.23	1.15	1.17	0.51	1.17	1.27	1.18	1.22	0.51	1.17	1.27	1.15	1.17	0.51	1.17	1.23
		1.22					1.08					1.08					1.22
Floor 3	$\phi M_r/M_u$ =	1.27					3.38					3.38					1.28
	$\phi V_r/V_{mgr}$ =	1.67					1.16					1.16					1.67
	$P/A_g f_c$ =	0.09					0.19					0.19					0.09
		1.24	1.17	1.24	0.52	1.15	1.32	1.21	1.31	0.52	1.15	1.32	1.17	1.24	0.52	1.15	1.24
Story 2		1.11					0.98					0.98					1.11
	$\phi M_r/M_u$ =	1.65					2.81					2.82					1.66
	$\phi V_r/V_{mgr}$ =	1.82					1.26					1.26					1.82
	$P/A_g f_c$ =	0.13					0.25					0.25					0.13
Floor 2			$(\phi M_r/M_u)_{neg}$ =										$(\phi M_r/M_u)_{neg}$ =				
			$(\phi M_r/M_u)_{pos}$ =										$(\phi M_r/M_u)_{pos}$ =				
			$M_{u, pos}/M_{u, neg}$ =										$M_{u, pos}/M_{u, neg}$ =				
			$\phi V_r/V_{mgr}$ =										$\phi V_r/V_{mgr}$ =				
Story 1																	

Modeling Documentation (1 of 1)																						
Floor 5	Story 4	$M_{y, exp}$ (k-in) =	8361	5906	-10738	0.35	1.675E+08	0.0414	-0.059	0.100	100	10767	5906	-10738	0.35	1.675E+08	0.0414	-0.059	0.100	100	8361	
		E_{lsf}/E_lg =	0.35										0.35									0.35
		M_u/M_y =	1.21										1.20									1.21
		$\Theta_{cap, pl}$ (rad) =	0.070										0.068									0.070
Floor 4	Story 3	Θ_{pc} (rad) =	0.100									0.100									0.100	
		λ =	116	5610	-12064	0.35	1.675E+08	0.041	-0.062	0.100	100	112	5610	-12064	0.35	1.675E+08	0.041	-0.062	0.100	100	116	
		$(P/A_g f_c)_{exp}$ =	0.02										0.04									0.02
		$M_{y, exp}$ (k-in) =	8628										12572									8628
Floor 3	Story 2	E_{lsf}/E_lg =	0.35									0.35									0.35	
		M_u/M_y =	1.20									1.20									1.20	
		$\Theta_{cap, pl}$ (rad) =	0.066									0.063									0.066	
		Θ_{pc} (rad) =	0.100									0.100									0.100	
Floor 2	Story 1	λ =	112	5854	-13201	0.35	2.077E+08	0.037	-0.057	0.096	99	106	5854	-13201	0.35	2.077E+08	0.037	-0.057	0.096	99	112	
		$(P/A_g f_c)_{exp}$ =	0.04										0.09									0.04
		$M_{y, exp}$ (k-in) =	9548										14320									9548
		E_{lsf}/E_lg =	0.35										0.35									0.35
Floor 1	Story 1	M_u/M_y =	1.20									1.19									1.20	
		$\Theta_{cap, pl}$ (rad) =	0.061									0.058									0.061	
		Θ_{pc} (rad) =	0.100									0.100									0.100	
		λ =	106	6304	-13887	0.35	2.077E+08	0.038	-0.058	0.099	99	94	6304	-13887	0.35	2.077E+08	0.038	-0.058	0.099	99	106	
Floor 5	Story 4	$(P/A_g f_c)_{exp}$ =	0.09									0.18									0.09	
		$M_{y, pos, exp}$ (k-in) =	10451										16020								10451	
		$M_{y, neg, exp}$ (k-in) =	0.35									0.36									0.35	
		E_{lsf}/E_lg =	1.20									1.19									1.20	
Floor 4	Story 3	$\Theta_{cap, pl}$ (rad) =	0.061									0.054									0.061	
		Θ_{pc} (rad) =	0.100									0.100									0.100	
		λ =	106	6304	-13887	0.35	2.077E+08	0.038	-0.058	0.099	99	94	6304	-13887	0.35	2.077E+08	0.038	-0.058	0.099	99	106	
		$(P/A_g f_c)_{exp}$ =	0.09										0.18									0.09
Floor 3	Story 2	$M_{y, pos, exp}$ (k-in) =	10451										16020								10451	
		$M_{y, neg, exp}$ (k-in) =	0.35									0.36									0.35	
		E_{lsf}/E_lg =	1.20									1.19									1.20	
		$\Theta_{cap, pl}$ (rad) =	0.061										0.054									0.061
Floor 2	Story 1	Θ_{pc} (rad) =	0.100									0.100									0.100	
		λ =	106	6304	-13887	0.35	2.077E+08	0.038	-0.058	0.099	99	94	6304	-13887	0.35	2.077E+08	0.038	-0.058	0.099	99	106	
		$(P/A_g f_c)_{exp}$ =	0.09										0.18									0.09
		$M_{y, pos, exp}$ (k-in) =	10451										16020									10451
Floor 1	Story 1	$M_{y, neg, exp}$ (k-in) =	0.35									0.36									0.35	
		E_{lsf}/E_lg =	1.20									1.19									1.20	
		$\Theta_{cap, pl}$ (rad) =	0.061										0.054									0.061
		Θ_{pc} (rad) =	0.100									0.100									0.100	

Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.34											
Model periods (sec): T ₁ = 0.97 T ₂ = 0.30 T ₃ = 0.16											
f _{y, rebar, expected} = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2005

Number of Stories: 4

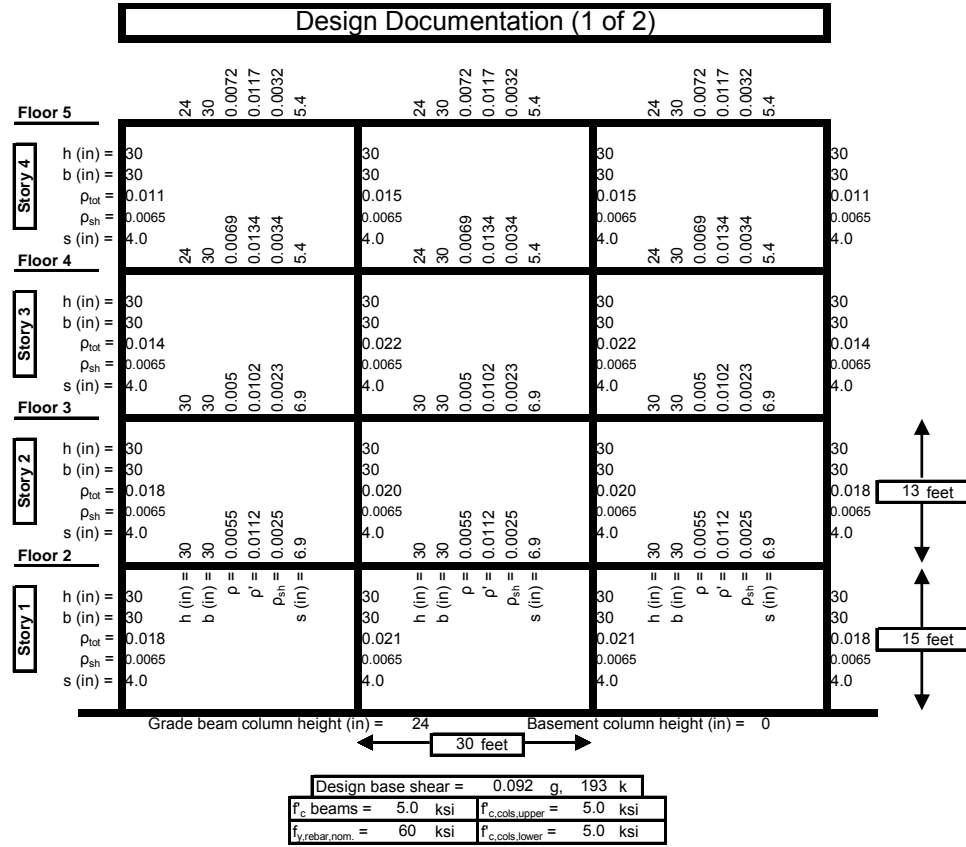
Fundamental Period (sec): 0.86

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2005 SCWB=1.5

Beam depths were controlled by joint shear. Both negative and positive flexural beam strength was controlled by strength demands. Four bays have reinforcement added to keep reinforcement constant for each level. Two stories had added reinforcement to meet the positive/negative strength ratio. All column flexural strengths were controlled by the strong-column weak-beam ratio. Beam stirrups were controlled by the capacity shear design, were as column stirrups were controlled by minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)									
Story 4	SCWB =	0.74	1.17	1.16	0.63	1.16	0.69	1.23	1.27
	Joint $\Phi V_r/V_u =$	1.57					1.30		
	$\Phi M_r/M_u =$	1.17					3.60		
	$\Phi V_r/V_{mgr} =$	1.6					1.53		
Floor 4	$P/A_g f'_c =$	0.03					0.06		
		1.53	1.16	1.17	0.53	1.16	1.63	1.18	1.21
		1.15					1.01		
Story 3	$\Phi M_r/M_u =$	1.72					4.07		
	$\Phi V_r/V_{mgr} =$	1.64					1.04		
	$P/A_g f'_c =$	0.05					0.11		
		1.58	1.17	1.24	0.50	1.17	1.57	1.21	1.43
Floor 3		1.27					1.14		
Story 2	$\Phi M_r/M_u =$	2.05					3.10		
	$\Phi V_r/V_{mgr} =$	1.2					0.98		
	$P/A_g f'_c =$	0.08					0.16		
		1.66	1.17	1.24	0.50	1.16	1.52	1.25	1.52
Floor 2		1.12					1.00		
Story 1	$\Phi M_r/M_u =$	2.58					3.67		
	$\Phi V_r/V_{mgr} =$	1.34					1.05		
	$P/A_g f'_c =$	0.11					0.22		

Design Drifts:

0.8%

1.0%

1.2%

1.4%

Modeling Documentation (1 of 1)									
Floor 5	$M_{y,exp}$ (k-in) =	3583	6062	-11077	1.354E+08	0.0452	-0.064	0.100	100
	$E I_{eff}/E I_g =$	0.35							
	$M_c/M_f =$	1.21							
	$\Theta_{cap,pl}$ (rad) =	0.069							
Floor 4	Θ_{pc} (rad) =	0.100							
	$\lambda =$	119							
	$(P/A_g f'_c)_{exp} =$	0.02							
		5856	-12453	0.35	1.354E+08	0.045	-0.068	0.100	100
Floor 3	$M_{y,exp}$ (k-in) =	11782	7024	-15861	2.591E+08	0.033	-0.051	0.082	99
	$E I_{eff}/E I_g =$	0.35							
	$M_c/M_f =$	1.20							
	$\Theta_{cap,pl}$ (rad) =	0.068							
Floor 2	Θ_{pc} (rad) =	0.100							
	$\lambda =$	116							
	$(P/A_g f'_c)_{exp} =$	0.04							
		7712	-17177	0.35	2.591E+08	0.034	-0.053	0.088	99
Floor 1	$M_{y,exp}$ (k-in) =	16401	22123	-17177	2.591E+08	0.034	-0.053	0.088	99
	$E I_{eff}/E I_g =$	0.35							
	$M_c/M_f =$	1.20							
	$\Theta_{cap,pl}$ (rad) =	0.066							
Floor 5	Θ_{pc} (rad) =	0.100							
	$\lambda =$	110							
	$(P/A_g f'_c)_{exp} =$	0.08							
		7712	-17177	0.35	2.591E+08	0.034	-0.053	0.088	99
Floor 4	$M_{y,exp}$ (k-in) =	11782	7024	-15861	2.591E+08	0.033	-0.051	0.082	99
	$E I_{eff}/E I_g =$	0.35							
	$M_c/M_f =$	1.20							
	$\Theta_{cap,pl}$ (rad) =	0.068							
Floor 3	Θ_{pc} (rad) =	0.100							
	$\lambda =$	116							
	$(P/A_g f'_c)_{exp} =$	0.04							
		7712	-17177	0.35	2.591E+08	0.034	-0.053	0.088	99
Floor 2	$M_{y,exp}$ (k-in) =	16401	22123	-17177	2.591E+08	0.034	-0.053	0.088	99
	$E I_{eff}/E I_g =$	0.35							
	$M_c/M_f =$	1.20							
	$\Theta_{cap,pl}$ (rad) =	0.066							
Floor 1	Θ_{pc} (rad) =	0.100							
	$\lambda =$	110							
	$(P/A_g f'_c)_{exp} =$	0.08							
		7712	-17177	0.35	2.591E+08	0.034	-0.053	0.088	99

Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.34

Model periods (sec): $T_1 = 0.86$ $T_2 = 0.27$ $T_3 = 0.15$

f_y rebar expected = 67 ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2006

Number of Stories: 4

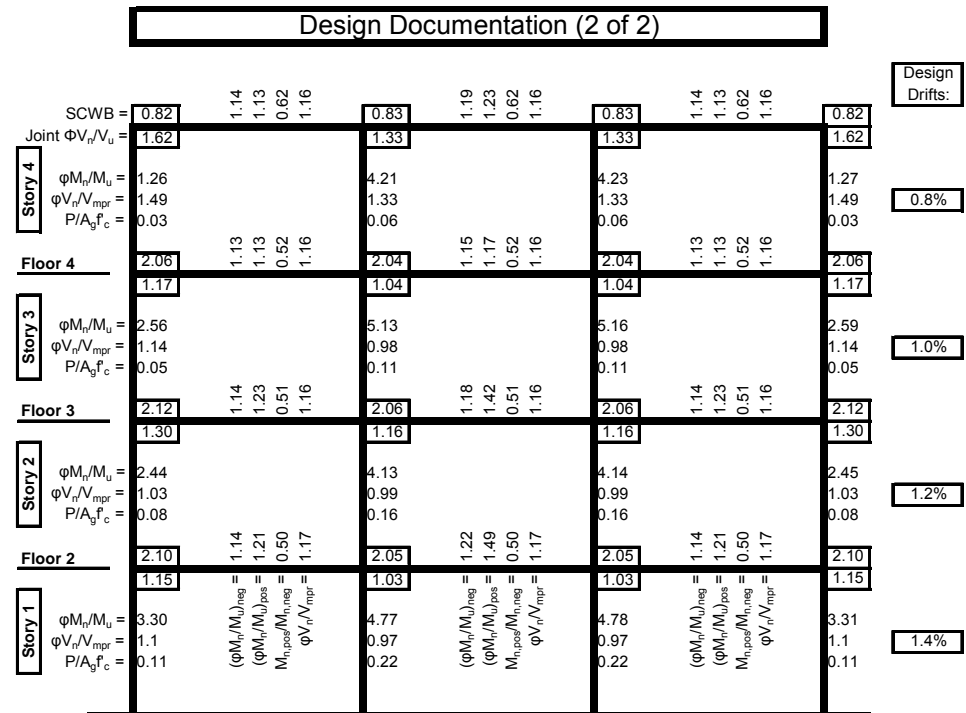
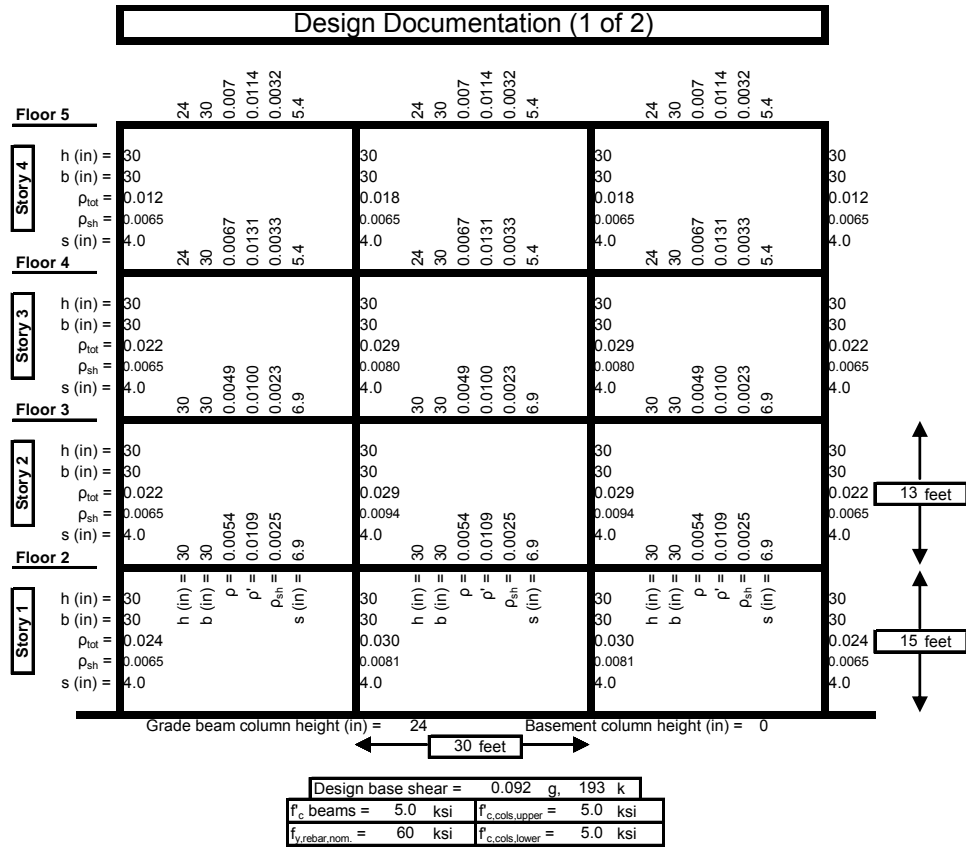
Fundamental Period (sec): 0.85

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2006 SCWB=2.0

Column sizes were controlled by joint shear as in baseline case. To satisfy SCWB adding more reinforcement was required. Both negative and positive flexural beam strength was controlled by strength demands. Four bays' reinforcements were added to alter beam design to use same ρ and ρ_{Prime} for each level. All column flexural strengths were controlled by the strong-column weak-beam ratio. Beam stirrups were controlled by capacity shear design. Column stirrups were controlled by both capacity shear design as well as minimum reinforcement requirement (half-half).

DESIGN AND MODELING DOCUMENTATION FIGURES



Modeling Documentation (1 of 1)											
Floor 5		Story 4		Story 3		Story 2		Story 1			
$M_{y,exp}$ (k-in) =	9259	$M_{y,exp}$ (k-in) =	14458	$M_{y,exp}$ (k-in) =	23716	$M_{y,exp}$ (k-in) =	25592	$M_{y,exp}$ (k-in) =	28099	$M_{y,exp}$ (k-in) =	20390
$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35
M_c/M_f =	1.21	M_c/M_f =	1.20	M_c/M_f =	1.20	M_c/M_f =	1.19	M_c/M_f =	1.19	M_c/M_f =	1.20
$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.071	$\Theta_{cap,pl}$ (rad) =	0.078	$\Theta_{cap,pl}$ (rad) =	0.078	$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.069
Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100
λ =	119	λ =	116	λ =	110	λ =	105	λ =	105	λ =	113
$(P/A_g f_c)_{exp}$ =	0.02	$(P/A_g f_c)_{exp}$ =	0.04	$(P/A_g f_c)_{exp}$ =	0.08	$(P/A_g f_c)_{exp}$ =	0.12	$(P/A_g f_c)_{exp}$ =	0.15	$(P/A_g f_c)_{exp}$ =	0.08
$M_{y,exp}$ (k-in) =	17130	$M_{y,exp}$ (k-in) =	23716	$M_{y,exp}$ (k-in) =	25592	$M_{y,exp}$ (k-in) =	28099	$M_{y,exp}$ (k-in) =	20390	$M_{y,exp}$ (k-in) =	9259
$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35
M_c/M_f =	1.20	M_c/M_f =	1.20	M_c/M_f =	1.20	M_c/M_f =	1.19	M_c/M_f =	1.19	M_c/M_f =	1.20
$\Theta_{cap,pl}$ (rad) =	0.073	$\Theta_{cap,pl}$ (rad) =	0.078	$\Theta_{cap,pl}$ (rad) =	0.078	$\Theta_{cap,pl}$ (rad) =	0.078	$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.069
Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100
λ =	116	λ =	110	λ =	105	λ =	105	λ =	113	λ =	119
$(P/A_g f_c)_{exp}$ =	0.04	$(P/A_g f_c)_{exp}$ =	0.08	$(P/A_g f_c)_{exp}$ =	0.12	$(P/A_g f_c)_{exp}$ =	0.15	$(P/A_g f_c)_{exp}$ =	0.08	$(P/A_g f_c)_{exp}$ =	0.02
$M_{y,exp}$ (k-in) =	18101	$M_{y,exp}$ (k-in) =	25592	$M_{y,exp}$ (k-in) =	28099	$M_{y,exp}$ (k-in) =	20390	$M_{y,exp}$ (k-in) =	9259	$M_{y,exp}$ (k-in) =	17130
$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35
M_c/M_f =	1.20	M_c/M_f =	1.19	M_c/M_f =	1.19	M_c/M_f =	1.20	M_c/M_f =	1.20	M_c/M_f =	1.21
$\Theta_{cap,pl}$ (rad) =	0.070	$\Theta_{cap,pl}$ (rad) =	0.078	$\Theta_{cap,pl}$ (rad) =	0.078	$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.069
Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100
λ =	113	λ =	105	λ =	105	λ =	113	λ =	116	λ =	110
$(P/A_g f_c)_{exp}$ =	0.06	$(P/A_g f_c)_{exp}$ =	0.12	$(P/A_g f_c)_{exp}$ =	0.15	$(P/A_g f_c)_{exp}$ =	0.08	$(P/A_g f_c)_{exp}$ =	0.04	$(P/A_g f_c)_{exp}$ =	0.04
$M_{y,exp}$ (k-in) =	20390	$M_{y,exp}$ (k-in) =	28099	$M_{y,exp}$ (k-in) =	20390	$M_{y,exp}$ (k-in) =	9259	$M_{y,exp}$ (k-in) =	17130	$M_{y,exp}$ (k-in) =	18101
$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35	$E I_{eff}/E I_g$ =	0.35
M_c/M_f =	1.20	M_c/M_f =	1.19	M_c/M_f =	1.19	M_c/M_f =	1.20	M_c/M_f =	1.20	M_c/M_f =	1.20
$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.069	$\Theta_{cap,pl}$ (rad) =	0.069
Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =	0.100
λ =	110	λ =	105	λ =	105	λ =	110	λ =	116	λ =	113
$(P/A_g f_c)_{exp}$ =	0.08	$(P/A_g f_c)_{exp}$ =	0.15	$(P/A_g f_c)_{exp}$ =	0.08	$(P/A_g f_c)_{exp}$ =	0.06	$(P/A_g f_c)_{exp}$ =	0.04	$(P/A_g f_c)_{exp}$ =	0.06
Mass tributary to one frame for lateral load (each floor) (k-s/in): 1.34											
Model periods (sec): $T_1 = 0.85$ $T_2 = 0.27$ $T_3 = 0.15$											
f_y rebar, expected = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2007

Number of Stories: 4

Fundamental Period (sec): 0.79

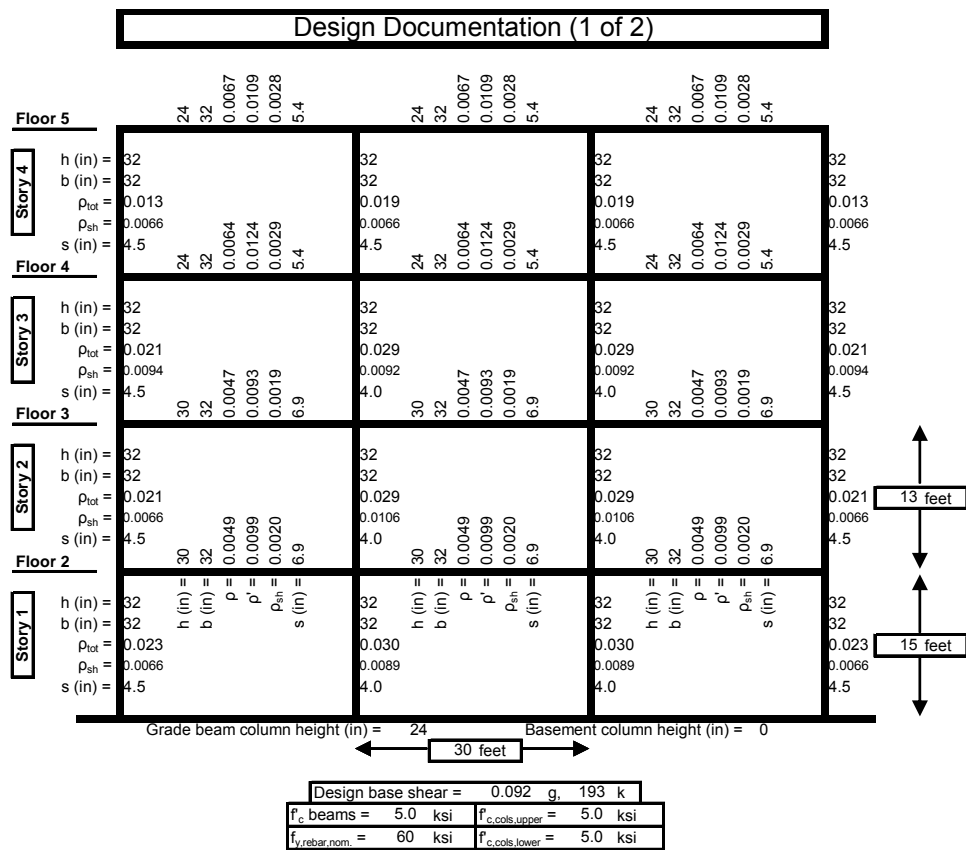
SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2007 SCWB=2.5

Compared to the baseline design, column sizes were increased to meet strong-column weak-beam ratio (>2.5). Both negative and positive flexural beam strength was controlled by

strength demands. Few additional beam reinforcements in two floors were added to alter beam design to use same rho and rhoPrime. Two floors had additional reinforcement added to meet the minimum positive/negative strength ratio. All column flexural strengths were controlled by the strong-column weak-beam ratio. All beam stirrups were controlled by the capacity shear design. Most column stirrups were controlled by the capacity shear design while 4 story columns were controlled by minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)									
Story 4	SCWB =	1.05	1.18	0.63	1.16	1.03	1.23	0.63	1.16
	Joint $\Phi V_r/V_u$ =	1.80				1.48			1.80
	$\Phi M_r/M_u$ =	1.60				5.16			5.18
	$\Phi V_r/V_{mgr}$ =	1.33				1.21			1.21
Floor 4	$P/A_g f'_c$ =	0.02				0.05			0.05
		2.49	1.17	1.17	0.53	1.17	1.18	1.20	0.53
		1.33				1.17			1.17
		2.50				2.50			2.50
Story 3	$\Phi M_r/M_u$ =	2.98				6.27			6.32
	$\Phi V_r/V_{mgr}$ =	1.15				1.02			1.02
	$P/A_g f'_c$ =	0.05				0.09			0.09
		2.50	1.14	1.27	0.52	1.17	1.18	1.20	0.52
Floor 3		1.50				1.33			1.33
		2.50				2.51			2.51
		1.50				1.33			1.33
		2.50				2.51			2.51
Story 2	$\Phi M_r/M_u$ =	2.88				5.05			5.07
	$\Phi V_r/V_{mgr}$ =	1.02				1.03			1.03
	$P/A_g f'_c$ =	0.07				0.14			0.14
		2.50	1.13	1.21	0.50	1.16	1.18	1.20	0.50
Floor 2		1.35				1.21			1.21
		2.50				2.51			2.51
		1.35				1.21			1.21
		2.50				2.51			2.51
Story 1	$\Phi M_r/M_u$ =	3.74				5.78			5.79
	$\Phi V_r/V_{mgr}$ =	1.09				1			1
	$P/A_g f'_c$ =	0.10				0.19			0.19
		2.50	1.13	1.21	0.50	1.16	1.18	1.20	0.50
Design Drifts:									
0.7%									
0.9%									
1.0%									
1.1%									

Modeling Documentation (1 of 1)									
Floor 5	$M_{y,exp}$ (k-in) =	12047	6074	-11028	0.35	1.387E+08	0.0423	-0.059	0.096
	E_{stf}/E_g =	0.35				18287			12047
	M_c/M_y =	1.21				1.21			1.21
	$\Theta_{cap,pl}$ (rad) =	0.068				0.070			0.068
Floor 4	Θ_{pc} (rad) =	0.100				0.100			0.100
	λ =	117				115			117
	$(P/A_g f'_c)_{exp}$ =	0.02	5792	-12290	0.35	1.387E+08	0.0423	-0.059	0.096
						0.03			100
Floor 3	$M_{y,exp}$ (k-in) =	19935	7065	-15406	0.35	2.654E+08	0.032	-0.050	0.072
	E_{stf}/E_g =	0.35				28781			19935
	M_c/M_y =	1.21				1.20			1.21
	$\Theta_{cap,pl}$ (rad) =	0.082				0.084			0.082
Floor 2	Θ_{pc} (rad) =	0.100				0.100			0.100
	λ =	115				114			115
	$(P/A_g f'_c)_{exp}$ =	0.03	7065	-15406	0.35	2.654E+08	0.032	-0.050	0.072
						0.07			99
Floor 1	$M_{y,exp}$ (k-in) =	23447	7299	-16318	0.35	32991			23447
	E_{stf}/E_g =	0.35				30596			23447
	M_c/M_y =	1.20				1.20			1.20
	$\Theta_{cap,pl}$ (rad) =	0.067				0.073			0.067
Story 1	Θ_{pc} (rad) =	0.100				0.100			0.100
	λ =	110				105			110
	$(P/A_g f'_c)_{exp}$ =	0.07	7299	-16318	0.35	32991			23447
						0.14			0.07
Mass tributary to one frame for lateral load (each floor) (k-s/in): 1.34									
Model periods (sec): $T_1 = 0.79$ $T_2 = 0.25$ $T_3 = 0.13$									
f_y rebar expected = 67 ksi									

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2008

Number of Stories: 12

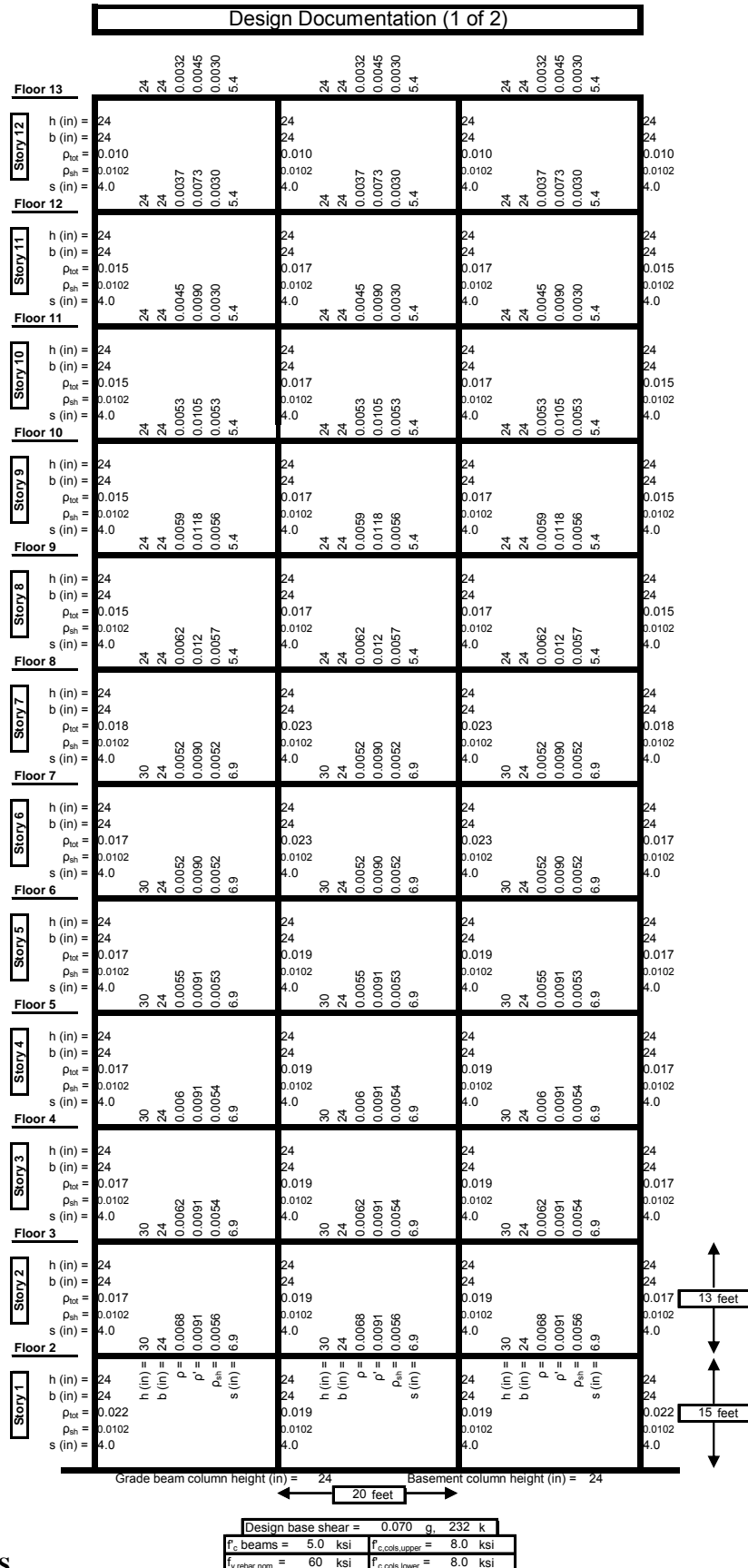
Fundamental Period (sec): 1.83

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2008 $R=4$

From the baseline design, beam and column sizes were made larger as the lateral demand is larger from smaller R value. Joint shear controls as using a concrete strength of 8ksi in column is not enough; thus beams larger. Above explains the reason for low drift. Positive bending strengths were controlled by primarily strength demand, 6 bays had min reinforcement imposed. Negative bending strengths were controlled by strength demands. Additional beam reinforcements were added to alter beam design to use same ρ and ρ_{Prime} in each floor and to meet the minimum positive/negative strength ratio. All column flexural strengths were controlled by the strong-column weak-beam ratio. Beam stirrups were controlled by both the capacity shear design and min reinforcement requirement. Column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION



FIGURES

Design Documentation (2 of 2)																	
	SCWB =	0.91				0.63				0.49				0.91			
	Joint $\Phi V_r/V_u$	4.12				3.23				3.23				4.12			
Story 12	$\Phi M_r/M_u$	1.38				2.03				2.03				1.38			
	$\Phi V_r/V_{npr}$	3.52				3.16				3.16				3.52			
	$P/A_g f_c$	0.01				0.02				0.02				0.01			
Floor 12		1.59				1.27				1.27				1.59			
		2.15				1.90				1.90				2.15			
Story 11	$\Phi M_r/M_u$	2.24				2.55				2.55				2.24			
	$\Phi V_r/V_{npr}$	2.28				1.88				1.88				2.28			
	$P/A_g f_c$	0.03				0.04				0.04				0.03			
Floor 11		1.63				1.38				1.38				1.63			
		1.73				1.54				1.54				1.73			
Story 10	$\Phi M_r/M_u$	1.85				1.97				1.97				1.85			
	$\Phi V_r/V_{npr}$	2.09				2.32				2.32				2.09			
	$P/A_g f_c$	0.04				0.07				0.07				0.04			
Floor 10		1.47				1.28				1.28				1.47			
		1.45				1.29				1.29				1.45			
Story 9	$\Phi M_r/M_u$	1.62				1.69				1.69				1.62			
	$\Phi V_r/V_{npr}$	3.09				2.53				2.53				3.09			
	$P/A_g f_c$	0.06				0.09				0.09				0.06			
Floor 9		1.37				1.23				1.23				1.37			
		1.27				1.13				1.13				1.27			
Story 8	$\Phi M_r/M_u$	1.42				1.55				1.55				1.42			
	$\Phi V_r/V_{npr}$	3.34				2.73				2.73				3.34			
	$P/A_g f_c$	0.07				0.11				0.11				0.07			
Floor 8		1.48				1.40				1.40				1.48			
		1.24				1.09				1.09				1.24			
Story 7	$\Phi M_r/M_u$	1.34				1.56				1.56				1.34			
	$\Phi V_r/V_{npr}$	2.83				2.2				2.2				2.83			
	$P/A_g f_c$	0.09				0.14				0.14				0.09			
Floor 7		1.24				1.24				1.24				1.24			
		1.37				1.15				1.15				1.37			
Story 6	$\Phi M_r/M_u$	1.36				1.69				1.69				1.36			
	$\Phi V_r/V_{npr}$	2.81				2.09				2.09				2.81			
	$P/A_g f_c$	0.11				0.16				0.16				0.11			
Floor 6		1.22				1.23				1.23				1.22			
		1.37				1.15				1.15				1.37			
Story 5	$\Phi M_r/M_u$	1.36				1.54				1.54				1.36			
	$\Phi V_r/V_{npr}$	2.65				2.2				2.2				2.66			
	$P/A_g f_c$	0.12				0.18				0.18				0.12			
Floor 5		1.22				1.19				1.19				1.22			
		1.36				1.13				1.13				1.36			
Story 4	$\Phi M_r/M_u$	1.33				1.58				1.58				1.33			
	$\Phi V_r/V_{npr}$	2.56				2.11				2.11				2.56			
	$P/A_g f_c$	0.14				0.21				0.21				0.14			
Floor 4		1.22				1.20				1.20				1.22			
		1.36				1.09				1.09				1.36			
Story 3	$\Phi M_r/M_u$	1.32				1.64				1.64				1.32			
	$\Phi V_r/V_{npr}$	2.46				2.03				2.03				2.46			
	$P/A_g f_c$	0.16				0.23				0.23				0.16			
Floor 3		1.22				1.23				1.23				1.22			
		1.36				1.07				1.07				1.36			
Story 2	$\Phi M_r/M_u$	1.35				1.68				1.68				1.35			
	$\Phi V_r/V_{npr}$	2.38				1.96				1.96				2.38			
	$P/A_g f_c$	0.18				0.26				0.26				0.18			
Floor 2		1.36				1.24				1.24				1.36			
		1.36				1.03				1.03				1.36			
Story 1	$\Phi M_r/M_u$	1.22				1.62				1.62				1.22			
	$\Phi V_r/V_{npr}$	2.06				1.57				1.57				2.06			
	$P/A_g f_c$	0.19				0.28				0.28				0.19			
	$(\Phi M_r/M_u)_{neg}$	1.13								$(\Phi M_r/M_u)_{neg}$				1.13			
	$(\Phi M_r/M_u)_{pos}$	1.13								$(\Phi M_r/M_u)_{pos}$				1.13			
	$M_{top}/M_{1,neg}$	0.76								$M_{top}/M_{1,neg}$				0.76			
	$\Phi V_r/V_{npr}$	1.16								$\Phi V_r/V_{npr}$				1.16			
	$(\Phi M_r/M_u)_{neg}$					$(\Phi M_r/M_u)_{neg}$								$(\Phi M_r/M_u)_{neg}$			
	$(\Phi M_r/M_u)_{pos}$					$(\Phi M_r/M_u)_{pos}$								$(\Phi M_r/M_u)_{pos}$			
	$M_{top}/M_{1,neg}$					$M_{top}/M_{1,neg}$								$M_{top}/M_{1,neg}$			
	$\Phi V_r/V_{npr}$					$\Phi V_r/V_{npr}$								$\Phi V_r/V_{npr}$			
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							
	$(\Phi M_r/M_u)_{neg}$									$(\Phi M_r/M_u)_{neg}$							
	$(\Phi M_r/M_u)_{pos}$									$(\Phi M_r/M_u)_{pos}$							
	$M_{top}/M_{1,neg}$									$M_{top}/M_{1,neg}$							
	$\Phi V_r/V_{npr}$									$\Phi V_r/V_{npr}$							

Mass tributary to one frame for lateral load (each floor) (k-s/in):	0.60
Model periods (sec): $T_1 = 1.83$ $T_2 = 0.62$ $T_3 = 0.36$	
$f_{y, \text{rebar, expected}} =$	67 ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2009

Number of Stories: 12

Fundamental Period (sec): 1.99

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2009 (R=8, SCWB=1.2, Drift Limit=0.02)

This twelve story baseline design is controlled both by joint shear and drift limits. Both positive and negative bending strength were controlled by strength demands. Few additional beam reinforcements in 12 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. Then 24 bays had additional reinforcement added to meet the minimum positive/negative strength ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 3 by strength demand. Beam stirrups were controlled by the capacity shear design and minimum requirement (half-half). All column stirrups were controlled by the minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)												
Floor 13												
Story 12	h (in) =	24	22	24	0.0035	0.0053	0.0034	4.9	22	24	0.0035	0.0053
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.010	0.010	0.010	0.0064	0.0034	0.0034	4.9	0.010	0.010	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	4.9	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	4.9	4.9	5.0	5.0	5.0	5.0	5.0
Floor 12												
Story 11	h (in) =	24	22	24	0.0039	0.0078	0.0034	4.9	22	24	0.0039	0.0078
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.012	0.012	0.012	0.0064	0.0034	0.0034	4.9	0.012	0.012	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	4.9	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	4.9	4.9	5.0	5.0	5.0	5.0	5.0
Floor 11												
Story 10	h (in) =	24	22	24	0.0045	0.0090	0.0034	4.9	22	24	0.0045	0.0090
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.010	0.010	0.010	0.0064	0.0034	0.0034	4.9	0.010	0.010	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	4.9	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	4.9	4.9	5.0	5.0	5.0	5.0	5.0
Floor 10												
Story 9	h (in) =	24	22	24	0.005	0.0100	0.0034	4.9	22	24	0.005	0.0100
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.010	0.010	0.010	0.0064	0.0034	0.0034	4.9	0.010	0.010	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	4.9	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	4.9	4.9	5.0	5.0	5.0	5.0	5.0
Floor 9												
Story 8	h (in) =	24	22	24	0.0054	0.0108	0.0034	4.9	22	24	0.0054	0.0108
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.015	0.015	0.015	0.0064	0.0034	0.0034	4.9	0.015	0.015	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	4.9	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	4.9	4.9	5.0	5.0	5.0	5.0	5.0
Floor 8												
Story 7	h (in) =	24	22	24	0.0055	0.011	0.0034	4.9	22	24	0.0055	0.011
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.012	0.012	0.012	0.0064	0.0034	0.0034	4.9	0.012	0.012	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	4.9	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	4.9	4.9	5.0	5.0	5.0	5.0	5.0
Floor 7												
Story 6	h (in) =	24	28	24	0.004	0.0078	0.0046	6.4	28	24	0.004	0.0078
	b (in) =	24	28	24	0.0064	0.0034	0.0034	6.4	28	24	0.0064	0.0034
	ρ_{tot} =	0.015	0.015	0.015	0.0064	0.0034	0.0034	6.4	0.015	0.015	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	6.4	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	6.4	6.4	5.0	5.0	5.0	5.0	5.0
Floor 6												
Story 5	h (in) =	24	28	24	0.004	0.0078	0.0046	6.4	28	24	0.004	0.0078
	b (in) =	24	28	24	0.0064	0.0034	0.0034	6.4	28	24	0.0064	0.0034
	ρ_{tot} =	0.016	0.016	0.016	0.0064	0.0034	0.0034	6.4	0.016	0.016	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	6.4	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	6.4	6.4	5.0	5.0	5.0	5.0	5.0
Floor 5												
Story 4	h (in) =	24	28	24	0.0037	0.0073	0.0045	6.4	28	24	0.0037	0.0073
	b (in) =	24	28	24	0.0064	0.0034	0.0034	6.4	28	24	0.0064	0.0034
	ρ_{tot} =	0.014	0.014	0.014	0.0064	0.0034	0.0034	6.4	0.014	0.014	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	6.4	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	6.4	6.4	5.0	5.0	5.0	5.0	5.0
Floor 4												
Story 3	h (in) =	24	28	24	0.004	0.0073	0.0045	6.4	28	24	0.004	0.0073
	b (in) =	24	28	24	0.0064	0.0034	0.0034	6.4	28	24	0.0064	0.0034
	ρ_{tot} =	0.015	0.015	0.015	0.0064	0.0034	0.0034	6.4	0.015	0.015	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	6.4	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	6.4	6.4	5.0	5.0	5.0	5.0	5.0
Floor 3												
Story 2	h (in) =	24	28	24	0.0043	0.0071	0.0046	6.4	28	24	0.0043	0.0071
	b (in) =	24	28	24	0.0064	0.0034	0.0034	6.4	28	24	0.0064	0.0034
	ρ_{tot} =	0.014	0.014	0.014	0.0064	0.0034	0.0034	6.4	0.014	0.014	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	6.4	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	6.4	6.4	5.0	5.0	5.0	5.0	5.0
Floor 2												
Story 1	h (in) =	24	28	24	0.0043	0.0071	0.0046	6.4	28	24	0.0043	0.0071
	b (in) =	24	28	24	0.0064	0.0034	0.0034	6.4	28	24	0.0064	0.0034
	ρ_{tot} =	0.017	0.017	0.017	0.0064	0.0034	0.0034	6.4	0.017	0.017	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0034	0.0034	6.4	0.0064	0.0064	0.0064	0.0034
	s (in) =	5.0	5.0	5.0	5.0	6.4	6.4	5.0	5.0	5.0	5.0	5.0
Basement												
Grade beam column height (in) = 24												
Basement column height (in) = 24												
20 feet												
13 feet												
15 feet												
Design base shear = 0.044 g, 232 k												
f'_c beams = 5.0 ksi f'_c cols upper = 5.0 ksi												
f'_c rebar nom. = 60 ksi f'_c cols lower = 5.0 ksi												

Design Documentation (2 of 2)											
SCWB = 0.96											
Joint $\Phi V_r/V_u = 2.93$											
Story 12	$\Phi M_r/M_u = 1.45$ $\Phi V_r/V_{mpr} = 2.44$ $P/A_g f'_c = 0.01$			2.32 2.55 0.00			2.35 2.58 0.00			1.31 2.58 0.00	
Floor 12	1.52 1.70			1.30 1.51			1.32 1.51			1.47 1.70	
Story 11	$\Phi M_r/M_u = 2.01$ $\Phi V_r/V_{mpr} = 1.99$ $P/A_g f'_c = 0.01$			3.53 1.41 0.00			3.56 1.41 0.00			2.04 2.08 0.00	
Floor 11	1.31 1.47			1.21 1.30			1.21 1.30			1.30 1.47	
Story 10	$\Phi M_r/M_u = 1.38$ $\Phi V_r/V_{mpr} = 2.41$ $P/A_g f'_c = 0.01$			1.65 2.24 0.00			1.63 2.3 0.00			1.33 2.6 0.00	
Floor 10	1.30 1.29			1.20 1.15			1.19 1.15			1.28 1.29	
Story 9	$\Phi M_r/M_u = 1.70$ $\Phi V_r/V_{mpr} = 2.03$ $P/A_g f'_c = 0.01$			2.42 1.46 0.00			2.44 1.47 0.00			1.71 2.15 0.00	
Floor 9	1.27 1.19			1.20 1.05			1.20 1.05			1.27 1.19	
Story 8	$\Phi M_r/M_u = 1.17$ $\Phi V_r/V_{mpr} = 3.03$ $P/A_g f'_c = 0.00$			1.39 2.49 0.00			1.38 2.55 0.00			1.16 3.24 0.01	
Floor 8	1.26 1.16			1.20 1.03			1.20 1.03			1.27 1.16	
Story 7	$\Phi M_r/M_u = 1.28$ $\Phi V_r/V_{mpr} = 2.33$ $P/A_g f'_c = 0.00$			1.68 1.65 0.00			1.69 1.64 0.00			1.30 2.29 0.01	
Floor 7	1.19 1.33			1.20 1.17			1.20 1.17			1.20 1.33	
Story 6	$\Phi M_r/M_u = 1.44$ $\Phi V_r/V_{mpr} = 2.19$ $P/A_g f'_c = 0.00$			1.84 1.53 0.00			1.82 1.55 0.00			1.43 2.21 0.01	
Floor 6	1.20 1.33			1.21 1.17			1.20 1.17			1.21 1.33	
Story 5	$\Phi M_r/M_u = 1.35$ $\Phi V_r/V_{mpr} = 2.35$ $P/A_g f'_c = 0.00$			1.69 1.63 0.00			1.70 1.64 0.00			1.37 2.21 0.01	
Floor 5	1.20 1.41			1.21 1.25			1.21 1.25			1.20 1.41	
Story 4	$\Phi M_r/M_u = 1.27$ $\Phi V_r/V_{mpr} = 2.43$ $P/A_g f'_c = 0.00$			1.59 1.72 0.00			1.59 1.71 0.00			1.26 2.48 0.00	
Floor 4	1.20 1.41			1.20 1.25			1.19 1.25			1.19 1.41	
Story 3	$\Phi M_r/M_u = 1.34$ $\Phi V_r/V_{mpr} = 2.31$ $P/A_g f'_c = 0.00$			1.63 1.65 0.00			1.62 1.67 0.00			1.34 2.31 0.00	
Floor 3	1.20 1.41			1.20 1.22			1.20 1.22			1.20 1.41	
Story 2	$\Phi M_r/M_u = 1.25$ $\Phi V_r/V_{mpr} = 2.51$ $P/A_g f'_c = 0.00$			1.61 1.63 0.00			1.61 1.64 0.00			1.24 2.46 0.00	
Floor 2	1.31 1.46			1.19 1.21			1.20 1.21			1.31 1.46	
Story 1	$\Phi M_r/M_u = 1.16$ $\Phi V_r/V_{mpr} = 2.06$ $P/A_g f'_c = 0.00$			1.43 1.65 0.00			1.44 1.64 0.00			1.16 2.09 0.00	
Design Drifts:											
0.7%											
1.1%											
1.4%											
1.7%											
1.9%											
1.8%											
1.6%											
1.6%											
1.6%											
1.7%											
1.5%											

Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60

Model periods (sec): $T_1 = 1.99$ $T_2 = 0.68$ $T_3 = 0.39$

$f_{y, \text{rebar, expected}} = 67$ ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2010

Number of Stories: 12

Fundamental Period (sec): 2.40

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2038 Perimeter Frame, $R=12$

Note: In order to see the effects of design strength changes the minimum base shear demand from eqn 9.5.5.2.1-3 was not imposed in this design.

From the baseline design, beam and column sizes were reduced as the lateral demand is reduced from larger R value. Both drift and joint shear controlled. Concrete strength of 7 ksi is used to satisfy joint shear like the baseline design and still have small section size to put drift towards allowable limit.

Both positive and negative beam bending strength were mainly controlled by strength demands. Additional beam reinforcements were added to alter beam design to use same ρ and ρ_{Prime} in each floor. Then 24 bays had additional reinforcement added to meet the minimum positive/negative strength ratio. Column strength governed by SCWB ratio except two were governed by flexural strength demand. Beam stirrups were controlled by both minimum requirement and capacity demand. Column stirrups were controlled by the minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)											
Floor 13											
Story 12	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 12											
Story 11	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 11											
Story 10	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 10											
Story 9	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 9											
Story 8	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 8											
Story 7	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 7											
Story 6	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 6											
Story 5	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 5											
Story 4	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 4											
Story 3	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 3											
Story 2	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Floor 2											
Story 1	h (in) =	20	20	20	20	20	20	20	20	20	20
	b (in) =	20	20	20	20	20	20	20	20	20	20
	ρ_{ext} =	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012	0.012
	ρ_{int} =	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123	0.0123
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Basement											
Grade beam column height (in) = 24											
Basement column height (in) = 20											
Design base shear = 0.023 g _s 232 k											
f _c beams = 5.0 ksi f _c cols upper = 8.0 ksi											
f _y rebar nom. = 60 ksi f _y cols lower = 8.0 ksi											

Page 108 of 240

Modeling Documentation (1 of 1)											
Floor 13	Story 12	Story 11	Story 10	Story 9	Story 8	Story 7	Story 6	Story 5	Story 4	Story 3	Story 2
$M_{y,exp}$ (k-in) = 3113 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.077 Θ_{pc} (rad) = 0.100 λ = 93 $(P/A_y f_c)_{exp}$ = 0.01	$M_{y,exp}$ (k-in) = 3413 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.075 Θ_{pc} (rad) = 0.100 λ = 91 $(P/A_y f_c)_{exp}$ = 0.02	$M_{y,exp}$ (k-in) = 3708 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.073 Θ_{pc} (rad) = 0.100 λ = 90 $(P/A_y f_c)_{exp}$ = 0.04	$M_{y,exp}$ (k-in) = 3999 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.072 Θ_{pc} (rad) = 0.100 λ = 89 $(P/A_y f_c)_{exp}$ = 0.05	$M_{y,exp}$ (k-in) = 4285 E_{eff}/E_g = 0.39 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.070 Θ_{pc} (rad) = 0.100 λ = 87 $(P/A_y f_c)_{exp}$ = 0.06	$M_{y,exp}$ (k-in) = 4569 E_{eff}/E_g = 0.40 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.069 Θ_{pc} (rad) = 0.100 λ = 86 $(P/A_y f_c)_{exp}$ = 0.07	$M_{y,exp}$ (k-in) = 4849 E_{eff}/E_g = 0.41 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.067 Θ_{pc} (rad) = 0.100 λ = 85 $(P/A_y f_c)_{exp}$ = 0.08	$M_{y,exp}$ (k-in) = 5128 E_{eff}/E_g = 0.43 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.066 Θ_{pc} (rad) = 0.100 λ = 83 $(P/A_y f_c)_{exp}$ = 0.10	$M_{y,exp}$ (k-in) = 5400 E_{eff}/E_g = 0.44 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.064 Θ_{pc} (rad) = 0.100 λ = 82 $(P/A_y f_c)_{exp}$ = 0.11	$M_{y,exp}$ (k-in) = 5154 E_{eff}/E_g = 0.45 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.061 Θ_{pc} (rad) = 0.100 λ = 81 $(P/A_y f_c)_{exp}$ = 0.12	$M_{y,exp}$ (k-in) = 5424 E_{eff}/E_g = 0.46 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.060 Θ_{pc} (rad) = 0.100 λ = 79 $(P/A_y f_c)_{exp}$ = 0.13	$M_{y,exp}$ (k-in) = 5691 E_{eff}/E_g = 0.47 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.059 Θ_{pc} (rad) = 0.100 λ = 78 $(P/A_y f_c)_{exp}$ = 0.15
$M_{y,exp}$ (k-in) = 4297 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.078 Θ_{pc} (rad) = 0.100 λ = 91 $(P/A_y f_c)_{exp}$ = 0.02	$M_{y,exp}$ (k-in) = 4874 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.075 Θ_{pc} (rad) = 0.100 λ = 89 $(P/A_y f_c)_{exp}$ = 0.05	$M_{y,exp}$ (k-in) = 5437 E_{eff}/E_g = 0.36 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.072 Θ_{pc} (rad) = 0.100 λ = 86 $(P/A_y f_c)_{exp}$ = 0.07	$M_{y,exp}$ (k-in) = 5989 E_{eff}/E_g = 0.38 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.068 Θ_{pc} (rad) = 0.100 λ = 83 $(P/A_y f_c)_{exp}$ = 0.10	$M_{y,exp}$ (k-in) = 6531 E_{eff}/E_g = 0.45 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.066 Θ_{pc} (rad) = 0.100 λ = 81 $(P/A_y f_c)_{exp}$ = 0.12	$M_{y,exp}$ (k-in) = 7063 E_{eff}/E_g = 0.47 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.063 Θ_{pc} (rad) = 0.100 λ = 78 $(P/A_y f_c)_{exp}$ = 0.15	$M_{y,exp}$ (k-in) = 7587 E_{eff}/E_g = 0.50 M_u/M_y = 1.16 $\Theta_{exp,pl}$ (rad) = 0.060 Θ_{pc} (rad) = 0.100 λ = 76 $(P/A_y f_c)_{exp}$ = 0.17	$M_{y,exp}$ (k-in) = 7076 E_{eff}/E_g = 0.52 M_u/M_y = 1.16 $\Theta_{exp,pl}$ (rad) = 0.055 Θ_{pc} (rad) = 0.100 λ = 73 $(P/A_y f_c)_{exp}$ = 0.19	$M_{y,exp}$ (k-in) = 7586 E_{eff}/E_g = 0.54 M_u/M_y = 1.16 $\Theta_{exp,pl}$ (rad) = 0.052 Θ_{pc} (rad) = 0.100 λ = 71 $(P/A_y f_c)_{exp}$ = 0.22	$M_{y,exp}$ (k-in) = 7746 E_{eff}/E_g = 0.57 M_u/M_y = 1.15 $\Theta_{exp,pl}$ (rad) = 0.049 Θ_{pc} (rad) = 0.100 λ = 69 $(P/A_y f_c)_{exp}$ = 0.24	$M_{y,exp}$ (k-in) = 10478 E_{eff}/E_g = 0.59 M_u/M_y = 1.15 $\Theta_{exp,pl}$ (rad) = 0.047 Θ_{pc} (rad) = 0.100 λ = 67 $(P/A_y f_c)_{exp}$ = 0.27	$M_{y,exp}$ (k-in) = 10815 E_{eff}/E_g = 0.62 M_u/M_y = 1.15 $\Theta_{exp,pl}$ (rad) = 0.045 Θ_{pc} (rad) = 0.100 λ = 65 $(P/A_y f_c)_{exp}$ = 0.29
$M_{y,exp}$ (k-in) = 3113 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.077 Θ_{pc} (rad) = 0.100 λ = 93 $(P/A_y f_c)_{exp}$ = 0.01	$M_{y,exp}$ (k-in) = 3413 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.075 Θ_{pc} (rad) = 0.100 λ = 91 $(P/A_y f_c)_{exp}$ = 0.02	$M_{y,exp}$ (k-in) = 3708 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.073 Θ_{pc} (rad) = 0.100 λ = 90 $(P/A_y f_c)_{exp}$ = 0.04	$M_{y,exp}$ (k-in) = 3999 E_{eff}/E_g = 0.35 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.072 Θ_{pc} (rad) = 0.100 λ = 89 $(P/A_y f_c)_{exp}$ = 0.05	$M_{y,exp}$ (k-in) = 4285 E_{eff}/E_g = 0.39 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.070 Θ_{pc} (rad) = 0.100 λ = 87 $(P/A_y f_c)_{exp}$ = 0.06	$M_{y,exp}$ (k-in) = 4569 E_{eff}/E_g = 0.40 M_u/M_y = 1.18 $\Theta_{exp,pl}$ (rad) = 0.069 Θ_{pc} (rad) = 0.100 λ = 86 $(P/A_y f_c)_{exp}$ = 0.07	$M_{y,exp}$ (k-in) = 4849 E_{eff}/E_g = 0.41 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.067 Θ_{pc} (rad) = 0.100 λ = 85 $(P/A_y f_c)_{exp}$ = 0.08	$M_{y,exp}$ (k-in) = 5128 E_{eff}/E_g = 0.43 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.066 Θ_{pc} (rad) = 0.100 λ = 83 $(P/A_y f_c)_{exp}$ = 0.10	$M_{y,exp}$ (k-in) = 5400 E_{eff}/E_g = 0.44 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.064 Θ_{pc} (rad) = 0.100 λ = 82 $(P/A_y f_c)_{exp}$ = 0.11	$M_{y,exp}$ (k-in) = 5154 E_{eff}/E_g = 0.45 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.061 Θ_{pc} (rad) = 0.100 λ = 81 $(P/A_y f_c)_{exp}$ = 0.12	$M_{y,exp}$ (k-in) = 5424 E_{eff}/E_g = 0.46 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.060 Θ_{pc} (rad) = 0.100 λ = 79 $(P/A_y f_c)_{exp}$ = 0.13	$M_{y,exp}$ (k-in) = 5691 E_{eff}/E_g = 0.47 M_u/M_y = 1.17 $\Theta_{exp,pl}$ (rad) = 0.059 Θ_{pc} (rad) = 0.100 λ = 78 $(P/A_y f_c)_{exp}$ = 0.15
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60											
Model periods (sec): T_1 = 2.40 T_2 = 0.81 T_3 = 0.48											
$f_{v,brbr,exp}$ = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2012

Number of Stories: 12

Fundamental Period (sec): 1.99

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2012 SCWB=1.5

Compared to the baseline 12 story, this build simply have more reinforcement in the columns. Both positive and negative bending strength were controlled by strength demands. Few additional beam reinforcements in 12 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. Then 24 bays had additional reinforcement added to meet the minimum positive/negative strength ratio. All column flexural strengths were controlled by the strong-column weak-beam ratio. Beam stirrups were controlled by the capacity shear design and minimum requirement (half-half). All column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

Floor 13																
Story 12	h (in) =	24	22	24	0.0032	24	22	24	0.0032	24	22	24				
	b (in) =	24	22	24	0.0050	24	22	24	0.0050	24	22	24				
	ρ_{tot} =	0.010	0.0064	0.0039	0.0075	0.0064	0.0039	0.0075	0.0064	0.0039	0.0075	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9				
Floor 12																
Story 11	h (in) =	24	22	24	0.0045	24	22	24	0.0045	24	22	24				
	b (in) =	24	22	24	0.0088	24	22	24	0.0088	24	22	24				
	ρ_{tot} =	0.012	0.0064	0.0045	0.0088	0.0064	0.0045	0.0088	0.0064	0.0045	0.0088	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9				
Floor 11																
Story 10	h (in) =	24	22	24	0.005	24	22	24	0.005	24	22	24				
	b (in) =	24	22	24	0.0098	24	22	24	0.0098	24	22	24				
	ρ_{tot} =	0.013	0.0064	0.005	0.0098	0.0064	0.005	0.0098	0.0064	0.005	0.0098	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9				
Floor 10																
Story 9	h (in) =	24	22	24	0.0054	24	22	24	0.0054	24	22	24				
	b (in) =	24	22	24	0.011	24	22	24	0.011	24	22	24				
	ρ_{tot} =	0.017	0.0064	0.0054	0.011	0.0064	0.0054	0.011	0.0064	0.0054	0.011	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9				
Floor 9																
Story 8	h (in) =	24	22	24	0.0057	24	22	24	0.0057	24	22	24				
	b (in) =	24	22	24	0.0113	24	22	24	0.0113	24	22	24				
	ρ_{tot} =	0.017	0.0064	0.0057	0.0113	0.0064	0.0057	0.0113	0.0064	0.0057	0.0113	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9				
Floor 8																
Story 7	h (in) =	24	22	24	0.0037	24	22	24	0.0037	24	22	24				
	b (in) =	24	22	24	0.0045	24	22	24	0.0045	24	22	24				
	ρ_{tot} =	0.020	0.0064	0.0037	0.0045	0.0064	0.0037	0.0045	0.0064	0.0037	0.0045	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4				
Floor 7																
Story 6	h (in) =	24	22	24	0.0037	24	22	24	0.0037	24	22	24				
	b (in) =	24	22	24	0.0045	24	22	24	0.0045	24	22	24				
	ρ_{tot} =	0.019	0.0064	0.0037	0.0045	0.0064	0.0037	0.0045	0.0064	0.0037	0.0045	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4				
Floor 6																
Story 5	h (in) =	24	22	24	0.0037	24	22	24	0.0037	24	22	24				
	b (in) =	24	22	24	0.0045	24	22	24	0.0045	24	22	24				
	ρ_{tot} =	0.019	0.0064	0.0037	0.0045	0.0064	0.0037	0.0045	0.0064	0.0037	0.0045	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4				
Floor 5																
Story 4	h (in) =	24	22	24	0.004	24	22	24	0.004	24	22	24				
	b (in) =	24	22	24	0.0076	24	22	24	0.0076	24	22	24				
	ρ_{tot} =	0.019	0.0064	0.004	0.0076	0.0064	0.004	0.0076	0.0064	0.004	0.0076	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4				
Floor 4																
Story 3	h (in) =	24	22	24	0.0042	24	22	24	0.0042	24	22	24				
	b (in) =	24	22	24	0.0076	24	22	24	0.0076	24	22	24				
	ρ_{tot} =	0.019	0.0064	0.0042	0.0076	0.0064	0.0042	0.0076	0.0064	0.0042	0.0076	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4				
Floor 3																
Story 2	h (in) =	24	22	24	0.0046	24	22	24	0.0046	24	22	24				
	b (in) =	24	22	24	0.0073	24	22	24	0.0073	24	22	24				
	ρ_{tot} =	0.019	0.0064	0.0046	0.0073	0.0064	0.0046	0.0073	0.0064	0.0046	0.0073	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4				
Floor 2																
Story 1	h (in) =	24	22	24	0.0047	24	22	24	0.0047	24	22	24				
	b (in) =	24	22	24	0.0073	24	22	24	0.0073	24	22	24				
	ρ_{tot} =	0.019	0.0064	0.0047	0.0073	0.0064	0.0047	0.0073	0.0064	0.0047	0.0073	0.0064				
	ρ_{sh} =	0.0064	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034	0.0034				
	s (in) =	5.0	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4	6.4				
Grade beam column height (in) = 24																
Basement column height (in) = 24																
20 feet																
Design base shear = 0.044 g, 232 k																
<table><tr><td>f'_c beams = 5.0 ksi</td><td>f'_c cols upper = 5.0 ksi</td></tr><tr><td>f'_c rebar nom. = 60 ksi</td><td>f'_c cols lower = 5.0 ksi</td></tr></table>													f'_c beams = 5.0 ksi	f'_c cols upper = 5.0 ksi	f'_c rebar nom. = 60 ksi	f'_c cols lower = 5.0 ksi
f'_c beams = 5.0 ksi	f'_c cols upper = 5.0 ksi															
f'_c rebar nom. = 60 ksi	f'_c cols lower = 5.0 ksi															
13 feet																
15 feet																

Page 112 of 240

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	3794	1839	4089	0.35	7.428E+07	0.0407	0.068	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 12	Story 11	$M_{y,exp}$ (k-in) =	4812	2197	5414	0.35	7.428E+07	0.041	0.070	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 11	Story 10	$M_{y,exp}$ (k-in) =	5493	2533	6071	0.35	7.428E+07	0.043	0.070	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 10	Story 9	$M_{y,exp}$ (k-in) =	7131	2805	6594	0.35	7.428E+07	0.043	0.071	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	7474	3046	7245	0.35	7.428E+07	0.044	0.072	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	8772	3195	7376	0.35	7.428E+07	0.045	0.071	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	8628	3639	8835	0.35	1.522E+08	0.041	0.068	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	9120	3639	8835	0.35	1.522E+08	0.041	0.068	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	9370	3639	8835	0.35	1.522E+08	0.041	0.068	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	9856	3856	8914	0.35	1.522E+08	0.042	0.068	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	9943	4113	8917	0.35	1.522E+08	0.043	0.067	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	10265	4438	8693	0.35	1.522E+08	0.044	0.066	0.100	100
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60											
Model periods (sec): $T_1 = 1.99$ $T_2 = 0.68$ $T_3 = 0.39$											
$f_{v,brbr,exp}$ = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2013

Number of Stories: 12

Fundamental Period (sec): 1.97

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2013 SCWB=2.0

Simply adding more reinforcements in the column was insufficient to obtain SCWB=2.0 from the baseline design. Thus, columns were made larger. However, after the columns were larger inter story drift and joint shear was too conservative, so beams were sized down, until joint shear controls. Both positive and negative bending strength were controlled by strength demands. Additional beam reinforcements in 12 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. Then 21 bays had additional reinforcement added to meet the minimum positive/negative strength ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 6 by strength demand. Beam stirrups were controlled by the capacity shear design and minimum requirement (half-half). All column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES

Page 115 of 240

Design Documentation (2 of 2)									
	SCWB =	1.28	1.16	1.22	5.77	1.19	1.23	1.34	0.66
	Joint $\Phi V_n/V_u$	2.94				2.38			0.99
Story 12	$\Phi M_r/M_u$	1.72				4.18			4.18
	$\Phi V_n/V_{npr}$	2.13				1.48			1.48
	$P/A_g f'_c$	0.02				0.03			0.03
Floor 12		2.18	1.17	1.39	3.61	2.11	1.33	1.47	3.61
		1.82				1.60			1.60
Story 11	$\Phi M_r/M_u$	2.63				4.18			4.18
	$\Phi V_n/V_{npr}$	1.67				1.72			1.72
	$P/A_g f'_c$	0.03				0.06			0.06
Floor 11		2.20	1.17	1.42	2.93	2.11	1.30	1.53	2.93
		1.59				1.40			1.40
Story 10	$\Phi M_r/M_u$	2.38				3.67			3.67
	$\Phi V_n/V_{npr}$	2.18				1.59			1.59
	$P/A_g f'_c$	0.05				0.09			0.09
Floor 10		2.06	1.17	1.44	2.50	2.03	1.30	1.53	2.50
		1.38				1.23			1.23
Story 9	$\Phi M_r/M_u$	2.36				3.32			3.32
	$\Phi V_n/V_{npr}$	2.4				1.74			1.74
	$P/A_g f'_c$	0.07				0.12			0.12
Floor 9		2.01	1.16	1.45	2.27	2.02	1.28	1.55	2.27
		1.28				1.13			1.13
Story 8	$\Phi M_r/M_u$	1.99				2.92			2.92
	$\Phi V_n/V_{npr}$	2.54				1.84			1.84
	$P/A_g f'_c$	0.09				0.16			0.16
Floor 8		2.60	1.16	1.44	2.26	2.44	1.26	1.55	2.26
		1.28				1.13			1.13
Story 7	$\Phi M_r/M_u$	2.38				2.74			2.75
	$\Phi V_n/V_{npr}$	1.73				1.41			1.41
	$P/A_g f'_c$	0.11				0.19			0.19
Floor 7		2.18	1.16	1.22	1.16	2.04	1.36	1.29	1.16
		1.40				1.25			1.25
Story 6	$\Phi M_r/M_u$	2.29				2.78			2.78
	$\Phi V_n/V_{npr}$	1.93				1.5			1.5
	$P/A_g f'_c$	0.13				0.22			0.22
Floor 6		2.07	1.17	1.28	1.16	2.04	1.34	1.34	1.16
		1.45				1.28			1.28
Story 5	$\Phi M_r/M_u$	2.33				2.91			2.91
	$\Phi V_n/V_{npr}$	1.85				1.44			1.44
	$P/A_g f'_c$	0.15				0.26			0.26
Floor 5		2.13	1.15	1.18	1.15	2.18	1.30	1.25	1.15
		1.48				1.32			1.32
Story 4	$\Phi M_r/M_u$	2.35				2.98			2.98
	$\Phi V_n/V_{npr}$	1.79				1.3			1.3
	$P/A_g f'_c$	0.18				0.29			0.29
Floor 4		2.16	1.15	1.19	1.15	2.22	1.29	1.30	1.15
		1.48				1.29			1.29
Story 3	$\Phi M_r/M_u$	2.38				3.07			3.08
	$\Phi V_n/V_{npr}$	1.73				1.32			1.32
	$P/A_g f'_c$	0.20				0.32			0.32
Floor 3		2.19	1.17	1.20	1.15	2.26	1.28	1.38	1.15
		1.48				1.26			1.26
Story 2	$\Phi M_r/M_u$	2.35				3.09			3.09
	$\Phi V_n/V_{npr}$	1.68				1.35			1.35
	$P/A_g f'_c$	0.22				0.36			0.36
Floor 2		2.34	1.16	1.18	1.16	2.41	1.25	1.55	1.16
		1.57				1.31			1.31
Story 1	$\Phi M_r/M_u$	1.84				2.86			2.86
	$\Phi V_n/V_{npr}$	1.63				1.37			1.37
	$P/A_g f'_c$	0.24				0.39			0.39
	$(\Phi M_r/M_u)_{neg}$								
	$(\Phi M_r/M_u)_{pos}$								
	$M_{top}/M_{n,neg}$								
	$\Phi V_n/V_{npr}$								
	$(\Phi M_r/M_u)_{neg}$								
	$(\Phi M_r/M_u)_{pos}$								
	$M_{top}/M_{n,neg}$								
	$\Phi V_n/V_{npr}$								
Design Drifts:									
		1.28				2.94			1.28
									0.8%
		2.18				2.11			2.18
									1.1%
		2.20				2.11			2.20
									1.5%
		2.06				2.03			2.06
									1.8%
		2.01				2.02			2.01
									2.0%
		2.60				2.44			2.60
									1.8%
		2.18				2.04			2.18
									1.5%
		2.07				2.04			2.07
									1.5%
		2.13				2.18			2.13
									1.6%
		2.16				2.22			2.16
									1.6%
		2.19				2.26			2.19
									1.6%
		2.34				2.41			2.34
									1.3%

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	4840	1917	4840	1917	4840	1917	4840	1917	4840
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21
		$\Theta_{cap,pl}$ (rad) =	0.064	0.065	0.065	0.065	0.065	0.065	0.065	0.064	0.064
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	108	107	107	107	107	107	107	108	108
Floor 12	Story 11	$(P/A)_g^f c_{l,exp}$ =	0.01	2151	0.02	2151	0.02	2151	0.02	2151	0.01
		$M_{y,exp}$ (k-in) =	6087	2519	6087	2519	6087	2519	6087	2519	6087
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.21	1.20	1.20	1.20	1.20	1.20	1.20	1.21	1.21
		$\Theta_{cap,pl}$ (rad) =	0.064	0.063	0.063	0.063	0.063	0.063	0.063	0.064	0.064
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 11	Story 10	$(P/A)_g^f c_{l,exp}$ =	0.02	2442	0.05	2442	0.05	2442	0.05	2442	0.02
		$M_{y,exp}$ (k-in) =	6477	2578	6477	2578	6477	2578	6477	2578	6477
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.21	1.20	1.20	1.20	1.20	1.20	1.20	1.21	1.21
		$\Theta_{cap,pl}$ (rad) =	0.062	0.061	0.061	0.061	0.061	0.061	0.061	0.062	0.062
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 10	Story 9	$(P/A)_g^f c_{l,exp}$ =	0.03	2676	0.07	2676	0.07	2676	0.07	2676	0.03
		$M_{y,exp}$ (k-in) =	6862	2885	6862	2885	6862	2885	6862	2885	6862
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20
		$\Theta_{cap,pl}$ (rad) =	0.061	0.058	0.058	0.058	0.058	0.058	0.058	0.061	0.061
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 9	Story 8	$(P/A)_g^f c_{l,exp}$ =	0.05	2885	0.09	2885	0.09	2885	0.09	2885	0.05
		$M_{y,exp}$ (k-in) =	7242	2897	7242	2897	7242	2897	7242	2897	7242
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.20	1.19	1.19	1.19	1.19	1.19	1.19	1.20	1.20
		$\Theta_{cap,pl}$ (rad) =	0.060	0.056	0.056	0.056	0.056	0.056	0.056	0.060	0.060
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 8	Story 7	$(P/A)_g^f c_{l,exp}$ =	0.06	2897	0.11	2897	0.11	2897	0.11	2897	0.06
		$M_{y,exp}$ (k-in) =	10957	3758	10957	3758	10957	3758	10957	3758	10957
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.20	1.19	1.19	1.19	1.19	1.19	1.19	1.20	1.20
		$\Theta_{cap,pl}$ (rad) =	0.063	0.057	0.057	0.057	0.057	0.057	0.057	0.063	0.063
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 7	Story 6	$(P/A)_g^f c_{l,exp}$ =	0.07	3758	0.14	3758	0.14	3758	0.14	3758	0.07
		$M_{y,exp}$ (k-in) =	9659	3759	9659	3759	9659	3759	9659	3759	9659
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.20	1.19	1.19	1.19	1.19	1.19	1.19	1.20	1.20
		$\Theta_{cap,pl}$ (rad) =	0.059	0.053	0.053	0.053	0.053	0.053	0.053	0.059	0.059
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 6	Story 5	$(P/A)_g^f c_{l,exp}$ =	0.08	3759	0.16	3759	0.16	3759	0.16	3759	0.08
		$M_{y,exp}$ (k-in) =	10025	3541	10025	3541	10025	3541	10025	3541	10025
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.20	1.18	1.18	1.18	1.18	1.18	1.18	1.20	1.20
		$\Theta_{cap,pl}$ (rad) =	0.058	0.051	0.051	0.051	0.051	0.051	0.051	0.058	0.058
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 5	Story 4	$(P/A)_g^f c_{l,exp}$ =	0.09	3541	0.18	3541	0.18	3541	0.18	3541	0.09
		$M_{y,exp}$ (k-in) =	10388	3541	10388	3541	10388	3541	10388	3541	10388
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.20	1.18	1.18	1.18	1.18	1.18	1.18	1.20	1.20
		$\Theta_{cap,pl}$ (rad) =	0.057	0.048	0.048	0.048	0.048	0.048	0.048	0.057	0.057
		Θ_{pc} (rad) =	0.100	0.099	0.099	0.099	0.099	0.099	0.099	0.100	0.100
Floor 4	Story 3	$(P/A)_g^f c_{l,exp}$ =	0.10	3759	0.21	3759	0.21	3759	0.21	3759	0.10
		$M_{y,exp}$ (k-in) =	10749	3759	10749	3759	10749	3759	10749	3759	10749
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_d/M_y =	1.19	1.18	1.18	1.18	1.18	1.18	1.18	1.19	1.19
		$\Theta_{cap,pl}$ (rad) =	0.056	0.046	0.046	0.046	0.046	0.046	0.046	0.056	0.056
		Θ_{pc} (rad) =	0.100	0.091	0.091	0.091	0.091	0.091	0.091	0.100	0.100
Floor 3	Story 2	$(P/A)_g^f c_{l,exp}$ =	0.11	3994	0.23	3994	0.23	3994	0.23	3994	0.11
		$M_{y,exp}$ (k-in) =	11107	3994	11107	3994	11107	3994	11107	3994	11107
		E_{eff}/E_g =	0.37	0.35	0.35	0.35	0.35	0.35	0.35	0.37	0.37
		M_d/M_y =	1.19	1.17	1.17	1.17	1.17	1.17	1.17	1.19	1.19
		$\Theta_{cap,pl}$ (rad) =	0.055	0.045	0.045	0.045	0.045	0.045	0.045	0.055	0.055
		Θ_{pc} (rad) =	0.100	0.084	0.084	0.084	0.084	0.084	0.084	0.100	0.100
Floor 2	Story 1	$(P/A)_g^f c_{l,exp}$ =	0.13	4076	0.25	4076	0.25	4076	0.25	4076	0.13
		$M_{y,exp}$ (k-in) =	11462	4076	11462	4076	11462	4076	11462	4076	11462
		E_{eff}/E_g =	0.38	0.52	0.52	0.52	0.52	0.52	0.52	0.38	0.38
		M_d/M_y =	1.19	1.17	1.17	1.17	1.17	1.17	1.17	1.19	1.19
		$\Theta_{cap,pl}$ (rad) =	0.053	0.043	0.043	0.043	0.043	0.043	0.043	0.053	0.053
		Θ_{pc} (rad) =	0.100	0.078	0.078	0.078	0.078	0.078	0.078	0.100	0.100
Mass tributary to one frame for lateral load (each floor) (k-s/in): 0.60											
Model periods (sec): $T_1 = 1.97$ $T_2 = 0.67$ $T_3 = 0.38$											
$f_{v,tribar,expected} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2014

Number of Stories: 12

Fundamental Period (sec): 1.97

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2014 SCWB=2.5

Simply adding more reinforcements in the column from the 2013 was sufficient to obtain SCWB=2.5. In this design drift controlled over joint shear. Both positive and negative bending strength were controlled by strength demands. Additional beam reinforcements in 12 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. Then 24 bays had additional reinforcement added to meet the minimum positive/negative strength ratio. Column flexural strengths were controlled by the strong-column weak-beam ratio. Beam stirrups were controlled by the capacity shear design and minimum requirement (half-half). All column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)											
<div> <div>Floor 13</div> <div> <div>20</div> <div>26</div> <div>0.0036</div> <div>0.0035</div> <div>4.4</div> </div> </div>											
<div> <div>Story 12</div> <div>Floor 12</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.010	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.010
	ρ_{sh} =	0.0063	0.0096	0.0096	0.0096	0.0096	0.0096	0.0096	0.0096	0.0096	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 11</div> <div>Floor 11</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.015	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.015
	ρ_{sh} =	0.0068	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0067
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 10</div> <div>Floor 10</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.015	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.015
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 9</div> <div>Floor 9</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.015	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.015
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 8</div> <div>Floor 8</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.015	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.022	0.015
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 7</div> <div>Floor 7</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.020	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.020
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 6</div> <div>Floor 6</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.020	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.020
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 5</div> <div>Floor 5</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.020	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.020
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 4</div> <div>Floor 4</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.020	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.020
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 3</div> <div>Floor 3</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.020	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.020
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 2</div> <div>Floor 2</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.018	0.023	0.023	0.023	0.023	0.023	0.023	0.023	0.023	0.018
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Story 1</div> <div>Floor 1</div> </div>	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.018	0.023	0.023	0.023	0.023	0.023	0.023	0.023	0.023	0.018
	ρ_{sh} =	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063	0.0063
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
<div> <div>Grade beam column height (in) = 24</div> <div>Basement column height (in) = 26</div> <div>20 feet</div> <div>13 feet</div> <div>15 feet</div> </div>											
<div> <div>Design base shear = 0.044 g, 232 k</div> <div> <div>f'_c beams = 5.0 ksi</div> <div>f'_c cols upper = 5.0 ksi</div> <div>f'_c rebars nom. = 60 ksi</div> <div>f'_c cols lower = 5.0 ksi</div> </div> </div>											

Design Documentation (2 of 2)											
SCWB = 1.32											
Joint $\Phi V_r/V_u = 3.07$											
Story 12	$\Phi M_r/M_u = 1.72$										
	$\Phi V_r/V_{mpc} = 2.13$										
	$P/A_g f'_c = 0.02$										
Floor 12	2.50	1.14	1.31	0.51	3.85	2.56	1.29	1.39	0.51	3.85	2.50
Story 11	$\Phi M_r/M_u = 3.20$										
	$\Phi V_r/V_{mpc} = 1.5$										
	$P/A_g f'_c = 0.03$										
Floor 11	2.73	1.14	1.36	0.51	3.09	2.62	1.27	1.45	0.51	3.09	2.73
Story 10	$\Phi M_r/M_u = 2.88$										
	$\Phi V_r/V_{mpc} = 1.84$										
	$P/A_g f'_c = 0.05$										
Floor 10	2.53	1.15	1.44	0.52	2.55	2.55	1.27	1.53	0.52	2.55	2.53
Story 9	$\Phi M_r/M_u = 2.83$										
	$\Phi V_r/V_{mpc} = 2.05$										
	$P/A_g f'_c = 0.07$										
Floor 9	2.46	1.14	1.39	0.51	2.36	2.63	1.25	1.49	0.51	2.36	2.46
Story 8	$\Phi M_r/M_u = 2.38$										
	$\Phi V_r/V_{mpc} = 2.2$										
	$P/A_g f'_c = 0.09$										
Floor 8	2.85	1.13	1.39	0.51	2.35	2.88	1.23	1.48	0.51	2.35	2.85
Story 7	$\Phi M_r/M_u = 2.38$										
	$\Phi V_r/V_{mpc} = 1.73$										
	$P/A_g f'_c = 0.11$										
Floor 7	2.44	1.13	1.22	0.52	1.16	2.34	1.32	1.29	0.52	1.16	2.44
Story 6	$\Phi M_r/M_u = 2.74$										
	$\Phi V_r/V_{mpc} = 1.67$										
	$P/A_g f'_c = 0.13$										
Floor 6	2.54	1.14	1.21	0.51	1.16	2.51	1.31	1.27	0.51	1.16	2.54
Story 5	$\Phi M_r/M_u = 2.79$										
	$\Phi V_r/V_{mpc} = 1.62$										
	$P/A_g f'_c = 0.15$										
Floor 5	2.62	1.12	1.18	0.52	1.15	2.62	1.26	1.25	0.52	1.15	2.62
Story 4	$\Phi M_r/M_u = 2.80$										
	$\Phi V_r/V_{mpc} = 1.57$										
	$P/A_g f'_c = 0.18$										
Floor 4	2.65	1.12	1.12	0.52	1.15	2.71	1.25	1.23	0.52	1.15	2.65
Story 3	$\Phi M_r/M_u = 2.83$										
	$\Phi V_r/V_{mpc} = 1.53$										
	$P/A_g f'_c = 0.20$										
Floor 3	2.57	1.13	1.14	0.55	1.15	2.66	1.24	1.30	0.55	1.15	2.57
Story 2	$\Phi M_r/M_u = 2.57$										
	$\Phi V_r/V_{mpc} = 1.58$										
	$P/A_g f'_c = 0.22$										
Floor 2	2.63	1.12	1.12	0.60	1.16	2.75	1.21	1.47	0.60	1.16	2.63
Story 1	$\Phi M_r/M_u = 2.01$										
	$\Phi V_r/V_{mpc} = 1.54$										
	$P/A_g f'_c = 0.24$										
Design Drifts:											
0.8%											
1.1%											
1.5%											
1.8%											
2.0%											
1.8%											
1.5%											
1.6%											
1.6%											
1.3%											

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	4840	1798	1798	1798	1798	1798	1798	1798	4840
		E_{eff}/E_g =	0.35	-3928	0.35	-3928	0.35	-3928	0.35	-3928	0.35
Floor 12	Story 11	M_u/M_y =	1.21	0.35	0.35	0.35	0.35	0.35	0.35	0.35	1.21
		$\Theta_{exp,pl}$ (rad) =	0.064	5.695E+07	0.064	5.695E+07	0.064	5.695E+07	0.064	5.695E+07	0.064
Floor 11	Story 10	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	108	107	107	107	107	107	107	107	108
Floor 10	Story 9	$(P/A_g f_c)_{exp}$ =	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
		$M_{y,exp}$ (k-in) =	7356	10215	10215	10215	10215	10215	10215	10215	7356
Floor 9	Story 8	E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.21	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.21
Floor 8	Story 7	$\Theta_{exp,pl}$ (rad) =	0.067	0.065	0.065	0.065	0.065	0.065	0.065	0.065	0.067
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 7	Story 6	λ =	107	103	103	103	103	103	103	103	107
		$(P/A_g f_c)_{exp}$ =	0.02	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.02
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	7740	10957	10957	10957	10957	10957	10957	10957	7740
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 5	Story 4	M_u/M_y =	1.21	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.21
		$\Theta_{exp,pl}$ (rad) =	0.064	0.063	0.063	0.063	0.063	0.063	0.063	0.063	0.064
Floor 4	Story 3	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	105	100	100	100	100	100	100	100	105
Floor 3	Story 2	$(P/A_g f_c)_{exp}$ =	0.03	0.07	0.07	0.07	0.07	0.07	0.07	0.07	0.03
		$M_{y,exp}$ (k-in) =	8121	12519	12519	12519	12519	12519	12519	12519	8121
Floor 2	Story 1	E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60											
Model periods (sec): $T_1 = 1.97$ $T_2 = 0.67$ $T_3 = 0.38$											
$f_{rubber, expected} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2015

Number of Stories: 12

Fundamental Period (sec): 1.59

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2015 Drift Limit = 0.01

To decrease the drift from the baseline design, beams were made larger as well as columns (a little bit larger). Positive bending strength was controlled by strength demand. Negative bending strength was controlled by min reinforcement requirement, since beams were made large to control drift. *NOTE: However, the minimum reinforcement requirement was ignored for the purpose changing 1 variable in the sensitivity analysis.* Additional beam reinforcements were added to alter beam design to use same ρ and ρ_{prime} in each floor and to meet the positive and negative strength ratio for the same floor and to meet the minimum positive to negative ratio requirement. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 5 by strength demand. *Minimum column reinforcement ratio is ignored for the purpose of sensitivity analysis.* Most beam stirrups were controlled by the capacity shear design. All column stirrups were controlled by the minimum confinement requirement. Joint shear did not control since beams were large for stiffness.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)														
Floor 13			28	26	0.002	0.0027	0.0024	6.4	28	26	0.002	0.0027	0.0024	6.4
Story 12	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.012							0.015					0.012
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 12			28	26	0.0022	0.0044	0.0024	6.4	28	26	0.0022	0.0044	0.0024	6.4
Story 11	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.012							0.015					0.012
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 11			28	26	0.0025	0.0050	0.0024	6.4	28	26	0.0025	0.0050	0.0024	6.4
Story 10	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.012							0.015					0.012
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 10			28	26	0.0028	0.0055	0.0024	6.4	28	26	0.0028	0.0055	0.0024	6.4
Story 9	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.012							0.016					0.012
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 9			28	26	0.0031	0.006	0.0024	6.4	28	26	0.0031	0.006	0.0024	6.4
Story 8	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.012							0.016					0.012
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 8			28	26	0.0031	0.006	0.0024	6.4	28	26	0.0031	0.006	0.0024	6.4
Story 7	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.015							0.018					0.015
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 7			34	26	0.0024	0.0047	0.0035	7.9	34	26	0.0024	0.0047	0.0035	7.9
Story 6	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.015							0.018					0.015
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 6			34	26	0.0024	0.0047	0.0035	7.9	34	26	0.0024	0.0047	0.0035	7.9
Story 5	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.014							0.018					0.014
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 5			34	26	0.0023	0.0045	0.0034	7.9	34	26	0.0023	0.0045	0.0034	7.9
Story 4	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.014							0.014					0.014
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 4			34	26	0.0023	0.0045	0.0034	7.9	34	26	0.0023	0.0045	0.0034	7.9
Story 3	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.014							0.014					0.014
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 3			34	26	0.0025	0.0045	0.0035	7.9	34	26	0.0025	0.0045	0.0035	7.9
Story 2	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.013							0.012					0.013
	ρ_{sh} =	0.0063							0.0063					0.0063
Floor 2			34	26	0.0028	0.0043	0.0035	7.9	34	26	0.0028	0.0043	0.0035	7.9
Story 1	h (in) =	26							26					26
	b (in) =	26							26					26
	ρ_{tot} =	0.013							0.011					0.013
	ρ_{sh} =	0.0063							0.0063					0.0063
Basement			24	26	0.0028	0.0043	0.0035	7.9	24	26	0.0028	0.0043	0.0035	7.9
Grade beam column height (in) =			24											24
Basement column height (in) =			26											26
Design base shear =			0.044 g, 232 k											0.044 g, 232 k
f'_c beams =			5.0 ksi											5.0 ksi
f'_c rebar nom. =			60 ksi											60 ksi
f'_c cols upper =			5.0 ksi											5.0 ksi
f'_c cols lower =			5.0 ksi											5.0 ksi

Design Documentation (2 of 2)											
SCWB = 0.85											
Joint $\Phi V_r/V_u = 5.52$											
Story 12	$\phi M_r/M_u = 2.40$			$\phi M_r/M_u = 5.22$			$\phi M_r/M_u = 5.23$			$\phi M_r/M_u = 2.40$	
	$\Phi V_r/V_{npr} = 1.71$			$\Phi V_r/V_{npr} = 1.32$			$\Phi V_r/V_{npr} = 1.32$			$\Phi V_r/V_{npr} = 1.71$	
	$P/A_g f'_c = 0.02$			$P/A_g f'_c = 0.03$			$P/A_g f'_c = 0.03$			$P/A_g f'_c = 0.02$	
Floor 12											
1.38											
2.69											
Story 11	$\phi M_r/M_u = 2.53$			$\phi M_r/M_u = 3.92$			$\phi M_r/M_u = 3.92$			$\phi M_r/M_u = 2.54$	
	$\Phi V_r/V_{npr} = 1.56$			$\Phi V_r/V_{npr} = 1.71$			$\Phi V_r/V_{npr} = 1.71$			$\Phi V_r/V_{npr} = 1.56$	
	$P/A_g f'_c = 0.04$			$P/A_g f'_c = 0.06$			$P/A_g f'_c = 0.06$			$P/A_g f'_c = 0.04$	
Floor 11											
1.36											
2.40											
Story 10	$\phi M_r/M_u = 2.27$			$\phi M_r/M_u = 3.19$			$\phi M_r/M_u = 3.19$			$\phi M_r/M_u = 2.27$	
	$\Phi V_r/V_{npr} = 2.03$			$\Phi V_r/V_{npr} = 1.57$			$\Phi V_r/V_{npr} = 1.57$			$\Phi V_r/V_{npr} = 2.03$	
	$P/A_g f'_c = 0.06$			$P/A_g f'_c = 0.09$			$P/A_g f'_c = 0.09$			$P/A_g f'_c = 0.06$	
Floor 10											
1.33											
2.11											
Story 9	$\phi M_r/M_u = 2.11$			$\phi M_r/M_u = 2.95$			$\phi M_r/M_u = 2.95$			$\phi M_r/M_u = 2.11$	
	$\Phi V_r/V_{npr} = 2.25$			$\Phi V_r/V_{npr} = 1.67$			$\Phi V_r/V_{npr} = 1.67$			$\Phi V_r/V_{npr} = 2.25$	
	$P/A_g f'_c = 0.08$			$P/A_g f'_c = 0.12$			$P/A_g f'_c = 0.12$			$P/A_g f'_c = 0.08$	
Floor 9											
1.31											
1.88											
Story 8	$\phi M_r/M_u = 1.97$			$\phi M_r/M_u = 2.75$			$\phi M_r/M_u = 2.75$			$\phi M_r/M_u = 1.97$	
	$\Phi V_r/V_{npr} = 2.51$			$\Phi V_r/V_{npr} = 1.86$			$\Phi V_r/V_{npr} = 1.86$			$\Phi V_r/V_{npr} = 2.51$	
	$P/A_g f'_c = 0.10$			$P/A_g f'_c = 0.15$			$P/A_g f'_c = 0.15$			$P/A_g f'_c = 0.10$	
Floor 8											
1.48											
1.88											
Story 7	$\phi M_r/M_u = 2.00$			$\phi M_r/M_u = 2.50$			$\phi M_r/M_u = 2.50$			$\phi M_r/M_u = 2.01$	
	$\Phi V_r/V_{npr} = 2.07$			$\Phi V_r/V_{npr} = 1.65$			$\Phi V_r/V_{npr} = 1.65$			$\Phi V_r/V_{npr} = 2.07$	
	$P/A_g f'_c = 0.12$			$P/A_g f'_c = 0.19$			$P/A_g f'_c = 0.19$			$P/A_g f'_c = 0.12$	
Floor 7											
1.33											
2.03											
Story 6	$\phi M_r/M_u = 2.14$			$\phi M_r/M_u = 2.67$			$\phi M_r/M_u = 2.67$			$\phi M_r/M_u = 2.14$	
	$\Phi V_r/V_{npr} = 1.98$			$\Phi V_r/V_{npr} = 1.58$			$\Phi V_r/V_{npr} = 1.58$			$\Phi V_r/V_{npr} = 1.98$	
	$P/A_g f'_c = 0.14$			$P/A_g f'_c = 0.22$			$P/A_g f'_c = 0.22$			$P/A_g f'_c = 0.14$	
Floor 6											
1.32											
2.03											
Story 5	$\phi M_r/M_u = 2.07$			$\phi M_r/M_u = 2.77$			$\phi M_r/M_u = 2.77$			$\phi M_r/M_u = 2.07$	
	$\Phi V_r/V_{npr} = 1.97$			$\Phi V_r/V_{npr} = 1.52$			$\Phi V_r/V_{npr} = 1.52$			$\Phi V_r/V_{npr} = 1.97$	
	$P/A_g f'_c = 0.16$			$P/A_g f'_c = 0.25$			$P/A_g f'_c = 0.25$			$P/A_g f'_c = 0.16$	
Floor 5											
1.34											
2.10											
Story 4	$\phi M_r/M_u = 2.12$			$\phi M_r/M_u = 2.48$			$\phi M_r/M_u = 2.48$			$\phi M_r/M_u = 2.12$	
	$\Phi V_r/V_{npr} = 1.9$			$\Phi V_r/V_{npr} = 1.63$			$\Phi V_r/V_{npr} = 1.63$			$\Phi V_r/V_{npr} = 1.9$	
	$P/A_g f'_c = 0.18$			$P/A_g f'_c = 0.28$			$P/A_g f'_c = 0.28$			$P/A_g f'_c = 0.18$	
Floor 4											
1.36											
2.10											
Story 3	$\phi M_r/M_u = 2.19$			$\phi M_r/M_u = 2.62$			$\phi M_r/M_u = 2.62$			$\phi M_r/M_u = 2.19$	
	$\Phi V_r/V_{npr} = 1.83$			$\Phi V_r/V_{npr} = 1.5$			$\Phi V_r/V_{npr} = 1.5$			$\Phi V_r/V_{npr} = 1.83$	
	$P/A_g f'_c = 0.20$			$P/A_g f'_c = 0.31$			$P/A_g f'_c = 0.31$			$P/A_g f'_c = 0.20$	
Floor 3											
1.34											
2.10											
Story 2	$\phi M_r/M_u = 2.16$			$\phi M_r/M_u = 2.56$			$\phi M_r/M_u = 2.56$			$\phi M_r/M_u = 2.16$	
	$\Phi V_r/V_{npr} = 1.83$			$\Phi V_r/V_{npr} = 1.61$			$\Phi V_r/V_{npr} = 1.61$			$\Phi V_r/V_{npr} = 1.83$	
	$P/A_g f'_c = 0.22$			$P/A_g f'_c = 0.35$			$P/A_g f'_c = 0.35$			$P/A_g f'_c = 0.22$	
Floor 2											
1.37											
2.22											
Story 1	$\phi M_r/M_u = 1.91$			$\phi M_r/M_u = 2.52$			$\phi M_r/M_u = 2.52$			$\phi M_r/M_u = 1.91$	
	$\Phi V_r/V_{npr} = 1.77$			$\Phi V_r/V_{npr} = 1.69$			$\Phi V_r/V_{npr} = 1.69$			$\Phi V_r/V_{npr} = 1.77$	
	$P/A_g f'_c = 0.24$			$P/A_g f'_c = 0.38$			$P/A_g f'_c = 0.38$			$P/A_g f'_c = 0.24$	
$(\phi M_r/M_u)_{neg} = 1.16$											
$(\phi M_r/M_u)_{pos} = 1.16$											
$M_{u,post}/M_{u,neg} = 0.67$											
$\Phi V_r/V_{npr} = 1.16$											
7068.58											
Design Drifts:											
0.4%											
0.6%											
0.8%											
1.0%											
1.1%											
1.0%											
0.9%											
0.9%											
1.0%											
1.0%											

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	5693	2112	4379	1.574E+08	0.0327	0.046	0.084	99	5693
		E_{eff}/E_g =	0.35								0.35
Floor 12	Story 11	M_u/M_y =	1.21	2388	-6097	0.35	0.0327	0.046	0.084	99	1.21
		$\Theta_{cap,pl}$ (rad) =	0.065								0.065
Floor 11	Story 10	Θ_{pc} (rad) =	0.100	108							0.100
		λ =	0.01	2388	-6097	0.35	0.0327	0.046	0.084	99	0.01
Floor 10	Story 9	$(P/A_y f_c)_{exp}$ =	8087	2648	-6648	1.574E+08	0.0327	0.046	0.084	99	8087
		$M_{y,exp}$ (k-in) =	8087	2648	-6648	1.574E+08	0.0327	0.046	0.084	99	8087
Floor 9	Story 8	E_{eff}/E_g =	0.35								0.35
		M_u/M_y =	1.21	2648	-6648	0.35	0.0327	0.046	0.084	99	1.21
Floor 8	Story 7	$\Theta_{cap,pl}$ (rad) =	0.064								0.064
		Θ_{pc} (rad) =	0.100	107							0.100
Floor 7	Story 6	λ =	0.02	2648	-6648	1.574E+08	0.0327	0.046	0.084	99	0.02
		$(P/A_y f_c)_{exp}$ =	8872	2993	-7148	1.574E+08	0.0327	0.046	0.084	99	8872
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	8872	2993	-7148	1.574E+08	0.0327	0.046	0.084	99	8872
		E_{eff}/E_g =	0.35								0.35
Floor 5	Story 4	M_u/M_y =	1.20	2993	-7148	0.35	0.0327	0.046	0.084	99	1.20
		$\Theta_{cap,pl}$ (rad) =	0.061								0.061
Floor 4	Story 3	Θ_{pc} (rad) =	0.100	103							0.100
		λ =	0.05	3252	-7647	1.574E+08	0.0327	0.046	0.084	99	0.05
Floor 3	Story 2	$(P/A_y f_c)_{exp}$ =	10025	2993	-7148	1.574E+08	0.0327	0.046	0.084	99	10025
		$M_{y,exp}$ (k-in) =	10025	2993	-7148	1.574E+08	0.0327	0.046	0.084	99	10025
Floor 2	Story 1	E_{eff}/E_g =	0.35								0.35
		M_u/M_y =	1.20	2993	-7148	0.35	0.0327	0.046	0.084	99	1.20
<div>Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60</div> <div>Model periods (sec): $T_1 = 1.59$ $T_2 = 0.54$ $T_3 = 0.31$</div> <div>$f_{y,ribber,expected} = 67$ ksi</div>											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2017

Number of Stories: 12

Fundamental Period (sec): 2.20

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2017 Drift Limit = 0.03

Note: in order to reach high drift and have design not be overly concrete in strength for the benefit of the sensitivity analysis, the minimum steel reinforcement requirement in the concrete column is neglected, such that SCWB ratio is not too high and conservative. In attempting to increase the drift from the baseline design, beams were made smaller. The SCWB came out to be between 1.9 – 1.46 and max limit at 2.9%.

Both positive and negative bending strength were controlled by strength demands. Additional beam reinforcements were added to alter beam design to use same ρ and ρ_{Prime} in each floor and to meet the positive and negative strength ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 6 by strength demand (note again min reinforcement is not applied). Beam stirrups were controlled by the capacity shear design and minimum requirement. All column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)												
Floor 13												
Story 12	h (in) =	24	18	24	0.005	0.0080	0.0042	3.9	18	24	0.005	0.0080
	b (in) =	24	18	24	0.0059	0.0113	0.0042	3.9	18	24	0.0059	0.0113
	ρ_{tot} =	0.009	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.009	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 12												
Story 11	h (in) =	24	18	24	0.007	0.0140	0.0042	3.9	18	24	0.007	0.0140
	b (in) =	24	18	24	0.007	0.0140	0.0042	3.9	18	24	0.007	0.0140
	ρ_{tot} =	0.008	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.008	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 11												
Story 10	h (in) =	24	18	24	0.008	0.0158	0.0042	3.9	18	24	0.008	0.0158
	b (in) =	24	18	24	0.008	0.0158	0.0042	3.9	18	24	0.008	0.0158
	ρ_{tot} =	0.008	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.008	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 10												
Story 9	h (in) =	24	18	24	0.0087	0.017	0.0042	3.9	18	24	0.0087	0.017
	b (in) =	24	18	24	0.0087	0.017	0.0042	3.9	18	24	0.0087	0.017
	ρ_{tot} =	0.008	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.008	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 9												
Story 8	h (in) =	24	18	24	0.0084	0.0168	0.0042	3.9	18	24	0.0084	0.0168
	b (in) =	24	18	24	0.0084	0.0168	0.0042	3.9	18	24	0.0084	0.0168
	ρ_{tot} =	0.008	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.008	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 8												
Story 7	h (in) =	24	24	24	0.006	0.0115	0.0056	5.4	24	24	0.006	0.0115
	b (in) =	24	24	24	0.006	0.0115	0.0056	5.4	24	24	0.006	0.0115
	ρ_{tot} =	0.012	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.012	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 7												
Story 6	h (in) =	24	24	24	0.0057	0.0113	0.0055	5.4	24	24	0.0057	0.0113
	b (in) =	24	24	24	0.0057	0.0113	0.0055	5.4	24	24	0.0057	0.0113
	ρ_{tot} =	0.012	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.012	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 6												
Story 5	h (in) =	24	24	24	0.0057	0.0113	0.0055	5.4	24	24	0.0057	0.0113
	b (in) =	24	24	24	0.0057	0.0113	0.0055	5.4	24	24	0.0057	0.0113
	ρ_{tot} =	0.012	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.012	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 5												
Story 4	h (in) =	24	24	24	0.0057	0.0113	0.0055	5.4	24	24	0.0057	0.0113
	b (in) =	24	24	24	0.0057	0.0113	0.0055	5.4	24	24	0.0057	0.0113
	ρ_{tot} =	0.010	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.010	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 4												
Story 3	h (in) =	24	24	24	0.0057	0.0106	0.0054	5.4	24	24	0.0057	0.0106
	b (in) =	24	24	24	0.0057	0.0106	0.0054	5.4	24	24	0.0057	0.0106
	ρ_{tot} =	0.010	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.010	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 3												
Story 2	h (in) =	24	24	24	0.0058	0.0098	0.0053	5.4	24	24	0.0058	0.0098
	b (in) =	24	24	24	0.0058	0.0098	0.0053	5.4	24	24	0.0058	0.0098
	ρ_{tot} =	0.010	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.010	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Floor 2												
Story 1	h (in) =	24	24	24	0.0058	0.0098	0.0053	5.4	24	24	0.0058	0.0098
	b (in) =	24	24	24	0.0058	0.0098	0.0053	5.4	24	24	0.0058	0.0098
	ρ_{tot} =	0.013	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	ρ_{sh} =	0.013	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102	0.0102
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Grade beam column height (in) = 24												
Basement column height (in) = 24												
20 feet												
13 feet												
15 feet												
Design base shear = 0.044 g, 232 k												
f'_c beams = 5.0 ksi f'_c cols upper = 8.0 ksi												
f_y rebar nom. = 60 ksi f_y cols lower = 8.0 ksi												

Design Documentation (2 of 2)											
SCWB = 0.93											
Joint $\Phi V_r/V_u = 2.76$											
Story 12	$\Phi M_r/M_u = 1.16$										
	$\Phi V_r/V_{mpc} = 4.22$										
	$P/A_g f_c = 0.01$										
Design Drifts:											
1.1%											
Floor 12											
Story 11	$\Phi M_r/M_u = 1.43$										
	$\Phi V_r/V_{mpc} = 3.87$										
	$P/A_g f_c = 0.02$										
1.7%											
Floor 11											
Story 10	$\Phi M_r/M_u = 1.32$										
	$\Phi V_r/V_{mpc} = 3.39$										
	$P/A_g f_c = 0.04$										
2.2%											
Floor 10											
Story 9	$\Phi M_r/M_u = 1.37$										
	$\Phi V_r/V_{mpc} = 4.79$										
	$P/A_g f_c = 0.05$										
2.7%											
Floor 9											
Story 8	$\Phi M_r/M_u = 1.17$										
	$\Phi V_r/V_{mpc} = 4.85$										
	$P/A_g f_c = 0.07$										
2.9%											
Floor 8											
Story 7	$\Phi M_r/M_u = 1.23$										
	$\Phi V_r/V_{mpc} = 3.67$										
	$P/A_g f_c = 0.08$										
2.6%											
Floor 7											
Story 6	$\Phi M_r/M_u = 1.46$										
	$\Phi V_r/V_{mpc} = 3.45$										
	$P/A_g f_c = 0.10$										
2.2%											
Floor 6											
Story 5	$\Phi M_r/M_u = 1.50$										
	$\Phi V_r/V_{mpc} = 3.26$										
	$P/A_g f_c = 0.11$										
2.2%											
Floor 5											
Story 4	$\Phi M_r/M_u = 1.34$										
	$\Phi V_r/V_{mpc} = 3.34$										
	$P/A_g f_c = 0.13$										
2.2%											
Floor 4											
Story 3	$\Phi M_r/M_u = 1.35$										
	$\Phi V_r/V_{mpc} = 3.18$										
	$P/A_g f_c = 0.14$										
2.3%											
Floor 3											
Story 2	$\Phi M_r/M_u = 1.33$										
	$\Phi V_r/V_{mpc} = 3.04$										
	$P/A_g f_c = 0.16$										
2.2%											
Floor 2											
Story 1	$\Phi M_r/M_u = 1.16$										
	$\Phi V_r/V_{mpc} = 2.7$										
	$P/A_g f_c = 0.17$										
1.8%											

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) = 3328 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.19 $\Theta_{cap,pl}$ (rad) = 0.071 Θ_{pc} (rad) = 0.100 λ = 110 $(P/A_g f_c)_{exp}$ = 0.01	1761 -3747 0.35 3.902E+0 0.0514 -0.082 0.100 101	3539 0.35 1.18 0.069 0.100 109 0.02	1761 -3747 0.35 3.902E+0 0.0514 -0.082 0.100 101	3539 0.35 1.18 0.069 0.100 109 0.02	1761 -3747 0.35 3.902E+0 0.0514 -0.082 0.100 101	3539 0.35 1.18 0.069 0.100 109 0.02	1761 -3747 0.35 3.902E+0 0.0514 -0.082 0.100 101	3539 0.35 1.18 0.069 0.100 109 0.02	1761 -3747 0.35 3.902E+0 0.0514 -0.082 0.100 101
Floor 12	Story 11	$M_{y,exp}$ (k-in) = 3539 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.069 Θ_{pc} (rad) = 0.100 λ = 109 $(P/A_g f_c)_{exp}$ = 0.02	2064 -4808 0.35 3.902E+0 0.053 -0.086 0.100 101	4930 0.35 1.18 0.068 0.100 106 0.03	2064 -4808 0.35 3.902E+0 0.053 -0.086 0.100 101	4930 0.35 1.18 0.068 0.100 106 0.03	2064 -4808 0.35 3.902E+0 0.053 -0.086 0.100 101	4930 0.35 1.18 0.068 0.100 106 0.03	2064 -4808 0.35 3.902E+0 0.053 -0.086 0.100 101	4930 0.35 1.18 0.068 0.100 106 0.03	2064 -4808 0.35 3.902E+0 0.053 -0.086 0.100 101
Floor 11	Story 10	$M_{y,exp}$ (k-in) = 3911 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.068 Θ_{pc} (rad) = 0.100 λ = 108 $(P/A_g f_c)_{exp}$ = 0.03	2437 -5697 0.35 3.902E+0 0.055 -0.089 0.100 101	5643 0.35 1.18 0.066 0.100 104 0.05	2437 -5697 0.35 3.902E+0 0.055 -0.089 0.100 101	5643 0.35 1.18 0.066 0.100 104 0.05	2437 -5697 0.35 3.902E+0 0.055 -0.089 0.100 101	5643 0.35 1.18 0.066 0.100 104 0.05	2437 -5697 0.35 3.902E+0 0.055 -0.089 0.100 101	5643 0.35 1.18 0.066 0.100 104 0.05	2437 -5697 0.35 3.902E+0 0.055 -0.089 0.100 101
Floor 10	Story 9	$M_{y,exp}$ (k-in) = 4277 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.067 Θ_{pc} (rad) = 0.100 λ = 106 $(P/A_g f_c)_{exp}$ = 0.03	2770 -6259 0.35 3.902E+0 0.057 -0.090 0.100 101	6339 0.35 1.18 0.064 0.100 102 0.07	2770 -6259 0.35 3.902E+0 0.057 -0.090 0.100 101	6339 0.35 1.18 0.064 0.100 102 0.07	2770 -6259 0.35 3.902E+0 0.057 -0.090 0.100 101	6339 0.35 1.18 0.064 0.100 102 0.07	2770 -6259 0.35 3.902E+0 0.057 -0.090 0.100 101	6339 0.35 1.18 0.064 0.100 102 0.07	2770 -6259 0.35 3.902E+0 0.057 -0.090 0.100 101
Floor 9	Story 8	$M_{y,exp}$ (k-in) = 4639 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.066 Θ_{pc} (rad) = 0.100 λ = 105 $(P/A_g f_c)_{exp}$ = 0.04	3001 -6659 0.35 3.902E+0 0.058 -0.090 0.100 101	7022 0.35 1.18 0.062 0.100 100 0.08	3001 -6659 0.35 3.902E+0 0.058 -0.090 0.100 101	7022 0.35 1.18 0.062 0.100 100 0.08	3001 -6659 0.35 3.902E+0 0.058 -0.090 0.100 101	7022 0.35 1.18 0.062 0.100 100 0.08	3001 -6659 0.35 3.902E+0 0.058 -0.090 0.100 101	7022 0.35 1.18 0.062 0.100 100 0.08	3001 -6659 0.35 3.902E+0 0.058 -0.090 0.100 101
Floor 8	Story 7	$M_{y,exp}$ (k-in) = 5288 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.067 Θ_{pc} (rad) = 0.100 λ = 104 $(P/A_g f_c)_{exp}$ = 0.05	2918 -6579 0.35 9.678E+0 0.052 -0.083 0.100 100	8965 0.37 1.17 0.062 0.100 97 0.10	2918 -6579 0.35 9.678E+0 0.052 -0.083 0.100 100	8965 0.37 1.17 0.062 0.100 97 0.10	2918 -6579 0.35 9.678E+0 0.052 -0.083 0.100 100	8965 0.37 1.17 0.062 0.100 97 0.10	2918 -6579 0.35 9.678E+0 0.052 -0.083 0.100 100	8965 0.37 1.17 0.062 0.100 97 0.10	2918 -6579 0.35 9.678E+0 0.052 -0.083 0.100 100
Floor 7	Story 6	$M_{y,exp}$ (k-in) = 5636 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.066 Θ_{pc} (rad) = 0.100 λ = 103 $(P/A_g f_c)_{exp}$ = 0.06	4074 -9020 0.35 9.678E+0 0.052 -0.082 0.100 100	8986 0.39 1.17 0.059 0.100 95 0.12	4074 -9020 0.35 9.678E+0 0.052 -0.082 0.100 100	8986 0.39 1.17 0.059 0.100 95 0.12	4074 -9020 0.35 9.678E+0 0.052 -0.082 0.100 100	8986 0.39 1.17 0.059 0.100 95 0.12	4074 -9020 0.35 9.678E+0 0.052 -0.082 0.100 100	8986 0.39 1.17 0.059 0.100 95 0.12	4074 -9020 0.35 9.678E+0 0.052 -0.082 0.100 100
Floor 6	Story 5	$M_{y,exp}$ (k-in) = 5981 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.065 Θ_{pc} (rad) = 0.100 λ = 102 $(P/A_g f_c)_{exp}$ = 0.07	3908 -8861 0.35 9.678E+0 0.052 -0.083 0.100 100	9000 0.40 1.17 0.057 0.100 93 0.13	3908 -8861 0.35 9.678E+0 0.052 -0.083 0.100 100	9000 0.40 1.17 0.057 0.100 93 0.13	3908 -8861 0.35 9.678E+0 0.052 -0.083 0.100 100	9000 0.40 1.17 0.057 0.100 93 0.13	3908 -8861 0.35 9.678E+0 0.052 -0.083 0.100 100	9000 0.40 1.17 0.057 0.100 93 0.13	3908 -8861 0.35 9.678E+0 0.052 -0.083 0.100 100
Floor 5	Story 4	$M_{y,exp}$ (k-in) = 6682 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.063 Θ_{pc} (rad) = 0.100 λ = 101 $(P/A_g f_c)_{exp}$ = 0.08	3894 -8914 0.35 9.678E+0 0.052 -0.083 0.100 100	9639 0.42 1.17 0.055 0.100 91 0.15	3894 -8914 0.35 9.678E+0 0.052 -0.083 0.100 100	9639 0.42 1.17 0.055 0.100 91 0.15	3894 -8914 0.35 9.678E+0 0.052 -0.083 0.100 100	9639 0.42 1.17 0.055 0.100 91 0.15	3894 -8914 0.35 9.678E+0 0.052 -0.083 0.100 100	9639 0.42 1.17 0.055 0.100 91 0.15	3894 -8914 0.35 9.678E+0 0.052 -0.083 0.100 100
Floor 4	Story 3	$M_{y,exp}$ (k-in) = 7022 E_{eff}/E_{lg} = 0.35 M_d/M_y = 1.18 $\Theta_{cap,pl}$ (rad) = 0.062 Θ_{pc} (rad) = 0.100 λ = 100 $(P/A_g f_c)_{exp}$ = 0.08	3894 -8440 0.35 9.678E+0 0.052 -0.081 0.100 100	10270 0.44 1.16 0.053 0.100 89 0.17	3894 -8440 0.35 9.678E+0 0.052 -0.081 0.100 100	10270 0.44 1.16 0.053 0.100 89 0.17	3894 -8440 0.35 9.678E+0 0.052 -0.081 0.100 100	10270 0.44 1.16 0.053 0.100 89 0.17	3894 -8440 0.35 9.678E+0 0.052 -0.081 0.100 100	10270 0.44 1.16 0.053 0.100 89 0.17	3894 -8440 0.35 9.678E+0 0.052 -0.081 0.100 100
Floor 3	Story 2	$M_{y,exp}$ (k-in) = 7359 E_{eff}/E_{lg} = 0.36 M_d/M_y = 1.17 $\Theta_{cap,pl}$ (rad) = 0.061 Θ_{pc} (rad) = 0.100 λ = 99 $(P/A_g f_c)_{exp}$ = 0.09	3909 -8440 0.35 9.678E+0 0.052 -0.078 0.100 100	10260 0.45 1.16 0.051 0.100 87 0.18	3909 -8440 0.35 9.678E+0 0.052 -0.078 0.100 100	10260 0.45 1.16 0.051 0.100 87 0.18	3909 -8440 0.35 9.678E+0 0.052 -0.078 0.100 100	10260 0.45 1.16 0.051 0.100 87 0.18	3909 -8440 0.35 9.678E+0 0.052 -0.078 0.100 100	10260 0.45 1.16 0.051 0.100 87 0.18	3909 -8440 0.35 9.678E+0 0.052 -0.078 0.100 100
Floor 2	Story 1	$M_{y,exp}$ (k-in) = 8488 E_{eff}/E_{lg} = 0.37 M_d/M_y = 1.17 $\Theta_{cap,pl}$ (rad) = 0.062 Θ_{pc} (rad) = 0.100 λ = 97 $(P/A_g f_c)_{exp}$ = 0.10	3971 -7965 0.35 9.678E+0 0.052 -0.078 0.100 100	10875 0.47 1.16 0.049 0.100 85 0.20	3971 -7965 0.35 9.678E+0 0.052 -0.078 0.100 100	10875 0.47 1.16 0.049 0.100 85 0.20	3971 -7965 0.35 9.678E+0 0.052 -0.078 0.100 100	10875 0.47 1.16 0.049 0.100 85 0.20	3971 -7965 0.35 9.678E+0 0.052 -0.078 0.100 100	10875 0.47 1.16 0.049 0.100 85 0.20	3971 -7965 0.35 9.678E+0 0.052 -0.078 0.100 100
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60											
Model periods (sec): $T_1 = 2.20$ $T_2 = 0.74$ $T_3 = 0.43$											
$I_{n,relax,expected} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2018

Number of Stories: 12

Fundamental Period (sec): 2.64

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2018 Drift unlimited

Note: in order to reach high drift and have design not be overly concrete in strength for the benefit of the sensitivity analysis, the minimum steel reinforcement requirement in the concrete column is neglected. In this design drift does not control. Beam sizes were reduced until beams were controlled by flexural (beams exceeds the maximum allowable steel reinforcement). Eventually with the min beam size that would still be strong enough for bending and column strength large enough for joint shear, the max inter-story drift came out to be 3.62%.

Both positive and negative bending strength were controlled by max reinforced ratio. Few additional beam reinforcements in 10 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 6 by strength demand. Less than $\frac{1}{2}$ of the Beam stirrups were controlled by the capacity shear design and most others by minimum reinforcement requirement. All column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

Floor 13										16	27	0.006	0.0100	0.0043	3.4	16	27	0.006	0.0100	0.0043	3.4
Story 12	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.007													0.006					0.007	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 12										16	27	0.0069	0.0133	0.0043	3.4	16	27	0.0069	0.0133	0.0043	3.4
Story 11	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.005													0.006					0.005	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 11										16	27	0.008	0.0155	0.0043	3.4	16	27	0.008	0.0155	0.0043	3.4
Story 10	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.005													0.006					0.005	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 10										16	27	0.009	0.0168	0.0043	3.4	16	27	0.009	0.0168	0.0043	3.4
Story 9	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.006													0.006					0.006	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 9										18	27	0.0082	0.015	0.0038	3.9	18	27	0.0082	0.015	0.0038	3.9
Story 8	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.006													0.006					0.006	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 8										18	27	0.0084	0.0163	0.0038	3.9	18	27	0.0084	0.0163	0.0038	3.9
Story 7	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.006													0.006					0.006	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 7										18	27	0.0095	0.0180	0.0066	3.9	18	27	0.0095	0.0180	0.0066	3.9
Story 6	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.006													0.006					0.006	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 6										18	27	0.0092	0.0183	0.0066	3.9	18	27	0.0092	0.0183	0.0066	3.9
Story 5	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.006													0.006					0.006	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 5										18	27	0.0102	0.0183	0.0067	3.9	18	27	0.0102	0.0183	0.0067	3.9
Story 4	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.005													0.003					0.005	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 4										18	27	0.0097	0.0183	0.0066	3.9	18	27	0.0097	0.0183	0.0066	3.9
Story 3	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.006													0.003					0.006	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 3										18	27	0.0092	0.0181	0.0066	3.9	18	27	0.0092	0.0181	0.0066	3.9
Story 2	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.009													0.003					0.009	
	ρ_{int} =	0.0102													0.0102					0.0102	
Floor 2										18	27	0.0078	0.0153	0.0038	3.9	18	27	0.0078	0.0153	0.0038	3.9
Story 1	h (in) =	27													27					27	
	b (in) =	27													27					27	
	ρ_{ext} =	0.016													0.007					0.016	
	ρ_{int} =	0.0102													0.0102					0.0102	
Basement										24	27	0.007	0.0102	4.0	27	24	27	0.007	0.0102	4.0	27
Grade beam column height (in) = 24												Basement column height (in) = 27									
												20 feet									
Design base shear = 0.044 g _s 232 k																					
f _c beams = 5.0 ksi												f _c cols upper = 8.0 ksi									
f _y rebar nom. = 60 ksi												f _c cols lower = 8.0 ksi									

13 feet

15 feet

Design Documentation (2 of 2)												
SCWB = 1.10												
Joint $\Phi V_r/V_u = 2.71$												
Story 12	$\Phi M_r/M_u = 1.26$ $\Phi V_r/V_{npr} = 4.58$ $P/A_g f'_c = 0.01$			1.11 1.21 0.61 2.90			0.70 2.26			0.60 2.26		
	1.26 4.58 0.01			1.11 1.21 0.61 2.90			0.70 2.26			0.60 2.26		
	1.26 4.58 0.01			1.11 1.21 0.61 2.90			0.70 2.26			0.60 2.26		
Floor 12												
1.59 1.86			1.16 1.36 0.53 2.32			1.23 1.63			1.23 1.63			
Story 11	$\Phi M_r/M_u = 1.17$ $\Phi V_r/V_{npr} = 5.11$ $P/A_g f'_c = 0.02$			1.11 1.44 0.53 2.00			1.81 3.73 0.04			1.81 3.73 0.04		
	1.17 5.11 0.02			1.11 1.44 0.53 2.00			1.81 3.73 0.04			1.81 3.73 0.04		
	1.17 5.11 0.02			1.11 1.44 0.53 2.00			1.81 3.73 0.04			1.81 3.73 0.04		
Floor 11												
1.30 1.59			1.19 1.44 0.53 2.00			1.24 1.40			1.24 1.40			
Story 10	$\Phi M_r/M_u = 1.15$ $\Phi V_r/V_{npr} = 4.39$ $P/A_g f'_c = 0.03$			1.17 1.51 0.55 1.83			1.81 4.19 0.05			1.81 4.19 0.05		
	1.15 4.39 0.03			1.17 1.51 0.55 1.83			1.81 4.19 0.05			1.81 4.19 0.05		
	1.15 4.39 0.03			1.17 1.51 0.55 1.83			1.81 4.19 0.05			1.81 4.19 0.05		
Floor 10												
1.39 1.45			1.17 1.51 0.55 1.83			1.28 1.26			1.28 1.26			
Story 9	$\Phi M_r/M_u = 1.46$ $\Phi V_r/V_{npr} = 4.11$ $P/A_g f'_c = 0.04$			1.11 1.42 0.56 1.83			1.79 4.27 0.07			1.79 4.27 0.07		
	1.46 4.11 0.04			1.11 1.42 0.56 1.83			1.79 4.27 0.07			1.79 4.27 0.07		
	1.46 4.11 0.04			1.11 1.42 0.56 1.83			1.79 4.27 0.07			1.79 4.27 0.07		
Floor 9												
1.31 1.45			1.11 1.42 0.56 1.83			1.19 1.25			1.19 1.25			
Story 8	$\Phi M_r/M_u = 1.48$ $\Phi V_r/V_{npr} = 5.46$ $P/A_g f'_c = 0.05$			1.15 1.31 0.53 1.70			1.82 4.18 0.09			1.82 4.18 0.09		
	1.48 5.46 0.05			1.15 1.31 0.53 1.70			1.82 4.18 0.09			1.82 4.18 0.09		
	1.48 5.46 0.05			1.15 1.31 0.53 1.70			1.82 4.18 0.09			1.82 4.18 0.09		
Floor 8												
1.27 1.34			1.15 1.31 0.53 1.70			1.23 1.17			1.23 1.17			
Story 7	$\Phi M_r/M_u = 1.44$ $\Phi V_r/V_{npr} = 5$ $P/A_g f'_c = 0.06$			1.21 1.32 0.54 1.16			1.81 3.79 0.11			1.81 3.79 0.11		
	1.44 5 0.06			1.21 1.32 0.54 1.16			1.81 3.79 0.11			1.81 3.79 0.11		
	1.44 5 0.06			1.21 1.32 0.54 1.16			1.81 3.79 0.11			1.81 3.79 0.11		
Floor 7												
1.21 1.21			1.21 1.32 0.54 1.16			1.22 1.05			1.22 1.05			
Story 6	$\Phi M_r/M_u = 1.40$ $\Phi V_r/V_{npr} = 4.62$ $P/A_g f'_c = 0.08$			1.18 1.19 0.52 1.16			1.78 3.49 0.13			1.78 3.49 0.13		
	1.40 4.62 0.08			1.18 1.19 0.52 1.16			1.78 3.49 0.13			1.78 3.49 0.13		
	1.40 4.62 0.08			1.18 1.19 0.52 1.16			1.78 3.49 0.13			1.78 3.49 0.13		
Floor 6												
1.23 1.19			1.18 1.19 0.52 1.16			1.32 1.05			1.32 1.05			
Story 5	$\Phi M_r/M_u = 1.35$ $\Phi V_r/V_{npr} = 4.3$ $P/A_g f'_c = 0.09$			1.16 1.25 0.57 1.16			1.77 3.24 0.15			1.77 3.24 0.15		
	1.35 4.3 0.09			1.16 1.25 0.57 1.16			1.77 3.24 0.15			1.77 3.24 0.15		
	1.35 4.3 0.09			1.16 1.25 0.57 1.16			1.77 3.24 0.15			1.77 3.24 0.15		
Floor 5												
1.19 1.18			1.16 1.25 0.57 1.16			1.24 1.01			1.24 1.01			
Story 4	$\Phi M_r/M_u = 1.16$ $\Phi V_r/V_{npr} = 4.26$ $P/A_g f'_c = 0.10$			1.16 1.17 0.54 1.15			1.46 3.43 0.17			1.46 3.43 0.17		
	1.16 4.26 0.10			1.16 1.17 0.54 1.15			1.46 3.43 0.17			1.46 3.43 0.17		
	1.16 4.26 0.10			1.16 1.17 0.54 1.15			1.46 3.43 0.17			1.46 3.43 0.17		
Floor 4												
1.22 1.18			1.16 1.17 0.54 1.15			1.23 1.03			1.23 1.03			
Story 3	$\Phi M_r/M_u = 1.18$ $\Phi V_r/V_{npr} = 3.8$ $P/A_g f'_c = 0.11$			1.19 1.16 0.52 1.17			1.45 3.2 0.19			1.45 3.2 0.19		
	1.18 3.8 0.11			1.19 1.16 0.52 1.17			1.45 3.2 0.19			1.45 3.2 0.19		
	1.18 3.8 0.11			1.19 1.16 0.52 1.17			1.45 3.2 0.19			1.45 3.2 0.19		
Floor 3												
1.53 1.20			1.19 1.16 0.52 1.17			1.36 1.06			1.36 1.06			
Story 2	$\Phi M_r/M_u = 1.23$ $\Phi V_r/V_{npr} = 3.15$ $P/A_g f'_c = 0.12$			1.27 0.52 1.83			1.35 3.01 0.21			1.35 3.01 0.21		
	1.23 3.15 0.12			1.27 0.52 1.83			1.35 3.01 0.21			1.35 3.01 0.21		
	1.23 3.15 0.12			1.27 0.52 1.83			1.35 3.01 0.21			1.35 3.01 0.21		
Floor 2												
2.54 1.42			1.20 1.27 0.52 1.83			1.88 1.25			1.88 1.25			
Story 1	$\Phi M_r/M_u = 1.19$ $\Phi V_r/V_{npr} = 2.36$ $P/A_g f'_c = 0.13$			$(\Phi M_r/M_u)_{neg} = 1.20$ $(\Phi M_r/M_u)_{pos} = 1.27$ $M_{u, pos}/M_{u, neg} = 0.52$ $\Phi V_r/V_{npr} = 1.83$			$(\Phi M_r/M_u)_{neg} = 1.22$ $(\Phi M_r/M_u)_{pos} = 1.42$ $M_{u, pos}/M_{u, neg} = 0.52$ $\Phi V_r/V_{npr} = 1.83$			$(\Phi M_r/M_u)_{neg} = 1.20$ $(\Phi M_r/M_u)_{pos} = 1.27$ $M_{u, pos}/M_{u, neg} = 0.52$ $\Phi V_r/V_{npr} = 1.83$		
	1.19 2.36 0.13			$(\Phi M_r/M_u)_{neg} = 1.20$ $(\Phi M_r/M_u)_{pos} = 1.27$ $M_{u, pos}/M_{u, neg} = 0.52$ $\Phi V_r/V_{npr} = 1.83$			$(\Phi M_r/M_u)_{neg} = 1.22$ $(\Phi M_r/M_u)_{pos} = 1.42$ $M_{u, pos}/M_{u, neg} = 0.52$ $\Phi V_r/V_{npr} = 1.83$			$(\Phi M_r/M_u)_{neg} = 1.20$ $(\Phi M_r/M_u)_{pos} = 1.27$ $M_{u, pos}/M_{u, neg} = 0.52$ $\Phi V_r/V_{npr} = 1.83$		
	1.19 2.36 0.13			$(\Phi M_r/M_u)_{neg} = 1.20$ $(\Phi M_r/M_u)_{pos} = 1.27$ $M_{u, pos}/M_{u, neg} = 0.52$ $\Phi V_r/V_{npr} = 1.83$			$(\Phi M_r/M_u)_{neg} = 1.22$ $(\Phi M_r/M_u)_{pos} = 1.42$ $M_{u, pos}/M_{u, neg} = 0.52$ $\Phi V_r/V_{npr} = 1.83$			$(\Phi M_r/M_u)_{neg} = 1.20$ $(\Phi M_r/M_u)_{pos} = 1.27$ $M_{u, pos}/M_{u, neg} = 0.52$ $\Phi V_r/V_{npr} = 1.83$		
Floor 1												
Design Drifts:												
1.4%												
1.9%												
2.4%												
2.7%												
2.9%												
3.1%												
3.4%												
3.5%												
3.6%												
3.6%												
3.2%												
1.8%												

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	4004	1789	3946	1789	3946	1789	3946	1789	3946
		E_{eff}/E_g =	0.35	-3746	0.35	-3746	0.35	-3746	0.35	-3746	0.35
		M_u/M_y =	1.19	4640	1.18	4640	1.18	4640	1.18	4640	1.19
		$\Theta_{cap,pl}$ (rad) =	0.070	0.069	0.069	0.069	0.069	0.069	0.069	0.069	0.070
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	116	115	115	115	115	115	115	115	116
Floor 12	Story 11	$(P/A)_g^* c_{l,exp}$ =	0.01	2043	0.01	2043	0.01	2043	0.01	2043	0.01
		$M_{y,exp}$ (k-in) =	3455	2357	4796	2357	4796	2357	4796	2357	3455
		E_{eff}/E_g =	0.35	-5253	0.35	-5253	0.35	-5253	0.35	-5253	0.35
		M_u/M_y =	1.18	5591	1.18	5591	1.18	5591	1.18	5591	1.18
		$\Theta_{cap,pl}$ (rad) =	0.068	0.067	0.067	0.067	0.067	0.067	0.067	0.067	0.068
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 11	Story 10	λ =	115	113	113	113	113	113	113	115	115
		$(P/A)_g^* c_{l,exp}$ =	0.01	2357	0.01	2357	0.01	2357	0.01	2357	0.01
		$M_{y,exp}$ (k-in) =	3886	2637	5624	2637	5624	2637	5624	2637	3886
		E_{eff}/E_g =	0.35	-5591	0.35	-5591	0.35	-5591	0.35	-5591	0.35
		M_u/M_y =	1.18	5591	1.18	5591	1.18	5591	1.18	5591	1.18
		$\Theta_{cap,pl}$ (rad) =	0.067	0.065	0.065	0.065	0.065	0.065	0.065	0.065	0.067
Floor 10	Story 9	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	114	111	111	111	111	111	111	111	114
		$(P/A)_g^* c_{l,exp}$ =	0.02	2637	0.04	2637	0.04	2637	0.04	2637	0.02
		$M_{y,exp}$ (k-in) =	4796	3284	6434	3284	6434	3284	6434	3284	4796
		E_{eff}/E_g =	0.35	-5591	0.35	-5591	0.35	-5591	0.35	-5591	0.35
		M_u/M_y =	1.18	5591	1.18	5591	1.18	5591	1.18	5591	1.18
Floor 9	Story 8	$\Theta_{cap,pl}$ (rad) =	0.067	0.064	0.064	0.064	0.064	0.064	0.064	0.067	0.067
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	113	109	109	109	109	109	109	109	113
		$(P/A)_g^* c_{l,exp}$ =	0.03	3191	0.05	3191	0.05	3191	0.05	3191	0.03
		$M_{y,exp}$ (k-in) =	5213	3673	7227	3673	7227	3673	7227	3673	5213
		E_{eff}/E_g =	0.35	-6555	0.35	-6555	0.35	-6555	0.35	-6555	0.35
Floor 8	Story 7	M_u/M_y =	1.18	6555	1.18	6555	1.18	6555	1.18	6555	1.18
		$\Theta_{cap,pl}$ (rad) =	0.066	0.062	0.062	0.062	0.062	0.062	0.062	0.062	0.066
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	112	107	107	107	107	107	107	107	112
		$(P/A)_g^* c_{l,exp}$ =	0.03	3284	0.07	3284	0.07	3284	0.07	3284	0.03
		$M_{y,exp}$ (k-in) =	5624	3673	8005	3673	8005	3673	8005	3673	5624
Floor 7	Story 6	E_{eff}/E_g =	0.35	-7106	0.35	-7106	0.35	-7106	0.35	-7106	0.35
		M_u/M_y =	1.18	6555	1.18	6555	1.18	6555	1.18	6555	1.18
		$\Theta_{cap,pl}$ (rad) =	0.065	0.061	0.061	0.061	0.061	0.061	0.061	0.061	0.065
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	111	106	106	106	106	106	106	106	111
		$(P/A)_g^* c_{l,exp}$ =	0.04	3673	0.08	3673	0.08	3673	0.08	3673	0.04
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	6031	3759	8770	3759	8770	3759	8770	3759	6031
		E_{eff}/E_g =	0.35	-7736	0.35	-7736	0.35	-7736	0.35	-7736	0.35
		M_u/M_y =	1.18	7736	1.17	7736	1.17	7736	1.17	7736	1.18
		$\Theta_{cap,pl}$ (rad) =	0.065	0.059	0.059	0.059	0.059	0.059	0.059	0.059	0.065
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	110	104	104	104	104	104	104	104	110
Floor 5	Story 4	$(P/A)_g^* c_{l,exp}$ =	0.05	3759	0.09	3759	0.09	3759	0.09	3759	0.05
		$M_{y,exp}$ (k-in) =	6434	3579	9523	3579	9523	3579	9523	3579	6434
		E_{eff}/E_g =	0.35	-7858	0.35	-7858	0.35	-7858	0.35	-7858	0.35
		M_u/M_y =	1.18	7858	1.17	7858	1.17	7858	1.17	7858	1.18
		$\Theta_{cap,pl}$ (rad) =	0.064	0.058	0.058	0.058	0.058	0.058	0.058	0.058	0.064
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 4	Story 3	λ =	109	103	102	102	102	102	102	109	109
		$(P/A)_g^* c_{l,exp}$ =	0.05	3945	0.11	3945	0.11	3945	0.11	3945	0.05
		$M_{y,exp}$ (k-in) =	6357	3579	8862	3579	8862	3579	8862	3579	6357
		E_{eff}/E_g =	0.35	-7857	0.35	-7857	0.35	-7857	0.35	-7857	0.35
		M_u/M_y =	1.18	7857	1.17	7857	1.17	7857	1.17	7857	1.18
		$\Theta_{cap,pl}$ (rad) =	0.063	0.055	0.055	0.055	0.055	0.055	0.055	0.055	0.063
Floor 3	Story 2	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	108	100	100	100	100	100	100	100	108
		$(P/A)_g^* c_{l,exp}$ =	0.06	3759	0.12	3759	0.12	3759	0.12	3759	0.06
		$M_{y,exp}$ (k-in) =	7227	3579	9596	3579	9596	3579	9596	3579	7227
		E_{eff}/E_g =	0.35	-7766	0.37	7766	0.37	7766	0.37	7766	0.35
		M_u/M_y =	1.18	7766	1.17	7766	1.17	7766	1.17	7766	1.18
Floor 2	Story 1	$\Theta_{cap,pl}$ (rad) =	0.062	0.054	0.054	0.054	0.054	0.054	0.054	0.062	0.063
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	107	99	99	99	99	99	99	99	107
		$(P/A)_g^* c_{l,exp}$ =	0.07	3579	0.13	3579	0.13	3579	0.13	3579	0.07
		$M_{y,exp}$ (k-in) =	9034	3055	10320	3055	10320	3055	10320	3055	9034
		E_{eff}/E_g =	0.35	-6776	0.38	6776	0.38	6776	0.38	6776	0.35
Floor 1	Story 1	M_u/M_y =	1.18	6776	1.17	6776	1.17	6776	1.17	6776	1.18
		$\Theta_{cap,pl}$ (rad) =	0.066	0.054	0.052	0.052	0.052	0.052	0.052	0.052	0.066
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	106	95	97	97	97	97	97	95	106
		$(P/A)_g^* c_{l,exp}$ =	0.08	3055	0.15	3055	0.15	3055	0.15	3055	0.08
		$M_{y,exp}$ (k-in) =	12713	12899	12899	12899	12899	12899	12899	12899	12713
Mass tributary to one frame for lateral load (each floor) (k-s/s-in): 0.60											
Model periods (sec): T ₁ = 2.64 T ₂ = 0.86 T ₃ = 0.49											
f _{y,triber,expected} = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2020

Number of Stories: 4

Fundamental Period (sec): 0.77

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2020 $R=5.33$

Compared to the baseline design, beam sizes were increased to meet the stronger lateral load, due to a lower R value. As in baseline design, joint shear controlled the beam depth more than drift. Both negative and positive flexural beam strength were controlled by strength demands. Few additional beam reinforcements in four bays were added to alter beam design to use same ρ and ρ_{Prime} for each level. Three bays had additional reinforcement added to meet the minimum positive/negative strength ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 2 by strength demand. All beam stirrups were controlled by the capacity shear design. Most column stirrups were controlled by the capacity shear design while one column was controlled by minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)									
Floor 5									
Story 4	h (in) =	30	28	30	0.005	0.0090	0.0023	6.4	
	b (in) =	30	28	30	0.005	0.0090	0.0023	6.4	
	ρ_{tot} =	0.011	0.014	0.0065	0.0053	0.0109	0.0026	6.4	
	ρ_{sh} =	0.0065	0.0065	0.0065	0.0065	0.0065	0.0065	0.0065	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Floor 4									
Story 3	h (in) =	30	34	30	0.0043	0.0088	0.0018	7.9	
	b (in) =	30	34	30	0.0043	0.0088	0.0018	7.9	
	ρ_{tot} =	0.015	0.020	0.0065	0.0043	0.0088	0.0018	7.9	
	ρ_{sh} =	0.0065	0.0065	0.0065	0.0065	0.0065	0.0065	0.0065	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Floor 3									
Story 2	h (in) =	30	34	30	0.0046	0.0091	0.0019	7.9	
	b (in) =	30	34	30	0.0046	0.0091	0.0019	7.9	
	ρ_{tot} =	0.015	0.020	0.0065	0.0046	0.0091	0.0019	7.9	
	ρ_{sh} =	0.0065	0.0065	0.0065	0.0065	0.0065	0.0065	0.0065	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Floor 2									
Story 1	h (in) =	30	34	30	0.0046	0.0091	0.0019	7.9	
	b (in) =	30	34	30	0.0046	0.0091	0.0019	7.9	
	ρ_{tot} =	0.017	0.020	0.0065	0.0046	0.0091	0.0019	7.9	
	ρ_{sh} =	0.0065	0.0065	0.0065	0.0065	0.0065	0.0065	0.0065	
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Grade beam column height (in) = 24 Basement column height (in) = 0									
30 feet									
13 feet									
15 feet									
Design base shear = 0.139 g, 290 k									
f'_c beams = 5.0 ksi					$f'_{c,cols,upper}$ = 5.0 ksi				
$f_{y,rebar,nom}$ = 60 ksi					$f'_{c,cols,lower}$ = 5.0 ksi				

Design Documentation (2 of 2)

		Design Drifts:			
Story 4	SCWB =	0.67	1.18	0.61	1.25
	Joint $\Phi V_r/V_u$ =	1.90		1.63	
	$\Phi M_r/M_u$ =	1.18		2.66	
	$\Phi V_r/V_{mgr}$ =	1.53		1.56	
Floor 4	$P/A_g f'_c$ =	0.03		0.06	
		1.37	1.15	1.34	1.18
		1.26	1.22	1.13	1.32
			0.50		0.50
Story 3	$\Phi M_r/M_u$ =	1.53		2.56	
	$\Phi V_r/V_{mgr}$ =	1.5		1.08	
	$P/A_g f'_c$ =	0.06		0.11	
		1.30	1.12	1.33	1.19
Floor 3		1.37	1.20	1.23	1.48
			0.50		0.50
			1.17		1.17
Story 2	$\Phi M_r/M_u$ =	1.35		2.08	
	$\Phi V_r/V_{mgr}$ =	1.31		0.94	
	$P/A_g f'_c$ =	0.09		0.16	
		1.35	1.13	1.37	1.22
Floor 2		1.26	1.20	1.23	1.59
			0.52		0.52
			1.16		1.16
Story 1	$\Phi M_r/M_u$ =	1.16		1.71	
	$\Phi V_r/V_{mgr}$ =	1.36		1.05	
	$P/A_g f'_c$ =	0.12		0.22	
		1.35	1.13	1.37	1.22
		1.26	1.20	1.23	1.59
			0.52		0.52
			1.16		1.16

Modeling Documentation (1 of 1)

		6084			
Floor 5	$M_{y,exp}$ (k-in) =	8752	-12381	0.35	2.128E+08
	$E I_{eff}/E I_g$ =	0.35		0.0355	
	M_c/M_y =	1.21		-0.053	
	$\Theta_{cap,pl}$ (rad) =	0.069		0.080	
Story 4	Θ_{pc} (rad) =	0.100		104	
	λ =	119			
	$(P/A_g f'_c)_{exp}$ =	0.02			
		6379	-14418	0.35	2.128E+08
Floor 4	$M_{y,exp}$ (k-in) =	12452		117731	
	$E I_{eff}/E I_g$ =	0.35		0.35	
	M_c/M_y =	1.20		1.20	
	$\Theta_{cap,pl}$ (rad) =	0.069		0.067	
Story 3	Θ_{pc} (rad) =	0.100		0.100	
	λ =	116		110	
	$(P/A_g f'_c)_{exp}$ =	0.04		0.08	
		7976	-17983	0.35	3.675E+08
Floor 3	$M_{y,exp}$ (k-in) =	13437		19617	
	$E I_{eff}/E I_g$ =	0.35		0.35	
	M_c/M_y =	1.20		1.19	
	$\Theta_{cap,pl}$ (rad) =	0.066		0.062	
Story 2	Θ_{pc} (rad) =	0.100		0.100	
	λ =	113		105	
	$(P/A_g f'_c)_{exp}$ =	0.06		0.12	
		8556	-18613	0.35	3.675E+08
Floor 2	$M_{y,exp}$ (k-in) =	15570		21461	
	$E I_{eff}/E I_g$ =	0.35		0.35	
	M_c/M_y =	1.20		1.19	
	$\Theta_{cap,pl}$ (rad) =	0.065		0.058	
Story 1	Θ_{pc} (rad) =	0.100		0.100	
	λ =	110		100	
	$(P/A_g f'_c)_{exp}$ =	0.08		0.15	
		8556	-18613	0.35	3.675E+08

Mass tributary to one frame for lateral load (each floor) (k-s/in): 1.34

Model periods (sec): $T_1 = 0.77$ $T_2 = 0.25$ $T_3 = 0.14$ $f_{y, \text{rebar, expected}} = 67$ ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2021

Number of Stories: 12

Fundamental Period (sec): 1.97

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2021 R=5.33

First from the baseline design, a larger lateral force was applied. After the preliminary analysis only joint shear was insufficient. After few iterations between sizes and column concrete strength, it was found that by using same geometry with a higher concrete strength was perfect for this design. Both positive and negative bending strengths were controlled by strength demand. Additional beam reinforcements were added to alter beam design to use same ρ and ρ_{Prime} in each floor and to meet the minimum positive/negative strength ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 2 by strength demand. Beam stirrups were controlled by both the capacity shear design and min reinforcement requirement. Column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

Floor 13											
Story 12	h (in) =	24	22	24	0.0032	0.0055	0.0034	4.9	22	24	0.0032
	b (in) =	24	22	24	0.0055	0.0034	4.9	22	24	0.0055	
	ρ_{tot} =	0.010	0.0077	0.0042	0.0083	0.0034	4.9	0.0077	0.0042	0.0083	
	ρ_{sh} =	0.0077	0.0042	0.0083	0.0034	4.9	0.0077	0.0042	0.0083	0.0034	
Floor 12											
Story 11	h (in) =	24	22	24	0.0042	0.0083	0.0034	4.9	22	24	0.0042
	b (in) =	24	22	24	0.0083	0.0034	4.9	22	24	0.0083	
	ρ_{tot} =	0.010	0.0077	0.0047	0.0093	0.0034	4.9	0.0077	0.0047	0.0093	
	ρ_{sh} =	0.0077	0.0047	0.0093	0.0034	4.9	0.0077	0.0047	0.0093	0.0034	
Floor 11											
Story 10	h (in) =	24	22	24	0.0047	0.0093	0.0034	4.9	22	24	0.0047
	b (in) =	24	22	24	0.0093	0.0034	4.9	22	24	0.0093	
	ρ_{tot} =	0.010	0.0077	0.0055	0.0108	0.0034	4.9	0.0077	0.0055	0.0108	
	ρ_{sh} =	0.0077	0.0055	0.0108	0.0034	4.9	0.0077	0.0055	0.0108	0.0034	
Floor 10											
Story 9	h (in) =	24	22	24	0.0055	0.0108	0.0034	4.9	22	24	0.0055
	b (in) =	24	22	24	0.0108	0.0034	4.9	22	24	0.0108	
	ρ_{tot} =	0.011	0.0077	0.0059	0.0118	0.0034	4.9	0.0077	0.0059	0.0118	
	ρ_{sh} =	0.0077	0.0059	0.0118	0.0034	4.9	0.0077	0.0059	0.0118	0.0034	
Floor 9											
Story 8	h (in) =	24	22	24	0.0059	0.0118	0.0034	4.9	22	24	0.0059
	b (in) =	24	22	24	0.0118	0.0034	4.9	22	24	0.0118	
	ρ_{tot} =	0.011	0.0077	0.0066	0.012	0.0034	4.9	0.0077	0.0066	0.012	
	ρ_{sh} =	0.0077	0.0066	0.012	0.0034	4.9	0.0077	0.0066	0.012	0.0034	
Floor 8											
Story 7	h (in) =	24	22	24	0.0066	0.012	0.0034	4.9	22	24	0.0066
	b (in) =	24	22	24	0.012	0.0034	4.9	22	24	0.012	
	ρ_{tot} =	0.015	0.0077	0.0045	0.0090	0.005	6.4	0.0077	0.0045	0.0090	
	ρ_{sh} =	0.0077	0.0045	0.0090	0.005	6.4	0.0077	0.0045	0.0090	0.005	
Floor 7											
Story 6	h (in) =	24	28	24	0.0045	0.0090	0.005	6.4	28	24	0.0045
	b (in) =	24	28	24	0.0090	0.005	6.4	28	24	0.0090	
	ρ_{tot} =	0.013	0.0077	0.0045	0.0088	0.0049	6.4	0.0077	0.0045	0.0088	
	ρ_{sh} =	0.0077	0.0045	0.0088	0.0049	6.4	0.0077	0.0045	0.0088	0.0049	
Floor 6											
Story 5	h (in) =	24	28	24	0.0045	0.0088	0.0049	6.4	28	24	0.0045
	b (in) =	24	28	24	0.0088	0.0049	6.4	28	24	0.0088	
	ρ_{tot} =	0.013	0.0077	0.0047	0.0088	0.005	6.4	0.0077	0.0047	0.0088	
	ρ_{sh} =	0.0077	0.0047	0.0088	0.005	6.4	0.0077	0.0047	0.0088	0.005	
Floor 5											
Story 4	h (in) =	24	28	24	0.0047	0.0088	0.005	6.4	28	24	0.0047
	b (in) =	24	28	24	0.0088	0.005	6.4	28	24	0.0088	
	ρ_{tot} =	0.013	0.0077	0.0047	0.0088	0.005	6.4	0.0077	0.0047	0.0088	
	ρ_{sh} =	0.0077	0.0047	0.0088	0.005	6.4	0.0077	0.0047	0.0088	0.005	
Floor 4											
Story 3	h (in) =	24	28	24	0.0047	0.0088	0.005	6.4	28	24	0.0047
	b (in) =	24	28	24	0.0088	0.005	6.4	28	24	0.0088	
	ρ_{tot} =	0.013	0.0077	0.0047	0.0088	0.005	6.4	0.0077	0.0047	0.0088	
	ρ_{sh} =	0.0077	0.0047	0.0088	0.005	6.4	0.0077	0.0047	0.0088	0.005	
Floor 3											
Story 2	h (in) =	24	28	24	0.0056	0.0096	0.0051	6.4	28	24	0.0056
	b (in) =	24	28	24	0.0096	0.0051	6.4	28	24	0.0096	
	ρ_{tot} =	0.013	0.0077	0.0056	0.0096	0.0051	6.4	0.0077	0.0056	0.0096	
	ρ_{sh} =	0.0077	0.0056	0.0096	0.0051	6.4	0.0077	0.0056	0.0096	0.0051	
Floor 2											
Story 1	h (in) =	24	28	24	0.0056	0.0096	0.0051	6.4	28	24	0.0056
	b (in) =	24	28	24	0.0096	0.0051	6.4	28	24	0.0096	
	ρ_{tot} =	0.014	0.0077	0.0056	0.0096	0.0051	6.4	0.0077	0.0056	0.0096	
	ρ_{sh} =	0.0077	0.0056	0.0096	0.0051	6.4	0.0077	0.0056	0.0096	0.0051	
Basement											
Grade beam column height (in) = 24											
Basement column height (in) = 24											
20 feet											
13 feet											
15 feet											
Design base shear = 0.053 g, 232 k											
f'_c beams = 5.0 ksi f'_c cols upper = 6.0 ksi											
f_y rebar nom. = 60 ksi f_y cols lower = 6.0 ksi											

Page 139 of 240

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	3807	1804	7.428E+07	4176	1804	7.428E+07	4176	1804	3807
		E_{eff}/E_g =	0.35	-4356	0.35	0.35	-4356	0.35	0.35	-4356	0.35
Floor 12	Story 11	$M_{y,exp}$ (k-in) =	4176	2351	7.428E+07	5513	2351	7.428E+07	5513	2351	4176
		E_{eff}/E_g =	0.35	-5808	0.35	0.35	-5808	0.35	0.35	-5808	0.35
Floor 11	Story 10	$M_{y,exp}$ (k-in) =	4176	2624	7.428E+07	5513	2624	7.428E+07	5513	2624	4176
		E_{eff}/E_g =	0.35	-6332	0.35	0.35	-6332	0.35	0.35	-6332	0.35
Floor 10	Story 9	$M_{y,exp}$ (k-in) =	4176	3076	7.428E+07	5513	3076	7.428E+07	5513	3076	4176
		E_{eff}/E_g =	0.35	-7116	0.35	0.35	-7116	0.35	0.35	-7116	0.35
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	4176	3316	7.428E+07	5513	3316	7.428E+07	5513	3316	4176
		E_{eff}/E_g =	0.35	-7635	0.35	0.35	-7635	0.35	0.35	-7635	0.35
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	4176	3329	7.428E+07	5513	3329	7.428E+07	5513	3329	4176
		E_{eff}/E_g =	0.35	-7765	0.35	0.35	-7765	0.35	0.35	-7765	0.35
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	4176	3439	7.428E+07	5513	3439	7.428E+07	5513	3439	4176
		E_{eff}/E_g =	0.35	-10197	0.35	0.35	-10197	0.35	0.35	-10197	0.35
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	4176	3449	7.428E+07	5513	3449	7.428E+07	5513	3449	4176
		E_{eff}/E_g =	0.35	-9972	0.35	0.35	-9972	0.35	0.35	-9972	0.35
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	4176	3455	7.428E+07	5513	3455	7.428E+07	5513	3455	4176
		E_{eff}/E_g =	0.35	-10051	0.35	0.35	-10051	0.35	0.35	-10051	0.35
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	4176	3455	7.428E+07	5513	3455	7.428E+07	5513	3455	4176
		E_{eff}/E_g =	0.35	-10051	0.35	0.35	-10051	0.35	0.35	-10051	0.35
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	4176	3455	7.428E+07	5513	3455	7.428E+07	5513	3455	4176
		E_{eff}/E_g =	0.35	-10051	0.35	0.35	-10051	0.35	0.35	-10051	0.35
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	4176	3455	7.428E+07	5513	3455	7.428E+07	5513	3455	4176
		E_{eff}/E_g =	0.35	-10051	0.35	0.35	-10051	0.35	0.35	-10051	0.35
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60											
Model periods (sec): $T_1 = 1.97$ $T_2 = 0.67$ $T_3 = 0.38$											
$f_{steel, expected} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2022

Number of Stories: 4

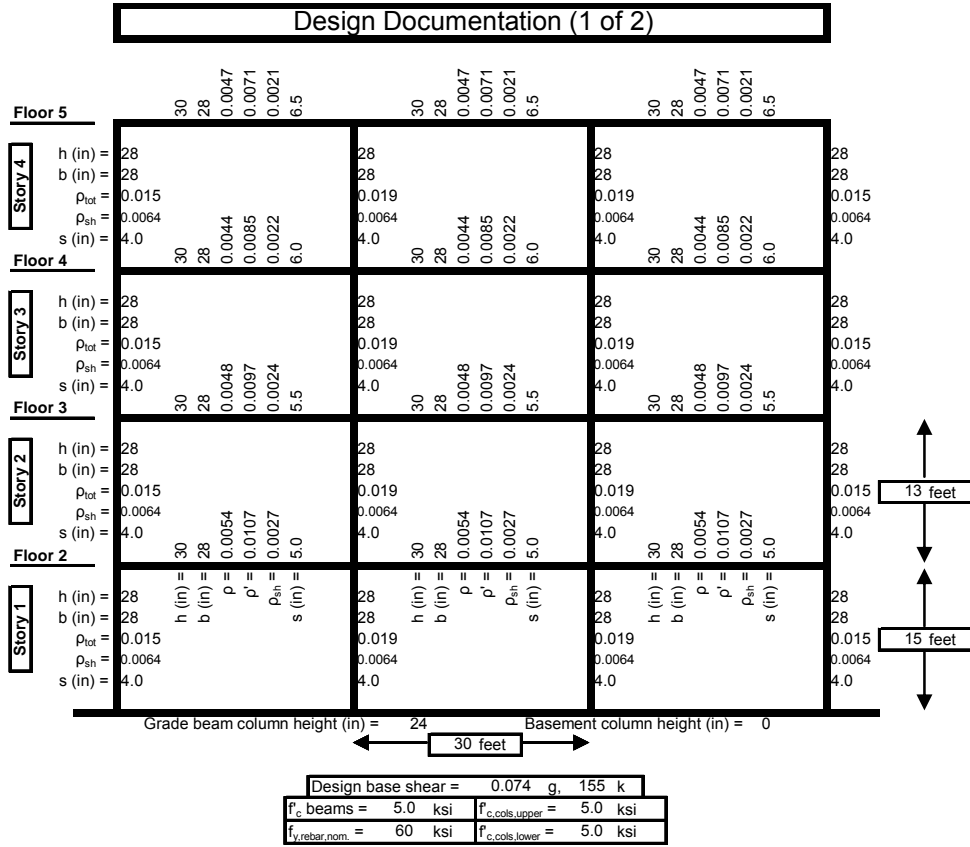
Fundamental Period (sec): 0.91

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2022 R=10

This design was an add-on after the majority of the design—as it can be seen from the design number 2022. In this design, beam and column sizes were controlled by joint shear demand, where drift was only at 1.27%. Both negative and positive flexural beam strength was controlled by strength demands. More reinforcements were added to beams in four bays in order make consistent reinforcement for each floor. Six beams had additional reinforcement added to meet the minimum positive and negative ratio. All column flexural strengths were controlled by the strong-column weak-beam ratio. All beam stirrups were controlled by the capacity shear design. All of column stirrups were controlled by minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)									
Story 4	SCWB =	0.81	1.13	1.10	0.67	1.16	0.67	1.13	0.81
	Joint $\Phi V_r/V_u =$	2.22			1.78		1.78		2.22
	$\Phi M_r/M_u =$	1.65			4.95		4.98		1.65
	$\Phi V_r/V_{mgr} =$	1.22			1.3		1.3		1.22
Floor 4	$P/A_g f'_c =$	0.03			0.07		0.07		0.03
		1.44	1.12	1.12	1.32	1.16	1.32	1.12	1.44
		1.44			1.26		1.26		1.44
Story 3	$\Phi M_r/M_u =$	1.82			4.47		4.49		1.82
	$\Phi V_r/V_{mgr} =$	1.56			1.17		1.17		1.56
	$P/A_g f'_c =$	0.06			0.12		0.12		0.06
		1.36	1.14	1.16	1.31	1.19	1.31	1.14	1.36
Floor 3		1.25			1.12		1.12		1.25
Story 2	$\Phi M_r/M_u =$	1.56			3.07		3.08		1.56
	$\Phi V_r/V_{mgr} =$	1.43			1.05		1.05		1.43
	$P/A_g f'_c =$	0.09			0.19		0.19		0.09
		1.31	1.14	1.18	1.31	1.22	1.31	1.14	1.31
Floor 2		1.10			0.97		0.97		1.10
Story 1	$\Phi M_r/M_u =$	2.17			3.40		3.41		2.18
	$\Phi V_r/V_{mgr} =$	1.57			1.15		1.15		1.57
	$P/A_g f'_c =$	0.13			0.26		0.26		0.13

Design Drifts:

0.5%

0.8%

1.1%

1.3%

Modeling Documentation (1 of 1)												
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	9302									
		E_{lag}/E_{lg} =	0.35	6205	-11062	0.35	2.527E+08	0.0321	0.046	0.075	99	
		M_c/M_y =	1.21									
		$\Theta_{cap,pl}$ (rad) =	0.071									
Floor 4	Story 3	Θ_{pc} (rad) =	0.100									
		λ =	116	5795	-12722	0.35	2.527E+08	0.033	-0.051	0.080	103	
		$(P/A_g f'_c)_{exp}$ =	0.02									
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	10233									
		E_{lag}/E_{lg} =	0.35	6246	-14259	0.35	2.527E+08	0.036	-0.056	0.086	108	
		M_c/M_y =	1.20									
		$\Theta_{cap,pl}$ (rad) =	0.068									
Floor 2	Story 1	Θ_{pc} (rad) =	0.100									
		λ =	112	7029	-15492	0.35	2.527E+08	0.039	-0.060	0.093	112	
		$(P/A_g f'_c)_{exp}$ =	0.07									
Floor 1	Story 1	$M_{y,exp}$ (k-in) =	12101									
		E_{lag}/E_{lg} =	0.35	6205	-11062	0.35	2.527E+08	0.0321	0.046	0.075	99	
		M_c/M_y =	1.20									
		$\Theta_{cap,pl}$ (rad) =	0.070									
Floor 5	Story 4	Θ_{pc} (rad) =	0.100									
		λ =	112	5795	-12722	0.35	2.527E+08	0.033	-0.051	0.080	103	
		$(P/A_g f'_c)_{exp}$ =	0.04									
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	13898									
		E_{lag}/E_{lg} =	0.35	6246	-14259	0.35	2.527E+08	0.036	-0.056	0.086	108	
		M_c/M_y =	1.20									
		$\Theta_{cap,pl}$ (rad) =	0.064									
Floor 3	Story 2	Θ_{pc} (rad) =	0.100									
		λ =	106	7029	-15492	0.35	2.527E+08	0.039	-0.060	0.093	112	
		$(P/A_g f'_c)_{exp}$ =	0.09									
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	15642									
		E_{lag}/E_{lg} =	0.35	6246	-14259	0.35	2.527E+08	0.036	-0.056	0.086	108	
		M_c/M_y =	1.19									
		$\Theta_{cap,pl}$ (rad) =	0.059									
Floor 1	Story 1	Θ_{pc} (rad) =	0.100									
		λ =	100	7029	-15492	0.35	2.527E+08	0.039	-0.060	0.093	112	
		$(P/A_g f'_c)_{exp}$ =	0.13									

Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.34

Model periods (sec): $T_1 = 0.91$ $T_2 = 0.29$ $T_3 = 0.16$

$f_{y, \text{rebar, expected}} = 67$ ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2023

Number of Stories: 4

Fundamental Period (sec): 0.86

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2023 SCWB=1.0

From the baseline design to satisfy a lower SCWB ratio wasn't hard. Keeping the same sizes by simply reducing column reinforcement were able to get us sufficient SCWB. Like the baseline design, beam depths were controlled by joint shear. Both negative and positive flexural beam strength was controlled by strength demands. About six bays have reinforcement added to keep reinforcement constant for each level, and to meet the positive/negative strength ratio. All column flexural strengths were controlled by the strong-column weak-beam ratio, except two. 3 beam stirrups were controlled by the capacity shear design except 6 by min requirement. Column stirrups were controlled by minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

Floor 5

24300.00720.01170.00325.424300.00720.01170.00325.424300.00720.01170.00325.4

Story 4

h (in) = 30

b (in) = 30

ρ_{tot} = 0.011

ρ_{sh} = 0.0065

s (in) = 4.0

30

30

0.012

0.0065

4.0

30

30

0.0069

0.0134

0.0034

5.4

30

30

0.012

0.0065

4.0

30

30

0.0069

0.0134

0.0034

5.4

30

30

0.012

0.0065

4.0

30

30

0.0069

0.0134

0.0034

5.4

30

30

0.012

0.0065

4.0

30

30

0.0069

0.0134

0.0034

5.4

Floor 4

24300.00690.01340.00345.424300.00690.01340.00345.424300.00690.01340.00345.4

Story 3

h (in) = 30

b (in) = 30

ρ_{tot} = 0.011

ρ_{sh} = 0.0065

s (in) = 4.0

30

30

0.012

0.0065

4.0

30

30

0.005

0.0102

0.0023

6.9

30

30

0.005

0.0102

0.0023

6.9

30

30

0.012

0.0065

4.0

30

30

0.005

0.0102

0.0023

6.9

30

30

0.012

0.0065

4.0

30

30

0.005

0.0102

0.0023

6.9

Floor 3

30300.0050.01020.00236.930300.0050.01020.00236.930300.0050.01020.00236.9

Story 2

h (in) = 30

b (in) = 30

ρ_{tot} = 0.011

ρ_{sh} = 0.0065

s (in) = 4.0

30

30

0.012

0.0065

4.0

30

30

0.0055

0.0112

0.0025

6.9

30

30

0.0055

0.0112

0.0025

6.9

30

30

0.012

0.0065

4.0

30

30

0.0055

0.0112

0.0025

6.9

30

30

0.012

0.0065

4.0

30

30

0.0055

0.0112

0.0025

6.9

Floor 2

30300.00550.01120.00256.930300.00550.01120.00256.930300.00550.01120.00256.9

Story 1

h (in) = 30

b (in) = 30

ρ_{tot} = 0.011

ρ_{sh} = 0.0065

s (in) = 4.0

30

30

0.012

0.0065

4.0

30

30

0.0055

0.0112

0.0025

6.9

30

30

0.0055

0.0112

0.0025

6.9

30

30

0.012

0.0065

4.0

30

30

0.0055

0.0112

0.0025

6.9

30

30

0.012

0.0065

4.0

30

30

0.0055

0.0112

0.0025

6.9

Grade beam column height (in) = 24

Basement column height (in) = 0

30 feet

13 feet

15 feet

Design base shear = 0.092 g, 193 k	
f'_c beams = 5.0 ksi	f'_c cols upper = 5.0 ksi
f_y rebar nom. = 60 ksi	f'_c cols lower = 5.0 ksi

Design Documentation (2 of 2)

Story 4	SCWB =	0.74	1.17	1.16	0.63	1.16	0.57	1.23	1.27	1.16	0.57	1.17	0.63	1.16	0.74
	Joint $\Phi V_r/V_u =$	1.57					1.30				1.30				1.57
Story 4	$\Phi M_r/M_u =$	1.17					2.99				3.00				1.17
	$\Phi V_r/V_{mgr} =$	1.6					1.81				1.81				1.6
Floor 4	$P/A_g f'_c =$	0.03					0.06				0.06				0.03
		1.36	1.16	1.17	0.53	1.16	1.15	1.18	1.21	0.53	1.15	1.16	0.53	1.16	1.36
Story 3	$\Phi M_r/M_u =$	1.37					2.55				2.57				1.39
	$\Phi V_r/V_{mgr} =$	1.99					1.57				1.57				1.99
Floor 3	$P/A_g f'_c =$	0.05					0.11				0.11				0.05
		1.13	1.17	1.24	0.50	1.17	1.04	1.21	1.43	0.50	1.04	1.17	1.24	0.50	1.13
Story 2	$\Phi M_r/M_u =$	1.34					2.18				2.18				1.35
	$\Phi V_r/V_{mgr} =$	1.7					1.3				1.3				1.7
Floor 2	$P/A_g f'_c =$	0.08					0.16				0.16				0.08
		1.09	1.17	1.24	0.50	1.16	1.07	1.25	1.52	0.50	1.07	1.18	1.24	0.50	1.09
Story 1	$\Phi M_r/M_u =$	1.71					2.57				2.57				1.72
	$\Phi V_r/V_{mgr} =$	1.85					1.39				1.39				1.85
Floor 1	$P/A_g f'_c =$	0.11					0.22				0.22				0.11
		1.12	$(\Phi M_r/M_u)_{neg} = 1.17$ $(\Phi M_r/M_u)_{pos} = 1.24$ $M_{n, pos}/M_{n, neg} = 0.50$ $\Phi V_r/V_{mgr} = 1.16$				1.00	$(\Phi M_r/M_u)_{neg} = 1.25$ $(\Phi M_r/M_u)_{pos} = 1.52$ $M_{n, pos}/M_{n, neg} = 0.50$ $\Phi V_r/V_{mgr} = 1.16$			1.00	$(\Phi M_r/M_u)_{neg} = 1.18$ $(\Phi M_r/M_u)_{pos} = 1.24$ $M_{n, pos}/M_{n, neg} = 0.50$ $\Phi V_r/V_{mgr} = 1.16$			1.12

Design Drifts:

0.8%

1.0%

1.2%

1.4%

Modeling Documentation (1 of 1)

Floor 5	Story 4	$M_{y,exp}$ (k-in) =	3583	6062	-11077	0.35	1.354E+08	0.0452	-0.064	0.100	100	6062	-11077	0.35	1.354E+08	0.0452	-0.064	0.100	100	6062	-11077	0.35	1.354E+08	0.0452	-0.064	0.100	100
		E_{slg}/EI_g =	0.35									0.35								0.35							0.35
Floor 4	Story 3	M_c/M_y =	1.21									1.20								1.21							1.20
		$\Theta_{cap,pl}$ (rad) =	0.069									0.067								0.069							0.067
Floor 3	Story 2	Θ_{pc} (rad) =	0.100									0.100								0.100							0.100
		λ =	119									116								119							116
Floor 2	Story 1	$(P/A_g f'_c)_{exp}$ =	0.02	5856	-12453	0.35	1.354E+08	0.045	-0.068	0.100	100	0.04	5856	-12453	0.35	1.354E+08	0.045	-0.068	0.100	0.04	5856	-12453	0.35	1.354E+08	0.045	-0.068	0.100
		$M_{y,exp}$ (k-in) =	9602									12412								9602							12412
Floor 1	Story 1	E_{slg}/EI_g =	0.35									0.35								0.35							0.35
		M_c/M_y =	1.20									1.20								1.20							1.20
Floor 5	Story 4	$\Theta_{cap,pl}$ (rad) =	0.066									0.063								0.066							0.063
		Θ_{pc} (rad) =	0.100									0.100								0.100							0.100
Floor 4	Story 3	λ =	116									110								116							110
		$(P/A_g f'_c)_{exp}$ =	0.04	7024	-15861	0.35	2.591E+08	0.035	-0.055	0.082	108	0.08	7024	-15861	0.35	2.591E+08	0.035	-0.055	0.082	0.08	7024	-15861	0.35	2.591E+08	0.035	-0.055	0.082
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	10601									14317								10601							14317
		E_{slg}/EI_g =	0.35									0.35								0.35							0.35
Floor 2	Story 1	M_c/M_y =	1.20									1.19								1.20							1.19
		$\Theta_{cap,pl}$ (rad) =	0.064									0.058								0.064							0.058
Floor 1	Story 1	Θ_{pc} (rad) =	0.100									0.100								0.100							0.100
		λ =	113									105								113							105
Floor 5	Story 4	$(P/A_g f'_c)_{exp}$ =	0.06	7712	-17177	0.35	2.591E+08	0.039	-0.059	0.088	112	0.12	7712	-17177	0.35	2.591E+08	0.039	-0.059	0.088	0.12	7712	-17177	0.35	2.591E+08	0.039	-0.059	0.088
		$M_{y,exp}$ (k-in) =	11580									16168								11580							16168
Floor 4	Story 3	E_{slg}/EI_g =	0.35									0.35								0.35							0.35
		M_c/M_y =	1.20									1.19								1.20							1.19
Floor 3	Story 2	$\Theta_{cap,pl}$ (rad) =	0.062									0.054								0.062							0.054
		Θ_{pc} (rad) =	0.100									0.100								0.100							0.100
Floor 2	Story 1	λ =	110									100								110							100
		$(P/A_g f'_c)_{exp}$ =	0.08	$M_{y,pos,exp}$ (k-in) =	$M_{y,slab,exp}$ (k-in) =	E_{slg}/EI_g =	$\Theta_{cap,pl,pos}$ (rad) =	$\Theta_{cap,pl,neg}$ (rad) =	Θ_{pc} (rad) =	λ =		$M_{y,pos,exp}$ (k-in) =	$M_{y,slab,exp}$ (k-in) =	E_{slg}/EI_g =	$\Theta_{cap,pl,pos}$ (rad) =	$\Theta_{cap,pl,neg}$ (rad) =	Θ_{pc} (rad) =	λ =		$M_{y,pos,exp}$ (k-in) =	$M_{y,slab,exp}$ (k-in) =	E_{slg}/EI_g =	$\Theta_{cap,pl,pos}$ (rad) =	$\Theta_{cap,pl,neg}$ (rad) =	Θ_{pc} (rad) =	λ =	

Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.34

Model periods (sec): $T_1 = 0.85$ $T_2 = 0.27$ $T_3 = 0.15$ $f_{y, rebar, expected} = 67$ ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2024

Number of Stories: 4

Fundamental Period (sec): 0.85

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2024 SCWB=0.8

From the 2023 design to satisfy a lower SCWB, columns were made smaller. Again, beam depths were controlled by joint shear. Both negative and positive flexural beam strength was controlled by strength demands. Few bays have reinforcement added to keep reinforcement constant for each level, and to meet the positive/negative strength ratio. All column flexural strengths were controlled by the strong-column weak-beam ratio, except for two. Half of beam stirrups were controlled by the capacity shear design, half by minimum requirement. All column stirrups were controlled by minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

[illegible]

Design Documentation (2 of 2)									
Story 4	SCWB =	0.52	1.18	0.40	1.26	0.40	1.18	0.52	Design Drifts:
	Joint $\Phi V_r/V_u$ =	2.52		2.01		2.01		2.52	
	$\Phi M_r/M_u$ =	1.16		2.88		2.90		1.17	
	$\Phi V_r/V_{mgr}$ =	1.66		1.85		1.85		1.66	
Floor 4	$P/A_g f'_c$ =	0.03		0.07		0.07		0.03	0.5%
		0.90	1.18	0.79	1.22	0.79	1.18	0.90	
		1.49	1.20	1.31	1.32	1.31	1.20	1.49	
			0.52		0.52		0.52		
Story 3	$\Phi M_r/M_u$ =	1.20		2.40		2.41		1.20	0.8%
	$\Phi V_r/V_{mgr}$ =	2.02		1.57		1.57		2.02	
	$P/A_g f'_c$ =	0.06		0.12		0.12		0.06	
		0.91	1.16	0.82	1.22	0.82	1.17	0.91	
Floor 3		1.30	1.26	1.15	1.51	1.15	1.26	1.30	1.1%
			0.53		0.53		0.53		
			1.18		1.18		1.18		
Story 2	$\Phi M_r/M_u$ =	1.16		1.78		1.79		1.16	1.4%
	$\Phi V_r/V_{mgr}$ =	1.67		1.34		1.34		1.67	
	$P/A_g f'_c$ =	0.10		0.19		0.19		0.10	
		0.86	1.18	0.83	1.28	0.83	1.18	0.86	
Floor 2		1.09	1.23	0.97	1.82	0.97	1.23	1.09	
			0.51		0.51		0.51		
			1.16		1.16		1.16		
Story 1	$\Phi M_r/M_u$ =	1.36		2.11		2.11		1.36	1.4%
	$\Phi V_r/V_{mgr}$ =	1.92		1.42		1.42		1.92	
	$P/A_g f'_c$ =	0.13		0.26		0.26		0.13	

Modeling Documentation (1 of 1)									
Floor 5	$M_{y,exp}$ (k-in) =	6607	6773	-11985	3.299E+08	0.028	-0.04	0.068	97
	$E I_{eff}/E I_g$ =	0.35							
	M_c/M_y =	1.21							
	$\Theta_{cap,pl}$ (rad) =	0.068							
Floor 4	Θ_{pc} (rad) =	0.100							
	λ =	116	6379	-14270	3.299E+08	0.027	-0.042	0.068	97
	$(P/A_g f'_c)_{exp}$ =	0.02							
Floor 3	$M_{y,exp}$ (k-in) =	7556	7292	-15912	3.299E+08	0.029	-0.045	0.077	97
	$E I_{eff}/E I_g$ =	0.35							
	M_c/M_y =	1.20							
	$\Theta_{cap,pl}$ (rad) =	0.065							
Floor 2	Θ_{pc} (rad) =	0.100							
	λ =	112	7292	-15912	3.299E+08	0.029	-0.045	0.077	97
	$(P/A_g f'_c)_{exp}$ =	0.04							
Story 1	$M_{y,exp}$ (k-in) =	9149	8088	-17811	3.299E+08	0.031	-0.047	0.084	97
	$E I_{eff}/E I_g$ =	0.35							
	M_c/M_y =	1.20							
	$\Theta_{cap,pl}$ (rad) =	0.063							
Story 2	Θ_{pc} (rad) =	0.100							
	λ =	109	8088	-17811	3.299E+08	0.031	-0.047	0.084	97
	$(P/A_g f'_c)_{exp}$ =	0.07							
Story 3	$M_{y,exp}$ (k-in) =	9389	8088	-17811	3.299E+08	0.031	-0.047	0.084	97
	$E I_{eff}/E I_g$ =	0.35							
	M_c/M_y =	1.20							
	$\Theta_{cap,pl}$ (rad) =	0.061							
Story 4	Θ_{pc} (rad) =	0.100							
	λ =	106	8088	-17811	3.299E+08	0.031	-0.047	0.084	97
	$(P/A_g f'_c)_{exp}$ =	0.09							

Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.34

Model periods (sec): $T_1 = 0.85$ $T_2 = 0.27$ $T_3 = 0.15$

f_y rebar, expected = 67 ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2025

Number of Stories: 4

Fundamental Period (sec): 0.87

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2025 SCWB=0.6

Simply making the columns sizes smaller and making beams larger (columns 26", beams 34") weren't enough to satisfy low SCWB and sufficient joint shear force. Hence, strong concrete strength were use to meet the joint shear strength. Both negative and positive flexural beam strength was controlled by strength demands. Few bays have reinforcement added to keep reinforcement constant for each level, and to meet the positive/negative requirement ratio. Exterior column flexural strengths were controlled by the flexural demand, where as the interior columns by the strong-column weak-beam ratio. Most of beam stirrups were controlled by the capacity shear design, rest by minimum requirement. All column stirrups were controlled by minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)											
Floor 5											
Story 4	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013
	ρ_{sh} =	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Floor 4											
Story 3	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013
	ρ_{sh} =	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Floor 3											
Story 2	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{sh} =	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Floor 2											
Story 1	h (in) =	26	26	26	26	26	26	26	26	26	26
	b (in) =	26	26	26	26	26	26	26	26	26	26
	ρ_{tot} =	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
	ρ_{sh} =	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089	0.0089
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Grade beam column height (in) = 24 Basement column height (in) = 0											
30 feet											
<div> <div>Design base shear = 0.092 g, 193 k</div> <div> <div>f'_c beams = 5.1 ksi</div> <div>f'_c cols upper = 7.0 ksi</div> </div> <div> <div>f_y rebar nom = 60 ksi</div> <div>f'_c cols lower = 7.0 ksi</div> </div> </div>											

Design Documentation (2 of 2)

		SCWB =	0.48	1.16	0.31	1.23	0.31	1.16	0.48	Design Drifts:
		Joint $\Phi V_r/V_u$	2.77	2.21	2.21	2.21	2.21	2.77		
Story 4	$\Phi M_r/M_u$	=	1.15	2.45	2.46	1.16	1.15	2.46	1.16	0.6%
	$\Phi V_r/V_{mpr}$	=	2.11	2.65	2.65	2.11	2.11	2.65	2.11	
	$P/A_g f'_c$	=	0.03	0.06	0.06	0.03	0.03	0.06	0.03	
Floor 4			0.86	1.15	0.61	1.20	0.61	1.15	0.86	0.9%
Story 3	$\Phi M_r/M_u$	=	1.60	1.39	1.39	1.60	1.39	1.60	1.60	
	$\Phi V_r/V_{mpr}$	=	1.15	1.80	1.80	1.16	1.15	1.80	1.16	
	$\Phi V_r/V_{mpr}$	=	1.73	2.17	2.17	1.73	1.73	2.17	1.73	
$P/A_g f'_c$	=	0.05	0.10	0.10	0.05	0.05	0.10	0.05		
Floor 3			0.90	1.13	0.66	1.20	0.66	1.13	0.90	1.2%
Story 2	$\Phi M_r/M_u$	=	1.41	1.24	1.24	1.41	1.24	1.41	1.41	
	$\Phi V_r/V_{mpr}$	=	1.16	1.41	1.42	1.16	1.16	1.41	1.16	
	$\Phi V_r/V_{mpr}$	=	1.93	1.81	1.81	1.93	1.93	1.81	1.93	
$P/A_g f'_c$	=	0.08	0.16	0.16	0.08	0.08	0.16	0.08		
Floor 2			0.80	1.18	0.67	1.25	0.67	1.18	0.80	1.6%
Story 1	$\Phi M_r/M_u$	=	1.18	1.03	1.03	1.18	1.03	1.18	1.18	
	$\Phi V_r/V_{mpr}$	=	1.16	1.71	1.72	1.15	1.16	1.71	1.15	
	$\Phi V_r/V_{mpr}$	=	2.46	1.9	1.9	2.49	2.46	1.9	2.49	
$P/A_g f'_c$	=	0.11	0.21	0.21	0.11	0.11	0.21	0.11		

Modeling Documentation (1 of 1)

		6727	-12002	3.823E+08	0.0272	-0.039	0.070	99	6713	-11969	3.490E+08	0.0284	-0.04	0.070	99	6713	-11969	3.724E+08	0.0284	-0.04	0.070	99
Floor 5		$M_{y,exp}$ (k-in) =	6117	6259	6117	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	M_z/M_y =	1.19	1.19	1.19	1.19	$\Theta_{cap,pl}$ (rad) =	0.068	0.064	0.068	0.068	0.068	
Story 4	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	λ =	102	100	102	100	$(P/A_g f'_c)_{exp}$ =	0.02	0.04	0.04	0.02	$M_{y,exp}$ (k-in) =	7314	7971	7314	7971	7314	
	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	M_z/M_y =	1.19	1.18	1.19	1.18	$\Theta_{cap,pl}$ (rad) =	0.066	0.060	0.060	0.066	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	
	λ =	100	95	100	95	$(P/A_g f'_c)_{exp}$ =	0.04	0.07	0.07	0.04	$M_{y,exp}$ (k-in) =	9010	9615	9010	9615	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	0.35	
Floor 4		6717	-14239	3.490E+08	-0.046	0.076	102	100	6717	-14239	3.490E+08	0.030	-0.046	0.076	102	6717	-14239	3.490E+08	0.030	-0.046	0.076	102
Story 3	$\Theta_{cap,pl}$ (rad) =	0.100	0.100	0.100	0.100	λ =	100	95	100	95	$(P/A_g f'_c)_{exp}$ =	0.04	0.07	0.07	0.04	$M_{y,exp}$ (k-in) =	9010	9615	9010	9615	9010	
	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	M_z/M_y =	1.19	1.18	1.19	1.18	$\Theta_{cap,pl}$ (rad) =	0.066	0.060	0.060	0.066	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	
	λ =	100	95	100	95	$(P/A_g f'_c)_{exp}$ =	0.04	0.07	0.07	0.04	$M_{y,exp}$ (k-in) =	9010	9615	9010	9615	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	0.35	
Floor 3		7229	-15860	3.490E+08	-0.046	0.083	99	90	7229	-15860	3.490E+08	0.032	-0.046	0.083	99	7229	-15860	3.490E+08	0.032	-0.046	0.083	99
Story 2	$\Theta_{cap,pl}$ (rad) =	0.100	0.100	0.100	0.100	λ =	97	90	97	90	$(P/A_g f'_c)_{exp}$ =	0.06	0.11	0.11	0.06	$M_{y,exp}$ (k-in) =	8284	11203	8284	11203	8284	
	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	M_z/M_y =	1.19	1.18	1.19	1.18	$\Theta_{cap,pl}$ (rad) =	0.065	0.056	0.056	0.065	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	
	λ =	95	86	95	86	$(P/A_g f'_c)_{exp}$ =	0.07	0.15	0.15	0.07	$M_{y,exp}$ (k-in) =	8284	11203	8284	11203	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	0.35	
Floor 2		8410	-17750	3.490E+08	-0.049	0.092	99	86	8410	-17750	3.490E+08	0.032	-0.049	0.092	99	8410	-17750	3.490E+08	0.032	-0.049	0.092	99
Story 1	$\Theta_{cap,pl}$ (rad) =	0.100	0.100	0.100	0.100	λ =	95	86	95	86	$(P/A_g f'_c)_{exp}$ =	0.07	0.15	0.15	0.07	$M_{y,exp}$ (k-in) =	8284	11203	8284	11203	8284	
	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	M_z/M_y =	1.18	1.17	1.18	1.17	$\Theta_{cap,pl}$ (rad) =	0.062	0.053	0.053	0.062	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	
	λ =	95	86	95	86	$(P/A_g f'_c)_{exp}$ =	0.07	0.15	0.15	0.07	$M_{y,exp}$ (k-in) =	8284	11203	8284	11203	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	0.35	

Mass tributary to one frame for lateral load (each floor) (k-s/in): 1.34

Model periods (sec): $T_1 = 0.87$ $T_2 = 0.27$ $T_3 = 0.15$ $f_{v,reqd}$ expected = 67 ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2027

Number of Stories: 4

Fundamental Period (sec): 0.85

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2027 SCWB=3.0

Compared to the 2007 design, column sizes were increased to meet strong-column weak-beam ratio (>3.0). Both negative and positive flexural beam strength was controlled by strength demands. Few additional beam reinforcements in four beams were added to alter beam design to use same ρ and ρ_{Prime} . Two floors had additional reinforcement added to meet the minimum positive/negative strength ratio. All column flexural strengths were controlled by the strong-column weak-beam ratio. Six beam stirrups and six columns were controlled by the capacity shear design, and the rest controlled by minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

Floor 5																				
Story 4	h (in) =	34	24	34	0.0062	0.0096	0.0023	5.4	24	34	0.0062	0.0096	0.0023	5.4	24	34	0.0062	0.0096	0.0023	5.4
	b (in) =	34	34	34	0.018	0.0088	0.0062	0.0111	0.0025	5.4	34	34	34	0.018	0.0088	0.0062	0.0111	0.0025	5.4	34
	ρ_{tot} =	0.011	0.0066	0.0062	0.0111	0.0025	5.4	3.5	0.0088	0.0062	0.0111	0.0025	5.4	3.5	0.0088	0.0062	0.0111	0.0025	5.4	3.5
	ρ_{sh} =	0.0066	0.0066	0.0062	0.0111	0.0025	5.4	3.5	0.0088	0.0062	0.0111	0.0025	5.4	3.5	0.0088	0.0062	0.0111	0.0025	5.4	3.5
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Floor 4																				
Story 3	h (in) =	34	30	34	0.0044	0.0088	0.0017	6.9	30	34	0.0044	0.0088	0.0017	6.9	30	34	0.0044	0.0088	0.0017	6.9
	b (in) =	34	34	34	0.030	0.0107	0.0044	0.0088	0.0017	6.9	34	34	34	0.030	0.0107	0.0044	0.0088	0.0017	6.9	34
	ρ_{tot} =	0.023	0.0107	0.0044	0.0088	0.0017	6.9	3.0	0.0107	0.0044	0.0088	0.0017	6.9	3.0	0.0107	0.0044	0.0088	0.0017	6.9	3.0
	ρ_{sh} =	0.0107	0.0107	0.0044	0.0088	0.0017	6.9	3.0	0.0107	0.0044	0.0088	0.0017	6.9	3.0	0.0107	0.0044	0.0088	0.0017	6.9	3.0
	s (in) =	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Floor 3																				
Story 2	h (in) =	34	30	34	0.0046	0.0092	0.0017	6.9	30	34	0.0046	0.0092	0.0017	6.9	30	34	0.0046	0.0092	0.0017	6.9
	b (in) =	34	34	34	0.030	0.0119	0.0046	0.0092	0.0017	6.9	34	34	34	0.030	0.0119	0.0046	0.0092	0.0017	6.9	34
	ρ_{tot} =	0.020	0.0119	0.0046	0.0092	0.0017	6.9	3.0	0.0119	0.0046	0.0092	0.0017	6.9	3.0	0.0119	0.0046	0.0092	0.0017	6.9	3.0
	ρ_{sh} =	0.0066	0.0066	0.0046	0.0092	0.0017	6.9	3.0	0.0066	0.0046	0.0092	0.0017	6.9	3.0	0.0066	0.0046	0.0092	0.0017	6.9	3.0
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Floor 2																				
Story 1	h (in) =	34	30	34	0.0046	0.0092	0.0017	6.9	30	34	0.0046	0.0092	0.0017	6.9	30	34	0.0046	0.0092	0.0017	6.9
	b (in) =	34	34	34	0.030	0.0119	0.0046	0.0092	0.0017	6.9	34	34	34	0.030	0.0119	0.0046	0.0092	0.0017	6.9	34
	ρ_{tot} =	0.024	0.0119	0.0046	0.0092	0.0017	6.9	3.0	0.0119	0.0046	0.0092	0.0017	6.9	3.0	0.0119	0.0046	0.0092	0.0017	6.9	3.0
	ρ_{sh} =	0.0066	0.0066	0.0046	0.0092	0.0017	6.9	3.0	0.0066	0.0046	0.0092	0.0017	6.9	3.0	0.0066	0.0046	0.0092	0.0017	6.9	3.0
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Grade beam column height (in) = 24										Basement column height (in) = 0										
30 feet																				
Design base shear = 0.092 g, 193 k																				
f'c beams = 5.0 ksi										f'c cols upper = 5.0 ksi										
fy rebar, nom. = 60 ksi										f'c cols lower = 5.0 ksi										

13 feet

15 feet

Design Documentation (2 of 2)									
Story 4	SCWB =	1.11	1.13	1.17	0.66	1.15	1.23	1.18	1.24
	Joint $\Phi V_r/V_u$ =	2.16	1.75	1.75	1.75	1.75	1.75	1.75	1.75
	$\Phi M_r/M_u$ =	1.59	5.80	5.82	5.82	5.82	5.82	5.82	5.82
	$\Phi V_r/V_{mgr}$ =	1.5	1.15	1.15	1.15	1.15	1.15	1.15	1.15
Floor 4	$P/A_g f'_c$ =	0.02	0.05	0.05	0.05	0.05	0.05	0.05	0.05
		3.09	1.14	1.20	0.57	1.17	3.05	1.14	1.20
		1.57	1.35	1.35	1.35	1.35	1.35	1.35	1.35
		3.07	1.57	1.57	1.57	1.57	3.07	1.57	1.57
Story 3	$\Phi M_r/M_u$ =	3.92	7.81	7.87	7.87	7.87	7.87	7.87	7.87
	$\Phi V_r/V_{mgr}$ =	1.15	1.06	1.09	1.09	1.09	1.09	1.09	1.09
	$P/A_g f'_c$ =	0.04	0.08	0.08	0.08	0.08	0.08	0.08	0.08
		3.05	1.17	1.28	0.52	1.25	3.08	1.17	1.28
Floor 3		1.69	1.49	1.49	1.49	1.49	1.69	1.49	1.49
		3.07	1.69	1.69	1.69	1.69	3.07	1.69	1.69
Story 2	$\Phi M_r/M_u$ =	3.39	6.37	6.39	6.39	6.39	6.39	6.39	6.39
	$\Phi V_r/V_{mgr}$ =	1.02	1.04	1.04	1.04	1.04	1.04	1.04	1.04
	$P/A_g f'_c$ =	0.07	0.13	0.13	0.13	0.13	0.13	0.13	0.13
		3.06	1.12	1.22	0.52	1.19	3.09	1.12	1.22
Floor 2		1.56	1.38	1.38	1.38	1.38	1.56	1.38	1.38
		3.06	1.56	1.56	1.56	1.56	3.06	1.56	1.56
Story 1	$\Phi M_r/M_u$ =	4.45	6.97	6.98	6.98	6.98	6.98	6.98	6.98
	$\Phi V_r/V_{mgr}$ =	1	1.01	1.03	1.03	1.03	1.03	1.03	1.03
	$P/A_g f'_c$ =	0.09	0.17	0.17	0.17	0.17	0.17	0.17	0.17
		1.33	1.18	1.44	0.52	1.19	1.38	1.18	1.44

Design Drifts:

0.8%

0.9%

1.1%

1.3%

Modeling Documentation (1 of 1)									
Floor 5	$M_{y,exp}$ (k-in) =	12175	20877	20877	11668				
	E_{eff}/E_g =	0.35	0.35	0.35	0.35				
	M_c/M_y =	1.21	1.21	1.21	1.21				
	$\Theta_{cap,pl}$ (rad) =	0.065	0.084	0.084	0.065				
Floor 4	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100				
	λ =	116	127	127	116				
	$(P/A_g f'_c)_{exp}$ =	0.02	0.03	0.03	0.02				
		5921	5921	5921	5921				
Floor 3	$M_{y,exp}$ (k-in) =	25886	35341	35341	26136				
	E_{eff}/E_g =	0.35	0.35	0.35	0.35				
	M_c/M_y =	1.21	1.20	1.20	1.21				
	$\Theta_{cap,pl}$ (rad) =	0.095	0.098	0.098	0.095				
Floor 2	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100				
	λ =	127	127	127	127				
	$(P/A_g f'_c)_{exp}$ =	0.03	0.06	0.06	0.03				
		7116	7116	7116	7116				
Floor 1	$M_{y,exp}$ (k-in) =	24000	37503	37503	24000				
	E_{eff}/E_g =	0.35	0.35	0.35	0.35				
	M_c/M_y =	1.20	1.20	1.20	1.20				
	$\Theta_{cap,pl}$ (rad) =	0.066	0.097	0.097	0.066				
Story 1	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100				
	λ =	112	122	122	112				
	$(P/A_g f'_c)_{exp}$ =	0.05	0.09	0.09	0.05				
		7365	7365	7365	7365				

Mass tributary to one frame for lateral load (each floor) (k-s/s/in): 1.34

Model periods (sec): $T_1 = 0.74$ $T_2 = 0.23$ $T_3 = 0.12$

$f_{y, rebar, expected} = 67$ ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2028

Number of Stories: 12

Fundamental Period (sec): 2.27

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Note: In order to see the effects of design strength changes the minimum base shear demand from eqn 9.5.5.2.1-3 was not imposed in this design.

From the baseline design, beam sizes were reduced as the lateral demand is reduced from larger R value. Since joint shear controlled, a higher concrete strength was used. Positive beam bending strength was controlled by strength demands. 12 negative bending strength is controlled by minimum reinforcement requirement, the rest by strength demand. The beam is made large for stiffness, thus controlled by min requirement. Additional beam reinforcements were added to alter beam design to use same ρ and ρ_{Prime} in each floor and to meet the minimum positive/negative strength ratio. Column strength governed by SCWB ratio. Both beam and column stirrups were controlled by the minimum confinement requirements.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)												
Floor 13	22	20	0.0045	0.0051	0.0040	4.9	22	20	0.0045	0.0051	0.0040	4.9
Story 12	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.014$	$\rho_{sh} = 0.0123$	s (in) = 4.5	22	20	0.0041	0.0083	0.0040	4.9	20
Floor 12	22	20	0.0041	0.0083	0.0040	4.9	22	20	0.0041	0.0083	0.0040	4.9
Story 11	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.014$	$\rho_{sh} = 0.0123$	s (in) = 4.5	22	20	0.0046	0.0091	0.0040	4.9	20
Floor 11	22	20	0.0046	0.0091	0.0040	4.9	22	20	0.0046	0.0091	0.0040	4.9
Story 10	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.014$	$\rho_{sh} = 0.0123$	s (in) = 4.5	22	20	0.0051	0.0101	0.0040	4.9	20
Floor 10	22	20	0.0051	0.0101	0.0040	4.9	22	20	0.0051	0.0101	0.0040	4.9
Story 9	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.014$	$\rho_{sh} = 0.0123$	s (in) = 4.5	28	20	0.0032	0.0061	0.0031	6.4	20
Floor 9	28	20	0.0032	0.0061	0.0031	6.4	28	20	0.0032	0.0061	0.0031	6.4
Story 8	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.016$	$\rho_{sh} = 0.0123$	s (in) = 4.5	28	20	0.0032	0.0063	0.0031	6.4	20
Floor 8	28	20	0.0032	0.0063	0.0031	6.4	28	20	0.0032	0.0063	0.0031	6.4
Story 7	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.016$	$\rho_{sh} = 0.0123$	s (in) = 4.5	28	20	0.0037	0.0074	0.0031	6.4	20
Floor 7	28	20	0.0037	0.0074	0.0031	6.4	28	20	0.0037	0.0074	0.0031	6.4
Story 6	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.016$	$\rho_{sh} = 0.0123$	s (in) = 4.5	28	20	0.0037	0.0073	0.0031	6.4	20
Floor 6	28	20	0.0037	0.0073	0.0031	6.4	28	20	0.0037	0.0073	0.0031	6.4
Story 5	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.016$	$\rho_{sh} = 0.0123$	s (in) = 4.5	28	20	0.0037	0.0072	0.0031	6.4	20
Floor 5	28	20	0.0037	0.0072	0.0031	6.4	28	20	0.0037	0.0072	0.0031	6.4
Story 4	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.016$	$\rho_{sh} = 0.0123$	s (in) = 4.5	28	20	0.0036	0.0071	0.0031	6.4	20
Floor 4	28	20	0.0036	0.0071	0.0031	6.4	28	20	0.0036	0.0071	0.0031	6.4
Story 3	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.016$	$\rho_{sh} = 0.0123$	s (in) = 4.5	28	20	0.0037	0.0070	0.0031	6.4	20
Floor 3	28	20	0.0037	0.0070	0.0031	6.4	28	20	0.0037	0.0070	0.0031	6.4
Story 2	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.012$	$\rho_{sh} = 0.0123$	s (in) = 4.5	28	20	0.0041	0.0067	0.0031	6.4	20
Floor 2	28	20	0.0041	0.0067	0.0031	6.4	28	20	0.0041	0.0067	0.0031	6.4
Story 1	h (in) = 20	b (in) = 20	$\rho_{tot} = 0.012$	$\rho_{sh} = 0.0123$	s (in) = 4.5	28	20	0.0041	0.0067	0.0031	6.4	20
	28	20	0.0041	0.0067	0.0031	6.4	28	20	0.0041	0.0067	0.0031	6.4
Grade beam column height (in) = 24 Basement column height (in) = 20 20 feet 13 feet 15 feet												
Design base shear = 0.028 g, 232 k f'_c beams = 5.0 ksi f'_c cols upper = 8.0 ksi f'_c rebar nom. = 60 ksi f'_c cols lower = 8.0 ksi												

Design Documentation (2 of 2)											
SCWB = 0.87											
Joint $\Phi V_r/V_u = 3.16$											
Story 12	$\Phi M_r/M_u = 1.34$ $\Phi V_r/V_{npr} = 3.81$ $P/A_g f_c = 0.02$			4.33 2.59 0.04			4.34 2.59 0.04			1.34 3.81 0.02	
Floor 12	1.27 1.69			1.31 1.50			1.31 1.50			1.27 1.69	
Story 11	$\Phi M_r/M_u = 1.55$ $\Phi V_r/V_{npr} = 3.39$ $P/A_g f_c = 0.04$			3.37 2.99 0.06			3.38 2.99 0.06			1.55 3.39 0.04	
Floor 11	1.26 1.53			1.30 1.35			1.30 1.35			1.26 1.53	
Story 10	$\Phi M_r/M_u = 1.44$ $\Phi V_r/V_{npr} = 3.91$ $P/A_g f_c = 0.05$			2.81 2.69 0.10			2.81 2.69 0.10			1.45 3.91 0.05	
Floor 10	1.22 1.35			1.29 1.19			1.29 1.19			1.22 1.35	
Story 9	$\Phi M_r/M_u = 1.40$ $\Phi V_r/V_{npr} = 4.22$ $P/A_g f_c = 0.07$			2.52 2.9 0.13			2.52 2.9 0.13			1.40 4.22 0.07	
Floor 9	1.22 1.78			1.25 1.55			1.25 1.55			1.22 1.78	
Story 8	$\Phi M_r/M_u = 1.49$ $\Phi V_r/V_{npr} = 4.14$ $P/A_g f_c = 0.09$			2.40 3.07 0.17			2.40 3.07 0.17			1.49 4.14 0.09	
Floor 8	1.31 1.74			1.33 1.53			1.33 1.53			1.31 1.74	
Story 7	$\Phi M_r/M_u = 1.32$ $\Phi V_r/V_{npr} = 3.88$ $P/A_g f_c = 0.11$			2.04 2.87 0.20			2.04 2.87 0.20			1.32 3.88 0.11	
Floor 7	1.20 1.48			1.23 1.31			1.23 1.31			1.20 1.48	
Story 6	$\Phi M_r/M_u = 1.43$ $\Phi V_r/V_{npr} = 3.65$ $P/A_g f_c = 0.14$			2.21 2.71 0.23			2.21 2.71 0.23			1.43 3.65 0.14	
Floor 6	1.25 1.50			1.32 1.32			1.32 1.32			1.25 1.50	
Story 5	$\Phi M_r/M_u = 1.47$ $\Phi V_r/V_{npr} = 3.46$ $P/A_g f_c = 0.16$			2.31 2.12 0.27			2.31 2.12 0.27			1.47 3.46 0.16	
Floor 5	1.30 1.51			1.39 1.33			1.39 1.33			1.30 1.51	
Story 4	$\Phi M_r/M_u = 1.52$ $\Phi V_r/V_{npr} = 3.3$ $P/A_g f_c = 0.18$			2.41 2.17 0.30			2.41 2.17 0.30			1.52 3.3 0.18	
Floor 4	1.35 1.54			1.50 1.36			1.50 1.36			1.35 1.54	
Story 3	$\Phi M_r/M_u = 1.59$ $\Phi V_r/V_{npr} = 3.16$ $P/A_g f_c = 0.20$			2.55 2.22 0.34			2.55 2.22 0.34			1.59 3.16 0.20	
Floor 3	1.30 1.57			1.39 1.36			1.39 1.36			1.30 1.57	
Story 2	$\Phi M_r/M_u = 1.40$ $\Phi V_r/V_{npr} = 3.35$ $P/A_g f_c = 0.22$			2.10 2.84 0.37			2.10 2.84 0.37			1.40 3.35 0.22	
Floor 2	1.27 1.62			1.28 1.35			1.28 1.35			1.27 1.62	
Story 1	$\Phi M_r/M_u = 1.33$ $\Phi V_r/V_{npr} = 3.21$ $P/A_g f_c = 0.24$			2.05 2.9 0.41			2.05 2.9 0.41			1.33 3.21 0.24	
$(\Phi M_r/M_u)_{neg} = 1.12$ $(\Phi M_r/M_u)_{pos} = 1.17$ $M_{u,pcr}/M_{u,neg} = 0.62$ $\Phi V_r/V_{npr} = 2.87$				$(\Phi M_r/M_u)_{neg} = 1.21$ $(\Phi M_r/M_u)_{pos} = 1.91$ $M_{u,pcr}/M_{u,neg} = 0.62$ $\Phi V_r/V_{npr} = 2.87$				$(\Phi M_r/M_u)_{neg} = 1.12$ $(\Phi M_r/M_u)_{pos} = 1.17$ $M_{u,pcr}/M_{u,neg} = 0.62$ $\Phi V_r/V_{npr} = 2.87$			
Design Drifts:											
0.6%											
1.0%											
1.3%											
1.6%											
1.8%											
1.7%											
1.6%											
1.6%											
1.7%											
1.7%											

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	2934	2106	3674	0.35	6.927E+0	0.0467	0.07	0.100	100
		E_{eff}/E_{lg} =	0.35	0.35	0.35	0.35	0.35	0.0467	0.07	0.100	100
Floor 12	Story 11	$M_{y,exp}$ (k-in) =	3235	1944	5047	0.35	6.927E+07	0.044	0.078	0.100	100
		E_{eff}/E_{lg} =	0.35	0.35	0.35	0.35	0.35	0.044	0.078	0.100	100
Floor 11	Story 10	$M_{y,exp}$ (k-in) =	3531	2167	5428	0.35	6.927E+07	0.045	0.078	0.100	100
		E_{eff}/E_{lg} =	0.35	0.35	0.35	0.35	0.35	0.045	0.078	0.100	100
Floor 10	Story 9	$M_{y,exp}$ (k-in) =	3823	2394	5863	0.35	6.927E+08	0.046	0.078	0.100	100
		E_{eff}/E_{lg} =	0.35	0.35	0.35	0.35	0.35	0.046	0.078	0.100	100
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	4460	2620	6600	0.35	1.419E+08	0.035	0.060	0.100	99
		E_{eff}/E_{lg} =	0.35	0.35	0.35	0.35	1.419E+08	0.035	0.060	0.100	99
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	4743	2620	6694	0.35	1.419E+08	0.035	0.060	0.100	99
		E_{eff}/E_{lg} =	0.40	0.35	0.35	0.35	1.419E+08	0.035	0.060	0.100	99
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	5022	3016	7454	0.35	1.419E+08	0.0354	0.060	0.100	99
		E_{eff}/E_{lg} =	0.41	0.35	0.35	0.35	1.419E+08	0.0354	0.060	0.100	99
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	5298	3016	7424	0.35	1.419E+08	0.0354	0.060	0.100	99
		E_{eff}/E_{lg} =	0.43	0.35	0.35	0.35	1.419E+08	0.0354	0.060	0.100	99
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	5572	3016	7328	0.35	1.419E+08	0.0351	0.060	0.100	99
		E_{eff}/E_{lg} =	0.44	0.35	0.35	0.35	1.419E+08	0.0351	0.060	0.100	99
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	5843	2917	7235	0.35	1.419E+08	0.0355	0.059	0.100	99
		E_{eff}/E_{lg} =	0.45	0.35	0.35	0.35	1.419E+08	0.0355	0.059	0.100	99
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	6424	3033	7235	0.35	1.419E+08	0.0355	0.059	0.100	99
		E_{eff}/E_{lg} =	0.46	0.35	0.35	0.35	1.419E+08	0.0355	0.059	0.100	99
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	5691	3305	7049	0.35	1.419E+08	0.036	0.058	0.100	99
		E_{eff}/E_{lg} =	0.47	0.35	0.35	0.35	1.419E+08	0.036	0.058	0.100	99
Mass tributary to one frame for lateral load (each floor) (k-s/in): 0.60 Model periods (sec): $T_1 = 2.27$ $T_2 = 0.77$ $T_3 = 0.45$ $I_{rebar, expected} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2029

Number of Stories: 12

Fundamental Period (sec): 1.99

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2029 SCWB=1.0

Simply using less reinforcements in the column from the baseline design was sufficient to obtain SCWB=1.0. Like the baseline, joint shear and drift controls this design—joint shear more dominate. Both positive and negative bending strength were controlled by strength demands. Additional beam reinforcements in 12 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. Then 24 bays had additional reinforcement added to meet the minimum positive/negative strength ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 5 by flexural demand. Beam stirrups were controlled by the capacity shear design and minimum requirement (half-half). Column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)												
Floor 13												
Story 12	h (in) =	24	22	24	0.0035	0.0053	0.0034	4.9	22	24	0.0035	0.0053
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.010	0.014	0.014	0.0064	0.0034	0.0034	4.9	0.014	0.014	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	4.9	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	4.9	5.0	5.0	5.0	5.0
Floor 12												
Story 11	h (in) =	24	22	24	0.0039	0.0078	0.0034	4.9	22	24	0.0039	0.0078
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.010	0.014	0.014	0.0064	0.0034	0.0034	4.9	0.014	0.014	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	4.9	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	4.9	5.0	5.0	5.0	5.0
Floor 11												
Story 10	h (in) =	24	22	24	0.0045	0.0090	0.0034	4.9	22	24	0.0045	0.0090
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.010	0.014	0.014	0.0064	0.0034	0.0034	4.9	0.014	0.014	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	4.9	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	4.9	5.0	5.0	5.0	5.0
Floor 10												
Story 9	h (in) =	24	22	24	0.005	0.0100	0.0034	4.9	22	24	0.005	0.0100
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.010	0.014	0.014	0.0064	0.0034	0.0034	4.9	0.014	0.014	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	4.9	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	4.9	5.0	5.0	5.0	5.0
Floor 9												
Story 8	h (in) =	24	22	24	0.0054	0.0108	0.0034	4.9	22	24	0.0054	0.0108
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.010	0.014	0.014	0.0064	0.0034	0.0034	4.9	0.014	0.014	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	4.9	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	4.9	5.0	5.0	5.0	5.0
Floor 8												
Story 7	h (in) =	24	22	24	0.0055	0.011	0.0034	4.9	22	24	0.0055	0.011
	b (in) =	24	22	24	0.0064	0.0034	0.0034	4.9	22	24	0.0064	0.0034
	ρ_{tot} =	0.013	0.019	0.019	0.0064	0.0034	0.0034	4.9	0.019	0.019	0.0064	0.0034
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	4.9	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	4.9	5.0	5.0	5.0	5.0
Floor 7												
Story 6	h (in) =	24	28	24	0.004	0.0078	0.0046	6.4	28	24	0.004	0.0078
	b (in) =	24	28	24	0.0064	0.0046	0.0046	6.4	28	24	0.0064	0.0046
	ρ_{tot} =	0.014	0.019	0.019	0.0064	0.0046	0.0046	6.4	0.019	0.019	0.0064	0.0046
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	6.4	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	6.4	5.0	5.0	5.0	5.0
Floor 6												
Story 5	h (in) =	24	28	24	0.004	0.0078	0.0046	6.4	28	24	0.004	0.0078
	b (in) =	24	28	24	0.0064	0.0046	0.0046	6.4	28	24	0.0064	0.0046
	ρ_{tot} =	0.013	0.019	0.019	0.0064	0.0046	0.0046	6.4	0.019	0.019	0.0064	0.0046
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	6.4	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	6.4	5.0	5.0	5.0	5.0
Floor 5												
Story 4	h (in) =	24	28	24	0.0037	0.0073	0.0045	6.4	28	24	0.0037	0.0073
	b (in) =	24	28	24	0.0064	0.0045	0.0045	6.4	28	24	0.0064	0.0045
	ρ_{tot} =	0.013	0.019	0.019	0.0064	0.0045	0.0045	6.4	0.019	0.019	0.0064	0.0045
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	6.4	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	6.4	5.0	5.0	5.0	5.0
Floor 4												
Story 3	h (in) =	24	28	24	0.004	0.0073	0.0045	6.4	28	24	0.004	0.0073
	b (in) =	24	28	24	0.0064	0.0045	0.0045	6.4	28	24	0.0064	0.0045
	ρ_{tot} =	0.013	0.019	0.019	0.0064	0.0045	0.0045	6.4	0.019	0.019	0.0064	0.0045
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	6.4	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	6.4	5.0	5.0	5.0	5.0
Floor 3												
Story 2	h (in) =	24	28	24	0.0043	0.0071	0.0046	6.4	28	24	0.0043	0.0071
	b (in) =	24	28	24	0.0064	0.0046	0.0046	6.4	28	24	0.0064	0.0046
	ρ_{tot} =	0.013	0.019	0.019	0.0064	0.0046	0.0046	6.4	0.019	0.019	0.0064	0.0046
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	6.4	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	6.4	5.0	5.0	5.0	5.0
Floor 2												
Story 1	h (in) =	24	28	24	0.0043	0.0071	0.0046	6.4	28	24	0.0043	0.0071
	b (in) =	24	28	24	0.0064	0.0046	0.0046	6.4	28	24	0.0064	0.0046
	ρ_{tot} =	0.017	0.019	0.019	0.0064	0.0046	0.0046	6.4	0.019	0.019	0.0064	0.0046
	ρ_{sh} =	0.0064	0.0064	0.0064	0.0064	0.0064	0.0064	6.4	0.0064	0.0064	0.0064	0.0064
	s (in) =	5.0	5.0	5.0	5.0	5.0	5.0	6.4	5.0	5.0	5.0	5.0
Grade beam column height (in) = 24												
Basement column height (in) = 24												
20 feet												
Design base shear = 0.044 g, 232 k												
f'_c beams = 5.0 ksi f'_c cols upper = 5.0 ksi f'_c rebar nom. = 60 ksi f'_c cols lower = 5.0 ksi												
<div>13 feet</div> <div>15 feet</div>												

Design Documentation (2 of 2)															
SCWB = 0.96 1.19 1.20 6.36 0.81 1.25 1.36 0.67 0.66 1.19 1.20 0.67 6.36 0.87															
Story 12	Joint $\Phi V_r/V_u$	=	2.93												2.93
	$\Phi M_r/M_u$	=	1.45					3.23				3.26			1.31
	$\Phi V_r/V_{npr}$	=	2.44					1.85				1.86			2.58
	$P/A_g f_c$	=	0.01					0.00				0.00			0.00
Floor 12			1.38	1.17	1.37	0.51	3.68	1.28		1.37	1.46	0.51	0.67	6.36	0.87
Story 11	$\Phi M_r/M_u$	=	1.70					1.51				1.51			1.70
	$\Phi V_r/V_{npr}$	=	1.65					2.70				2.68			1.60
	$\Phi V_r/V_{npr}$	=	2.38					1.85				1.86			2.64
	$P/A_g f_c$	=	0.01					0.00				0.00			0.00
Floor 11			1.18	1.18	1.43	0.51	2.90	1.13		1.35	1.54	0.51	0.51	2.90	1.14
Story 10	$\Phi M_r/M_u$	=	1.47					1.30				1.30			1.47
	$\Phi V_r/V_{npr}$	=	1.38					2.00				2.02			1.33
	$\Phi V_r/V_{npr}$	=	2.41					1.85				1.86			2.6
	$P/A_g f_c$	=	0.01					0.00				0.00			0.00
Floor 10			1.06	1.16	1.46	0.51	2.48	1.03		1.32	1.55	0.51	0.51	2.48	1.05
Story 9	$\Phi M_r/M_u$	=	1.29					1.15				1.15			1.29
	$\Phi V_r/V_{npr}$	=	1.18					1.61				1.63			1.18
	$\Phi V_r/V_{npr}$	=	2.84					2.18				2.2			3.08
	$P/A_g f_c$	=	0.01					0.00				0.00			0.00
Floor 9			1.12	1.15	1.45	0.52	2.22	1.13		1.30	1.58	0.52	0.52	2.22	1.19
Story 8	$\Phi M_r/M_u$	=	1.19					1.05				1.05			1.19
	$\Phi V_r/V_{npr}$	=	1.32					1.88				1.89			1.46
	$\Phi V_r/V_{npr}$	=	2.69					1.86				1.87			2.55
	$P/A_g f_c$	=	0.00					0.00				0.00			0.01
Floor 8			1.27	1.14	1.40	0.51	2.17	1.28		1.27	1.50	0.51	0.51	2.17	1.37
Story 7	$\Phi M_r/M_u$	=	1.16					1.03				1.03			1.16
	$\Phi V_r/V_{npr}$	=	1.15					1.49				1.48			1.21
	$\Phi V_r/V_{npr}$	=	2.58					1.86				1.86			2.44
	$P/A_g f_c$	=	0.00					0.00				0.00			0.01
Floor 7			1.02	1.17	1.34	0.53	1.16	1.03		1.38	1.41	0.53	1.16	1.07	1.33
Story 6	$\Phi M_r/M_u$	=	1.33					1.17				1.17			1.33
	$\Phi V_r/V_{npr}$	=	1.17					1.51				1.52			1.23
	$\Phi V_r/V_{npr}$	=	2.67					1.86				1.87			2.56
	$P/A_g f_c$	=	0.00					0.00				0.00			0.01
Floor 6			1.02	1.18	1.33	0.53	1.16	1.02		1.37	1.40	0.53	1.16	1.03	1.33
Story 5	$\Phi M_r/M_u$	=	1.33					1.17				1.17			1.33
	$\Phi V_r/V_{npr}$	=	1.20					1.48				1.49			1.16
	$\Phi V_r/V_{npr}$	=	2.65					1.86				1.87			2.56
	$P/A_g f_c$	=	0.00					0.00				0.00			0.01
Floor 5			1.08	1.11	1.09	0.52	1.15	1.08		1.27	1.27	0.52	1.15	1.06	1.41
Story 4	$\Phi M_r/M_u$	=	1.41					1.25				1.25			1.41
	$\Phi V_r/V_{npr}$	=	1.16					1.46				1.46			1.17
	$\Phi V_r/V_{npr}$	=	2.65					1.87				1.87			2.66
	$P/A_g f_c$	=	0.00					0.00				0.00			0.00
Floor 4			1.07	1.11	1.09	0.52	1.15	1.08		1.25	1.24	0.52	1.15	1.07	1.41
Story 3	$\Phi M_r/M_u$	=	1.41					1.25				1.25			1.41
	$\Phi V_r/V_{npr}$	=	1.16					1.45				1.46			1.17
	$\Phi V_r/V_{npr}$	=	2.65					1.86				1.87			2.65
	$P/A_g f_c$	=	0.00					0.00				0.00			0.00
Floor 3			1.08	1.12	1.11	0.55	1.15	1.06		1.24	1.32	0.55	1.15	1.07	1.41
Story 2	$\Phi M_r/M_u$	=	1.41					1.22				1.22			1.41
	$\Phi V_r/V_{npr}$	=	1.16					1.41				1.41			1.15
	$\Phi V_r/V_{npr}$	=	2.7					1.86				1.86			2.64
	$P/A_g f_c$	=	0.00					0.00				0.00			0.00
Floor 2			1.27	1.14	1.13	0.62	1.16	1.05		1.24	1.37	0.62	1.16	1.28	1.46
Story 1	$\Phi M_r/M_u$	=	1.46					1.21				1.21			1.46
	$\Phi V_r/V_{npr}$	=	1.16					1.27				1.27			1.17
	$\Phi V_r/V_{npr}$	=	2.06					1.86				1.86			2.06
	$P/A_g f_c$	=	0.00					0.00				0.00			0.00
Design Drifts:															
0.7% 1.1% 1.4% 1.7% 1.9% 1.8% 1.6% 1.6% 1.6% 1.7% 1.5%															

Mass tributary to one frame for lateral load (each floor) (k-s/in):	0.60
Model periods (sec): $T_1 = 1.99$ $T_2 = 0.68$ $T_3 = 0.39$	
$f_{y, \text{rebar, expected}} =$	67 ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2030

Number of Stories: 12

Fundamental Period (sec): 1.99

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2030 SCWB=0.8

Note: for the purpose of sensitivity analysis, this design ignores the minimum requirement ratio in the columns. Furthermore, the beam and column sizes were kept as in 2029, where only the column reinforcement are make less dense to reach smaller SCWB ratio. Reason: By making column sizes smaller, beam sizes would have to be larger to satisfy drift; and my making beam sizes larger, minimum reinforcement requirement in the beam will be imposed.

Both positive and negative bending strength were controlled by strength demands. Additional beam reinforcements in 12 bays were added to alter beam design to use same rho and rhoPrime in each floor. Then 24 bays had additional reinforcement added to meet the minimum positive/negative strength ratio. Few column flexural strengths were controlled by the strong-column weak-beam ratio, 28 columns (mostly exterior columns) by flexural demand. Beam stirrups were controlled by the capacity shear design and minimum requirement (half-half). Column stirrups were controlled by the minimum confinement requirement.

Note: Design for smaller SCWB were done but ignored for analysis because majority of the columns in this design (2030) were already controlled by flexural strength. Thus, even by making a less SCWB requirement will not increase capacity performance, and will analysis result will be misleading.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

Floor 13	22	24	0.0032	0.0050	0.0034	4.9	22	24	0.0032	0.0050	0.0034	4.9	22	24	0.0032	0.0050	0.0034	4.9			
Story 12	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.009$			0.010			0.010			0.010			0.010			0.010			0.010		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			22	24	0.0039	0.0075	0.0034	4.9	22	24	0.0039	0.0075	0.0034	4.9	22	24	0.0039	0.0075	0.0034	4.9	
Floor 12	22	24	0.0039	0.0075	0.0034	4.9	22	24	0.0039	0.0075	0.0034	4.9	22	24	0.0039	0.0075	0.0034	4.9			
Story 11	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.009$			0.010			0.010			0.010			0.010			0.010			0.010		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			22	24	0.0045	0.0088	0.0034	4.9	22	24	0.0045	0.0088	0.0034	4.9	22	24	0.0045	0.0088	0.0034	4.9	
Floor 11	22	24	0.0045	0.0088	0.0034	4.9	22	24	0.0045	0.0088	0.0034	4.9	22	24	0.0045	0.0088	0.0034	4.9			
Story 10	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.009$			0.010			0.010			0.010			0.010			0.010			0.010		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			22	24	0.005	0.0098	0.0034	4.9	22	24	0.005	0.0098	0.0034	4.9	22	24	0.005	0.0098	0.0034	4.9	
Floor 10	22	24	0.005	0.0098	0.0034	4.9	22	24	0.005	0.0098	0.0034	4.9	22	24	0.005	0.0098	0.0034	4.9			
Story 9	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.010$			0.010			0.010			0.010			0.010			0.010			0.010		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			22	24	0.0054	0.011	0.0034	4.9	22	24	0.0054	0.011	0.0034	4.9	22	24	0.0054	0.011	0.0034	4.9	
Floor 9	22	24	0.0054	0.011	0.0034	4.9	22	24	0.0054	0.011	0.0034	4.9	22	24	0.0054	0.011	0.0034	4.9			
Story 8	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.013$			0.015			0.015			0.015			0.015			0.015			0.015		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			22	24	0.0057	0.0113	0.0034	4.9	22	24	0.0057	0.0113	0.0034	4.9	22	24	0.0057	0.0113	0.0034	4.9	
Floor 8	22	24	0.0057	0.0113	0.0034	4.9	22	24	0.0057	0.0113	0.0034	4.9	22	24	0.0057	0.0113	0.0034	4.9			
Story 7	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.014$			0.015			0.015			0.015			0.015			0.015			0.015		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			28	24	0.0037	0.0075	0.0045	6.4	28	24	0.0037	0.0075	0.0045	6.4	28	24	0.0037	0.0075	0.0045	6.4	
Floor 7	28	24	0.0037	0.0075	0.0045	6.4	28	24	0.0037	0.0075	0.0045	6.4	28	24	0.0037	0.0075	0.0045	6.4			
Story 6	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.013$			0.015			0.015			0.015			0.015			0.015			0.015		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			28	24	0.0037	0.0075	0.0045	6.4	28	24	0.0037	0.0075	0.0045	6.4	28	24	0.0037	0.0075	0.0045	6.4	
Floor 6	28	24	0.0037	0.0075	0.0045	6.4	28	24	0.0037	0.0075	0.0045	6.4	28	24	0.0037	0.0075	0.0045	6.4			
Story 5	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.013$			0.015			0.015			0.015			0.015			0.015			0.015		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			28	24	0.0037	0.0076	0.0045	6.4	28	24	0.0037	0.0076	0.0045	6.4	28	24	0.0037	0.0076	0.0045	6.4	
Floor 5	28	24	0.0037	0.0076	0.0045	6.4	28	24	0.0037	0.0076	0.0045	6.4	28	24	0.0037	0.0076	0.0045	6.4			
Story 4	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.013$			0.015			0.015			0.015			0.015			0.015			0.015		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			28	24	0.004	0.0076	0.0046	6.4	28	24	0.004	0.0076	0.0046	6.4	28	24	0.004	0.0076	0.0046	6.4	
Floor 4	28	24	0.004	0.0076	0.0046	6.4	28	24	0.004	0.0076	0.0046	6.4	28	24	0.004	0.0076	0.0046	6.4			
Story 3	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.013$			0.015			0.015			0.015			0.015			0.015			0.015		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			28	24	0.0042	0.0076	0.0046	6.4	28	24	0.0042	0.0076	0.0046	6.4	28	24	0.0042	0.0076	0.0046	6.4	
Floor 3	28	24	0.0042	0.0076	0.0046	6.4	28	24	0.0042	0.0076	0.0046	6.4	28	24	0.0042	0.0076	0.0046	6.4			
Story 2	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.013$			0.016			0.016			0.016			0.016			0.016			0.016		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			28	24	0.0046	0.0073	0.0047	6.4	28	24	0.0046	0.0073	0.0047	6.4	28	24	0.0046	0.0073	0.0047	6.4	
Floor 2	28	24	0.0046	0.0073	0.0047	6.4	28	24	0.0046	0.0073	0.0047	6.4	28	24	0.0046	0.0073	0.0047	6.4			
Story 1	h (in) = 24			24			24			24			24			24			24		
	b (in) = 24			24			24			24			24			24			24		
	$\rho_{ext} = 0.017$			0.017			0.017			0.017			0.017			0.017			0.017		
	$\rho_{sh} = 0.0064$			0.0064			0.0064			0.0064			0.0064			0.0064			0.0064		
s (in) = 5.0			28	24	0.004	0.0073	0.0047	6.4	28	24	0.004	0.0073	0.0047	6.4	28	24	0.004	0.0073	0.0047	6.4	
Floor 1	28	24	0.004	0.0073	0.0047	6.4	28	24	0.004	0.0073	0.0047	6.4	28	24	0.004	0.0073	0.0047	6.4			

Grade beam column height (in) = 24 Basement column height (in) = 24

← 20 feet →

Design base shear = 0.044 g, 232 k

f_c beams = 5.0 ksi f_c cols upper = 5.0 ksi

f_y rebar nom. = 60 ksi f_c cols lower = 5.0 ksi

Design Documentation (2 of 2)																			
		SCWB =	0.91	1.14	1.11	0.65	7.29	0.61	1.19	1.26	0.65	7.29	0.49	1.14	1.11	0.65	7.29	0.81	
	Joint $\Phi V_r/V_u$		3.07					2.49					2.49					3.07	Design Drifts:
Story 12	$\Phi M_r/M_u$		1.32					2.32					2.35					1.18	
	$\Phi V_r/V_{npr}$		2.68					2.55					2.58					2.85	0.7%
	$P/A_g f'_c$		0.01					0.00					0.00					0.00	
Floor 12			1.28	1.14	1.37	0.53	3.82	0.94	1.33				0.94	1.14	1.37	0.53	3.82	1.19	
			1.76					1.55					1.55					1.76	
Story 11	$\Phi M_r/M_u$		1.49					1.95					1.93					1.44	
	$\Phi V_r/V_{npr}$		2.61					2.55					2.58					2.93	1.1%
	$P/A_g f'_c$		0.01					0.00					0.00					0.00	
Floor 11			1.09	1.15	1.43	0.52	2.98	0.83	1.31				0.83	1.15	1.43	0.52	2.98	1.05	
			1.51					1.33					1.33					1.51	
Story 10	$\Phi M_r/M_u$		1.25					1.44					1.45					1.20	
	$\Phi V_r/V_{npr}$		2.65					2.56					2.58					2.88	1.4%
	$P/A_g f'_c$		0.01					0.00					0.00					0.00	
Floor 10			1.03	1.13	1.46	0.52	2.54	0.75	1.29				0.76	1.13	1.46	0.52	2.54	1.00	
			1.33					1.17					1.17					1.33	
Story 9	$\Phi M_r/M_u$		1.18					1.16					1.18					1.15	
	$\Phi V_r/V_{npr}$		2.84					3.01					3.04					3.16	1.7%
	$P/A_g f'_c$		0.01					0.00					0.00					0.00	
Floor 9			1.10	1.17	1.45	0.51	2.17	0.85	1.33				0.86	1.17	1.45	0.51	2.17	1.17	
			1.16					1.03					1.03					1.16	
Story 8	$\Phi M_r/M_u$		1.32					1.49					1.50					1.49	
	$\Phi V_r/V_{npr}$		2.69					2.33					2.34					2.51	1.9%
	$P/A_g f'_c$		0.00					0.00					0.00					0.01	
Floor 8			1.24	1.16	1.46	0.52	2.08	0.99	1.30				0.99	1.17	1.46	0.52	2.08	1.34	
			1.13					1.00					1.00					1.13	
Story 7	$\Phi M_r/M_u$		1.15					1.18					1.18					1.19	
	$\Phi V_r/V_{npr}$		2.58					2.33					2.34					2.48	1.8%
	$P/A_g f'_c$		0.00					0.00					0.00					0.01	
Floor 7			1.05	1.14	1.25	0.51	1.16	0.85	1.34				0.85	1.14	1.26	0.51	1.16	1.10	
			1.38					1.23					1.23					1.38	
Story 6	$\Phi M_r/M_u$		1.17					1.20					1.20					1.25	
	$\Phi V_r/V_{npr}$		2.67					2.33					2.35					2.52	1.6%
	$P/A_g f'_c$		0.00					0.00					0.00					0.01	
Floor 6			1.05	1.15	1.25	0.51	1.16	0.84	1.32				0.85	1.15	1.25	0.51	1.16	1.08	
			1.38					1.23					1.23					1.38	
Story 5	$\Phi M_r/M_u$		1.20					1.18					1.18					1.19	
	$\Phi V_r/V_{npr}$		2.65					2.33					2.35					2.52	1.6%
	$P/A_g f'_c$		0.00					0.00					0.00					0.01	
Floor 5			1.05	1.15	1.17	0.50	1.15	0.84	1.31				0.84	1.15	1.17	0.50	1.15	1.04	
			1.36					1.22					1.22					1.36	
Story 4	$\Phi M_r/M_u$		1.16					1.16					1.16					1.17	
	$\Phi V_r/V_{npr}$		2.65					2.35					2.35					2.66	1.6%
	$P/A_g f'_c$		0.00					0.00					0.00					0.00	
Floor 4			1.04	1.15	1.16	0.53	1.15	0.83	1.29				0.83	1.15	1.16	0.53	1.15	1.04	
			1.36					1.19					1.19					1.36	
Story 3	$\Phi M_r/M_u$		1.16					1.17					1.16					1.17	
	$\Phi V_r/V_{npr}$		2.65					2.3					2.35					2.65	1.6%
	$P/A_g f'_c$		0.00					0.00					0.00					0.00	
Floor 3			1.05	1.16	1.18	0.57	1.15	0.83	1.28				0.83	1.16	1.18	0.57	1.15	1.04	
			1.36					1.17					1.17					1.36	
Story 2	$\Phi M_r/M_u$		1.16					1.15					1.16					1.15	
	$\Phi V_r/V_{npr}$		2.7					2.26					2.27					2.64	1.7%
	$P/A_g f'_c$		0.00					0.00					0.00					0.00	
Floor 2			1.24	1.17	1.20	0.63	1.16	0.87	1.29				0.88	1.18	1.20	0.63	1.16	1.23	
			1.41					1.16					1.16					1.41	
Story 1	$\Phi M_r/M_u$		1.16	$(\Phi M_r/M_u)_{neg}$	$(\Phi M_r/M_u)_{pos}$	M_{pos}/M_{npr}	$\Phi V_r/V_{npr}$	1.15	$(\Phi M_r/M_u)_{neg}$	1.29			1.15	$(\Phi M_r/M_u)_{neg}$	1.18			1.16	1.5%
	$\Phi V_r/V_{npr}$		2.06					2.04	$(\Phi M_r/M_u)_{pos}$	1.66			2.05	$(\Phi M_r/M_u)_{pos}$	1.20			2.09	
	$P/A_g f'_c$		0.00					0.00	M_{pos}/M_{npr}	0.63			0.00	M_{pos}/M_{npr}	0.63			0.00	

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp} (k-in) = 3465$			$M_{y,exp} (k-in) = 4160$			$M_{y,exp} (k-in) = 4160$			3465
		$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.35$			
Floor 12	Story 11	$M_{y,exp} (k-in) = 3833$			$M_{y,exp} (k-in) = 4876$			$M_{y,exp} (k-in) = 4876$			3833
		$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.35$			
Floor 11	Story 10	$M_{y,exp} (k-in) = 4195$			$M_{y,exp} (k-in) = 5572$			$M_{y,exp} (k-in) = 5572$			4195
		$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.35$			
Floor 10	Story 9	$M_{y,exp} (k-in) = 4876$			$M_{y,exp} (k-in) = 6252$			$M_{y,exp} (k-in) = 6252$			4795
		$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.35$			
Floor 9	Story 8	$M_{y,exp} (k-in) = 5191$			$M_{y,exp} (k-in) = 6506$			$M_{y,exp} (k-in) = 6506$			6271
		$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.40$			$E_{eff}/E_{lg} = 0.40$			
Floor 8	Story 7	$M_{y,exp} (k-in) = 5694$			$M_{y,exp} (k-in) = 9155$			$M_{y,exp} (k-in) = 9155$			6614
		$E_{eff}/E_{lg} = 0.35$			$E_{eff}/E_{lg} = 0.43$			$E_{eff}/E_{lg} = 0.43$			
Floor 7	Story 6	$M_{y,exp} (k-in) = 5873$			$M_{y,exp} (k-in) = 9794$			$M_{y,exp} (k-in) = 9794$			6953
		$E_{eff}/E_{lg} = 0.36$			$E_{eff}/E_{lg} = 0.46$			$E_{eff}/E_{lg} = 0.46$			
Floor 6	Story 5	$M_{y,exp} (k-in) = 7208$			$M_{y,exp} (k-in) = 10423$			$M_{y,exp} (k-in) = 10423$			7288
		$E_{eff}/E_{lg} = 0.38$			$E_{eff}/E_{lg} = 0.48$			$E_{eff}/E_{lg} = 0.48$			
Floor 5	Story 4	$M_{y,exp} (k-in) = 7541$			$M_{y,exp} (k-in) = 11044$			$M_{y,exp} (k-in) = 11044$			7621
		$E_{eff}/E_{lg} = 0.39$			$E_{eff}/E_{lg} = 0.51$			$E_{eff}/E_{lg} = 0.51$			
Floor 4	Story 3	$M_{y,exp} (k-in) = 7870$			$M_{y,exp} (k-in) = 11736$			$M_{y,exp} (k-in) = 11657$			7950
		$E_{eff}/E_{lg} = 0.40$			$E_{eff}/E_{lg} = 0.54$			$E_{eff}/E_{lg} = 0.54$			
Floor 3	Story 2	$M_{y,exp} (k-in) = 8197$			$M_{y,exp} (k-in) = 12421$			$M_{y,exp} (k-in) = 12421$			8276
		$E_{eff}/E_{lg} = 0.42$			$E_{eff}/E_{lg} = 0.56$			$E_{eff}/E_{lg} = 0.56$			
Floor 2	Story 1	$M_{y,exp} (k-in) = 8789$			$M_{y,exp} (k-in) = 14552$			$M_{y,exp} (k-in) = 14552$			9710
		$E_{eff}/E_{lg} = 0.43$			$E_{eff}/E_{lg} = 0.59$			$E_{eff}/E_{lg} = 0.59$			

Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60

Model periods (sec): $T_1 = 1.99$ $T_2 = 0.68$ $T_3 = 0.39$

$I_{n,relax,expected} = 67$ ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2033

Number of Stories: 12

Fundamental Period (sec): 1.97

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2033 SCWB=3.0

Simply adding more reinforcements in the column from the 2014 was sufficient to obtain SCWB=3.0. Drift controls this design. Both positive and negative bending strength were controlled by strength demands. Additional beam reinforcements in 12 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. Then 24 bays had additional reinforcement added to meet the minimum positive/negative strength ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except 1 by flexural demand. Beam stirrups were controlled by the capacity shear design and minimum requirement (half-half). Most column stirrups were controlled by the minimum confinement requirement, four by shear demand.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design base shear = 0.044 g, 232 k	
f'_c beams = 5.0 ksi	$f'_{c,cols,upper}$ = 5.0 ksi
f_y rebar, nom. = 60 ksi	$f'_{c,cols,lower}$ = 5.0 ksi

Design Documentation (2 of 2)											
SCWB = 1.28											
Joint $\Phi V_r/V_u = 2.94$											
Story 12	$\Phi M_r/M_u = 1.72$										
	$\Phi V_r/V_{mpc} = 2.13$										
	$P/A_g f'_c = 0.02$										
Design Drifts:											
0.8%											
Floor 12											
Story 11	$\Phi M_r/M_u = 5.44$										
	$\Phi V_r/V_{mpc} = 1.15$										
	$P/A_g f'_c = 0.03$										
Design Drifts:											
1.1%											
Floor 11											
Story 10	$\Phi M_r/M_u = 2.04$										
	$\Phi V_r/V_{mpc} = 2.48$										
	$P/A_g f'_c = 0.05$										
Design Drifts:											
1.5%											
Floor 10											
Story 9	$\Phi M_r/M_u = 5.13$										
	$\Phi V_r/V_{mpc} = 1.21$										
	$P/A_g f'_c = 0.07$										
Design Drifts:											
1.8%											
Floor 9											
Story 8	$\Phi M_r/M_u = 1.72$										
	$\Phi V_r/V_{mpc} = 2.83$										
	$P/A_g f'_c = 0.09$										
Design Drifts:											
2.0%											
Floor 8											
Story 7	$\Phi M_r/M_u = 3.22$										
	$\Phi V_r/V_{mpc} = 1.34$										
	$P/A_g f'_c = 0.11$										
Design Drifts:											
1.8%											
Floor 7											
Story 6	$\Phi M_r/M_u = 3.22$										
	$\Phi V_r/V_{mpc} = 1.47$										
	$P/A_g f'_c = 0.13$										
Design Drifts:											
1.5%											
Floor 6											
Story 5	$\Phi M_r/M_u = 3.50$										
	$\Phi V_r/V_{mpc} = 1.35$										
	$P/A_g f'_c = 0.15$										
Design Drifts:											
1.5%											
Floor 5											
Story 4	$\Phi M_r/M_u = 3.12$										
	$\Phi V_r/V_{mpc} = 1.45$										
	$P/A_g f'_c = 0.18$										
Design Drifts:											
1.6%											
Floor 4											
Story 3	$\Phi M_r/M_u = 3.45$										
	$\Phi V_r/V_{mpc} = 1.32$										
	$P/A_g f'_c = 0.20$										
Design Drifts:											
1.6%											
Floor 3											
Story 2	$\Phi M_r/M_u = 3.04$										
	$\Phi V_r/V_{mpc} = 1.4$										
	$P/A_g f'_c = 0.22$										
Design Drifts:											
1.6%											
Floor 2											
Story 1	$\Phi M_r/M_u = 2.35$										
	$\Phi V_r/V_{mpc} = 1.38$										
	$P/A_g f'_c = 0.24$										
Design Drifts:											
1.3%											

Modeling Documentation (1 of 1)												
13	Story 12	12	11	10	9	8	7	6	5	4	3	2
$M_{y,exp}$ (k-in) =	4840	5239	5828	6266	7557	13248	14236	13142	14126	13757	14527	13119
E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21
$\Theta_{exp,pl}$ (rad) =	0.075	0.066	0.066	0.075	0.075	0.066	0.067	0.065	0.064	0.062	0.064	0.060
Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
λ =	108	92	92	103	97	95	92	89	86	84	81	79
$(P/A_g f_c)_{exp}$ =	0.01	0.02	0.02	0.05	0.09	0.11	0.14	0.16	0.18	0.21	0.23	0.25
$M_{y,exp}$ (k-in) =	12289	16788	14911	10614	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.21	1.20	1.20	1.21	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.084	0.082	0.071	0.082	0.065	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	107	114	103	112	103	102	92	89	86	84	81	79
$(P/A_g f_c)_{exp}$ =	0.02	0.05	0.05	0.05	0.09	0.11	0.14	0.16	0.18	0.21	0.23	0.25
$M_{y,exp}$ (k-in) =	5632	8037	9810	8266	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.21	1.20	1.20	1.21	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.072	0.068	0.068	0.072	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	105	100	100	105	97	95	92	89	86	84	81	79
$(P/A_g f_c)_{exp}$ =	0.03	0.07	0.07	0.07	0.09	0.11	0.14	0.16	0.18	0.21	0.23	0.25
$M_{y,exp}$ (k-in) =	14181	19589	17924	12721	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.20	1.20	1.20	1.20	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.071	0.071	0.065	0.071	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	103	102	97	103	97	95	92	89	86	84	81	79
$(P/A_g f_c)_{exp}$ =	0.05	0.09	0.09	0.05	0.09	0.11	0.14	0.16	0.18	0.21	0.23	0.25
$M_{y,exp}$ (k-in) =	8403	10230	11786	7557	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.35	0.36	0.36	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.20	1.19	1.19	1.20	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.069	0.062	0.062	0.069	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	102	95	95	102	95	92	89	86	84	81	79	77
$(P/A_g f_c)_{exp}$ =	0.06	0.11	0.11	0.06	0.11	0.14	0.16	0.18	0.21	0.23	0.25	0.28
$M_{y,exp}$ (k-in) =	14497	19968	18513	13248	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.35	0.38	0.38	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.20	1.19	1.19	1.20	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.068	0.060	0.060	0.068	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	100	92	92	100	92	89	86	84	81	79	77	77
$(P/A_g f_c)_{exp}$ =	0.07	0.14	0.14	0.07	0.14	0.16	0.18	0.21	0.23	0.25	0.28	0.28
$M_{y,exp}$ (k-in) =	13092	19733	21085	14236	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.35	0.41	0.41	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.20	1.19	1.19	1.20	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.067	0.057	0.057	0.067	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	99	89	89	99	86	84	81	79	77	77	77	77
$(P/A_g f_c)_{exp}$ =	0.08	0.16	0.16	0.08	0.16	0.18	0.21	0.23	0.25	0.28	0.28	0.28
$M_{y,exp}$ (k-in) =	14285	19804	18453	13142	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.35	0.43	0.43	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.20	1.18	1.18	1.20	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.065	0.055	0.055	0.065	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	97	86	86	97	81	79	77	77	77	77	77	77
$(P/A_g f_c)_{exp}$ =	0.09	0.18	0.18	0.09	0.18	0.21	0.23	0.25	0.28	0.28	0.28	0.28
$M_{y,exp}$ (k-in) =	13191	19037	20284	14126	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.35	0.45	0.45	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.20	1.18	1.18	1.20	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.064	0.053	0.053	0.064	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.099	0.099	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	96	84	84	96	81	79	77	77	77	77	77	77
$(P/A_g f_c)_{exp}$ =	0.10	0.21	0.21	0.10	0.21	0.23	0.25	0.28	0.28	0.28	0.28	0.28
$M_{y,exp}$ (k-in) =	14691	20862	19614	13757	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.36	0.47	0.47	0.36	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.19	1.18	1.18	1.19	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.062	0.051	0.051	0.062	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.091	0.091	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	95	81	81	95	81	79	77	77	77	77	77	77
$(P/A_g f_c)_{exp}$ =	0.11	0.23	0.23	0.11	0.23	0.25	0.28	0.28	0.28	0.28	0.28	0.28
$M_{y,exp}$ (k-in) =	13698	20185	21330	14527	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.37	0.50	0.50	0.37	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.19	1.17	1.17	1.19	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.061	0.049	0.049	0.061	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.084	0.084	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	93	79	79	93	81	79	77	77	77	77	77	77
$(P/A_g f_c)_{exp}$ =	0.13	0.25	0.25	0.13	0.25	0.28	0.28	0.28	0.28	0.28	0.28	0.28
$M_{y,exp}$ (k-in) =	13948	20333	19189	13119	11786	18513	21085	18453	20284	19614	21330	19189
E_{eff}/E_g =	0.38	0.52	0.52	0.38	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
M_u/M_y =	1.19	1.17	1.17	1.19	1.19	1.19	1.19	1.18	1.18	1.18	1.17	1.17
$\Theta_{exp,pl}$ (rad) =	0.060	0.047	0.047	0.060	0.062	0.060	0.057	0.055	0.053	0.051	0.049	0.049
Θ_{pc} (rad) =	0.100	0.078	0.078	0.100	0.100	0.100	0.100	0.100	0.099	0.091	0.084	0.084
λ =	92	77	77	92	81	79	77	77	77	77	77	77
$(P/A_g f_c)_{exp}$ =	0.14	0.28	0.28	0.14	0.28	0.28	0.28	0.28	0.28	0.28	0.28	0.28

Mass tributary to one frame for lateral load (each floor) (k-s/in): 0.60

Model periods (sec): $T_1 = 1.97$ $T_2 = 0.67$ $T_3 = 0.38$

$f_{y,steel}$ = 67 ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2034

Number of Stories: 4

Fundamental Period (sec): 0.87

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2034 SCWB=0.4

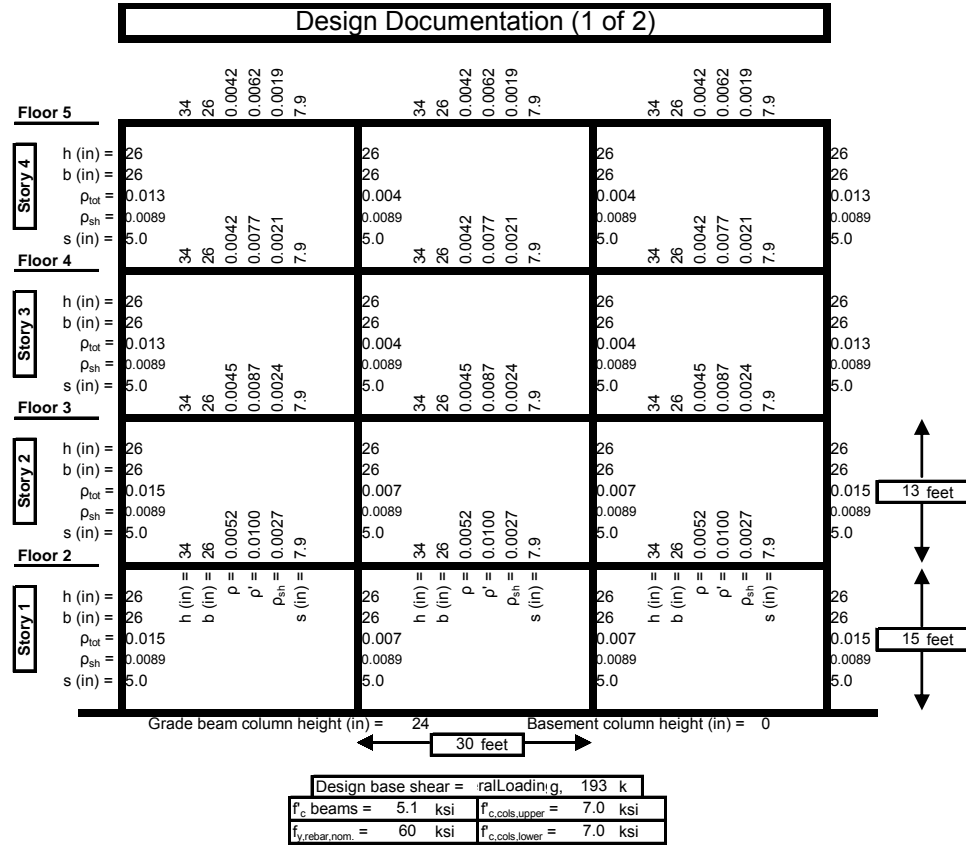
Note: for the purpose of sensitivity analysis, this design ignores the minimum requirement ratio in the columns. Furthermore, the beam and column sizes were kept as in design 2025, where only the column reinforcement are make less dense to reach smaller SCWB ratio.

Reason: By making column sizes smaller, beam sizes would have to be larger to satisfy drift.

Both positive and negative bending strength were controlled by strength demands. Additional beam reinforcements were added to alter beam design to use same ρ and ρ_{Prime} in each floor and to meet the minimum positive/negative strength ratio. All column flexural strengths were controlled by flexural strength demand. Beam stirrups were controlled by the capacity shear design and minimum requirement (half-half). Column stirrups were controlled by the minimum confinement requirement.

Note: Design for smaller SCWB were done but ignored for analysis because majority of the columns in this design (2034) were already controlled by flexural strength. Thus, even by making a less SCWB requirement will not increase capacity performance, and will analysis result will be misleading.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)									
Story 4	SCWB =	0.48	1.16	0.15	1.23	0.15	1.16	0.48	Design Drifts:
	Joint $\Phi V_n/V_u$	2.77		2.21		2.21		2.77	
	$\Phi M_r/M_u$	1.15		1.17		1.18		1.16	
	$\Phi V_n/V_{mpr}$	2.11		4.63		4.63		2.11	
	$P/A_g f_c$	0.03		0.06		0.06		0.03	0.6%
Floor 4		0.86	1.15		1.20		1.15	0.86	
		1.60		1.39		1.39		1.60	
Story 3	$\Phi M_r/M_u$	1.15		1.17		1.18		1.16	0.9%
	$\Phi V_n/V_{mpr}$	1.73		2.99		2.99		1.73	
	$P/A_g f_c$	0.05		0.10		0.10		0.05	
		0.90	1.13	0.49	1.20	0.49	1.13	0.90	
Floor 3		1.41		1.24		1.24		1.41	
Story 2	$\Phi M_r/M_u$	1.16		1.16		1.17		1.16	1.2%
	$\Phi V_n/V_{mpr}$	1.93		2.09		2.09		1.93	
	$P/A_g f_c$	0.08		0.16		0.16		0.08	
		0.80	1.14	0.50	1.25	0.50	1.14	0.80	
Floor 2		1.18		1.03		1.03		1.18	
Story 1	$\Phi M_r/M_u$	1.16		1.16		1.17		1.15	1.6%
	$\Phi V_n/V_{mpr}$	2.46		2.46		2.46		2.49	
	$P/A_g f_c$	0.11		0.21		0.21		0.11	

Modeling Documentation (1 of 1)													
Floor 5	Story 4	6727				-12002				3.823E+08			
		0.0272				-0.039				0.070			
Floor 4	Story 3	6717				-14239				3.490E+08			
		0.030				-0.046				0.076			
Floor 3	Story 2	7229				-15860				3.490E+08			
		0.030				-0.046				0.083			
Floor 2	Story 1	8410				-17750				3.490E+08			
		0.032				-0.049				0.092			
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.34													
Model periods (sec): T ₁ = 0.87 T ₂ = 0.27 T ₃ = 0.15													
f _{y, rebar, expected} = 67 ksi													

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2051

Number of Stories: 4

Fundamental Period (sec): 0.54

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

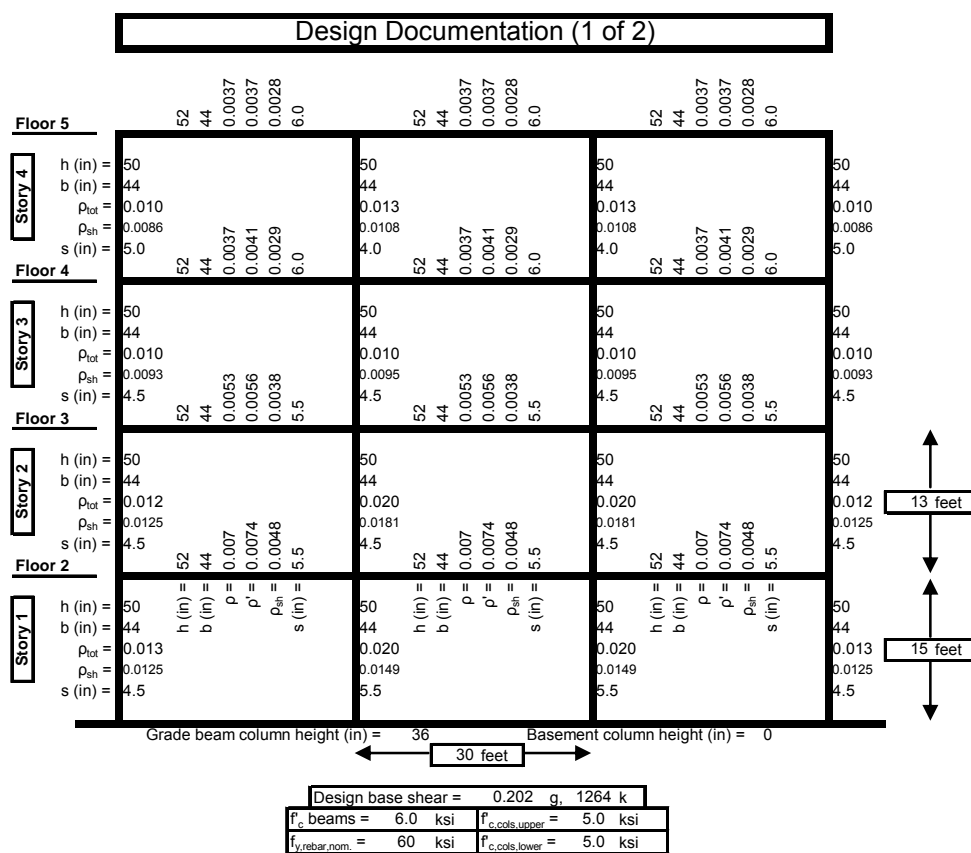
2051 Perimeter Frame, R=4

Note: this design contains very large element sizes in order to satisfy drift under a extremely large lateral force.

The beam and column sizes were controlled by drift limits, caused by large lateral force demand due to the low R value. Both negative and positive flexural beam strength was

controlled by strength demands, but then additional steel was added to 9 bays to meet the minimum reinforcement requirements (this was needed due to large beam sizes). Column flexural strengths were primarily controlled by the strong-column weak-beam ratio. In upper story exterior columns, the minimum longitudinal reinforcement ratio controlled (due to the large member sizes to control drift in lower stories) and caused the columns to be stronger. This could have been avoided by reducing column sizes in upper stories; we did not do this because the practitioner reviewing our designs said this would not be consistent with current practice. Beam stirrups were controlled by both minimum requirements and by the capacity shear design—half-half. Most of column stirrups were controlled by both minimum confinement requirements and 12 columns had added stirrups for capacity shear demand. Joint shear did not control design because elements were already large due to drift demands.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)									
Story 4	SCWB =	1.07	1.79	2.18	1.00	1.16	0.74	2.00	2.84
	Joint $\Phi V_n/V_u$ =	11.59					7.25		
	$\Phi M_n/M_u$ =	3.03					3.28		
	$\Phi V_n/V_{mpn}$ =	1.15					1.15		
Floor 4	$P/A_g f'_c$ =	0.01					0.01		
		1.92	1.15	1.22	0.91	1.15	1.31	1.28	1.41
		3.35					2.20		
Story 3	$\Phi M_n/M_u$ =	2.35					1.51		
	$\Phi V_n/V_{mpn}$ =	1.16					1.15		
	$P/A_g f'_c$ =	0.02					0.02		
		1.54	1.12	1.17	0.94	1.16	1.29	1.27	1.38
Floor 3		2.45					1.58		
Story 2	$\Phi M_n/M_u$ =	2.36					1.99		
	$\Phi V_n/V_{mpn}$ =	1.21					1.16		
	$P/A_g f'_c$ =	0.04					0.03		
		1.28	1.15	1.16	0.94	1.16	1.29	1.38	1.45
Floor 2		1.63					1.06		
Story 1	$\Phi M_n/M_u$ =	1.17					2.10		
	$\Phi V_n/V_{mpn}$ =	1.58					1.16		
	$P/A_g f'_c$ =	0.06					0.05		
		1.63	1.16	1.17	0.94	1.16	1.30	1.14	1.20
Design Drifts:									
0.7%									
1.2%									
1.5%									
2.0%									

Modeling Documentation (1 of 1)									
Floor 5	$M_{y,exp}$ (k-in) =	31915	25057	-28663	0.35	1.515E+09	0.0387	-0.041	0.096
	$E I_{eff}/E I_g$ =	0.35					129		
	M_c/M_y =	1.21							
	$\Theta_{cap,pl}$ (rad) =	0.074							
Floor 4	Θ_{pc} (rad) =	0.100							
	λ =	133							
	$(P/A_g f'_c)_{exp}$ =	0.00							
		25072	-31291	1.515E+09	0.039	-0.043	0.099	129	
Floor 3	$M_{y,exp}$ (k-in) =	32801	35448	-41192	0.35	1.515E+09	0.046	-0.050	0.100
	$E I_{eff}/E I_g$ =	0.35					134		
	M_c/M_y =	1.21							
	$\Theta_{cap,pl}$ (rad) =	0.078							
Floor 2	Θ_{pc} (rad) =	0.100							
	λ =	136							
	$(P/A_g f'_c)_{exp}$ =	0.01							
		35448	-41192	1.515E+09	0.046	-0.050	0.100	132	
Floor 1	$M_{y,exp}$ (k-in) =	40479	46683	-53334	0.35	1.515E+09	0.052	-0.056	0.100
	$E I_{eff}/E I_g$ =	0.35					133		
	M_c/M_y =	1.21							
	$\Theta_{cap,pl}$ (rad) =	0.089							
Story 1	Θ_{pc} (rad) =	0.100							
	λ =	134							
	$(P/A_g f'_c)_{exp}$ =	0.02							
		44360	46683	-53334	0.35	1.515E+09	0.052	-0.056	0.100
Mass tributary to one frame for lateral load (each floor) (k-s/in): 4.03									
Model periods (sec): $T_1 = 0.54$ $T_2 = 0.16$ $T_3 = 0.08$									
$f_{y,reb,expected} = 67$ ksi									

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2052

Number of Stories: 4

Fundamental Period (sec): 1.15

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2052 Perimeter Frame, R=12

The column size and beam depth were controlled by joint shear demand; drift did not control the design. Both negative and positive flexural beam strength was controlled by strength demands. More reinforcements were added to beams in four bays in order make consistent reinforcement for each floor, and few beams had additional reinforcement added to meet the minimum positive and negative ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, except the two exterior columns at the bottom story were controlled by strength demands. All beam stirrups were controlled by the capacity shear design. All of column stirrups were controlled by minimum confinement requirements, except 2.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (2 of 2)

Design Drifts:	0.7%	1.2%	1.4%	1.5%
Story 4 SCWB = 0.83 Joint $\Phi V_R/V_u = 3.71$ $\Phi M_{tr}/M_u = 1.70$ $\Phi V_{tr}/V_{mp} = 1.88$ $P/A_g f'_c = 0.01$	1.10 1.98 0.91 -1.79	0.75 2.91	1.15 2.61 0.91 -1.78	0.73 2.91
Floor 4 $\Phi M_{tr}/M_u = 1.70$ $\Phi V_{tr}/V_{mp} = 1.88$ $P/A_g f'_c = 0.01$	1.18 1.25 0.59 1.17	1.31 1.61	1.20 1.38 0.59 1.17	1.31 1.61
Story 3 $\Phi M_{tr}/M_u = 1.79$ $\Phi V_{tr}/V_{mp} = 1.42$ $P/A_g f'_c = 0.03$	1.18 1.12 0.62 1.17	2.00 1.52 0.04	2.05 1.49 0.04	2.05 1.49 0.04
Floor 3 $\Phi M_{tr}/M_u = 1.61$ $\Phi V_{tr}/V_{mp} = 1.15$ $P/A_g f'_c = 0.04$	1.18 1.12 0.62 1.17	1.27 1.23	1.21 1.23 0.62 1.16	1.28 1.23
Story 2 $\Phi M_{tr}/M_u = 1.61$ $\Phi V_{tr}/V_{mp} = 1.15$ $P/A_g f'_c = 0.04$	1.18 1.12 0.62 1.17	1.61 1.54 0.06	1.61 1.54 0.06	1.61 1.54 0.06
Floor 2 $\Phi M_{tr}/M_u = 1.16$ $\Phi V_{tr}/V_{mp} = 1.6$ $P/A_g f'_c = 0.05$	1.17 1.15 0.69 1.16	1.29 1.03	1.23 1.34 0.69 1.16	1.29 1.03
Story 1 $\Phi M_{tr}/M_u = 1.16$ $\Phi V_{tr}/V_{mp} = 1.6$ $P/A_g f'_c = 0.05$	$(\Phi M_{tr}/M_u)_{neg} = 1.17$ $(\Phi M_{tr}/M_u)_{pos} = 1.15$ $M_{n,pos}/M_{n,neg} = 0.69$ $\Phi V_{tr}/V_{mp} = 1.16$	1.16 1.73 0.08	$(\Phi M_{tr}/M_u)_{neg} = 1.16$ $(\Phi M_{tr}/M_u)_{pos} = 1.16$ $M_{n,pos}/M_{n,neg} = 0.69$ $\Phi V_{tr}/V_{mp} = 1.16$	1.16 1.73 0.08
Floor 1 $\Phi M_{tr}/M_u = 1.15$ $\Phi V_{tr}/V_{mp} = 1.61$ $P/A_g f'_c = 0.05$	$(\Phi M_{tr}/M_u)_{neg} = 1.18$ $(\Phi M_{tr}/M_u)_{pos} = 1.16$ $M_{n,pos}/M_{n,neg} = 0.69$ $\Phi V_{tr}/V_{mp} = 1.16$	1.15 1.61 0.05	$(\Phi M_{tr}/M_u)_{neg} = 1.18$ $(\Phi M_{tr}/M_u)_{pos} = 1.16$ $M_{n,pos}/M_{n,neg} = 0.69$ $\Phi V_{tr}/V_{mp} = 1.16$	1.15 1.61 0.05

Modeling Documentation (1 of 1)

Floor 5

Story 4

Floor 4

Story 3

Floor 3

Story 2

Floor 2

Story 1

$M_{y,exp}$ (k-in) =

8257

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.068

Θ_{pc} (rad) =

0.100

λ =

111

$(P/A_g f_c)_{exp}$ =

0.01

$M_{y,exp}$ (k-in) =

10638

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.068

Θ_{pc} (rad) =

0.100

λ =

110

$(P/A_g f_c)_{exp}$ =

0.02

$M_{y,exp}$ (k-in) =

12995

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.069

Θ_{pc} (rad) =

0.100

λ =

109

$(P/A_g f_c)_{exp}$ =

0.02

$M_{y,exp}$ (k-in) =

15504

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.069

Θ_{pc} (rad) =

0.100

λ =

108

$(P/A_g f_c)_{exp}$ =

0.03

$M_{y,pos,exp}$ (k-in) =

12521

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.078

Θ_{pc} (rad) =

0.100

λ =

127

$(P/A_g f_c)_{exp}$ =

0.01

$M_{y,pos,exp}$ (k-in) =

15024

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.077

Θ_{pc} (rad) =

0.100

λ =

125

$(P/A_g f_c)_{exp}$ =

0.03

$M_{y,pos,exp}$ (k-in) =

19248

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.074

Θ_{pc} (rad) =

0.100

λ =

119

$(P/A_g f_c)_{exp}$ =

0.04

$M_{y,pos,exp}$ (k-in) =

20437

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.19

$\Theta_{cap,pl}$ (rad) =

0.073

Θ_{pc} (rad) =

0.100

λ =

117

$(P/A_g f_c)_{exp}$ =

0.05

$M_{y,exp}$ (k-in) =

12267

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.078

Θ_{pc} (rad) =

0.100

λ =

127

$(P/A_g f_c)_{exp}$ =

0.01

$M_{y,exp}$ (k-in) =

15276

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.077

Θ_{pc} (rad) =

0.100

λ =

125

$(P/A_g f_c)_{exp}$ =

0.03

$M_{y,exp}$ (k-in) =

19248

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.074

Θ_{pc} (rad) =

0.100

λ =

119

$(P/A_g f_c)_{exp}$ =

0.04

$M_{y,exp}$ (k-in) =

20437

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.19

$\Theta_{cap,pl}$ (rad) =

0.073

Θ_{pc} (rad) =

0.100

λ =

117

$(P/A_g f_c)_{exp}$ =

0.05

$M_{y,exp}$ (k-in) =

12267

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.078

Θ_{pc} (rad) =

0.100

λ =

127

$(P/A_g f_c)_{exp}$ =

0.01

$M_{y,exp}$ (k-in) =

15276

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.077

Θ_{pc} (rad) =

0.100

λ =

125

$(P/A_g f_c)_{exp}$ =

0.03

$M_{y,exp}$ (k-in) =

19248

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.074

Θ_{pc} (rad) =

0.100

λ =

119

$(P/A_g f_c)_{exp}$ =

0.04

$M_{y,exp}$ (k-in) =

20437

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.19

$\Theta_{cap,pl}$ (rad) =

0.073

Θ_{pc} (rad) =

0.100

λ =

117

$(P/A_g f_c)_{exp}$ =

0.05

$M_{y,exp}$ (k-in) =

12267

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.078

Θ_{pc} (rad) =

0.100

λ =

127

$(P/A_g f_c)_{exp}$ =

0.01

$M_{y,exp}$ (k-in) =

15276

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.077

Θ_{pc} (rad) =

0.100

λ =

125

$(P/A_g f_c)_{exp}$ =

0.03

$M_{y,exp}$ (k-in) =

19248

E_{lsf}/E_{lg} =

0.35

M_z/M_y =

1.20

$\Theta_{cap,pl}$ (rad) =

0.074

Θ_{pc} (rad) =

0.100

λ =

119

$(P/A_g f_c)_{exp}$ =

0.04

$M_{y,exp}$ (k-in) =

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2053

Number of Stories: 12

Fundamental Period (sec): 1.50

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2053 Perimeter Frame, R=4

From the baseline design, beam and column sizes were made larger as the lateral demand is larger from smaller R value. Joint shear controls and made columns larger. Most positive bending strengths were controlled by primarily strength demand, 6 bays had min reinforcement imposed. Negative bending strengths were controlled by strength demands,

except 3. Additional beam reinforcements were added to alter beam design to use same ρ and ρ_{Prime} in each floor. Most column flexural strengths were controlled by the strong-column weak-beam ratio, but the bottom story columns were controlled by flexural demands. Beam stirrups were controlled by both the capacity shear design and min reinforcement requirement. Column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION

FIGURES

Design Documentation (1 of 2)															
Floor 13			40	30	0.0033	0.0033	0.0028	7.0	40	30	0.0033	0.0033	0.0028	7.0	
Story 12	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.010							0.010					0.010	0.010
	ρ_{sh} =	0.0085							0.0100					0.0100	0.0085
	s (in) =	4.5							4.5					4.5	4.5
Floor 12			40	30	0.0033	0.0035	0.0029	6.5	40	30	0.0033	0.0035	0.0029	6.5	
Story 11	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.010							0.010					0.010	0.010
	ρ_{sh} =	0.0085							0.0100					0.0100	0.0085
	s (in) =	4.5							4.5					4.5	4.5
Floor 11			40	30	0.0043	0.0045	0.0035	7.0	40	30	0.0043	0.0045	0.0035	7.0	
Story 10	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.010							0.010					0.010	0.010
	ρ_{sh} =	0.0085							0.0100					0.0100	0.0085
	s (in) =	4.5							4.5					4.5	4.5
Floor 10			40	30	0.0053	0.0058	0.0043	6.0	40	30	0.0053	0.0058	0.0043	6.0	
Story 9	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.010							0.013					0.013	0.010
	ρ_{sh} =	0.0085							0.0100					0.0100	0.0085
	s (in) =	4.5							4.5					4.5	4.5
Floor 9			40	30	0.006	0.0063	0.0047	5.5	40	30	0.006	0.0063	0.0047	5.5	
Story 8	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.012							0.013					0.013	0.012
	ρ_{sh} =	0.0085							0.0100					0.0100	0.0085
	s (in) =	4.5							4.5					4.5	4.5
Floor 8			40	30	0.0067	0.007	0.0052	5.0	40	30	0.0067	0.007	0.0052	5.0	
Story 7	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.013							0.016					0.016	0.013
	ρ_{sh} =	0.0085							0.0100					0.0100	0.0085
	s (in) =	4.5							4.5					4.5	4.5
Floor 7			40	30	0.0072	0.0075	0.0055	5.5	40	30	0.0072	0.0075	0.0055	5.5	
Story 6	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.014							0.016					0.016	0.014
	ρ_{sh} =	0.0119							0.0140					0.0140	0.0119
	s (in) =	4.5							4.5					4.5	4.5
Floor 6			40	30	0.0075	0.0080	0.0057	5.5	40	30	0.0075	0.0080	0.0057	5.5	
Story 5	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.017							0.016					0.016	0.017
	ρ_{sh} =	0.0119							0.0140					0.0140	0.0119
	s (in) =	4.5							4.5					4.5	4.5
Floor 5			40	30	0.0077	0.0083	0.0059	5.5	40	30	0.0077	0.0083	0.0059	5.5	
Story 4	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.017							0.016					0.016	0.017
	ρ_{sh} =	0.0119							0.0140					0.0140	0.0119
	s (in) =	4.5							4.5					4.5	4.5
Floor 4			40	30	0.008	0.0083	0.006	5.0	40	30	0.008	0.0083	0.006	5.0	
Story 3	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.019							0.016					0.016	0.019
	ρ_{sh} =	0.0119							0.0140					0.0140	0.0119
	s (in) =	4.5							4.5					4.5	4.5
Floor 3			40	30	0.0077	0.0081	0.0058	5.5	40	30	0.0077	0.0081	0.0058	5.5	
Story 2	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.019							0.016					0.016	0.019
	ρ_{sh} =	0.0119							0.0140					0.0140	0.0119
	s (in) =	4.5							4.5					4.5	4.5
Floor 2			40	30	0.0071	0.0073	0.0054	6.0	40	30	0.0071	0.0073	0.0054	6.0	
Story 1	h (in) =	38							44					44	38
	b (in) =	30							30					30	30
	ρ_{ext} =	0.024							0.024					0.024	0.024
	ρ_{sh} =	0.0119							0.0140					0.0140	0.0119
	s (in) =	4.5							4.5					4.5	4.5
Grade beam column height (in) =			24						Basement column height (in) = 44						
			20 feet												
Design base shear =			0.079 g, 663 k												
f'_c beams =			5.0 ksi			f'_c cols upper =			5.0 ksi						
f_y rebar nom =			60 ksi			f'_c cols lower =			7.0 ksi						

13 feet

15 feet

Design Documentation (2 of 2)										
	SCWB =	1.12	3.01	5.01	1.00	1.16				
	Joint $\Phi V_n/V_u$	5.40								
Story 12	$\Phi M_r/M_u$ =	5.00								
	$\Phi V_n/V_{npr}$ =	1.72								
	$P/A_g f_c$ =	0.01								
Floor 12		2.14	1.54	2.04	0.95	1.15				
		3.18								
Story 11	$\Phi M_r/M_u$ =	3.07								
	$\Phi V_n/V_{npr}$ =	1.63								
	$P/A_g f_c$ =	0.01								
Floor 11		1.71	1.33	1.57	0.95	1.16				
		2.47								
Story 10	$\Phi M_r/M_u$ =	2.05								
	$\Phi V_n/V_{npr}$ =	1.54								
	$P/A_g f_c$ =	0.03								
Floor 10		1.35	1.32	1.42	0.92	1.16				
		1.82								
Story 9	$\Phi M_r/M_u$ =	1.56								
	$\Phi V_n/V_{npr}$ =	1.73								
	$P/A_g f_c$ =	0.04								
Floor 9		1.30	1.21	1.32	0.96	1.16				
		1.57								
Story 8	$\Phi M_r/M_u$ =	1.49								
	$\Phi V_n/V_{npr}$ =	2.43								
	$P/A_g f_c$ =	0.05								
Floor 8		1.30	1.20	1.29	0.96	1.16				
		1.40								
Story 7	$\Phi M_r/M_u$ =	1.44								
	$\Phi V_n/V_{npr}$ =	2.11								
	$P/A_g f_c$ =	0.07								
Floor 7		1.29	1.18	1.26	0.97	1.15				
		1.55								
Story 6	$\Phi M_r/M_u$ =	1.36								
	$\Phi V_n/V_{npr}$ =	2.57								
	$P/A_g f_c$ =	0.06								
Floor 6		1.31	1.19	1.21	0.94	1.15				
		1.45								
Story 5	$\Phi M_r/M_u$ =	1.44								
	$\Phi V_n/V_{npr}$ =	2.22								
	$P/A_g f_c$ =	0.07								
Floor 5		1.29	1.19	1.19	0.93	1.16				
		1.39								
Story 4	$\Phi M_r/M_u$ =	1.29								
	$\Phi V_n/V_{npr}$ =	2.14								
	$P/A_g f_c$ =	0.08								
Floor 4		1.33	1.16	1.18	0.96	1.16				
		1.39								
Story 3	$\Phi M_r/M_u$ =	1.41								
	$\Phi V_n/V_{npr}$ =	1.89								
	$P/A_g f_c$ =	0.10								
Floor 3		1.40	1.12	1.14	0.96	1.15				
		1.43								
Story 2	$\Phi M_r/M_u$ =	1.24								
	$\Phi V_n/V_{npr}$ =	1.83								
	$P/A_g f_c$ =	0.11								
Floor 2		1.71	1.12	1.13	0.97	1.15				
		1.58								
Story 1	$\Phi M_r/M_u$ =	1.15								
	$\Phi V_n/V_{npr}$ =	1.53								
	$P/A_g f_c$ =	0.12								
			$(\Phi M_r/M_u)_{neg}$ = 1.12	$(\Phi M_r/M_u)_{pos}$ = 1.13	M_{top}/M_{neg} = 0.97	$\Phi V_n/V_{npr}$ = 1.15				
							$(\Phi M_r/M_u)_{neg}$ = 1.14	$(\Phi M_r/M_u)_{pos}$ = 1.13	M_{top}/M_{neg} = 0.97	$\Phi V_n/V_{npr}$ = 1.16

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	12061	8790	12061	16808	8790	16808	12061	8790	12061
		E_{eff}/E_g =	0.35	-10578	0.35	0.35	-10578	0.35	0.35	-10578	0.35
		M_u/M_y =	1.21	0.35	1.21	1.21	0.35	1.21	1.21	0.35	1.21
		$\Theta_{exp,pl}$ (rad) =	0.077	0.100	0.077	0.082	0.100	0.082	0.077	0.100	0.077
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	127	132	127	132	132	127	127	132	127
Floor 12	Story 11	$(P/A_y f_c)_{exp}$ =	0.00	8792	-11053	0.01	-11053	0.01	0.01	8792	127
		$M_{y,exp}$ (k-in) =	12359	17497	12359	17497	17497	12359	12359	17497	12359
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21
		$\Theta_{exp,pl}$ (rad) =	0.076	0.081	0.076	0.081	0.081	0.076	0.076	0.081	0.076
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 11	Story 10	λ =	126	131	126	131	131	126	126	131	126
		$(P/A_y f_c)_{exp}$ =	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
		$M_{y,exp}$ (k-in) =	12656	18180	12656	18180	18180	12656	12656	18180	12656
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21
		$\Theta_{exp,pl}$ (rad) =	0.076	0.080	0.076	0.080	0.080	0.076	0.076	0.080	0.076
Floor 10	Story 9	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	126	130	126	130	130	126	126	130	126
		$(P/A_y f_c)_{exp}$ =	0.01	0.02	0.01	0.02	0.02	0.01	0.01	0.02	0.01
		$M_{y,exp}$ (k-in) =	12952	23539	12952	23539	23539	12952	12952	23539	12952
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21
Floor 9	Story 8	$\Theta_{exp,pl}$ (rad) =	0.075	0.081	0.075	0.081	0.081	0.075	0.075	0.081	0.075
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	125	129	125	129	129	125	125	129	125
		$(P/A_y f_c)_{exp}$ =	0.01	0.02	0.01	0.02	0.02	0.01	0.01	0.02	0.01
		$M_{y,exp}$ (k-in) =	15239	24204	15239	24204	24204	15239	15239	24204	15239
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 8	Story 7	M_u/M_y =	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21
		$\Theta_{exp,pl}$ (rad) =	0.076	0.080	0.076	0.080	0.080	0.076	0.076	0.080	0.076
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	125	128	125	128	128	125	125	128	125
		$(P/A_y f_c)_{exp}$ =	0.02	0.03	0.02	0.03	0.03	0.02	0.02	0.03	0.02
		$M_{y,exp}$ (k-in) =	17232	29526	17232	29526	29526	17232	17232	29526	17232
Floor 7	Story 6	E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21	1.21
		$\Theta_{exp,pl}$ (rad) =	0.076	0.081	0.076	0.081	0.081	0.076	0.076	0.081	0.076
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	124	127	124	127	127	124	124	127	124
		$(P/A_y f_c)_{exp}$ =	0.02	0.04	0.02	0.04	0.04	0.02	0.02	0.04	0.02
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	18740	30317	18740	30317	30317	18740	18740	30317	18740
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19
		$\Theta_{exp,pl}$ (rad) =	0.082	0.087	0.082	0.087	0.087	0.082	0.082	0.087	0.082
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	125	128	125	128	128	125	125	128	125
Floor 5	Story 4	$(P/A_y f_c)_{exp}$ =	0.02	0.03	0.02	0.03	0.03	0.02	0.02	0.03	0.02
		$M_{y,exp}$ (k-in) =	21581	30974	21581	30974	30974	21581	21581	30974	21581
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19
		$\Theta_{exp,pl}$ (rad) =	0.083	0.086	0.083	0.086	0.086	0.083	0.083	0.086	0.083
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
Floor 4	Story 3	λ =	124	127	124	127	127	124	124	127	124
		$(P/A_y f_c)_{exp}$ =	0.02	0.04	0.02	0.04	0.04	0.02	0.02	0.04	0.02
		$M_{y,exp}$ (k-in) =	24981	32279	24981	32279	32279	24981	24981	32279	24981
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19
		$\Theta_{exp,pl}$ (rad) =	0.084	0.085	0.084	0.085	0.085	0.084	0.084	0.085	0.084
Floor 3	Story 2	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	124	126	124	126	126	124	124	126	124
		$(P/A_y f_c)_{exp}$ =	0.02	0.04	0.02	0.04	0.04	0.02	0.02	0.04	0.02
		$M_{y,exp}$ (k-in) =	25263	32926	25263	32926	32926	25263	25263	32926	25263
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19
Floor 2	Story 1	$\Theta_{exp,pl}$ (rad) =	0.083	0.084	0.083	0.084	0.084	0.083	0.083	0.084	0.083
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	123	125	123	125	125	123	123	125	123
		$(P/A_y f_c)_{exp}$ =	0.03	0.05	0.03	0.05	0.05	0.03	0.03	0.05	0.03
		$M_{y,exp}$ (k-in) =	31190	46347	31190	46347	46347	31190	31190	46347	31190
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
		M_u/M_y =	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19	1.19
		$\Theta_{exp,pl}$ (rad) =	0.087	0.089	0.087	0.089	0.089	0.087	0.087	0.089	0.087
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
		λ =	123	125	123	125	125	123	123	125	123
		$(P/A_y f_c)_{exp}$ =	0.03	0.05	0.03	0.05	0.05	0.03	0.03	0.05	0.03
Mass tributary to one frame for lateral load (each floor) (k-s/s-in):										1.79	
Model periods (sec): $T_1 = 1.50$ $T_2 = 0.49$ $T_3 = 0.28$											
$f_{y,triber,exp} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2054

Number of Stories: 12

Fundamental Period (sec): 2.84

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2054 Perimeter Frame, $R=12$

From the baseline design, beam and column sizes were made larger as the lateral demand is larger from smaller R value. Joint shear controls thus columns larger. Positive and negative bending strengths were controlled by primarily strength demand, 6 bays had min reinforcement imposed. Additional beam reinforcements were added to alter beam design to use same ρ and ρ_{Prime} in each floor and to meet the minimum positive/negative strength ratio. Most column flexural strengths were controlled by the strong-column weak-beam ratio, 8 by strength demand. Beam stirrups were controlled by both the capacity shear design and min reinforcement requirement. Column stirrups were controlled by the minimum confinement requirement.

DESIGN AND MODELING DOCUMENTATION

FIGURES

Design Documentation (1 of 2)																				
Floor 13			30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0
Story 12	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
	p _{tot} = 0.010			0.010			0.010			0.010										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
Floor 12	s (in) = 4.5			5.0			5.0			4.5										
	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0		
	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
Story 11	p _{tot} = 0.010			0.010			0.010			0.010										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
	s (in) = 4.5			5.0			5.0			4.5										
	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0		
Floor 11	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
	p _{tot} = 0.010			0.010			0.010			0.010										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
Story 10	s (in) = 4.5			5.0			5.0			4.5										
	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0		
	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
Story 9	p _{tot} = 0.010			0.010			0.010			0.010										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
	s (in) = 4.5			5.0			5.0			4.5										
	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0		
Floor 10	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
	p _{tot} = 0.010			0.010			0.010			0.010										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
Story 8	s (in) = 4.5			5.0			5.0			4.5										
	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0	30	24	0.0032	0.0032	0.0025	6.0		
	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
Story 7	p _{tot} = 0.012			0.013			0.013			0.012										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
	s (in) = 4.5			5.0			5.0			4.5										
	30	24	0.0042	0.0058	0.0033	6.5	30	24	0.0042	0.0058	0.0033	6.5	30	24	0.0042	0.0058	0.0033	6.5		
Floor 9	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
	p _{tot} = 0.012			0.013			0.013			0.012										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
Story 6	s (in) = 4.5			5.0			5.0			4.5										
	30	24	0.005	0.0063	0.0036	6.5	30	24	0.005	0.0063	0.0036	6.5	30	24	0.005	0.0063	0.0036	6.5		
	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
Story 5	p _{tot} = 0.014			0.015			0.015			0.014										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
	s (in) = 4.5			5.0			5.0			4.5										
	30	24	0.0052	0.0068	0.0037	6.5	30	24	0.0052	0.0068	0.0037	6.5	30	24	0.0052	0.0068	0.0037	6.5		
Floor 7	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
	p _{tot} = 0.014			0.015			0.015			0.014										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
Story 4	s (in) = 4.5			5.0			5.0			4.5										
	30	24	0.0057	0.0073	0.004	6.5	30	24	0.0057	0.0073	0.004	6.5	30	24	0.0057	0.0073	0.004	6.5		
	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
Story 3	p _{tot} = 0.015			0.015			0.015			0.015										
	p _{sh} = 0.0081			0.0081			0.0081			0.0081										
	s (in) = 5.0			5.0			5.0			5.0										
	30	24	0.006	0.0076	0.0041	6.5	30	24	0.006	0.0076	0.0041	6.5	30	24	0.006	0.0076	0.0041	6.5		
Floor 4	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
	p _{tot} = 0.016			0.015			0.015			0.016										
	p _{sh} = 0.0096			0.0114			0.0114			0.0096										
Story 2	s (in) = 5.0			5.0			5.0			5.0										
	30	24	0.0062	0.0076	0.0042	6.5	30	24	0.0062	0.0076	0.0042	6.5	30	24	0.0062	0.0076	0.0042	6.5		
	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
Story 1	p _{tot} = 0.016			0.015			0.015			0.016										
	p _{sh} = 0.0096			0.0114			0.0114			0.0096										
	s (in) = 5.0			5.0			5.0			5.0										
	30	24	0.0058	0.0071	0.0040	6.5	30	24	0.0058	0.0071	0.004	6.5	30	24	0.0058	0.0071	0.004	6.5		
Floor 2	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
	p _{tot} = 0.016			0.015			0.015			0.016										
	p _{sh} = 0.0096			0.0114			0.0114			0.0096										
Story 1	s (in) = 5.0			5.0			5.0			5.0										
	30	24	0.0058	0.0071	0.004	6.5	30	24	0.0058	0.0071	0.004	6.5	30	24	0.0058	0.0071	0.004	6.5		
	h (in) = 26			30			30			26										
	b (in) = 24			24			24			24										
Grade beam column height (in) = 24																				
Basement column height (in) = 30																				
20 feet																				
13 feet																				
15 feet																				
Design base shear = 0.023 g, 196 k																				
f'c beams = 5.0 ksi				f'c cols upper = 5.0 ksi				f'c cols lower = 7.0 ksi				fy rebar nom = 60 ksi								

Design Documentation (2 of 2)																
	SCWB =	0.85	2.36	4.00	1.17	0.68	1.89	3.87	1.00	1.17	0.68	2.37	4.00	1.17	0.85	Design Drifts:
Joint $\Phi V_r/V_u$ =		3.61				2.60				2.60					3.61	
Story 12	$\Phi M_r/M_u$ =	3.00				3.62				3.63					3.01	
	$\Phi V_r/V_{mpr}$ =	2.25				2.18				2.18					2.25	0.7%
	$P/A_g f_c$ =	0.01				0.02				0.02					0.01	
Floor 12		1.72	1.27	2.86	1.00	1.41	1.24	2.01	1.00	1.16	1.41	1.28	2.87	1.00	1.72	
		2.78				2.01				2.01					2.78	
Story 11	$\Phi M_r/M_u$ =	2.46				2.57				2.57					2.47	
	$\Phi V_r/V_{mpr}$ =	2.09				1.98				1.98					2.09	1.0%
	$P/A_g f_c$ =	0.02				0.03				0.03					0.02	
Floor 11		1.50	1.18	1.70	0.81	1.35	1.17	1.37	0.81	1.15	1.35	1.18	1.70	0.81	1.50	
		2.26				1.80				1.80					2.26	
Story 10	$\Phi M_r/M_u$ =	1.88				1.85				1.85					1.88	
	$\Phi V_r/V_{mpr}$ =	1.95				1.87				1.87					1.95	1.3%
	$P/A_g f_c$ =	0.03				0.04				0.04					0.03	
Floor 10		1.39	1.20	1.43	0.76	1.33	1.21	1.22	0.76	1.16	1.33	1.20	1.44	0.76	1.39	
		1.74				1.44				1.44					1.74	
Story 9	$\Phi M_r/M_u$ =	1.85				1.91				1.91					1.85	
	$\Phi V_r/V_{mpr}$ =	1.88				2.29				2.29					1.88	1.5%
	$P/A_g f_c$ =	0.05				0.06				0.06					0.05	
Floor 9		1.34	1.19	1.30	0.74	1.36	1.22	1.16	0.74	1.17	1.36	1.19	1.31	0.74	1.34	
		1.47				1.22				1.22					1.47	
Story 8	$\Phi M_r/M_u$ =	1.61				1.69				1.69					1.61	
	$\Phi V_r/V_{mpr}$ =	2.96				2.62				2.62					2.97	1.7%
	$P/A_g f_c$ =	0.06				0.07				0.07					0.06	
Floor 8		1.35	1.17	1.31	0.80	1.32	1.20	1.21	0.80	1.15	1.32	1.17	1.31	0.80	1.35	
		1.35				1.09				1.09					1.35	
Story 7	$\Phi M_r/M_u$ =	1.70				1.71				1.71					1.70	
	$\Phi V_r/V_{mpr}$ =	2.51				2.32				2.32					2.51	1.8%
	$P/A_g f_c$ =	0.08				0.09				0.09					0.08	
Floor 7		1.35	1.17	1.23	0.78	1.34	1.21	1.17	0.78	1.16	1.34	1.17	1.23	0.78	1.35	
		1.48				1.20				1.20					1.48	
Story 6	$\Phi M_r/M_u$ =	1.55				1.63				1.63					1.55	
	$\Phi V_r/V_{mpr}$ =	3.23				2.98				2.98					3.23	1.9%
	$P/A_g f_c$ =	0.07				0.07				0.07					0.07	
Floor 6		1.34	1.16	1.18	0.79	1.34	1.20	1.16	0.79	1.16	1.34	1.16	1.18	0.79	1.34	
		1.43				1.15				1.15					1.43	
Story 5	$\Phi M_r/M_u$ =	1.58				1.59				1.59					1.58	
	$\Phi V_r/V_{mpr}$ =	2.93				2.87				2.87					2.93	1.9%
	$P/A_g f_c$ =	0.08				0.08				0.08					0.08	
Floor 5		1.30	1.17	1.16	0.79	1.32	1.22	1.17	0.79	1.16	1.32	1.17	1.16	0.79	1.30	
		1.36				1.10				1.10					1.36	
Story 4	$\Phi M_r/M_u$ =	1.45				1.58				1.58					1.45	
	$\Phi V_r/V_{mpr}$ =	2.88				2.77				2.77					2.88	2.0%
	$P/A_g f_c$ =	0.09				0.10				0.10					0.09	
Floor 4		1.27	1.18	1.15	0.79	1.31	1.24	1.19	0.79	1.16	1.31	1.18	1.15	0.79	1.27	
		1.32				1.06				1.06					1.32	
Story 3	$\Phi M_r/M_u$ =	1.50				1.58				1.58					1.50	
	$\Phi V_r/V_{mpr}$ =	2.65				2.68				2.68					2.65	2.0%
	$P/A_g f_c$ =	0.10				0.11				0.11					0.10	
Floor 3		1.29	1.18	1.18	0.83	1.33	1.24	1.25	0.83	1.16	1.33	1.18	1.18	0.83	1.30	
		1.32				1.04				1.04					1.32	
Story 2	$\Phi M_r/M_u$ =	1.40				1.56				1.56					1.40	
	$\Phi V_r/V_{mpr}$ =	2.57				2.59				2.59					2.57	1.8%
	$P/A_g f_c$ =	0.11				0.12				0.12					0.11	
Floor 2		1.34	1.18	1.15	0.83	1.45	1.25	1.30	0.83	1.16	1.45	1.18	1.15	0.83	1.34	
		1.41				1.12				1.12					1.41	
Story 1	$\Phi M_r/M_u$ =	0.93				1.15				1.15					0.94	
	$\Phi V_r/V_{mpr}$ =	2.5				2.51				2.51					2.5	1.3%
	$P/A_g f_c$ =	0.12				0.13				0.13					0.12	
	$(\Phi M_r/M_u)_{req}$ =		1.18									$(\Phi M_r/M_u)_{req}$ =	1.18			
	$(\Phi M_r/M_u)_{pos}$ =		1.15									$(\Phi M_r/M_u)_{pos}$ =	1.15			
	M_{top}/M_{bot} =		0.83									M_{top}/M_{bot} =	0.83			
	$\Phi V_r/V_{mpr}$ =		1.16									$\Phi V_r/V_{mpr}$ =	1.16			

$$f_{y \text{ rebar expected}} = 67 \text{ ksi}$$

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2055

Number of Stories: 12

Fundamental Period (sec): 2.01

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2055 Perimeter Frame, SCWB=1.5

Compared to the baseline 12 story, this build simply have more reinforcement in the columns. Both positive and negative beam bending strength were controlled by strength demands, except for 3 beams. Few additional beam reinforcements in 12 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. All column flexural strengths were controlled by the strong-column weak-beam ratio except for 4 outer columns by flexural and few columns at the upper floors of the building by minimum requirement. Beam stirrups were controlled by the capacity shear design and minimum requirement (36 by demand). Column stirrups were controlled by the capacity shear design and minimum requirement (4 by demand). As in the baseline design, joint shear controls the beam and column sizes.

DESIGN AND MODELING DOCUMENTATION

FIGURES

Design Documentation (1 of 2)												
Floor 13												
Story 12	h (in) =	32	28	0.0033	0.0025	5.5	32	28	0.0033	0.0025	5.5	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.010					0.010					0.010
	ρ_{sh} =	0.0075					0.0075					0.0075
	s (in) =	5.5					5.5					5.5
Floor 12												
Story 11	h (in) =	32	28	0.0033	0.0026	5.0	32	28	0.0033	0.0026	5.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.010					0.010					0.010
	ρ_{sh} =	0.0075					0.0075					0.0075
	s (in) =	5.5					5.5					5.5
Floor 11												
Story 10	h (in) =	32	28	0.004	0.0031	6.5	32	28	0.004	0.0031	6.5	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.010					0.015					0.010
	ρ_{sh} =	0.0075					0.0089					0.0075
	s (in) =	5.5					4.5					5.5
Floor 10												
Story 9	h (in) =	32	28	0.005	0.0036	5.5	32	28	0.005	0.0036	5.5	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.012					0.015					0.012
	ρ_{sh} =	0.0075					0.0075					0.0075
	s (in) =	5.5					5.5					5.5
Floor 9												
Story 8	h (in) =	32	28	0.0037	0.0048	6.5	32	28	0.0037	0.0048	6.5	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.012					0.020					0.012
	ρ_{sh} =	0.0075					0.0103					0.0075
	s (in) =	5.5					4.0					5.5
Floor 8												
Story 7	h (in) =	32	28	0.004	0.0034	6.0	32	28	0.004	0.0034	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.012					0.020					0.012
	ρ_{sh} =	0.0075					0.0075					0.0075
	s (in) =	5.5					5.5					5.5
Floor 7												
Story 6	h (in) =	32	28	0.0052	0.0041	5.0	32	28	0.0052	0.0041	5.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.015					0.025					0.015
	ρ_{sh} =	0.0075					0.0075					0.0075
	s (in) =	5.5					5.5					5.5
Floor 6												
Story 5	h (in) =	32	28	0.0057	0.0044	6.0	32	28	0.0057	0.0044	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.018					0.025					0.018
	ρ_{sh} =	0.0105					0.0105					0.0105
	s (in) =	4.0					4.0					4.0
Floor 5												
Story 4	h (in) =	32	28	0.006	0.0045	6.0	32	28	0.006	0.0045	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.019					0.028					0.019
	ρ_{sh} =	0.0105					0.0105					0.0105
	s (in) =	4.0					4.0					4.0
Floor 4												
Story 3	h (in) =	32	28	0.006	0.0045	6.0	32	28	0.006	0.0045	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.019					0.028					0.019
	ρ_{sh} =	0.0105					0.0105					0.0105
	s (in) =	4.0					4.0					4.0
Floor 3												
Story 2	h (in) =	32	28	0.0061	0.0066	6.0	32	28	0.0061	0.0066	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.020					0.028					0.020
	ρ_{sh} =	0.0105					0.0105					0.0105
	s (in) =	4.0					4.0					4.0
Floor 2												
Story 1	h (in) =	32	28	0.0061	0.0066	6.0	32	28	0.0061	0.0066	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.024					0.028					0.024
	ρ_{sh} =	0.0105					0.0105					0.0105
	s (in) =	4.0					4.0					4.0
<div> <div>Grade beam column height (in) = 24</div> <div>Basement column height (in) = 32</div> <div>20 feet</div> </div> <div> <div>Design base shear = 0.044 g, 368 k</div> <div> <div>f'_c beams = 5.0 ksi</div> <div>f'_c cols upper = 5.0 ksi</div> <div>f_y rebar nom = 60 ksi</div> <div>f'_c cols lower = 7.0 ksi</div> </div> </div>												

[illegible]

Modeling Documentation (1 of 1)												
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	7808	4981	4981	4981	4981	4981	4981	4981	4981	
		E_{eff}/E_g =	0.35	-6358	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 12	Story 11	$M_{y,exp}$ (k-in) =	8058	8058	8058	8058	8058	8058	8058	8058	8058	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 11	Story 10	$M_{y,exp}$ (k-in) =	9766	9766	9766	9766	9766	9766	9766	9766	9766	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 10	Story 9	$M_{y,exp}$ (k-in) =	10010	10010	10010	10010	10010	10010	10010	10010	10010	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	10254	10254	10254	10254	10254	10254	10254	10254	10254	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	12669	12669	12669	12669	12669	12669	12669	12669	12669	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	15149	15149	15149	15149	15149	15149	15149	15149	15149	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	15387	15387	15387	15387	15387	15387	15387	15387	15387	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	16347	16347	16347	16347	16347	16347	16347	16347	16347	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	16583	16583	16583	16583	16583	16583	16583	16583	16583	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	17719	17719	17719	17719	17719	17719	17719	17719	17719	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	20653	20653	20653	20653	20653	20653	20653	20653	20653	
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.79												
Model periods (sec): $T_1 = 2.01$ $T_2 = 0.68$ $T_3 = 0.39$												
$f_{y,eliber,exp}$ = 67 ksi												

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2056

Number of Stories: 12

Fundamental Period (sec): 2.01

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2056 Perimeter Frame, SCWB=2.0

Compared to the baseline 12 story, this build simply have more reinforcement in the columns. Both positive and negative bending strength were controlled by strength demands, except for 3 elements. Few additional beam reinforcements in 12 bays were added to alter beam design to use same rho and rhoPrime in each floor. All column flexural strengths were controlled by the strong-column weak-beam ratio except for 4 lower outer columns and 8 upper columns. Beam stirrups were controlled by the capacity shear design and minimum requirement (36 by demand). Column stirrups were controlled by the capacity shear design and minimum requirement (4 by demand). As in the baseline design, joint shear controls the beam and column sizes.

DESIGN AND MODELING DOCUMENTATION

FIGURES

Design Documentation (1 of 2)																
Floor 13																
Story 12	h (in) =	36	28	28	0.0032	28	28	0.0032	28	28	0.0032	36				
	b (in) =	28	28	28	0.0032	28	28	0.0032	28	28	0.0032	28				
	ρ_{tot} =	0.010	0.0086	0.0086	0.0045	0.0086	0.0086	0.0045	0.0086	0.0086	0.0086	0.010				
	ρ_{sh} =	0.0086	0.0086	0.0086	0.0026	0.0086	0.0086	0.0026	0.0086	0.0086	0.0086	0.0086				
	s (in) =	5.5	5.5	5.5	5.0	5.5	5.5	5.0	5.5	5.5	5.5	5.5				
Floor 12																
Story 11	h (in) =	36	28	28	0.0045	28	28	0.0045	28	28	0.0045	36				
	b (in) =	28	28	28	0.0058	28	28	0.0058	28	28	0.0058	28				
	ρ_{tot} =	0.013	0.0086	0.0086	0.0031	0.0102	0.0102	0.0031	0.0102	0.0102	0.0102	0.013				
	ρ_{sh} =	0.0086	0.0086	0.0086	0.0003	0.0086	0.0086	0.0003	0.0086	0.0086	0.0086	0.0086				
	s (in) =	5.5	5.5	5.5	6.0	4.5	4.5	6.0	4.5	4.5	4.5	5.5				
Floor 11																
Story 10	h (in) =	36	28	28	0.0055	28	28	0.0055	28	28	0.0055	36				
	b (in) =	28	28	28	0.0065	28	28	0.0065	28	28	0.0065	28				
	ρ_{tot} =	0.013	0.0086	0.0086	0.0035	0.0102	0.0102	0.0035	0.0102	0.0102	0.0102	0.013				
	ρ_{sh} =	0.0086	0.0086	0.0086	0.0003	0.0086	0.0086	0.0003	0.0086	0.0086	0.0086	0.0086				
	s (in) =	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5	5.5				
Floor 10																
Story 9	h (in) =	36	34	28	0.0052	28	28	0.0052	28	34	0.0052	36				
	b (in) =	28	28	28	0.0063	28	28	0.0063	28	28	0.0063	28				
	ρ_{tot} =	0.015	0.0086	0.0086	0.0039	0.0105	0.0105	0.0039	0.0105	0.0105	0.0105	0.015				
	ρ_{sh} =	0.0086	0.0086	0.0086	0.0003	0.0086	0.0086	0.0003	0.0086	0.0086	0.0086	0.0086				
	s (in) =	5.5	5.5	5.5	5.0	4.5	4.5	5.0	4.5	4.5	4.5	5.5				
Floor 9																
Story 8	h (in) =	36	34	28	0.0057	28	28	0.0057	28	34	0.0057	36				
	b (in) =	28	28	28	0.0065	28	28	0.0065	28	28	0.0065	28				
	ρ_{tot} =	0.015	0.0086	0.0086	0.0041	0.0105	0.0105	0.0041	0.0105	0.0105	0.0105	0.015				
	ρ_{sh} =	0.0086	0.0086	0.0086	0.0003	0.0086	0.0086	0.0003	0.0086	0.0086	0.0086	0.0086				
	s (in) =	5.5	5.5	5.5	5.0	5.5	5.5	5.0	5.5	5.5	5.5	5.5				
Floor 8																
Story 7	h (in) =	36	34	28	0.006	28	28	0.006	28	34	0.006	36				
	b (in) =	28	28	28	0.0070	28	28	0.0070	28	28	0.0070	28				
	ρ_{tot} =	0.015	0.0086	0.0086	0.0043	0.0105	0.0105	0.0043	0.0105	0.0105	0.0105	0.015				
	ρ_{sh} =	0.0086	0.0086	0.0086	0.0003	0.0086	0.0086	0.0003	0.0086	0.0086	0.0086	0.0086				
	s (in) =	5.5	5.5	5.5	4.5	5.5	5.5	4.5	5.5	5.5	5.5	5.5				
Floor 7																
Story 6	h (in) =	36	34	28	0.0062	28	28	0.0062	28	34	0.0062	36				
	b (in) =	28	28	28	0.0075	28	28	0.0075	28	28	0.0075	28				
	ρ_{tot} =	0.015	0.0120	0.0120	0.0045	0.0120	0.0120	0.0045	0.0120	0.0120	0.0120	0.015				
	ρ_{sh} =	0.0120	0.0120	0.0120	0.0003	0.0120	0.0120	0.0003	0.0120	0.0120	0.0120	0.0120				
	s (in) =	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Floor 6																
Story 5	h (in) =	36	34	28	0.0067	28	28	0.0067	28	34	0.0067	36				
	b (in) =	28	28	28	0.0078	28	28	0.0078	28	28	0.0078	28				
	ρ_{tot} =	0.018	0.0120	0.0120	0.0047	0.0120	0.0120	0.0047	0.0120	0.0120	0.0120	0.018				
	ρ_{sh} =	0.0120	0.0120	0.0120	0.0003	0.0120	0.0120	0.0003	0.0120	0.0120	0.0120	0.0120				
	s (in) =	4.5	4.5	4.5	5.5	4.5	4.5	5.5	4.5	4.5	4.5	4.5				
Floor 5																
Story 4	h (in) =	36	34	28	0.007	28	28	0.007	28	34	0.007	36				
	b (in) =	28	28	28	0.0078	28	28	0.0078	28	28	0.0078	28				
	ρ_{tot} =	0.018	0.0120	0.0120	0.0047	0.0120	0.0120	0.0047	0.0120	0.0120	0.0120	0.018				
	ρ_{sh} =	0.0120	0.0120	0.0120	0.0003	0.0120	0.0120	0.0003	0.0120	0.0120	0.0120	0.0120				
	s (in) =	4.5	4.5	4.5	5.5	4.5	4.5	5.5	4.5	4.5	4.5	4.5				
Floor 4																
Story 3	h (in) =	36	34	28	0.007	28	28	0.007	28	34	0.007	36				
	b (in) =	28	28	28	0.0078	28	28	0.0078	28	28	0.0078	28				
	ρ_{tot} =	0.018	0.0120	0.0120	0.0048	0.0120	0.0120	0.0048	0.0120	0.0120	0.0120	0.018				
	ρ_{sh} =	0.0120	0.0120	0.0120	0.0003	0.0120	0.0120	0.0003	0.0120	0.0120	0.0120	0.0120				
	s (in) =	4.5	4.5	4.5	5.5	4.5	4.5	5.5	4.5	4.5	4.5	4.5				
Floor 3																
Story 2	h (in) =	36	34	28	0.0063	28	28	0.0063	28	34	0.0063	36				
	b (in) =	28	28	28	0.0073	28	28	0.0073	28	28	0.0073	28				
	ρ_{tot} =	0.018	0.0120	0.0120	0.0044	0.0120	0.0120	0.0044	0.0120	0.0120	0.0120	0.018				
	ρ_{sh} =	0.0120	0.0120	0.0120	0.0003	0.0120	0.0120	0.0003	0.0120	0.0120	0.0120	0.0120				
	s (in) =	4.5	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	4.5	4.5				
Floor 2																
Story 1	h (in) =	36	34	28	0.0022	28	28	0.0022	28	34	0.0022	36				
	b (in) =	28	28	28	0.0073	28	28	0.0073	28	28	0.0073	28				
	ρ_{tot} =	0.022	0.0120	0.0120	0.0044	0.0120	0.0120	0.0044	0.0120	0.0120	0.0120	0.022				
	ρ_{sh} =	0.0120	0.0120	0.0120	0.0003	0.0120	0.0120	0.0003	0.0120	0.0120	0.0120	0.0120				
	s (in) =	4.5	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	4.5	4.5				
Grade beam column height (in) = 24																
Basement column height (in) = 36																
20 feet																
13 feet																
15 feet																
Design base shear = 0.044 g, 368 k																
<table><tr><td>f'_c beams = 5.0 ksi</td><td>f'_c cols upper = 5.0 ksi</td></tr><tr><td>f_y rebar nom. = 60 ksi</td><td>f_y rebar lower = 7.0 ksi</td></tr></table>													f'_c beams = 5.0 ksi	f'_c cols upper = 5.0 ksi	f_y rebar nom. = 60 ksi	f_y rebar lower = 7.0 ksi
f'_c beams = 5.0 ksi	f'_c cols upper = 5.0 ksi															
f_y rebar nom. = 60 ksi	f_y rebar lower = 7.0 ksi															

Design Documentation (2 of 2)									
SCWB = 2.04									
Joint $\Phi V_n/V_u = 5.12$									
Story 12	$\Phi M_r/M_u =$	3.59				2.79			2.80
	$\Phi V_n/V_{npr} =$	2.03				1.97			1.97
	$P/A_g f_c =$	0.01				0.01			0.01
Design Drifts:									
0.9%									
Floor 12									
Story 11	$\Phi M_r/M_u =$	3.60				3.40			3.40
	$\Phi V_n/V_{npr} =$	1.52				1.56			1.56
	$P/A_g f_c =$	0.01				0.02			0.02
Design Drifts:									
1.2%									
Floor 11									
Story 10	$\Phi M_r/M_u =$	3.48				2.69			2.69
	$\Phi V_n/V_{npr} =$	1.46				1.26			1.26
	$P/A_g f_c =$	0.02				0.03			0.03
Design Drifts:									
1.6%									
Floor 10									
Story 9	$\Phi M_r/M_u =$	2.49				2.44			2.51
	$\Phi V_n/V_{npr} =$	1.52				1.27			1.52
	$P/A_g f_c =$	0.03				0.04			0.03
Design Drifts:									
1.6%									
Floor 9									
Story 8	$\Phi M_r/M_u =$	2.55				2.39			2.55
	$\Phi V_n/V_{npr} =$	1.71				1.57			1.71
	$P/A_g f_c =$	0.05				0.05			0.05
Design Drifts:									
1.6%									
Floor 8									
Story 7	$\Phi M_r/M_u =$	2.28				2.25			2.29
	$\Phi V_n/V_{npr} =$	2.16				1.54			2.16
	$P/A_g f_c =$	0.06				0.06			0.06
Design Drifts:									
1.7%									
Floor 7									
Story 6	$\Phi M_r/M_u =$	2.06				2.13			2.07
	$\Phi V_n/V_{npr} =$	2.78				2.02			2.78
	$P/A_g f_c =$	0.05				0.05			0.05
Design Drifts:									
1.8%									
Floor 6									
Story 5	$\Phi M_r/M_u =$	2.33				2.37			2.34
	$\Phi V_n/V_{npr} =$	2.32				1.74			2.32
	$P/A_g f_c =$	0.06				0.06			0.06
Design Drifts:									
1.9%									
Floor 5									
Story 4	$\Phi M_r/M_u =$	2.17				2.32			2.17
	$\Phi V_n/V_{npr} =$	2.25				1.71			2.25
	$P/A_g f_c =$	0.07				0.07			0.07
Design Drifts:									
1.9%									
Floor 4									
Story 3	$\Phi M_r/M_u =$	1.99				2.29			1.99
	$\Phi V_n/V_{npr} =$	2.19				1.68			2.19
	$P/A_g f_c =$	0.08				0.07			0.08
Design Drifts:									
1.9%									
Floor 3									
Story 2	$\Phi M_r/M_u =$	1.77				2.19			1.77
	$\Phi V_n/V_{npr} =$	2.13				1.66			2.13
	$P/A_g f_c =$	0.09				0.08			0.09
Design Drifts:									
1.8%									
Floor 2									
Story 1	$\Phi M_r/M_u =$	1.15				1.72			1.15
	$\Phi V_n/V_{npr} =$	1.83				1.63			1.83
	$P/A_g f_c =$	0.10				0.09			0.10
Design Drifts:									
1.2%									

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	10049	10331	10331	10049					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.21	1.21	1.21	1.21					
		$\Theta_{cap,pl}$ (rad) =	0.072	0.071	0.071	0.072					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
		λ =	116	116	116	116					
Floor 12	Story 11	$(P/A_g f_c)_{exp}$ =	0.00	0.01	0.01	0.00					
		$M_{y,exp}$ (k-in) =	13173	15607	15607	13173					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.21	1.21	1.21	1.21					
		$\Theta_{cap,pl}$ (rad) =	0.073	0.084	0.084	0.073					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 11	Story 10	$(P/A_g f_c)_{exp}$ =	0.01	0.02	0.02	0.01					
		$M_{y,exp}$ (k-in) =	13449	16150	16150	13449					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.21	1.21	1.21	1.21					
		$\Theta_{cap,pl}$ (rad) =	0.073	0.072	0.072	0.073					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 10	Story 9	$(P/A_g f_c)_{exp}$ =	0.01	0.02	0.02	0.01					
		$M_{y,exp}$ (k-in) =	15137	23243	23243	15137					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.21	1.21	1.21	1.21					
		$\Theta_{cap,pl}$ (rad) =	0.073	0.088	0.088	0.073					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 9	Story 8	$(P/A_g f_c)_{exp}$ =	0.02	0.03	0.03	0.02					
		$M_{y,exp}$ (k-in) =	15409	23770	23770	15409					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.21	1.20	1.20	1.21					
		$\Theta_{cap,pl}$ (rad) =	0.072	0.074	0.074	0.072					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 8	Story 7	$(P/A_g f_c)_{exp}$ =	0.02	0.04	0.04	0.02					
		$M_{y,exp}$ (k-in) =	15680	24293	24293	15680					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.21	1.20	1.20	1.21					
		$\Theta_{cap,pl}$ (rad) =	0.072	0.073	0.073	0.072					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 7	Story 6	$(P/A_g f_c)_{exp}$ =	0.02	0.05	0.05	0.02					
		$M_{y,exp}$ (k-in) =	16028	24927	24927	16028					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.19	1.19	1.19	1.19					
		$\Theta_{cap,pl}$ (rad) =	0.082	0.084	0.084	0.082					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 6	Story 5	$(P/A_g f_c)_{exp}$ =	0.02	0.09	0.09	0.02					
		$M_{y,exp}$ (k-in) =	19354	29185	29185	19354					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.19	1.19	1.19	1.19					
		$\Theta_{cap,pl}$ (rad) =	0.084	0.086	0.086	0.084					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 5	Story 4	$(P/A_g f_c)_{exp}$ =	0.02	0.04	0.04	0.02					
		$M_{y,exp}$ (k-in) =	19623	29704	29704	19623					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.19	1.19	1.19	1.19					
		$\Theta_{cap,pl}$ (rad) =	0.083	0.085	0.085	0.083					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 4	Story 3	$(P/A_g f_c)_{exp}$ =	0.02	0.05	0.05	0.02					
		$M_{y,exp}$ (k-in) =	19891	30221	30221	19891					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.19	1.19	1.19	1.19					
		$\Theta_{cap,pl}$ (rad) =	0.083	0.084	0.084	0.083					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 3	Story 2	$(P/A_g f_c)_{exp}$ =	0.03	0.05	0.05	0.03					
		$M_{y,exp}$ (k-in) =	20158	30736	30736	20158					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.19	1.19	1.19	1.19					
		$\Theta_{cap,pl}$ (rad) =	0.082	0.083	0.083	0.082					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 2	Story 1	$(P/A_g f_c)_{exp}$ =	0.03	0.06	0.06	0.03					
		$M_{y,exp}$ (k-in) =	23933	31248	31248	23933					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.19	1.19	1.19	1.19					
		$\Theta_{cap,pl}$ (rad) =	0.084	0.083	0.083	0.084					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Floor 1	Story 0	$(P/A_g f_c)_{exp}$ =	0.03	0.07	0.07	0.03					
		$M_{y,exp}$ (k-in) =	29333	37248	37248	29333					
		E_{eff}/E_g =	0.35	0.35	0.35	0.35					
		M_u/M_y =	1.19	1.19	1.19	1.19					
		$\Theta_{cap,pl}$ (rad) =	0.084	0.083	0.083	0.084					
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100					
Mass tributary to one frame for lateral load (each floor) (k-s/s/in):							1.79				
Model periods (sec): $T_1 = 2.01$ $T_2 = 0.67$ $T_3 = 0.38$											
$f_{v,inter,exp,des} =$							67 ksi				

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2057

Number of Stories: 12

Fundamental Period (sec): 1.90

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2057 Perimeter Frame, SCWB=2.5

Initial column sizes were increased due to SCWB requirements as increasing column reinforcement was not sufficient, due to the maximum allowable reinforcement ratio. Both positive and negative bending strength were controlled by strength demands. Few additional beam reinforcements in 15 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. All column flexural strengths were controlled by the strong-column weak-beam ratio, except 2 lower exterior columns and 8 upper columns by minimum requirement. Beam stirrups were controlled by the capacity shear design and minimum requirement (34 by demand). Column stirrups were controlled by the capacity shear design and minimum requirement (4 by demand). As in the baseline design, joint shear controls the beam and column sizes.

DESIGN AND MODELING DOCUMENTATION

FIGURES

Design Documentation (1 of 2)

Floor 13		26	28	0.0032	0.0035	0.0024	5.5	26	28	0.0032	0.0035	0.0024	5.5	26	28	0.0032	0.0035	0.0024	5.5	
Story 12	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.010$		0.010		0.010		0.010		0.010		0.010		0.010		0.010		0.010		0.010	
	$\rho_{sh} = 0.0102$		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102	
	s (in) = 4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5	
Floor 12		26	28	0.0038	0.0050	0.0028	5.0	26	28	0.0038	0.0050	0.0028	5.0	26	28	0.0038	0.0050	0.0028	5.0	
Story 11	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.010$		0.010		0.010		0.010		0.010		0.010		0.010		0.010		0.010		0.010	
	$\rho_{sh} = 0.0102$		0.0102		0.0108		0.0108		0.0108		0.0108		0.0108		0.0108		0.0108		0.0102	
	s (in) = 4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5	
Floor 11		26	28	0.005	0.0063	0.0033	5.5	26	28	0.005	0.0063	0.0033	5.5	26	28	0.005	0.0063	0.0033	5.5	
Story 10	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.010$		0.010		0.010		0.010		0.010		0.010		0.010		0.010		0.010		0.010	
	$\rho_{sh} = 0.0102$		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102	
	s (in) = 4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5	
Floor 10		26	28	0.0058	0.0073	0.0036	5.5	26	28	0.0058	0.0073	0.0036	5.5	26	28	0.0058	0.0073	0.0036	5.5	
Story 9	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.010$		0.010		0.010		0.010		0.010		0.010		0.010		0.010		0.010		0.010	
	$\rho_{sh} = 0.0102$		0.0102		0.0107		0.0107		0.0107		0.0107		0.0107		0.0107		0.0107		0.0102	
	s (in) = 4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5	
Floor 9		32	28	0.0062	0.0073	0.0043	4.5	32	28	0.0062	0.0073	0.0043	4.5	32	28	0.0062	0.0073	0.0043	4.5	
Story 8	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.013$		0.013		0.020		0.020		0.020		0.020		0.020		0.020		0.020		0.013	
	$\rho_{sh} = 0.0102$		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102	
	s (in) = 4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5	
Floor 8		32	28	0.0065	0.0075	0.0044	4.5	32	28	0.0065	0.0075	0.0044	4.5	32	28	0.0065	0.0075	0.0044	4.5	
Story 7	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.013$		0.013		0.022		0.022		0.022		0.022		0.022		0.022		0.022		0.013	
	$\rho_{sh} = 0.0102$		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102		0.0102	
	s (in) = 4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5		4.5	
Floor 7		32	28	0.0067	0.0080	0.0046	4.5	32	28	0.0067	0.0080	0.0046	4.5	32	28	0.0067	0.0080	0.0046	4.5	
Story 6	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.013$		0.013		0.022		0.022		0.022		0.022		0.022		0.022		0.022		0.013	
	$\rho_{sh} = 0.0143$		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143	
	s (in) = 4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		3.0	
Floor 6		32	28	0.0072	0.0085	0.0049	4.0	32	28	0.0072	0.0085	0.0049	4.0	32	28	0.0072	0.0085	0.0049	4.0	
Story 5	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.015$		0.015		0.022		0.022		0.022		0.022		0.022		0.022		0.022		0.015	
	$\rho_{sh} = 0.0143$		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143	
	s (in) = 4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0	
Floor 5		32	28	0.0075	0.0088	0.005	5.5	32	28	0.0075	0.0088	0.005	5.5	32	28	0.0075	0.0088	0.005	5.5	
Story 4	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.015$		0.015		0.022		0.022		0.022		0.022		0.022		0.022		0.022		0.015	
	$\rho_{sh} = 0.0143$		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143	
	s (in) = 4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0	
Floor 4		32	28	0.0077	0.0088	0.0051	5.0	32	28	0.0077	0.0088	0.0051	5.0	32	28	0.0077	0.0088	0.0051	5.0	
Story 3	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.015$		0.015		0.022		0.022		0.022		0.022		0.022		0.022		0.022		0.015	
	$\rho_{sh} = 0.0143$		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143	
	s (in) = 4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0	
Floor 3		32	28	0.0077	0.0088	0.0051	5.0	32	28	0.0077	0.0088	0.0051	5.0	32	28	0.0077	0.0088	0.0051	5.0	
Story 2	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.016$		0.016		0.022		0.022		0.022		0.022		0.022		0.022		0.022		0.016	
	$\rho_{sh} = 0.0143$		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143	
	s (in) = 4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0	
Floor 2		32	28	0.0088	0.0078	0.0046	6.0	32	28	0.0088	0.0078	0.0046	6.0	32	28	0.0088	0.0078	0.0046	6.0	
Story 1	h (in) = 42		42		42		42		42		42		42		42		42		42	
	b (in) = 28		28		28		28		28		28		28		28		28		28	
	$\rho_{ext} = 0.016$		0.016		0.022		0.022		0.022		0.022		0.022		0.022		0.022		0.016	
	$\rho_{sh} = 0.0143$		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143		0.0143	
	s (in) = 4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0		4.0	

Grade beam column height (in) = 24

Basement column height (in) = 42

20 feet

Design base shear = 0.044 g, 368 k

$f_{c, beams} = 5.0$ ksi	$f_{c, cols upper} = 5.0$ ksi
$f_{c, rebar nom} = 60$ ksi	$f_{c, cols lower} = 7.0$ ksi

13 feet

15 feet

Design Documentation (2 of 2)											
Story 12	SCWB =	3.11	1.20		1.87	1.11		1.87	1.21		Design Drifts:
	Joint $\Phi V_n/V_u$ =	5.70	2.10		3.71	1.51		3.71	2.11		
	$\Phi M_r/M_u$ =	4.51			3.46			3.47			
	$\Phi V_n/V_{npr}$ =	2.08	1.37		2.03	1.12		2.03	1.37		
	$P/A_g f_c$ =	0.01			0.01			0.01			1.0%
Floor 12		4.72	1.17		3.01	1.12		3.01	1.18		1.4%
Story 11		3.24			2.31			2.31			
	$\Phi M_r/M_u$ =	3.48			2.96			2.96			
	$\Phi V_n/V_{npr}$ =	1.99	1.27		2.06	1.11		2.06	1.27		
	$P/A_g f_c$ =	0.01			0.02			0.02			1.7%
Floor 11		3.94	1.12		2.48	1.11		2.48	1.13		1.7%
Story 10		2.59			1.80			1.80			
	$\Phi M_r/M_u$ =	3.62			2.68			2.67			
	$\Phi V_n/V_{npr}$ =	1.9	1.24		1.84	1.11		1.84	1.25		
	$P/A_g f_c$ =	0.02			0.03			0.03			1.7%
Floor 10		3.47	1.16		2.24	1.15		2.24	1.17		1.7%
Story 9		2.17			1.51			1.51			
	$\Phi M_r/M_u$ =	2.17			1.52			1.53			
	$\Phi V_n/V_{npr}$ =	2.16	1.20		2.2	1.11		2.2	1.21		
	$P/A_g f_c$ =	0.03			0.04			0.04			1.6%
Floor 9		2.48	1.12		2.04	1.16		2.04	1.12		1.6%
Story 8		1.78			1.20			1.20			
	$\Phi M_r/M_u$ =	3.04			2.97			2.97			
	$\Phi V_n/V_{npr}$ =	2	1.21		1.33	1.11		1.33	1.22		
	$P/A_g f_c$ =	0.04			0.04			0.04			1.7%
Floor 8		2.62	1.13		2.71	1.17		2.71	1.13		1.7%
Story 7		1.73			1.16			1.16			
	$\Phi M_r/M_u$ =	2.78			3.10			3.10			
	$\Phi V_n/V_{npr}$ =	2.41	1.16		1.51	1.12		1.51	1.16		
	$P/A_g f_c$ =	0.05			0.05			0.05			1.7%
Floor 7		2.43	1.14		2.73	1.19		2.73	1.14		1.7%
Story 6		1.91			1.30			1.30			
	$\Phi M_r/M_u$ =	2.52			2.94			2.94			
	$\Phi V_n/V_{npr}$ =	2.55	1.16		1.64	1.15		1.64	1.16		
	$P/A_g f_c$ =	0.04			0.04			0.04			1.8%
Floor 6		2.48	1.15		2.60	1.21		2.60	1.15		1.8%
Story 5		1.80			1.22			1.22			
	$\Phi M_r/M_u$ =	2.78			2.82			2.82			
	$\Phi V_n/V_{npr}$ =	2.64	1.13		1.97	1.15		1.97	1.13		
	$P/A_g f_c$ =	0.05			0.05			0.05			1.8%
Floor 5		2.56	1.15		2.55	1.23		2.55	1.15		1.8%
Story 4		1.73			1.17			1.17			
	$\Phi M_r/M_u$ =	2.57			2.75			2.75			
	$\Phi V_n/V_{npr}$ =	2.56	1.13		1.94	1.19		1.94	1.13		
	$P/A_g f_c$ =	0.06			0.06			0.06			1.7%
Floor 4		2.49	1.13		2.55	1.22		2.54	1.13		1.8%
Story 3		1.73			1.16			1.16			
	$\Phi M_r/M_u$ =	2.34			2.70			2.70			
	$\Phi V_n/V_{npr}$ =	2.48	1.13		1.9	1.19		1.9	1.13		
	$P/A_g f_c$ =	0.07			0.06			0.06			1.7%
Floor 3		2.53	1.15		2.58	1.25		2.58	1.15		1.7%
Story 2		1.73			1.15			1.15			
	$\Phi M_r/M_u$ =	2.23			2.56			2.56			
	$\Phi V_n/V_{npr}$ =	2.29	1.13		1.87	1.22		1.87	1.13		
	$P/A_g f_c$ =	0.08			0.07			0.07			1.1%
Floor 2		2.86	1.15		2.92	1.29		2.92	1.15		1.1%
Story 1		1.95			1.30			1.30			
	$\Phi M_r/M_u$ =	1.15			1.96			1.96			
	$\Phi V_n/V_{npr}$ =	2.23	1.12		1.84	1.29		1.84	1.12		
	$P/A_g f_c$ =	0.09			0.08			0.08			1.1%
			$(\Phi M_r/M_u)_{neg}$ = 1.15						$(\Phi M_r/M_u)_{neg}$ = 1.15		
			$(\Phi M_r/M_u)_{pos}$ = 1.12						$(\Phi M_r/M_u)_{pos}$ = 1.12		
			M_{top}/M_{neg} = 0.88						M_{top}/M_{neg} = 0.88		
			$\Phi V_n/V_{npr}$ = 1.16						$\Phi V_n/V_{npr}$ = 1.16		

Modeling Documentation (1 of 1)											
Floor 13	Story 12	Story 11	Story 10	Story 9	Story 8	Story 7	Story 6	Story 5	Story 4	Story 3	Story 2
Floor 12	Story 11	Story 10	Story 9	Story 8	Story 7	Story 6	Story 5	Story 4	Story 3	Story 2	Story 1
Floor 11	Story 10	Story 9	Story 8	Story 7	Story 6	Story 5	Story 4	Story 3	Story 2	Story 1	Story 0
Floor 10	Story 9	Story 8	Story 7	Story 6	Story 5	Story 4	Story 3	Story 2	Story 1	Story 0	Story -1
Floor 9	Story 8	Story 7	Story 6	Story 5	Story 4	Story 3	Story 2	Story 1	Story 0	Story -1	Story -2
Floor 8	Story 7	Story 6	Story 5	Story 4	Story 3	Story 2	Story 1	Story 0	Story -1	Story -2	Story -3
Floor 7	Story 6	Story 5	Story 4	Story 3	Story 2	Story 1	Story 0	Story -1	Story -2	Story -3	Story -4
Floor 6	Story 5	Story 4	Story 3	Story 2	Story 1	Story 0	Story -1	Story -2	Story -3	Story -4	Story -5
Floor 5	Story 4	Story 3	Story 2	Story 1	Story 0	Story -1	Story -2	Story -3	Story -4	Story -5	Story -6
Floor 4	Story 3	Story 2	Story 1	Story 0	Story -1	Story -2	Story -3	Story -4	Story -5	Story -6	Story -7
Floor 3	Story 2	Story 1	Story 0	Story -1	Story -2	Story -3	Story -4	Story -5	Story -6	Story -7	Story -8
Floor 2	Story 1	Story 0	Story -1	Story -2	Story -3	Story -4	Story -5	Story -6	Story -7	Story -8	Story -9
Floor 1	Story 0	Story -1	Story -2	Story -3	Story -4	Story -5	Story -6	Story -7	Story -8	Story -9	Story -10
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.79											
Model periods (sec): T ₁ = 1.90 T ₂ = 0.63 T ₃ = 0.35											
f _{1,triber,exped} = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2058

Number of Stories: 12

Fundamental Period (sec): 1.84

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2058 Perimeter Frame, SCWB=3.0

Initial column sizes were increased due to SCWB requirements as increasing column reinforcement was not sufficient. Both positive and negative bending strength were controlled by strength demands. Few additional beam reinforcements in 15 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. All column flexural strengths were controlled by the strong-column weak-beam ratio, except 8 upper columns by minimum requirement. Beam stirrups were controlled by the capacity shear design and minimum requirement (36 by demand). Column stirrups were controlled by the capacity shear design and minimum requirement (4 by demand). As in the baseline design, joint shear controls the beam and column sizes.

DESIGN AND MODELING DOCUMENTATION

FIGURES

Page 203 of 240

Design Documentation (2 of 2)											
											Design Drifts:
	SCWB =	3.26	1.29	2.10	1.17						
	Joint $\phi V_r/V_u =$	5.57				2.00	1.19	1.51	0.86	1.17	
						3.75					
Story 12	$\phi M_r/M_u =$	4.98				3.82				3.83	
	$\phi V_r/V_{mpf} =$	2.09				2.04				2.04	
	$P/A_g f_c =$	0.01				0.01				0.01	1.0%
Floor 12		5.01	1.23	1.37	1.15	3.24	1.17	1.12	0.72	3.23	
		3.23				2.35				2.35	
Story 11	$\phi M_r/M_u =$	3.84				3.26				3.84	
	$\phi V_r/V_{mpf} =$	2				2.38				2	
	$P/A_g f_c =$	0.01				0.02				0.02	1.4%
Floor 11		4.21	1.17	1.27	1.17	4.14	1.15	1.11	0.78	4.14	
		2.61				1.84				1.84	
Story 10	$\phi M_r/M_u =$	3.99				6.11				6.10	
	$\phi V_r/V_{mpf} =$	1.92				0.93				0.93	
	$P/A_g f_c =$	0.02				0.03				0.03	1.7%
Floor 10		4.62	1.20	1.24	1.16	5.00	1.18	1.11	0.77	5.00	
		2.20				1.58				1.58	
Story 9	$\phi M_r/M_u =$	3.56				3.43				3.44	
	$\phi V_r/V_{mpf} =$	1.54				1.28				1.28	
	$P/A_g f_c =$	0.03				0.03				0.03	1.7%
Floor 9		3.54	1.15	1.20	1.16	3.14	1.20	1.12	0.84	3.13	
		1.81				1.24				1.24	
Story 8	$\phi M_r/M_u =$	4.03				3.57				3.57	
	$\phi V_r/V_{mpf} =$	1.74				1.23				1.23	
	$P/A_g f_c =$	0.04				0.04				0.04	1.6%
Floor 8		3.39	1.17	1.21	1.15	3.07	1.21	1.13	0.84	3.07	
		1.75				1.19				1.19	
Story 7	$\phi M_r/M_u =$	3.70				3.41				3.41	
	$\phi V_r/V_{mpf} =$	1.67				1.51				1.51	
	$P/A_g f_c =$	0.05				0.05				0.05	1.7%
Floor 7		3.16	1.17	1.16	1.15	3.14	1.22	1.12	0.82	3.14	
		1.94				1.34				1.34	
Story 6	$\phi M_r/M_u =$	3.37				3.63				3.63	
	$\phi V_r/V_{mpf} =$	2.25				1.48				1.48	
	$P/A_g f_c =$	0.04				0.04				0.04	1.7%
Floor 6		3.14	1.18	1.16	1.15	3.17	1.24	1.15	0.83	3.17	
		1.83				1.25				1.25	
Story 5	$\phi M_r/M_u =$	3.52				3.48				3.48	
	$\phi V_r/V_{mpf} =$	1.97				1.45				1.45	
	$P/A_g f_c =$	0.05				0.05				0.05	1.8%
Floor 5		3.17	1.18	1.13	1.15	3.10	1.26	1.15	0.83	3.10	
		1.77				1.21				1.21	
Story 4	$\phi M_r/M_u =$	3.27				3.38				3.38	
	$\phi V_r/V_{mpf} =$	2.34				1.74				1.74	
	$P/A_g f_c =$	0.06				0.05				0.05	1.8%
Floor 4		3.10	1.16	1.13	1.16	3.09	1.25	1.19	0.86	3.09	
		1.77				1.19				1.19	
Story 3	$\phi M_r/M_u =$	3.00				3.32				3.32	
	$\phi V_r/V_{mpf} =$	2.28				1.72				1.72	
	$P/A_g f_c =$	0.07				0.06				0.06	1.8%
Floor 3		3.03	1.18	1.13	1.16	3.12	1.29	1.22	0.86	3.12	
		1.77				1.19				1.19	
Story 2	$\phi M_r/M_u =$	2.61				3.14				3.14	
	$\phi V_r/V_{mpf} =$	2.22				1.69				1.69	
	$P/A_g f_c =$	0.07				0.07				0.07	1.7%
Floor 2		3.27	1.18	1.12	1.16	3.53	1.33	1.29	0.85	3.53	
		1.98				1.34				1.34	
Story 1	$\phi M_r/M_u =$	1.35				2.41				2.41	
	$\phi V_r/V_{mpf} =$	2.17				1.67				1.67	
	$P/A_g f_c =$	0.08				0.07				0.07	1.1%

Modeling Documentation (1 of 1)													
Floor 13	Story 12	$M_{y,exp} (k-in) = 15388$				$M_{y,exp} (k-in) = 15734$				$M_{y,exp} (k-in) = 15734$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 12	Story 11	$M_{y,exp} (k-in) = 15734$				$M_{y,exp} (k-in) = 16422$				$M_{y,exp} (k-in) = 16422$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 11	Story 10	$M_{y,exp} (k-in) = 16079$				$M_{y,exp} (k-in) = 34560$				$M_{y,exp} (k-in) = 34560$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 10	Story 9	$M_{y,exp} (k-in) = 23720$				$M_{y,exp} (k-in) = 35214$				$M_{y,exp} (k-in) = 35214$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 9	Story 8	$M_{y,exp} (k-in) = 24055$				$M_{y,exp} (k-in) = 35865$				$M_{y,exp} (k-in) = 35865$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 8	Story 7	$M_{y,exp} (k-in) = 24389$				$M_{y,exp} (k-in) = 36513$				$M_{y,exp} (k-in) = 36513$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 7	Story 6	$M_{y,exp} (k-in) = 24824$				$M_{y,exp} (k-in) = 41645$				$M_{y,exp} (k-in) = 41645$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 6	Story 5	$M_{y,exp} (k-in) = 28069$				$M_{y,exp} (k-in) = 42291$				$M_{y,exp} (k-in) = 42291$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 5	Story 4	$M_{y,exp} (k-in) = 28402$				$M_{y,exp} (k-in) = 42935$				$M_{y,exp} (k-in) = 42935$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 4	Story 3	$M_{y,exp} (k-in) = 28734$				$M_{y,exp} (k-in) = 43576$				$M_{y,exp} (k-in) = 43576$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 3	Story 2	$M_{y,exp} (k-in) = 29065$				$M_{y,exp} (k-in) = 44214$				$M_{y,exp} (k-in) = 44214$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
Floor 2	Story 1	$M_{y,exp} (k-in) = 29395$				$M_{y,exp} (k-in) = 44851$				$M_{y,exp} (k-in) = 44851$			
		$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$				$E_{eff}/E_g = 0.35$			
<div>Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 1.79</div> <div>Model periods (sec): $T_1 = 1.84$ $T_2 = 0.61$ $T_3 = 0.34$</div> <div>$f_{rebar,expected} = 67$ ksi</div>													

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2060

Number of Stories: 12

Fundamental Period (sec): 2.00

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2060 Perimeter Frame, SCWB=0.9

Note: We chose the SCWB=0.9 value for this design because this is the point that flexural controls over SCWB for a significant number of the columns in the frame. In another words, this is virtually the lowest SCWB ratio we can design for without flexural controlling for all columns in the frame.

Column and beam sizes were kept the same as the baseline design. Simply reducing the reinforcement was enough to meet the SCWB ratio desired. Both positive and negative bending strength were controlled by strength demands, except 3 by strength minimum requirements. Few additional beam reinforcements in 14 bays were added to alter beam design to use same ρ and ρ_{Prime} in each floor. All column flexural strengths were controlled by the strong-column weak-beam ratio, except for top 4 story's columns by minimum requirement (again most columns were very close to being flexural controlled). Beam stirrups were controlled by the capacity shear design and minimum requirement (36 by demand). Column stirrups were controlled by the minimum requirement. As in the baseline design, joint shear controls the beam and column sizes.

DESIGN AND MODELING DOCUMENTATION

FIGURES

Design Documentation (1 of 2)												
Floor 13												
Story 12	h (in) =	32	28	0.0033	0.0025	5.5	32	28	0.0033	0.0025	5.5	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.010	0.010				0.010	0.010				0.010
	ρ_{sh} =	0.0105	0.0105	0.0033	0.0038	0.0026	0.0105	0.0105	0.0033	0.0038	0.0026	0.0105
	s (in) =	4.0	4.0	5.0	5.0	5.0	4.0	4.0	5.0	5.0	5.0	4.0
Floor 12												
Story 11	h (in) =	32	28	0.0038	0.0050	0.0030	32	28	0.0038	0.0050	0.0030	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.010	0.010				0.010	0.010				0.010
	ρ_{sh} =	0.0105	0.0105	0.0038	0.0050	0.0030	0.0105	0.0105	0.0038	0.0050	0.0030	0.0105
	s (in) =	4.0	4.0	7.0	7.0	7.0	4.0	4.0	7.0	7.0	7.0	4.0
Floor 11												
Story 10	h (in) =	32	28	0.0048	0.0036	5.5	32	28	0.0048	0.0036	5.5	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.010	0.010				0.010	0.010				0.010
	ρ_{sh} =	0.0105	0.0105	0.0048	0.0036	5.5	0.0105	0.0105	0.0048	0.0036	5.5	0.0105
	s (in) =	4.0	4.0	5.5	5.5	5.5	4.0	4.0	5.5	5.5	5.5	4.0
Floor 10												
Story 9	h (in) =	32	28	0.004	0.0033	6.0	32	28	0.004	0.0033	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.010	0.010				0.010	0.010				0.010
	ρ_{sh} =	0.0105	0.0105	0.004	0.0033	6.0	0.0105	0.0105	0.004	0.0033	6.0	0.0105
	s (in) =	4.0	4.0	6.0	6.0	6.0	4.0	4.0	6.0	6.0	6.0	4.0
Floor 9												
Story 8	h (in) =	32	28	0.0042	0.0035	6.0	32	28	0.0042	0.0035	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.010	0.010				0.010	0.010				0.010
	ρ_{sh} =	0.0105	0.0105	0.0042	0.0035	6.0	0.0105	0.0105	0.0042	0.0035	6.0	0.0105
	s (in) =	4.0	4.0	6.0	6.0	6.0	4.0	4.0	6.0	6.0	6.0	4.0
Floor 8												
Story 7	h (in) =	32	28	0.0055	0.0063	4.5	32	28	0.0055	0.0063	4.5	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.012	0.012				0.012	0.012				0.012
	ρ_{sh} =	0.0105	0.0105	0.0055	0.0063	4.5	0.0105	0.0105	0.0055	0.0063	4.5	0.0105
	s (in) =	4.0	4.0	4.5	4.5	4.5	4.0	4.0	4.5	4.5	4.5	4.0
Floor 7												
Story 6	h (in) =	32	28	0.0055	0.0063	4.5	32	28	0.0055	0.0063	4.5	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.012	0.012				0.012	0.012				0.012
	ρ_{sh} =	0.0105	0.0105	0.0055	0.0063	4.5	0.0105	0.0105	0.0055	0.0063	4.5	0.0105
	s (in) =	4.0	4.0	4.5	4.5	4.5	4.0	4.0	4.5	4.5	4.5	4.0
Floor 6												
Story 5	h (in) =	32	28	0.006	0.0066	6.0	32	28	0.006	0.0066	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.013	0.013				0.013	0.013				0.013
	ρ_{sh} =	0.0105	0.0105	0.006	0.0066	6.0	0.0105	0.0105	0.006	0.0066	6.0	0.0105
	s (in) =	4.0	4.0	6.0	6.0	6.0	4.0	4.0	6.0	6.0	6.0	4.0
Floor 5												
Story 4	h (in) =	32	28	0.0062	0.0068	6.0	32	28	0.0062	0.0068	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.015	0.015				0.015	0.015				0.015
	ρ_{sh} =	0.0105	0.0105	0.0062	0.0068	6.0	0.0105	0.0105	0.0062	0.0068	6.0	0.0105
	s (in) =	4.0	4.0	6.0	6.0	6.0	4.0	4.0	6.0	6.0	6.0	4.0
Floor 4												
Story 3	h (in) =	32	28	0.0062	0.0068	6.0	32	28	0.0062	0.0068	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.015	0.015				0.015	0.015				0.015
	ρ_{sh} =	0.0105	0.0105	0.0062	0.0068	6.0	0.0105	0.0105	0.0062	0.0068	6.0	0.0105
	s (in) =	4.0	4.0	6.0	6.0	6.0	4.0	4.0	6.0	6.0	6.0	4.0
Floor 3												
Story 2	h (in) =	32	28	0.0063	0.0066	6.0	32	28	0.0063	0.0066	6.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.015	0.015				0.015	0.015				0.015
	ρ_{sh} =	0.0105	0.0105	0.0063	0.0066	6.0	0.0105	0.0105	0.0063	0.0066	6.0	0.0105
	s (in) =	4.0	4.0	6.0	6.0	6.0	4.0	4.0	6.0	6.0	6.0	4.0
Floor 2												
Story 1	h (in) =	32	28	0.0024	0.0024	4.0	32	28	0.0024	0.0024	4.0	32
	b (in) =	28	28				28	28				28
	ρ_{ext} =	0.024	0.024				0.024	0.024				0.024
	ρ_{sh} =	0.0105	0.0105				0.0105	0.0105				0.0105
	s (in) =	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Grade beam column height (in) = 24												
Basement column height (in) = 32												
20 feet												
Design base shear = 0.044 g, 368 k												
f'_c beams = 5.0 ksi f'_c cols upper = 7.0 ksi f_y rebar nom = 60 ksi f'_c cols lower = 7.0 ksi												
<div>13 feet</div> <div>15 feet</div>												

Design Documentation (2 of 2)											
	SCWB =	1.22	2.10	4.52	1.16	0.69	1.67	2.49	1.16	0.69	2.11
	Joint $\Phi V_r/V_u$	5.17				3.23				3.23	5.17
Story 12	$\Phi M_r/M_u$	3.67				2.88				2.89	3.68
	$\Phi V_r/V_{mpr}$	2.72				2.62				2.62	2.72
	$P/A_g f_c$	0.01				0.01				0.01	0.01
Floor 12		2.19	1.28	2.03	1.15	1.32	1.19	1.34	1.15	1.32	2.19
		3.36				2.24				2.24	3.36
Story 11	$\Phi M_r/M_u$	2.87				1.82				1.82	2.87
	$\Phi V_r/V_{mpr}$	2.56				2.46				2.46	2.56
	$P/A_g f_c$	0.01				0.02				0.02	0.01
Floor 11		1.74	1.20	1.34	1.16	1.13	1.20	1.10	1.16	1.13	1.74
		2.52				1.80				1.80	2.52
Story 10	$\Phi M_r/M_u$	2.12				1.33				1.33	2.12
	$\Phi V_r/V_{mpr}$	2.4				2.3				2.3	2.4
	$P/A_g f_c$	0.02				0.02				0.02	0.02
Floor 10		1.43	1.19	1.22	1.16	1.12	1.23	1.11	1.16	1.12	1.43
		1.93				1.37				1.37	1.93
Story 9	$\Phi M_r/M_u$	1.64				1.50				1.50	1.65
	$\Phi V_r/V_{mpr}$	2.68				1.98				1.98	2.68
	$P/A_g f_c$	0.03				0.03				0.03	0.03
Floor 9		1.24	1.14	1.22	1.16	1.13	1.21	1.16	1.16	1.13	1.24
		2.14				1.46				1.46	2.14
Story 8	$\Phi M_r/M_u$	1.31				1.29				1.29	1.31
	$\Phi V_r/V_{mpr}$	3.05				2.33				2.33	3.05
	$P/A_g f_c$	0.04				0.04				0.04	0.04
Floor 8		1.24	1.12	1.18	1.16	1.19	1.19	1.16	1.16	1.19	1.24
		2.03				1.38				1.38	2.03
Story 7	$\Phi M_r/M_u$	1.15				1.28				1.28	1.16
	$\Phi V_r/V_{mpr}$	2.6				1.94				1.94	2.6
	$P/A_g f_c$	0.05				0.05				0.05	0.05
Floor 7		1.07	1.13	1.19	1.16	1.06	1.26	1.23	1.16	1.06	1.07
		1.63				1.08				1.08	1.63
Story 6	$\Phi M_r/M_u$	1.18				1.31				1.31	1.18
	$\Phi V_r/V_{mpr}$	3.19				2.48				2.48	3.19
	$P/A_g f_c$	0.06				0.06				0.06	0.06
Floor 6		1.08	1.11	1.16	1.16	1.08	1.25	1.22	1.16	1.08	1.08
		1.63				1.08				1.08	1.63
Story 5	$\Phi M_r/M_u$	1.15				1.30				1.30	1.15
	$\Phi V_r/V_{mpr}$	2.93				2.41				2.41	2.93
	$P/A_g f_c$	0.07				0.07				0.07	0.07
Floor 5		1.13	1.13	1.19	1.16	1.03	1.29	1.30	1.16	1.03	1.13
		1.54				1.01				1.01	1.54
Story 4	$\Phi M_r/M_u$	1.31				1.29				1.29	1.31
	$\Phi V_r/V_{mpr}$	2.5				2.35				2.35	2.5
	$P/A_g f_c$	0.08				0.07				0.07	0.08
Floor 4		1.14	1.15	1.19	1.16	1.02	1.34	1.36	1.16	1.02	1.14
		1.49				0.97				0.97	1.49
Story 3	$\Phi M_r/M_u$	1.17				1.30				1.30	1.17
	$\Phi V_r/V_{mpr}$	2.47				2.29				2.29	2.47
	$P/A_g f_c$	0.09				0.08				0.08	0.09
Floor 3		1.09	1.13	1.16	1.16	1.04	1.35	1.38	1.16	1.04	1.09
		1.49				0.97				0.97	1.49
Story 2	$\Phi M_r/M_u$	1.17				1.30				1.30	1.17
	$\Phi V_r/V_{mpr}$	2.32				2.23				2.23	2.32
	$P/A_g f_c$	0.10				0.09				0.09	0.10
Floor 2		1.51	1.11	1.16	1.16	1.09	1.42	1.56	1.16	1.09	1.51
		1.54				0.98				0.98	1.54
Story 1	$\Phi M_r/M_u$	1.16				1.16				1.16	1.16
	$\Phi V_r/V_{mpr}$	1.69				2.14				2.14	1.69
	$P/A_g f_c$	0.11				0.10				0.10	0.11
	$(\Phi M_r/M_u)_{neg}$	1.11									
	$(\Phi M_r/M_u)_{pos}$	1.16									
	M_{top}/M_{base}	0.96									
	$\Phi V_r/V_{mp}$	1.16									
	$(\Phi M_r/M_u)_{neg}$	1.11									
	$(\Phi M_r/M_u)_{pos}$	1.16									
	M_{top}/M_{base}	0.96									
	$\Phi V_r/V_{mp}$	1.16									
Design Drifts:											
											0.8%
											1.2%
											1.5%
											1.6%
											1.6%
											1.7%
											1.8%
											1.9%
											1.9%
											1.9%
											1.9%
											1.4%

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	7844	4981	-6358	0.35	2.379E+08	0.0389	-0.044	0.086	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 12	Story 11	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 11	Story 10	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 10	Story 9	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	8097	4981	-7079	0.35	2.379E+08	0.0389	-0.047	0.089	111
		E_{eff}/E_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Mass tributary to one frame for lateral load (each floor) (k-s/s/in): 1.79											
Model periods (sec): $T_1 = 2.00$ $T_2 = 0.67$ $T_3 = 0.38$											
$f_{y,triber,exp} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2061

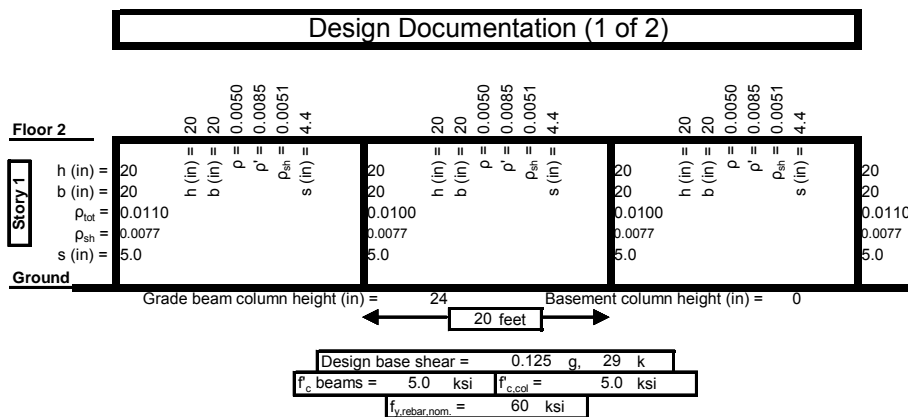
Number of Stories: 1

Fundamental Period (sec): 0.42

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Column size is designed to satisfy joint shear requirements. Beams strength were controlled by force demand, but one beams had additional reinforcement added to keep the same rho ratio between the floors. Exterior column strengths are controlled by flexural demands and interior by columns controlled by minimum the reinforcement requirement. The columns could not be made smaller than 20" because of the joint bond requirements of 20*db; 20" is the smallest dimension allowable for a #8 rebar. Beam stirrups were controlled by the minimum requirement. The column stirrups were controlled by confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES



Modeling Documentation (1 of 1)

Mass tributary to one frame for lateral load (each floor) (k-s/in):	0.60
Model periods (sec): $T_1 = 0.42$ $T_2 = 0.40$ $T_3 = 0.03$	
$f_{v, \text{rebar expected}} =$	67 ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2062

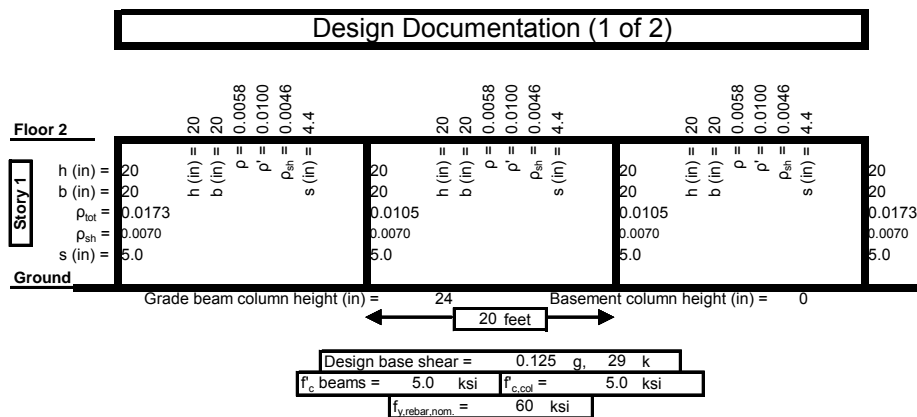
Number of Stories: 1

Fundamental Period (sec): 0.42

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Column size is designed to satisfy joint shear requirements. Beams strength were controlled by force demand, but one beams had additional reinforcement added to keep the same rho ratio between the floors. Exterior column strengths are controlled by flexural demands and interior by columns controlled by minimum the reinforcement requirement. The columns could not be made smaller than 20" because of the joint bond requirements of $20 \times db$; 20" is the smallest dimension allowable for a #8 rebar. Beam stirrups were controlled by the minimum requirement. The column stirrups were controlled by confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES



Modeling Documentation (1 of 1)

Mass tributary to one frame for lateral load (each floor) (k-s/in):	0.60
Model periods (sec): $T_1 = 0.42$ $T_2 = 0.40$ $T_3 = 0.03$	
$f_{v, \text{rebar expected}} =$	67 ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2063

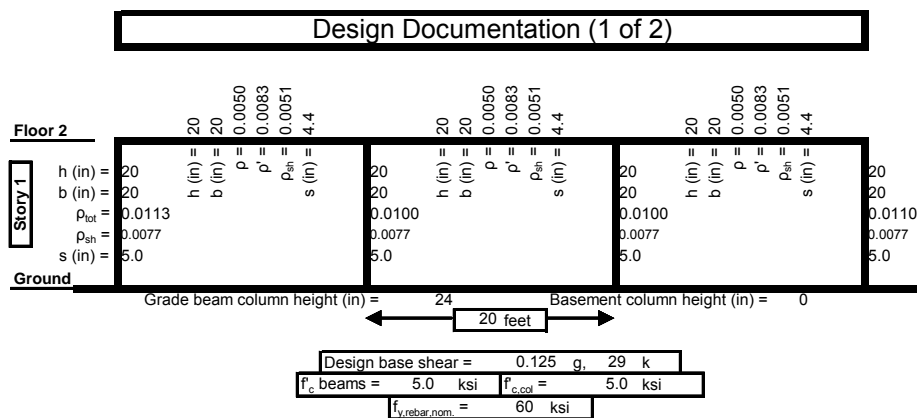
Number of Stories: 1

Fundamental Period (sec): 0.43

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

This design is not too much different from the 2061 design. Column size is designed to satisfy joint shear requirements. Beams strength were controlled by force demand, but one beams had additional reinforcement added to keep the same rho ratio between the floors. Exterior column strengths are controlled by flexural demands and interior by columns controlled by minimum the reinforcement requirement. The columns could not be made smaller than 20" because of the joint bond requirements of $20 \times d_b$; 20" is the smallest dimension allowable for a #8 rebar. Beam stirrups were controlled by the minimum requirement. The column stirrups were controlled by confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES



STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2064

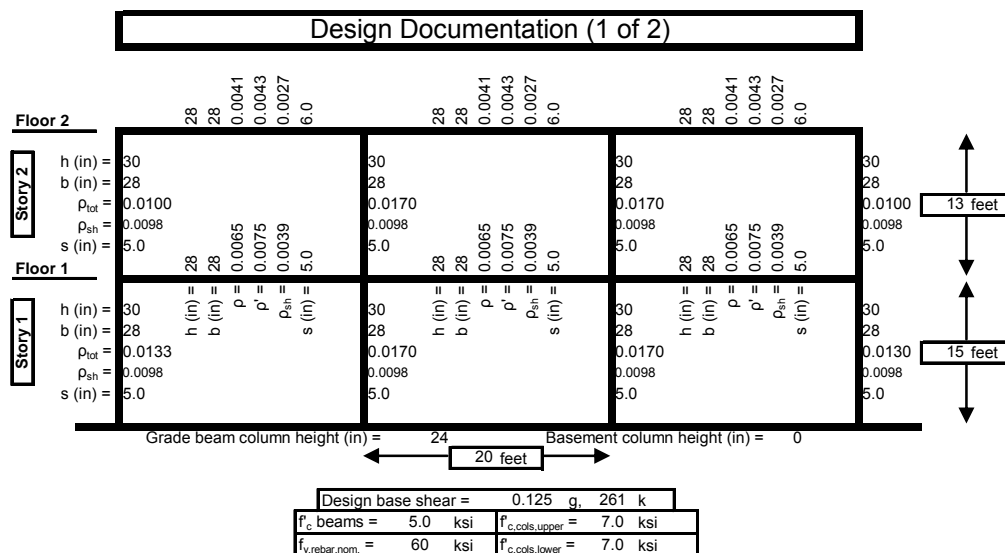
Number of Stories: 2

Fundamental Period (sec): 0.66

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Column size concrete strength are designed to satisfy joint shear requirements, specifically the column widths were increased to increase column area. Beams strength were controlled by force demand, but four beams had additional reinforcement added to keep the same rho ratio between the floors. Column strengths are controlled by SCWB except two exterior lower columns that were controlled by flexural demands and two upper story exterior columns controlled by minimum the reinforcement requirement. Beam stirrups were controlled by the minimum requirement. The column stirrups were controlled by confinement requirement.

DESIGN AND MODELING DOCUMENTATION FIGURES



Design Documentation (2 of 2)									
Story 2	SCWB =	1.09	1.12	1.07	1.24	1.07	1.14	1.09	Design Drifts:
	Joint $\Phi V_r/V_u$ =	3.79	2.43	2.43	3.79	3.79	3.79	3.79	
	$\phi M_r/M_u$ =	1.62	2.16	2.18	1.65	1.62	2.16	1.65	
	$\Phi V_r/V_{mpr}$ =	2.75	1.66	1.66	2.75	2.75	2.75	2.75	
Floor 1	$P/A_g f'_c$ =	0.01	0.01	0.01	0.01	0.01	0.01	0.01	1.0%
	$\phi M_r/M_u$ =	1.59	1.13	1.38	1.32	1.38	1.15	1.57	
	$\phi M_r/M_u$ =	1.73	1.14	1.16	1.43	1.16	1.16	1.73	
	$M_{n,post}/M_{n,neg}$ =	0.87	0.87	0.87	0.87	0.87	0.87	0.87	
Story 1	$\phi M_r/M_u$ =	1.16	1.53	1.54	1.16	1.16	1.53	1.16	1.5%
	$\Phi V_r/V_{mpr}$ =	2.01	1.57	1.57	2.04	2.01	1.57	2.04	
	$P/A_g f'_c$ =	0.02	0.02	0.02	0.02	0.02	0.02	0.02	
	$\phi M_r/M_u$ =	1.16	1.53	1.54	1.16	1.16	1.53	1.16	
Story 1	$\Phi V_r/V_{mpr}$ =	2.01	1.57	1.57	2.04	2.01	1.57	2.04	1.5%
	$P/A_g f'_c$ =	0.02	0.02	0.02	0.02	0.02	0.02	0.02	
	$\phi M_r/M_u$ =	1.16	1.53	1.54	1.16	1.16	1.53	1.16	
	$\Phi V_r/V_{mpr}$ =	2.01	1.57	1.57	2.04	2.01	1.57	2.04	
Modeling Documentation (1 of 1)									
Story 2	$M_{y,exp}$ (k-in) =	3827	11500	11500	3827	3827	11500	11500	6827
	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
	M_u/M_y =	1.19	1.19	1.19	1.19	1.19	1.19	1.19	
	$\Theta_{cap,pl}$ (rad) =	0.072	0.076	0.076	0.072	0.072	0.076	0.076	
Story 1	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100
	λ =	112	111	111	112	112	111	111	
	$(P/A_g f'_c)_{exp}$ =	0.00	0.01	0.01	0.00	0.00	0.01	0.01	
	$M_{y,exp}$ (k-in) =	3130	11954	11954	3130	3130	11954	11954	
Story 1	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
	M_u/M_y =	1.19	1.19	1.19	1.19	1.19	1.19	1.19	
	$\Theta_{cap,pl}$ (rad) =	0.074	0.075	0.075	0.074	0.074	0.075	0.075	
	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	
Story 1	λ =	112	111	111	112	112	111	111	112
	$(P/A_g f'_c)_{exp}$ =	0.00	0.01	0.01	0.00	0.00	0.01	0.01	
	$M_{y,exp}$ (k-in) =	3130	11954	11954	3130	3130	11954	11954	
	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
Story 1	M_u/M_y =	1.19	1.19	1.19	1.19	1.19	1.19	1.19	0.35
	$\Theta_{cap,pl}$ (rad) =	0.074	0.075	0.075	0.074	0.074	0.075	0.075	
	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	
	λ =	112	111	111	112	112	111	111	
Story 1	$(P/A_g f'_c)_{exp}$ =	0.00	0.01	0.01	0.00	0.00	0.01	0.01	0.00
	$M_{y,exp}$ (k-in) =	3130	11954	11954	3130	3130	11954	11954	
	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	
	M_u/M_y =	1.19	1.19	1.19	1.19	1.19	1.19	1.19	
Story 1	$\Theta_{cap,pl}$ (rad) =	0.074	0.075	0.075	0.074	0.074	0.075	0.075	0.073
	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100	0.100	0.100	0.100	
	λ =	112	111	111	112	112	111	111	
	$(P/A_g f'_c)_{exp}$ =	0.00	0.01	0.01	0.00	0.00	0.01	0.01	
Mass tributary to one frame for lateral load (each floor) (k-s-s/in):									2.69
Model periods (sec):									$T_1 = 0.66$ $T_2 = 0.18$ $T_3 = 0.05$
f_y rebar, expected =									67 ksi

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2065

Number of Stories: 8

Fundamental Period (sec): 1.57

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2065 Perimeter Frame, 65% Weak Story in first story only

This design was created using design ID 1023 as the baseline. Design ID 1023 had the first two stories weak, and this design is intended to have only the first story weak. Therefore, to make this design, we strengthened the second story columns and the first above ground beam. We based the 65% ratio of story strengths on the strengths computed only from the columns of the first and second stories, assuming both column ends hinged. Even with the weak story, this design is fully code conforming. The bottom story exterior columns were controlled by flexural.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)

--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--

Design Documentation (2 of 2)																			
		SCWB =	0.93	1.77	1.73	0.75	5.75	0.78	1.67	2.01	0.75	5.75	0.78	1.77	1.73	0.75	5.75	0.93	Design Drifts:
Joint $\Phi V_r/V_u =$			2.50					1.91					1.91					2.50	
Story 8	$\phi M_r/M_u =$		2.68					6.77					6.78					2.68	
	$\phi V_r/V_{mpr} =$		1.87					1.52					1.52					1.87	0.6%
	$P/A_g f'_c =$		0.02					0.03					0.03					0.02	
Floor 8			1.49	1.73	1.86	0.51	3.45	1.36	1.87	1.95	0.51	3.45	1.36	1.73	1.86	0.51	3.45	1.49	
Story 7	$\phi M_r/M_u =$		1.45					1.29					1.29					1.45	
	$\phi V_r/V_{mpr} =$																		0.9%
	$P/A_g f'_c =$																		
Floor 7			1.41	1.73	2.00	0.51	2.70	1.31	1.85	2.14	0.51	2.70	1.31	1.73	2.00	0.51	2.70	1.41	
Story 6	$\phi M_r/M_u =$		1.26					1.12					1.12					1.26	
	$\phi V_r/V_{mpr} =$																		1.3%
	$P/A_g f'_c =$																		
Floor 6			1.34	1.74	2.06	0.51	0.72	1.26	1.86	2.21	0.51	0.72	1.26	1.74	2.06	0.51	0.72	1.34	
Story 5	$\phi M_r/M_u =$		1.09					0.97					0.97					1.09	
	$\phi V_r/V_{mpr} =$																		1.5%
	$P/A_g f'_c =$																		
Floor 5			1.31	1.74	2.04	0.51	0.69	1.26	1.85	2.25	0.51	0.69	1.26	1.74	2.04	0.51	0.69	1.31	
Story 4	$\phi M_r/M_u =$		1.15					1.02					1.02					1.15	
	$\phi V_r/V_{mpr} =$																		1.7%
	$P/A_g f'_c =$																		
Floor 4			1.31	1.72	1.99	0.51	0.68	1.30	1.82	2.25	0.51	0.68	1.30	1.72	1.99	0.51	0.68	1.31	
Story 3	$\phi M_r/M_u =$		1.11					0.99					0.99					1.11	
	$\phi V_r/V_{mpr} =$																		1.8%
	$P/A_g f'_c =$																		
Floor 3			1.32	1.74	1.98	0.51	0.67	1.34	1.81	2.25	0.51	0.67	1.34	1.74	1.98	0.51	0.67	1.32	
Story 2	$\phi M_r/M_u =$		1.09					0.97					0.97					1.09	
	$\phi V_r/V_{mpr} =$																		1.8%
	$P/A_g f'_c =$																		
Floor 2			1.37	1.51	1.56	0.51	2.52	1.32	1.59	1.97	0.51	2.52	1.32	1.51	1.56	0.51	2.52	1.37	
Story 1	$\phi M_r/M_u =$		1.34					1.19					1.19					1.34	
	$\phi V_r/V_{mpr} =$																		1.4%
	$P/A_g f'_c =$																		

Modeling Documentation (1 of 1)									
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	6098	7426	7426	6098			
		EI_{stf}/EI_g =	0.35	0.35	0.35	0.35			
Floor 8	Story 7	M_c/M_y =	1.20	1.20	1.20	1.20			
		$\Theta_{cap,pl}$ (rad) =	0.081	0.081	0.081	0.081			
Floor 7	Story 6	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
		λ =	116	114	114	116			
Floor 6	Story 5	$(P/A_g f_c)_{exp}$ =	0.01	0.02	0.02	0.01			
		$M_{y,exp}$ (k-in) =	6455	8122	8122	6455			
Floor 5	Story 4	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35			
		M_c/M_y =	1.20	1.20	1.20	1.20			
Floor 4	Story 3	$\Theta_{cap,pl}$ (rad) =	0.079	0.078	0.078	0.079			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor 3	Story 2	λ =	114	111	111	114			
		$(P/A_g f_c)_{exp}$ =	0.02	0.04	0.04	0.02			
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	6808	8806	8806	6808			
		EI_{stf}/EI_g =	0.35	0.35	0.35	0.35			
Floor 1	Story 0	M_c/M_y =	1.20	1.19	1.19	1.20			
		$\Theta_{cap,pl}$ (rad) =	0.078	0.075	0.075	0.078			
Floor 0	Story -1	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
		λ =	112	107	107	112			
Floor -1	Story -2	$(P/A_g f_c)_{exp}$ =	0.03	0.07	0.07	0.03			
		$M_{y,exp}$ (k-in) =	7157	9478	9478	7157			
Floor -2	Story -3	EI_{stf}/EI_g =	0.35	0.35	0.35	0.35			
		M_c/M_y =	1.20	1.19	1.19	1.20			
Floor -3	Story -4	$\Theta_{cap,pl}$ (rad) =	0.076	0.072	0.072	0.076			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor -4	Story -5	λ =	111	104	104	111			
		$(P/A_g f_c)_{exp}$ =	0.04	0.09	0.09	0.04			
Floor -5	Story -6	$M_{y,exp}$ (k-in) =	7550	10212	10212	7550			
		EI_{stf}/EI_g =	0.35	0.35	0.35	0.35			
Floor -6	Story -7	M_c/M_y =	1.18	1.18	1.18	1.18			
		$\Theta_{cap,pl}$ (rad) =	0.070	0.067	0.067	0.070			
Floor -7	Story -8	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
		λ =	111	105	105	111			
Floor -8	Story -9	$(P/A_g f_c)_{exp}$ =	0.04	0.08	0.08	0.04			
		$M_{y,exp}$ (k-in) =	7897	10873	10873	7897			
Floor -9	Story -10	EI_{stf}/EI_g =	0.35	0.37	0.37	0.35			
		M_c/M_y =	1.18	1.17	1.17	1.18			
Floor -10	Story -11	$\Theta_{cap,pl}$ (rad) =	0.069	0.065	0.065	0.069			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor -11	Story -12	λ =	110	103	103	110			
		$(P/A_g f_c)_{exp}$ =	0.05	0.10	0.10	0.05			
Floor -12	Story -13	$M_{y,exp}$ (k-in) =	8241	11525	11525	8241			
		EI_{stf}/EI_g =	0.35	0.39	0.39	0.35			
Floor -13	Story -14	M_c/M_y =	1.18	1.17	1.17	1.18			
		$\Theta_{cap,pl}$ (rad) =	0.068	0.063	0.063	0.068			
Floor -14	Story -15	Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
		λ =	109	101	101	109			
Floor -15	Story -16	$(P/A_g f_c)_{exp}$ =	0.06	0.12	0.12	0.06			
		$M_{y,exp}$ (k-in) =	8901	13050	13050	8901			
Floor -16	Story -17	EI_{stf}/EI_g =	0.35	0.40	0.40	0.35			
		M_c/M_y =	1.18	1.17	1.17	1.18			
Floor -17	Story -18	$\Theta_{cap,pl}$ (rad) =	0.064	0.055	0.055	0.064			
		Θ_{pc} (rad) =	0.100	0.100	0.100	0.100			
Floor -18	Story -19	λ =	107	98	98	107			
		$(P/A_g f_c)_{exp}$ =	0.07	0.13	0.13	0.07			
Mass tributary to one frame for lateral load (each floor) (k-s-s/in):							0.60		
Model periods (sec):							$T_1 = 1.57$	$T_2 = 0.51$	$T_3 = 0.29$
f_y rebar expected =							67	ksi	

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2066

Number of Stories: 8

Fundamental Period (sec): 1.71

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2066 Perimeter Frame, 80% Weak Story in first story only

This design was created using design ID 1024 as the baseline. Design ID 1024 had the first two stories weak, and this design is intended to have only the first story weak. Therefore, to make this design, we strengthened the second story columns and the first above ground beam. We based the 80% ratio of story strengths on the strengths computed only from the columns of the first and second stories, assuming both column ends hinged. Even with the weak story, this design is fully code conforming. The bottom story exterior columns were controlled by flexural.

DESIGN AND MODELING DOCUMENTATION FIGURES

Page 225 of 240

Design Documentation (2 of 2)																		
SCWB =		0.86	1.37	1.07	0.64	7.26	0.80	1.24	1.27	0.64	7.26	0.80	1.37	1.07	0.64	7.26	0.86	Design Drifts:
Joint $\Phi V_r/V_u =$		2.81					2.29					2.29					2.81	
Story 8	$\Phi M_r/M_u =$	2.25					5.95					5.96					2.25	0.6%
	$\Phi V_r/V_{mpr} =$	1.94					1.54					1.54					1.94	
	$P/A_g f'_c =$	0.02					0.04					0.04					0.02	
Floor 8		1.35	1.41	1.51	0.51	3.34	1.28	1.52	1.59	0.51	3.34	1.28	1.41	1.51	0.51	3.34	1.35	
		1.47					1.31					1.31					1.47	
Story 7	$\Phi M_r/M_u =$	2.63					4.48					4.48					2.64	0.9%
	$\Phi V_r/V_{mpr} =$	1.80					1.92					1.92					1.80	
	$P/A_g f'_c =$	0.04					0.07					0.07					0.04	
Floor 7		1.30	1.39	1.57	0.51	2.73	1.25	1.48	1.72	0.51	2.73	1.25	1.39	1.57	0.51	2.73	1.30	
		1.29					1.15					1.15					1.29	
Story 6	$\Phi M_r/M_u =$	2.29					3.46					3.46					2.29	1.2%
	$\Phi V_r/V_{mpr} =$	2.26					1.77					1.77					2.26	
	$P/A_g f'_c =$	0.06					0.11					0.11					0.06	
Floor 6		1.23	1.42	1.68	0.51	2.24	1.21	1.52	1.87	0.51	2.24	1.21	1.42	1.68	0.51	2.24	1.23	
		1.10					0.98					0.98					1.10	
Story 5	$\Phi M_r/M_u =$	2.12					3.02					3.02					2.12	1.5%
	$\Phi V_r/V_{mpr} =$	2.51					1.94					1.94					2.51	
	$P/A_g f'_c =$	0.08					0.15					0.15					0.08	
Floor 5		1.22	1.40	1.63	0.51	2.06	1.23	1.49	1.85	0.51	2.06	1.23	1.40	1.63	0.51	2.06	1.22	
		1.18					1.05					1.05					1.18	
Story 4	$\Phi M_r/M_u =$	2.04					2.84					2.84					2.04	1.7%
	$\Phi V_r/V_{mpr} =$	3.45					2.62					2.62					3.45	
	$P/A_g f'_c =$	0.08					0.14					0.14					0.08	
Floor 4		1.22	1.39	1.58	0.51	1.95	1.27	1.48	1.85	0.51	1.95	1.27	1.39	1.58	0.51	1.95	1.22	
		1.14					1.01					1.01					1.14	
Story 3	$\Phi M_r/M_u =$	1.98					2.85					2.85					1.99	1.8%
	$\Phi V_r/V_{mpr} =$	3.27					2.45					2.45					3.27	
	$P/A_g f'_c =$	0.09					0.17					0.17					0.09	
Floor 3		1.23	1.40	1.55	0.51	1.90	1.33	1.46	1.84	0.51	1.90	1.33	1.40	1.55	0.51	1.90	1.23	
		1.12					0.99					0.99					1.12	
Story 2	$\Phi M_r/M_u =$	1.93					2.64					2.64					1.93	1.8%
	$\Phi V_r/V_{mpr} =$	3.12					2.33					2.33					3.12	
	$P/A_g f'_c =$	0.11					0.20					0.20					0.11	
Floor 2		1.22	1.37	1.45	0.53	2.02	1.31	1.46	1.94	0.53	2.02	1.31	1.37	1.45	0.53	2.02	1.22	
		1.18					1.04					1.04					1.18	
Story 1	$\Phi M_r/M_u =$	1.16	$(\Phi M_r/M_u)_{neg} =$	$(\Phi M_r/M_u)_{pos} =$	$M_{r,pos}/M_{r,neg} =$	$\Phi V_r/V_{mpr} =$	2.14	$(\Phi M_r/M_u)_{neg} =$	$(\Phi M_r/M_u)_{pos} =$	$M_{r,pos}/M_{r,neg} =$	$\Phi V_r/V_{mpr} =$	2.14	$(\Phi M_r/M_u)_{neg} =$	$(\Phi M_r/M_u)_{pos} =$	$M_{r,pos}/M_{r,neg} =$	$\Phi V_r/V_{mpr} =$	1.16	1.4%
	$\Phi V_r/V_{mpr} =$	3.31					2.59					2.59					3.31	
	$P/A_g f'_c =$	0.13					0.23					0.23					0.13	

Modeling Documentation (1 of 1)											
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	4889	1828	-5532	0.35	7.863E+07	0.0406	-0.068	0.100	100
		$E I_{stf}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	5213	2329	-7205	0.35	7.863E+07	0.042	-0.071	0.100	100
		$E I_{stf}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	5533	2641	-7799	0.35	7.863E+07	0.043	-0.071	0.100	100
		$E I_{stf}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	5849	3014	-8510	0.35	7.863E+07	0.044	-0.072	0.100	100
		$E I_{stf}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	6204	3324	-9101	0.35	7.863E+07	0.045	-0.072	0.100	100
		$E I_{stf}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	6518	3385	-9219	0.35	7.863E+07	0.045	-0.073	0.100	100
		$E I_{stf}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	6828	3324	-8866	0.35	7.863E+07	0.045	-0.072	0.100	100
		$E I_{stf}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	6360	3324	-8866	0.35	7.863E+07	0.045	-0.072	0.100	100
		$E I_{stf}/E I_g$ =	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35
<div>Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60</div> <div>Model periods (sec): $T_1 = 1.71$ $T_2 = 0.56$ $T_3 = 0.32$</div> <div>$f_{y, rebar, expected} = 67$ ksi</div>											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2067

Number of Stories: 12

Fundamental Period (sec): 1.92

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2067 Perimeter Frame, 65% Weak Story in first story only

This design was created using design ID 1017 as the baseline. Design ID 1017 had the first two stories weak, and this design is intended to have only the first story weak. Therefore, to make this design, we strengthened the second story columns and the first above ground beam. We based the 65% ratio of story strengths on the strengths computed only from the columns of the first and second stories, assuming both column ends hinged. Even with the weak story, this design is fully code conforming. The bottom story exterior columns were controlled by flexural.

Design 2067 ends up being about 30% stronger than 1017 because the second story was strengthened in the effort to make only the first story weak. During the pushover, the second story contributed to the yield mechanism, so the additional strength in that story increased the yield base shear strength.

DESIGN AND MODELING DOCUMENTATION FIGURES

Design Documentation (1 of 2)											
Floor 13											
Story 12	h (in) =	24	28	24	0.0033	28	24	0.0033	28	24	0.0033
	b (in) =	24	24	24	0.0045	24	24	0.0045	24	24	0.0045
	ρ_{tot} =	0.0205			0.0026	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0077			0.0086	0.0086			0.0086		0.0077
	s (in) =	4.0			5.5	3.5			3.5		4.0
Floor 12											
Story 11	h (in) =	24	28	24	0.0038	28	24	0.0038	28	24	0.0038
	b (in) =	24	24	24	0.0075	24	24	0.0075	24	24	0.0075
	ρ_{tot} =	0.0205			0.0044	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0077			0.0086	0.0086			0.0086		0.0077
	s (in) =	4.0			5.5	3.5			3.5		4.0
Floor 11											
Story 10	h (in) =	24	28	24	0.0043	28	24	0.0043	28	24	0.0043
	b (in) =	24	24	24	0.0085	24	24	0.0085	24	24	0.0085
	ρ_{tot} =	0.0205			0.0047	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0077			0.0086	0.0086			0.0086		0.0077
	s (in) =	4.0			5.0	4.0			4.0		4.0
Floor 10											
Story 9	h (in) =	24	28	24	0.0048	28	24	0.0048	28	24	0.0048
	b (in) =	24	24	24	0.0095	24	24	0.0095	24	24	0.0095
	ρ_{tot} =	0.0205			0.0049	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0077			0.0086	0.0086			0.0086		0.0077
	s (in) =	4.0			6.0	4.0			4.0		4.0
Floor 9											
Story 8	h (in) =	24	28	24	0.0050	28	24	0.0050	28	24	0.0050
	b (in) =	24	24	24	0.0100	24	24	0.0100	24	24	0.0100
	ρ_{tot} =	0.0205			0.0051	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0077			0.0086	0.0086			0.0086		0.0077
	s (in) =	4.0			6.0	4.0			4.0		4.0
Floor 8											
Story 7	h (in) =	24	28	24	0.0053	28	24	0.0053	28	24	0.0053
	b (in) =	24	24	24	0.0105	24	24	0.0105	24	24	0.0105
	ρ_{tot} =	0.0205			0.0052	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0077			0.0086	0.0086			0.0086		0.0077
	s (in) =	4.0			6.0	4.0			4.0		4.0
Floor 7											
Story 6	h (in) =	24	28	24	0.0054	28	24	0.0054	28	24	0.0054
	b (in) =	24	24	24	0.0108	24	24	0.0108	24	24	0.0108
	ρ_{tot} =	0.0205			0.0053	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0077			0.0086	0.0086			0.0086		0.0077
	s (in) =	4.0			6.0	4.0			4.0		4.0
Floor 6											
Story 5	h (in) =	24	28	24	0.0055	28	24	0.0055	28	24	0.0055
	b (in) =	24	24	24	0.0110	24	24	0.0110	24	24	0.0110
	ρ_{tot} =	0.0205			0.0054	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0089			0.0089	0.0089			0.0089		0.0089
	s (in) =	3.5			6.0	3.5			3.5		3.5
Floor 5											
Story 4	h (in) =	24	28	24	0.0056	28	24	0.0056	28	24	0.0056
	b (in) =	24	24	24	0.0113	24	24	0.0113	24	24	0.0113
	ρ_{tot} =	0.0205			0.0056	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0089			0.0089	0.0089			0.0089		0.0089
	s (in) =	3.5			5.5	3.5			3.5		3.5
Floor 4											
Story 3	h (in) =	24	28	24	0.0055	28	24	0.0055	28	24	0.0055
	b (in) =	24	24	24	0.0113	24	24	0.0113	24	24	0.0113
	ρ_{tot} =	0.0205			0.0045	0.0270			0.0270		0.0205
	ρ_{sh} =	0.0089			0.0089	0.0089			0.0089		0.0089
	s (in) =	3.5			5.0	3.5			3.5		3.5
Floor 3											
Story 2	h (in) =	24	28	24	0.0050	28	24	0.0050	28	24	0.0050
	b (in) =	24	24	24	0.0095	24	24	0.0095	24	24	0.0095
	ρ_{tot} =	0.0210			0.0045	0.0270			0.0270		0.0210
	ρ_{sh} =	0.0089			0.0089	0.0089			0.0089		0.0089
	s (in) =	3.5			5.0	3.5			3.5		3.5
Floor 2											
Story 1	h (in) =	24	28	24	0.0120	28	24	0.0120	28	24	0.0120
	b (in) =	24	24	24	0.0089	24	24	0.0089	24	24	0.0089
	ρ_{tot} =	0.0120			0.0089	0.0100			0.0100		0.0120
	ρ_{sh} =	0.0089			0.0089	0.0089			0.0089		0.0089
	s (in) =	3.5			3.5	3.5			3.5		3.5
Basement											
Grade beam column height (in) = 24											
Basement column height (in) = 24											
Design base shear = 0.044 g, 123 k											
f'_c beams = 5.0 ksi $f'_{c,cds,upper}$ = 6.0 ksi f_y rebar, nom = 60 ksi $f'_{c,cds,lower}$ = 7.0 ksi											

Page 231 of 240

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	7234	3175	3175	1.522E+08	0.0336	0.089	99	99	7234
		E_{eff}/E_g =	0.35	-8021	0.35	1.522E+08	0.0336	0.089	99	99	7234
Floor 12	Story 11	$M_{y,exp}$ (k-in) =	7587	3648	3648	1.522E+08	0.0336	0.089	99	99	7587
		E_{eff}/E_g =	0.35	-10732	0.35	1.522E+08	0.0336	0.089	99	99	7587
Floor 11	Story 10	$M_{y,exp}$ (k-in) =	7936	4121	4121	1.522E+08	0.0336	0.089	99	99	7936
		E_{eff}/E_g =	0.35	-11633	0.35	1.522E+08	0.0336	0.089	99	99	7936
Floor 10	Story 9	$M_{y,exp}$ (k-in) =	8282	4594	4594	1.522E+08	0.0336	0.089	99	99	8282
		E_{eff}/E_g =	0.35	-12532	0.35	1.522E+08	0.0336	0.089	99	99	8282
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	8625	4830	4830	1.522E+08	0.0336	0.089	99	99	8625
		E_{eff}/E_g =	0.35	-12981	0.35	1.522E+08	0.0336	0.089	99	99	8625
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	8965	5065	5065	1.522E+08	0.0336	0.089	99	99	8965
		E_{eff}/E_g =	0.35	-13429	0.35	1.522E+08	0.0336	0.089	99	99	8965
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	9334	5183	5183	1.522E+08	0.0336	0.089	99	99	9334
		E_{eff}/E_g =	0.35	-13877	0.35	1.522E+08	0.0336	0.089	99	99	9334
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	9671	5301	5301	1.522E+08	0.0336	0.089	99	99	9671
		E_{eff}/E_g =	0.35	-14098	0.35	1.522E+08	0.0336	0.089	99	99	9671
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	10005	5418	5418	1.522E+08	0.0336	0.089	99	99	10005
		E_{eff}/E_g =	0.35	-14101	0.35	1.522E+08	0.0336	0.089	99	99	10005
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	10337	5301	5301	1.522E+08	0.0336	0.089	99	99	10337
		E_{eff}/E_g =	0.35	-14098	0.35	1.522E+08	0.0336	0.089	99	99	10337
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	10825	5301	5301	1.522E+08	0.0336	0.089	99	99	10825
		E_{eff}/E_g =	0.35	-14098	0.35	1.522E+08	0.0336	0.089	99	99	10825
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	8293	4830	4830	1.522E+08	0.0336	0.089	99	99	8293
		E_{eff}/E_g =	0.35	-12537	0.35	1.522E+08	0.0336	0.089	99	99	8293
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60											
Model periods (sec): $T_1 = 1.92$ $T_2 = 0.63$ $T_3 = 0.37$											
$f_{v,star\ expected} = 67$ ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2068

Number of Stories: 12

Fundamental Period (sec): 2.09

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

2068 Perimeter Frame, 80% Weak Story in first story only

This design was created using design ID 1018 as the baseline. Design ID 1018 had the first two stories weak, and this design is intended to have only the first story weak. Therefore, to make this design, we strengthened the second story columns and the first above ground beam. We based the 80% ratio of story strengths on the strengths computed only from the columns of the first and second stories, assuming both column ends hinged. Even with the weak story, this design is fully code conforming. The bottom story exterior columns were controlled by flexural.

DESIGN AND MODELING DOCUMENTATION FIGURES

Page 235 of 240

Design Documentation (2 of 2)																	
SCWB = 0.83																	
Joint $\Phi V_r/V_u = 4.64$																	
Story 12	$\Phi M_r/M_u = 2.37$																
	$\Phi V_r/V_{mpr} = 1.75$																
	$P/A_g f_c = 0.02$																
Floor 12																	
Story 11	$\Phi M_r/M_u = 2.29$																
	$\Phi V_r/V_{mpr} = 1.62$																
	$P/A_g f_c = 0.04$																
Floor 11																	
Story 10	$\Phi M_r/M_u = 2.05$																
	$\Phi V_r/V_{mpr} = 2.03$																
	$P/A_g f_c = 0.06$																
Floor 10																	
Story 9	$\Phi M_r/M_u = 1.93$																
	$\Phi V_r/V_{mpr} = 2.27$																
	$P/A_g f_c = 0.09$																
Floor 9																	
Story 8	$\Phi M_r/M_u = 1.85$																
	$\Phi V_r/V_{mpr} = 2.55$																
	$P/A_g f_c = 0.11$																
Floor 8																	
Story 7	$\Phi M_r/M_u = 1.81$																
	$\Phi V_r/V_{mpr} = 2.43$																
	$P/A_g f_c = 0.14$																
Floor 7																	
Story 6	$\Phi M_r/M_u = 1.79$																
	$\Phi V_r/V_{mpr} = 2.37$																
	$P/A_g f_c = 0.14$																
Floor 6																	
Story 5	$\Phi M_r/M_u = 1.80$																
	$\Phi V_r/V_{mpr} = 2.28$																
	$P/A_g f_c = 0.16$																
Floor 5																	
Story 4	$\Phi M_r/M_u = 1.82$																
	$\Phi V_r/V_{mpr} = 2.19$																
	$P/A_g f_c = 0.18$																
Floor 4																	
Story 3	$\Phi M_r/M_u = 1.86$																
	$\Phi V_r/V_{mpr} = 2.12$																
	$P/A_g f_c = 0.20$																
Floor 3																	
Story 2	$\Phi M_r/M_u = 1.91$																
	$\Phi V_r/V_{mpr} = 1.8$																
	$P/A_g f_c = 0.22$																
Floor 2																	
Story 1	$\Phi M_r/M_u = 1.45$																
	$\Phi V_r/V_{mpr} = 1.88$																
	$P/A_g f_c = 0.24$																
Design Drifts:																	
0.5%																	
0.8%																	
1.1%																	
1.3%																	
1.5%																	
1.6%																	
1.7%																	
1.8%																	
1.9%																	
1.9%																	
1.8%																	

Modeling Documentation (1 of 1)											
Floor 13	Story 12	$M_{y,exp}$ (k-in) =	5375	2463	2463	2463	2463	2463	2463	2463	5375
		E_{eff}/E_g =	0.35	-6375	0.35	-6375	0.35	-6375	0.35	-6375	0.35
Floor 12	Story 11	$M_{y,exp}$ (k-in) =	5697	2575	2575	2575	2575	2575	2575	2575	5697
		E_{eff}/E_g =	0.35	-9674	0.35	-9674	0.35	-9674	0.35	-9674	0.35
Floor 11	Story 10	$M_{y,exp}$ (k-in) =	6015	2947	2947	2947	2947	2947	2947	2947	6015
		E_{eff}/E_g =	0.35	-9379	0.35	-9379	0.35	-9379	0.35	-9379	0.35
Floor 10	Story 9	$M_{y,exp}$ (k-in) =	6329	3224	3224	3224	3224	3224	3224	3224	6329
		E_{eff}/E_g =	0.35	-9906	0.35	-9906	0.35	-9906	0.35	-9906	0.35
Floor 9	Story 8	$M_{y,exp}$ (k-in) =	6641	3317	3317	3317	3317	3317	3317	3317	6641
		E_{eff}/E_g =	0.35	-10082	0.35	-10082	0.35	-10082	0.35	-10082	0.35
Floor 8	Story 7	$M_{y,exp}$ (k-in) =	6949	3501	3501	3501	3501	3501	3501	3501	6949
		E_{eff}/E_g =	0.35	-10432	0.35	-10432	0.35	-10432	0.35	-10432	0.35
Floor 7	Story 6	$M_{y,exp}$ (k-in) =	7282	3594	3594	3594	3594	3594	3594	3594	7282
		E_{eff}/E_g =	0.35	-10608	0.35	-10608	0.35	-10608	0.35	-10608	0.35
Floor 6	Story 5	$M_{y,exp}$ (k-in) =	7587	3778	3778	3778	3778	3778	3778	3778	7587
		E_{eff}/E_g =	0.35	-10811	0.35	-10811	0.35	-10811	0.35	-10811	0.35
Floor 5	Story 4	$M_{y,exp}$ (k-in) =	7889	3908	3908	3908	3908	3908	3908	3908	7889
		E_{eff}/E_g =	0.35	-10937	0.35	-10937	0.35	-10937	0.35	-10937	0.35
Floor 4	Story 3	$M_{y,exp}$ (k-in) =	8189	4045	4045	4045	4045	4045	4045	4045	8189
		E_{eff}/E_g =	0.35	-11044	0.35	-11044	0.35	-11044	0.35	-11044	0.35
Floor 3	Story 2	$M_{y,exp}$ (k-in) =	8488	4181	4181	4181	4181	4181	4181	4181	8488
		E_{eff}/E_g =	0.35	-11151	0.35	-11151	0.35	-11151	0.35	-11151	0.35
Floor 2	Story 1	$M_{y,exp}$ (k-in) =	8790	4317	4317	4317	4317	4317	4317	4317	8790
		E_{eff}/E_g =	0.35	-11258	0.35	-11258	0.35	-11258	0.35	-11258	0.35
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 0.60 Model periods (sec): $T_1 = 2.09$ $T_2 = 0.69$ $T_3 = 0.40$ $f_{v,taber,exp,acc}$ = 67 ksi											

STRUCTURAL DESIGN AND MODELING SUMMARY

Building Type: Special RC Frame, designed per 2003 IBC

Building Design ID: 2069

Number of Stories: 1

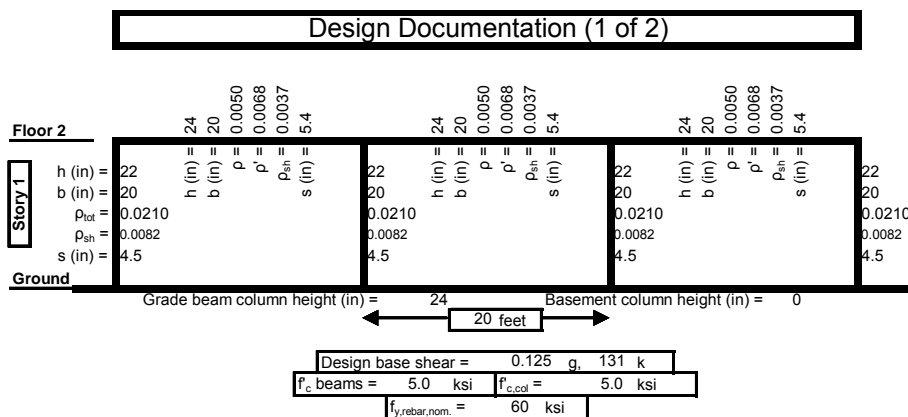
Fundamental Period (sec): 0.71

SUMMARY OF DESIGN DECISIONS AND WHAT CONTROLLED DESIGN

Sizes are controlled by both drift and joint shear, but more drift than joint shear. Beams strength were controlled by force demand, but one beams had additional reinforcement added to keep the same rho ratio between the floors. Column strengths are controlled by flexural demands. Beam stirrups were controlled by the minimum requirement. The column stirrups were controlled by confinement requirement.

The fundamental period is so high for this building because elements are relatively small and the building is a perimeter frame system. The relatively small element sizes from the fact that lateral forces and joint shear demands are smaller compared to taller buildings of this type.

DESIGN AND MODELING DOCUMENTATION FIGURES



Modeling Documentation (1 of 1)

Story 1	SCWB =	0.99	1.18	1.15	0.75	1.17	0.65	1.42	2.40	0.75	1.17	0.65	1.19	1.20	0.75	1.17	0.99				
	Joint $\Phi V_n/V_u$ =	1.58					1.14					1.14					1.58				
	$\Phi M_r/M_u$ =	1.16	$(\Phi M_r/M_u)_{neg}$ =				1.17	$(\Phi M_r/M_u)_{neg}$ =				1.19	$(\Phi M_r/M_u)_{neg}$ =				1.20				
	$\Phi V_n/V_{mgr}$ =	1.65	$(\Phi M_r/M_u)_{pos}$ =				1.61	$(\Phi M_r/M_u)_{pos}$ =				1.61	$(\Phi M_r/M_u)_{pos}$ =				1.65				
	$P/A_g f_c$ =	0.02	$M_{100\psi}/M_u$ =				0.03	$M_{100\psi}/M_u$ =				0.03	$M_{100\psi}/M_u$ =				0.02				
			$\Phi V_n/V_{mgr}$ =					$\Phi V_n/V_{mgr}$ =					$\Phi V_n/V_{mgr}$ =								
Modeling Documentation (1 of 1)																					
Floor 2	Story 4	$M_{y,exp}$ (k-in) =	4959	2875	4779	0.35	6.139E+07	0.046	-0.059	0.100	100	5119	2875	4779	0.35	6.139E+07	0.046	-0.059	0.100	100	4959
		E_{lat}/E_g =	0.35	$M_{y,stab,exp}$ (k-in) =									0.35	$M_{y,stab,exp}$ (k-in) =							0.35
		M_u/M_y =	1.21	E_{lat}/E_g =									1.21	E_{lat}/E_g =							1.21
		$\Theta_{cap,pl}$ (rad) =	0.081	$\Theta_{cap,pl}$ (rad) =									0.079	$\Theta_{cap,pl}$ (rad) =							0.081
		Θ_{pc} (rad) =	0.100	Θ_{pc} (rad) =							0.100	Θ_{pc} (rad) =								0.100	
		λ =	99	λ =							99	λ =								99	
		$(P/A_g f_c)_{exp}$ =	0.01	$(P/A_g f_c)_{exp}$ =							0.02	$(P/A_g f_c)_{exp}$ =								0.01	
Mass tributary to one frame for lateral load (each floor) (k-s-s/in): 2.69																					
Model periods (sec): $T_1 = 0.71$ $T_2 = 0.42$ $T_3 = 0.05$																					
f_y rebarr. expected = 67 ksi																					

Design Drifts: 1.7%

