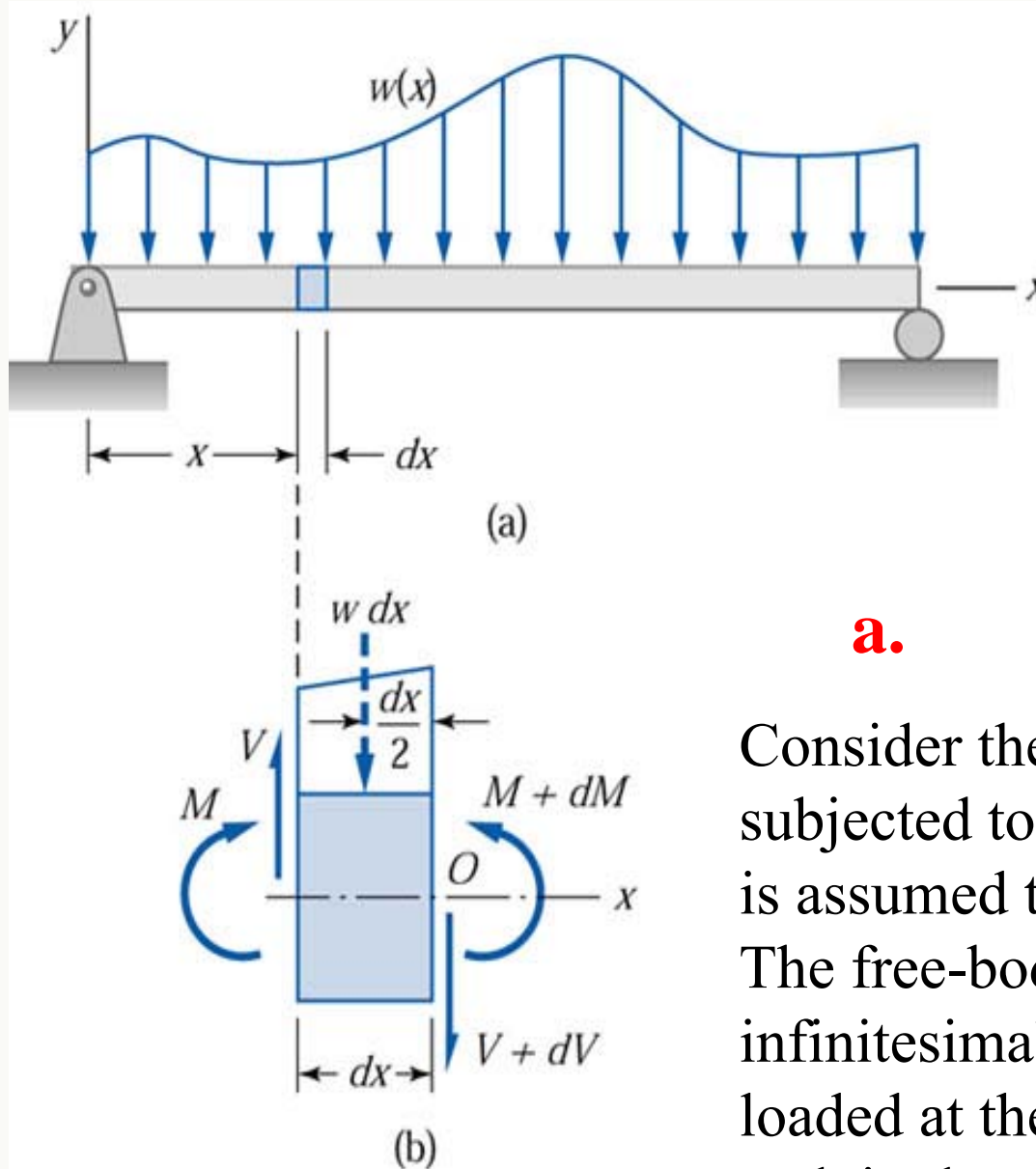


## 4.4 *Area Method for Drawing Shear- Moment Diagrams*

- ❑ Useful **relationships** between **the loading**, **shear force**, and **bending moment** can be derived from the equilibrium equations.
- ❑ These relationships enable us to **plot the shear force diagram directly from the load diagram**, and then construct the **bending moment diagram from the shear force diagram**. This technique, called the ***area method***, allows us to draw the shear force and bending moment diagrams **without having to derive the equations for  $V$  and  $M$** .
- ❑ First consider beam subjected to **distributed loading** and then discuss **concentrated forces and couples**.



**4.4 Figures (a) Simply supported beam carrying distributed loading; (b) free-body diagram of an infinitesimal beam segment.**

### **a. *Distributed loading***

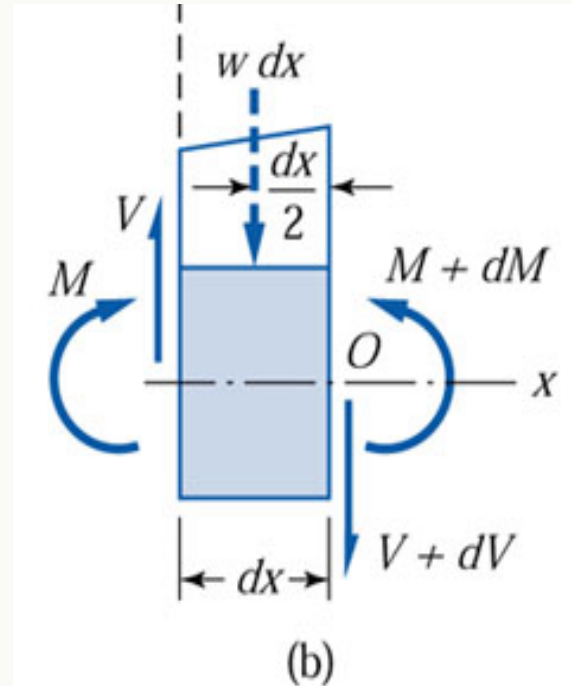
Consider the beam in Fig. 4.4 (a) that is subjected to a **the distributed load  $w(x)$**  is assumed to be a **continuous function**. The free-body diagram of an infinitesimal element of the beam, loaded at the distance  $x$  from the left end, is shown in Fig. 4.4 (b)

The force equation of equilibrium is

$$\Sigma F_y = 0 + \uparrow V - w dx - (V + dV) = 0$$

From which we get

$$w = -\frac{dV}{dx} \quad (4.1)$$



The moment equation of equilibrium yields

$$\Sigma M_O = 0 + \curvearrowright -M - V dx + (M + dM) + w dx (dx/2) = 0$$

After canceling M and dividing by dx, we get

$$-V + \frac{dM}{dx} + \cancel{\frac{w dx}{2}} = 0 \quad V = \frac{dM}{dx} \quad (4.2)$$

Equations (4.1) and (4.2) are called the ***differential equations of equilibrium*** for beams.



The following **five theorems relating the load, the shear force, and the bending moment diagrams** follow from these equations.

1. The load intensity  $w$  at any section of a beam **is equal to the negative of the slope** of the shear force diagram at the section.

*Proof*— follows directly from Eq. (4.1).

$$w = -\frac{dV}{dx}$$

2. The shear force  $V$  at any section **is equal to the slope** of the bending moment diagram at that section.

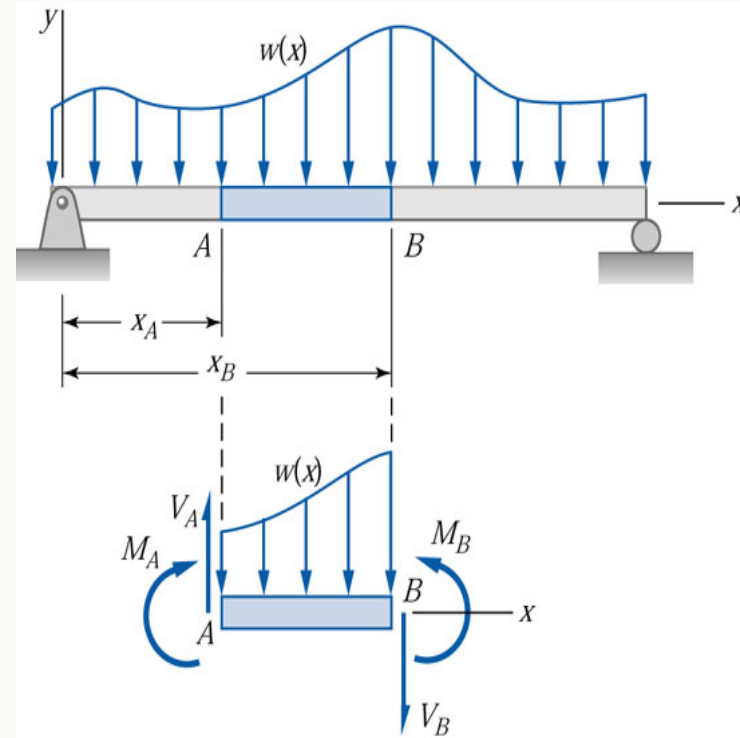
*Proof*— follows directly from Eq. (4.2).

$$V = \frac{dM}{dx}$$



3. The difference between the shear forces at **two sections** of a beam is equal to the **negative of the area under the load diagram** between those two sections.

*Proof*—integrating Eq. (4.1) between sections  $A$  and  $B$  in Fig. 4.5, we obtain



$$w = -\frac{dV}{dx} \quad \int_{x_A}^{x_B} \frac{dV}{dx} dx = V_B - V_A = -\int_{x_A}^{x_B} w dx$$

$$V_B - V_A = -[\text{area of } w\text{-diagram}]_A^B$$

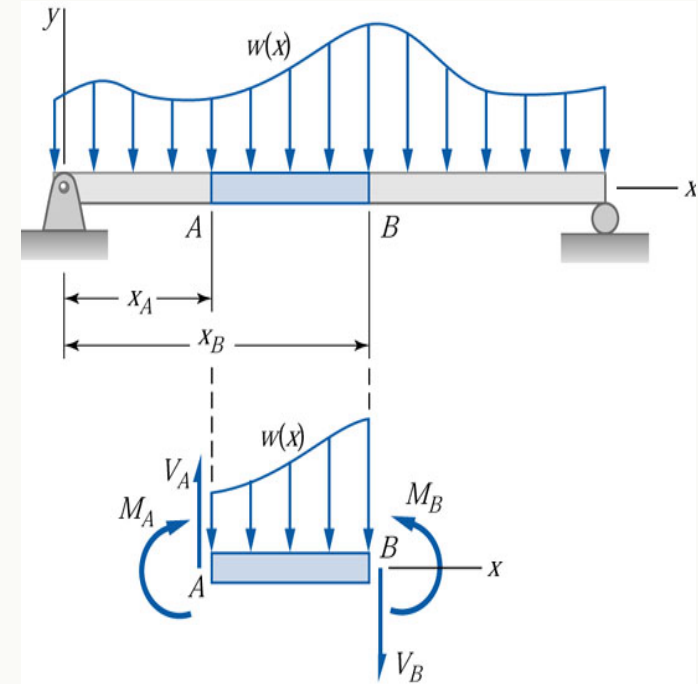
$$V_B = V_A - [\text{area of } w\text{-diagram}]_A^B \quad (4.3)$$

Note that the signs in Eq. (4.3) are correct only if  $x_B > x_A$ .



4. The difference between the bending moments at two sections of a beam is equal to the area of the shear force diagram between these two sections.

*Proof*—integrating Eq. (4.2) between sections A and B in ( see Fig. 4.5),



$$V = \frac{dM}{dx} \quad \int_{x_A}^{x_B} \frac{dM}{dx} dx = M_B - M_A = \int_{x_A}^{x_B} V dx$$

$$M_B - M_A = \text{area of } V\text{-diagram}]_A^B \quad \text{Q.E.D}$$

$$M_B = M_A + \text{area of } V\text{-diagram}]_A^B \quad (4.4)$$

The signs in Eq. (4.4) are correct only if  $x_B > x_A$ .



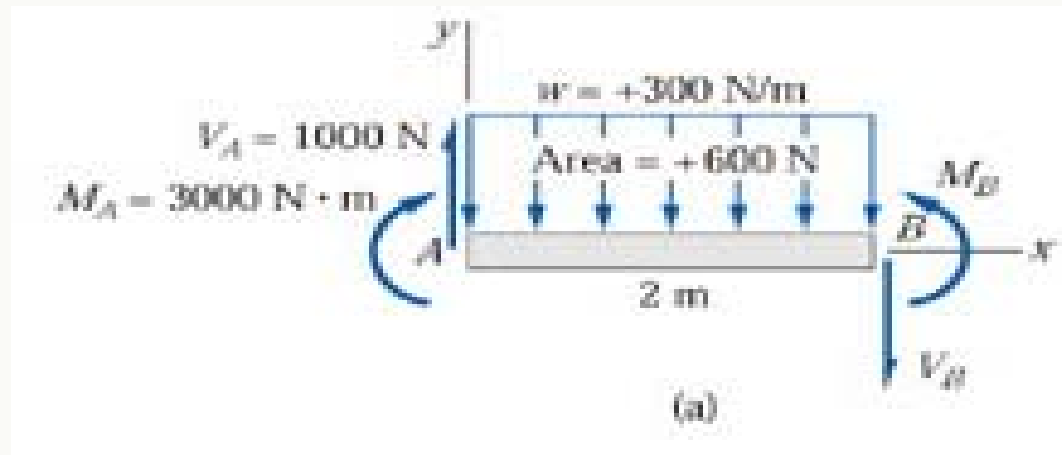
5. If the **load**  $W$  diagram is a **polynomial of degree**  $n$ , then **shear force**  $V$  diagram is polynomial of **degree**  $(n + 1)$ , and the **bending moment**  $M$  diagram is polynomial of **degree**  $(n + 2)$ .

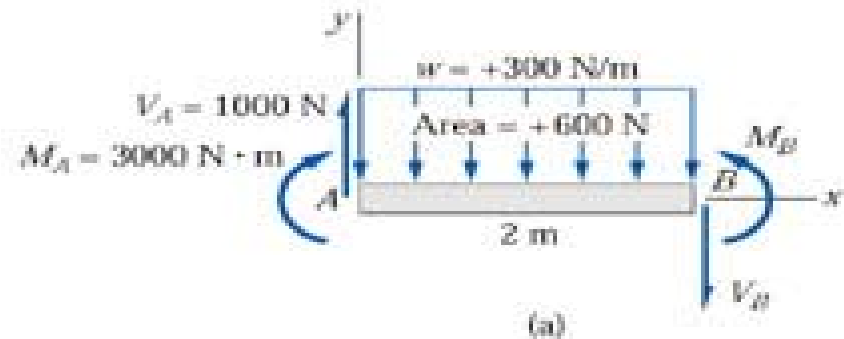
*Proof*— followings directly from the integration of Eqs. (4.1) and (4.2).

$$w = -\frac{dV}{dx}$$

$$V = \frac{dM}{dx}$$

- Consider the beam segment shown in Fig. 4.6 (a), which is 2 m long and is subjected to a uniformly distributed load  $w = 300$  N /m. Figure 4.6 (a) shows the shear force and the bending moment at the left end are  $V_A = +1000$  N and  $M_A = +3000$  N· m.



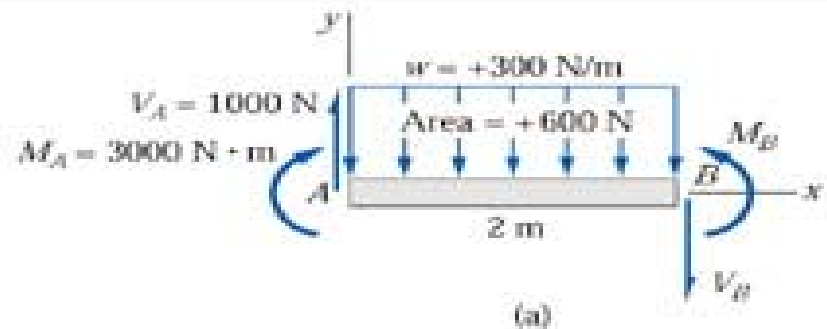


$w = +300 \text{ N/m (const.)}$	
$V_A = +1000 \text{ N (given)}$	
$V_B = V_A - \text{area of } w\text{-diagram} \Big _A^B$ $= 1000 - 600 = +400 \text{ N}$	
$\frac{dV}{dx} = -w = -300 \text{ N/m (const.)}$ <p>V-diagram is a straight line</p>	

**Figure 4.6 (a) Free-body diagram of a beam segment carrying uniform loading;**

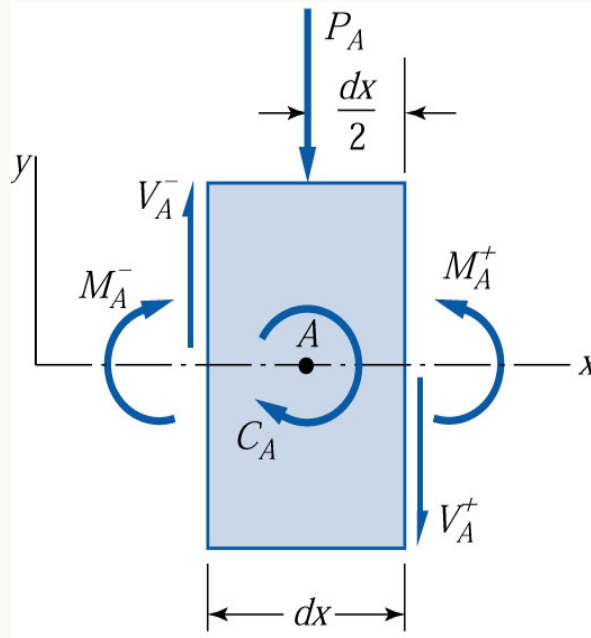






$M_A = +3000 \text{ N} \cdot \text{m}$ (given)	
$M_B = M_A + \text{area of } V\text{-diagram} \Big _A^B$ $= 3000 + 1400 = +4400 \text{ N} \cdot \text{m}$	
$(dM/dx)_A = V_A = +1000 \text{ N}$ $(dM/dx)_B = V_B = +400 \text{ N}$	
$M$ -diagram is a parabola	

**Figure 4.6(b) constructing shear force and bending moment diagrams for the beam segment.**



**Figure 4.7 Free-body diagram of an infinitesimal beam element carrying a concentrated force  $P_A$  and a concentrated couple  $C_A$ .**

### ***b. Concentrated forces and couples.***

□ The force equilibrium equation

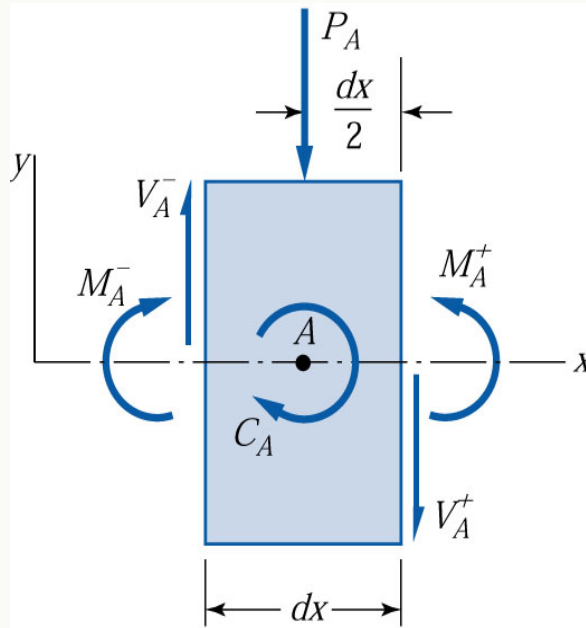
$$\sum F_y = 0 + \uparrow \quad V_A^- - P_A - V_A^+ = 0 \quad \boxed{V_A^+ = V_A^- - P_A} \quad (4.5)$$

Equation (4.5) indicates that a **positive concentrated force** causes a **negative jump discontinuity** in the shear force diagram at A (a concentrated **couple** does not affect the shear force diagram).



The moment equilibrium equation yields

$$\sum M_A = 0 + \curvearrowright \quad M_A^+ - M_A^- - C_A - V_A^+ \frac{dx}{2} - V_A^- \frac{dx}{2} = 0$$



$$M_A^+ = M_A^- + C_A$$

Thus, a **positive concentrated couple** causes a **positive jump** in the bending moment diagram.

**Figure 4.7 Free-body diagram of an infinitesimal beam element carrying a concentrated force  $P_A$  and a concentrated couple  $C_A$ .**

### *c. Summary*

The area method is useful only if the area under the load and shear force diagrams can be easily computed.

$$w = -\frac{dV}{dx} \quad (4.1)$$

$$V = \frac{dM}{dx} \quad (4.2)$$

$$V_B = V_A - \text{area of } w\text{-diagram}]_A^B \quad (4.3)$$

$$M_B = M_A + \text{area of } V\text{-diagram}]_A^B \quad (4.4)$$

$$V_A^+ = V_A^- - P_A \quad (4.5)$$

$$M_A^+ = M_A^- + C_A \quad (4.6)$$



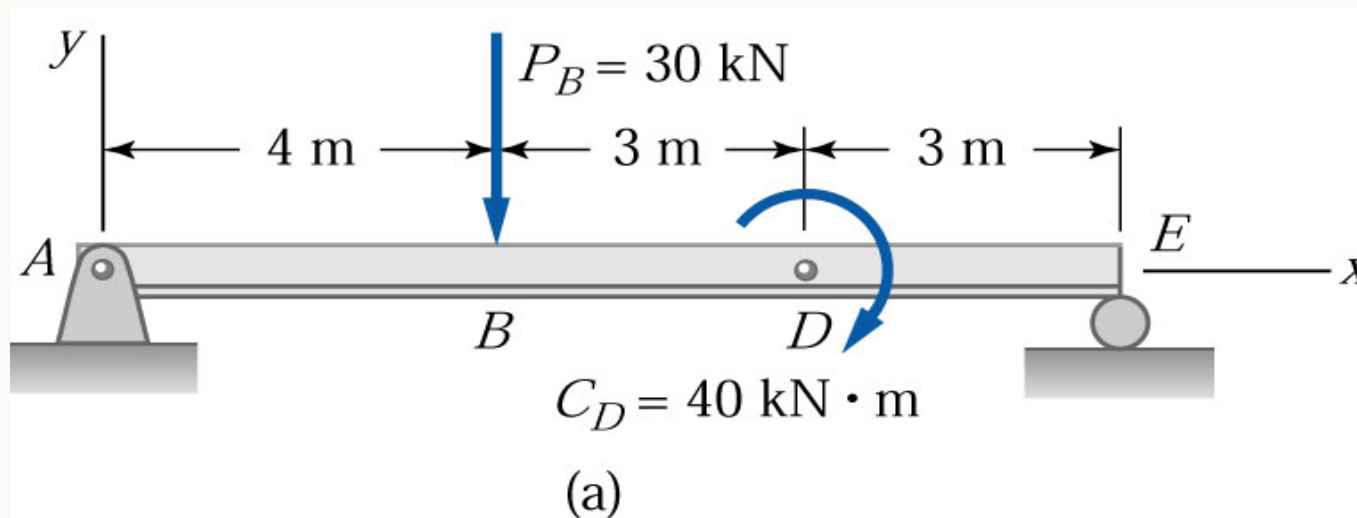
## *Procedure for the Area Method*

- ❑ Compute the support reactions force the free-body diagrams (FBD) of the entire beam.
- ❑ Draw the load diagram of the beam (which is essentially a FBD) showing the values of the loads, including the support reactions. Use the sign conventions in Fig. 4.3 to determine the correct sign of each load.
- ❑ Working from left to right, construct the  $V$ -and  $M$ -diagram for each segment of the beam using Eqs. (4.1)-(4.6).
- ❑ When reach the right end of the beam, check to see whether the computed values of  $V$ -and  $M$  are consistent with the end conditions. If they are not, you made an error in the computations.



## Sample Problem 4.4

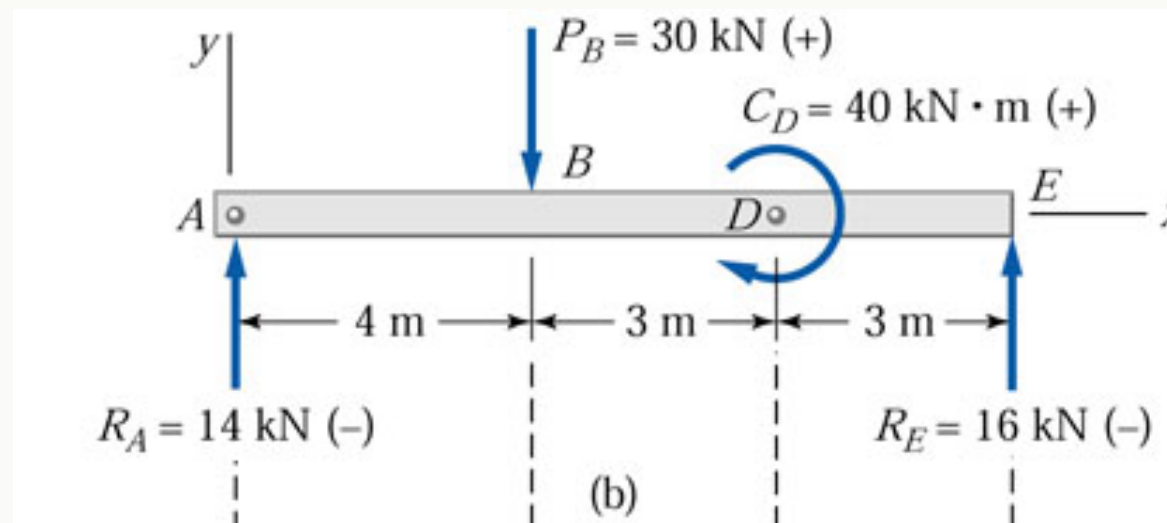
The simply supported beam in Fig. (a) supports 30-kN concentrated force at  $B$  and a 40-kN·m couple at  $D$ . Sketch the shear force and bending moment diagrams by the area method. Neglect the weight of the beam.



## *Solution*

### *Load Diagram*

The load diagram for the beam is shown in Fig. (b). The **reactions** at  $A$  and  $E$  are found from equilibrium analysis. Indicating its sign as established by the sign conventions in Fig. 4.3.



## Shear Force Diagram

There are concentrated forces at  $A, B$ , noting that  $V_A^- = 0$  because no load is applied to the left of  $A$

$$V_A^+ = V_A^- - R_A = 0 - (-14) = (+14 \text{ kN})$$

Plot point. (a)

$$V_B^- = V_A^+ - \text{area of } w\text{-diagram}]_A^B$$

$$= 14 - 0 = 14 \text{ kN} \quad \text{Plot point. (b)}$$

Because  $w = -dV/dx = 0$  between  $A$  and  $B$ ,

Connect (a) and (b) with  
a horizontal straight line

$$V_B^+ = V_B^- - P_B = 14 - (+30) = -16 \text{ kN} \quad \text{Plot point. (c)}$$

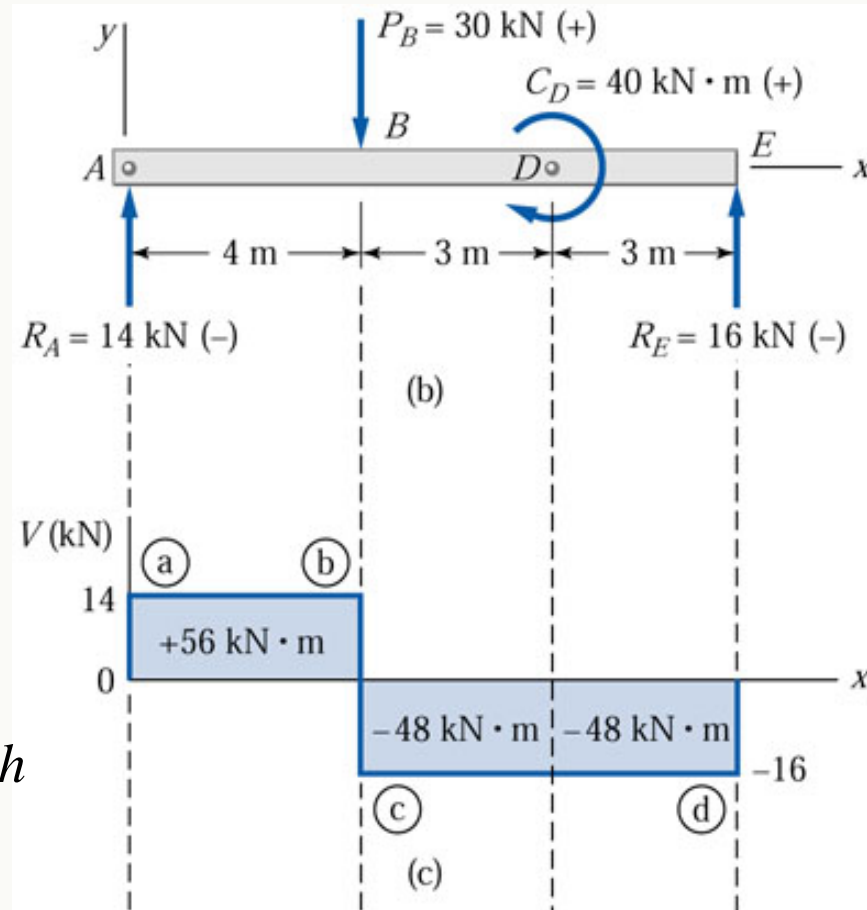
$$V_E^- = V_B^+ - \text{area of } w\text{-diagram}]_B^E = -16 - 0 = -16 \text{ kN} \quad \text{Plot point. (d)}$$

Because  $w = -dV/dx = 0$  between  $B$  and  $E$

Connect (c) and (d) with a horizontal line

$$V_E^+ = V_E^- - R_E = -16 - (-16) = 0$$

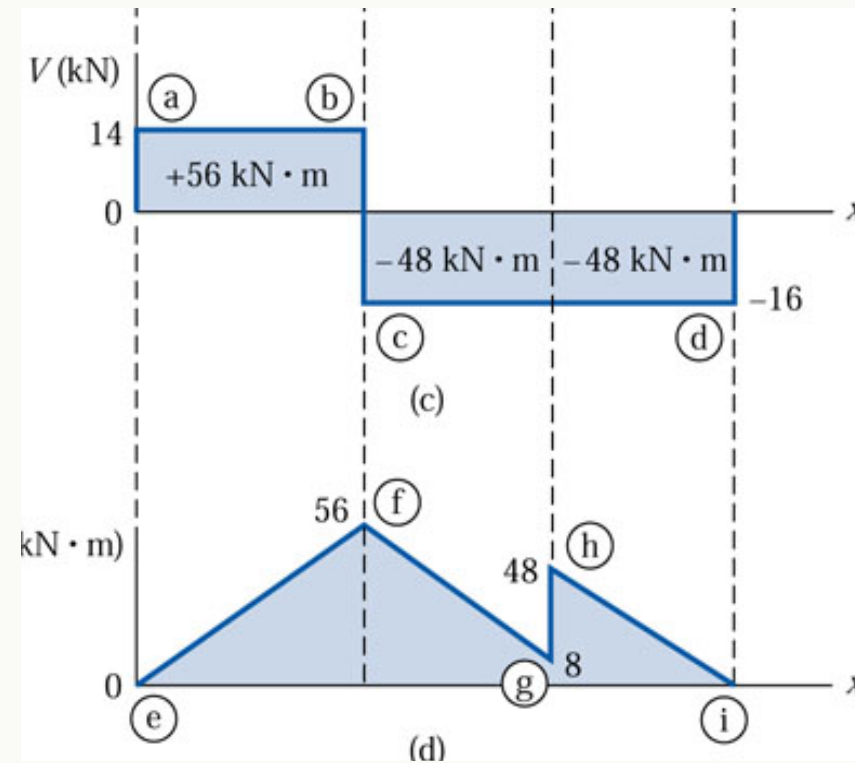
Check !





## Bending Moment Diagram

- The applied couple is cause a jump in the bending moment diagram at D.
- The areas are either positive or negative, depending on the sign of the shear force in Fig. (c).  $M_A = 0$  (there is no couple applied at A). point (e)



$$M_B = M_A + \text{area of V-diagram}]_A^B = 0 + (+56) = 56 \text{ kN} \cdot \text{m}, \text{ point (f)}$$

*The shear force between A and B is constant and positive. The slope of the M-diagram between these two sections is also constant and positive. ( recall that  $V = dM/dx = 0$  ), connect (e) and (f) with straight line.*

$$M_D^- = M_B + \text{area of V-diagram}]_B^D = 56 + (-48) = 8 \text{ kN} \cdot \text{m}, \text{ point (g)}$$



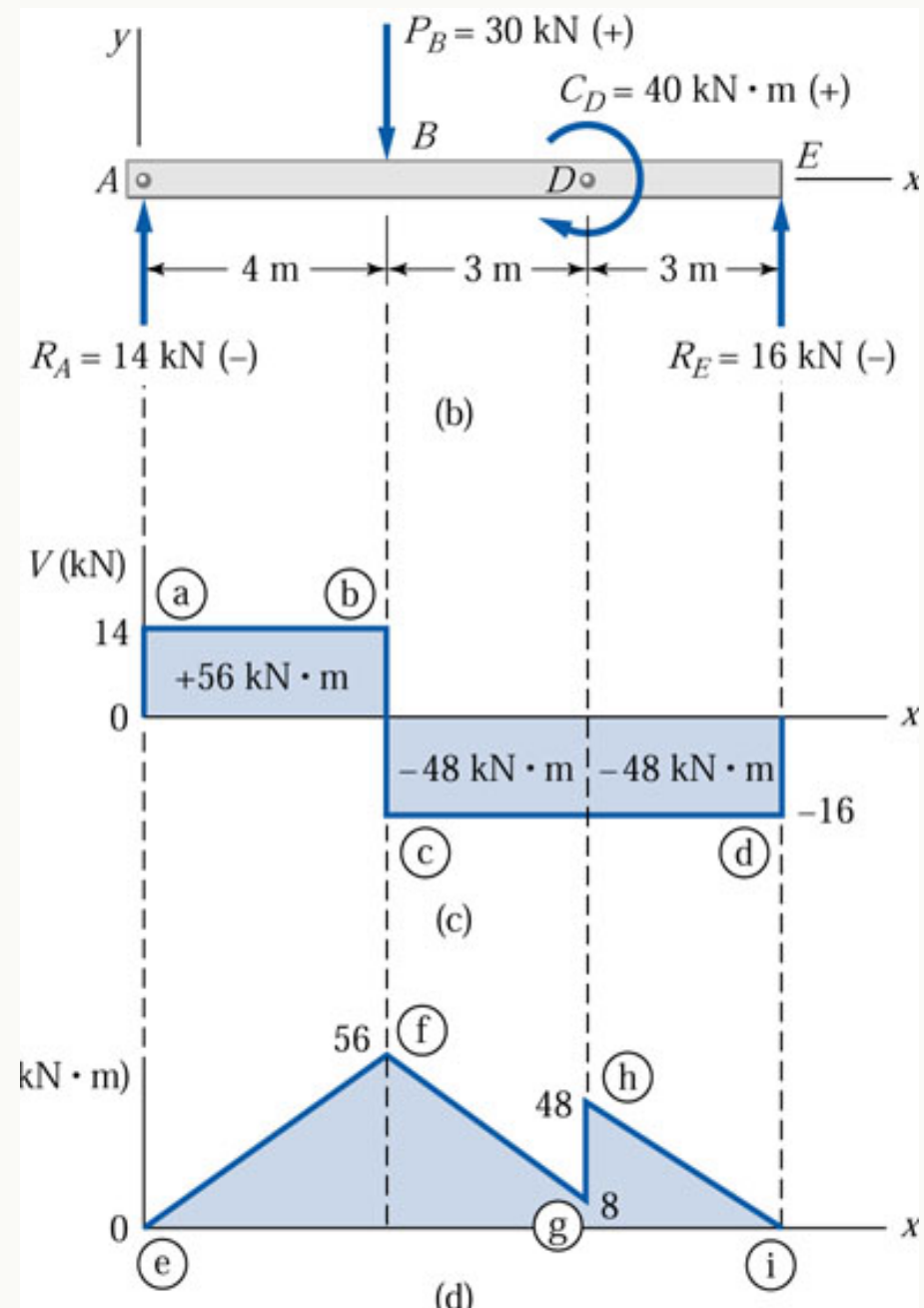
The slope of the  $V$  diagram between  $B$  and  $D$  is negative and constant, the  $M$ -diagram has a constant, negative slope in this segment, ), connect (f) and (g) with straight line.

$$\begin{aligned} M_D^+ &= M_D^- + C_D \\ &= 8 + (+40) = 48 \text{ kN} \cdot \text{m} \end{aligned}$$

Point (h), note that  $M_E = 0$  (there is no couple applied at  $E$ ).

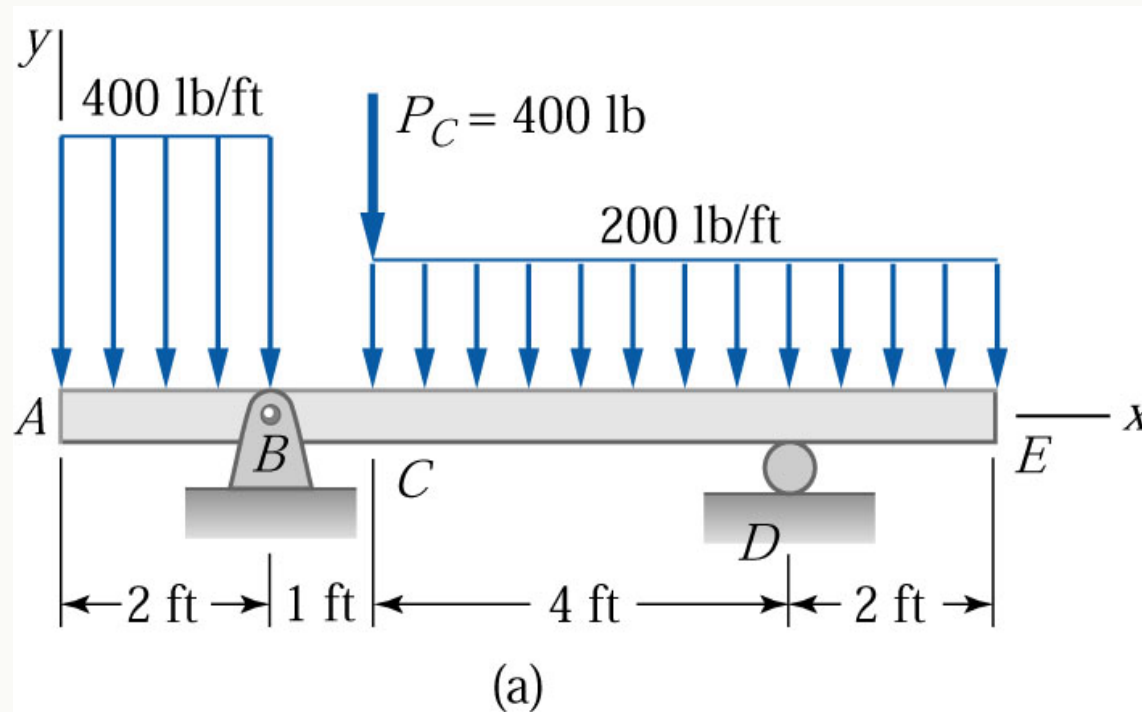
$$\begin{aligned} M_E &= M_D^+ + \text{area of } V\text{-diagram}]_D^E \\ &= 48 + (-48) = 0 \text{ Check !} \end{aligned}$$

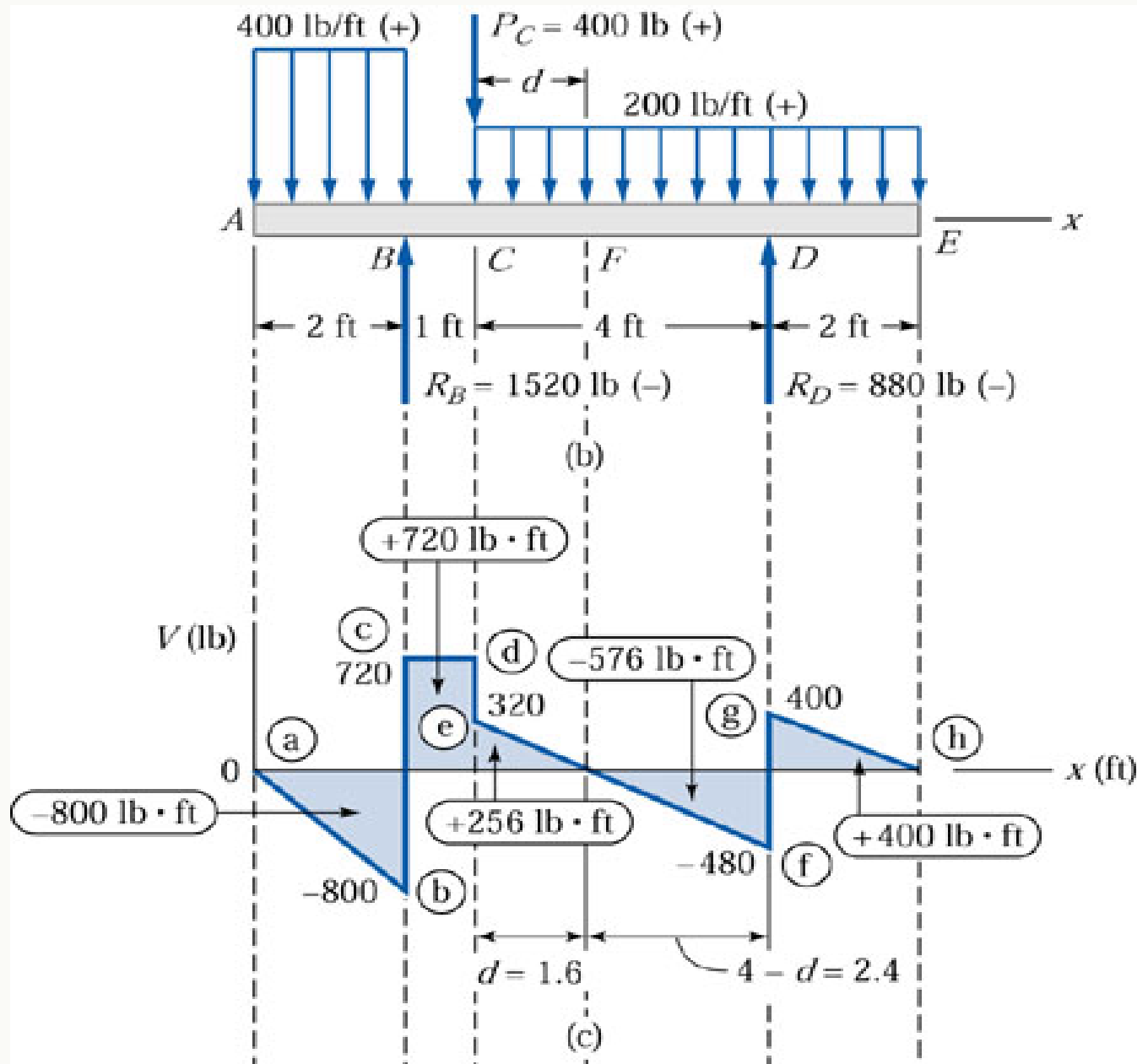
The shear force between  $D$  and  $E$  is negative and constant, which means that the slope of the  $M$ -diagram for this segment is also constant and negative, connect (h) and (i) with straight line.



### Sample Problem 4.5

The overhanging beam in Fig. (a) carries two uniformly distributed loads and a concentrated load. Using the area method. Draw the shear force and bending moment diagrams for the beam.





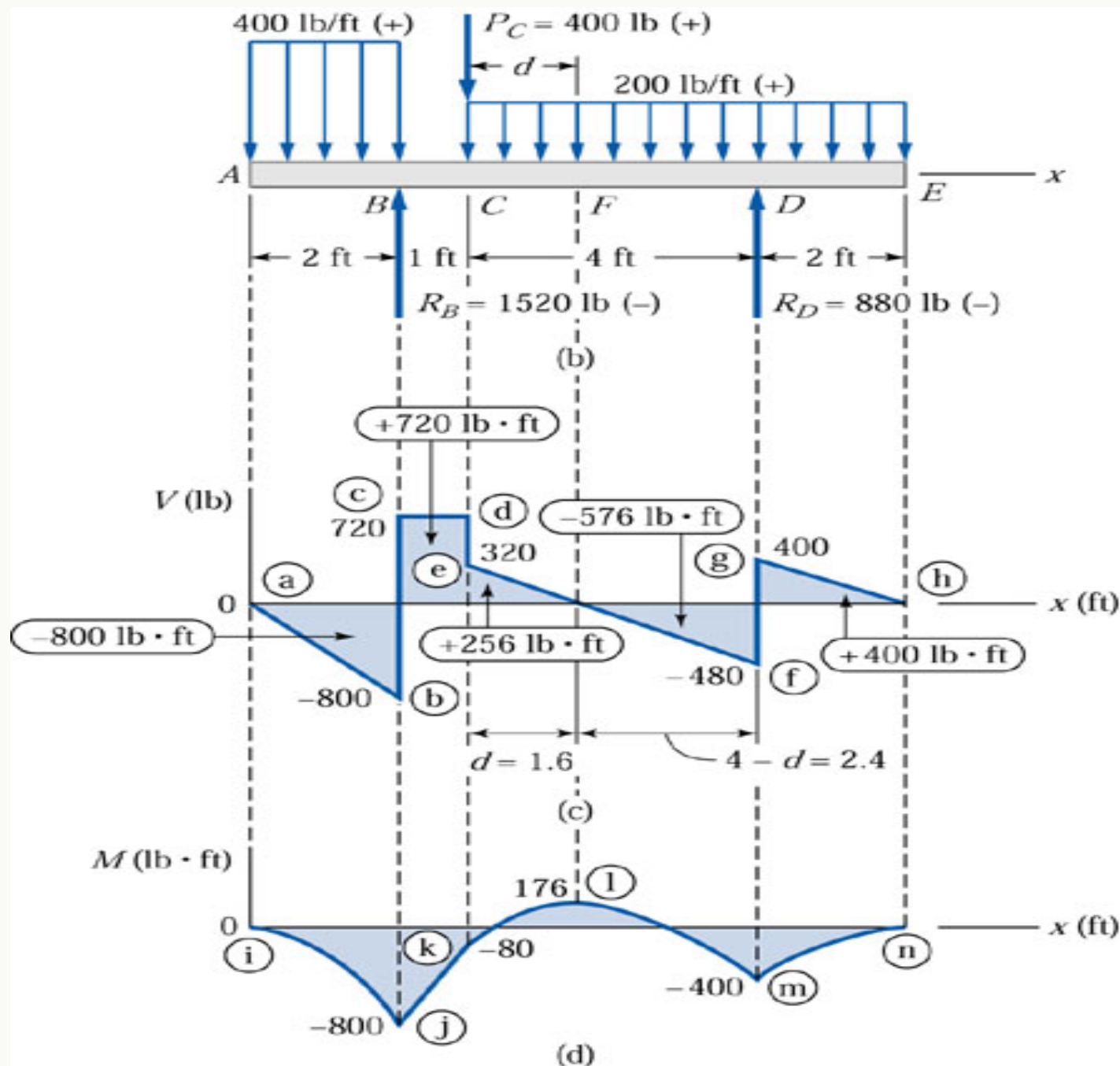
## Load Diagram

The load diagram for the beam is given in Fig. (b)

## Shear Force Diagram

The steps required to construct the shear force diagram in Fig. (c) are now detailed.





## Bending Moment Diagram

The slope of the  $M$ -diagram is discontinuous at **j** and **m**.