

# RETAINING WALL STABILITY ANALYSIS

Project: Sample Project

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## 1. INPUT PARAMETERS

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Parameter	Symbol	Value	Unit
Wall type	Type	Cantilever	
Wall height	H	5.00	m
Base width	B	3.00	m
Toe length	t <sub>toe</sub>	0.30	m
Heel length	t <sub>heel</sub>	2.10	m
Stem thickness (top)	t <sub>stem,top</sub>	0.30	m
Stem thickness (base)	t <sub>stem,base</sub>	0.60	m
Base slab thickness	t <sub>base</sub>	0.60	m
Surcharge	q <sub>s</sub>	10.0	kPa
Backfill unit weight	γ <sub>backfill</sub>	18.0	kN/m <sup>3</sup>
Backfill friction angle	φ <sub>backfill</sub>	30.0	deg
Concrete unit weight	γ <sub>concrete</sub>	24.0	kN/m <sup>3</sup>
Earth pressure theory	Method	Rankine	
Required FOS (sliding)	FOS <sub>s,req</sub>	1.5	
Required FOS (overturning)	FOS <sub>ot,req</sub>	2.0	

## 1. EARTH PRESSURE COMPUTATION

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### Rankine Active Earth Pressure Coefficient

$$K_a = \tan^2(45^\circ - \varphi/2)$$

$$K_a = \tan^2(45^\circ - 30.0^\circ/2)$$

$$K_a = \textcolor{blue}{0.3333}$$

Rankine (1857)

## Active Pressure Height

$$H_{active} = H(\text{nobackfillslope})$$

$$H_{active} = 5.00$$

$$H_{active} = \mathbf{5.000} \text{ m}$$

## Total Active Horizontal Force

$$P_a = 0.5 \times K_a \times \gamma \times H^2 + K_a \times q \times H$$

$$P_a = 0.5 \times 0.3333 \times 18.0 \times 5.000^2 + 0.3333 \times 10.0 \times 5.000$$

$$P_a = \mathbf{91.7} \text{ kN/m}$$

Das Ch. 13

## 1. WALL WEIGHTS & STABILIZING MOMENTS

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### Wall Component Weights and Moments about Toe

Component	Weight (kN/m)	Arm from Toe (m)	Moment (kN·m/m)
Base slab	43.2	1.500	64.8
Stem (rect)	31.7	0.600	19.0
Stem (taper)	15.8	0.700	11.1
Soil on heel	166.3	1.950	324.3
Surcharge on heel	21.0	1.950	40.9
TOTAL	278.0	—	460.1

Moments computed about toe for overturning check.

## 1. SLIDING STABILITY CHECK

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### Base Friction Angle

$$\delta_b = (2/3) \times \varphi_{foundation}$$

$$\delta_b = (2/3) \times 30.0^\circ$$

$$\delta_b = \mathbf{20.00} \text{ deg}$$

Das Ch. 13; typically 1/2 to 2/3 of  $\varphi$

### Resisting Force (Sliding)

$$R = V \times \tan(\delta_b) + c_a \times B$$

$$R = 278.0 \times \tan(20.00^\circ)$$

$$R = \mathbf{101.2} \text{ kN/m}$$

### Factor of Safety Against Sliding

$$FOS_{sliding} = R / P_a$$

$$FOS_{sliding} = 101.2 / 91.7$$

$$FOS_{sliding} = \mathbf{1.185}$$

AASHTO 11.6.3 (min 1.5)

**FAIL** Sliding stability

$$FOS_{required} = 1.5 > FOS_{sliding} = 1.185 \quad (\text{D/C} = 1.27)$$

## 1. OVERTURNING STABILITY CHECK

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### Overturning Moment about Toe

$$M_{ot} = P_a \times z_{P_a}$$

$$M_{ot} = 91.7 \times 1.818$$

$$M_{ot} = \mathbf{166.7} \text{ kN}\cdot\text{m/m}$$

### Stabilizing Moment about Toe

$$M_{stab} = \Sigma(W_i \times x_i)$$

(see weight table above)

$$M_{stab} = 460.1 \text{ kN}\cdot\text{m}/\text{m}$$

### Factor of Safety Against Overturning

$$FOS_{ot} = M_{stab} / M_{ot}$$

$$FOS_{ot} = 460.1 / 166.7$$

$$FOS_{ot} = 2.733$$

AASHTO 11.6.3 (min 2.0)

**PASS** Overturning stability

$$FOS_{required} = 2.0 \leq FOS_{overturning} = 2.733 \quad (\text{D/C} = 0.73)$$

## 1. BEARING PRESSURE & ECCENTRICITY

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### Resultant Location from Toe

$$x_R = (M_{stab} - M_{ot}) / V$$

$$x_R = (460.1 - 166.7) / 278.0$$

$$x_R = 1.056 \text{ m}$$

### Eccentricity of Resultant

$$e = B/2 - x_R$$

$$e = 3.00/2 - 1.056$$

$$e = \mathbf{0.461} \text{ m}$$

B/6 = 0.500 m. Resultant within middle third.

### Toe Bearing Pressure (Trapezoidal Distribution)

$$q_{toe} = (V/B)(1 + 6e/B)$$

$$q_{toe} = (278.0/3.00)(1 + 6 \times 0.461/3.00)$$

$$q_{toe} = \mathbf{178.2} \text{ kPa}$$

### Heel Bearing Pressure

$$q_{heel} = (V/B)(1 - 6e/B)$$

$$q_{heel} = (278.0/3.00)(1 - 6 \times 0.461/3.00)$$

$$q_{heel} = \mathbf{7.1} \text{ kPa}$$

## 1. STABILITY SUMMARY

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### Stability Check Summary

Check	FOS Computed	FOS Required	Status
Sliding	1.185	1.5	FAIL
Overturning	2.733	2.0	OK
Bearing	99.900	N/A	OK

Eccentricity:  $e = 0.461 \text{ m}$  ( $B/6 = 0.500 \text{ m}$ ) — Within middle third.

## 1. FIGURES

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### Cantilever Retaining Wall Cross-Section

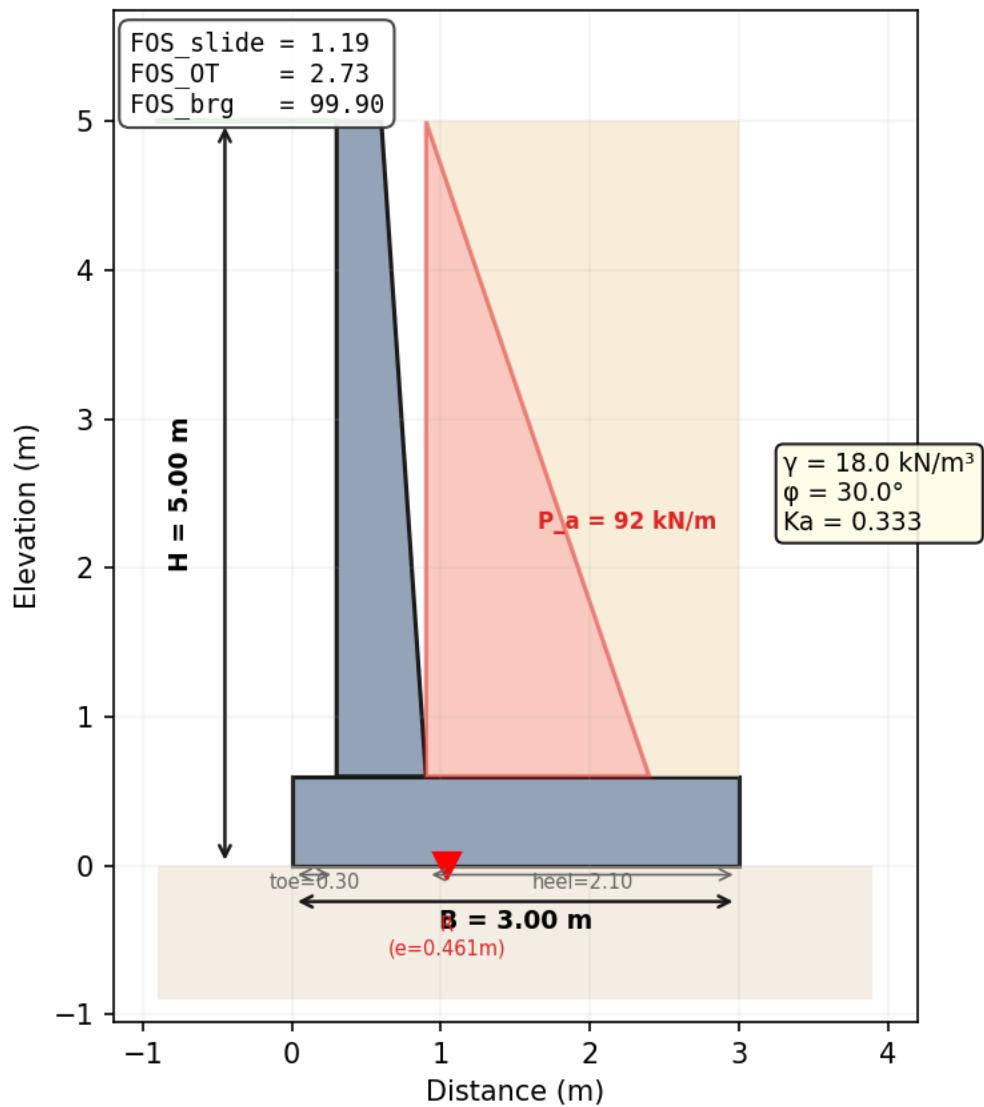


Figure 1:  
Figure 1: Cantilever wall cross-section.  $H = 5.00$  m,  $B = 3.00$  m.

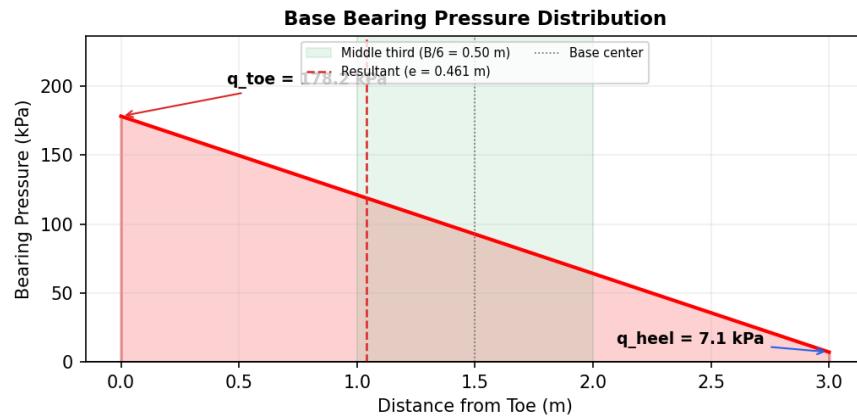


Figure 2: \*

Figure 2: Bearing pressure distribution along base.  $q_{\text{toe}} = 178.2 \text{ kPa}$ ,  $q_{\text{heel}} = 7.1 \text{ kPa}$ .

## 1. REFERENCES

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1. FHWA GEC-11 (FHWA-NHI-10-024): Design of Mechanically Stabilized Earth Walls and Reinforced Slopes. FHWA, 2009.
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5. Rankine, W.J.M. (1857). "On the Stability of Loose Earth." Philosophical Transactions of the Royal Society, 147.