

**REXCONN****D E S I G N****STRUCTURAL ENGINEERING**

JOB _____ NO. _____

SHEET NO. 2 OF H

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

DESCRIPTION HSS6.625X0.25 T <=50 kips

Hanger: HSS6.625x0.25

$$A_g = 4.68 \text{ in}^2$$

$$F_y = 42 \text{ ksi}$$

$$F_u = 58 \text{ ksi}$$

$$t = 0.233 \text{ in}$$

$$T = 50 \text{ kips}$$

HSS Check

-HSS to WF web weld

Try 1/4 fillet weld (4 side)

Try L = 6.75" Min

$$D_{eff} = 1/4 - 1/16 = 3/16$$

$$\phi R_n = 1.392 \cdot D_{eff} \cdot 4 \cdot L = 1.392 \cdot 3 \cdot 6.75 \cdot 4 = 112.8 \text{ kips} > T \text{ Okay}$$

-Tension Yielding HSS

$$\phi R_n = 0.9 F_y A_g = 0.9 \cdot 42 \cdot 4.68 = 176.9 \text{ kips} \geq T \text{ Okay}$$

-Tension Rupture HSS

$$A_g = 4.68 \text{ in}^2$$

$$A_n = A_g - 2 \cdot (t_w + 0.125) \cdot t = 4.68 - 2 \cdot (0.375 + 0.125) \cdot 0.233 = 4.45 \text{ in}^2$$

$$X = D/\pi = 6.625/3.14 = 2.1088$$

$$U = 1 - (X/L) = 1 - (2.1088/6.75) = 0.6875$$

$$\text{If } L > 1.3 \cdot D \text{ then } U = 1$$

$$A_e = U A_n = 3.06 \text{ in}^2$$

$$\phi R_n = 0.75 \cdot F_u \cdot A_e = 0.75 \cdot 58 \cdot 3.06 = 133.1 \text{ kips} > T \text{ Okay}$$

WebBeam $F_y = 50 \text{ ksi}$; Beam $F_u = 65 \text{ ksi}$

-Shear Yielding of WF Web

$$\text{Web thickness } t_w = 0.375 \text{ in} \times 8$$

$$\phi R_n = 1 \cdot 0.6 \cdot F_y \cdot t_w \cdot L \cdot 2 = 1 \cdot 0.6 \cdot 50 \cdot 0.375 \cdot 6.75 \cdot 2 = 151.9 \text{ kips}$$

-Tension Yielding

$$\phi R_n = 0.9 \cdot A_g \cdot F_y = 135 \text{ kips} > T \text{ Okay}$$

-Tension Rupture

$$\phi R_n = 0.75 \cdot 0.85 \cdot A_g \cdot F_u = 124.3 \text{ kips} > T \text{ Okay}$$

Flange

$$\text{Flange width } b_f = 8 \text{ in}$$

$$\text{Flange thickness } t_f = 0.375 \text{ in}$$

Try 3/4" A325-N-STD (row x column) 2x2

$$\phi r_n = 15.9 \text{ k/bolt}$$

$$\phi R_n = \phi r_n \cdot \text{row} \cdot \text{column} = 15.9 \cdot 2 \cdot 2 = 63.6 \text{ kips} > T/2 \text{ Okay}$$

**REXCONN****D E S I G N****STRUCTURAL ENGINEERING**

JOB _____ NO. _____

SHEET NO. 3 OF H

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

DESCRIPTION HSS6.625X0.25T=50K

-Tension Yielding

$$\phi R_n = 0.9 * F_y * b_f * t_f = 135 \text{ kips} > T/2 \text{ Okay}$$

-Tension Rupture

$$\phi R_n = 0.75 * F_u * (b_f - (2 * (0.75 + 0.125))) t_f = 114.3 \text{ kips} > T/2 \text{ Okay}$$

-Block shear rupture

$$\phi R_n = 94.1 \text{ kips} > T/2 \text{ Okay}$$

Flange Plate

Plate thickness $t_p = 0.5 \text{ in}$

Plate width $b_p = 8 \text{ in}$

Plate $F_y = 36 \text{ ksi}$

Plate $F_u = 58 \text{ ksi}$

-Tension Yielding

$$\phi R_n = 0.9 * F_y * b_p * t_p = 129.6 \text{ kips} > T/2 \text{ Okay}$$

-Tension Rupture

$$\phi R_n = 0.75 * F_u * (b_p - (2 * (0.75 + 0.125))) t_p > 135.9 \text{ kips} > T/2 \text{ Okay}$$

-Block shear rupture

$$\phi R_n = 104.2 \text{ kips} > T/2 \text{ Okay}$$

-WT Check

$e = 8'' \text{ Max}$

$T/2 = 25 \text{ kips}$

$M = T * e / 2 = 200 \text{ kips-in}$

Try 3/8 WT stem

$D_{eff} = 3.94 \text{ #16's}$

$L_w = 15'' \quad S_w = L_w^2 / 3 = 75 \text{ in}^2$

$f_{req} = M / S_w + (T/2) / (2 * L_w) = 3.5 \text{ k/in}$

$F_{allow} = 1.392 * D_{eff} = 5.48 \text{ k/in} > f_{req} \text{ Okay}$

$\phi M_n = L_w^2 * 0.375 * .25 * 50 * 0.9 = 949.2 \text{ kips-in} > M \text{ Okay}$