

Toshio Mura

Micromechanics of Defects in Solids

Second, Revised Edition

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Preface

This book stems from a course on Micromechanics that I started about fifteen years ago at Northwestern University. At that time, micromechanics was a rather unfamiliar subject. Although I repeated the course every year, I was never convinced that my notes have quite developed into a final manuscript because new topics emerged constantly requiring revisions, and additions. I finally came to realize that if this is continued, then I will never complete the book to my total satisfaction. Meanwhile, T. Mori and I had coauthored a book in Japanese, entitled *Micromechanics*, published by Baifu-kan, Tokyo, in 1975. It received an extremely favorable response from students and researchers in Japan. This encouraged me to go ahead and publish my course notes in their latest version, as this book, which contains further development of the subject and is more comprehensive than the one published in Japanese.

Micromechanics encompasses mechanics related to microstructures of materials. The method employed is a continuum theory of elasticity yet its applications cover a broad area relating to the mechanical behavior of materials: plasticity, fracture and fatigue, constitutive equations, composite materials, polycrystals, etc. These subjects are treated in this book by means of a powerful and unified method which is called the ‘eigenstrain method.’ In particular, problems relating to inclusions and dislocations are most effectively analyzed by this method, and therefore, special emphasis is placed on these topics. When this book is used as a text for a graduate course, Sections 3, 11, and 22 should be emphasized.

Eigenstrain is a generic name given by the author to such nonelastic strains as thermal expansion, phase transformation, and misfit strains. J.D. Eshelby, who is a pioneer in this area, refers to eigenstrains as stress-free transformation strains in his celebrated papers (1957, 1959). The term eigenstrain should not be confused with the term ‘eigenvalue’ which occurs in mathematical physics, and relates to an entirely different concept.

No particular background is required of readers of this book because necessary mathematics and physics are explained in the text and Appendix.

Although I have tried to be fair in citing literature, I have to apologize if some papers do not receive proper credit or are not cited.

The sections and subsections marked with an asterisk (*), can be skipped in the first reading, since the subjects discussed there are peripheral to the main theme.

I wish to express my thanks to all the people who have helped me during the course of the preparation of the manuscript: my previous graduate students, Zisis A. Moschovidis, Minoru Taya, Carl R. Vilman, and Ronald B. Castles, as well as my friends R. Furuhashi, N. Kinoshita, T. Morita, M. Inokuti, and T. Mori. Mori receives my special thanks for having advised me on the subject matter, for discussing with me whole chapters, and for helping me to write Chapter 7. The manuscript reached the final form in his hands. I also wish to thank S. Nemat-Naser who has read through the manuscript and has given valuable comments.

I give my thanks to Vera Fisher for her skillful typing and her great patience with me, to the secretaries whom I involved in various aspects of the work over the years: Erika Ivansons, Miriam Littell, Masa Sumikura, and Carolyn Andrews, and to my family for their patience and understanding.

Finally, I acknowledge the National Science Foundation and the U.S. Army Research Office for their support of my research in the area of micromechanics.

November 13, 1980

T.M.

Contents

Preface	v
Chapter 1. General theory of eigenstrains	1
1. Definition of eigenstrains	1
2. Fundamental equations of elasticity	3
Hooke's law	3
Equilibrium conditions	5
Compatibility conditions	6
3. General expressions of elastic fields for given eigenstrain distributions	7
Periodic solutions	7
Method of Fourier series and Fourier integrals	9
Method of Green's functions	11
Isotropic materials	13
Cubic crystals	14
Hexagonal crystals (transversely isotropic)	14
4. Exercises of general formulae	15
A straight screw dislocation	15
A straight edge dislocation	18
Periodic distribution of cuboidal precipitates	20
5. Static Green's functions	21
Isotropic materials	22
* Anisotropic materials	25
* Transversely isotropic materials	26
* Kröner's formula	31
* Derivatives of Green's functions	32
* Two-dimensional Green's function	34
6. Inclusions and inhomogeneities	38
Inclusions	38
Inhomogeneities	40
* Effect of isotropic elastic moduli on stress	42

7. Dislocations	44
Volterra and Mura formulas	45
* The Indenbom and Orlov formula	48
* Disclinations	49
8. Dynamic solutions	53
Uniformly moving edge dislocation	55
Uniformly moving screw dislocation	57
*9. Dynamic Green's functions	57
Isotropic materials	61
Steady State	64
*10. Incompatibility	65
* Riemann-Christoffel curvature tensor	71
 Chapter 2. Isotropic inclusions	 74
11. Eshelby's solution	74
Interior points	75
Sphere	79
Elliptic cylinder	80
Penny-shape	81
Flat ellipsoid	83
Oblate spheroid	84
Prolate spheroid	84
Exterior points	84
Thermal expansion with central symmetry	88
*12. Ellipsoidal inclusions with polynomial eigenstrains	89
* The I-integrals	92
* Sphere	93
* Elliptic cylinder	94
* Oblate spheroid	94
* Prolate spheroid	94
* Elliptical plate	95
* The Ferrers and Dyson formula	95
13. Energies of inclusions	97
Elastic strain energy	97
Interaction energy	99
Strain energy due to a spherical inclusion	101
Elliptic cylinder	101
Penny-shaped flat ellipsoid	101
Spheroid	102
*14. Cuboidal inclusions	104

<i>Contents</i>	ix
15. Inclusions in a half space	110
Green's functions	110
Ellipsoidal inclusion with a uniform dilatational eigenstrain	114
* Cuboidal inclusion with uniform eigenstrains	121
* Periodic distribution of eigenstrains	121
Joined half-spaces	123
Chapter 3. Anisotropic inclusions	129
16. Elastic field of an ellipsoidal inclusion	129
17. Formulae for interior points	133
Uniform eigenstrains	134
Spheroid	137
Cylinder (elliptic inclusion)	141
Flat ellipsoid	143
Eigenstrains with polynomial variation	144
Eigenstrains with a periodic form	144
*18. Formulae for exterior points	149
Examples	156
19. Ellipsoidal inclusions with polynomial eigenstrains in anisotropic media	158
Special cases	160
*20. Harmonic eigenstrains	161
21. Periodic distribution of spherical inclusions	165
Chapter 4. Ellipsoidal inhomogeneities	177
22. Equivalent inclusion method	178
Isotropic materials	181
Sphere	183
Penny shape	184
Rod	185
Anisotropic inhomogeneities in isotropic matrices	187
Stress field for exterior points	187
23. Numerical calculations	188
Two ellipsoidal inhomogeneities	192
*24. Impotent eigenstrains	198
25. Energies of inhomogeneities	204
Elastic strain energy	204
Interaction energy	208
Colunnetti's theorem	211
Uniform plastic deformation in a matrix	213
Energy balance	215

26. Precipitates and martensites	218
Isotropic precipitates	219
Anisotropic precipitates	220
Incoherent precipitates	226
Martensitic transformation	229
Stress orienting precipitation	237
Chapter 5. Cracks	240
27. Critical stresses of cracks in isotropic media	240
Penny-shaped cracks	240
Slit-like cracks	242
Flat ellipsoidal cracks	244
Crack opening displacement	247
28. Critical stresses of cracks in anisotropic media	248
Uniform applied stress	248
Non-uniform applied stress	253
* Π integrals for a penny-shaped crack	255
* Π integrals for cubic crystals	255
* Π integrals for transversely isotropic materials	257
29. Stress intensity factor for a flat ellipsoidal crack	260
Uniform applied stresses	264
Non-uniform applied stresses	268
30. Stress intensity factor for a slit-like crack	271
Uniform applied stresses	272
Non-uniform applied stresses	274
Isotropic materials	274
31. Stress concentration factors	277
Simple tension	278
Pure shear	279
32. Dugdale-Barenblatt cracks	280
BCS model	288
Penny shaped crack	292
*33. Stress intensity factor for an arbitrarily shaped plane crack	297
* Numerical examples	305
34. Crack growth	307
Energy release rate	307
The J-integral	311
Fatigue	314
Dynamic crack growth	319

Chapter 6. Dislocations	324
35. Displacement fields	324
Parallel dislocations	325
A straight dislocation	327
36. Stress fields	327
Dislocation segments	328
Willis' formula	333
The Asaro et al. formula	334
Dislocation loops	335
37. Dislocation density tensor	338
Surface dislocation density	341
Impotent distribution of dislocations	343
38. Dislocation flux tensor	345
Line integral expression of displacement and plastic distortion fields	348
The elastic field of moving dislocationswave equations of tensor potentials.	351
Wave equations of tensor potentials	352
39. Energies and forces	353
Dynamic consideration	354
40. Plasticity	361
Mathematical theory of plasticity	361
Dislocation theory	363
Plane strain problems	365
Beams and cylinders	373
41. Dislocation model for fatigue crack initiation	379
Chapter 7. Material properties and related topics	388
42. Macroscopic average	388
Average of internal stresses	388
Macroscopic strains	389
Tanaka-Mori's theorem	390
Image stress	393
Random distribution of inclusions-Mori and Tanaka's theory	394
43. Work-hardening of dispersion hardened alloys	398
Work-hardening in simple shear	398
Dislocations around an inclusion	402
Uniformity of plastic deformation	405
44. Diffusional relaxation of internal and external stresses	406
Relaxation of the internal stress in a plastically deformed dispersion strengthened alloy	407

Diffusional relaxation process, climb rate of an Orowan loop	408
Recovery creep of a dispersion strengthened alloy	412
Interfacial diffusional relaxation	414
45. Average elastic moduli of composite materials	421
The Voigt approximation	421
The Reuss approximation	424
Hill's theory	426
Eshelby's method	428
Self-consistent method	430
Upper and lower bounds	433
Other related works	437
46. Plastic behavior of polycrystalline metals and composites	439
Taylor's analysis	439
Self-consistent method	443
Embedded weakened zone	448
47. Viscoelasticity of composite materials	449
Homogeneous inclusions	449
Inhomogeneous inclusions	452
Waves in an infinite medium	453
48. Elastic wave scattering	455
Dynamic equivalent inclusion method	459
Green's formula	460
49. Interaction between dislocations and inclusions	463
Inclusions and dislocations	463
Cracks in two-phase materials	471
50. Eigenstrains in lattice theory	477
A uniformly moving screw dislocation	480
51. Sliding inclusions	484
Shearing Eigenstrains	486
Spheroidal inhomogeneous inclusions	488
52. Recent developments	492
Inclusions, precipitates, and composites	492
Half-spaces	494
Non-elastic matrices	494
Cracks and inclusions	495
Sliding and debonding inclusions	497
Dynamic cases	497
Miscellaneous	498

<i>Contents</i>	xiii
Appendix 1	499
Einstein summation convention	499
Kronecker delta	499
Permutation tensor	499
Appendix 2	501
The elastic moduli for isotropic materials	501
Appendix 3	505
Fourier series and integrals	505
Dirac's delta function and Heaviside's unit function	507
Laplace transform	508
Appendix 4	510
Dislocations pile-up	510
References	513
Author index	572
Subject index	582

General theory of eigenstrains

The definition of eigenstrains is given first. Then the associated general solutions for elastic fields for given eigenstrains are expressed by Fourier integrals and Green's functions. Some details of calculations for Green's functions are described for static and dynamic cases.

As fundamental formulae for the subsequent chapters, general expressions of elastic fields are given for inclusions, dislocations, and disclinations. The stress discontinuity on boundaries of inclusions and the incompatibility of eigenstrains are discussed as general theories.

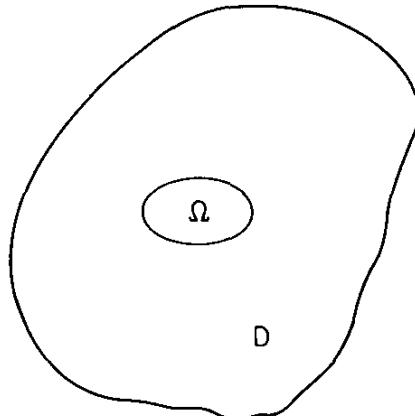
Throughout this work, a fixed rectangular Cartesian coordinate system with coordinate axes x_i , $i = 1, 2, 3$, is used.

1. Definition of eigenstrains

'Eigenstrain' is a generic name given by the author to such nonelastic strains as thermal expansion, phase transformation, initial strains, plastic strains, and misfit strains. 'Eigenstress' is a generic name given to self-equilibrated internal stresses caused by one or several of these eigenstrains in bodies which are free from any other external force and surface constraint. The eigenstress fields are created by the incompatibility of the eigenstrains.

This new English terminology was adapted from the German 'Eigenspannungen und Eigenspannungsquellen,' which is the title of H. Reissner's paper (1931) on residual stresses. Eshelby (1957) referred to eigenstrains as stress-free transformation strains in his celebrated paper which has stimulated the present author to work on inclusion and dislocation problems. The term 'elastic polarization' was used by Kröner (1958) for eigenstrains in a slightly different context—when the nonhomogeneity of polycrystal deformation is under consideration.

Engineers have used the term 'residual stresses' for the self-equilibrated internal stresses when they remain in materials after fabrication or plastic deformation. Eigenstresses are called thermal stresses when thermal expansion

Fig 1.1 Inclusion Ω

is a cause of the corresponding elastic fields. For example, when a part Ω of a material (Fig. 1.1) has its temperature raised by T , thermal stress σ_{ij} is induced in the material D by the constraint from the part which surrounds Ω . The thermal expansion αT , where α is the linear thermal expansion coefficient, constitutes the thermal expansion strain,

$$\epsilon_{ij}^* = \delta_{ij} \alpha T, \quad (1.1)$$

where δ_{ij} is the Kronecker delta (see Appendix 1). The thermal expansion strain is the strain caused when Ω can be expanded freely with the removal of the constraint from the surrounding part.

The actual strain is then the sum of the thermal and elastic strains. The elastic strain is related to the thermal stress by Hooke's law. The thermal expansion strain (1.1) is a typical example of an eigenstrain. In the elastic theory of eigenstrains and eigenstresses, however, it is not necessary to attribute ϵ_{ij}^* to any specific source. The source could be phase transformation, precipitation, plastic deformation or a fictitious source necessary for the equivalent inclusion method (to be discussed in Section 22).

When an eigenstrain ϵ_{ij}^* is prescribed in a finite subdomain Ω in a homogeneous material D (see Fig. 1.1) and it is zero in the matrix $D-\Omega$, then Ω is called an inclusion. The elastic moduli of the material are assumed to be homogeneous when inclusions are under consideration.

If a subdomain Ω in a material D has elastic moduli different from those of the matrix, then Ω is called an inhomogeneity. Applied stresses will be disturbed by the existence of the inhomogeneity. This disturbed stress field will be simulated by an eigenstress field by considering a fictitious eigenstrain ϵ_{ij}^* in Ω in a *homogeneous* material.

When Ω in Fig. 1.1 is a plane embedded in a three-dimensional material D and ϵ_{ij}^* is given on Ω as a plastic strain caused by a finite slip b , the boundary

of Ω is called a dislocation loop. If ϵ_{ij}^* is created by a rigid rotation of plane Ω by ω , the boundary of Ω is called a disclination loop.

2. Fundamental equations of elasticity

In this section the field equations for the elasticity theory will be reviewed with particular reference to solving eigenstrain problems. These problems consist of finding displacement u_i , strain ϵ_{ij} , and stress σ_{ij} at an arbitrary point $x(x_1, x_2, x_3)$ when a free body D is subjected to a given distribution of eigenstrain ϵ_{ij}^* . A free body is one which is free from any external surface or body force.

Hooke's law

For infinitesimal deformations considered in this book, the total strain ϵ_{ij} is regarded as the sum of elastic strain e_{ij} and eigenstrain ϵ_{ij}^* ,

$$\epsilon_{ij} = e_{ij} + \epsilon_{ij}^*. \quad (2.1)$$

The total strain must be compatible,

$$\epsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i}), \quad (2.2)$$

where $u_{i,j} = \partial u_i / \partial x_j$.

The elastic strain is related to stress σ_{ij} by Hooke's law;

$$\sigma_{ij} = C_{ijkl}e_{kl} = C_{ijkl}(\epsilon_{kl} - \epsilon_{kl}^*) \quad (2.3)$$

or

$$\sigma_{ij} = C_{ijkl}(u_{k,l} - \epsilon_{kl}^*), \quad (2.4)$$

where C_{ijkl} are the elastic moduli (constants) (see Appendix 2), and the summation convention for the repeated indices is employed (see Appendix 1). Since C_{ijkl} is symmetric ($C_{ijk} = C_{ijkl}$), we have $C_{ijkl}u_{l,k} = C_{ijkl}u_{k,l}$. In the domain where $\epsilon_{ij}^* = 0$, (2.4) becomes

$$\sigma_{ij} = C_{ijkl}\epsilon_{kl} = C_{ijkl}u_{k,l}. \quad (2.5)$$

The inverse expression of (2.3) is

$$\epsilon_{ij} - \epsilon_{ij}^* = C_{ijkl}^{-1}\sigma_{kl}, \quad (2.6)$$

where C_{ijkl}^{-1} is the elastic compliance.

For isotropic materials, (2.3) and (2.6) can be written as

$$\begin{aligned}\sigma_{ij} &= 2\mu(\epsilon_{ij} - \epsilon_{ij}^*) + \lambda\delta_{ij}(\epsilon_{kk} - \epsilon_{kk}^*), \\ \epsilon_{ij} - \epsilon_{ij}^* &= \{\sigma_{ij} - \delta_{ij}\sigma_{kk}\nu/(1+\nu)\}/2\mu,\end{aligned}\quad (2.7)$$

where λ and μ are the Lamé constants, and ν is Poisson's ratio. Young's modulus E , the shear modulus μ , and the bulk modulus K are connected by $2\mu = E/(1+\nu)$, $K = E/3(1-2\nu)$, and $\lambda = 2\mu\nu/(1-2\nu)$. The alternative expressions for (2.7) are

$$\begin{aligned}\sigma_x &= \frac{E}{1+\nu}\left\{(\epsilon_x - \epsilon_x^*) + \frac{\nu}{1-2\nu}(\epsilon_{kk} - \epsilon_{kk}^*)\right\}, \\ \sigma_y &= \frac{E}{1+\nu}\left\{(\epsilon_y - \epsilon_y^*) + \frac{\nu}{1-2\nu}(\epsilon_{kk} - \epsilon_{kk}^*)\right\}, \\ \sigma_z &= \frac{E}{1+\nu}\left\{(\epsilon_z - \epsilon_z^*) + \frac{\nu}{1-2\nu}(\epsilon_{kk} - \epsilon_{kk}^*)\right\}, \\ \sigma_{xy} &= \frac{E}{1+\nu}(\epsilon_{xy} - \epsilon_{xy}^*), \\ \sigma_{yz} &= \frac{E}{1+\nu}(\epsilon_{yz} - \epsilon_{yz}^*), \\ \sigma_{zx} &= \frac{E}{1+\nu}(\epsilon_{zx} - \epsilon_{zx}^*),\end{aligned}\quad (2.8)$$

and

$$\begin{aligned}\epsilon_x - \epsilon_x^* &= \{\sigma_x - \nu(\sigma_y + \sigma_z)\}/E, \\ \epsilon_y - \epsilon_y^* &= \{\sigma_y - \nu(\sigma_z + \sigma_x)\}/E, \\ \epsilon_z - \epsilon_z^* &= \{\sigma_z - \nu(\sigma_x + \sigma_y)\}/E, \\ \epsilon_{xy} - \epsilon_{xy}^* &= \frac{1+\nu}{E}\sigma_{xy}, \\ \epsilon_{yz} - \epsilon_{yz}^* &= \frac{1+\nu}{E}\sigma_{yz}, \\ \epsilon_{zx} - \epsilon_{zx}^* &= \frac{1+\nu}{E}\sigma_{zx},\end{aligned}\quad (2.9)$$

where $\epsilon_{kk} = \epsilon_x + \epsilon_y + \epsilon_z$ and $\epsilon_{kk}^* = \epsilon_x^* + \epsilon_y^* + \epsilon_z^*$. It is convenient to use (2.8) for the plane strain case where $\epsilon_z = 0$. Expression (2.9) is recommended for the plane stress case where $\sigma_z = \sigma_{zx} = \sigma_{zy} = 0$. It should be noted that solutions for the plane stress can be obtained directly from those for the plane strain by replacing $E/(1 - \nu^2)$ with E and $\nu/(1 - \nu)$ with ν .

When Hooke's law (2.8) is rewritten for the two-dimensional case, we have

$$\begin{aligned}\sigma_x &= \frac{\mu}{\kappa - 1} \{ (\kappa + 1)(\epsilon_x - \epsilon_x^*) + (3 - \kappa)(\epsilon_y - \epsilon_y^*) \}, \\ \sigma_y &= \frac{\mu}{\kappa - 1} \{ (\kappa + 1)(\epsilon_y - \epsilon_y^*) + (3 - \kappa)(\epsilon_x - \epsilon_x^*) \}, \\ \sigma_{xy} &= 2\mu(\epsilon_{xy} - \epsilon_{xy}^*), \\ \sigma_z &= \sigma_{zx} = \sigma_{zy} = 0,\end{aligned}\tag{2.9.1}$$

for the plane stress and $\kappa = (3 - \nu)/(1 + \nu)$. For the plane strain, we have

$$\begin{aligned}\sigma_x &= \frac{\mu}{\kappa - 1} \{ (\kappa + 1)(\epsilon_x - \epsilon_x^* - \nu\epsilon_z^*) + (3 - \kappa)(\epsilon_y - \epsilon_y^* - \nu\epsilon_z^*) \}, \\ \sigma_y &= \frac{\mu}{\kappa - 1} \{ (\kappa + 1)(\epsilon_y - \epsilon_y^* - \nu\epsilon_z^*) + (3 - \kappa)(\epsilon_x - \epsilon_x^* - \nu\epsilon_z^*) \}, \\ \sigma_{xy} &= 2\mu(\epsilon_{xy} - \epsilon_{xy}^*), \\ \sigma_z &= -\frac{\kappa + 1}{\kappa - 1}\mu\epsilon_z^* + \frac{3 - \kappa}{\kappa - 1}\mu(\epsilon_x + \epsilon_y - \epsilon_x^* - \epsilon_y^*), \\ \sigma_{zx} &= \sigma_{zy} = 0,\end{aligned}\tag{2.9.2}$$

where $\kappa = 3 - 4\nu$.

Equilibrium conditions

When eigenstresses are calculated, material domain D is assumed to be free from any external force and any surface constraint. If these conditions for the free body are not satisfied, the stress field can be constructed from the superposition of the eigenstress of the free body and the solution of a proper boundary value problem.

The equations of equilibrium are

$$\sigma_{ij,j} = 0 \quad (i = 1, 2, 3).\tag{2.10}$$

The boundary conditions for free external surface forces are

$$\sigma_{ij}n_j = 0, \quad (2.11)$$

where n_j is the exterior unit normal vector on the boundary of D .

By substituting (2.4) into (2.10) and (2.11), we have

$$C_{ijkl}u_{k,lj} = C_{ijkl}\epsilon_{kl,j}^* \quad (2.12)$$

and

$$C_{ijkl}u_{k,lj}n_j = C_{ijkl}\epsilon_{kl,j}^*n_j. \quad (2.13)$$

It can be seen that the contribution of ϵ_{ij}^* to the equations of equilibrium is similar to that of a body force since the equations of equilibrium under body force X_i with zero ϵ_{ij}^* are $C_{ijkl}u_{k,lj} = -X_i$. Similarly, $C_{ijkl}\epsilon_{kl,j}^*n_j$ behaves like a surface force on the boundary. Thus, it can be said that the elastic displacement field caused by ϵ_{ij}^* in a free body is equivalent to that caused by body force $-C_{ijkl}\epsilon_{kl,j}^*$ and surface force $C_{ijkl}\epsilon_{kl,j}^*n_j$.

In subsequent chapters, D in most cases is considered as an infinitely extended body (infinite body), and condition (2.11) is replaced by the condition $\sigma_{ij}(x) \rightarrow 0$ for $x \rightarrow \infty$.

Compatibility conditions

The strain tensor ϵ_{ij} has six components, while the displacement vector u_i has three components. The tensor and the vector are related to each other through the relation (2.2), which can be called the condition for the compatibility of strain ϵ_{ij} . Generally, however, the equations of compatibility are referred to the relations which are derived from (2.2) by eliminating u_i ,

$$\epsilon_{pki}\epsilon_{qlj}\epsilon_{ij,kl} = 0, \quad (2.14)$$

where ϵ_{pki} is the permutation tensor (see Appendix 1). Relation (2.14) will be discussed in Section 10.

The displacement differential equations of the elasticity theory are given by (2.12). In some cases, however, it is more convenient to consider (2.10), (2.3), and (2.14). Boundary conditions and various side conditions, such as singularity conditions, continuity conditions, etc., arise in problems from time to time. We can say at this point that the fundamental equations to be solved are equations (2.12).

Eigenstresses are caused by constraint from the surrounding elastic medium which prohibits the geometrically incompatible deformation of ϵ_{ij}^* . The incompatibility of ϵ_{ij}^* was discussed by Reissner (1931) and Neményi (1931). Dislocations due to incompatibility were studied by Weingarten (1901), Cesáro (1906), Volterra (1907), and Moriguti (1947) from the viewpoint of the elasticity theory in connection with the multiple values of displacements and rotations. Another viewpoint on dislocations, from the plasticity theory, was developed by Kondo (1955), Bilby (1960), and Kröner (1958).

In the following sections we investigate the methods of finding the associated elastic fields (displacements, strains, stresses) and the related problems for given distributions of ϵ_{ij}^* . Particular emphasis will be placed on the case when a uniform ϵ_{ij}^* is given in an ellipsoidal domain Ω in an infinitely extended medium D . The results are useful for the study of the mechanical properties of solids which may contain precipitates, inclusions, voids, cracks, etc. The most fundamental contribution to this study was made by Eshelby (1951, 1956, 1957, 1959 and 1961).

3. General expressions of elastic fields for given eigenstrain distributions

The case where a given material is infinitely extended is of particular interest for the mathematical simplicity of the solution as well as for its practical importance. When the solution is applied to inclusion problems, it can be assumed with sufficient accuracy that the materials are infinitely extended since the size of the inclusions is relatively small compared to the size of the macroscopic material samples.

The fundamental equations to be solved for given ϵ_{ij}^* , (2.12), are

$$C_{ijkl} u_{k,lj} = C_{ijkl} \epsilon_{kl,j}^*. \quad (3.1)$$

Periodic solutions

Suppose $\epsilon_{ij}^*(x)$ is given in the form of a single wave of amplitude $\bar{\epsilon}_{ij}^*(\xi)$, where ξ is the wave vector corresponding to the given period of the distribution,

$$\epsilon_{ij}^*(x) = \bar{\epsilon}_{ij}^*(\xi) \exp(i\xi \cdot x), \quad (3.2)$$

where $i = \sqrt{-1}$ and $\xi \cdot x = \xi_k x_k$.

The solution of (3.1) corresponding to this distribution may also be expressed in the form of a single wave of the same period, that is,

$$u_i(x) = \bar{u}_i(\xi) \exp(i\xi \cdot x). \quad (3.3)$$

Substituting (3.2) and (3.3) into (3.1), we have

$$C_{ijkl}\bar{u}_k\xi_l\xi_j = -iC_{ijkl}\bar{\epsilon}_{kl}^*\xi_j \quad (3.4)$$

where in the derivation $(i\xi \cdot x)_{,l} = i\xi_l$ is used. Expression (3.4) stands for three equations ($i = 1, 2, 3$) for determining the three unknowns \bar{u}_i for given $\bar{\epsilon}_{ij}^*$.

Using the notation

$$\begin{aligned} K_{ik}(\xi) &= C_{ijkl}\xi_j\xi_l, \\ X_i &= -iC_{ijkl}\bar{\epsilon}_{kl}^*\xi_j, \end{aligned} \quad (3.5)$$

we can write (3.4) as

$$\begin{aligned} K_{11}\bar{u}_1 + K_{12}\bar{u}_2 + K_{13}\bar{u}_3 &= X_1, \\ K_{21}\bar{u}_1 + K_{22}\bar{u}_2 + K_{23}\bar{u}_3 &= X_2, \\ K_{31}\bar{u}_1 + K_{32}\bar{u}_2 + K_{33}\bar{u}_3 &= X_3. \end{aligned} \quad (3.6)$$

Then, \bar{u}_i is obtained as

$$\bar{u}_i(\xi) = X_j N_{ij}(\xi)/D(\xi), \quad (3.7)$$

where N_{ij} are cofactors of the matrix

$$K(\xi) = \begin{pmatrix} K_{11} & K_{12} & K_{13} \\ K_{21} & K_{22} & K_{23} \\ K_{31} & K_{32} & K_{33} \end{pmatrix} \quad (3.8)$$

and $D(\xi)$ is the determinant of $K(\xi)$. Note that $K_{ki} = C_{kjl}\xi_j\xi_l = C_{kli}\xi_l\xi_j = C_{ijkl}\xi_l\xi_j = K_{ik}$ due to the symmetry of the elastic constants $C_{ijkl} = C_{klij}$, and that $N_{ij} = N_{ji}$. The explicit expressions for $D(\xi)$ and $N_{ij}(\xi)$ are

$$\begin{aligned} D(\xi) &= \epsilon_{mn} K_{m1} K_{n2} K_{l3}, \\ N_{ij}(\xi) &= \frac{1}{2} \epsilon_{ikl} \epsilon_{jmn} K_{km} K_{ln} \\ &= K_{im} K_{mj} - K_{mm} K_{ij} \\ &\quad + (\epsilon_{mn1} K_{m2} K_{n3} + \epsilon_{mn2} K_{m3} K_{n1} + \epsilon_{mn3} K_{m1} K_{n2}) \delta_{ij}, \end{aligned} \quad (3.9)$$

where ϵ_{ijk} is the permutation tensor.

Substituting (3.7) into (3.3), we have

$$u_i(x) = -iC_{jlmn}\bar{\epsilon}_{mn}^*(\xi)\xi_lN_{ij}(\xi)D^{-1}(\xi)\exp(i\xi \cdot x). \quad (3.10)$$

The corresponding strain and stress are obtained from (2.2) and (2.4) as

$$\epsilon_{ij}(x) = \frac{1}{2}C_{klmn}\bar{\epsilon}_{mn}^*(\xi)\xi_l\{\xi_jN_{ik}(\xi) + \xi_iN_{jk}(\xi)\}D^{-1}(\xi)\exp(i\xi \cdot x) \quad (3.11)$$

and

$$\sigma_{ij}(x) = C_{ijkl}\left\{C_{pqmn}\bar{\epsilon}_{mn}^*(\xi)\xi_q\xi_lN_{kp}(\xi)D^{-1}(\xi)\exp(i\xi \cdot x) - \epsilon_{kl}^*(x)\right\}, \quad (3.12)$$

where $D^{-1} = 1/D$. The above result was used by Mura (1964) for periodic distributions of dislocations and by Khachaturyan (1967) for a coherent inclusion of a new phase.

Method of Fourier series and Fourier integrals

The linear theory of elasticity allows for the superposition of solutions. If $\epsilon_{ij}^*(x)$ is given in the Fourier series form,

$$\epsilon_{ij}^*(x) = \sum \bar{\epsilon}_{ij}^*(\xi) \exp(i\xi \cdot x), \quad (3.13)$$

the corresponding displacement, strain and stress are then obtained as superpositions of the solutions for single waves of the form (3.2), namely,

$$\begin{aligned} u_i(x) &= -i \sum C_{jlmn}\bar{\epsilon}_{mn}^*(\xi)\xi_lN_{ij}(\xi)D^{-1}(\xi)\exp(i\xi \cdot x), \\ \epsilon_{ij}(x) &= \frac{1}{2} \sum C_{klmn}\bar{\epsilon}_{mn}^*(\xi)\xi_l\{\xi_jN_{ik}(\xi) + \xi_iN_{jk}(\xi)\}D^{-1}(\xi)\exp(i\xi \cdot x), \\ \sigma_{ij}(x) &= C_{ijkl}\left\{\sum C_{pqmn}\bar{\epsilon}_{mn}^*(\xi)\xi_q\xi_lN_{kp}(\xi)D^{-1}(\xi)\exp(i\xi \cdot x) - \epsilon_{kl}^*(x)\right\}, \end{aligned} \quad (3.14)$$

$$\sigma_{ij}(x) = C_{ijkl}\left\{\sum C_{pqmn}\bar{\epsilon}_{mn}^*(\xi)\xi_q\xi_lN_{kp}(\xi)D^{-1}(\xi)\exp(i\xi \cdot x) - \epsilon_{kl}^*(x)\right\},$$

where the summations in (3.13) and (3.14) are taken with respect to ξ . Similarly, if ϵ_{ij}^* is given by the Fourier integral form (see Appendix 3),

$$\epsilon_{ij}^*(x) = \int_{-\infty}^{\infty} \bar{\epsilon}_{ij}^*(\xi) \exp(i\xi \cdot x) d\xi, \quad (3.15)$$

where

$$\bar{\epsilon}_{ij}^*(\xi) = (2\pi)^{-3} \int_{-\infty}^{\infty} \epsilon_{ij}^*(x) \exp(-i\xi \cdot x) dx, \quad (3.16)$$

we have

$$\begin{aligned} u_i(x) &= -i \int_{-\infty}^{\infty} C_{jlmn} \bar{\epsilon}_{mn}^*(\xi) \xi_l N_{ij}(\xi) D^{-1}(\xi) \exp(i\xi \cdot x) d\xi, \\ \epsilon_{ij}(x) &= \frac{1}{2} \int_{-\infty}^{\infty} C_{klmn} \bar{\epsilon}_{mn}^*(\xi) \xi_l \{ \xi_j N_{ik}(\xi) + \xi_i N_{jk}(\xi) \} D^{-1}(\xi) \\ &\quad \times \exp(i\xi \cdot x) d\xi, \end{aligned} \quad (3.17)$$

$$\begin{aligned} \sigma_{ij}(x) &= C_{ijkl} \left\{ \int_{-\infty}^{\infty} C_{pqmn} \bar{\epsilon}_{mn}^*(\xi) \xi_q \xi_l N_{kp}(\xi) D^{-1}(\xi) \right. \\ &\quad \left. \times \exp(i\xi \cdot x) d\xi - \epsilon_{kl}^*(x) \right\}, \end{aligned}$$

where

$$\begin{aligned} \int_{-\infty}^{\infty} d\xi &= \int_{-\infty}^{\infty} d\xi_1 \int_{-\infty}^{\infty} d\xi_2 \int_{-\infty}^{\infty} d\xi_3, \\ \int_{-\infty}^{\infty} dx &= \int_{-\infty}^{\infty} dx_1 \int_{-\infty}^{\infty} dx_2 \int_{-\infty}^{\infty} dx_3. \end{aligned} \quad (3.18)$$

When (3.16) is substituted into (3.17), we have

$$\begin{aligned} u_i(x) &= -i(2\pi)^{-3} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} C_{jlmn} \epsilon_{mn}^*(x') \xi_l N_{ij}(\xi) D^{-1}(\xi) \\ &\quad \times \exp\{i\xi \cdot (x - x')\} d\xi dx' \\ &= -(2\pi)^{-3} \frac{\partial}{\partial x_i} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} C_{jlmn} \epsilon_{mn}^*(x') N_{ij}(\xi) D^{-1}(\xi) \\ &\quad \times \exp\{i\xi \cdot (x - x')\} d\xi dx', \end{aligned} \quad (3.19)$$

$$\begin{aligned} \epsilon_{ij}(x) = & (2\pi)^{-3} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{1}{2} C_{klmn} \epsilon_{mn}^*(x') \\ & \times \xi_l \{ \xi_j N_{ik}(\xi) + \xi_i N_{jk}(\xi) \} D^{-1}(\xi) \\ & \times \exp\{i\xi \cdot (x - x')\} d\xi dx', \end{aligned} \quad (3.20)$$

$$\begin{aligned} \sigma_{ij}(x) = & C_{ijkl} \left\{ (2\pi)^{-3} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} C_{pqmn} \epsilon_{mn}^*(x') \xi_q \xi_l N_{kp}(\xi) D^{-1}(\xi) \right. \\ & \left. \times \exp\{i\xi \cdot (x - x')\} d\xi dx' - \epsilon_{kl}^*(x) \right\}. \end{aligned} \quad (3.21)$$

Method of Green's functions

When Green's functions $G_{ij}(x - x')$ are defined as

$$G_{ij}(x - x') = (2\pi)^{-3} \int_{-\infty}^{\infty} N_{ij}(\xi) D^{-1}(\xi) \exp\{i\xi \cdot (x - x')\} d\xi, \quad (3.22)$$

(3.19) can be written as

$$u_i(x) = - \int_{-\infty}^{\infty} C_{jlmn} \epsilon_{mn}^*(x') G_{ij,l}(x - x') dx', \quad (3.23)$$

where $G_{ij,l}(x - x') = \partial/\partial x_l G_{ij}(x - x') = -\partial/\partial x'_l G_{ij}(x - x')$. Sometimes Green's functions are called the fundamental solutions.

The corresponding expressions for the strain and stress become

$$\epsilon_{ij}(x) = -\frac{1}{2} \int_{-\infty}^{\infty} C_{klmn} \epsilon_{mn}^*(x') \{ G_{ik,lj}(x - x') + G_{jk,li}(x - x') \} dx' \quad (3.24)$$

and

$$\sigma_{ij}(x) = -C_{ijkl} \left\{ \int_{-\infty}^{\infty} C_{pqmn} \epsilon_{mn}^*(x') G_{kp,qj}(x - x') dx' + \epsilon_{kl}^*(x) \right\}. \quad (3.25)$$

Mura (1963) rewrote (3.25) in the form

$$\sigma_{ij}(x) = C_{ijkl} \int_{-\infty}^{\infty} \epsilon_{sth} \epsilon_{lnh} C_{pqmn} G_{kp,ql}(x - x') \epsilon_{sm}^*(x') dx', \quad (3.26)$$

which will be useful for the dislocation theory given in later sections. It is easy to prove that (3.26) is equivalent to (3.25). Since $\epsilon_{sth}\epsilon_{lnh} = \delta_{sl}\delta_{tn} - \delta_{sn}\delta_{tl}$, (3.26) becomes

$$\sigma_{ij}(x) = C_{ijkl} \int_{-\infty}^{\infty} C_{pqmn} (G_{kp,qn}\epsilon_{ml}^* - G_{kp,ql}\epsilon_{mn}^*) dx'. \quad (3.27)$$

In Section 5 it is shown that

$$C_{mnpq}G_{pk,qn}(x - x') = -\delta_{mk}\delta(x - x'), \quad (3.28)$$

where $\delta(x - x')$ is Dirac's delta function having the property

$$\int_{-\infty}^{\infty} \epsilon_{ml}^*(x')\delta(x - x') dx' = \epsilon_{ml}^*(x); \quad (3.29)$$

therefore, (3.25) follows from (3.27).

It is seen from (3.28) that Green's function $G_{pk}(x - x')$ is the displacement component in the x_p -direction at point x when a unit body force in the x_k -direction is applied at point x' in the infinitely extended material. By this definition of Green's function we can directly derive (3.23) from (3.1). As was mentioned in Section 2, the displacement u_i in (3.1) can be considered as a displacement caused by the body force $-C_{ilmn}\epsilon_{mn,l}^*$ applied in the x_i -direction. Since $G_{ij}(x - x')$ is the solution for a unit body force applied in the x_j -direction, the solution for the present problem is the product of G_{ij} and the body force $-C_{jlmn}\epsilon_{mn,l}^*$, namely,

$$u_i(x) = - \int_{-\infty}^{\infty} G_{ij}(x - x') C_{jlmn}\epsilon_{mn,l}^*(x') dx'. \quad (3.30)$$

Integrating by parts and assuming that the boundary terms vanish, we have

$$u_i(x) = \int_{-\infty}^{\infty} C_{jlmn}\epsilon_{mn}^*(x') \frac{\partial}{\partial x'_l} G_{ij}(x - x') dx'. \quad (3.31)$$

For an infinite body it holds that $(\partial/\partial x'_l)G_{ij}(x - x') = -(\partial/\partial x_l)G_{ij}(x - x')$; (3.23) is thereby obtained.

Expression (3.31) or (3.23) is preferable to expression (3.30). When ϵ_{mn}^* is constant in Ω and is zero in $D-\Omega$, it can be seen that the integrand in (3.30) vanishes except on the boundary of Ω .

As will be seen in (5.9), $G_{ij}(x - x')$ has a singularity at $x = x'$ with the order of $|x - x'|^{-1}$. Thus, the integrals in (3.24) and (3.25) do not exist in the

sense of Riemann integrals. This difficulty can be avoided by writing (3.25) in the form

$$\sigma_{ij}(x) = -C_{ijkl} \left\{ \frac{\partial}{\partial x_l} \int_{-\infty}^{\infty} C_{pqmn} \epsilon_{mn}^*(x') G_{kp,q}(x-x') dx' + \epsilon_{kl}^*(x) \right\}. \quad (3.32)$$

Expressions (3.25) or (3.17) are permissible in the context of generalized functions (Lighthill 1964).

Expressions (3.23), (3.25) and their equivalents were developed by Fredholm (1900). In connection with the solution for dislocations, many papers have discussed these expressions more extensively: Weingarten (1901), Volterra (1907), Somigliana (1914), Burgers (1939), Leibfried (1953), Eshelby (1961), Kröner (1958), Steketee (1958), de Wit (1960), Indenbom (1966), Kunin (1964), Kosevich (1965), Bacon, Barnett, and Scattergood (1978), Hirth and Lothe (1982), Teodosiu (1982), and Steeds and Willis (1979), among others.

As will be seen in Section 5, Green's functions have been obtained explicitly only for isotropic and transversely isotropic materials. Therefore, for practical calculations the usage of Green's functions as seen in (3.23) ~ (3.25) is limited, and the use of Fourier integral expressions (3.17) is much more convenient. For this reason the integrands appearing in (3.17) are written down in detail.

Isotropic materials

$$D(\xi) = \mu^2(\lambda + 2\mu)\xi^6, \quad (3.33)$$

$$N_{ij}(\xi) = \mu\xi^2 \left\{ (\lambda + 2\mu)\delta_{ij}\xi^2 - (\lambda + \mu)\xi_i\xi_j \right\},$$

where $\xi^2 = \xi_k\xi_k$,

$$C_{jlmn}\xi_l N_{ij}(\xi) D^{-1}(\xi) = (\lambda + 2\mu)^{-1}\xi^{-4} \left\{ \lambda\delta_{mn}\xi_i\xi^2 + (\lambda + 2\mu)\delta_{im}\xi_n\xi^2 \right. \\ \left. + (\lambda + 2\mu)\delta_{in}\xi_m\xi^2 - 2(\lambda + \mu)\xi_i\xi_m\xi_n \right\}, \quad (3.34)$$

$$C_{ijkl}C_{pqmn}\xi_q\xi_l N_{kp}(\xi) D^{-1}(\xi) \\ = (\lambda + 2\mu)^{-1}\xi^{-4} \left\{ \lambda^2\delta_{ij}\delta_{mn}\xi^4 + 2\lambda\mu\delta_{mn}\xi_i\xi_j\xi^2 + 2\lambda\mu\delta_{ij}\xi_m\xi_n\xi^2 \right. \\ \left. + \mu(\lambda + 2\mu)(\delta_{im}\xi_j\xi_n + \delta_{jm}\xi_i\xi_n + \delta_{in}\xi_j\xi_m + \delta_{jn}\xi_i\xi_m)\xi^2 \right. \\ \left. - 4\mu(\lambda + \mu)\xi_i\xi_j\xi_m\xi_n \right\}.$$

Cubic crystals

$$D(\xi) = \mu^2(\lambda + 2\mu + \mu')\xi^6 + \mu\mu'(2\lambda + 2\mu + \mu')\xi^2(\xi_1^2\xi_2^2 + \xi_2^2\xi_3^2 + \xi_3^2\xi_1^2) + \mu'^2(3\lambda + 3\mu + \mu')\xi_1^2\xi_2^2\xi_3^2, \quad (3.35)$$

$$\begin{aligned} N_{11}(\xi) &= \mu^2\xi^4 + \beta\xi^2(\xi_2^2 + \xi_3^2) + \gamma\xi_2^2\xi_3^2, \\ N_{12}(\xi) &= -(\lambda + \mu)\xi_1\xi_2(\mu\xi^2 + \mu'\xi_3^2), \end{aligned} \quad (3.36)$$

and the other components are obtained by the cyclic permutation of 1, 2, 3, where

$$\begin{aligned} \xi^2 &= \xi_i\xi_i, \\ \beta &= \mu(\lambda + \mu + \mu'), \\ \gamma &= \mu'(2\lambda + 2\mu + \mu'), \\ \lambda &= C_{12}, \\ \mu &= C_{44}, \\ \mu' &= C_{11} - C_{12} - 2C_{44}. \end{aligned} \quad (3.37)$$

Hexagonal crystals (transversely isotropic)

$$\begin{aligned} D(\xi) &= (\alpha'\eta^2 + \gamma\xi_3^2)\{\alpha\gamma\eta^4 + (\alpha\beta + \gamma^2 - \gamma'^2)\eta^2\xi_3^2 + \beta\gamma\xi_3^4\} \\ &= (\alpha'\eta^2 + \gamma\xi_3^2)\{(\gamma\eta^2 + \beta\xi_3^2)(\alpha\eta^2 + \gamma\xi_3^2) - \gamma'^2\eta^2\xi_3^2\}, \end{aligned} \quad (3.38)$$

$$\begin{aligned} N_{11}(\xi) &= (\alpha'\xi_1^2 + \alpha\xi_2^2 + \gamma\xi_3^2)(\gamma\eta^2 + \beta\xi_3^2) - \gamma'^2\xi_2^2\xi_3^2, \\ N_{12}(\xi) &= \gamma'^2\xi_1\xi_2\xi_3^2 - (\alpha - \alpha')\xi_1\xi_2(\gamma\eta^2 + \beta\xi_3^2), \\ N_{13}(\xi) &= (\alpha - \alpha')\gamma'\xi_1\xi_2\xi_3 - \gamma'\xi_1\xi_3(\alpha'\xi_1^2 + \alpha\xi_2^2 + \gamma\xi_3^2), \\ N_{22}(\xi) &= (\alpha\xi_1^2 + \alpha'\xi_2^2 + \gamma\xi_3^2)(\gamma\eta^2 + \beta\xi_3^2) - \gamma'^2\xi_1^2\xi_3^2, \\ N_{23}(\xi) &= (\alpha - \alpha')\gamma'\xi_1^2\xi_2\xi_3 - \gamma'\xi_2\xi_3(\alpha\xi_1^2 + \alpha'\xi_2^2 + \gamma\xi_3^2), \\ N_{33}(\xi) &= (\alpha\xi_1^2 + \alpha'\xi_2^2 + \gamma\xi_3^2)(\alpha'\xi_1^2 + \alpha\xi_2^2 + \gamma\xi_3^2) - (\alpha - \alpha')^2\xi_1^2\xi_2^2, \end{aligned} \quad (3.39)$$

where

$$\begin{aligned}\alpha &= C_{11} = C_{22}, \quad \alpha' = C_{66} = \frac{1}{2}(C_{11} - C_{12}), \\ \beta &= C_{33}, \quad \gamma' - \gamma = C_{13} = C_{23}, \\ \gamma &= C_{44} = C_{55}, \quad \eta^2 = \xi_1^2 + \xi_2^2.\end{aligned}\tag{3.40}$$

For isotropic materials, $\alpha = \beta = \lambda + 2\mu$, $\gamma' = \lambda + \mu$, and $\gamma = \alpha' = \mu$.

4. Exercises of general formulae

General formulae are given in Section 3 for the elastic fields associated with prescribed eigenstrains. This section provides exercises in the usage of these formulae: the best way to understand general statements is to work out specific examples.

A straight screw dislocation

Let us consider the case where Ω in Fig. 1.1 is the half plane ($x_2 = 0$, $x_1 < 0$) (see Fig. 4.1), and ϵ_{23}^* is prescribed on Ω . Other components of ϵ_{ij}^* are zero.

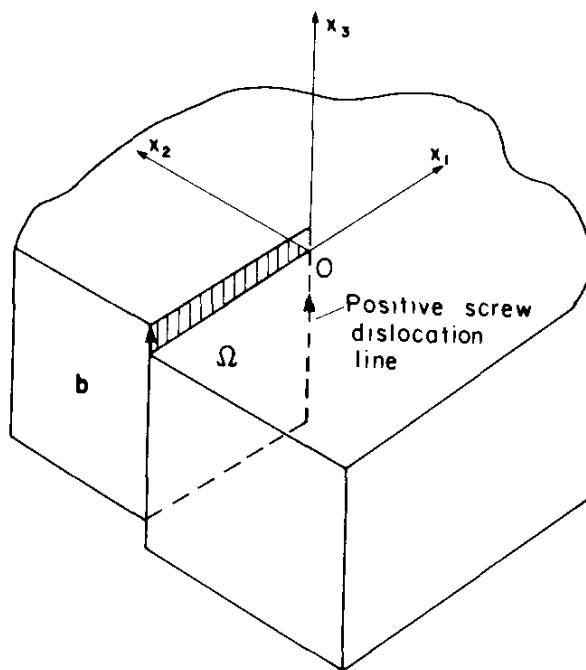


Fig. 4.1 A screw dislocation

The boundary of Ω (the x_3 -axis) is called a straight positive screw dislocation when $\epsilon_{23}^* = \epsilon_{32}^*$ is given by

$$\epsilon_{23}^*(x) = \frac{1}{2}b\delta(x_2)H(-x_1), \quad (4.1)$$

where $H(-x_1)$ is the Heaviside step function (see Appendix 3),

$$H(-x_1) = \begin{cases} 1 & x_1 < 0 \\ 0 & x_1 > 0, \end{cases} \quad (4.2)$$

and $\delta(x_2)$ is Dirac's delta function. The plastic strain (eigenstrain) (4.1) is caused by a relative slip \mathbf{b} on the half plane ($x_2 = 0, x_1 < 0$) in the x_3 -direction (Fig. 4.1).

The Fourier transform of (4.1) is, from (3.16),

$$\begin{aligned} \bar{\epsilon}_{23}^*(\xi) &= (2\pi)^{-3} \iiint_{-\infty}^{\infty} \frac{1}{2}b\delta(x_2)H(-x_1) \\ &\quad \times \exp\{-i(\xi_1x_1 + \xi_2x_2 + \xi_3x_3)\} dx_1 dx_2 dx_3. \end{aligned} \quad (4.3)$$

By using

$$\begin{aligned} \int_{-\infty}^{\infty} H(-x_1) \exp(-i\xi_1 x_1) dx_1 &= -1/i\xi_1, \\ \int_{-\infty}^{\infty} \exp(-i\xi_3 x_3) dx_3 &= 2\pi\delta(\xi_3), \end{aligned} \quad (4.4)$$

we have

$$\bar{\epsilon}_{23}^*(\xi) = -\frac{\frac{1}{2}b}{(2\pi)^2} \frac{\delta(\xi_3)}{i\xi_1}. \quad (4.5)$$

Formulae (3.17) are used as follows: assuming, for simplicity, that the material is isotropic, (3.34) is used in (3.17); then, we have

$$\begin{aligned} u_3(x) &= \int_{-\infty}^{\infty} \frac{\xi_2 N_{33} + \xi_3 N_{23}}{(\lambda + 2\mu)\xi^6} \frac{b}{(2\pi)^2} \frac{\delta(\xi_3)}{\xi_1} \exp(i\xi \cdot x) d\xi \\ &= \frac{b}{(2\pi)^2} \iint_{-\infty}^{\infty} \frac{\xi_2}{\xi_1(\xi_1^2 + \xi_2^2)} \exp\{i(\xi_1 x_1 + \xi_2 x_2)\} d\xi_1 d\xi_2. \end{aligned} \quad (4.6)$$

The following integrals are useful for further calculations; these are obtained from formulae 444, 633, 415, 632, and 638·1 in the Table of Fourier Integrals by Campbell and Foster (1948):

$$\begin{aligned}
 & \iint_{-\infty}^{\infty} \frac{\exp\{i(\xi_1 x_1 + \xi_2 x_2)\}}{\xi_1^2 + \xi_2^2} d\xi_1 d\xi_2 = -\pi \log(x_1^2 + x_2^2), \\
 & \iint_{-\infty}^{\infty} \frac{\xi_2 \exp\{i(\xi_1 x_1 + \xi_2 x_2)\}}{\xi_1 (\xi_1^2 + \xi_2^2)} d\xi_1 d\xi_2 = 2\pi \tan^{-1}(x_2/x_1), \\
 & \iint_{-\infty}^{\infty} \frac{\xi_1 \xi_2 \exp\{i(\xi_1 x_1 + \xi_2 x_2)\}}{(\xi_1^2 + \xi_2^2)^2} d\xi_1 d\xi_2 = \frac{-\pi x_1 x_2}{x_1^2 + x_2^2}, \\
 & \iint_{-\infty}^{\infty} \frac{\xi_2^2 \exp\{i(\xi_1 x_1 + \xi_2 x_2)\}}{(\xi_1^2 + \xi_2^2)^2} d\xi_1 d\xi_2 = -\frac{1}{2}\pi \log(x_1^2 + x_2^2) - \frac{\pi x_2^2}{x_1^2 + x_2^2}.
 \end{aligned} \tag{4.7}$$

Then, (4.6) becomes

$$u_3(x) = (b/2\pi) \tan^{-1}(x_2/x_1) \tag{4.8}$$

which has been obtained by Burgers (1939). Other components of u_i are zero. Equation (4.8) can also be obtained from (3.23), where G_{ij} for isotropic materials is given in Section 5.

When (4.1) is substituted into (3.23), one arrives at

$$\begin{aligned}
 u_3(x) &= -\iiint_{-\infty}^{\infty} C_{3232} b \delta(x'_2) H(-x'_1) \\
 &\quad \times \{G_{33,2}(x - x') + G_{32,3}(x - x')\} dx'_1 dx'_2 dx'_3 \\
 &= \frac{b}{4\pi} \int_{-\infty}^{\infty} dx'_1 \int_{-\infty}^{\infty} dx'_3 \left(\frac{u}{\lambda + 2\mu} \frac{x_2}{\bar{x}^3} + 3 \frac{\lambda + \mu}{\lambda + 2\mu} \frac{x_2 (x'_3)^2}{\bar{x}^5} \right),
 \end{aligned} \tag{4.9}$$

where

$$\bar{x} = [(x_1 - x'_1)^2 + x_2^2 + (x'_3)^2]^{1/2}.$$

Now, with the aid of integrals

$$\begin{aligned} \int_{-\infty}^{\infty} \frac{dx'_3}{\bar{x}^3} &= \frac{2}{(x_1 - x'_1)^2 + x_2^2}, \\ \int_{-\infty}^{\infty} \frac{(x'_3)^2 dx'_3}{\bar{x}^5} &= \frac{2/3}{(x_1 - x'_1)^2 + x_2^2}, \end{aligned} \quad (4.10)$$

(4.9) is reduced to (4.8). The stress field can be obtained from the displacement field by the use of (2.4).

A straight edge dislocation

Let Ω be the half plane ($x_2 = 0, x_1 < 0$), and ϵ_{21}^* be prescribed by

$$\epsilon_{21}^*(x) = \frac{1}{2}b\delta(x_2)H(-x_1). \quad (4.11)$$

The x_3 -axis is then a straight positive edge dislocation (see Fig. 4.2). The plastic strain ϵ_{21}^* is caused by the relative slip \mathbf{b} on the half plane ($x_2 = 0, x_1 < 0$) in the x_1 -direction.

Substitution of (4.11) into (3.16) leads to

$$\bar{\epsilon}_{21}^* = -(2\pi)^{-2}\frac{1}{2}b\delta(\xi_3)/i\xi_1. \quad (4.12)$$

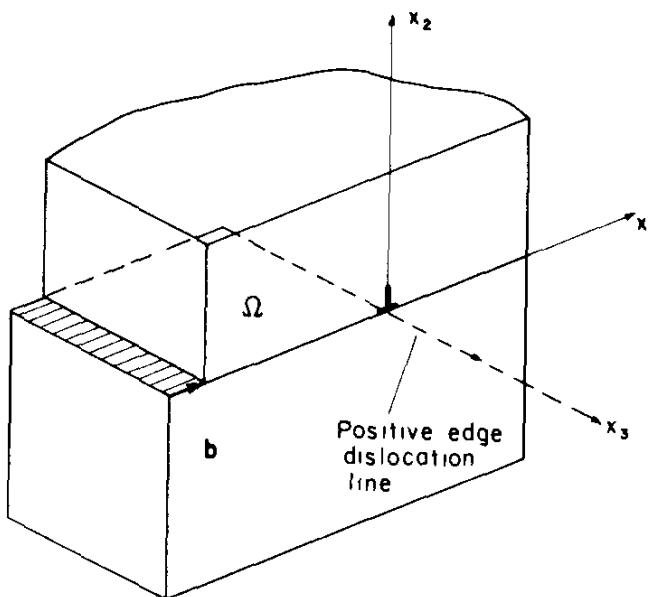


Fig. 4.2. An edge dislocation

For isotropic materials, we have, from (3.17) and (3.34),

$$\begin{aligned} u_1(x) &= (2\pi)^{-2} b \iint_{-\infty}^{\infty} \left\{ \frac{\xi_2}{\xi_1(\xi_1^2 + \xi_2^2)} - \frac{1}{1-\nu} \frac{\xi_1 \xi_2}{(\xi_1^2 + \xi_2^2)^2} \right\} \\ &\quad \times \exp\{i(\xi_1 x_1 + \xi_2 x_2)\} d\xi_1 d\xi_2 \\ &= (b/2\pi) \tan^{-1}(x_2/x_1) + (b/4\pi) \frac{1}{1-\nu} \frac{x_1 x_2}{x_1^2 + x_2^2}. \end{aligned} \quad (4.13)$$

This result was obtained by Koehler (1941), correcting Taylor's result (1934). Similarly, other components become

$$\begin{aligned} u_2(x) &= (2\pi)^{-2} b \iint_{-\infty}^{\infty} \left\{ \frac{1}{\xi_1^2 + \xi_2^2} - \frac{1}{1-\nu} \frac{\xi_2^2}{(\xi_1^2 + \xi_2^2)^2} \right\} \\ &\quad \times \exp\{i(\xi_1 x_1 + \xi_2 x_2)\} d\xi_1 d\xi_2 \\ &= \frac{b(2\nu - 1)}{8\pi(1-\nu)} \log(x_1^2 + x_2^2) + \frac{b}{4\pi(1-\nu)} \frac{x_2^2}{x_1^2 + x_2^2}, \end{aligned} \quad (4.14)$$

$$u_3(x) = 0.$$

Note that $u_2(x)$ in (4.14) differs by a constant from that in most text books on dislocations (e.g. Read 1953, p. 116), because differences in constant values of displacement components are admissible in the elasticity theory.

The same result can be obtained by the use of Green's functions for isotropic materials. From (3.23) and (4.11) we have

$$u_i(x) = - \int_{-\infty}^0 dx'_1 \int_{-\infty}^{\infty} C_{jl21} b G_{ij,l}(x - x') dx'_3. \quad (4.15)$$

The above expression becomes, from (5.10),

$$u_i(x) = \frac{b}{8\pi(1-\nu)} \int_{-\infty}^0 dx'_1 \int_{-\infty}^{\infty} \left\{ (1-2\nu) \frac{\delta_{2i}\bar{x}_1 + \delta_{1i}\bar{x}_2}{\bar{x}^3} + 3 \frac{\bar{x}_2\bar{x}_1\bar{x}_i}{\bar{x}^5} \right\} dx'_3, \quad (4.16)$$

where $\bar{x}_1 = x_1 - x'_1$, $\bar{x}_2 = x_2$, $\bar{x}_3 = -x'_3$. By using (4.10) the integrations in (4.16) can be done easily, leading to (4.13) and (4.14).

Periodic distribution of cuboidal precipitates

Consider cuboidal regions of inelastic strain (eigenstrain) due to solute segregation forming cuboidal precipitates. Let the edge length of each be $2a$ (with edges along x_1 , x_2 , and x_3), and let these regions be spaced $2L$ apart in all three directions. Within each region the eigenstrain will be assumed to have a constant value, ϵ , and be zero outside this region,

$$\epsilon_{ij}^*(x) = \begin{cases} \delta_{ij}\epsilon & \text{in cuboidal regions} \\ 0 & \text{outside.} \end{cases} \quad (4.17)$$

According to Appendix 3, the Fourier series expression for (4.17) is

$$\epsilon_{ij}^*(x) = \delta_{ij}\epsilon \sum_{p,q,r=-\infty}^{\infty} a_{pqr} \exp\left\{i\frac{\pi}{L}(px_1 + qx_2 + rx_3)\right\}, \quad (4.18)$$

where p , q , and r are integers and

$$a_{pqr} = (\pi)^{-3} (pqr)^{-1} \sin\left(\frac{p\pi a}{L}\right) \sin\left(\frac{q\pi a}{L}\right) \sin\left(\frac{r\pi a}{L}\right). \quad (4.19)$$

Expressions in (3.14) are used for the elastic field, where

$$\begin{aligned} \bar{\epsilon}_{mn}^*(p, q, r) &= \delta_{mn}\epsilon a_{pqr}, \\ \xi_1 &= \pi p/L, \quad \xi_2 = \pi q/L, \quad \xi_3 = \pi r/L, \end{aligned} \quad (4.20)$$

and the summations are taken with respect to p , q , and r . Namely,

$$\begin{aligned} u_i(x) &= -i \sum_{p,q,r=-\infty}^{\infty} C_{jlmn} \delta_{mn} \epsilon a_{pqr} \xi_l N_{ij}(\xi) D^{-1}(\xi) \\ &\times \exp\left\{i\frac{\pi}{L}(px_1 + qx_2 + rx_3)\right\}. \end{aligned} \quad (4.21)$$

For isotropic materials we have

$$\begin{aligned} u_1(x) &= 8 \frac{3\lambda + 2\mu}{\lambda + 2\mu} \frac{L\epsilon}{\pi^4} \sum_{p,q,r=0}^{\infty} \frac{\sin\left(\frac{p\pi a}{L}\right) \sin\left(\frac{q\pi a}{L}\right) \sin\left(\frac{r\pi a}{L}\right)}{(p^2 + q^2 + r^2) qr} \\ &\times \sin\left(\frac{p\pi x_1}{L}\right) \cos\left(\frac{q\pi x_2}{L}\right) \cos\left(\frac{r\pi x_3}{L}\right). \end{aligned} \quad (4.22)$$

The other components of the displacement can be obtained by the cyclic permutation of p, q, r and x_1, x_2, x_3 . The summations in (4.18) and (4.22) exclude $p = q = r = 0$ in order to have the convergency of series. Since the term corresponding to $p = q = r = 0$ in (4.18) represents a uniform eigenstrain in the whole space, the term in the displacement field (4.22) corresponding to the uniform eigenstrain can be ignored. Sass, Mura and Cohen (1967) calculated the diffraction contrast of the cuboidal inclusions by using (4.22) and two-beam dynamical equations. Their electron microscope observation of an Ni-Ti alloy provides good support of the theory.

5. Static Green's functions

In Section 3 Green's functions $G_{ij}(x - x')$ have been formally defined by (3.22),

$$G_{ij}(x - x') = (2\pi)^{-3} \int_{-\infty}^{\infty} N_{ij}(\xi) D^{-1}(\xi) \exp\{i\xi \cdot (x - x')\} d\xi. \quad (5.1)$$

The elastic field quantities for a given $\epsilon_i^*(x)$ are determined by (3.23) ~ (3.25) if $G_{ij}(x - x')$ are known.

It can easily be shown that G_{ij} satisfy

$$C_{ijkl} G_{km,lj}(x - x') + \delta_{im} \delta(x - x') = 0, \quad (5.2)$$

where $\delta(x - x')$ is the three-dimensional delta function which is zero everywhere, except at point $x = x'$, and it gives

$$\int f(x') \delta(x - x') dx' = f(x) \quad (5.3)$$

for any continuous function $f(x)$ where the integration domain contains point x . The proof of (5.2) is as follows:

From (5.1), we have

$$\begin{aligned} C_{ijkl} G_{km,lj}(x - x') &= -(2\pi)^{-3} \int_{-\infty}^{\infty} C_{ijkl} N_{km} D^{-1} \xi_l \xi_j \exp\{i\xi \cdot (x - x')\} d\xi \\ &= -(2\pi)^{-3} \int_{-\infty}^{\infty} K_{ik} N_{km} D^{-1} \exp\{i\xi \cdot (x - x')\} d\xi, \end{aligned} \quad (5.4)$$

where $K_{ik} = C_{ijkl}\xi_j\xi_l$. Since N_{km} is the cofactor of K , we have

$$K_{ik}N_{km}D^{-1} = \delta_{im}. \quad (5.5)$$

On the other hand, Dirac's delta function can be defined (see Appendix 3) as

$$\begin{aligned} \delta(\mathbf{x} - \mathbf{x}') &= \delta(x_1 - x'_1)\delta(x_2 - x'_2)\delta(x_3 - x'_3) \\ &= (2\pi)^{-3} \int_{-\infty}^{\infty} \exp\{i\boldsymbol{\xi} \cdot (\mathbf{x} - \mathbf{x}')\} d\boldsymbol{\xi}. \end{aligned} \quad (5.6)$$

Thus, (5.2) is proved. Equation (5.2) is the equation of equilibrium with respect to a displacement $G_{km}(\mathbf{x} - \mathbf{x}')$ when the body force $X_i(\mathbf{x}) = \delta_{im}\delta(\mathbf{x} - \mathbf{x}')$ is applied. In other words, $G_{ij}(\mathbf{x} - \mathbf{x}')$ is the x_i -component of displacement at point \mathbf{x} when a unit body force in the x_j -direction is applied at point \mathbf{x}' . The region occupied by the material is assumed to be infinitely extended. Since $G_{ij}(\mathbf{x} - \mathbf{x}') = G_{ji}(\mathbf{x} - \mathbf{x}')$, the directions x_i and x_j in the above statement are interchangeable. It should be noted that $N_{ij}(\boldsymbol{\xi})$ and $D(\boldsymbol{\xi})$ are homogeneous polynomials with degrees four and six, respectively. Thus we have, $G_{ij}(\mathbf{x} - \mathbf{x}') = G_{ij}(\mathbf{x}' - \mathbf{x})$.

The integration in (5.1) requires explicit expressions for K , N_{ij} , and D mentioned in Section 3.

Isotropic materials

By substituting (3.33) into (5.1), it follows that

$$G_{ij}(\mathbf{x}) = (2\pi)^{-3} \int_{-\infty}^{\infty} \frac{(\lambda + 2\mu)\delta_{ij}\xi^2 - (\lambda + \mu)\xi_i\xi_j}{\mu(\lambda + 2\mu)\xi^4} \exp(i\boldsymbol{\xi} \cdot \mathbf{x}) d\boldsymbol{\xi}, \quad (5.7)$$

where \mathbf{x}' is taken as zero without loss of generality. After the triple integrations are carried out, we obtain

$$\begin{aligned} G_{ij}(\mathbf{x}) &= \frac{1}{8\pi\mu x} \left\{ 2\delta_{ij} - \frac{\lambda + \mu}{\lambda + 2\mu} \left(\delta_{ij} - x_i x_j / x^2 \right) \right\} \\ &= \frac{1}{16\pi\mu(1 - \nu)x} \left\{ (3 - 4\nu)\delta_{ij} + x_i x_j / x^2 \right\}, \end{aligned} \quad (5.8)$$

where $x = (x_i x_i)^{1/2}$, or

$$G_{ij}(\mathbf{x} - \mathbf{x}') = \frac{1}{4\pi\mu} \frac{\delta_{ij}}{|\mathbf{x} - \mathbf{x}'|} - \frac{1}{16\pi\mu(1 - \nu)} \frac{\partial^2}{\partial x_i \partial x_j} |\mathbf{x} - \mathbf{x}'|, \quad (5.9)$$

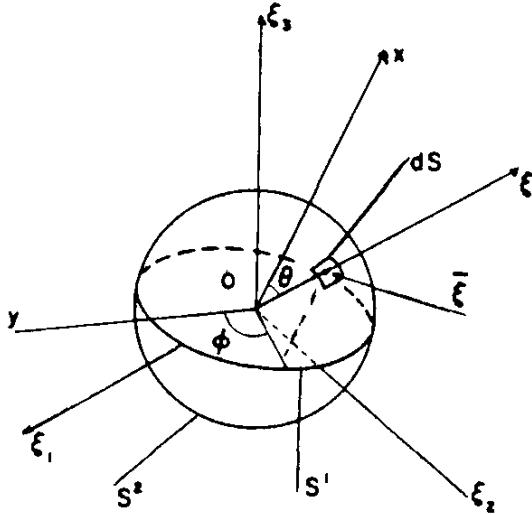


Fig. 5.1 The unit sphere S^2 in the ξ -space Green's function at point x is expressed by a line integral along S^1 which lies on the plane perpendicular to x

where λ, μ are the Lamé constants (μ = shear modulus), ν is Poisson's ratio, δ_{ij} is the Kronecker delta, and $|x - x'|^2 = (x_i - x'_i)(x_i - x'_i)$. This result was found by Lord Kelvin (1882).

The following expression is frequently used for isotropic materials:

$$C_{jlmn} G_{ij,l}(\bar{x}) = \frac{-1}{8\pi(1-\nu)} \left\{ (1-2\nu) \frac{\delta_{mi}\bar{x}_n + \delta_{ni}\bar{x}_m - \delta_{mn}\bar{x}_i}{\bar{x}^3} + 3 \frac{\bar{x}_m\bar{x}_n\bar{x}_i}{\bar{x}^5} \right\}, \quad (5.10)$$

where $\bar{x} = x - x'$ and $|x - x'| = \bar{x}$.

The details of the integration of (5.7) are as follows: The volume element $d\xi$ is defined as

$$d\xi = \xi^2 d\xi dS, \quad (5.11)$$

where $\xi = (\xi_i \xi_i)^{1/2}$ and dS is the surface element on the unit sphere S^2 in the ξ -space, centered at the origin of the coordinates ξ_i (see Fig. 5.1). By denoting

$$\xi = \xi \bar{\xi}, \quad x = x \bar{x} \quad (5.12)$$

(note that \bar{x} has been used in (5.10) with a different meaning), we can write (5.7) as

$$G_{ij}(x) = (2\pi)^{-3} \int_0^\infty d\xi \int_{S^2} \frac{(\lambda + 2\mu)\delta_{ij} - (\lambda + \mu)\bar{\xi}_i \bar{\xi}_j}{\mu(\lambda + 2\mu)} \exp(i\xi x \bar{\xi} \cdot \bar{x}) dS(\bar{\xi}). \quad (5.13)$$

When ξ in (5.7) is replaced by $-\xi$ as a new variable, we have

$$G_{ij}(x) = (2\pi)^{-3} \int_{-\infty}^{\infty} \frac{(\lambda + 2\mu)\delta_{ij}\xi^2 - (\lambda + \mu)\xi_i\xi_j}{\mu(\lambda + 2\mu)\xi^4} \exp(-i\xi \cdot x) d\xi. \quad (5.14)$$

Then, similarly,

$$\begin{aligned} G_{ij}(x) &= (2\pi)^{-3} \int_0^{\infty} d\xi \int_{S^2} \frac{(\lambda + 2\mu)\delta_{ij} - (\lambda + \mu)\bar{\xi}_i\bar{\xi}_j}{\mu(\lambda + 2\mu)} \exp(-i\xi x \bar{\xi} \cdot \bar{x}) dS(\bar{\xi}) \\ &= (2\pi)^{-3} \int_{-\infty}^0 d\xi \int_{S^2} \frac{(\lambda + 2\mu)\delta_{ij} - (\lambda + \mu)\bar{\xi}_i\bar{\xi}_j}{\mu(\lambda + 2\mu)} \exp(i\xi x \bar{\xi} \cdot \bar{x}) dS(\bar{\xi}). \end{aligned} \quad (5.15)$$

The last result has been obtained by the transformation of $\xi \rightarrow -\xi$. By adding (5.13) and (5.15), we have

$$G_{ij}(x) = \frac{(2\pi)^{-3}}{2} \int_{-\infty}^{\infty} d\xi \int_{S^2} \frac{(\lambda + 2\mu)\delta_{ij} - (\lambda + \mu)\bar{\xi}_i\bar{\xi}_j}{\mu(\lambda + 2\mu)} \exp(i\xi x \bar{\xi} \cdot \bar{x}) dS(\bar{\xi}). \quad (5.16)$$

The integration with respect to ξ leads to Dirac's delta function. Thus,

$$G_{ij}(x) = \frac{(2\pi)^{-2}}{2} \int_{S^2} \delta(x \bar{\xi} \cdot \bar{x}) \frac{(\lambda + 2\mu)\delta_{ij} - (\lambda + \mu)\bar{\xi}_i\bar{\xi}_j}{\mu(\lambda + 2\mu)} dS(\bar{\xi}). \quad (5.17)$$

Denoting the angle between $\bar{\xi}$ and \bar{x} by θ (see Fig. 5.1), we have

$$\begin{aligned} \bar{\xi} \cdot \bar{x} &= \cos \theta, \quad d(\bar{\xi} \cdot \bar{x}) = -\sin \theta d\theta, \\ dS(\bar{\xi}) &= \sin \theta d\theta d\phi = -d(\bar{\xi} \cdot \bar{x}) d\phi, \end{aligned} \quad (5.18)$$

where ϕ is defined on the plane perpendicular to x as shown in Fig. 5.1. The starting line for measuring ϕ can be arbitrary and (5.17) becomes

$$\begin{aligned} G_{ij}(x) &= \frac{(2\pi)^{-2}}{2} \int_{-1}^1 d(\bar{\xi} \cdot \bar{x}) \delta(x \bar{\xi} \cdot \bar{x}) \int_0^{2\pi} d\phi \frac{(\lambda + 2\mu)\delta_{ij} - (\lambda + \mu)\bar{\xi}_i\bar{\xi}_j}{\mu(\lambda + 2\mu)} \\ &= \frac{(2\pi)^{-2}}{2x} \oint_{S^1} \frac{(\lambda + 2\mu)\delta_{ij} - (\lambda + \mu)\bar{\xi}_i\bar{\xi}_j}{\mu(\lambda + 2\mu)} d\phi, \end{aligned} \quad (5.19)$$

where S^1 is the unit circle on S^2 intersected by the plane perpendicular to x .

$\bar{\xi}$ located on S^1 can be written as

$$\bar{\xi}_i = a_{1i} \cos \phi + a_{2i} \sin \phi, \quad (5.20)$$

where a_{1i} is the unit vector along $0y$, and a_{2i} is the unit vector lying on the plane of S^1 and normal to $0y$. Then,

$$\oint_{S^1} \bar{\xi}_i \bar{\xi}_j d\phi = \pi(a_{1i}a_{1j} + a_{2i}a_{2j}) = \pi(\delta_{ij} - \bar{x}_i \bar{x}_j). \quad (5.21)$$

The last result is obtained by the orthogonality of the three unit vectors a_{1i} , a_{2i} , and \bar{x}_i ,

$$a_{1i}a_{1j} + a_{2i}a_{2j} + \bar{x}_i \bar{x}_j = \delta_{ij} \quad (5.22)$$

which is the scalar product of the unit vectors along the x_i - and x_j -axes.

Green's functions for the two-dimensional plane strain problems can be obtained by considering the elastic field due to a distributed line force along the x_3 -axis,

$$G_{ij}(x_1 - x'_1, x_2 - x'_2) = \int_{-\infty}^{\infty} G_{ij}(x - x') dx'_3, \quad (5.23)$$

where $G_{ij}(x - x')$ is defined by (5.1). For isotropic media,

$$G_{ij}(x_1 - x'_1, x_2 - x'_2) = \left\{ \bar{x}_i \bar{x}_j / \bar{R}^2 - (3 - 4\nu) \delta_{ij} \log \bar{R} \right\} / 8\pi(1 - \nu)\mu \quad (5.24)$$

where $\bar{R}^2 = (x_1 - x'_1)^2 + (x_2 - x'_2)^2$.

Green's functions for the plane stress can be derived from those for the plane strain by replacing E with $E(1 + 2\nu)/(1 + \nu)^2$ and ν with $\nu/(1 + \nu)$, since in both cases these replacements give the same form of Hooke's law.

* Anisotropic materials

As seen in (3.9), $N_{ij}(\xi)$ and $D(\xi)$ are homogeneous polynomials with degrees four and six, respectively. We can therefore rewrite (5.1) in the same way as we derived (5.17) and (5.19),

$$G_{ij}(x) = \frac{1}{8\pi^2} \int_{S^2} \delta(x \bar{\xi} \cdot \bar{x}) N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) dS(\bar{\xi}) \quad (5.25)$$

and

$$G_{ij}(x) = \frac{1}{8\pi^2 x} \oint_{S^1} N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) d\phi, \quad (5.26)$$

where $x = |\mathbf{x}|$ and the unit sphere S^2 and circle S^1 are shown in Fig. 5.1. The line integral expression (5.26) has been investigated by Fredholm (1900), Lifshitz and Rozenzweig (1947), and Synge (1957).

As will be seen in later sections, it is not always necessary to have explicit expressions of Green's functions. The form (5.25) is sometimes more convenient than explicit expressions or numerical representations, when it is applied to problems of inclusions and dislocations. However, much effort has been devoted to deriving explicit expressions, approximate forms, or numerical values of Green's functions. Kröner (1953) has factorized $D(\xi)$ for hexagonal crystals into three polynomials of degree two and obtained an explicit expression of Green's function for hexagonal crystals. He has also expanded Green's function for general anisotropic materials into a series of surface harmonic functions. Further analytical investigations of Green's functions for anisotropic materials have been done by Willis (1965), Indenbom and Orlov (1968), and Mura and Kinoshita (1971), among others. For cubic crystals, no analytical expressions of Green's functions are available except for the two-dimensional case; see, for instance, Eshelby, Read, and Shockley (1953) and Foreman (1955). However, series expressions are given for approximated solutions of Green's functions, as seen in the work of Mann, Jan, and Seeger (1961), Lie and Koehler (1968), and Bross (1968).

* Transversely isotropic materials

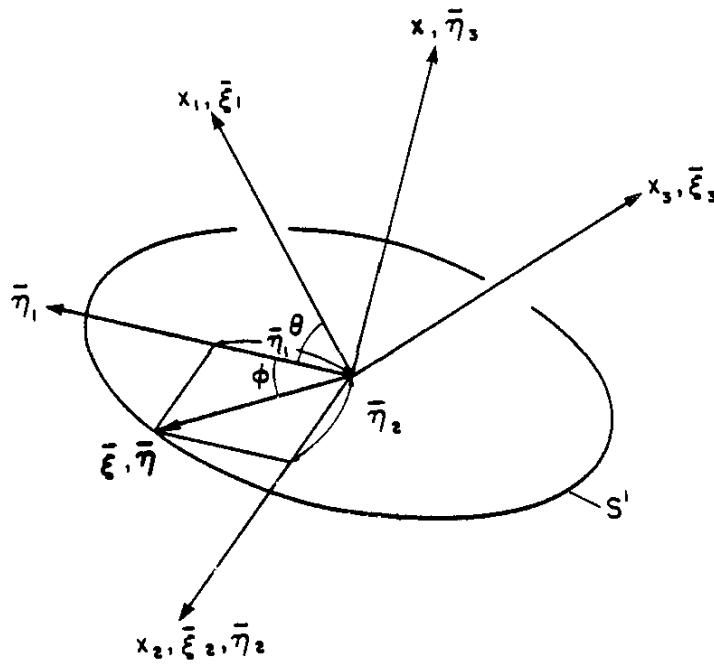
The presentation in this subsection follows to a large extent the work of Willis (1965). For transversely isotropic materials, Willis has performed the integration in (5.26) by the residue calculation. Expression $D(\xi)$ in (3.38) is written in the form

$$D(\xi) = \beta\gamma^2 \prod_{i=1}^3 \left\{ A_i (\bar{\xi}_1^2 + \bar{\xi}_2^2) + \bar{\xi}_3^2 \right\}, \quad (5.27)$$

where

$$\begin{aligned} A_1 &= \alpha'/\gamma, \quad A_2 = \frac{1}{2} \left[\frac{\gamma^2 + \alpha\beta - \gamma'^2}{\beta\gamma} + \left\{ \left(\frac{\gamma^2 + \alpha\beta - \gamma'^2}{\beta\gamma} \right)^2 - 4\frac{\alpha}{\beta} \right\}^{1/2} \right], \\ A_3 &= \frac{1}{2} \left[\frac{\gamma^2 + \alpha\beta - \gamma'^2}{\beta\gamma} - \left\{ \left(\frac{\gamma^2 + \alpha\beta - \gamma'^2}{\beta\gamma} \right)^2 - 4\frac{\alpha}{\beta} \right\}^{1/2} \right]. \end{aligned} \quad (5.28)$$

For transversely isotropic materials the direction of the x_3 -axis is chosen as an

Fig. 5.2 Coordinate systems and a unit circle S^1

axis of symmetry, but the other two directions, x_1 and x_2 , can be arbitrary. For a given x we choose the coordinate system shown in Fig. 5.2. The x_1 - and x_3 -axes and vector x lie on the same plane, and the x_2 -axis is normal to the plane; that is, the x_1 -axis is chosen on the plane containing the x_3 -axis and vector x , and the x_2 -axis is normal to the plane. A new coordinate system $\bar{\eta}_1$, $\bar{\eta}_2$, and $\bar{\eta}_3$ is chosen as shown in Fig. 5.2. The $\bar{\eta}_1$ - and $\bar{\eta}_2$ -axes lie on the plane containing S^1 , and $\bar{\eta}_1$, $\bar{\xi}_1$, $\bar{\eta}_3$ and $\bar{\xi}_3$ are on the same plane. The angle between the $\bar{\eta}_1$ - and x_1 -axes is denoted by θ . Then for a point ξ on S^1 ,

$$\bar{\xi}_1 = \bar{\eta}_1 \cos \theta, \quad \bar{\xi}_2 = \bar{\eta}_2, \quad \bar{\xi}_3 = -\bar{\eta}_1 \sin \theta. \quad (5.29)$$

Further transformation is introduced. We set $\zeta = \bar{\eta}_1 + i\bar{\eta}_2$, where

$$\begin{aligned} \zeta &= \exp(i\phi), & \bar{\eta}_1 &= \frac{1}{2}(\zeta + 1/\zeta), \\ \bar{\eta}_2 &= (1/2i)(\zeta - 1/\zeta), & d\phi &= d\xi/i\zeta. \end{aligned} \quad (5.30)$$

Then the factor in (5.27) is written as

$$\begin{aligned} A_i(\bar{\xi}_1^2 + \bar{\xi}_2^2) + \bar{\xi}_3^2 &= \sin^2 \theta (1 - A_i)(\xi^4 + 2B_i\xi^2 + 1)/4\xi^2 \\ \text{with } B_i &= (A_i \cos^2 \theta + \sin^2 \theta + A_i)/\sin^2 \theta (1 - A_i). \end{aligned} \quad (5.31)$$

Now Cauchy's theory of residues is applied to (5.26). If ζ is a root of the

equation

$$\xi^4 + 2B_i\xi^2 + 1 = 0, \quad (5.32)$$

then so are $-\xi$, $1/\xi$, and $-1/\xi$. Hence, only two roots, α_i and $-\alpha_i$, exist inside the unit circle S^1 . Let us consider the case where all A_i are real so that α_1^2 , α_2^2 , and α_3^2 are also real.

When the α_i ($i = 1, 2, 3$) are all distinct and non-zero, the residue calculation yields

$$G_{ij}(x) = \frac{C}{\pi x} \left\{ \frac{h_{ij}(\alpha_1^2)}{(\alpha_1^2 - \alpha_2^2)(\alpha_1^2 - \alpha_3^2)} + \frac{h_{ij}(\alpha_2^2)}{(\alpha_2^2 - \alpha_1^2)(\alpha_2^2 - \alpha_3^2)} + \frac{h_{ij}(\alpha_3^2)}{(\alpha_3^2 - \alpha_1^2)(\alpha_3^2 - \alpha_2^2)} \right\}, \quad (5.33)$$

where

$$C = \left\{ \beta \gamma^2 \sin^6 \theta \prod_{i=1}^3 (1 - A_i) \right\}^{-1},$$

$$h_{ij}(\alpha_i^2) = \frac{N_{ij} \left((\alpha_i^2 - 1) \cos \theta, \frac{\alpha_i^2 - 1}{i}, -(\alpha_i^2 + 1) \sin \theta \right)}{(\alpha_i^2 - \beta_1^2)(\alpha_i^2 - \beta_2^2)(\alpha_i^2 - \beta_3^2)}, \quad (5.34)$$

$$\beta_1^2 = 1/\alpha_1^2, \quad \beta_2^2 = 1/\alpha_2^2, \quad \beta_3^2 = 1/\alpha_3^2.$$

Expressions $N_{ij}(\xi_1, \xi_2, \xi_3)$ have been given by (3.39).

We now discuss a degenerate case, which occurs when $\alpha' = \gamma$, so that $A_1 = 1$. In this case, $A_1(\xi_1^2 + \xi_2^2) + \xi_3^2 = 1$ everywhere on the contour of integration and may therefore be omitted from the integrands. Similar residue calculation leads to

$$G_{ij}(x) = \frac{\{\beta \gamma^2 \sin^4 \theta (1 - A_2)(1 - A_3)\}^{-1}}{4\pi x}$$

$$\times \left\{ N_{ij}(\xi_1) + \frac{N_{ij}(\xi_2) \alpha_2^2 \alpha_3^2}{(\alpha_2^4 - 1)(\alpha_2^2 \alpha_3^2 - 1)(\alpha_2^2 - \alpha_3^2)} \right.$$

$$\left. + \frac{N_{ij}(\xi_3) \alpha_3^2 \alpha_2^2}{(\alpha_3^4 - 1)(\alpha_3^2 \alpha_2^2 - 1)(\alpha_3^2 - \alpha_2^2)} \right\}, \quad (5.35)$$

where

$$\begin{aligned}\xi_1 &= (\cos \theta, i, -\sin \theta), \\ \xi_2 &= \sqrt{2} \left((1 - B_2)^{1/2} \cos \theta, (1 + B_2)^{1/2}, -(1 - B_2)^{1/2} \sin \theta \right), \\ \xi_3 &= \sqrt{2} \left((1 - B_3)^{1/2} \cos \theta, (1 + B_3)^{1/2}, -(1 - B_3)^{1/2} \sin \theta \right).\end{aligned}\quad (5.36)$$

Alternative analytical expressions of Green's functions for transversely isotropic materials have been obtained by Lifshitz and Rozenzweig (1947), Elliott (1948), Kröner (1953), Lejček (1969), and Pan and Chou (1976). The result of Pan and Chou is written as follows:

$$\begin{aligned}G_{11}(\mathbf{x}) &= \sum_{i=1}^2 \left\{ (A'_i - B'_i) \left(\nu_i / R_i - \nu_i x_1^2 / R_i^3 \right) + 2\nu_i B'_i \left(1 / R_i^* - x_1^2 / R_i R_i^{*2} \right) \right\} \\ &\quad + D \left(1 / R_3^* - x_2^2 / R_3 R_3^{*2} \right), \\ G_{12}(x) = G_{21}(x) &= \sum_{i=1}^2 \left\{ - (A'_i - B'_i) \nu_i x_1 x_2 / R_i^3 - 2\nu_i B'_i x_1 x_2 / R_i R_i^{*2} \right\} \\ &\quad + D x_1 x_2 / R_3 R_3^{*2}, \\ G_{3j}(\mathbf{x}) &= \sum_{i=1}^2 \left\{ - (A'_i - B'_i) \nu_i^2 k_i x_j z_i / R_i^3 \right. \\ &\quad \left. - (C_{11} A'_i - C_{44} \nu_i^2 B'_i) 2x_j / (C_{13} + C_{44}) R_i R_i^* \right\}, \quad (j=1, 2) \\ G_{13}(\mathbf{x}) &= \sum_{i=1}^2 \left\{ \nu_i A_i x_1 / R_i R_i^* - \nu_i (A_i + B_i) x_1 z_i / R_i^3 \right\}, \\ G_{23}(\mathbf{x}) &= \sum_{i=1}^2 \left\{ \nu_i A_i x_2 / R_i R_i^* - \nu_i (A_i + B_i) x_2 z_i / R_i^3 \right\}, \\ G_{33}(\mathbf{x}) &= \sum_{i=1}^2 \left\{ - (C_{11} B_i + C_{44} \nu_i^2 A_i) / (C_{13} + C_{44}) R_i \right. \\ &\quad \left. - (A_i + B_i) \nu_i^2 (C_{44} \rho^2 + C_{11} x_3^2) / (C_{13} + C_{44}) R_i^3 \right\},\end{aligned}\quad (5.37)$$

$$G_{22}(x) = \sum_{i=1}^2 \left\{ (A'_i - B'_i) \left(\nu_i/R_i - \nu_i x_2^2/R_i^3 \right) + 2\nu_i B'_i \left(1/R_i^* - x_2^2/R_i R_i^{*2} \right) \right\} \\ + D \left(1/R_3^* - x_1^2/R_3 R_3^{*2} \right),$$

where

$$\begin{aligned} \nu_1 &= (\tilde{C}_{13} - C_{13})^{1/2} (\tilde{C}_{13} + C_{13} + 2C_{44})^{1/2} / (4C_{33}C_{44})^{1/2} \\ &\quad + (\tilde{C}_{13} + C_{13})^{1/2} (\tilde{C}_{13} - C_{13} - 2C_{44})^{1/2} / (4C_{33}C_{44})^{1/2}, \\ \nu_2 &= (\tilde{C}_{13} - C_{13})^{1/2} (\tilde{C}_{13} + C_{13} + 2C_{44})^{1/2} / (4C_{33}C_{44})^{1/2} \\ &\quad - (\tilde{C}_{13} + C_{13})^{1/2} (\tilde{C}_{13} - C_{13} - 2C_{44})^{1/2} / (4C_{33}C_{44})^{1/2}, \\ \nu_3 &= (C_{66}/C_{44})^{1/2}, \quad k_i = (C_{11}/\nu_i^2 - C_{44}) / (C_{13} + C_{44}), \\ \tilde{C}_{13} &= (C_{11}C_{33})^{1/2}, \quad z_1 = \nu_1 x_3, \quad z_2 = \nu_2 x_3, \quad z_3 = \nu_3 x_3, \\ R_i &= (x_1^2 + x_2^2 + z_i^2)^{1/2}, \quad R_i^* = R_i + z_i, \quad D = \frac{1}{4}\pi C_{44} \nu_3. \end{aligned} \tag{5.38}$$

For $\tilde{C}_{13} - C_{13} - 2C_{44} \neq 0$,

$$\begin{aligned} A'_1 &= B'_1 = - (C_{44} - C_{33}\nu_1^2) / 8\pi C_{33}C_{44} (\nu_1^2 - \nu_2^2) \nu_1^2, \\ A'_2 &= B'_2 = (C_{44} - C_{33}\nu_2^2) / 8\pi C_{33}C_{44} (\nu_1^2 - \nu_2^2) \nu_2^2, \\ \nu_1 A_1 &= -\nu_2 A_2 = (C_{13} + C_{44}) / 4\pi C_{33}C_{44} (\nu_2^2 - \nu_1^2), \\ B_i &= -A_i. \end{aligned} \tag{5.39}$$

For $\tilde{C}_{13} - C_{13} - 2C_{44} = 0$,

$$\begin{aligned} A'_1 &= A'_2 = 1/16\pi C_{11}, \\ B'_1 &= B'_2 = 1/16\pi C_{44} \nu_1^2, \\ A_1 &= A_2 = 0, \\ B_1 &= B_2 = -(C_{13} + C_{44}) / 16\pi C_{11} C_{44}. \end{aligned} \tag{5.40}$$

Pan and Chou (1979) have extended their solution to a two-phase material (see also Sveklo 1969).

* *Kröner's formula*

Kröner's formula (1953) is a series expression of Green's functions for general anisotropic materials. Since $N_{ij}(\xi)/D(\xi)$ in (5.25) and (5.26) is a continuous function on S^2 , it can be expanded in a series of surface harmonic functions which uniformly converge on S^2 , as follows (Hobson, 1931):

$$N_{ij}(\xi)/D(\xi) = \sum_{n=0}^{\infty} U_n(\xi), \quad (5.41)$$

where

$$U_n(\xi) = \frac{2n+1}{4\pi} \int_{S^2} P_n(\xi \cdot \xi') N_{ij}(\xi') D^{-1}(\xi') dS(\xi'), \quad n = 0, 1, 2, \dots, \quad (5.42)$$

and P_n is the Legendre polynomial. From the orthogonality of the Legendre polynomials, we can easily derive

$$U_n(\xi) = \frac{2n+1}{4\pi} \int_{S^2} P_n(\xi \cdot \xi') U_n(\xi') dS(\xi'). \quad (5.43)$$

When (5.43) is integrated over S^1 , we have

$$\int_{S^1} U_n(\xi) ds(\xi) = \frac{2n+1}{4\pi} \int_{S^2} U_n(\xi') dS(\xi') \int_{S^1} P_n(\xi \cdot \xi') ds(\xi), \quad (5.44)$$

where $ds(\xi) = d\phi$ is a line element of S^1 .

The addition theorem of the Legendre polynomials is

$$P_n(\xi \cdot \xi') = P_n(\xi \cdot \bar{x}) P_n(\xi' \cdot \bar{x}) + 2 \sum_{m=1}^n \frac{(n-m)!}{(n+m)!} P_n^m(\xi \cdot \bar{x}) P_n^m(\xi' \cdot \bar{x}) \cos m(\phi - \phi'), \quad (5.45)$$

where $P_n^m(z)$ is the associated Legendre function of degree n and of order m .

When (5.45) is integrated over S^1 with respect to ξ for a fixed value of ξ' , we have, since $\xi \cdot \bar{x} = 0$ and $\int_{S^1} \cos m(\phi - \phi') d\phi = 0$,

$$\int_{S^1} P_n(\xi \cdot \xi') ds(\xi) = 2\pi P_n(0) P_n(\xi' \cdot \bar{x}). \quad (5.46)$$

Substituting (5.46) into (5.44) and using (5.43), we have

$$\int_{S^1} U_n(\xi) ds(\xi) = 2\pi P_n(0) U_n(\bar{x}). \quad (5.47)$$

Therefore, (5.26) can be written as

$$G_{ij}(x) = \frac{1}{4\pi x} \sum_{n=0}^{\infty} P_n(0) U_n(\bar{x}) \quad (5.48)$$

which is an expression of Kröner's formula (1953) modified by Mura and Kinoshita (1971). The integer n in (5.48) takes only even numbers, since $P_n(0) = 0$ for odd numbers of n . $U_n(\bar{x})$ is calculated from

$$U_n(\bar{x}) = \frac{2n+1}{4\pi} \int_{S^2} P_n(\bar{x} \cdot \bar{\xi}) N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) dS(\bar{\xi}). \quad (5.49)$$

The uniform convergency of (5.48) has been proved by Mura and Kinoshita (1971). They also showed that

$$G_{ij}(x) = \frac{1}{16\pi^2} \Delta \int_{S^2} N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) |x \cdot \bar{\xi}| dS(\bar{\xi}), \quad (5.50)$$

where Δ is the Laplacian. The above expression is continuously differentiable.

* Derivatives of Green's functions

As seen in Eqs. (3.23) ~ (3.25), derivatives of Green's function appear in these formulae rather than Green's function itself. The plane of the unit circle S^1 in (5.26), however, is orthogonal to the direction of x and therefore depends on x . Therefore, the derivatives of G_{ij} need special consideration. The following argument follows the work done by Barnett (1972) and Willis (1975). Differentiating (5.25) with respect to x_k leads to

$$G_{ij,k}(x) = \frac{1}{8\pi^2} \int_{S^2} \delta'(\bar{\xi} \cdot x) \bar{\xi}_k N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) dS(\bar{\xi}), \quad (5.51)$$

where δ' is the derivative of δ with respect to the argument. Since

$$\int_{S^2} dS(\bar{\xi}) = \oint_{S^1} d\phi \int_{-1}^1 d(\bar{\xi} \cdot \bar{x}) \quad (5.52)$$

and $\delta(x\bar{\xi} \cdot \bar{x})\bar{\xi}_k N_{ij}(\bar{\xi})D^{-1} = 0$ at $\bar{\xi} \cdot \bar{x} = 1$ and -1 ($\theta = 0, \pi$), integration by parts of (5.51) yields

$$G_{i_l, k}(x) = \frac{-1}{8\pi^2} \int_{S^2} \delta(\bar{\xi} \cdot x) \partial[\bar{\xi}_k N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi})] / \partial(\bar{\xi} \cdot x) dS(\bar{\xi}) \quad (5.53)$$

and therefore

$$G_{i_j, k}(x) = \frac{-1}{8\pi^2 x} \oint_{S^1} \partial[\bar{\xi}_k N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi})] / \partial(\bar{\xi} \cdot x) d\phi. \quad (5.54)$$

On S^1 we have (see Fig. 5.1)

$$\begin{aligned} \partial(\bar{\xi} \cdot x) &= x \partial(\bar{\xi} \cdot \bar{x}) = -x \sin \theta d\theta = -x d\theta, \\ \partial \bar{\xi}_k / \partial \theta &= -\bar{x}_k, \end{aligned} \quad (5.55)$$

$$\partial K_{lm} / \partial \theta = -C_{lpmq} (\bar{x}_p \bar{\xi}_q + \bar{\xi}_p \bar{x}_q),$$

where K_{lm} has been defined in (3.5). On the other hand, differentiating $K_{lm} N_{mj} D^{-1} = \delta_{lj}$ with respect to θ , we obtain

$$\frac{\partial}{\partial \theta} (K_{lm}) N_{mj} D^{-1} + K_{lm} \frac{\partial}{\partial \theta} (N_{mj} D^{-1}) = 0 \quad (5.56)$$

and

$$\begin{aligned} \frac{\partial}{\partial \theta} (N_{ij} D^{-1}) &= -N_{li} D^{-1} \frac{\partial}{\partial \theta} (K_{lm}) N_{mj} D^{-1} \\ &= N_{il} N_{jm} D^{-2} C_{lpmq} (\bar{x}_p \bar{\xi}_q + \bar{\xi}_p \bar{x}_q) \end{aligned} \quad (5.57)$$

since $N_{li} D^{-1} K_{lm} = \delta_{im}$. Thus, (5.54) is written as

$$\begin{aligned} G_{i_j, k}(x) &= \frac{1}{8\pi^2 x^2} \oint_{S^1} [-\bar{x}_k N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) \\ &\quad + \bar{\xi}_k C_{lpmq} (\bar{x}_p \bar{\xi}_q + \bar{\xi}_p \bar{x}_q) N_{li} N_{mj} D^{-2}] d\phi. \end{aligned} \quad (5.58)$$

Higher derivatives can be obtained in a similar manner.

Differentiating (5.25) with respect to x_k and x_l , we have

$$G_{ij,kl}(x) = \frac{1}{8\pi^2} \oint_{S^1} d\phi \int_{-1}^1 \delta''(\bar{\xi} \cdot x) \bar{\xi}_k \bar{\xi}_l N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) d(\bar{\xi} \cdot \bar{x}). \quad (5.59)$$

Integrating twice by parts, we arrive at

$$G_{ij,kl}(x) = \frac{1}{8\pi^2 x^3} \oint_{S^1} d\phi \partial^2 (\bar{\xi}_k \bar{\xi}_l N_{ij} D^{-1}) / \partial \theta^2 \quad (5.60)$$

which is finally written as

$$\begin{aligned} G_{ij,kl}(x) = & \frac{1}{8\pi^2 x^3} \oint_{S^1} [2 \bar{x}_k \bar{x}_l N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) \\ & - 2 \{ (\bar{x}_k \bar{\xi}_l + \bar{\xi}_k \bar{x}_l) (\bar{x}_p \bar{\xi}_q + \bar{\xi}_p \bar{x}_q) + \bar{\xi}_k \bar{\xi}_l \bar{x}_p \bar{x}_q \} \\ & \times N_{ih}(\bar{\xi}) N_{jm}(\bar{\xi}) C_{hpmq} D^{-2}(\bar{\xi}) \\ & + \bar{\xi}_k \bar{\xi}_l C_{hpmq} (\bar{x}_p \bar{\xi}_q + \bar{\xi}_p \bar{x}_q) C_{satb} (\bar{x}_a \bar{\xi}_b + \bar{\xi}_a \bar{x}_b) \\ & \times \{ N_{jm}(\bar{\xi}) N_{is}(\bar{\xi}) N_{ht}(\bar{\xi}) \\ & + N_{ih}(\bar{\xi}) N_{js}(\bar{\xi}) N_{mt}(\bar{\xi}) \} D^{-3}(\bar{\xi})] d\phi. \end{aligned} \quad (5.61)$$

* Two-dimensional Green's function

Green's function for two-dimensional problems has been given by (5.24) for isotropic materials. For anisotropic materials, the two-dimensional Green's function has been developed by Eshelby, Read and Shockley (1953), Stroh (1962), and Bullough and Bilby (1954). It is particularly convenient to use Green's function for generalized plane problems (mixture of plane strain and antiplane strain).

Green's function $G_{ij}(x - x')$ for generalized plane problems can be obtained as the displacement $u_i(x)$ when a unit line force F_j acting at x' parallel to the x_3 -axis is applied. The equations of equilibrium for u_i are

$$C_{i\beta k\alpha} u_{k,\alpha\beta} = 0, \quad i, k = 1, 2, 3; \quad \alpha, \beta = 1, 2, \quad (5.62)$$

for all points except x' . Hooke's law is

$$\sigma_{ij} = C_{ijkl} e_{kl}, \quad (5.63)$$

and the strain and displacement relation is

$$e_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i}). \quad (5.64)$$

The solution is given by an arbitrary function of a linear combination of the variables x_1 and x_2 and can be written as

$$u_k = A_k f(z), \quad (5.65)$$

where

$$z = x_1 + px_2. \quad (5.66)$$

Substituting (5.65) into (5.62), we have

$$\phi_{ik} A_k = 0 \quad (5.67)$$

with

$$\phi_{ik} = C_{i1k1} + p(C_{i1k2} + C_{i2k1}) + p^2 C_{i2k2}. \quad (5.68)$$

Equation (5.67) has a nontrivial solution for the vector A_k if and only if the determinant of the coefficients equals zero,

$$|\phi_{ik}| = 0. \quad (5.69)$$

This determinant is a six-order polynomial in p . Its complex roots occur in conjugate pairs. For a given root $p_{(l)}$, the corresponding values of A_k are determined from (5.67). However, the displacements must be real; this condition requires that the imaginary parts of corresponding pairs of solutions cancel out, and we need take only three roots $p_{(1)}$, $p_{(2)}$ and $p_{(3)}$, no two of which are complex conjugates. The displacement equation can be written more explicitly as

$$u_k = \operatorname{Re} \left\{ \sum_{l=1}^3 A_{(l)k} f_{(l)}[z_{(l)}] \right\}, \quad (5.70)$$

where $z_{(l)} = x_1 + p_{(l)}x_2$, and Re stands for the real part of the complex function in the braces. The stress components are

$$\sigma_{ij} = \text{Re} \left\{ \sum_{l=1}^3 [C_{ijk1} + p_{(l)}C_{ijk2}] A_{(l)k} f'_{(l)}[z_{(l)}] \right\}. \quad (5.71)$$

If $f_{(l)}[z_{(l)}]$ is not a single-valued function, the singular line represents a Volterra dislocation. On the other hand, if $f_{(l)}[z_{(l)}]$ is a single-valued function but the resultant force around the singular line is not zero, the singular line then represents a line force per unit length, F_i . The conditions necessary for obtaining Green's function are

$$\int_c \sigma_{ij} n_j \, ds = F_i \quad (5.72)$$

and

$$\int_c du_k \, ds = 0, \quad (5.73)$$

where c is a closed curve around point x' , and n_i is a unit vector normal to c . Condition (5.72) can be written as

$$-\int_c d\psi_i = F_i, \quad (5.74)$$

where ψ_i is defined by

$$\sigma_{i1} = -\partial\psi_i/\partial x_2, \quad \sigma_{i2} = \partial\psi_i/\partial x_1. \quad (5.75)$$

From (5.71), (5.68) and (5.75), we have

$$\psi_i = \text{Re} \left\{ \sum_{l=1}^3 [C_{i2k1} + p_{(l)}C_{i2k2}] A_{(l)k} f_{(l)}[z_{(l)}] \right\}. \quad (5.76)$$

Eshelby, Read, and Shockley (1953) have shown that

$$f_{(l)}[z_{(l)}] = (1/\pm 2\pi i) D_{(l)} \log[z_{(l)} - z'_{(l)}] \quad (5.77)$$

with

$$z'_{(l)} = x'_1 + p_{(l)}x'_2, \quad (5.78)$$

where the sign of $2\pi i$ is taken to be the same as the sign of the imaginary part of $p_{(l)}$. Then, conditions (5.74) and (5.73) become

$$\operatorname{Re} \left\{ \sum_{l=1}^3 [C_{i2k1} + P_{(l)} C_{i2k2}] A_{(l)k} D_{(l)} \right\} = F_i \quad (5.79)$$

and

$$\operatorname{Re} \left\{ \sum_{l=1}^3 A_{(l)k} D_{(l)} \right\} = 0. \quad (5.80)$$

Equations (5.79) and (5.80) represent a system of six simultaneous linear equations for six unknowns $D_{(l)}$.

The properties of the coefficients $A_{(l)k}$ were studied in detail by Stroh (1962). $D_{(l)}$ can always be expressed as a linear combination of F_i ,

$$D_{(l)} = d_{(l)m} F_m. \quad (5.81)$$

Now let us define a complex variable $X = x_1 + ix_2$ (with its conjugate $\bar{X} = x_1 - ix_2$). It follows that

$$z_{(l)} = \frac{1}{2} [X + \bar{X} - ip_{(l)} X + ip_{(l)} \bar{X}]. \quad (5.82)$$

The displacement then is

$$u_k(x) = \operatorname{Re} \left\{ \sum_{l=1}^3 A_{(l)k} \frac{d_{(l)m} F_m}{\pm 2\pi i} \log \frac{1}{2} [(X + \bar{X} - ip_{(l)} X + ip_{(l)} \bar{X}) \right. \\ \left. - (X' + \bar{X}' - ip_{(l)} X' + ip_{(l)} \bar{X}')] \right\}. \quad (5.83)$$

Finally, Green's function is obtained from (5.83) as

$$G_{km}(x - x') = \operatorname{Re} \left\{ \sum_{l=1}^3 A_{(l)k} \frac{d_{(l)m}}{\pm 2\pi i} \log \frac{1}{2} [(X + \bar{X} - ip_{(l)} X + ip_{(l)} \bar{X}) \right. \\ \left. - (X' + \bar{X}' - ip_{(l)} X' + ip_{(l)} \bar{X}')] \right\}. \quad (5.84)$$

6. Inclusions and inhomogeneities

Inclusions

An inclusion is defined as a sub-domain Ω in domain D , where eigenstrain $\epsilon_{ij}^*(x)$ is given in Ω and is zero in $D-\Omega$. The elastic moduli in Ω and $D-\Omega$ are assumed to be the same. The remaining domain $D-\Omega$ is called the matrix. From the result in Section 3, the elastic field due to the inclusion can be written as

$$\begin{aligned} u_i(x) &= - \int_{\Omega} C_{jlmn} \epsilon_{mn}^*(x') G_{ij,l}(x-x') dx', \\ \epsilon_{ij}(x) &= -\frac{1}{2} \int_{\Omega} C_{klmn} \epsilon_{mn}^*(x') \{ G_{ik,lj}(x-x') + G_{jk,li}(x-x') \} dx', \end{aligned} \quad (6.1)$$

$$\sigma_{ij}(x) = -C_{ijkl} \left\{ \int_{\Omega} C_{pqmn} \epsilon_{mn}^*(x') G_{kp,ql}(x-x') dx' + \epsilon_{kl}^*(x) \right\},$$

or

$$\begin{aligned} u_i(x) &= -i(2\pi)^{-3} \int_{-\infty}^{\infty} \int_{\Omega} C_{jlmn} \epsilon_{mn}^*(x') \xi_l N_{ij}(\xi) D^{-1}(\xi) \\ &\quad \times \exp\{i\xi \cdot (x-x')\} d\xi dx', \\ \epsilon_{ij}(x) &= (2\pi)^{-3} \int_{-\infty}^{\infty} \int_{\Omega} \frac{1}{2} C_{klmn} \epsilon_{mn}^*(x') \xi_l \{ \xi_j N_{ik}(\xi) + \xi_i N_{jk}(\xi) \} \\ &\quad \times D^{-1}(\xi) \exp\{i\xi \cdot (x-x')\} d\xi dx', \end{aligned} \quad (6.2)$$

$$\begin{aligned} \sigma_{ij}(x) &= C_{ijkl} \left\{ (2\pi)^{-3} \int_{-\infty}^{\infty} \int_{\Omega} C_{pqmn} \epsilon_{mn}^*(x') \xi_q \xi_l N_{kp}(\xi) D^{-1}(\xi) \right. \\ &\quad \left. \times \exp\{i\xi \cdot (x-x')\} d\xi dx' - \epsilon_{kl}^*(x) \right\}, \end{aligned}$$

where $\epsilon_{kl}^*(x) = 0$ for $x \in D - \Omega$.

Since $\epsilon_{kl}^*(x)$ is discontinuous on the boundary of domain Ω , some quanti-

ties may also be discontinuous on that boundary. However, the displacement and the interfacial traction across the boundary must be continuous; that is,

$$\begin{aligned} [u_i] &\equiv u_i(\text{out}) - u_i(\text{in}) = 0, \\ [\sigma_{ij}]n_j &\equiv \{\sigma_{ij}(\text{out}) - \sigma_{ij}(\text{in})\}n_j = 0, \end{aligned} \quad (6.3)$$

where n_i is the outward unit normal to the boundary of Ω . When an inclusion or inhomogeneity can slide on the interfacial surface, the first condition in (6.3) does not hold. This case will be discussed in Section 51. As discussed by Hill (1961) and by Walpole (1967), the displacement gradient (distortion) $u_{i,j}$ is continuous inside Ω and $D-\Omega$ but discontinuous at the interface between the two domains. The jump across the interface can be written as

$$[u_{i,j}] \equiv u_{i,j}(\text{out}) - u_{i,j}(\text{in}) = \lambda_i n_j. \quad (6.4)$$

λ_i is the proportionality constant (the magnitude of the jump) to be determined. The above equation is designed so that the condition $[u_{i,j}] dx_j = 0$ for dx along the boundary of Ω can be satisfied. This condition stems from the continuity of displacement along the boundary ($n_j dx_j = 0$ for dx along the boundary).

Since

$$\sigma_{ij} = C_{ijkl}(u_{k,l} - \epsilon_{kl}^*), \quad (6.5)$$

the second equation in (6.3) is written as

$$C_{ijkl}\{[u_{k,l}] - [\epsilon_{kl}^*]\}n_j = 0, \quad (6.6)$$

where $[\epsilon_{kl}^*] = \epsilon_{kl}^*(\text{out}) - \epsilon_{kl}^*(\text{in})$, $\epsilon_{kl}^*(\text{out}) = 0$, and $\epsilon_{kl}^*(\text{in}) = \epsilon_{kl}^*$. Substitution of (6.4) into (6.6) gives

$$C_{ijkl}\lambda_k n_l n_j = -C_{ijkl}\epsilon_{kl}^* n_j. \quad (6.7)$$

Equation (6.7) is a system of equations to determine λ for given n and ϵ_{ij}^* . Since the system has the same form as (3.4), we obtain

$$\lambda_i = -C_{jkmn}\epsilon_{mn}^* n_k N_{ij}(n)/D(n), \quad (6.8)$$

where $N_{ij}(\mathbf{n})$ and $D(\mathbf{n})$ are the cofactor and determinant of the matrix $\mathbf{K}(\mathbf{n})$, namely,

$$\begin{aligned} D(\mathbf{n}) &= \epsilon_{mn} K_{m1} K_{n2} K_{l3}, \\ N_{ij}(\mathbf{n}) &= \frac{1}{2} \epsilon_{ikl} \epsilon_{jmn} K_{km} K_{ln}, \end{aligned} \quad (6.9)$$

where $K_{ik} = C_{ijkl} n_j n_l$. Substituting (6.8) into (6.4), we arrive at

$$[u_{i,j}] = -C_{lkmn} \epsilon_{mn}^* n_k n_l N_{il}(\mathbf{n}) / D(\mathbf{n}) \quad (6.10)$$

and, therefore,

$$\begin{aligned} [\sigma_{ij}] &\equiv \sigma_{ij}(\text{out}) - \sigma_{ij}(\text{in}) = C_{ijkl} \{ [u_{k,l}] - [\epsilon_{kl}^*] \} \\ &= C_{ijkl} \{ -C_{pqmn} \epsilon_{mn}^* n_q n_l N_{kp}(\mathbf{n}) / D(\mathbf{n}) + \epsilon_{kl}^* \}. \end{aligned} \quad (6.11)$$

Equation (6.11) is useful for evaluating the stress just outside the inclusion when the stress just inside the inclusion is known. As shown in later chapters, eigenstresses inside ellipsoidal inclusions are usually easy to obtain. This interesting observation, relation (6.11), was first made by Goodier (1937) in his paper on thermal stresses.

Stress concentration factors of ellipsoidal inhomogeneities are easily obtained from (6.11), as seen in Section 31. The integrations in (6.1) and (6.2) are performed in Chapters 2 and 3 for ellipsoidal inclusions.

When the eigenstrain ϵ_{mn}^* is uniform in Ω and Ω has an arbitrary shape, it is sometimes convenient to rewrite (6.1), by integration by parts. Then, the first equation in (6.1) becomes

$$u_i(\mathbf{x}) = \int_{|\Omega|} C_{jlmn} \epsilon_{mn}^* G_{ij}(\mathbf{x} - \mathbf{x}') n_l dS, \quad (6.12)$$

where $|\Omega|$ is the boundary of Ω and dS is its surface element.

Inhomogeneities

When a subdomain of a material has elastic moduli different from those of the remainder (matrix), the subdivision is called an inhomogeneity. A uniform applied stress at infinity is not uniform in the neighbourhood of the inhomogeneity. This stress disturbance due to the inhomogeneity can be simulated by

an eigenstress field caused by an inclusion when the eigenstrain is chosen properly, as seen in Section 22.

For plane strain or plane stress inhomogeneity problems (composite problems), the complex potential method of Muskhelishvili (1953) is more effective than the equivalent inclusion method (see Sherman (1959), Hardiman (1954), Sendeckyj (1970), and Theocaris and Ioakimidis (1977), among others). Numerical examples of the application of the complex potential method are given for multiple circular inclusion problems by Kim and Ukadgaonker (1971) and Yu and Sendeckyj (1974). Sendeckyj (1970, 1971) also gives the exact solution of the antiplane longitudinal shear deformation of composites. In antiplane problems, the nonzero displacement and stress components are given in terms of one analytic function (see Milne-Thomson, 1962). Recently, Wheeler (1985) has considered the problem of minimizing stress concentration at a rigid inclusion.

Equations similar to (6.3) and (6.4) hold for an inhomogeneity under an applied stress. Let us assume that Ω is an inhomogeneity with the elastic moduli C_{ijkl}^* . The displacement and the interfacial traction across the boundary $|\Omega|$ must be continuous and the relations (6.3) and (6.4) hold.

Since

$$\begin{aligned}\sigma_{ij}(\text{out}) &= C_{ijkl}u_{k,l}(\text{out}), \\ \sigma_{ij}(\text{in}) &= C_{ijkl}^*u_{k,l}(\text{in}),\end{aligned}\tag{6.12.1}$$

the second equation in (6.3) is written as

$$C_{ijkl}u_{k,l}(\text{out})n_j = C_{ijkl}^*u_{k,l}(\text{in})n_j.\tag{6.12.2}$$

The relation (6.4),

$$u_{i,j}(\text{out}) = \lambda_i n_j + u_{i,j}(\text{in}),\tag{6.12.3}$$

is substituted into (6.12.2). Then, we have the equation to determine the unknown vector λ ,

$$C_{ijkl}\lambda_k n_l n_j = \Delta C_{ijkl}u_{k,l}(\text{in})n_j,\tag{6.12.4}$$

where $\Delta C_{ijkl} = C_{ijkl}^* - C_{ijkl}$.

According to the method for obtaining (3.7) from (3.4), we have

$$\lambda_i = X_k N_{ik}(\mathbf{n}) / D(\mathbf{n}), \quad (6.12.5)$$

where

$$X_k = \Delta C_{klmn} u_{m,n}(\text{in}) n_l. \quad (6.12.6)$$

Substituting (6.12.5) into (6.12.3), we have

$$u_{i,j}(\text{out}) = u_{i,j}(\text{in}) + N_{ik}(\mathbf{n}) n_j n_l \Delta C_{klmn} u_{m,n}(\text{in}) / D(\mathbf{n}). \quad (6.12.7)$$

Consequently, the first equation in (6.12.1) is written as

$$\sigma_{i,l}(\text{out}) = C_{ijkl} [u_{k,l}(\text{in}) + N_{kp}(\mathbf{n}) n_l n_q \Delta C_{pqmn} u_{m,n}(\text{in}) / D(\mathbf{n})] \quad (6.12.8)$$

or

$$\sigma_{i,l}(\text{out}) = C_{ijkl} [S_{klab}^* + N_{kp}(\mathbf{n}) N_l n_q \Delta C_{pqmn} S_{mnab}^* / D(\mathbf{n})] \sigma_{ab}(\text{in}) \quad (6.12.9)$$

where S_{ijkl}^* is the elastic compliance and

$$\frac{1}{2} \{ u_{i,j}(\text{in}) + u_{j,i}(\text{in}) \} = S_{ijkl}^* \sigma_{kl}(\text{in}). \quad (6.12.10)$$

The above result (6.12.9) has been obtained by Hill (1961) and applied to a thin-coated inhomogeneity by Walpole (1978). Numerical calculations for a coated inhomogeneity have been obtained by Mikata and Taya (1985) by the use of Papkovich-Neuber functions.

It must be emphasized that the equations (6.12.7) and (6.12.9) hold for any shape of inhomogeneity.

* Effect of isotropic elastic moduli on stress

Suppose that a composite body consisting of two isotropic elastic phases is loaded by prescribed surface traction $\lambda \mathbf{F}$, where \mathbf{F} is a fixed vector and λ is the load parameter. According to the linear theory, and when proportional loading is involved, the induced stress is proportional to the magnitude of the applied tractions. It then follows from dimensional analysis (e.g., Langhaar 1951) that in three-dimensional cases, the stress at any point in the composite is a function of three parameters involving the four elastic moduli. It is

obvious that σ_{ij}/λ , λ/μ_1 , μ_2/μ_1 , ν_1 , and ν_2 are dimensionless and that σ_{ij}/λ can be expressed as

$$\sigma_{ij}/\lambda = f(\lambda/\mu_1, \mu_2/\mu_1, \nu_1, \nu_2) \quad (6.13)$$

where μ_1 and ν_1 are the shear modulus and Poisson's ratio of the matrix and μ_2 , ν_2 are the corresponding constants of the inhomogeneity. Since σ_{ij} is a linear function of λ , (6.13) must have the form

$$\sigma_{ij}/\lambda = f(\mu_2/\mu_1, \nu_1, \nu_2). \quad (6.14)$$

Therefore, σ_{ij}/λ is a function of three parameters μ_2/μ_1 , ν_1 and ν_2 , or three combinations of them.

An important reduction in the dependence of stress on the elastic moduli takes place when the geometry and loading of the composite body are such that it is in a state of either plane strain or plane stress. If the composite body is multiply connected, and if the vector sums of tractions on holes vanish for every individual hole, Dundurs (1967) has shown that, under these conditions, the stress depends on only two combinations of elastic moduli. In a sense this result is the counterpart of the theorem by Michell (1899) establishing the circumstances under which stress is independent of elastic constants in a homogeneous material. Choice of these two combinations (parameters) of elastic moduli is not unique. Dundurs (1969, 1970) has proposed the following two parameters (Dundurs' constants):

$$\alpha = \frac{(\kappa_1 + 1)\Gamma - (\kappa_2 + 1)}{(\kappa_1 + 1)\Gamma + \kappa_2 + 1}, \quad (6.15)$$

$$\beta = \frac{(\kappa_1 - 1)\Gamma - (\kappa_2 - 1)}{(\kappa_1 + 1)\Gamma + \kappa_2 + 1},$$

where $\kappa = 3 - 4\nu$ for plane strain and $\kappa = (3 - \nu)/(1 + \nu)$ for plane stress, ν being Poisson's ratio. The subscripts 1 and 2 are used on the elastic moduli to distinguish between the two phases, and $\Gamma = \mu_2/\mu_1$, where μ is the shear modulus. The α, β -plane provides a convenient means for classifying composite materials regarding their physical behavior and for exhibiting such results as stress concentration factors that depend on the elastic moduli. Because of the physical limits $\Gamma \geq 0$ and $1 \leq \kappa \leq 3$, the admissible values of α and β are restricted to a bounded region or, more specifically, a parallelogram in the α, β -plane, as shown in Fig. 6.1. Identical materials are represented by $\alpha = \beta = 0$, and the case of equal shear moduli, or $\Gamma = 1$, corresponds to the

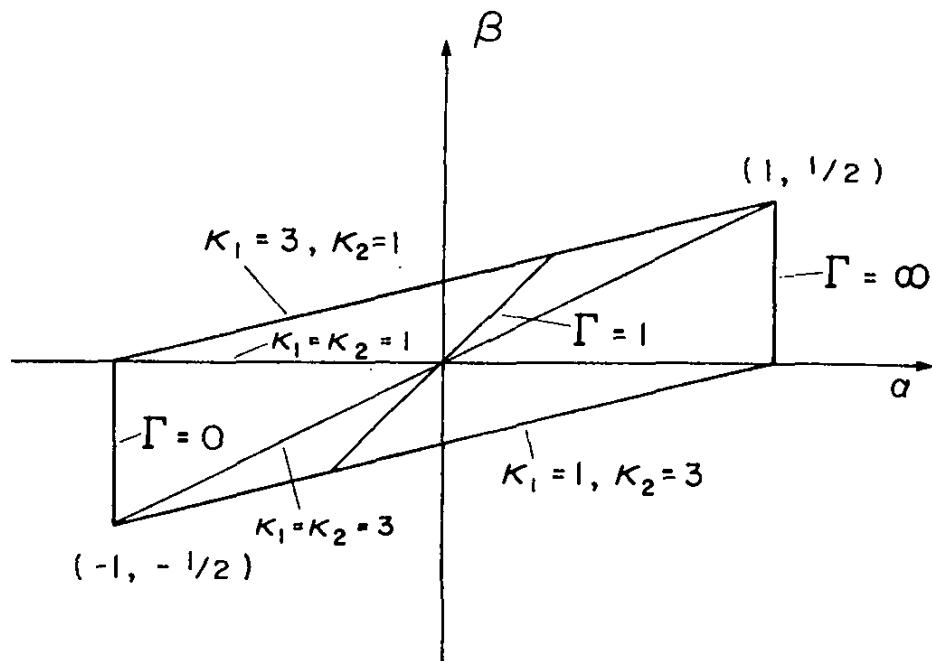
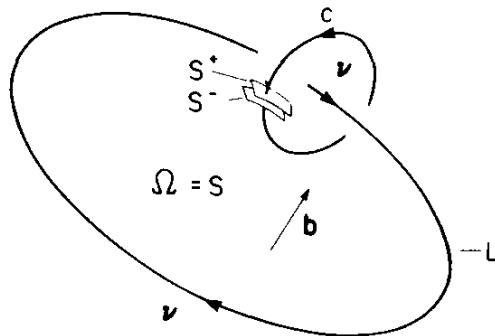


Fig. 6.1 The Dundurs constants diagram

straight line $\alpha = \beta$. Representation of stress concentration factors for a composite in terms of α and β is found in the paper by Gerstner and Dundurs (1969).

7. Dislocations

The mathematical theory of dislocations in elastic continua was first systematically studied by Volterra (1907) and Weingarten (1901). Taylor (1934), Orowan (1934) and Polanyi (1934) independently used dislocations to explain plastic deformation of single crystals. They considered dislocations as imperfections in crystals and explained why observed yield stresses of crystals are much lower than the theoretical values calculated from atomic theory on the basis of perfect-lattice state. The existence of dislocations was first confirmed directly by Hirsch, Howie, and Whelan (1960) by transmission electron microscopy (indirect observations by etch pits, etc., had been done earlier—see Hirth and Lothe 1982). The direct observation of dislocations has been extensively made (see Amelinckx 1964), and the dislocation theory has been developed by many scientists to explain not only the mechanical, but also the optical and electromagnetic properties of crystals (see Nabarro 1967, 1979).

Fig 7.1 Dislocation L and the Burgers circuit c

As already explained by Figs. 4.1 and 4.2 in Section 4, the dislocation (line) is defined as a part of the boundary of a slip plane, which is embedded in a material. The part of the slip boundary which is exposed on the surface of the material is not called the dislocation line. To define the direction of a dislocation line in a more precise manner, consider a slip plane S inside a material as shown in Fig. 7.1. It is assumed that, for instance, the upper plane (denoted by S^+) slips by \mathbf{b} relative to the lower plane (denoted by S^-). In order to specify this configuration of the slip, we define the direction ν of the dislocation line L in the following manner: Go around a linking circuit c (the Burgers circuit) in the direction of rotation of a right-handed screw advancing along the direction of the dislocation. The surface S^+ that is displaced by \mathbf{b} (the Burgers vector) relative to the other surface S^- is designated as the surface on which the end point of the Burgers circuit is located; the circuit does not cross the boundary between S^+ and S^- , as shown in Fig. 7.1.

For a crystal, the Burgers vector of a dislocation is usually a lattice vector. Such a dislocation is called a perfect dislocation. A dislocation which has the Burgers vector not equal to a lattice vector is called an imperfect or a partial dislocation. The slip deformation in crystal plasticity is caused by the movement and generation of dislocations, and is generally irreversible due to the atomic potential barrier. Due to the singularity along the dislocation line, an elastic field is introduced in the material. This dislocation state of the material is simulated by an eigenstress field caused by eigenstrain ϵ_{ij}^* , as discussed in Section 4.

Volterra and Mura formulas

According to Kröner (1958), the displacement gradient $u_{i,j}$ (total distortion) is assumed to consist of elastic distortion β_{ji} and plastic distortion β_{ji}^* ,

$$u_{i,j} = \beta_{ji} + \beta_{ji}^*. \quad (7.1)$$

The (total) strain ϵ_{ij} , elastic strain e_{ij} , and eigenstrain ϵ_{ij}^* defined in (2.1) ~ (2.4) are

$$\begin{aligned}\epsilon_{ij} &= \frac{1}{2}(u_{i,j} + u_{j,i}), \\ e_{ij} &= \frac{1}{2}(\beta_{ij} + \beta_{ji}), \\ \epsilon_{ij}^* &= \frac{1}{2}(\beta_{ij}^* + \beta_{ji}^*).\end{aligned}\tag{7.2}$$

Since β_{ji}^* is caused by the slip b_i of plane S^+ whose normal vector toward S^- is n_j , we can write

$$\beta_{ji}^*(x) = -b_i n_j \delta(S - x)\tag{7.3}$$

where $\delta(S - x)$ is the one-dimensional Dirac delta function in the normal direction of S , being unbounded when x is on S and zero otherwise. As in (4.1) and (4.11) we write

$$\epsilon_{ij}^*(x) = -\frac{1}{2}(b_i n_j + b_j n_i) \delta(S - x).\tag{7.4}$$

When this expression is substituted into (6.1), and since

$$\int_{\Omega} \delta(S - x') dx' = \int_S dS,\tag{7.5}$$

then we have the Volterra formula (1907),

$$u_i(x) = \int_S C_{jlmn} b_m G_{ij,l}(x - x') n_n dS(x').\tag{7.6}$$

Differentiating (7.6) we have

$$u_{i,j}(x) = \int_S C_{klmn} b_m G_{ik,lj}(x - x') n_n dS(x').\tag{7.7}$$

The elastic distortion is obtained from (7.1) as

$$\beta_{ji}(x) = \int_S C_{klmn} b_m G_{ik,lj}(x - x') n_n dS(x') + b_i n_j \delta(S - x).\tag{7.8}$$

By integrating (7.8), the Mura formula (1963) is obtained as

$$\beta_{ji}(\mathbf{x}) = \int_L \epsilon_{jnh} C_{pqmn} G_{ip,q}(\mathbf{x} - \mathbf{x}') b_m v_h \, dl(\mathbf{x}'), \quad (7.9)$$

where v is the direction of the dislocation line L and dl is the dislocation line element. The equivalence of (7.8) and (7.9) is easily proved by the use of Stokes' theorem,

$$\int_L f v_h \, dl = \int_S \epsilon_{klh} f_{,l} n_k \, dS. \quad (7.10)$$

Applying (7.10) to (7.9) we have

$$\beta_{ji}(\mathbf{x}) = - \int_S \epsilon_{klh} \epsilon_{jnh} C_{pqmn} G_{ip,ql}(\mathbf{x} - \mathbf{x}') b_m n_k \, dS(\mathbf{x}'). \quad (7.11)$$

The minus sign comes from $(\partial/\partial x'_l) G_{ip,q} = -G_{ip,ql}$.

Since $\epsilon_{klh} \epsilon_{jnh} = \delta_{kj} \delta_{ln} - \delta_{kn} \delta_{lj}$,

$$\beta_{ji}(\mathbf{x}) = - \int_S (C_{pqmn} G_{ip,qn} b_m n_j - C_{pqmn} G_{ip,qj} b_m n_n) \, dS. \quad (7.12)$$

From (5.2), $C_{pqmn} G_{ip,qn}(\mathbf{x} - \mathbf{x}') = -\delta_{mi} \delta(\mathbf{x} - \mathbf{x}')$; therefore the first term in (7.12) is written as

$$\int_S \delta(\mathbf{x} - \mathbf{x}') b_i n_j \, dS(\mathbf{x}') = b_i n_j \delta(\mathbf{S} - \mathbf{x}). \quad (7.13)$$

The two Dirac delta functions in (7.13) are different. $\delta(\mathbf{x} - \mathbf{x}')$ is the three-dimensional delta function defined by (5.6), and $\delta(\mathbf{S} - \mathbf{x})$ is the one-dimensional delta function in the direction normal to S . Equation $\int_S \delta(\mathbf{x} - \mathbf{x}') \, dS(\mathbf{x}') = \delta(\mathbf{S} - \mathbf{x})$ can be regarded as the defining equation for $\delta(\mathbf{S} - \mathbf{x})$. Now it can be seen that (7.12) equals (7.8). Systematic derivations of (7.9) are presented by Willis (1967) and Teodosiu (1970).

The stress components are

$$\sigma_{ij} = C_{ijk} \beta_{lk}, \quad (7.14)$$

or, from (7.9),

$$\sigma_{ij}(\mathbf{x}) = C_{ijk} \int_L \epsilon_{lnh} C_{pqmn} G_{kp,q}(\mathbf{x} - \mathbf{x}') b_m v_h \, dl(\mathbf{x}'). \quad (7.15)$$

The fact that the elastic distortion and stress fields can be expressed by line integrals along the dislocation line proves that these fields depend only on the boundary of S . On the other hand, the displacement field cannot be expressed by such a line integral, which means that the displacement field cannot be determined uniquely by only L . It can, however, be determined by knowing S , as shown in (7.6). If a history of the origin of dislocation (i.e., how it has been created) is given, the displacement field can also be expressed by a line integral along the dislocation line (see Section 38). Giving S and \mathbf{b} for a dislocation is equivalent to giving a history of creation of the dislocation line.

The Fourier integral expression for (7.9) is

$$\begin{aligned} \beta_{ji}(\mathbf{x}) = & \frac{i}{(2\pi)^3} \int_L \nu_h \, dl(\mathbf{x}') \int_{-\infty}^{\infty} \epsilon_{jnh} C_{pqmn} \xi_q N_{ip}(\xi) D^{-1}(\xi) b_m \\ & \times \exp\{i\xi \cdot (\mathbf{x} - \mathbf{x}')\} \, d\xi \end{aligned} \quad (7.16)$$

which will be useful for further calculations (see Section 38).

It should be mentioned that $u_{i,j}(\mathbf{x}) = \beta_{ji}(\mathbf{x})$ when point \mathbf{x} is not on S , and therefore (7.9) or (7.16) can be regarded as expressions for $u_{i,j}(\mathbf{x})$ at such a point.

Equation (7.6) holds even when \mathbf{b} is a function of \mathbf{x} . Such a \mathbf{b} is called the *Somigliana dislocation*. The Somigliana dislocation is useful for describing some interfacial defects as seen in the papers by Asaro (1975), Bonnet, Marcon, and Ati (1985), and Mura and Taya (1985) and in Section 51.

* The Indenbom and Orlov formula

The line integral expression for $\beta_{ji}(\mathbf{x})$ or $\sigma_{ij}(\mathbf{x})$ as shown in (7.9) or (7.15) is not unique. An alternate expression was obtained by Indenbom and Orlov (1968),

$$\beta_{ji}(\mathbf{x}) = \frac{1}{2} \int_L \nu_p \nu_q \frac{\partial^2}{\partial x_p \partial x_q} \beta_{ji}(\nu; \mathbf{t}) \, dl(\mathbf{x}'), \quad (7.17)$$

where

$$\begin{aligned} \beta_{ji}(\nu; \mathbf{t}) = & \frac{i}{(2\pi)^3} \int_{-\infty}^{\infty} t_h \, ds \int_{-\infty}^{\infty} \epsilon_{jnh} C_{klmn} \xi_l N_{ik}(\xi) D^{-1}(\xi) b_m \\ & \times \exp\{i\xi \cdot (\nu - ts)\} \, d\xi, \end{aligned} \quad (7.18)$$

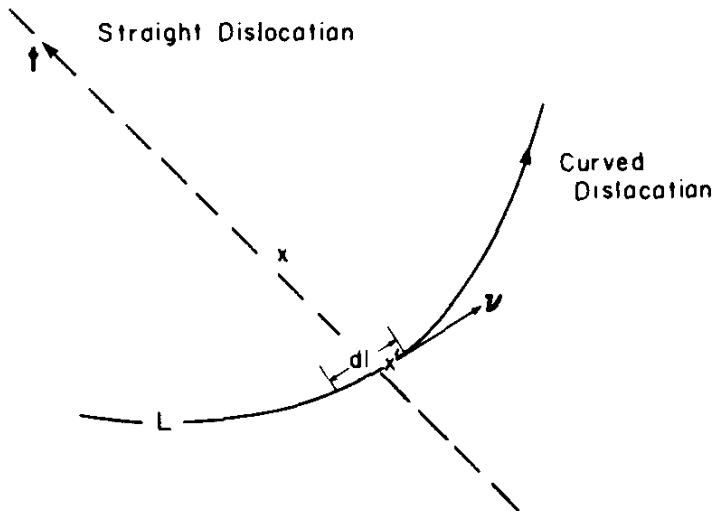


Fig. 7.2 Explanation of notations appearing in the Indenbom and Orlov formula

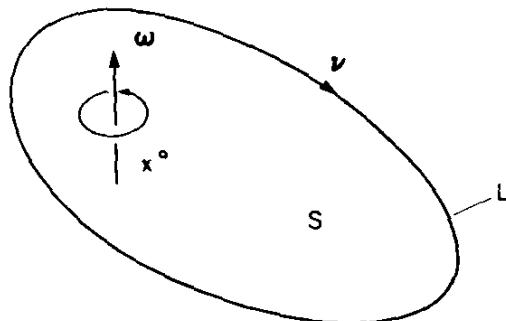
and ν is the unit tangent to L at x' . The unit vector along $x'x$ is denoted by t .

It can be seen from (7.16) that $\beta_{ji}(\nu; t)$ is the elastic distortion at the end point of vector ν caused by a straight, infinitely long dislocation connecting x and x' points (Fig. 7.2).

The interesting point of expression (7.17) is that the elastic stress field of curved dislocation line L can be expressed in terms of the stress field of a straight infinite dislocation. However, the appearance of the second derivatives in the integrand makes (7.17) less attractive unless it is modified as done by Asaro and Barnett (1976). This is discussed in Section 36. The equivalence of (7.17) and (7.9) has been proved by Asaro and Barnett (1976). Further modifications of (7.17) have been developed by Lothe (1960), Brown (1967), Asaro et al. (1973), Bacon, Barnett and Scattergood (1978) among others.

* Disclinations

Volterra (1907) considered two types of dislocations: one is translational and the other one is rotational. The translational dislocation is the dislocation shown by Fig. 7.1. The rotational dislocation is called disclination and is shown in Fig. 7.3. The name "disclination" was coined by Frank (1958) in his study of cholesteric liquid crystals. Liquid crystal disclinations have been observed as twisting discontinuities, allowing discrete jumps of one half-pitch of the helicoidal texture (e.g., Orsay Liquid Crystal Group 1969, Friedel and Kléman 1969, Robinson, Ward and Beevers 1958, Cladis, Kléman and Pieranski 1971, Williams 1975). Träuble and Essmann (1968) have made direct observations of disclinations in the lattice of flux lines in type II superconductors and have shown that disclinations cause strong elastic distortions and lattice bendings. Nabarro (1967, 1969) and Harris (1969) have extended the

Fig 7.3 Disclination L

concept of the disclination to surface defects and suggested possible applications to biological structures such as some plasma membranes, virus capsids and insect muscle (see also Harris and Scriven 1970, Harris and Thomas 1975, Chou and Ichikawa 1979, and Ichikawa and Chou 1979). Disclinations are also created by multiple twinning and have been observed in FCC crystals (Smith and Bechtoldt 1968). Li and Gilman (1970) have shown that disclinations are caused in polymers by chain kinking and twisting of molecules and that they play an important role in plastic deformation, internal friction, and glass transition. Recent developments of disclinations are seen in the monographs by Kléman (1980, 1983) and Bouligand (1980).

The disclination (line) L in Fig. 7.3 is created by twisting surface S^+ against surface S^- by rotation angle ω at point x^0 . Surfaces S^+ and S^- are defined by the direction ν of the disclination line and the advancing rule of the right-handed screw, as employed for the definition of dislocations (see Fig. 7.1). Surface S^+ will be denoted by S . When vector ω is normal to S , the disclination is of a twist type, and it is of a wedge type when ω is on S .

The elastic field caused by a disclination is simulated by the eigenstress field caused by a suitable eigenstrain ϵ_{ij}^* distributed on S . Since the jump in the displacement components across S is

$$[u_i(x)] = \epsilon_{ipq}\omega_p(x_q - x_q^0), \quad (7.19)$$

the plastic distortion on S is defined by

$$\beta_{ji}^*(x) = -\epsilon_{ipq}\omega_p(x_q - x_q^0)n_j\delta(S - x) \quad (7.20)$$

which is similar to the definition (7.3). The corresponding eigenstrain is defined by (7.2). When (7.20) is substituted into (6.1), where $\Omega = S$, we have

$$u_i(x) = \int_S C_{jlmn}\epsilon_{mpq}\omega_p(x'_q - x_q^0)G_{ij,l}(x - x')n_n dS(x') \quad (7.21)$$

and

$$u_{i,j}(x) = \int_S C_{klmn} \epsilon_{mpq} \omega_p (x'_q - x_q^0) G_{ik,lj}(x - x') n_n dS(x'). \quad (7.22)$$

$u_{i,j} - \beta_{ji}^* = \beta_{ji}$ cannot be fully expressed by a line integral along L as done in (7.9); however, the symmetric part, $\beta_{(ji)} \equiv \frac{1}{2}(\beta_{ji} + \beta_{ij})$, can be expressed by the line integral,

$$\begin{aligned} \beta_{(ji)} &= \int_L (\epsilon_{jnh} G_{ik,l}(x - x'))^s C_{klmn} \epsilon_{mpq} \omega_p (x'_q - x_q^0) v_n dl(x') \\ &\quad - \int_L (\epsilon_{jnh} G_{iklq}^*(x - x'))^s C_{klmn} \epsilon_{mpq} \omega_h v_p dl(x'), \end{aligned} \quad (7.23)$$

where

$$G_{iklq}^*(x) = -(2\pi)^{-3} \int_{-\infty}^{\infty} N_{ik}(\xi) D^{-1}(\xi) \xi_l \xi_q \xi^{-2} \exp(i\xi \cdot x) d\xi, \quad (7.24)$$

and $[]^s$ is the symmetric part of $[]$ with respect to i, j .

Mura (1972) found that

$$\begin{aligned} u_{i,ja}(x) &= \int_L \epsilon_{jnh} C_{klmn} G_{ik,la}(x - x') \epsilon_{mpq} \omega_p (x'_q - x_q^0) v_h dl(x') \\ &\quad - \int_L \epsilon_{jnh} C_{klmn} G_{ik,l}(x - x') \epsilon_{mab} \omega_h v_b dl(x') \\ &\quad + \beta_{ji,a}^*(x) + \epsilon_{jih} \phi_{ah}^*(x), \end{aligned} \quad (7.25)$$

where

$$\phi_{ah}^*(x) = -\omega_h n_a \delta(S - x). \quad (7.26)$$

ϕ_{ah}^* is called the plastic rotation. Contrary to the dislocation case, $u_{i,j}$ cannot be expressed by the line integral along L . One more derivative of $u_{i,j}$ is required. Quantities expressed by line integrals along L can be called state quantities since in a dislocated state of crystals the only measurable (visible) line imperfections are lines L of dislocations or disclinations. It can be said from (7.23) and (7.25) that $\beta_{(ji)}$ and $u_{i,ja} - \beta_{ji,a}^* - \epsilon_{jih} \phi_{ah}^*$ are state quantities.

In the dislocation case, $u_{i,j} - \beta_{ji}^*$ is a state quantity. A derivation similar to (7.25) is presented by Kossecka and deWit (1977).

Formula (7.25) is useful for the derivation of the dislocation density tensor, α_{si} . Since $u_{i,ja} = u_{i,aj}$, it holds that

$$\epsilon_{sja} u_{i,ja} = 0. \quad (7.27)$$

When (7.25) is substituted into (7.27), we have

$$\begin{aligned} & \int_L C_{klma} G_{ik,la} \epsilon_{mpq} \omega_p (x'_q - x_q^0) dx'_s - \int_L C_{klms} G_{ik,la} \epsilon_{mpq} \omega_p (x'_q - x_q^0) dx'_a \\ & - \int_L C_{klma} G_{ik,l} \epsilon_{mab} \omega_s dx'_b + \int_L C_{klms} G_{ik,l} \epsilon_{mab} \omega_a dx'_b \\ & + \epsilon_{sja} \beta_{ji,a}^* + \epsilon_{sja} \epsilon_{jih} \phi_{ah}^* = 0, \end{aligned} \quad (7.28)$$

where $dx'_s = v_s dl$. The first term becomes $-\int_L \delta_{mi} \delta(x - x') \epsilon_{mpq} \omega_p (x'_q - x_q^0) dx'_s$, where the property of Green's function is used. The second term becomes $-\int_L C_{klms} G_{ik,l} \epsilon_{mpq} \omega_p \delta_{qa} dx'_a$ by integrating by parts, and it cancels out with the fourth term. The third term is zero since $C_{klma} \epsilon_{mab} = 0$. If we define

$$\int_L \delta(x - x') dl(x') = \delta(\mathbf{L} - \mathbf{x}) \quad (7.29)$$

as in (7.13), equation (7.28) can be written as

$$-\epsilon_{ipq} \omega_p (x_q - x_q^0) v_s \delta(\mathbf{L} - \mathbf{x}) + \epsilon_{sja} \beta_{ji,a}^* + \epsilon_{sja} \epsilon_{jih} \phi_{ah}^* = 0 \quad (7.30)$$

or

$$\alpha_{si} = -\epsilon_{sa} \beta_{ji,a}^* - \epsilon_{sa} \epsilon_{jih} \phi_{ah}^*, \quad (7.31)$$

where

$$\alpha_{si} = \epsilon_{ipq} \omega_p (x_q - x_q^0) v_s \delta(\mathbf{L} - \mathbf{x}). \quad (7.32)$$

Equation (7.31) states that the disclination line can be expressed in terms of the plastic distortion and rotation. Suppose that a crystal is distorted by many disclination lines. The smearing-out of the density of disclination lines may be expressed in the same form as (7.31) with continuous functions of tensors, α_{si} ,

β_{ji}^* , and ϕ_{ah}^* . α_{si} is called the dislocation density tensor. The dislocation theory can be included in the theory of disclinations because $-\epsilon_{ipq}\omega_p x_q^0$ in (7.19) is a constant and can be replaced by the Burgers vector b_i . The dislocation theory is a special case where $\phi_{ah}^* = 0$. Then α_{si} becomes Nye's (1953) dislocation density tensor, and we have

$$\alpha_{si} = -\epsilon_{saj}\beta_{ji,a}^*. \quad (7.33)$$

The above equation was first obtained by Kröner (1955, 1956). In Section 37, (7.33) is derived by a different method.

The divergence of (7.31) leads to

$$\alpha_{si,s} = -\epsilon_{ijh}\theta_{jh}, \quad (7.34)$$

where

$$\theta_{jh} = -\epsilon_{saj}\phi_{ah,s}^*. \quad (7.35)$$

Anthony et al. (1968) first found (7.34), and they defined θ_{ij} as the disclination density tensor. In dislocation theory the relation $\alpha_{hi,h} = 0$ is well established. It has the form of a continuity equation and has been interpreted to mean that dislocations cannot end inside the medium. DeWit (1969, 1971) pointed out that (7.34) has the form of a continuity equation with a source or sink and that dislocations can start or end on disclinations (see also Das et al. 1973, Das, Marcinkowski, and Armstrong 1973, and Marukawa 1974). The recent development of generalized continua (e.g., Kröner 1968) has stimulated the study of moments, transport law of a motor (Schaefer 1968), the curvature tensor of Cosserat continua, and the disclination density tensor (e.g., deWit 1972, 1973, Anthony 1970, Kröner and Anthony 1975, and Günther 1972). Elastic stress fields and strain energies of disclinations have been investigated by Huang and Mura (1970, 1972), Chou (1971), Liu and Li (1971), Kuo and Mura (1972, 1973), Kuo, Mura, and Dundurs (1973), and Minagawa (1977).

8. Dynamic solutions

The eigenstrain problems discussed in Section 3 are easily extended to dynamic problems.

Consider an elastic homogeneous material which is infinitely extended. The

problem is to find the elastic field when prescribed eigenstrains ϵ_{ij}^* are given as functions of space and time. ϵ_{ij}^* is given by the Fourier integrals as

$$\begin{aligned}\epsilon_{ij}^*(x, t) &= \iint_{-\infty}^{\infty} \bar{\epsilon}_{ij}^*(\xi, \omega) \exp\{i(\xi \cdot x - \omega t)\} d\xi d\omega, \\ \bar{\epsilon}_{ij}^*(\xi, \omega) &= (2\pi)^{-4} \iint_{-\infty}^{\infty} \epsilon_{ij}^*(x, t) \exp\{-i(\xi \cdot x - \omega t)\} dx dt.\end{aligned}\quad (8.1)$$

The equations of motion are

$$\sigma_{ij,j} = \rho \ddot{u}_i, \quad (8.2)$$

where ρ is the density of the material. The stress-strain relations are

$$\sigma_{ij} = C_{ijkl}(u_{k,l} - \epsilon_{kl}^*). \quad (8.3)$$

We assume the solution of (8.2) to be

$$u_i(x, t) = \iint_{-\infty}^{\infty} \bar{u}_i(\xi, \omega) \exp\{i(\xi \cdot x - \omega t)\} d\xi d\omega. \quad (8.4)$$

Substitution of these equations into (8.2) leads to

$$C_{ijkl} \bar{u}_k \xi_l \xi_j - \rho \bar{u}_i \omega^2 = -i C_{ijkl} \bar{\epsilon}_{kl}^* \xi_j. \quad (8.5)$$

By using the notations

$$\begin{aligned}K_{ik} &= C_{ijkl} \xi_j \xi_l, \\ X_i &= -i C_{ijkl} \bar{\epsilon}_{kl}^* \xi_j,\end{aligned}\quad (8.6)$$

\bar{u}_i is obtained from (8.5) as

$$\bar{u}_i(\xi, \omega) = X_j N_{ji}(\xi, \omega) / D(\xi, \omega), \quad (8.7)$$

where N_{ij} and D are the cofactor and the determinant of the matrix

$$K(\xi, \omega) = \begin{pmatrix} K_{11} - \rho \omega^2 & K_{12} & K_{13} \\ K_{21} & K_{22} - \rho \omega^2 & K_{23} \\ K_{31} & K_{32} & K_{33} - \rho \omega^2 \end{pmatrix}. \quad (8.8)$$

Now the solution can be written as

$$u_i(x, t) = - \iint_{-\infty}^{\infty} iC_{jlmn} \tilde{\epsilon}_{mn}^*(\xi, \omega) \xi_l N_{ij}(\xi, \omega) D^{-1}(\xi, \omega) \\ \times \exp\{i(\xi \cdot x - \omega t)\} d\xi d\omega. \quad (8.9)$$

If we define

$$G_{ij}(x, t) = (2\pi)^{-4} \iint_{-\infty}^{\infty} N_{ij}(\xi, \omega) D^{-1}(\xi, \omega) \exp\{i(\xi \cdot x - \omega t)\} d\xi d\omega, \quad (8.10)$$

(8.9) becomes

$$u_i(x, t) = - \iint_{-\infty}^{\infty} C_{jlmn} \epsilon_{mn}^*(x', t') G_{ij,l}(x - x', t - t') dx' dt', \quad (8.11)$$

where the second equation in (8.1) has been used. $G_{ij}(x, t)$ defined by (8.10) is called the dynamic Green's function. Willis (1965) used (8.11) for a body which spontaneously tends to undergo an eigenstrain ϵ_{mn}^* .

Uniformly moving edge dislocation

The plastic distortion of a uniformly moving edge dislocation with velocity v_1 in the x_1 -direction is described in a form similar to (4.11),

$$\beta_{21}^*(x, t) = b_1 \delta(x_2) H(v_1 t - x_1), \quad (8.12)$$

where H is the Heaviside step function and $\epsilon_{12}^* = \epsilon_{21}^* = \frac{1}{2}\beta_{21}^*$. The Fourier transform of (8.12) becomes, from (8.1),

$$\bar{\beta}_{21}^* = -b_1 \delta(\xi_3) \delta(\xi_1 v_1 + \omega) / (2\pi)^2 i \xi_1 \quad (8.13)$$

and $\tilde{\epsilon}_{21}^* = \tilde{\epsilon}_{12}^* = \frac{1}{2}\bar{\beta}_{21}^*$. Assume that the material is isotropic. Then, (8.9) becomes, by use of (8.13),

$$u_i(x, t) = \iint_{-\infty}^{\infty} \frac{b\mu}{(2\pi)^2 \xi_1} \frac{\xi_1 N_{i2}(\xi_1, \xi_2, 0; -\xi_1 v_1) + \xi_2 N_{i1}(\xi_1, \xi_2, 0; -\xi_1 v_1)}{D(\xi_1, \xi_2, 0; -\xi_1 v_1)} \\ \times \exp[i\{\xi_1(x_1 - v_1 t) + \xi_2 x_2\}] d\xi_1 d\xi_2. \quad (8.14)$$

Therefore

$$\begin{aligned}
 u_1 &= \frac{b_1}{(2\pi)^2} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \frac{\xi_2}{\xi_1} \left[\frac{2c_1^2}{v_1^2} \left\{ \xi_1^2 \left(1 - v_1^2/c_2^2 \right) + \xi_2^2 \right\}^{-1} \right. \\
 &\quad + \left(1 - \frac{2c_1^2}{v_1^2} \right) \left\{ \xi_1^2 \left(1 - \frac{v_1^2}{c_1^2} \right) + \xi_2^2 \right\}^{-1} \\
 &\quad \times \exp[i\{\xi_1(x_1 - v_1 t) + \xi_2 x_2\}] \Big] d\xi_1 d\xi_2, \\
 u_2 &= \frac{b_1}{(2\pi)^2} \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \left[\frac{2c_1^2}{v_1^2} \left(\frac{v_1^2}{c_2^2} - 1 \right) \left\{ \xi_1^2 \left(1 - \frac{v_1^2}{c_2^2} \right) + \xi_2^2 \right\}^{-1} \right. \\
 &\quad + \left(\frac{2c_1^2}{v_1^2} - 1 \right) \left\{ \xi_1^2 \left(1 - \frac{v_1^2}{c_1^2} \right) + \xi_2^2 \right\}^{-1} \\
 &\quad \times \exp[i\{\xi_1(x_1 - v_1 t) + \xi_2 x_2\}] \Big] d\xi_1 d\xi_2,
 \end{aligned} \tag{8.15}$$

$$u_3 = 0,$$

where $c_1 = (\mu/\rho)^{1/2}$ and $c_2 = \{(\lambda + 2\mu)/\rho\}^{1/2}$ are the shear and the dilatational wave velocities. The integrations in (8.15) are easily performed by use of (4.7). Then,

$$\begin{aligned}
 u_1 &= \frac{b_1 c_1^2}{\pi v_1^2} \left\{ \tan^{-1} \frac{x_2 (1 - v_1^2/c_1^2)^{1/2}}{x_1 - v_1 t} + \left(\frac{v_1^2}{2c_1^2} - 1 \right) \tan^{-1} \frac{x_2 (1 - v_1^2/c_1^2)^{1/2}}{x_1 - v_1 t} \right\}, \\
 u_2 &= \frac{b_1 c_1^2}{\pi v_1^2} \left[- \frac{1 - v_1^2/2c_1^2}{(1 - v_1^2/c_1^2)^{1/2}} \log \left\{ \frac{(x_1 - v_1 t)^2}{1 - v_1^2/c_1^2} + x_2^2 \right\}^{1/2} \right. \\
 &\quad \left. + (1 - v_1^2/c_2^2)^{1/2} \log \left\{ \frac{(x_1 - v_1 t)^2}{1 - v_1^2/c_2^2} + x_2^2 \right\}^{1/2} \right],
 \end{aligned} \tag{8.16}$$

$$u_3 = 0.$$

The above result was first obtained by Eshelby (1949).

Uniformly moving screw dislocation

The plastic distortion of a uniformly moving screw dislocation with velocity v_1 in the x_1 -direction is

$$\beta_{23}^*(x, t) = b_3 \delta(x_2) H(v_1 t - x_1) \quad (8.17)$$

where $\epsilon_{23}^* = \epsilon_{32}^* = \frac{1}{2} \beta_{23}^*$. In the same way the reader can easily obtain

$$u_1 = 0, \quad u_2 = 0,$$

$$u_3 = \frac{b_3}{2\pi} \tan^{-1} \frac{x_2 (1 - v_1^2/c_1^2)^{1/2}}{x_1 - v_1 t} \quad (8.18)$$

which is the well-known solution found by Frank (1949).

*** 9. Dynamic Green's functions**

The dynamic Green's function defined by (8.10) is also defined as the solution of the equation of motion

$$C_{ijkl} G_{km,lj}(x, t) + \delta_{im} \delta(x) \delta(t) = \rho \ddot{G}_{im}(x, t). \quad (9.1)$$

$G_{ij}(x, t)$ is the elastic displacement component in the x_i -direction at point x and time t , produced by a unit impulsive force applied in the x_j -direction at point $x = 0$ and time $t = 0$. G_{ij} is a symmetric tensor, and therefore the x_i - and x_j -directions in the above explanation can be interchanged. The solution of (9.1) is

$$G_{ij}(x, t) = (2\pi)^{-4} \iint_{-\infty}^{\infty} N_{ij}(\xi, \omega) D^{-1}(\xi, \omega) \exp\{i(\xi \cdot x - \omega t)\} d\xi d\omega, \quad (9.2)$$

where N_{ij} and D are the cofactor and the determinant of the matrix as defined by (8.8).

It is easy to show that (9.2) is the solution of (9.1). Write

$$G_{ij}(x, t) = (2\pi)^{-4} \iint_{-\infty}^{\infty} \bar{G}_{ij}(\xi, \omega) \exp\{i(\xi \cdot x - \omega t)\} d\xi d\omega \quad (9.3)$$

and substitute it into (9.1). Then,

$$(K_{ik} - \rho\omega^2\delta_{ik})\bar{G}_{km} = \delta_{im}, \quad (9.4)$$

where

$$\delta(x)\delta(t) = (2\pi)^{-4} \iint_{-\infty}^{\infty} \exp\{i(\xi \cdot x - \omega t)\} d\xi d\omega \quad (9.5)$$

is used. The solution of (9.4) is

$$\bar{G}_{ij}(\xi, \omega) = N_{ij}(\xi, \omega)/D(\xi, \omega). \quad (9.6)$$

The explicit expressions for D and N_{ij} are

$$\begin{aligned} D(\xi, \omega) &= -(\rho\omega^2)^3 + (\rho\omega^2)^2 A - \rho\omega^2 B + C, \\ N_{ij}(\xi, \omega) &= K_{im}K_{mj} + (\rho\omega^2 - A)K_{ij} + \{(\rho\omega^2)^2 - \rho\omega^2 A + B\}\delta_{ij}, \end{aligned} \quad (9.7)$$

where

$$A = K_{mm},$$

$$B = \epsilon_{mn1}K_{m2}K_{n3} + \epsilon_{mn2}K_{m3}K_{n1} + \epsilon_{mn3}K_{m1}K_{n2}, \quad (9.8)$$

$$C = \epsilon_{mnl}K_{m1}K_{n2}K_{l3}.$$

The roots of $D(\xi, \omega) = 0$ with respect to $\rho\omega^2$ are denoted by $\rho\omega_1^2$, $\rho\omega_2^2$ and $\rho\omega_3^2$. Then we can write

$$D(\xi, \omega) = (\rho\omega_1^2 - \rho\omega^2)(\rho\omega_2^2 - \rho\omega^2)(\rho\omega_3^2 - \rho\omega^2), \quad (9.9)$$

and therefore,

$$\begin{aligned} A &= \rho\omega_1^2 + \rho\omega_2^2 + \rho\omega_3^2, \\ B &= \rho^2\omega_1^2\omega_2^2 + \rho^2\omega_2^2\omega_3^2 + \rho^2\omega_3^2\omega_1^2, \\ C &= \rho^3\omega_1^2\omega_2^2\omega_3^2. \end{aligned} \quad (9.10)$$

It should be noted that $D(\xi, \omega)$ becomes $D(\xi)$ when $\omega = 0$ and $D(\xi)$ is positive due to (9.10). \bar{G}_{ij} can then be written as

$$\bar{G}_{ij}(\xi, \omega) = \sum_{\nu=1}^3 \frac{\phi_i \phi_j}{\rho \omega_\nu^2 - \rho \omega^2}, \quad (9.11)$$

where

$$\phi_i \phi_j = \frac{(K_{im} - \rho \omega_2^2 \delta_{im})(K_{mj} - \rho \omega_3^2 \delta_{mj})}{(\rho \omega_1^2 - \rho \omega_2^2)(\rho \omega_1^2 - \rho \omega_3^2)} \quad (9.12)$$

and $\phi_i \phi_j$, $\phi_i \phi_j$ are obtained by cyclic permutation of (1, 2, 3). ϕ_i are eigenvectors of (8.8), and $\rho \omega_\nu$ are the corresponding eigenvalues,

$$\begin{aligned} (K_{ij} - \rho \omega_\nu^2 \delta_{ij}) \phi_j &= 0, \\ \sum_{\nu=1}^3 \phi_i \phi_j &\doteq \delta_{ij}. \end{aligned} \quad (9.13)$$

It should be noted that $\phi_i \phi_j$ are homogeneous functions of ξ of degree 0, since $\rho \omega_\nu^2$ are homogeneous functions of ξ of degree 2, that is,

$$\begin{aligned} \rho \omega_\nu^2 &= \xi^2 \rho \bar{\omega}_\nu^2, \\ \xi &= \xi \bar{\xi}. \end{aligned} \quad (9.14)$$

The integral (9.3) can be written as

$$\begin{aligned} G_{ij}(x, t) &= \frac{1}{(2\pi)^4 \rho} \int_{-\infty}^{\infty} \sum_{\nu=1}^3 \frac{\exp(-i\omega t)}{\omega_\nu^2 - \omega^2} d\omega \int_0^\infty \xi^2 d\xi \\ &\times \int_{S^2} \phi_i(\xi) \phi_j(\xi) \exp(i\xi \bar{\xi} \cdot x) dS(\xi) \end{aligned} \quad (9.15)$$

where $\xi = \xi \bar{\xi}$ and S^2 is the unit sphere $|\xi| = 1$. To integrate with respect to ω , one uses the Cauchy integral theorem. Since matrix (K_{ij}) is positive definite, the poles ω_ν are located on the real axes. For $t > 0$, Γ is the semi-infinite half-circle in the lower half-plane, and for $t < 0$, Γ is the semi-infinite half-circle in the upper half-plane. With this Γ , the initial condition $G_{ij} = 0$ for

$t < 0$ is satisfied. Thus, we have

$$G_{ij}(x, t) = \frac{H(t)}{(2\pi)^3 \rho} \sum_{\nu=1}^3 \int_0^\infty \xi^2 d\xi \frac{\sin(\omega_\nu t)}{\omega_\nu} \int_{S^2} \phi_i(\bar{\xi}) \phi_j(\bar{\xi}) \exp(i\bar{\xi} \cdot x) dS(\bar{\xi}), \quad (9.16)$$

where $H(t)$ is the Heaviside step function. We introduce an auxiliary function $F_{ij}(x, t)$ defined as

$$G_{ij}(x, t) = -\frac{\partial^2}{\partial x_k \partial x_k} F_{ij}(x, t). \quad (9.17)$$

Then,

$$F_{ij}(x, t) = \frac{H(t)}{(2\pi)^3 \rho} \sum_{\nu=1}^3 \int_0^\infty d\xi \int_{S^2} \phi_i(\bar{\xi}) \phi_j(\bar{\xi}) \frac{\sin(\omega_\nu t)}{\omega_\nu} \exp(i\bar{\xi} \cdot x) dS(\bar{\xi}). \quad (9.18)$$

Since the factor of $\exp(i\bar{\xi} \cdot x)$ in the integrand of (9.18) is an even function of $\bar{\xi}$, it can be written as

$$F_{ij}(x, t) = \frac{H(t)}{(2\pi)^3 \rho} \sum_{\nu=1}^3 \int_0^\infty d\xi \frac{\sin(\bar{\omega}_\nu t \xi)}{\bar{\omega}_\nu \xi} \cos(\bar{\xi} \cdot x) \int_{S^2} \phi_i(\bar{\xi}) \phi_j(\bar{\xi}) dS(\bar{\xi}), \quad (9.19)$$

where $\omega_\nu = \xi \bar{\omega}_\nu$. By using

$$\int_0^\infty \frac{\sin mx \cos nx}{2} dx = \frac{1}{4}\pi \{ \operatorname{sgn}(m+n) + \operatorname{sgn}(m-n) \} \quad \text{for } m, n > 0, \quad (9.20)$$

we have

$$F_{ij}(x, t) = \frac{H(t)}{(2\pi)^3 \rho} \sum_{\nu=1}^3 \int_{S^2} \frac{1}{4}\pi \{ \operatorname{sgn}(\bar{\omega}_\nu t + \bar{\xi} \cdot x) + \operatorname{sgn}(\bar{\omega}_\nu t - \bar{\xi} \cdot x) \} \times \frac{\phi_i(\bar{\xi}) \phi_j(\bar{\xi})}{\bar{\omega}_\nu} dS(\bar{\xi}). \quad (9.21)$$

Since

$$\frac{d}{du}(\operatorname{sgn} u) = 2\delta(u), \quad (9.22)$$

(9.17) becomes

$$G_{ij}(x, t) = \frac{-H(t)}{8\pi^2\rho} \frac{\partial}{\partial t} \sum_{\nu=1}^3 \int_{S^2} \delta(\bar{\omega}_\nu t - \bar{\xi} \cdot x) \phi_i^\nu(\bar{\xi}) \phi_j^\nu(\bar{\xi}) \bar{\omega}_\nu^{-2}(\bar{\xi}) dS(\bar{\xi}), \quad (9.23)$$

where

$$\begin{aligned} & \int_{S^2} \delta'(\bar{\omega}_\nu t + \bar{\xi} \cdot x) \phi_i^\nu(\bar{\xi}) \phi_j^\nu(\bar{\xi}) \bar{\omega}_\nu^{-1} dS(\bar{\xi}) \\ &= \int_{S^2} \delta'(\bar{\omega}_\nu t - \bar{\xi} \cdot x) \phi_i^\nu(\bar{\xi}) \phi_j^\nu(\bar{\xi}) \bar{\omega}_\nu^{-1} dS(\bar{\xi}) \end{aligned} \quad (9.24)$$

is used.

* Isotropic materials

In this case,

$$\begin{aligned} K_{ij} &= (\lambda + \mu) \xi_i \xi_j + \mu \delta_{ij} \xi^2, \\ D(\xi, \omega) &= (\mu \xi^2 - \rho \omega^2)^2 \{ (\lambda + 2\mu) \xi^2 - \rho \omega^2 \}, \\ \rho \omega_1^2 &= (\lambda + 2\mu) \xi^2, \\ \rho \omega_2^2 &= \rho \omega_3^2 = \mu \xi^2, \end{aligned} \quad (9.25)$$

and from (9.12)

$$\phi_i^\perp \phi_j^\perp = \xi_i \xi_j / \xi^2, \quad (9.26)$$

and from the second equation in (9.13)

$$\phi_i^2 \phi_j^2 + \phi_i^3 \phi_j^3 = \frac{\delta_{ij} \xi^2 - \xi_i \xi_j}{\xi^2}. \quad (9.27)$$

Substitution of the above result into (9.23) leads to

$$\begin{aligned} G_{ij}(x, t) &= \frac{-H(t)}{8\pi^2\rho} \frac{\partial}{\partial t} \left\{ \int_{S^2} \delta \left(\sqrt{\frac{\lambda+2\mu}{\rho}} t - \bar{\xi} \cdot x \right) \bar{\xi}_i \bar{\xi}_j \left(\frac{\lambda+2\mu}{\rho} \right)^{-1} dS(\bar{\xi}) \right. \\ &\quad \left. + \int_{S^2} \delta \left(\sqrt{\frac{\mu}{\rho}} t - \bar{\xi} \cdot x \right) (\delta_{ij} - \bar{\xi}_i \bar{\xi}_j) \left(\frac{\mu}{\rho} \right)^{-1} dS(\bar{\xi}) \right\}. \quad (9.28) \end{aligned}$$

The following integrals are used:

$$\begin{aligned} &\int_{S^2} \delta \left(\sqrt{\frac{\lambda+2\mu}{\rho}} t - \bar{\xi} \cdot x \right) \bar{\xi}_i \bar{\xi}_j dS(\bar{\xi}) \\ &= \int_{c_2 t - x}^{c_2 t + x} \delta(u) \bar{\xi}_i \bar{\xi}_j x^{-1} du \int_0^{2\pi} d\phi \\ &= \left(\frac{1}{x} \right) H \left(x - \sqrt{\frac{\lambda+2\mu}{\rho}} t \right) \int_0^{2\pi} \bar{\xi}_i \bar{\xi}_j d\phi, \quad (9.29) \end{aligned}$$

where

$$\sqrt{\frac{\lambda+2\mu}{\rho}} t - \bar{\xi} \cdot \bar{x} x = u, \quad c_2 = \sqrt{\frac{\lambda+2\mu}{\rho}}, \quad (9.30)$$

$$x = x \bar{x}, \quad -d(\bar{\xi} \cdot \bar{x}) x = du.$$

When $\sqrt{\frac{\lambda+2\mu}{\rho}} t - \bar{\xi} \cdot x = 0$ we have

$$\begin{aligned}\bar{\xi}_i &= \sqrt{\frac{\lambda + 2\mu}{\rho}} t \bar{x}_i / x + \eta_i, \\ \int_0^{2\pi} \eta_i \eta_j d\phi &= \pi (\delta_{ij} - \bar{x}_i \bar{x}_j) \left(1 - \frac{\lambda + 2\mu}{\rho} \frac{t^2}{x^2} \right).\end{aligned}\quad (9.31)$$

Therefore, when $\sqrt{\frac{\lambda + 2\mu}{\rho}} t - \bar{\xi} \cdot x = 0$,

$$\int_0^{2\pi} \bar{\xi}_i \bar{\xi}_j d\phi = 2\pi \left(\sqrt{\frac{\lambda + 2\mu}{\rho}} t / x \right)^2 \bar{x}_i \bar{x}_j + \pi (\delta_{ij} - \bar{x}_i \bar{x}_j) \left(1 - \frac{\lambda + 2\mu}{\rho} \frac{t^2}{x^2} \right). \quad (9.32)$$

Finally, we have

$$\begin{aligned}G_{ij}(x, t) &= \frac{-H(t)}{8\pi\rho x} \frac{\partial}{\partial t} \left[H \left(x - \sqrt{\frac{\lambda + 2\mu}{\rho}} t \right) \right. \\ &\quad \times \left. \left\{ (\delta_{ij} - \bar{x}_i \bar{x}_j) \middle/ \frac{\lambda + 2\mu}{\rho} - \frac{t^2}{x^2} (\delta_{ij} - 3\bar{x}_i \bar{x}_j) \right\} \right. \\ &\quad \left. + H \left(x - \sqrt{\frac{\mu}{\rho}} t \right) \left\{ (\delta_{ij} + \bar{x}_i \bar{x}_j) \middle/ \frac{\mu}{\rho} + \frac{t^2}{x^2} (\delta_{ij} - 3\bar{x}_i \bar{x}_j) \right\} \right] \quad (9.33)\end{aligned}$$

or

$$\begin{aligned}G_{ij}(x, t) &= \frac{H(t)}{4\pi\rho x} \left[\delta \left(x - \sqrt{\frac{\lambda + 2\mu}{\rho}} t \right) \bar{x}_i \bar{x}_j \middle/ \sqrt{\frac{\lambda + 2\mu}{\rho}} \right. \\ &\quad + \delta \left(x - \sqrt{\frac{\mu}{\rho}} t \right) (\delta_{ij} - \bar{x}_i \bar{x}_j) \middle/ \sqrt{\frac{\mu}{\rho}} + (\delta_{ij} - 3\bar{x}_i \bar{x}_j) (t/x^2) \\ &\quad \times \left. \left\{ H \left(x - \sqrt{\frac{\lambda + 2\mu}{\rho}} t \right) - H \left(x - \sqrt{\frac{\mu}{\rho}} t \right) \right\} \right]. \quad (9.34)\end{aligned}$$

The above result was first obtained by Stokes (1849). The static Green's function (5.8) can be obtained as

$$\begin{aligned}
 G_{ij}(x) &= \int_{-\infty}^{\infty} G_{ij}(x, t - t') dt' \\
 &= \int_{-\infty}^{\infty} \frac{H(\bar{t})}{8\pi\rho x} \frac{\partial}{\partial t'} \left[H\left(x - \sqrt{\frac{\lambda+2\mu}{\rho}} \bar{t}\right) \right. \\
 &\quad \times \left. \left\{ (\delta_{ij} - \bar{x}_i \bar{x}_j) \middle/ \frac{\lambda+2\mu}{\rho} - \frac{\bar{t}^2}{x^2} (\delta_{ij} - 3\bar{x}_i \bar{x}_j) \right\} \right. \\
 &\quad + H\left(x - \sqrt{\frac{\mu}{\rho}} \bar{t}\right) \left\{ (\delta_{ij} + \bar{x}_i \bar{x}_j) \middle/ \frac{\mu}{\rho} + \frac{\bar{t}^2}{x^2} (\delta_{ij} - 3\bar{x}_i \bar{x}_j) \right\} \left. \right] dt' \\
 &= \frac{1}{8\pi\rho x} \left\{ (\delta_{ij} - \bar{x}_i \bar{x}_j) \middle/ \frac{\lambda+2\mu}{\rho} + (\delta_{ij} + \bar{x}_i \bar{x}_j) \middle/ \frac{\mu}{\rho} \right\}, \tag{9.35}
 \end{aligned}$$

where $\bar{t} = t - t'$, $\partial/\partial t = -\partial/\partial t'$. The last result agrees with (5.8).

* Steady state

If a unit body force in the x_m -direction with time dependence $\exp(-i\omega t)$ is applied at the origin of coordinates, the i^{th} component of a displacement field at point x can be expressed by $g_{im}(x) \exp(-i\omega t)$. $g_{im}(x)$ is called the steady-state elastic wave Green's function. It satisfies the equation of motion

$$C_{ijkl} g_{km,lj}(x) + \rho \omega^2 g_{im}(x) + \delta_{im} \delta(x) = 0. \tag{9.36}$$

The solution can be written as

$$g_{ij}(x) = (2\pi)^{-3} \int_{-\infty}^{\infty} \bar{G}_{ij}(\xi, \omega) \exp(i\xi \cdot x) d\xi, \tag{9.37}$$

where $\bar{G}_{ij}(\xi, \omega)$ has been defined by (9.6) or (9.11). For isotropic materials, (9.26) and (9.27) yield

$$\bar{G}_{ij}(\xi, \omega) = \frac{\xi_i \xi_j / \xi^2}{(\lambda + 2\mu)(\xi^2 - \alpha^2)} + \frac{\delta_{ij} - \xi_i \xi_j / \xi^2}{\mu(\xi^2 - \beta^2)} \tag{9.38}$$

with

$$\alpha^2 = \rho\omega^2/(\lambda + 2\mu), \quad \beta^2 = \rho\omega^2/\mu. \quad (9.39)$$

Then, we have

$$g_{ij}(x) = \frac{1}{4\pi\rho\omega^2 x} \left[\beta^2 \delta_{ij} \exp(i\beta x) - x \frac{\partial^2}{\partial x_i \partial x_j} \left\{ \frac{\exp(i\alpha x)}{x} - \frac{\exp(i\beta x)}{x} \right\} \right], \quad (9.40)$$

where $x = (\delta_{ij}x_i x_j)^{1/2}$.

For the two-dimensional case, (9.40) is integrated with respect to x_3 from $-\infty$ to ∞ . Then we have

$$g_{ij}(x_1, x_2) = \frac{i}{4\rho\omega^2} \left[\beta^2 \delta_{ij} H_0^{(1)}(\beta r) - \frac{\partial^2}{\partial x_i \partial x_j} \{ H_0^{(1)}(\alpha r) - H_0^{(1)}(\beta r) \} \right], \quad (9.41)$$

where $H_0^{(1)}(z)$ is the zero order Hankel function of the first kind and $r = (x_1^2 + x_2^2)^{1/2}$.

* 10. Incompatibility

Eigenstresses are caused by the incompatibility of the eigenstrains. Uniform or linear distributions of eigenstrains throughout a free body (compatible eigenstrains) do not introduce any eigenstress in the material.

The incompatibility of ϵ_{ij}^* has been discussed by Reissner (1931) and Neményi (1931). Dislocations due to the incompatibility have been studied by Volterra (1907), Weingarten (1901), Cesáro (1906), and Moriguti (1947) from the viewpoint of elasticity theory in connection with the multiple-valueness of displacements and rotations. A plasticity theory viewpoint has been developed by Kondo (1955), Bilby (1960), and Kröner (1958). In this section we follow the work of Moriguti.

Let us consider an elastic body D which is free from external force but is subjected to a distribution of eigenstrain ϵ_{ij}^* . Imagine a slender rod AP inside the elastic body (see Fig. 10.1). If the rod were isolated from its surroundings and set free, it would change its form slightly and take on a form such as AP' , where one end of the rod, A , is fixed (see Fig. 10.1). If a Cartesian coordinate

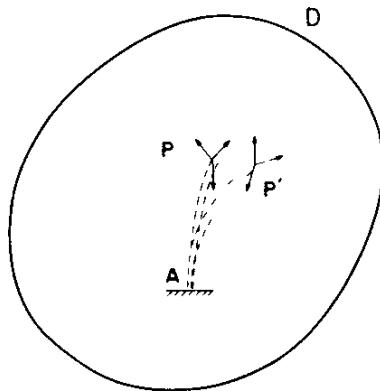


Fig. 10.1 Liberation of an imaginary slender rod AP in a continuum D gives deformation AP'

is attached at point P , it will rotate in a different direction. Both displacement \bar{u}_i and the rotation $\bar{\omega}_i$ at P are caused by the release of elastic strain e_{ij} along AP .

Let us try to express these components of displacement and rotation as functions of e_{ij} in the form of line integrals along AP . Since the deformation of AP to AP' is completely elastic with displacement \bar{u}_i , we can write

$$e_{ij} = -\frac{1}{2} \left(\frac{\delta \bar{u}_i}{\delta x_j} + \frac{\delta \bar{u}_j}{\delta x_i} \right), \quad (10.1)$$

$$\bar{\omega}_{ij} = \epsilon_{ijk} \bar{\omega}_k = -\frac{1}{2} \left(\frac{\delta \bar{u}_j}{\delta x_i} - \frac{\delta \bar{u}_i}{\delta x_j} \right)$$

where $\delta \bar{u}_i$ is the change of \bar{u}_i along a small line element $ds = (\delta x_i \delta x_j)^{1/2}$ along AP ($\delta \bar{u}_i$ is caused by the release of ds). Since \bar{u}_i is path-dependent, it does not have the usual derivatives $\bar{u}_{i,j}$, but it has $\delta \bar{u}_i / \delta x_j$ which are defined only along AP . The elastic strain components e_{ij} have been caused by ϵ_{ij}^* , which are piece-wise continuous functions with derivatives. The equations in (10.1) easily lead to

$$\delta \bar{u}_i = -(e_{ij} - \bar{\omega}_{ij}) \delta x_j. \quad (10.2)$$

We have also

$$e_{kj,i} = -\frac{1}{2} \left(\frac{\partial}{\partial x_i} \frac{\delta \bar{u}_k}{\delta x_j} + \frac{\partial}{\partial x_i} \frac{\delta \bar{u}_j}{\delta x_k} \right), \quad (10.3)$$

$$e_{ki,j} = -\frac{1}{2} \left(\frac{\partial}{\partial x_j} \frac{\delta \bar{u}_k}{\delta x_i} + \frac{\partial}{\partial x_j} \frac{\delta \bar{u}_i}{\delta x_k} \right),$$

and therefore

$$(e_{kj,i} - e_{ki,j}) = -\frac{1}{2} \left(\frac{\partial}{\partial x_i} \frac{\delta \bar{u}_j}{\delta x_k} - \frac{\partial}{\partial x_j} \frac{\delta \bar{u}_i}{\delta x_k} \right), \quad (10.4)$$

where

$$\frac{\partial}{\partial x_i} \frac{\delta \bar{u}_k}{\delta x_j} = \frac{\partial}{\partial x_j} \frac{\delta \bar{u}_k}{\delta x_i} \quad (10.5)$$

is used. Multiplying (10.4) by δx_k , we have

$$(e_{kj,i} - e_{ki,j}) \delta x_k = \delta \bar{\omega}_{ij} \quad (10.6)$$

or

$$\delta \bar{\omega}_{ij} = \epsilon_{hij} \epsilon_{hlm} e_{km,l} \delta x_k. \quad (10.7)$$

From (10.7) and (10.2),

$$\begin{aligned} \bar{\omega}_{ij} &= \int_A^P \epsilon_{hij} \epsilon_{hlm} e_{km,l} \, dx_k, \\ \bar{u}_i &= - \int_A^P (e_{ij} - \bar{\omega}_{ij}) \, dx_j, \end{aligned} \quad (10.8)$$

with the initial conditions

$$\bar{u}_i = \bar{\omega}_{ij} = 0 \quad \text{at } A. \quad (10.9)$$

Consider a special case where the end point P coincides with the starting point A (see Fig. 10.2). Then we have strain release along a closed curve c . Let us call the displacement and the rotation at the end point due to the strain release a dislocation and a disclination, respectively, as shown in Fig. 10.3. The end point shifts to A' , and the coordinate system x_i (fixed to the end point) rotates to the coordinate system x'_i after the release. The disclination is

$$\bar{\omega}_{ij} = \oint_c \epsilon_{hij} \epsilon_{hlm} e_{km,l} \, dx_k \quad (10.10)$$

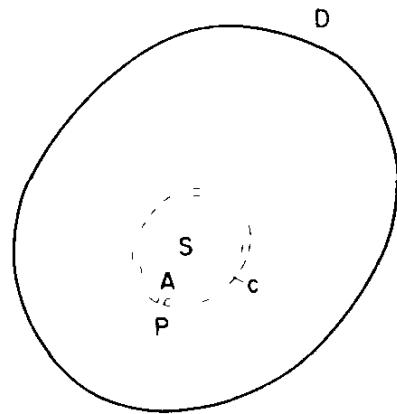


Fig 10.2 End point P in Fig. 10.1 coincides with the starting point A . The closed loop and the surface bounded by the loop are denoted by c and S respectively

and the dislocation is

$$\bar{u}_i = - \oint_c (e_{ij} - \bar{\omega}_{ij}) dx_j. \quad (10.11)$$

The closed curve c is called the Burgers circuit. The integrations are defined

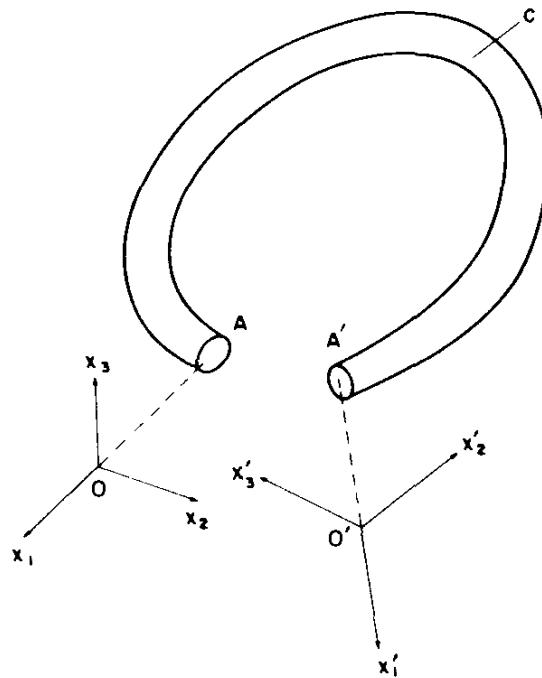


Fig 10.3 Dislocation and disclination are relative orientations between the x - and x' -coordinate systems

on the closed curve c before the release. Applying Stokes' theorem to (10.10), we have

$$\bar{\omega}_{ij} = \int_S \int \epsilon_{hij} R_{hq} n_q dS, \quad (10.12)$$

where n_q is the unit normal on dS , and

$$R_{hq} = \epsilon_{hlm} \epsilon_{qpk} e_{km,lp}. \quad (10.13)$$

The symmetric tensor R_{hq} is called the incompatibility tensor.

Before we apply Stokes' theorem, expression (10.11) is slightly modified. This is necessary since $\bar{\omega}_{ij}$ in the integrand (10.11) has no derivatives. Integration by parts of (10.11) and the definition of (10.7) lead to

$$\begin{aligned} \bar{u}_i &= - \oint_c e_{ij} dx_j + [\bar{\omega}_{ij} x_j]_A^P - \oint_c x_j \delta \bar{\omega}_{ij} \\ &= - \oint_c e_{ij} dx_j + \bar{\omega}_{ij} x_j^0 - \oint_c x_j \epsilon_{hij} \epsilon_{hlm} e_{km,l} dx_k, \end{aligned} \quad (10.14)$$

where x_j^0 is the coordinate of point $P (= A)$ in the x_i -coordinate system before the release. The initial conditions (10.9) have been considered. The last expression of (10.14) is called Cesáro's integral (1906). By the use of Stokes' theorem, (10.14) can be simplified as

$$\bar{u}_i = \bar{\omega}_{ij} x_j^0 - \int_S \int x_j \epsilon_{hij} R_{hq} n_q dS. \quad (10.15)$$

Since the components of the vector $\overrightarrow{OO'}$ are the sum of \bar{u}_i and the effect of the rotation $\bar{\omega}_{ij}$ at A' , they can be expressed as

$$\bar{\xi}_i = \bar{u}_i - \bar{\omega}_{ij} x_j^0 = \bar{u}_i + \epsilon_{ijk} \bar{\omega}_j x_k^0. \quad (10.16)$$

Then,

$$\bar{\xi}_i = - \int_S \int x_j \epsilon_{hij} R_{hq} n_q dS. \quad (10.17)$$

The result (10.16) corresponds to (7.19). $[u_i]$, $\epsilon_{ipq} \omega_p x_q$, and $-\epsilon_{ipq} \omega_p x_q^0$ in (7.19) are equivalent to $\bar{\xi}_i$, $\epsilon_{ijk} \bar{\omega}_j x_k^0$, and \bar{u}_i in (10.16), respectively, and c is the Burgers circuit.

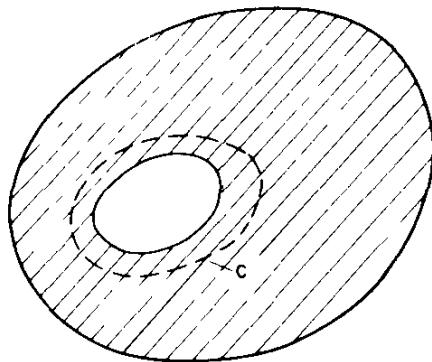


Fig. 10.4 In a multiply connected material, the Burgers circuit c may contain a cavity

An elastic body subjected to an applied load has a compatible elastic strain and $e_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})$. In this case, R_{hq} becomes zero when it is substituted into (10.13). Compatible elastic deformations do not cause any dislocation or disclination. If an elastic body has an internal stress (eigenstress) field due to an eigenstrain ϵ_{ij}^* , the dislocation and disclination defined by (10.17) and (10.12) are not zero. The incompatibility caused by ϵ_{ij}^* can be calculated by substitution of

$$e_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i}) - \epsilon_{ij}^* \quad (10.18)$$

into (10.13). Then, we have

$$R_{hq} = -\epsilon_{hlm}\epsilon_{qpk}\epsilon_{km,lp}^*. \quad (10.19)$$

Equations (10.12) and (10.17) are expressions for disclination and dislocation as functions of the eigenstrains.

The surface S in (10.12) and (10.17) should be bounded by the closed curve c and should consist of material points. When a multiply connected material is considered and a closed curve contains a cavity (see Fig. 10.4), the line integrals (10.10) and (10.14) defined on c cannot be transformed into (10.12)

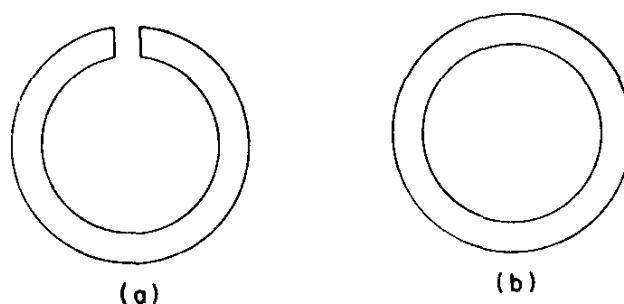


Fig. 10.5 Dislocation

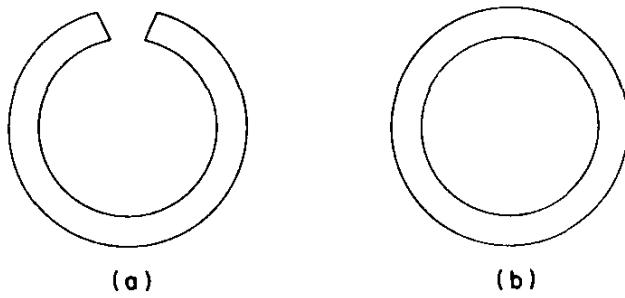


Fig. 10.6 Disclination

and (10.17). Therefore, dislocations and disclinations can exist even if $R_{hq} = 0$. It is easily shown that any two closed curves surrounding the same cavity define the same dislocations and disclinations if $R_{hq} = 0$. The proof can be shown by applying Stokes' theorem to the domain bounded by the two curves. Examples are Volterra's dislocation (Fig. 10.5(b)) and disclination (Fig. 10.6(b)) which are made by welding Figs. 10.5(a) and 10.6(a), respectively.

* Riemann-Christoffel curvature tensor

When the Burgers circuit is taken as an infinitesimal small loop, formula (10.17) can be written as

$$\bar{\xi}_i = -x_j \epsilon_{hij} R_{hq} n_q dS. \quad (10.20)$$

Furthermore, we put

$$n_q = \epsilon_{qmn} f_{mn}. \quad (10.21)$$

Then, (10.20) can be written as

$$\bar{\xi}_i = -x_j R_{mni} f_{mn} dS, \quad (10.22)$$

where

$$R_{mni} = \epsilon_{hij} \epsilon_{qmn} R_{hq}. \quad (10.23)$$

R_{mni} is called the Riemann-Christoffel curvature tensor and R_{hq} the Ricci tensor. Although the eigenstrain ϵ_{ij}^* in (10.18) is incompatible in the three-dimensional Euclidean space when $R_{hq} \neq 0$, it might be compatible in the three-dimensional Riemannian space. The curvature of the Riemannian space

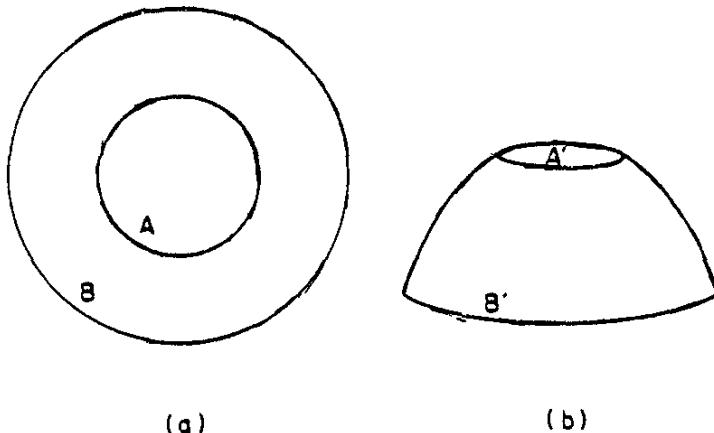


Fig. 10.7 Two-dimensional Euclidean space (a) and two-dimensional Riemannian space (b)
Plastic deformation can be characterized by the transformation from (a) to (b)

is expressed by R_{mnji} . ϵ_{ij}^* can be considered as a strain in the natural state in the Riemannian space with the fundamental metric tensor g_{ij} , that is,

$$\epsilon_{ij}^* = \frac{1}{2}(g_{ij} - \delta_{ij}). \quad (10.24)$$

When (10.19) is substituted into (10.23) by use of (10.24), we have

$$R_{mnji} = \frac{1}{2}(g_{mi,nj} + g_{nj,im} - g_{mj,ni} - g_{ni,mj}) \quad (10.25)$$

which is nothing more than the definition of the Riemann-Christoffel curvature tensor for small strains (see Sokolnikoff 1964, pp. 89 and 91).

In order to explain the physical meaning of the above result, let us consider a two-dimensional case for simplicity. Figure 10.7(a) shows a two-dimensional Euclidean space subjected to an eigenstress field due to a constraint eigenstrain ϵ_{ij}^* . This is the usual dislocated state of an imperfect crystal. Figure 10.7(b) is a two-dimensional Riemannian space with the fundamental metric tensor $2\epsilon_{ij}^* + \delta_{ij}$. This Riemannian space is an imaginary state of the material and is stress-free; it is called the natural state. The associated elastic stress and strain are necessary to make the natural state shown in Fig. 10.7(b) conform to the physical state shown in Fig. 10.7(a). The eigenstrain is compatible in Fig. 10.7(b), but it is constrained and incompatible in Fig. 10.7(a). The sum of the elastic strain and the eigenstrain is, of course, compatible in Fig. 10.7(a).

It is important to note that the two-dimensional Riemannian space is embedded in the three-dimensional Euclidean space. Similarly, a three-dimensional Riemannian space can be embedded in a six-dimensional Euclidean space. If a deformation is defined from the strained state (Fig. 10.7(a) for the two-dimensional case) into the natural state (Fig. 10.7(b) for the two-dimensional case), the deformation can be characterized by a deviation from one

space into a higher dimensional space, as seen in the buckling phenomena of elastic plates. Kondo (1949, 1955) used this buckling analogy in his explanation of the yielding phenomena of mild steel. He postulated that yielding is a buckling phenomenon of a three-dimensional Euclidean space into a six-dimensional Euclidean space (three-dimensional Riemannian space). The Riemann-Christoffel curvature tensor or the Euler-Schouten curvature tensor is the geometric object in Kondo's theory of plasticity. On the other hand, Bilby, Bullough, and Smith (1955), Bilby (1960), Kröner (1956, 1958), and Kröner and Rieder (1956) adopted the distant parallelism in a linear connection and described the Burgers vector by Schouten's torsion tensor. A more general expression for the Burgers vector has been given by Kondo (1955).

Isotropic inclusions

Explicit formulae are derived for elastic fields caused by inclusions. Most of the cases considered in this chapter are ellipsoidal inclusions in an isotropic infinite body, and the elastic moduli are the same for inclusions and matrices. The case when inclusions and matrices have different elastic moduli is treated in Chapter 4.

Eigenstrains in the inclusions are given by constants or by polynomials of coordinates. A special emphasis is placed on the case of constant eigenstrains.

11. Eshelby's solution

An ellipsoidal inclusion Ω is considered in an isotropic infinite body. Eigenstrains given in the ellipsoidal domain are assumed to be uniform (constant). The solution of the problem has been investigated by Goodier (1937) in the case when eigenstrains are thermal expansion strains. For general eigenstrains, the solution has been given by Eshelby (1957, 1959, 1961). Expressions for the solution are different for interior points (points inside the inclusion) and exterior points (points outside the inclusion). Eshelby's most valuable result is that the strain and stress fields become uniform for the interior points.

From (6.1) we have

$$u_i(x) = -C_{jkmn}\epsilon_{mn}^* \int_{\Omega} G_{ij,k}(x - x') dx', \quad (11.1)$$

where Ω (see Fig. 11.1) is given by

$$x_1^2/a_1^2 + x_2^2/a_2^2 + x_3^2/a_3^2 \leq 1 \quad (11.2)$$

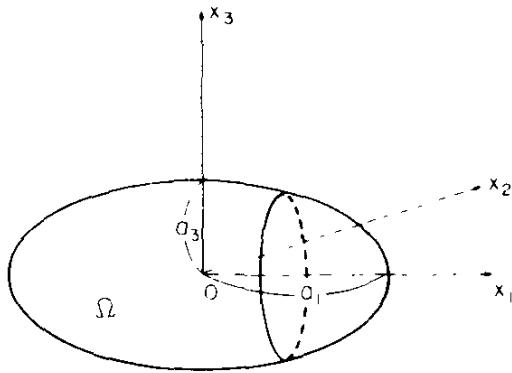


Fig. 11.1 An ellipsoidal inclusion with principal half axes a_1 , a_2 , and a_3

and $G_{ij}(\mathbf{x} - \mathbf{x}')$ is, from (5.8),

$$G_{ij}(\mathbf{x} - \mathbf{x}') = \frac{1}{16\pi\mu(1-\nu)|\mathbf{x} - \mathbf{x}'|} \left[(3 - 4\nu)\delta_{ij} + \frac{(\mathbf{x}_i - \mathbf{x}'_i)(\mathbf{x}_j - \mathbf{x}'_j)}{|\mathbf{x} - \mathbf{x}'|^2} \right]. \quad (11.3)$$

After some manipulation we obtain

$$u_i(\mathbf{x}) = \frac{-\epsilon_{jk}^*}{8\pi(1-\nu)} \int_{\Omega} g_{ijk}(\mathbf{l}) \frac{d\mathbf{x}'}{|\mathbf{x}' - \mathbf{x}|^2}, \quad (11.4)$$

where

$$g_{ijk}(\mathbf{l}) = (1 - 2\nu)(\delta_{ij}l_k + \delta_{ik}l_j - \delta_{jk}l_i) + 3l_i l_j l_k. \quad (11.5)$$

The vector \mathbf{l} is a unit vector $(\mathbf{x}' - \mathbf{x})/|\mathbf{x}' - \mathbf{x}|$.

Interior points

When point \mathbf{x} is located inside the inclusion, the integral in (11.4) is explicitly performed. As shown in Fig. 11.2, the volume element $d\mathbf{x}'$ in (11.4) can be written as

$$d\mathbf{x}' = dr dS = dr r^2 d\omega, \quad (11.6)$$

where $r = |\mathbf{x}' - \mathbf{x}|$ and $d\omega$ is a surface element of a unit sphere Σ centered at point \mathbf{x} . Upon integration with respect to r , we have

$$u_i(\mathbf{x}) = \frac{-\epsilon_{jk}^*}{8\pi(1-\nu)} \int_{\Sigma} r(\mathbf{l}) g_{ijk}(\mathbf{l}) d\omega, \quad (11.7)$$

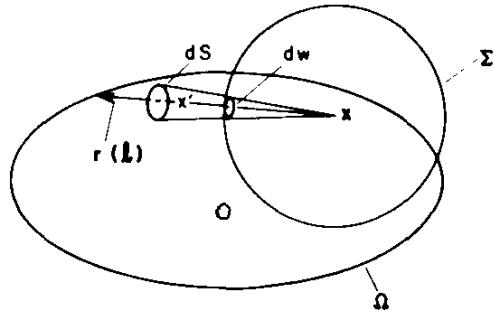


Fig. 11.2 Ω is an ellipsoidal inclusion, Σ is a unit sphere centered at point x

where $r(l)$ is the positive root of

$$(x_1 + rl_1)^2/a_1^2 + (x_2 + rl_2)^2/a_2^2 + (x_3 + rl_3)^2/a_3^2 = 1, \quad (11.8)$$

that is,

$$r(l) = -f/g + (f^2/g^2 + e/g)^{1/2}, \quad (11.9)$$

where

$$g = l_1^2/a_1^2 + l_2^2/a_2^2 + l_3^2/a_3^2,$$

$$f = l_1 x_1/a_1^2 + l_2 x_2/a_2^2 + l_3 x_3/a_3^2, \quad (11.10)$$

$$e = 1 - x_1^2/a_1^2 - x_2^2/a_2^2 - x_3^2/a_3^2.$$

When (11.9) is inserted in (11.7), the term $(f^2/g^2 + e/g)^{1/2}$ can be omitted, since it is even in l , while g_{ijk} is odd. To retain the advantages of suffix notation, we introduce the vector

$$\lambda_1 = l_1/a_1^2, \quad \lambda_2 = l_2/a_2^2, \quad \lambda_3 = l_3/a_3^2. \quad (11.11)$$

Then,

$$u_i(x) = \frac{x_m \epsilon_{jk}^*}{8\pi(1-\nu)} \int_{\Sigma} \frac{\lambda_m g_{ijk}}{g} d\omega \quad (11.12)$$

and the strain components become

$$\epsilon_{ij}(x) = \frac{\epsilon_{mn}^*}{16\pi(1-\nu)} \int_{\Sigma} \frac{\lambda_i g_{jm} + \lambda_j g_{im}}{g} d\omega. \quad (11.13)$$

The integral in (11.13) is independent of x . Therefore, we have an attractive conclusion in that the strain (and therefore the stress) is uniform inside the inclusion. The surface integrals of the type $\int_{\Sigma} l_1^m l_2^n l_3^k g^{-1} d\omega$ can be reduced to simple integrals according to the work by Routh (1895),

$$\begin{aligned} I_1 &= \int_{\Sigma} \frac{l_1^2 d\omega}{a_1^2 g} = 2\pi a_1 a_2 a_3 \int_0^{\infty} \frac{ds}{(a_1^2 + s) \Delta(s)}, \\ I_{11} &= \int_{\Sigma} \frac{l_1^4 d\omega}{a_1^4 g} = 2\pi a_1 a_2 a_3 \int_0^{\infty} \frac{ds}{(a_1^2 + s)^2 \Delta(s)}, \\ I_{12} &= 3 \int_{\Sigma} \frac{l_1^2 l_2^2 d\omega}{a_1^2 a_2^2 g} = 2\pi a_1 a_2 a_3 \int_0^{\infty} \frac{ds}{(a_1^2 + s)(a_2^2 + s) \Delta(s)}, \end{aligned} \quad (11.14)$$

with $\Delta(s) = (a_1^2 + s)^{1/2} (a_2^2 + s)^{1/2} (a_3^2 + s)^{1/2}$. The remaining coefficients are found by the simultaneous cyclic permutation of (1, 2, 3), (a_1 , a_2 , a_3) and (l_1 , l_2 , l_3).

It is convenient to write (11.13) as

$$\epsilon_{ij} = S_{ijkl} \epsilon_{kl}^*. \quad (11.15)$$

Then,

$$\begin{aligned} S_{ijkl} &= S_{jikl} = S_{ijlk}, \\ S_{1111} &= \frac{3}{8\pi(1-\nu)} a_1^2 I_{11} + \frac{1-2\nu}{8\pi(1-\nu)} I_1, \\ S_{1122} &= \frac{1}{8\pi(1-\nu)} a_2^2 I_{12} - \frac{1-2\nu}{8\pi(1-\nu)} I_1, \\ S_{1133} &= \frac{1}{8\pi(1-\nu)} a_3^2 I_{13} - \frac{1-2\nu}{8\pi(1-\nu)} I_1, \\ S_{1212} &= \frac{a_1^2 + a_2^2}{16\pi(1-\nu)} I_{12} + \frac{1-2\nu}{16\pi(1-\nu)} (I_1 + I_2). \end{aligned} \quad (11.16)$$

All other non-zero components are obtained by the cyclic permutation of (1, 2, 3). The components which cannot be obtained by the cyclic permutation are zero; for instance, $S_{1112} = S_{1223} = S_{1232} = 0$. S_{ijkl} is called Eshelby's tensor.

The integrals in (11.14) are expressed by the standard elliptic integrals (Gradshteyn and Ryzhik, 1965). Assuming $a_1 > a_2 > a_3$,

$$I_1 = \frac{4\pi a_1 a_2 a_3}{(a_1^2 - a_2^2)(a_1^2 - a_3^2)^{1/2}} \{ F(\theta, k) - E(\theta, k) \}, \quad (11.17)$$

$$I_3 = \frac{4\pi a_1 a_2 a_3}{(a_2^2 - a_3^2)(a_1^2 - a_3^2)^{1/2}} \left\{ \frac{a_2(a_1^2 - a_3^2)^{1/2}}{a_1 a_3} - E(\theta, k) \right\},$$

where

$$F(\theta, k) = \int_0^\theta \frac{dw}{(1 - k^2 \sin^2 w)^{1/2}}, \quad E(\theta, k) = \int_0^\theta (1 - k^2 \sin^2 w)^{1/2} dw, \quad (11.18)$$

$$\theta = \sin^{-1}(1 - a_3^2/a_1^2)^{1/2}, \quad k = \{(a_1^2 - a_2^2)/(a_1^2 - a_3^2)\}^{1/2}.$$

The definitions for g and I and (11.14) yield the following formulae:

$$\begin{aligned} I_1 + I_2 + I_3 &= 4\pi, \\ 3I_{11} + I_{12} + I_{13} &= 4\pi/a_1^2, \\ 3a_1^2 I_{11} + a_2^2 I_{12} + a_3^2 I_{13} &= 3I_1, \\ I_{12} &= (I_2 - I_1)/(a_1^2 - a_2^2). \end{aligned} \quad (11.19)$$

The last formula in (11.19) is obtained when we split the factor $(a_1^2 + s)^{-1}(a_2^2 + s)^{-1}$ in the integral (11.14) for I_{12} into partial fractions. Equations in (11.19) and their cyclic counterparts give sufficient relations to express I_i and I_{ij} in terms of I_1 and I_3 .

Finally, the stress components become

$$\begin{aligned} \sigma_{11}/2\mu &= \left[\frac{a_1^2}{8\pi(1-\nu)} \left\{ \frac{1-\nu}{1-2\nu} 3I_{11} + \frac{\nu}{1-2\nu} (I_{21} + I_{31}) \right\} \right. \\ &\quad \left. + \frac{1-2\nu}{8\pi(1-\nu)} \left\{ \frac{1-\nu}{1-2\nu} I_1 - \frac{\nu}{1-2\nu} (I_2 + I_3) \right\} - \frac{1-\nu}{1-2\nu} \right] \epsilon_{11}^* \end{aligned}$$

$$\begin{aligned}
& + \left[\frac{a_2^2}{8\pi(1-\nu)} \left\{ \frac{1-\nu}{1-2\nu} I_{12} + \frac{\nu}{1-2\nu} (3I_{22} + I_{32}) \right\} \right. \\
& \quad \left. - \frac{1-2\nu}{8\pi(1-\nu)} \left\{ \frac{1-\nu}{1-2\nu} I_1 - \frac{\nu}{1-2\nu} (I_2 - I_3) \right\} - \frac{\nu}{1-2\nu} \right] \epsilon_{22}^* \\
& + \left[\frac{a_3^2}{8\pi(1-\nu)} \left\{ \frac{1-\nu}{1-2\nu} I_{13} + \frac{\nu}{1-2\nu} (3I_{33} + I_{23}) \right\} \right. \\
& \quad \left. - \frac{1-2\nu}{8\pi(1-\nu)} \left\{ \frac{1-\nu}{1-2\nu} I_1 - \frac{\nu}{1-2\nu} (I_3 - I_2) \right\} - \frac{\nu}{1-2\nu} \right] \epsilon_{33}^*, \\
\end{aligned} \tag{11.20}$$

$$\sigma_{12}/2\mu = \left\{ \frac{a_1^2 + a_2^2}{8\pi(1-\nu)} I_{12} + \frac{1-2\nu}{8\pi(1-\nu)} (I_1 + I_2) - 1 \right\} \epsilon_{12}^*,$$

and other components are obtained by the cyclic permutation of (1, 2, 3).

The integrals (11.17) become elementary functions for special shapes of inclusions, as listed below.

Sphere ($a_1 = a_2 = a_3 = a$)

$$\begin{aligned}
I_1 &= I_2 = I_3 = 4\pi/3, \\
I_{11} &= I_{22} = I_{33} = I_{12} = I_{23} = I_{31} = 4\pi/5a^2, \\
S_{1111} &= S_{2222} = S_{3333} = \frac{7-5\nu}{15(1-\nu)}, \\
S_{1122} &= S_{2233} = S_{3311} = S_{1133} = S_{2211} = S_{3322} = \frac{5\nu-1}{15(1-\nu)}, \\
S_{1212} &= S_{2323} = S_{3131} = \frac{4-5\nu}{15(1-\nu)}. \\
\end{aligned} \tag{11.21}$$

From (11.20) we have

$$\begin{aligned}
\sigma_{11} &= -\mu \frac{16}{15(1-\nu)} \epsilon_{11}^* - 2\mu \frac{5\nu+1}{15(1-\nu)} \epsilon_{22}^* - 2\mu \frac{5\nu+1}{15(1-\nu)} \epsilon_{33}^*, \\
\sigma_{12} &= -2\mu \frac{7-5\nu}{15(1-\nu)} \epsilon_{12}^*. \\
\end{aligned} \tag{11.21.1}$$

All other stress components are obtained by the cyclic permutation of (1, 2, 3).

Elliptic cylinder ($a_3 \rightarrow \infty$)

$$I_1 = 4\pi a_2 / (a_1 + a_2), \quad I_2 = 4\pi a_1 / (a_1 + a_2), \quad I_3 = 0,$$

$$I_{12} = 4\pi / (a_1 + a_2)^2, \quad 3I_{11} = 4\pi / a_1^2 - I_{12},$$

$$3I_{22} = 4\pi / a_2^2 - I_{12}, \quad I_{13} = I_{23} = I_{33} = 0,$$

$$a_3^2 I_{13} = I_1, \quad a_3^2 I_{23} = I_2, \quad a_3^2 I_{33} = 0.$$

$$\begin{aligned} S_{1111} &= \frac{1}{2(1-\nu)} \left\{ \frac{a_2^2 + 2a_1a_2}{(a_1 + a_2)^2} + (1-2\nu) \frac{a_2}{a_1 + a_2} \right\}, \\ S_{2222} &= \frac{1}{2(1-\nu)} \left\{ \frac{a_1^2 + 2a_1a_2}{(a_1 + a_2)^2} + (1-2\nu) \frac{a_1}{a_1 + a_2} \right\}, \quad S_{3333} = 0, \\ S_{1122} &= \frac{1}{2(1-\nu)} \left\{ \frac{a_2^2}{(a_1 + a_2)^2} - (1-2\nu) \frac{a_2}{a_1 + a_2} \right\}, \\ S_{2233} &= \frac{1}{2(1-\nu)} \frac{2\nu a_1}{a_1 + a_2}, \quad S_{3311} = 0, \\ S_{1133} &= \frac{1}{2(1-\nu)} \frac{2\nu a_2}{a_1 + a_2}, \quad S_{2211} = \frac{1}{2(1-\nu)} \left\{ \frac{a_1^2}{(a_1 + a_2)^2} - (1-2\nu) \frac{a_1}{a_1 + a_2} \right\}, \\ S_{3322} &= 0, \quad S_{1212} = \frac{1}{2(1-\nu)} \left\{ \frac{a_1^2 + a_2^2}{2(a_1 + a_2)^2} + \frac{1-2\nu}{2} \right\}, \\ S_{2323} &= \frac{a_1}{2(a_1 + a_2)}, \quad S_{3131} = \frac{a_2}{2(a_1 + a_2)}, \end{aligned} \tag{11.22}$$

and

$$\begin{aligned} \sigma_{11} &= \frac{\mu}{1-\nu} \left\{ -2 + \frac{a_2^2 + 2a_1a_2}{(a_1 + a_2)^2} + \frac{a_2}{a_1 + a_2} \right\} \epsilon_{11}^* \\ &\quad + \frac{\mu}{1-\nu} \left\{ \frac{a_2^2}{(a_1 + a_2)^2} - \frac{a_2}{a_1 + a_2} \right\} \epsilon_{22}^* - \frac{2\mu\nu}{1-\nu} \frac{a_1}{a_1 + a_2} \epsilon_{33}^*, \end{aligned}$$

$$\begin{aligned}
\sigma_{22} &= \frac{\mu}{1-\nu} \left\{ -2 + \frac{a_1^2 + 2a_1a_2}{(a_1 + a_2)^2} + \frac{a_1}{a_1 + a_2} \right\} \epsilon_{22}^* \\
&\quad + \frac{\mu}{1-\nu} \left\{ \frac{a_1^2}{(a_1 + a_2)^2} - \frac{a_1}{a_1 + a_2} \right\} \epsilon_{11}^* - \frac{2\mu\nu}{1-\nu} \frac{a_2}{a_1 + a_2} \epsilon_{33}^*, \\
\sigma_{33} &= \frac{-2\mu\nu}{1-\nu} \frac{a_1}{a_1 + a_2} \epsilon_{11}^* - \frac{2\mu\nu}{1-\nu} \frac{a_2}{a_1 + a_2} \epsilon_{22}^* - \frac{2\mu}{1-\nu} \epsilon_{33}^*, \tag{11.22.1} \\
\sigma_{12} &= -\frac{2\mu}{1-\nu} \frac{a_1a_2}{(a_1 + a_2)^2} \epsilon_{12}^*, \quad \sigma_{23} = -2\mu \frac{a_2}{a_1 + a_2} \epsilon_{23}^*, \\
\sigma_{31} &= -2\mu \frac{a_1}{a_1 + a_2} \epsilon_{31}^*.
\end{aligned}$$

Penny shape ($a_1 = a_2 \gg a_3$)

$$I_1 = I_2 = \pi^2 a_3 / a_1, \quad I_3 = 4\pi - 2\pi^2 a_3 / a_1,$$

$$I_{12} = I_{21} = 3\pi^2 a_3 / 4a_1^3,$$

$$I_{13} = I_{23} = I_{31} = I_{32} = 3(\frac{4}{3}\pi - \pi^2 a_3 / a_1) / a_1^2,$$

$$I_{11} = I_{22} = 3\pi^2 a_3 / 4a_1^3, \quad I_{33} = \frac{4}{3}\pi / a_3^2;$$

$$S_{1111} = S_{2222} = \frac{13 - 8\nu}{32(1-\nu)} \pi \frac{a_3}{a_1}, \quad S_{3333} = 1 - \frac{1 - 2\nu}{1-\nu} \frac{\pi}{4} \frac{a_3}{a_1}, \tag{11.23}$$

$$S_{1122} = S_{2211} = \frac{8\nu - 1}{32(1-\nu)} \pi \frac{a_3}{a_1}, \quad S_{1133} = S_{2233} = \frac{2\nu - 1}{8(1-\nu)} \pi \frac{a_3}{a_1},$$

$$S_{3311} = S_{3322} = \frac{\nu}{1-\nu} \left(1 - \frac{4\nu + 1}{8\nu} \pi \frac{a_3}{a_1} \right),$$

$$S_{1212} = \frac{7 - 8\nu}{32(1-\nu)} \pi \frac{a_3}{a_1}, \quad S_{1313} = S_{2323} = \frac{1}{2} \left(1 + \frac{\nu - 2}{1-\nu} \frac{\pi}{4} \frac{a_3}{a_1} \right),$$

$$S_{kk11} = S_{kk22} = \frac{1 - 2\nu}{1-\nu} \frac{\pi}{4} \frac{a_3}{a_1} + \frac{\nu}{1-\nu}, \quad S_{kk33} = 1 - \frac{1 - 2\nu}{1-\nu} \frac{\pi}{2} \frac{a_3}{a_1};$$

and

$$\begin{aligned}
 \sigma_{11}/2\mu &= \frac{-\nu}{1-\nu}(\epsilon_{11}^* + \epsilon_{22}^*) - \epsilon_{11}^* + \frac{13}{32(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{11}^* + \frac{16\nu-1}{32(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{22}^* \\
 &\quad - \frac{2\nu+1}{8(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{33}^*, \\
 \sigma_{22}/2\mu &= \frac{-\nu}{1-\nu}(\epsilon_{11}^* + \epsilon_{22}^*) - \epsilon_{22}^* + \frac{16\nu-1}{32(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{11}^* + \frac{13}{32(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{22}^* \\
 &\quad - \frac{2\nu+1}{8(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{33}^*, \\
 \sigma_{33}/2\mu &= -\frac{2\nu+1}{8(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{11}^* - \frac{2\nu+1}{8(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{22}^* - \frac{1}{4(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{33}^*, \\
 \sigma_{23}/2\mu &= \frac{\nu-2}{4(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{23}^*, \quad \sigma_{31}/2\mu = \frac{\nu-2}{4(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{31}^*, \\
 \sigma_{12}/2\mu &= -\epsilon_{12}^* + \frac{7-8\nu}{16(1-\nu)} \frac{\pi a_3}{a_1} \epsilon_{12}^*;
 \end{aligned} \tag{11.23.1}$$

when $a_3 \rightarrow 0$

$$\begin{aligned}
 I_1 &= I_2 = 0, \quad I_3 = 4\pi, \\
 I_{12} &= 0, \quad I_{23} = 4\pi/a_2^2, \quad I_{31} = 4\pi/a_1^2, \\
 I_{11} &= I_{22} = 0, \quad a_3^2 I_{33} = 4\pi/3, \\
 S_{2323} &= S_{3131} = \frac{1}{2}, \\
 S_{3311} &= S_{3322} = \nu/(1-\nu), \\
 S_{3333} &= 1, \quad \text{and all other } S_{ijkl} = 0,
 \end{aligned} \tag{11.23.2}$$

and

$$\begin{aligned}
 \sigma_{11} &= -2\mu \left\{ \frac{\nu}{1-\nu} (\epsilon_{11}^* + \epsilon_{22}^*) + \epsilon_{11}^* \right\}, \\
 \sigma_{22} &= -2\mu \left\{ \frac{\nu}{1-\nu} (\epsilon_{11}^* + \epsilon_{22}^*) + \epsilon_{22}^* \right\}, \quad \sigma_{12} = -2\mu \epsilon_{12}^*, \\
 \sigma_{33} &= 0, \quad \sigma_{31} = 0, \quad \sigma_{32} = 0.
 \end{aligned} \tag{11.23.3}$$

Flat ellipsoid ($a_1 > a_2 \gg a_3$)

$$\begin{aligned}
 I_1 &= 4\pi a_2 a_3 \{ F(k) - E(k) \} / (a_1^2 - a_2^2), \\
 I_2 &= 4\pi a_3 E(k) / a_2 - 4\pi a_2 a_3 \{ F(k) - E(k) \} / (a_1^2 - a_2^2), \\
 I_3 &= 4\pi - 4\pi a_3 E(k) / a_2, \\
 I_{12} &= [4\pi a_3 E(k) / a_2 - 8\pi a_2 a_3 \{ F(k) - E(k) \} / (a_1^2 - a_2^2)] / (a_1^2 - a_2^2), \\
 I_{23} &= [4\pi - 8\pi a_3 E(k) / a_2 + 4\pi a_2 a_3 \{ F(k) - E(k) \} / (a_1^2 - a_2^2)] / a_2^2, \\
 I_{31} &= [4\pi - 4\pi a_2 a_3 \{ F(k) - E(k) \} / (a_1^2 - a_2^2) - 4\pi a_3 E(k) / a_2] / a_1^2, \\
 I_{33} &= 4\pi / 3a_3^2,
 \end{aligned} \tag{11.24}$$

where $F(k)$ and $E(k)$ are the complete elliptic integrals of the first and the second kind, respectively,

$$\begin{aligned}
 E(k) &= \int_0^{\pi/2} (1 - k^2 \sin^2 \phi)^{1/2} d\phi, \\
 F(k) &= \int_0^{\pi/2} (1 - k^2 \sin^2 \phi)^{-1/2} d\phi,
 \end{aligned} \tag{11.25}$$

$$k^2 = (a_1^2 - a_2^2) / a_1^2.$$

For $\epsilon_{ij}^* = \epsilon_{33}^*$, (11.20) becomes

$$\sigma_{33} / 2\mu = -a_3 \epsilon_{33}^* E(k) / 2(1 - \nu) a_2. \tag{11.26}$$

For $\epsilon_{ij}^* = \epsilon_{13}^*$,

$$\sigma_{31} / 2\mu = -a_3 \epsilon_{13}^* \left[\frac{\nu}{1 - \nu} a_2 \{ F(k) - E(k) \} / (a_1^2 - a_2^2) + E(k) / a_2 \right]. \tag{11.27}$$

Oblate spheroid ($a_1 = a_2 > a_3$)

$$I_1 = I_2 = \frac{2\pi a_1^2 a_3}{(a_1^2 - a_3^2)^{3/2}} \left\{ \cos^{-1} \frac{a_3}{a_1} - \frac{a_3}{a_1} \left(1 - \frac{a_3^2}{a_1^2} \right)^{1/2} \right\}, \quad I_3 = 4\pi - 2I_1,$$

$$I_{11} = I_{22} = I_{12}, \quad (11.28)$$

$$I_{12} = \pi/a_1^2 - \frac{1}{4}I_{13} = \pi/a_1^2 - \frac{(I_1 - I_3)}{4(a_3^2 - a_1^2)},$$

$$I_{13} = I_{23} = (I_1 - I_3)/(a_3^2 - a_1^2), \quad 3I_{33} = 4\pi/a_3^2 - 2I_{13}.$$

Prolate spheroid ($a_1 > a_2 = a_3$)

$$I_2 = I_3 = \frac{2\pi a_1 a_3^2}{(a_1^2 - a_3^2)^{3/2}} \left\{ \frac{a_1}{a_3} \left(\frac{a_1^2}{a_3^2} - 1 \right)^{1/2} - \cosh^{-1} \frac{a_1}{a_3} \right\},$$

$$I_1 = 4\pi - 2I_2, \quad I_{12} = (I_2 - I_1)/(a_1^2 - a_2^2), \quad 3I_{11} = 4\pi/a_1^2 - 2I_{12},$$

$$I_{22} = I_{33} = I_{23}, \quad (11.29)$$

$$3I_{22} = 4\pi/a_2^2 - I_{23} - (I_2 - I_1)/(a_1^2 - a_2^2),$$

$$I_{23} = \pi/a_2^2 - (I_2 - I_1)/4(a_1^2 - a_2^2).$$

Exterior points

When point \mathbf{x} is located outside the inclusion Ω , a slightly different approach is employed. When expression (5.9) for G_{ij} is substituted into (11.1), we have

$$u_i(\mathbf{x}) = \frac{1}{8\pi(1-\nu)} \left\{ \Psi_{jl,Jli} - 2\nu\Phi_{mm,i} - 4(1-\nu)\Phi_{il,l} \right\}, \quad (11.30)$$

where

$$\Psi_{ij}(\mathbf{x}) = \epsilon_{ij}^* \int_{\Omega} |\mathbf{x} - \mathbf{x}'| d\mathbf{x}', \quad \Phi_{ij}(\mathbf{x}) = \epsilon_{ij}^* \int_{\Omega} 1/|\mathbf{x} - \mathbf{x}'| d\mathbf{x}'. \quad (11.31)$$

From (11.30) the induced strain field can be written as

$$\epsilon_{ij}(\mathbf{x}) = \frac{1}{8\pi(1-\nu)} \left\{ \Psi_{kl,klij} - 2\nu \Phi_{kk,ij} - 2(1-\nu)(\Phi_{ik,kj} + \Phi_{jk,ki}) \right\} \quad (11.32)$$

or

$$\epsilon_{ij}(\mathbf{x}) = D_{ijkl}(\mathbf{x}) \epsilon_{kl}^*, \quad (11.33)$$

where

$$8\pi(1-\nu)D_{ijkl}(\mathbf{x}) = \psi_{,klij} - 2\nu \delta_{kl} \phi_{,ij} - (1-\nu) [\phi_{,kj} \delta_{il} + \phi_{,ki} \delta_{jl} + \phi_{,lj} \delta_{ik} + \phi_{,li} \delta_{jk}], \quad (11.34)$$

and

$$\begin{aligned} \psi(\mathbf{x}) &= \int_{\Omega} |\mathbf{x} - \mathbf{x}'| d\mathbf{x}', \\ \phi(\mathbf{x}) &= \int_{\Omega} 1/|\mathbf{x} - \mathbf{x}'| d\mathbf{x}'. \end{aligned} \quad (11.35)$$

As will be shown in the next section, Ferrers (1877) and Dyson (1891) expressed the above integrals in terms of the following elliptic integrals:

$$\begin{aligned} I(\lambda) &= 2\pi a_1 a_2 a_3 \int_{\lambda}^{\infty} \frac{ds}{\Delta(s)}, \\ I_i(\lambda) &= 2\pi a_1 a_2 a_3 \int_{\lambda}^{\infty} \frac{ds}{(a_i^2 + s) \Delta(s)}, \\ I_{ij}(\lambda) &= 2\pi a_1 a_2 a_3 \int_{\lambda}^{\infty} \frac{ds}{(a_i^2 + s)(a_j^2 + s) \Delta(s)}, \end{aligned} \quad (11.36)$$

where $\Delta(s) = \{(a_1^2 + s)(a_2^2 + s)(a_3^2 + s)\}^{1/2}$, and λ is the largest positive root of the equation

$$x_1^2/(a_1^2 + \lambda) + x_2^2/(a_2^2 + \lambda) + x_3^2/(a_3^2 + \lambda) = 1 \quad (11.37)$$

for an exterior point x . $\lambda = 0$ for interior points of Ω . The result is

$$\begin{aligned} \phi &= \frac{1}{2} [I(\lambda) - x_n x_n I_N(\lambda)], \\ \psi_{,i} &= \frac{1}{2} x_i \{I(\lambda) - x_n x_n I_N(\lambda) - a_I^2 [I_I(\lambda) - x_n x_n I_{IN}(\lambda)]\}, \end{aligned} \quad (11.38)$$

where the following summation convention has been used: repeated lower case indices are summed up from 1 to 3; upper case indices take on the same numbers as the corresponding lower case ones but are not summed. For example,

$$\begin{aligned} x_i x_i / (a_I^2 + \lambda) &= x_1^2 / (a_1^2 + \lambda) + x_2^2 / (a_2^2 + \lambda) + x_3^2 / (a_3^2 + \lambda), \\ x_k I_K &= x_1 I_1 \quad \text{where } k = 1, \\ &= x_2 I_2 \quad \text{where } k = 2, \\ \delta_{ij} I_J &= I_2 \quad \text{if } i = j = 2, \\ &= 0 \quad \text{if } i \neq j. \end{aligned} \quad (11.39)$$

Equation (11.34) requires higher derivatives of ϕ and ψ . Since the lower bound of the integral limits in (11.36) is only a function of x , the derivatives of I , I_i , I_{ij} can be reduced to the derivative of λ . From (11.37) we have

$$\lambda_{,l} = \frac{2x_l}{a_L^2 + \lambda} \sqrt{\frac{x_i x_i}{(a_I^2 + \lambda)^2}}. \quad (11.40)$$

The following obvious relations are useful for further calculations: Consider the derivative

$$I_{i-jk,p}(\lambda) = \frac{-2\pi a_1 a_2 a_3}{(a_i^2 + \lambda) \cdots (a_j^2 + \lambda) (a_k^2 + \lambda) \Delta(\lambda)} \lambda_{,p}, \quad (11.40.1)$$

from which it follows that

$$I_{i-jk,p}(\lambda) = \frac{1}{a_k^2 + \lambda} I_{i-j,p}(\lambda). \quad (11.40.2)$$

Using (11.39), it can be shown that

$$x_k x_k I_{i-jK,p}(\lambda) = \frac{x_k x_k}{a_K^2 + \lambda} I_{i-j,p}(\lambda) = I_{i-j,p}(\lambda). \quad (11.40.3)$$

It can be easily seen from (11.40.3) that

$$\frac{\partial}{\partial x_q} \{ I_{ij-k}(\lambda) - x_r x_r I_{Rij-k}(\lambda) \} = -2 x_q I_{Qij-k}(\lambda). \quad (11.40.4)$$

The above result (11.40.4) is used for derivatives of (11.38). Then, we have

$$\phi_{,ij} = -\delta_{ij} I_I(\lambda) - x_i I_{I,J}(\lambda), \quad (11.40.5)$$

$$\begin{aligned} \psi_{,ijkl} &= -\delta_{ij}\delta_{kl} [I_K(\lambda) - a_I^2 I_{IK}(\lambda)] - (\delta_{ik}\delta_{jl} + \delta_{jk}\delta_{il}) [I_J(\lambda) - a_I^2 I_{IJ}(\lambda)] \\ &\quad - \delta_{ij} x_k [I_K(\lambda) - a_I^2 I_{IK}(\lambda)]_{,l} - (\delta_{ik} x_j + \delta_{jk} x_i) [I_J(\lambda) - a_I^2 I_{IJ}(\lambda)]_{,l} \\ &\quad - (\delta_{il} x_j + \delta_{jl} x_i) [I_J(\lambda) - a_I^2 I_{IJ}(\lambda)]_{,k} - x_i x_j [I_J(\lambda) - a_I^2 I_{IJ}(\lambda)]_{,kl}. \end{aligned} \quad (11.40.6)$$

Finally, we have, from (11.34),

$$\begin{aligned} 8\pi(1-\nu) D_{ijkl}(x) &= 8\pi(1-\nu) S_{ijkl}(\lambda) + 2\nu \delta_{kl} x_i I_{I,j}(\lambda) \\ &\quad + (1-\nu) \{ \delta_{il} x_k I_{K,j}(\lambda) + \delta_{jl} x_k I_{K,i}(\lambda) + \delta_{ik} x_l I_{L,j}(\lambda) + \delta_{jk} x_l I_{L,i}(\lambda) \} \\ &\quad - \delta_{ij} x_k [I_K(\lambda) - a_I^2 I_{KI}(\lambda)]_{,l} - (\delta_{ik} x_j + \delta_{jk} x_i) [I_J(\lambda) - a_I^2 I_{IJ}(\lambda)]_{,l} \\ &\quad - (\delta_{il} x_j + \delta_{jl} x_i) [I_J(\lambda) - a_I^2 I_{IJ}(\lambda)]_{,k} - x_i x_j [I_J(\lambda) - a_I^2 I_{IJ}(\lambda)]_{,lk}, \end{aligned} \quad (11.41)$$

where

$$\begin{aligned} & 8\pi(1-\nu)S_{ijkl}(\lambda) \\ &= \delta_{ij}\delta_{kl}[2\nu I_I(\lambda) - I_K(\lambda) + a_I^2 I_{KI}(\lambda)] \\ &+ (\delta_{ik}\delta_{jl} + \delta_{jk}\delta_{il})\{a_I^2 I_{IJ}(\lambda) - I_J(\lambda) + (1-\nu)[I_K(\lambda) + I_L(\lambda)]\}. \quad (11.42) \end{aligned}$$

The above result holds for both exterior and interior points. For interior points, $\lambda = 0$ and all derivatives of I_i and I_{ij} vanish. Then D_{ijkl} becomes $S_{ijkl}(0)$, which can be found to be equal to (11.16), where the relation (11.19) is used to establish this equivalency.

Recently, Tanaka and Mura (1982) have proposed an alternative method to evaluate the elastic field for exterior points. First, obtain the stress field for the interior points of Ω and denote it by $\sigma_{ij}(\text{in})$. Next, find the stress field for the exterior points, assuming that Ω is a void and the applied stress is $-\sigma_{ij}(\text{in})$. Denote the stress field by $\sigma_{ij}(\text{out})$. Then, the stress field for the exterior points in the original problem is the sum of $\sigma_{ij}(\text{out})$ and $\sigma_{ij}(\text{in})$.

Thermal expansion with central symmetry

The thermal stress problem in an isotropic infinite body under a temperature T symmetrical with respect to a point, has been well-known and its solution is found to be

$$u = \frac{1+\nu}{1-\nu} r^{-2} \int_0^r \alpha T r^2 dr \quad (11.43)$$

(e.g., Timoshenko and Goodier 1951, p. 417) where r is the distance from the center of symmetry, u is the displacement in the r -direction, and α is the thermal expansion coefficient.

When T is constant inside a spherical domain Ω with radius a , and zero outside the sphere, we have

$$\epsilon_r = \epsilon_t = \frac{1+\nu}{3(1-\nu)} \alpha T \quad (11.44)$$

for interior points of Ω , and

$$\epsilon_r = -\frac{2}{3} \frac{1+\nu}{1-\nu} \frac{a^3}{r^3} \alpha T, \quad \epsilon_t = \frac{1}{3} \frac{1+\nu}{1-\nu} \frac{a^3}{r^3} \alpha T, \quad (11.45)$$

for exterior points, where ϵ_r and ϵ_t are the radial and tangential strains.

It will be shown that the same result can be obtained from (11.33) with (11.41), where $\epsilon_{ij}^* = \delta_{ij} \alpha T$.

The following calculation is straightforward. From (11.37), $\lambda = r^2 - a^2$. From (12.19) in the next section, $I_1(\lambda) = I_2(\lambda) = I_3(\lambda) = 4\pi a^3 / 3r^3$, $I_{ij}(\lambda) = 4\pi a^3 / 5r^5$. For interior points, $\lambda = 0$ and $I_1 = I_2 = I_3 = 4\pi / 3$, $I_{ij} = 4\pi / 5a^2$. When these results are substituted into (11.41) and (11.42), we have (11.44) or (11.45), depending on the position of x .

* 12. Ellipsoidal inclusions with polynomial eigenstrains

An ellipsoidal inclusion is considered in an isotropic infinite body. Eigenstrains in the ellipsoidal domain are given in the form of polynomials of coordinates such as

$$\epsilon_{ij}^*(x) = B_{ijk} x_k + B_{ijkl} x_k x_l + \dots, \quad (12.1)$$

where B_{ijk} , B_{ijkl} , ... are constants symmetric with respect to the free indices i , j and $B_{ijkl} = B_{ijlk}$, $B_{ijklm} = B_{ijkm}$, etc. The constant term is excluded in (12.1) since it has been discussed already in the last section.

In this section, we follow to a large extent the Ph.D. theses of Sendeckyj (1967) and Moschovidis (1975).

Both displacement fields for interior and exterior points of Ω can be expressed by (11.30),

$$u_i(x) = \frac{1}{8\pi(1-\nu)} \left\{ \Psi_{jl,jli} - 2\nu \Phi_{mm,i} - 4(1-\nu) \Phi_{il,l} \right\}, \quad (12.2)$$

where

$$\Psi_{ij}(x) = \int_{\Omega} |x - x'| \epsilon_{ij}^*(x') dx' \quad (12.3)$$

and

$$\Phi_{ij}(x) = \int_{\Omega} \frac{\epsilon_{ij}^*(x')}{|x - x'|} dx'. \quad (12.4)$$

$\Phi_{ij}(\mathbf{x})$ and $\Psi_{ij}(\mathbf{x})$ are the harmonic and biharmonic potentials due to a body Ω of density $\epsilon_{ij}^*(\mathbf{x}')$, and have the following properties:

$$\Psi_{ij,kk} = 2\Phi_{ij} \quad (12.5)$$

and

$$\Psi_{ij,kkl} = 2\Phi_{ij,ll} = \begin{cases} -8\pi\epsilon_{ij}^*(\mathbf{x}) & \text{for } \mathbf{x} \text{ in } \Omega, \\ 0 & \text{for } \mathbf{x} \text{ out of } \Omega. \end{cases} \quad (12.6)$$

The strain field can be expressed by (11.32). When (12.1) is substituted into (12.3) and (12.4), we have

$$\Psi_{ij} = B_{ijk}\psi_k + B_{ijkl}\psi_{kl} + \dots \quad (12.7)$$

and

$$\Phi_{ij} = B_{ijk}\phi_k + B_{ijkl}\phi_{kl} + \dots, \quad (12.8)$$

where the following definitions have been made:

$$\begin{aligned} \psi_{ij-k}(\mathbf{x}) &= \int_{\Omega} x'_i x'_j \cdots x'_k |\mathbf{x} - \mathbf{x}'| d\mathbf{x}', \\ \phi_{ij-k}(\mathbf{x}) &= \int_{\Omega} \frac{x'_i x'_j \cdots x'_k d\mathbf{x}'}{|\mathbf{x} - \mathbf{x}'|}. \end{aligned} \quad (12.9)$$

ψ and ϕ have the same definitions as (11.35). Substitution of (12.7) and (12.8) into (11.32) leads to

$$\epsilon_{ij}(\mathbf{x}) = D_{ijklq}(\mathbf{x})B_{klq} + D_{ijklqr}(\mathbf{x})B_{klqr} + \dots, \quad (12.10)$$

where

$$\begin{aligned} 8\pi(1-\nu)D_{ijklq} &= \psi_{q,klij} - 2\nu\delta_{kl}\phi_{q,ij} \\ &\quad - (1-\nu)[\phi_{q,kj}\delta_{il} + \phi_{q,ki}\delta_{jl} + \phi_{q,lj}\delta_{ik} + \phi_{q,li}\delta_{jk}], \end{aligned} \quad (12.11)$$

etc.

The expression for D_{ijklqr} is obtained from the above by replacing ψ_q and ϕ_q by ψ_{qr} and ϕ_{qr} , respectively.

As seen in the next subsection, the harmonic potentials in (12.9) can be written in terms of the following elliptic integrals:

$$\begin{aligned} V(\mathbf{x}) &= \pi a_1 a_2 a_3 \int_{\lambda}^{\infty} \frac{U(s)}{\Delta(s)} ds, \\ V_i(\mathbf{x}) &= \pi a_1 a_2 a_3 \int_{\lambda}^{\infty} \frac{U(s)}{(a_i^2 + s) \Delta(s)} ds, \\ V_{ij}(\mathbf{x}) &= \pi a_1 a_2 a_3 \int_{\lambda}^{\infty} \frac{U(s)}{(a_i^2 + s)(a_j^2 + s) \Delta(s)} ds, \end{aligned} \quad (12.12)$$

etc.,

where $\Delta(s) = \{(a_1^2 + s)(a_2^2 + s)(a_3^2 + s)\}^{1/2}$, $U(s) = 1 - \{x_1^2/(a_1^2 + s) + x_2^2/(a_2^2 + s) + x_3^2/(a_3^2 + s)\}$, and λ is the largest positive root of (11.37) for an exterior point \mathbf{x} . $\lambda = 0$ for interior points of Ω . According to Dyson (1891), (12.9) can be written as

$$\begin{aligned} \phi &= V, \\ \phi_n &= a_N^2 x_n V_N, \end{aligned} \quad (12.13)$$

$$\phi_{mn} = a_M^2 \left\{ x_m x_n a_N^2 V_{MN} + \frac{1}{4} \delta_{mn} [V - x_r x_r V_R - a_M^2 (V_M - x_r x_r V_{RM})] \right\},$$

and

$$\begin{aligned} \psi_{n,i} &= -\frac{1}{4} \delta_{in} a_N^2 \left\{ (V - x_r x_r V_R) - a_N^2 (V_N - x_r x_r V_{RN}) \right\} \\ &\quad + a_N^2 x_n x_i (V_I - a_N^2 V_{IN}), \end{aligned} \quad (12.14)$$

$$\begin{aligned} \psi_{mn,i} &= a_M^2 a_N^2 \left\{ -\frac{1}{4} (\delta_{mi} x_n + \delta_{ni} x_m) [V_M - x_r x_r V_{RM} - a_N^2 (V_{NM} - x_r x_r V_{RNM})] \right. \\ &\quad \left. + x_m x_n x_i (V_{MN} - a_I^2 V_{IMN}) \right\} \\ &\quad + \frac{1}{4} a_M^2 \delta_{mn} x_i \left\{ (V - x_r x_r V_R) - (a_I^2 + a_M^2) (V_M - x_r x_r V_{RM}) \right. \\ &\quad \left. + a_I^4 (V_{MI} - x_r x_r V_{RMI}) \right\}. \end{aligned}$$

In the above expressions, the summation convention demonstrated by (11.39)

was used. By definition, the V -integrals in (12.12) and the I -integrals in (11.36) are related,

$$\begin{aligned} V(\mathbf{x}) &= \frac{1}{2}\{I(\lambda) - x_r x_r I_R(\lambda)\}, \\ V_i(\mathbf{x}) &= \frac{1}{2}\{I_i(\lambda) - x_r x_r I_{Ri}(\lambda)\}, \\ V_{ij}(\mathbf{x}) &= \frac{1}{2}\{I_{ij}(\lambda) - x_r x_r I_{Rij}(\lambda)\}, \end{aligned} \quad (12.15)$$

etc.

When \mathbf{x} is an interior point of Ω , then $\lambda = 0$, and I, I_i, I_{ij}, \dots are constants and all their derivatives vanish. Thus, the first derivatives of the V -integrals are linear functions of \mathbf{x} , and the second derivatives are constants. Therefore, $D_{ijklq}(\mathbf{x})$ becomes a homogeneous linear function of \mathbf{x} and $D_{ijklqr}(\mathbf{x})$ becomes a sum of constants and quadratic functions of \mathbf{x} , that is, for interior points \mathbf{x} ,

$$\begin{aligned} D_{ijklq}(\mathbf{x}) &= D_{ijklq,m}(0)x_m, \\ D_{ijklqr}(\mathbf{x}) &= D_{ijklqr}(0) + \frac{1}{2}D_{ijklqr,mn}(0)x_m x_n. \end{aligned} \quad (12.16)$$

The \mathbf{x} dependency of higher order $D_{ijkl\dots}$ can be similarly investigated, and it is concluded that $\epsilon_{ij}(\mathbf{x})$ for interior points becomes an inhomogeneous polynomial of \mathbf{x} whose terms are of degree $n, (n-2), (n-4), \dots$, if $\epsilon_{kl}^*(\mathbf{x})$ is a homogeneous polynomial of \mathbf{x} with degree n . This general conclusion was first pointed out by Eshelby (1961). Asaro and Barnett (1975) reached the same conclusion for anisotropic materials.

* The I -integrals

The I -integrals in (11.36) can be expressed by the first and second elliptic integrals defined by (11.18); for $a_1 > a_2 > a_3$,

$$\begin{aligned} I(\lambda) &= 4\pi a_1 a_2 a_3 (a_1^2 - a_3^2)^{-1/2} F(\theta, k), \\ I_1(\lambda) &= 4\pi a_1 a_2 a_3 (a_1^2 - a_2^2)^{-1} (a_1^2 - a_3^2)^{-1/2} \{F(\theta, k) - E(\theta, k)\}, \\ I_2(\lambda) &= 4\pi a_1 a_2 a_3 \left\{ (a_1^2 - a_3^2)^{1/2} (a_1^2 - a_2^2)^{-1} (a_2^2 - a_3^2)^{-1} E(\theta, k) \right. \\ &\quad \left. - (a_1^2 - a_2^2)^{-1} (a_1^2 - a_3^2)^{-1/2} F(\theta, k) \right. \\ &\quad \left. - (a_2^2 - a_3^2)^{-1} (a_3^2 + \lambda)^{1/2} (a_1^2 + \lambda)^{-1/2} (a_2^2 + \lambda)^{-1/2} \right\}, \end{aligned} \quad (12.17)$$

$$I_3(\lambda) = 4\pi a_1 a_2 a_3 (a_2^2 - a_3^2)^{-1} (a_1^2 - a_3^2)^{-1/2} \\ \times \left\{ (a_2^2 + \lambda)^{1/2} (a_1^2 - a_3^2)^{1/2} (a_3^2 + \lambda)^{-1/2} (a_1^2 + \lambda)^{-1/2} - E(\theta, k) \right\},$$

where F , E , θ and k have been defined in (11.18).

The other higher order I -integrals can be expressed by I_i , using the following relations:

$$\begin{aligned} I_1(\lambda) + I_2(\lambda) + I_3(\lambda) &= 4\pi a_1 a_2 a_3 / \Delta(\lambda), \\ I_{12}(\lambda) &= [I_2(\lambda) - I_1(\lambda)] / (a_1^2 - a_2^2), \\ 3I_{11}(\lambda) &= 4\pi a_1 a_2 a_3 / (a_1^2 + \lambda) \Delta(\lambda) - I_{12}(\lambda) - I_{13}(\lambda), \\ I_{123}(\lambda) &= [I_{23}(\lambda) - I_{13}(\lambda)] / (a_1^2 - a_2^2), \\ I_{112}(\lambda) &= [I_{12}(\lambda) - I_{11}(\lambda)] / (a_1^2 - a_2^2), \\ I_{113}(\lambda) &= [I_{13}(\lambda) - I_{11}(\lambda)] / (a_1^2 - a_3^2), \\ 5I_{111}(\lambda) &= 4\pi a_1 a_2 a_3 / (a_1^2 + \lambda)^2 \Delta(\lambda) - I_{112}(\lambda) - I_{113}(\lambda), \\ &\vdots \end{aligned} \tag{12.18}$$

where

$$2 \frac{d}{ds} \{1/\Delta(s)\} = - \left\{ (a_1^2 + s)^{-1} + (a_2^2 + s)^{-1} + (a_3^2 + s)^{-1} \right\} / \Delta(s),$$

$$\Delta(\lambda) = \{(a_1^2 + \lambda)(a_2^2 + \lambda)(a_3^2 + \lambda)\}^{1/2}.$$

The cyclic permutation with respect to (1, 2, 3) can be applied in (12.18).

The I -integrals become elementary functions for special cases as shown in the following.

* *Sphere* ($a_1 = a_2 = a_3 = a$)

$$\begin{aligned} I(\lambda) &= 4\pi a^3 / (a^2 + \lambda)^{1/2}, \\ I_i(\lambda) &= 4\pi a^3 / 3(a^2 + \lambda)^{3/2}, \\ \underbrace{I_{ij\dots k}}_n(\lambda) &= \frac{4\pi a^3}{(2n+1)(a^2 + \lambda)^{n+1/2}}. \end{aligned} \tag{12.19}$$

* *Elliptic cylinder* ($a_3 \rightarrow \infty$)

$$\begin{aligned} I_1(\lambda) &= \frac{4\pi a_1 a_2}{a_2^2 - a_1^2} \left\{ \frac{(a_2^2 + \lambda)^{1/2}}{(a_1^2 + \lambda)^{1/2}} - 1 \right\}, \\ I_2(\lambda) &= \frac{4\pi a_1 a_2}{a_1^2 - a_2^2} \left\{ \frac{(a_1^2 + \lambda)^{1/2}}{(a_2^2 + \lambda)^{1/2}} - 1 \right\}, \\ I_3(\lambda) &= 0. \end{aligned} \quad (12.20)$$

* *Oblate spheroid* ($a_1 = a_2 > a_3$)

$$I(\lambda) = \frac{4\pi a_1^2 a_3}{\sqrt{a_1^2 - a_3^2}} \left[\frac{\pi}{2} - \text{arc tan} \left(\frac{\sqrt{a_3^2 + \lambda}}{\sqrt{a_1^2 - a_3^2}} \right) \right] = \frac{4\pi a_1^2 a_3}{\sqrt{a_1^2 - a_3^2}} \text{arc cos } b,$$

$$I_1(\lambda) = I_2(\lambda) = 2\pi a_1^2 a_3 (\text{arc cos } b - bd) / (a_1^2 - a_3^2)^{3/2}, \quad (12.21)$$

$$I_3(\lambda) = \frac{4\pi a_1^2 a_3}{\Delta(\lambda)} - 2I_1(\lambda) = 4\pi a_1^2 a_3 \left(\frac{d}{b} - \text{arc cos } b \right) / (a_1^2 - a_3^2)^{3/2},$$

where $b = \sqrt{(a_3^2 + \lambda) / (a_1^2 + \lambda)}$ and $d = \sqrt{(a_1^2 - a_3^2) / (a_1^2 + \lambda)}$.

* *Prolate spheroid* ($a_1 > a_2 = a_3$)

$$I(\lambda) = -\frac{2\pi a_1 a_3^2}{\sqrt{a_1^2 - a_3^2}} \ln \frac{\sqrt{a_1^2 + \lambda} - \sqrt{a_1^2 - a_3^2}}{\sqrt{a_1^2 + \lambda} + \sqrt{a_1^2 - a_3^2}} = \frac{4\pi a_1 a_3^2}{\sqrt{a_1^2 - a_3^2}} \text{arc cosh } \bar{b},$$

$$I_1(\lambda) = 4\pi a_1 a_3^2 (\text{arc cosh } \bar{b} - \bar{d}/\bar{b}) / (a_1^2 - a_3^2)^{3/2}, \quad (12.22)$$

$$I_2(\lambda) = I_3(\lambda) = 2\pi a_1 a_3^2 (\bar{b}\bar{d} - \text{arc cosh } \bar{b}) / (a_1^2 - a_3^2)^{3/2},$$

where $\bar{b} = \sqrt{(a_1^2 + \lambda) / (a_3^2 + \lambda)}$ and $\bar{d} = \sqrt{(a_1^2 - a_3^2) / (a_3^2 + \lambda)}$.

Table 12.1 Limits of the I -integrals as $a_3 \rightarrow 0$ for $\lambda \neq 0$ and $\lambda = 0$.

$\lim_{a_3 \rightarrow 0} (\dots)$	$\lambda \neq 0$	$\lambda = 0$
$I_{ij}(\lambda)$ with $i, j, \dots \neq 3$	0	0
$I_3(\lambda)$	0	4π
$I_{13}(\lambda)$	0	$4\pi/a_1^2$
$I_{23}(\lambda)$	0	$4\pi/a_2^2$
$I_{33}(\lambda)$	0	∞
$a_3^2 I_{33}(\lambda)$	0	$4\pi/3$
$I_{113}(\lambda)$	0	$4\pi/a_1^4$
$I_{223}(\lambda)$	0	$4\pi/a_2^4$
$I_{123}(\lambda)$	0	$4\pi/a_1^2 a_2^2$
$I_{133}(\lambda)$	0	∞
$I_{233}(\lambda)$	0	∞
$a_3^2 I_{133}(\lambda)$	0	$4\pi/3 a_1^2$
$a_3^2 I_{233}(\lambda)$	0	$4\pi/3 a_2^2$
$I_{333}(\lambda)$	0	∞
$a_3^4 I_{333}(\lambda)$	0	$4\pi/5$
$I_{1113}(\lambda)$	0	$4\pi/a_1^6$
$I_{2223}(\lambda)$	0	$4\pi/a_2^6$
$I_{1123}(\lambda)$	0	$4\pi/a_1^4 a_2^2$
$I_{1223}(\lambda)$	0	$4\pi/a_1^2 a_2^4$
...

* *Elliptical plate* ($a_1 \geq a_2 > a_3$, $a_3 \rightarrow 0$)

$$\lim_{a_3 \rightarrow 0} I_{ij}(\lambda) = 0 \quad \text{for } i, j, \dots \neq 3. \quad (12.23)$$

Other cases are listed in Table 12.1.

* *The Ferrers and Dyson formula*

Ferrers (1877) and Dyson (1891) investigated integration of the harmonic potential of an ellipsoid,

$$P(x) = \int_{\Omega} \frac{\rho(x') dx'}{|x - x'|}. \quad (12.24)$$

Their results have already been used in (11.38), (12.13), and (12.14).

A general result, given in Dyson's paper, states that if

$$\rho(\mathbf{x}) = \frac{m}{\pi a_1 a_2 a_3} \left(1 - \frac{x_i x_i}{a_I^2}\right)^{m-1} f\left(\frac{x_1}{a_1}, \frac{x_2}{a_2}, \frac{x_3}{a_3}\right) \quad \text{for } m > 0, \quad (12.25)$$

then

$$\begin{aligned} P(\mathbf{x}) &= \int_{\lambda}^{\infty} U^m(s) \sum_{n=0}^{\infty} \frac{s^n U^n(s)}{2^{2n} n! (n+m)! / m!} \\ &\quad \times L^n f\left(\frac{a_1 x_1}{a_1^2 + s}, \frac{a_2 x_2}{a_2^2 + s}, \frac{a_3 x_3}{a_3^2 + s}\right) \frac{ds}{\Delta(s)}, \end{aligned} \quad (12.26)$$

where

$$\begin{aligned} U(s) &= 1 - \frac{x_i x_i}{a_I^2 + s}, \quad \Delta(s) = \sqrt{(a_1^2 + s)(a_2^2 + s)(a_3^2 + s)}, \\ L &= \frac{a_I^2 + s}{a_I^2} \frac{\partial}{\partial x_i} \frac{\partial}{\partial x_i}. \end{aligned} \quad (12.27)$$

Furthermore, λ is the largest positive root of

$$\frac{x_i x_i}{a_I^2 + \lambda} = 1; \quad \text{i.e., } U(\lambda) = 0. \quad (12.28)$$

The summation convention in (11.39) has been used in the above equations.

All integrals in (12.9) can be expressed in this type of integral:

$$\phi_{ij}(\mathbf{x}) = \int_{\Omega} \frac{x'_i x'_j \cdots d\mathbf{x}'}{|\mathbf{x} - \mathbf{x}'|}, \quad (12.29)$$

since $|\mathbf{x} - \mathbf{x}'| = (x_i x_i - 2x_i x'_i + x'_i x'_i)/|\mathbf{x} - \mathbf{x}'|$. Comparing (12.25) with $x_i x_j \cdots$, we have $m = 1$ and

$$\begin{aligned} \phi_{ij}(\mathbf{x}) &= \pi a_1 a_2 a_3 a_I^2 a_J^2 \cdots \int_{\lambda}^{\infty} U(s) \sum_{n=0}^{\infty} \frac{s^n U^n(s)}{2^{2n} n! (n+1)!} \\ &\quad \times L^n \left[\frac{x_i}{a_I^2 + s} \frac{x_j}{a_J^2 + s} \cdots \right] \frac{ds}{\Delta(s)}. \end{aligned} \quad (12.30)$$

When the differential operator L^n in (12.30) is performed, the required integrals shown in (11.38), (12.13) and (12.14) can easily be obtained.

13. Energies of inclusions

Consider a finite body D containing inclusions Ω . The material is homogeneous, and anisotropic or isotropic. Ω is the sum of domains occupied by the inclusions. The volume of Ω is denoted by V .

Elastic strain energy

First we consider the case when the body D is free from any external force and surface constraint, but eigenstrains ϵ_{ij}^* are prescribed in Ω . The (elastic) strain energy is

$$W^* = \frac{1}{2} \int_D \sigma_{ij} e_{ij} \, dD, \quad (13.1)$$

where $e_{ij} = \dot{\epsilon}_{ij} - \epsilon_{ij}^*$ and $\epsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})$. Integrating by parts we obtain

$$\int_D \sigma_{ij} u_{i,j} \, dD = \int_S \sigma_{ij} u_i n_j \, dS - \int_D \sigma_{ij,j} u_i \, dD = 0, \quad (13.2)$$

since $\sigma_{ij} n_j = 0$ (free surface condition) on S and $\sigma_{ij,j} = 0$ (equilibrium condition) in D , where S is the boundary of D and n_i is the exterior unit vector normal to S . Therefore, we have

$$W^* = -\frac{1}{2} \int_\Omega \sigma_{ij} \epsilon_{ij}^* \, dD. \quad (13.3)$$

If Ω is an ellipsoidal inclusion and ϵ_{ij}^* is uniform, σ_{ij} in Ω is also uniform. Then, (13.3) is written as

$$W^* = -\frac{1}{2} V \sigma_{ij} \epsilon_{ij}^*. \quad (13.4)$$

σ_{ij} has been obtained in Section 11 for isotropic, and in Section 17 for anisotropic materials.

If Ω is the sum of two inclusions Ω_1 and Ω_2 , (13.3) is written as

$$W^* = -\frac{1}{2} \left[\int_{\Omega_1} (\sigma_{ij}^{(1)} + \sigma_{ij}^{(2)}) \epsilon_{ij}^{(1)} \, dD + \int_{\Omega_2} (\sigma_{ij}^{(1)} + \sigma_{ij}^{(2)}) \epsilon_{ij}^{(2)} \, dD \right], \quad (13.4.1)$$

where $\epsilon_{ij}^{(1)}$ and $\epsilon_{ij}^{(2)}$ are eigenstrains given in Ω_1 and Ω_2 , respectively, $\sigma_{ij}^{(1)}$ is the stress caused by $\epsilon_{ij}^{(1)}$, and $\sigma_{ij}^{(2)}$ is the stress caused by $\epsilon_{ij}^{(2)}$. It is shown that

$$\int_{\Omega_1} \sigma_{ij}^{(2)} \epsilon_{ij}^{(1)} \, dD = \int_{\Omega_2} \sigma_{ij}^{(1)} \epsilon_{ij}^{(2)} \, dD \quad (13.4.2)$$

as follows. Since $\epsilon_{ij}^{(1)} = 0$ in $D - \Omega$, it holds that

$$\int_{\Omega_1} \sigma_{ij}^{(2)} \epsilon_{ij}^{(1)} \, dD = \int_D \sigma_{ij}^{(2)} \epsilon_{ij}^{(1)} \, dD = \int_D \sigma_{ij}^{(2)} (\epsilon_{ij}^{(1)} - u_{i,j}^{(1)} + u_{i,j}^{(1)}) \, dD, \quad (13.4.3)$$

where $u_i^{(1)}$ is the displacement caused by $\epsilon_{ij}^{(1)}$. We know that $\sigma_{ij}^{(2)}(\epsilon_{ij}^{(1)} - u_{i,j}^{(1)}) = -\sigma_{ij}^{(2)}e_{ij}^{(1)}$ and $\int_D \sigma_{ij}^{(2)} u_{i,j}^{(1)} \, dD = 0$, where $e_{ij}^{(1)}$ is the elastic strain corresponding to $\sigma_{ij}^{(1)}$. Therefore, we have

$$\int_{\Omega_1} \sigma_{ij}^{(2)} \epsilon_{ij}^{(1)} \, dD = - \int_D \sigma_{ij}^{(2)} e_{ij}^{(1)} \, dD. \quad (13.4.4)$$

Similarly,

$$\int_{\Omega_2} \sigma_{ij}^{(1)} \epsilon_{ij}^{(2)} \, dD = - \int_D \sigma_{ij}^{(1)} e_{ij}^{(2)} \, dD. \quad (13.4.5)$$

Since $\sigma_{ij}^{(2)} e_{ij}^{(1)} = C_{ijkl} e_{kl}^{(2)} e_{ij}^{(1)} = \sigma_{kl}^{(1)} e_{kl}^{(2)}$, the equality (13.4.2) is proven. Due to this fact, (13.4.1) is written as

$$W^* = -\frac{1}{2} \left[\int_{\Omega_1} \sigma_{ij}^{(1)} \epsilon_{ij}^{(1)} \, dD + \int_{\Omega_2} \sigma_{ij}^{(2)} \epsilon_{ij}^{(2)} \, dD + 2 \int_{\Omega_1} \sigma_{ij}^{(2)} \epsilon_{ij}^{(1)} \, dD \right]. \quad (13.4.6)$$

Consider now the case when the body D , containing inclusions Ω , is additionally subjected to external surface tractions F_i . The displacement field is the sum of u_i^0 and u_i , where u_i^0 is the displacement if F_i act alone in the absence of eigenstrains, and u_i is due to the eigenstrains prescribed in the inclusions. Then, the (elastic) strain energy is

$$W^* = \frac{1}{2} \int_D (\sigma_{ij}^0 + \sigma_{ij}) (u_{i,j}^0 + u_{i,j} - \epsilon_{ij}^*) \, dD, \quad (13.5)$$

where $\sigma_{ij}^0 = C_{ijkl}u_{k,l}^0$. Since $\sigma_{ij,j} = 0$ in D and $\sigma_{ij}n_j = 0$ on S , the integration by parts gives

$$\int_D \sigma_{ij} (u_{i,j}^0 + u_{i,j}) \, dD = 0. \quad (13.6)$$

It is also seen that

$$\int_D \sigma_{ij}^0 (u_{i,j} - \epsilon_{ij}^*) \, dD = 0, \quad (13.7)$$

since $\sigma_{ij}^0 (u_{i,j} - \epsilon_{ij}^*) = C_{ijkl}u_{k,l}^0 (u_{i,j} - \epsilon_{ij}^*) = u_{k,l}^0 \sigma_{kl} = \sigma_{ij}u_{i,j}^0$ and $\int_D \sigma_{ij}u_{i,j}^0 \, dD = 0$.

The elastic strain energy, therefore, becomes

$$W^* = \frac{1}{2} \int_D \sigma_{ij}^0 u_{i,j}^0 \, dD - \frac{1}{2} \int_{\Omega} \sigma_{ij} \epsilon_{ij}^* \, dD. \quad (13.8)$$

It is interesting to note that the elastic strain energy is the sum of the two energies caused respectively by F_i and ϵ_{ij}^* . This is a Colonnetti's theorem which will be discussed again in Section 25.

Interaction energy

The total potential energy of a body subjected to a surface traction F_i and eigenstrains ϵ_{ij}^* in Ω is defined by

$$W = W^* - \int_S F_i (u_i^0 + u_i) \, dS, \quad (13.9)$$

where $\sigma_{ij}^0 n_j = F_i$ on S , and W^* is defined by (13.5). If $\epsilon_{ij}^* = 0$ in D , W becomes

$$W_0 = \frac{1}{2} \int_D \sigma_{ij}^0 u_{i,j}^0 \, dD - \int_S F_i u_i^0 \, ds. \quad (13.10)$$

If $F_i = 0$ on S , W becomes

$$W_1 = \frac{1}{2} \int_D \sigma_{ij} (u_{i,j} - \epsilon_{ij}^*) \, dD = -\frac{1}{2} \int_{\Omega} \sigma_{ij} \epsilon_{ij}^* \, dD. \quad (13.11)$$

The interaction energy between ϵ_{ij}^* and F_i is defined by

$$\Delta W = W - W_0 - W_1. \quad (13.12)$$

Then, we have

$$\Delta W = - \int_S F_i u_i \, dS = - \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* \, dD, \quad (13.13)$$

since $\int_S F_i u_i \, dS = \int_S \sigma_{ij}^0 n_j u_i \, dS = \int_D \sigma_{ij}^0 u_{i,j} \, dD = \int_D \sigma_{ij}^0 (u_{i,j} - \epsilon_{ij}^*) \, dD + \int_D \sigma_{ij}^0 \epsilon_{ij}^* \, dD$ due to (13.7).

If Ω is an ellipsoidal inclusion and ϵ_{ij}^* is uniform,

$$\Delta W = - V \sigma_{ij}^0 \epsilon_{ij}^*. \quad (13.14)$$

Under a constant temperature condition, the elastic strain energy of a body is the Helmholtz free energy of the body. The total potential energy (13.9) is the sum of the elastic strain energy of the body and the potential energy of an external force and corresponds to the Gibbs free energy of the body. This thermodynamic discussion was given by Eshelby (1956). One should mention that (13.11) can also be considered to be the Gibbs free energy of the body when an external force is absent and an inclusion is a source of the stress field. (13.10) is the Gibbs free energy of the body when only an external force is a source of the stress field. Therefore, the interaction term (13.12) is an extra part of the Gibbs free energy of the body, produced by the coexistence of the two sources of the stress field. It is often called the interaction (Gibbs-free) energy between ϵ_{ij}^* and F_i .

Suppose that a body D is subjected to F_i alone. If eigenstrains due to inclusions are introduced in Ω , the free energy will increase by

$$\Delta \bar{W} = W - W_0. \quad (13.15)$$

This $\Delta \bar{W}$ is an interesting quantity, particularly in fracture mechanics, as explained in Chapter 5. Equation (13.15) can be written as

$$\Delta \bar{W} = - \frac{1}{2} \int_{\Omega} \sigma_{ij} \epsilon_{ij}^* \, dD - \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* \, dD. \quad (13.16)$$

It should be emphasized that energies (13.3), (13.8), (13.11), (13.13) and (13.16) are expressed by the quantities defined inside Ω . Energy calculations, therefore, become easier.

Elastic strain energies in an isotropic infinite body, which are due to constant eigenstrains prescribed in inclusions of indicated geometries, are presented below.

Strain energy due to a spherical inclusion

$$W^* = \frac{8\pi a^3 \mu}{45(1-\nu)} \left\{ 4(\epsilon_{11}^{*2} + \epsilon_{22}^{*2} + \epsilon_{33}^{*2}) + (5\nu + 1)(\epsilon_{11}^* \epsilon_{22}^* + \epsilon_{22}^* \epsilon_{33}^* + \epsilon_{33}^* \epsilon_{11}^*) + (7 - 5\nu)(\epsilon_{12}^{*2} + \epsilon_{23}^{*2} + \epsilon_{31}^{*2}) \right\}, \quad (13.17)$$

where a is the radius of the sphere.

Elliptic cylinder ($a_3 \rightarrow \infty$)

$$\begin{aligned} W^* = & \frac{\pi a_1 a_2 \mu}{2(1-\nu)} \left[\left\{ 2 - \frac{a_2^2 + 2a_1 a_2}{(a_1 + a_2)^2} - \frac{a_2}{a_1 + a_2} \right\} \epsilon_{11}^{*2} \right. \\ & + \left\{ 2 - \frac{a_1^2 + 2a_1 a_2}{(a_1 + a_2)^2} - \frac{a_1}{a_1 + a_2} \right\} \epsilon_{22}^{*2} \\ & + 2\epsilon_{33}^{*2} + \frac{2a_1 a_2}{(a_1 + a_2)^2} \epsilon_{11}^* \epsilon_{22}^* + \frac{4\nu a_2}{a_1 + a_2} \epsilon_{22}^* \epsilon_{33}^* + \frac{4\nu a_1}{a_1 + a_2} \epsilon_{33}^* \epsilon_{11}^* \\ & \left. + \frac{4a_1 a_2}{(a_1 + a_2)^2} \epsilon_{12}^{*2} + 4(1-\nu) \frac{a_2}{a_1 + a_2} \epsilon_{23}^{*2} + 4(1-\nu) \frac{a_1}{a_1 + a_2} \epsilon_{31}^{*2} \right] \end{aligned} \quad (13.18)$$

which is defined per unit length of the cylinder.

Penny-shaped flat ellipsoid ($a_1 = a_2 = a$, $a_3 \ll a$)

$$W^* = \frac{4}{3}\pi a^2 a_3 \mu \left\{ (\epsilon_{11}^{*2} + \epsilon_{22}^{*2})/(1-\nu) + 2\nu \epsilon_{11}^* \epsilon_{22}^* / (1-\nu) + 2\epsilon_{12}^{*2} \right\}. \quad (13.19)$$

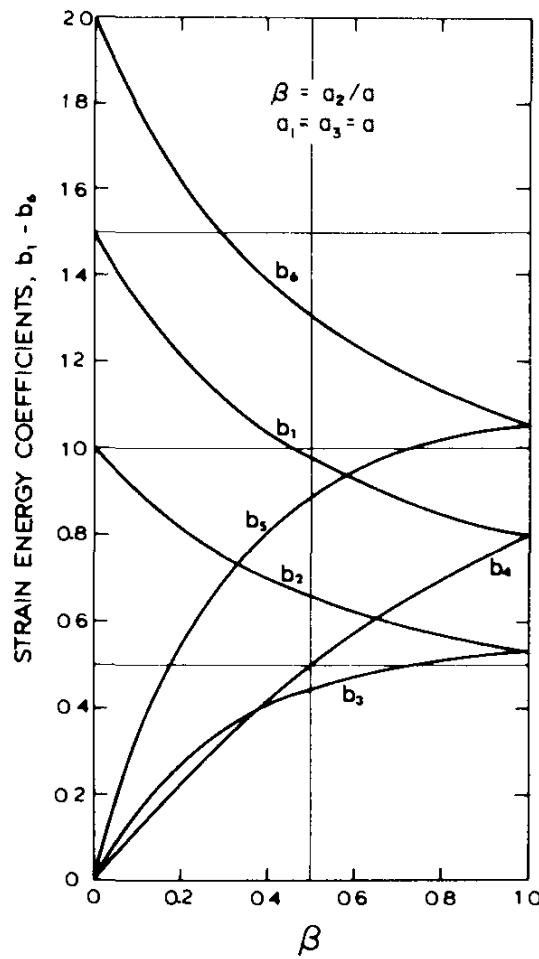


Fig. 13.1 Strain energy coefficients for an oblate spheroidal inclusion

Spheroid

For $a_1 = a_2 = a$, $\beta = a_3/a$,

$$W^* = \frac{4}{3}\pi a^2 a_3 \mu \left\{ b_1 (\epsilon_{11}^{*2} + \epsilon_{22}^{*2}) + b_2 \epsilon_{11}^* \epsilon_{22}^* + b_3 (\epsilon_{11}^* + \epsilon_{22}^*) \epsilon_{33}^* + b_4 \epsilon_{33}^{*2} + b_5 (\epsilon_{32}^{*2} + \epsilon_{31}^{*2}) + b_6 \epsilon_{12}^* \right\}. \quad (13.20)$$

For $a_3 = a_1 = a$, $\beta = a_2/a$,

$$W^* = \frac{4}{3}\pi a^2 a_2 \mu \left\{ b_1 (\epsilon_{11}^{*2} + \epsilon_{33}^{*2}) + b_2 \epsilon_{11}^* \epsilon_{33}^* + b_3 (\epsilon_{11}^* + \epsilon_{33}^*) \epsilon_{22}^* + b_4 \epsilon_{22}^{*2} + b_5 (\epsilon_{23}^{*2} + \epsilon_{21}^{*2}) + b_6 \epsilon_{13}^* \right\}. \quad (13.20.1)$$

Numerical values of b_i have been calculated by Shibata and Ono (1975) as functions of β for oblate spheroids as shown in Fig. 13.1.

It is interesting to note that for dilatational eigenstrains $\epsilon_{ij}^* = \delta_{ij}\epsilon^*$, the elastic strain energy per unit volume of inclusions is a constant independent of the shape of inclusions,

$$W^*/V = 2\mu(\epsilon^*)^2(1 + \nu)/(1 - \nu), \quad (13.21)$$

where $V = \frac{4}{3}\pi a_1 a_2 a_3$.

The above result (13.21) holds for any shape of inclusion Ω with volume V . This is because when $\epsilon_{ij}^* = \delta_{ij}\epsilon^*$, (2.7) and (11.33) lead to

$$\sigma_{ii} = \{(2\mu + 3\lambda)D_{iijj} - (6\mu + 9\lambda)\}\epsilon^*. \quad (13.22)$$

From (11.34) we have

$$D_{iijj} = (1 + \nu)/(1 - \nu), \quad (13.23)$$

where

$$\begin{aligned} \psi_{,jj} &= 2\phi, \\ \phi_{,ii} &= -4\pi \quad \text{in } \Omega, \\ &= 0 \quad \text{outside } \Omega, \end{aligned} \quad (13.24)$$

are used. Equation (13.24) follows directly from (12.5) and (12.6). Thus, it can be said that the hydrostatic pressure $\frac{1}{3}\sigma_{ii}$ is uniform for any shape of inclusion when the eigenstrain is dilatational. It becomes

$$\begin{aligned} \sigma_{ii} &= -4\mu \frac{1 + \nu}{1 - \nu} \epsilon^* \text{ in } \Omega, \\ &= 0 \quad \text{outside } \Omega. \end{aligned} \quad (13.25)$$

The elastic strain energy (13.3) becomes

$$W^* = -\frac{1}{2}\sigma_{ii}\epsilon^*V \quad (13.26)$$

which leads to (13.21).

The above results (13.25) hold also when $\epsilon_{ij}^* = \delta_{ij}\epsilon^*$ is a function of x . From (2.8) and (12.2) we have

$$\begin{aligned} \sigma_{ii} &= \frac{2\mu(1 + \nu)}{1 - 2\nu}(\epsilon_{ii} - \epsilon_{ii}^*), \\ \epsilon_{ii} &= \frac{-(1 + \nu)}{12\pi(1 - \nu)}\Phi_{mm,ii}, \end{aligned} \quad (13.27)$$

where (12.5) and $\Phi_{ij} = \frac{1}{3}\delta_{ij}\Phi_{mm}$ are used. Furthermore (12.6) leads to

$$\begin{aligned}\sigma_{ii} &= \frac{-4\mu(1+\nu)}{1-\nu}\epsilon^*(x) \text{ in } \Omega \\ &= 0 \quad \text{outside } \Omega.\end{aligned}\tag{13.28}$$

Then, the elastic strain energy (13.3) becomes

$$W^* = \frac{2\mu(1+\nu)}{1-\nu} \int_{\Omega} \{\epsilon^*(x)\}^2 dD.\tag{13.29}$$

In view of (13.28), two separate inclusions, Ω_1 and Ω_2 , with pure dilatational eigenstrains, $\epsilon_{ij}^* = \delta_{ij}\epsilon^*$, do not interact. For anisotropic materials, (13.28) does not hold generally and, therefore, there exists an interaction between two dilatational inclusions. This problem is discussed by considering specific examples in Section 26; see also Sines and Kikuchi (1958).

If the inclusion is an inhomogeneous inclusion with dilatational eigenstrain $\epsilon_{ij}^P = \delta_{ij}\epsilon^P$, the elastic strain energy depends on the shape of the inclusion even if the elastic moduli of the matrix and inhomogeneity are isotropic. As seen in (22.29) or (22.30), the equivalent eigenstrain ϵ_{ij}^{**} defined in (22.14) and (22.15) is not purely dilatational. Because of this, the hydrostatic stress in the inhomogeneous inclusion depends on its shape and (13.26) is not valid. These facts should be kept in mind when one discusses the elastic state of precipitates.

* 14. Cuboidal inclusions

The elastic fields due to a cuboidal inclusion with uniform eigenstrains have been calculated by Faivre (1964), Sankaran and Laird (1976), Lee and Johnson (1977), and Chiu (1977). The solid is assumed to be an infinite isotropic elastic medium. The calculation is interesting, because it shows how the stress distribution for interior points deviates from the result for the ellipsoidal inclusion where the stress distribution is uniform.

The cuboidal inclusion is shown in Fig. 14.1. Expression (6.2) is used, and ϵ_{ij}^* is constant in the cuboidal domain Ω . The integral in (6.2) with respect to x' is easily performed and we have

$$\begin{aligned}u_{i,q}(x) &= (i/8\pi^3)\epsilon_{mn}^* \sum_{p=1}^8 (-1)^p \int_{-\infty}^{\infty} C_{jlmn} \xi_l \xi_q N_{ij}(\xi) \\ &\quad \times \{\xi_1 \xi_2 \xi_3 D(\xi)\}^{-1} \exp(i\xi \cdot c_p) d\xi,\end{aligned}\tag{14.1}$$

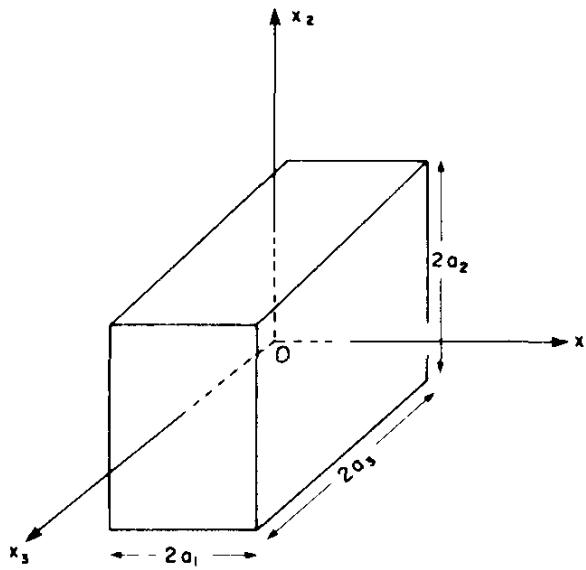


Fig. 14.1 Cuboidal inclusion

where

$$\begin{aligned}
 c_1 &= (x_1 - a_1, x_2 - a_2, x_3 - a_3), \\
 c_2 &= (x_1 + a_1, x_2 - a_2, x_3 - a_3), \\
 c_3 &= (x_1 + a_1, x_2 + a_2, x_3 - a_3), \\
 c_4 &= (x_1 - a_1, x_2 + a_2, x_3 - a_3), \\
 c_5 &= (x_1 - a_1, x_2 + a_2, x_3 + a_3), \\
 c_6 &= (x_1 - a_1, x_2 - a_2, x_3 + a_3), \\
 c_7 &= (x_1 + a_1, x_2 - a_2, x_3 + a_3), \\
 c_8 &= (x_1 + a_1, x_2 + a_2, x_3 + a_3).
 \end{aligned} \tag{14.2}$$

The following integrals are used for the integrations (14.1), and a detailed calculation is given by Chiu (1977):

$$\begin{aligned}
 \int_{-\infty}^{\infty} \frac{\xi_1 \xi_2 \exp(i\xi \cdot x) d\xi}{(\xi_1^2 + \xi_2^2 + \xi_3^2)^2} &= -\frac{\pi^2 x_1 x_2}{R^3}, \\
 i \int_{-\infty}^{\infty} \frac{\xi_1 \xi_2 \exp(i\xi \cdot x) d\xi}{\xi_3 (\xi_1^2 + \xi_2^2 + \xi_3^2)^2} &= \frac{\pi^2 x_1 x_2 x_3}{(x_1^2 + x_2^2) R}, \\
 \int_{-\infty}^{\infty} \frac{\xi_1^2 \exp(i\xi \cdot x) d\xi}{(\xi_1^2 + \xi_2^2 + \xi_3^2)^2} &= \pi^2 \frac{x_2^2 + x_3^2}{R^3},
 \end{aligned} \tag{14.3}$$

$$\begin{aligned}
& -i \int_{-\infty}^{\infty} \frac{\xi_1^2 \exp(i\xi \cdot \mathbf{x}) d\xi}{\xi_3 (\xi_1^2 + \xi_2^2 + \xi_3^2)^2} \\
&= \pi^2 \left[\operatorname{sgn}(x_3) \log \frac{R + |x_3|}{(x_1^2 + x_2^2)^{1/2}} - \frac{x_1^2 x_3}{(x_1^2 + x_2^2) R} \right], \\
& i \int_{-\infty}^{\infty} \frac{\xi_1^3 \exp(i\xi \cdot \mathbf{x}) d\xi}{\xi_2 \xi_3 (\xi_1^2 + \xi_2^2 + \xi_3^2)^2} \\
&= 2\pi^2 \left\{ \tan^{-1} \frac{x_2 x_3}{x_1 R} - \frac{x_1 x_2 x_3}{2R} \left(\frac{1}{x_1^2 + x_2^2} + \frac{1}{x_1^2 + x_3^2} \right) \right\},
\end{aligned}$$

where $R = (x_1^2 + x_2^2 + x_3^2)^{1/2}$.

The expression (14.1) can be reduced to

$$\begin{aligned}
2\mu u_{i,q}(\mathbf{x}) &= (\mu/4\pi^3) \sum_{n=1}^8 (-1)^n \left[\frac{\nu}{1-\nu} \epsilon_{kk}^* D_{iqmm}(\mathbf{c}_n) + 2\epsilon_{ij}^* D_{jqmm}(\mathbf{c}_n) \right. \\
&\quad \left. - \frac{1}{1-\nu} \epsilon_{mj}^* D_{iqmj}(\mathbf{c}_n) \right], \quad (14.4)
\end{aligned}$$

where

$$D_{ijkl}(\mathbf{c}) = (\partial^4 / \partial x_i \partial x_j \partial x_k \partial x_l) \int_{-\infty}^{\infty} \frac{i \exp(i\xi \cdot \mathbf{c})}{\xi_1 \xi_2 \xi_3 (\xi_1^2 + \xi_2^2 + \xi_3^2)^2} d\xi. \quad (14.5)$$

Integrations with respect to ξ lead to

$$\begin{aligned}
D_{1111}(\mathbf{c}) &= 2\pi^2 \left\{ \tan^{-1} \frac{c_2 c_3}{c_1 R} - \frac{c_1 c_2 c_3}{2R} \left(\frac{1}{c_1^2 + c_2^2} + \frac{1}{c_1^2 + c_3^2} \right) \right\}, \\
D_{1112}(\mathbf{c}) &= -\pi^2 \left\{ \operatorname{sgn}(c_3) \log \frac{R + |c_3|}{(c_1^2 + c_2^2)^{1/2}} - \frac{c_1^2 c_3}{(c_1^2 + c_2^2) R} \right\}, \\
D_{1122}(\mathbf{c}) &= \frac{\pi^2 c_1 c_2 c_3}{(c_1^2 + c_2^2) R}, \\
D_{1123}(\mathbf{c}) &= -\frac{\pi^2 c_1}{R},
\end{aligned} \quad (14.6)$$

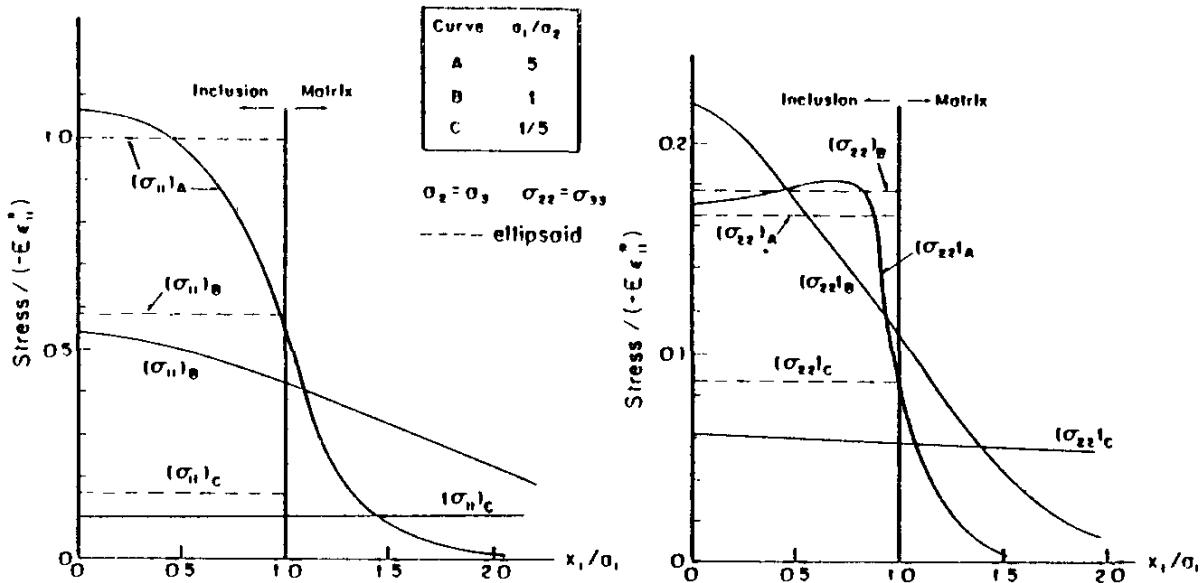


Fig. 14.2 Variations of normal stresses along the x_1 -axis for three different values of a_1/a_2 , where $a_2 = a_3$ (Dotted lines denote stress values for ellipsoidal inclusions)

where $\mathbf{c} = (c_1, c_2, c_3)$ and other components are obtained by the cyclic permutation of (1, 2, 3).

It can be seen that there are logarithmic singularities in certain edges and corners of the cuboid, depending upon the types of eigenstrains in the cuboid; i.e., the singularities only occur in shear stress components when ϵ_{ij}^* has no shear strain components.

The stress components are obtained from $\sigma_{ij}(x) = C_{ijkl}\{u_{k,l}(x) - \epsilon_{kl}^*(x)\}$. Figures 14.2 ~ 14.4 are taken from Chiu (1977). Poisson's ratio $\nu = 0.3$ is used in all calculations. For simplicity it is assumed that all components of the

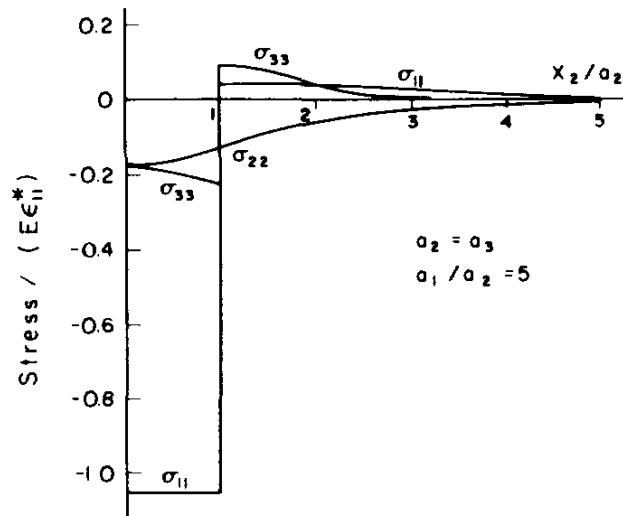


Fig 14.3 Variations of normal stresses along the x_2 -axis for $a_1/a_2 = 5$ and $a_2 = a_3$.

eigenstrain, except ϵ_{11}^* , vanish and that a_2 is equal to a_3 . Figure 14.2 shows variations of the normal stresses along the x_1 -axis for the three types of cuboidal domains. The curves show that the normal stress components are continuous across the boundary of the cuboid at $x_1 = a_1$. The dotted lines are the stress components in an ellipsoidal inclusion with the semi-axes a_1 , a_2 , $a_3 = a_2$. Figures 14.3 and 14.4 show that σ_{22} is continuous across the boundary of the cuboid at $x_2 = a_2$, while the other two normal components of the stress are discontinuous at $x_2 = a_2$. It can be seen from (6.11) that the discontinuities are local quantities which are independent of the overall shape of the inclusion (independent of a_1/a_2 in the present calculation). It is found that $[\sigma_{11}] = 1.099\epsilon_{11}^*E$ and $[\sigma_{33}] = 0.329\epsilon_{11}^*E$, where E is Young's modulus.

Lee and Johnson (1977) have performed an alternative calculation by using (11.30) and (11.31). The $\psi(\mathbf{x})$ and $\phi(\mathbf{x})$ defined by (11.35) have been obtained by MacMillan (1930) when Ω is a cuboidal domain. They are

$$\begin{aligned}\psi(\mathbf{x}) &= \sum_{n=1}^8 (-1)^n D(\mathbf{c}_n), \\ \phi(\mathbf{x}) &= \sum_{n=1}^8 (-1)^n E(\mathbf{c}_n),\end{aligned}\tag{14.7}$$

where

$$\begin{aligned}D(\mathbf{c}) &= \frac{1}{4}c_1c_2c_3R + \frac{1}{6}\left\{\left(R^2 - c_1^2\right)c_2c_3 \log(R + c_1)\right. \\ &\quad \left.+ \left(R^2 - c_2^2\right)c_3c_1 \log(R + c_2) + \left(R^2 - c_3^2\right)c_1c_2 \log(R + c_3)\right\} \\ &\quad - \frac{1}{12}\left\{c_1^4 \tan^{-1}(c_2c_3/c_1R) + c_2^4 \tan^{-1}(c_3c_1/c_2R)\right. \\ &\quad \left.+ c_3^4 \tan^{-1}(c_1c_2/c_3R)\right\},\end{aligned}\tag{14.8}$$

$$\begin{aligned}E(\mathbf{c}) &= c_1c_2 \log(R + c_3) + c_2c_3 \log(R + c_1) + c_3c_1 \log(R + c_2) \\ &\quad - \frac{1}{2}\left\{c_1^2 \tan^{-1}(c_2c_3/c_1R) + c_2^2 \tan^{-1}(c_3c_1/c_2R)\right. \\ &\quad \left.+ c_3^2 \tan^{-1}(c_1c_2/c_3R)\right\},\end{aligned}$$

where $\mathbf{c} = (c_1, c_2, c_3)$, $R = (x_1^2 + x_2^2 + x_3^2)^{1/2}$, and \mathbf{c}_n is defined in (14.2). The strain can be derived from (11.32) where $\Psi_{ij} = \epsilon_{ij}^* \psi$ and $\Phi_{ij} = \epsilon_{ij}^* \phi$.

Lee and Johnson (1977) further have calculated the elastic strain energy W^*

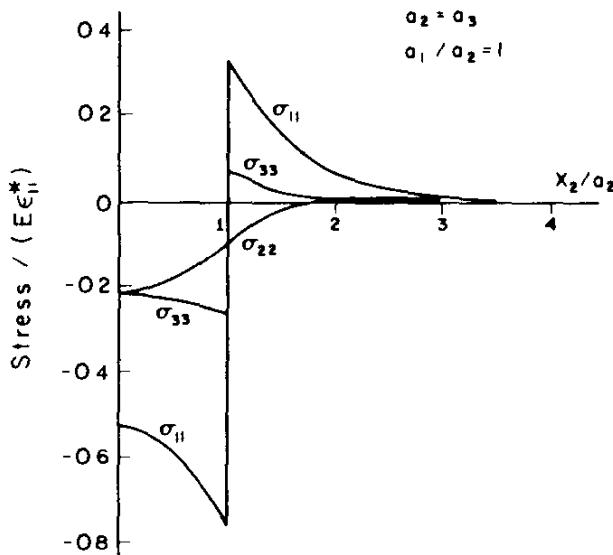


Fig 14.4. Variations of normal stresses along the x_2 -axis for a cuboidal inclusion $a_1 = a_2 = a_3$

of a single cuboidal inclusion and the interaction energy between two cuboidal inclusions, where $\epsilon_{ij}^* = \epsilon_{31}^*$, and other components are zero.

From (13.3) and σ_{ij} calculated from (14.7) or (14.4), Lee and Johnson (1977) obtained the elastic strain energy for $a_1 = a_2 = a_3/\beta$,

$$W^*/8a_1a_2a_3 = 2\mu(\epsilon_{31}^*)^2 \left\{ 1 - f(\beta)/4\pi(1-\nu)\beta \right\}, \quad (14.9)$$

where

$$\begin{aligned} f(\beta) = & 8(1-\nu)\beta \left[\tan^{-1} \left\{ 1/\beta(2+\beta^2)^{1/2} \right\} + \tan^{-1} \left\{ \beta/(2+\beta^2)^{1/2} \right\} \right] \\ & + 4 \left\{ \nu + (2-\nu)\beta^2 \right\} \tanh^{-1} \left\{ 1/(2+\beta^2)^{1/2} \right\} \\ & - 4(2-\nu)\beta^2 \tanh^{-1} \left\{ 1/(1+\beta^2)^{1/2} \right\} \\ & - \frac{4}{3} \left\{ \nu + (3-\nu)\beta^2 \right\} (2+\beta^2)^{1/2} \\ & + \frac{4}{3} \left\{ \nu + 2(3-\nu)\beta^2 \right\} (1+\beta^2)^{1/2} \\ & - \frac{4}{3}(3-\nu)\beta^3 + \nu [2 \log(3-2\sqrt{2}) + \frac{4}{3}(\sqrt{2}-1)]. \end{aligned} \quad (14.10)$$

The elastic strain energy per unit volume of a precipitate is smaller for a cuboid than for an ellipsoid when $\beta > 0.35$. This relation is reversed when $\beta < 0.35$.

Sankaran and Laird (1976) employed another method of calculation. Starting from (11.1) we can write

$$u_i(x) = C_{jkmn} \epsilon_{mn}^* \int_{|\Omega|} G_{ij}(x - x') n_k dS(x') \quad (14.11)$$

where Gauss' theorem is used, and where $|\Omega|$ is the boundary of Ω , n is the outward normal vector on the boundary, and $G_{ij,k} = -(\partial/\partial x'_k)G_{ij}$.

Sankaran and Laird have calculated the stress field around misfitting precipitates in plate morphology, such as θ' in Al-Cu and η in Al-Au (see Aaronson, Laird, and Kinsman 1970, Sankaran and Laird 1974). The stresses are shown to attain very high values near the interfaces at the edges. This can significantly influence the generation of dislocations to accommodate the mismatch.

Argon (1976) has obtained approximate solutions for the stresses around slender finite elastic rods and platelets with moduli different from those of the infinite matrix.

15. Inclusions in a half space

Green's functions

A semi-infinite domain is defined by $x_3 \geq 0$ as shown in Fig. 15.1. The surface $x_3 = 0$ is free from external tractions. The objective is to express the elastic field when the eigenstrain $\epsilon_{ij}^*(x)$ is given in the half space.

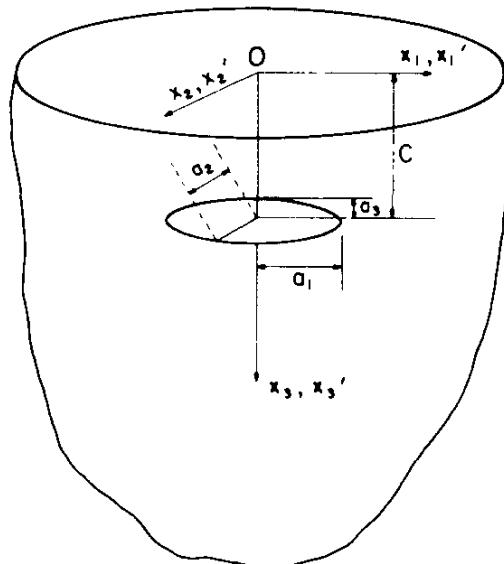


Fig 15.1 Half-space with an ellipsoidal inclusion at c from the surface

Green's functions $G_{ij}(x, x')$ for the half space are defined by the following equations: For $x_3 \geq 0$,

$$C_{ijkl}G_{km,lj}(x, x') + \delta_{im}\delta(x, x') = 0 \quad (15.1)$$

and on $x_3 = 0$,

$$C_{ijkl}G_{km,l}(x, x')n_j = \delta_{im}\delta_S(x, x'), \quad (15.2)$$

where $G_{km,l}(x, x') = (\partial/\partial x_l)G_{km}(x, x')$ and n_j is the unit normal on surface $x_3 = 0$. $\delta(x, x')$ and $\delta_S(x, x')$ are the three- and two-dimensional Dirac delta functions, respectively, having the following properties:

$$\int_0^\infty f(x')\delta(x, x') dx' = f(x) \quad (15.3)$$

and

$$\int_S f(x')\delta_S(x, x') dS(x') = f(x) \quad \text{at } x_3 = 0, \quad (15.4)$$

where S is the plane $x_3 = 0$ and $\delta(x, x') = \delta_S(x, x') = 0$ when $x \neq x'$. $G_{ij}(x, x')$ is the x_i component of displacement at point x when a unit force (body force when x' is an interior point and surface force when x' is on S) in the x_j -direction is applied at point x' .

The displacement $u_i(x)$ caused by $\epsilon_{ij}^*(x)$ is expressed in a form similar to (3.31),

$$u_i(x) = \int_0^\infty C_{jlmn}\epsilon_{mn}^*(x') \frac{\partial}{\partial x'_l} G_{ij}(x, x') dx'. \quad (15.5)$$

It can be shown that (15.5) satisfies the equations of equilibrium (2.12),

$$C_{ijkl}u_{k,lj}(x) = C_{ijkl}\epsilon_{kl,j}^*(x), \quad (15.6)$$

and the boundary condition (2.13),

$$C_{ijkl}u_{k,l}(x)n_j = C_{ijkl}\epsilon_{kl}^*(x)n_j. \quad (15.7)$$

In view of the properties of Green's functions, we have

$$\begin{aligned}
 C_{ijkl}u_{k,l}(x) &= C_{ijkl} \int_0^\infty C_{pqmn} \epsilon_{mn}^*(x') \frac{\partial}{\partial x'_q} G_{kp,l}(x, x') dx' \\
 &= - \int_0^\infty C_{pqmn} \epsilon_{mn}^*(x') \frac{\partial}{\partial x'_q} \delta_{ip} \delta(x, x') dx' \\
 &= - \int_0^\infty C_{ijkl} \epsilon_{kl}^*(x') \frac{\partial}{\partial x'_j} \delta(x, x') dx' \\
 &= \int_0^\infty C_{ijkl} \epsilon_{kl,j}^*(x') \delta(x, x') dx' = C_{ijkl} \epsilon_{kl,j}^*(x)
 \end{aligned} \tag{15.8}$$

and on the plane $x_3 = 0$

$$\begin{aligned}
 C_{ijkl}u_{k,l}(x)n_j &= C_{ijkl} \int_0^\infty C_{pqmn} \epsilon_{mn}^*(x') \frac{\partial}{\partial x'_q} G_{kp,l}(x, x') n_j dx' \\
 &= \int_0^\infty C_{pqmn} \epsilon_{mn}^*(x') \frac{\partial}{\partial x'_q} C_{ijkl} G_{kp,l}(x, x') n_j dx' \\
 &= \int_S C_{pqmn} \epsilon_{mn}^*(x') \delta_{ip} \delta_S(x, x') n_q dS(x') \\
 &= C_{ijkl} \epsilon_{kl}^*(x) n_j,
 \end{aligned} \tag{15.9}$$

where S is the surface $x_3 = 0$.

Green's functions $G_{ij}(x, x')$ for the semi-infinite isotropic medium have been found by Mindlin (1953); they are

$$\begin{aligned}
 G_{ij}(x, x') &= \frac{1}{16\pi\mu(1-\nu)} \left[\frac{3-4\nu}{R_1} \delta_{ij} + \frac{1}{R_2} \delta_{ij} + \frac{(x_i - x'_i)(x_j - x'_j)}{R_1^3} \right. \\
 &\quad + \frac{(3-4\nu)(x_i - x'_i)(x_j - x'_j)}{R_2^3} + \frac{2x_3x'_3}{R_2^3} \left\{ \delta_{ij} - \frac{3(x_i - x'_i)(x_j - x'_j)}{R_2^2} \right\} \\
 &\quad \left. + \frac{4(1-\nu)(1-2\nu)}{R_2 + x_3 + x'_3} \left\{ \delta_{ij} - \frac{(x_i - x'_i)(x_j - x'_j)}{R_2(R_2 + x_3 + x'_3)} \right\} \right] = G_{ji}(x, x'),
 \end{aligned}$$

$$\begin{aligned}
G_{3j}(x, x') &= \frac{(x_j - x'_j)}{16\pi\mu(1-\nu)} \left[\frac{(x_3 - x'_3)}{R_1^3} + \frac{(3-4\nu)(x_3 - x'_3)}{R_2^3} \right. \\
&\quad \left. - \frac{6x_3x'_3(x_3 + x'_3)}{R_2^5} + \frac{4(1-\nu)(1-2\nu)}{R_2(R_2 + x_3 + x'_3)} \right], \\
G_{i3}(x, x') &= \frac{(x_i - x'_i)}{16\pi\mu(1-\nu)} \left[\frac{(x_3 - x'_3)}{R_1^3} + \frac{(3-4\nu)(x_3 - x'_3)}{R_2^3} \right. \\
&\quad \left. + \frac{6x_3x'_3(x_3 + x'_3)}{R_2^5} - \frac{4(1-\nu)(1-2\nu)}{R_2(R_2 + x_3 + x'_3)} \right], \\
G_{33}(x, x') &= \frac{1}{16\pi\mu(1-\nu)} \left[\frac{3-4\nu}{R_1} + \frac{8(1-\nu)^2 - (3-4\nu)}{R_2} + \frac{(x_3 - x'_3)^2}{R_1^3} \right. \\
&\quad \left. + \frac{(3-4\nu)(x_3 + x'_3)^2 - 2x_3x'_3}{R_2^3} + \frac{6x_3x'_3(x_3 + x'_3)^2}{R_2^5} \right], \quad i, j = 1, 2,
\end{aligned} \tag{15.10}$$

where

$$\begin{aligned}
R_1^2 &= (x_1 - x'_1)^2 + (x_2 - x'_2)^2 + (x_3 - x'_3)^2, \\
R_2^2 &= (x_1 - x'_1)^2 + (x_2 - x'_2)^2 + (x_3 + x'_3)^2,
\end{aligned} \tag{15.11}$$

μ is the shear modulus, and ν is Poisson's ratio.

In these equations, the effect of the free surface appears in terms which contain R_2 . (A minor sign error in the original expressions has been corrected in a private communication; Mindlin 1975.)

The inclusion problem here occurs when ϵ_{ij}^* is given in domain Ω . If ϵ_{ij}^* is a constant, (15.5) becomes

$$u_i(x) = C_{lmn}\epsilon_{mn}^* \int_{\Omega} \frac{\partial}{\partial x'_l} G_{ij}(x, x') dx' \tag{15.12}$$

or

$$u_i(x) = C_{lmn}\epsilon_{mn}^* \int_{|\Omega|} G_{ij}(x, x') n_l(x') dS(x'), \tag{15.13}$$

where Gauss' theorem is used, and $|\Omega|$ is the surface of Ω .

Recently, N.G. Lee (1979) and Pan and Chou (1979) have obtained explicit expressions for Green's functions in a half space of a hexagonal continuum with its basal plane as the free surface. Pan and Chou (1979) further extended their solutions to the two-phase transversely isotropic materials.

Ellipsoidal inclusion with a uniform dilatational eigenstrain

The elastic field is calculated when an ellipsoidal inclusion with a uniform dilatational eigenstrain

$$\epsilon_{ij}^* = \delta_{ij} \epsilon^* \quad (15.14)$$

is located near the surface of a half space. When (15.10) and (15.14) are substituted into (15.12) we obtain

$$u_i(x) = \frac{\epsilon^*(1+\nu)}{4\pi(1-\nu)} \int_{\Omega} \left[-\left(\frac{1}{R_1} \right)_{,i} + (3-4\nu)(2\delta_{3i}-1)\left(\frac{1}{R_2} \right)_{,i} - 2x_3 \left(\frac{1}{R_2} \right)_{,3i} \right] dx', \quad (15.15)$$

where

$$\frac{\partial}{\partial x_i} \left(\frac{1}{R_1} \right) = \left(\frac{1}{R_1} \right)_{,i}$$

and the domain of integration is

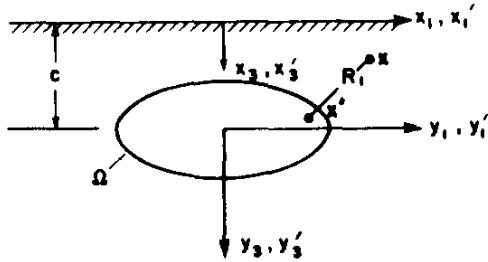
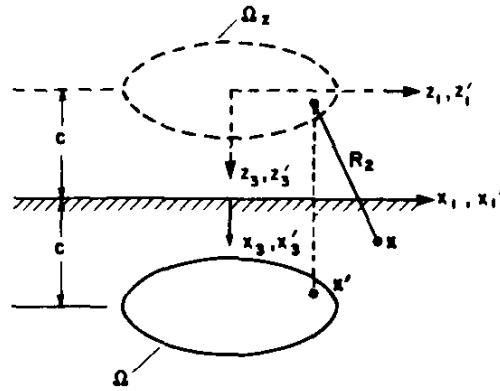
$$\frac{x_1'^2}{a_1^2} + \frac{x_2'^2}{a_2^2} + \frac{(x_3' - c)^2}{a_3^2} \leq 1.$$

If we set

$$x_1 = y_1, x_2 = y_2, x_3 = y_3 + c, x'_1 = y'_1, x'_2 = y'_2, x'_3 = y'_3 + c \quad (15.16)$$

and

$$x_1 = z_1, x_2 = z_2, x_3 = z_3 - c, x'_1 = z'_1, x'_2 = z'_2, x'_3 = -z'_3 + c, \quad (15.17)$$

Fig. 15.2a. Coordinate systems for R_1 integral.Fig. 15.2b. Coordinate systems for R_2 integral.

then R_1 and R_2 (see Fig. 15.2) become

$$\begin{aligned} R_1^2 &= (y_1 - y'_1)^2 + (y_2 - y'_2)^2 + (y_3 - y'_3)^2, \\ R_2^2 &= (z_1 - z'_1)^2 + (z_2 - z'_2)^2 + (z_3 - z'_3)^2. \end{aligned} \quad (15.18)$$

R_1 is the distance between points x and x' . R_2 is the distance between point x and the mirror image of x' . In order to use formulae (12.9) ~ (12.13) in (15.15), domain Ω is transformed into its mirror image Ω_z when the integral $\int_{\Omega} dx'/R_2$ is considered (this domain transformation is not necessary for $\int_{\Omega} dx'/R_1$). Referring to (12.9) and (12.13), we write

$$\phi(y) = \int_{\Omega} \frac{1}{R_1} dx' = \int_{\Omega} \frac{1}{R_1} dy' = \pi a_1 a_2 a_3 \int_{\lambda}^{\infty} \frac{U}{\Delta} ds = V(y), \quad (15.19)$$

where

$$\begin{aligned} U &= 1 - \left(\frac{y_1^2}{a_1^2 + s} + \frac{y_2^2}{a_2^2 + s} + \frac{y_3^2}{a_3^2 + s} \right), \\ \Delta &= \{ (a_1^2 + s) + (a_2^2 + s) + (a_3^2 + s) \}^{1/2}, \\ \frac{y_1^2}{a_1^2 + \lambda} + \frac{y_2^2}{a_2^2 + \lambda} + \frac{y_3^2}{a_3^2 + \lambda} &= 1 \text{ for exterior points } x \text{ of } \Omega, \\ \lambda &= 0 \text{ for interior points } x \text{ of } \Omega, \end{aligned} \quad (15.20)$$

and we write

$$\phi(z) = \int_{\Omega} \frac{1}{R_2} dx' = \int_{\Omega_z} \frac{1}{R_2} dz' = \pi a_1 a_2 a_3 \int_{\lambda}^{\infty} \frac{U}{\Delta} ds = V(z), \quad (15.20.1)$$

where

$$U = 1 - \left(\frac{z_1^2}{a_1^2 + s} + \frac{z_2^2}{a_2^2 + s} + \frac{z_3^2}{a_3^2 + s} \right),$$

$$\Delta = \left\{ (a_1^2 + s) + (a_2^2 + s) + (a_3^2 + s) \right\}^{1/2}, \quad (15.20.2)$$

$$\frac{z_1^2}{a_1^2 + \lambda} + \frac{z_2^2}{a_2^2 + \lambda} + \frac{z_3^2}{a_3^2 + \lambda} = 1 \text{ for any point } x.$$

Then (15.15) reduces to

$$u_i(x) = \frac{\epsilon^*(1+\nu)}{4\pi(1-\nu)} \left[-\phi_{,i}(y) + (3-4\nu)(2\delta_{3i}-1)\phi_{,i}(z) - 2x_3\phi_{,3i}(z) \right], \quad (15.21)$$

and, using (11.39), we obtain

$$\begin{aligned} \epsilon_{ij}(x) &= \frac{1}{2}(u_{i,j} + u_{j,i}) \\ &= \frac{\epsilon^*(1+\nu)}{4\pi(1-\nu)} \left[-\phi_{,ij}(y) + (3-4\nu)(\delta_{3I} + \delta_{3J} - 1)\phi_{,ij}(z) \right. \\ &\quad \left. - (\delta_{3I} + \delta_{3J})\phi_{,ij}(z) - 2x_3\phi_{,3ij}(z) \right]. \end{aligned} \quad (15.22)$$

The stress components are

$$\sigma_{ij}(x) = C_{ijkl} [\epsilon_{kl}(x) - \epsilon_{kl}^*(x)], \quad (15.23)$$

where $\epsilon_{ij}^* = 0$ for exterior points of Ω . For interior points, the stress components become

$$\begin{aligned} \sigma_{ij}(x) &= \frac{\epsilon^*(1+\nu)2\mu}{4\pi(1-\nu)} \left[-4\pi\delta_{ij} - \phi_{,ij}(y) + 4\nu\delta_{ij}\phi_{,33}(z) \right. \\ &\quad + (3-4\nu)(\delta_{3I} + \delta_{3J} - 1)\phi_{,ij}(z) \\ &\quad \left. - (\delta_{3I} + \delta_{3J})\phi_{,ij}(z) - 2x_3\phi_{,3ij}(z) \right], \end{aligned} \quad (15.24)$$

and for exterior points, they became

$$\begin{aligned}\sigma_{ij}(x) = & \frac{\epsilon^*(1+\nu)2\mu}{4\pi(1-\nu)} \left[-\phi_{,ij}(y) + 4\nu\delta_{ij}\phi_{,33}(z) \right. \\ & + (3-4\nu)(\delta_{3I} + \delta_{3J} - 1)\phi_{,ij}(z) \\ & \left. - (\delta_{3I} + \delta_{3J})\phi_{,ij}(z) - 2x_3\phi_{,3ij}(z) \right].\end{aligned}\quad (15.25)$$

Mindlin and Cheng (1950) have treated the same problem, except that their inclusion is a sphere ($a_1 = a_2 = a_3$).

The following numerical calculations are taken from the paper by Seo and Mura (1979), with $\nu = 0.3$ for all cases.

First consider the case when the inclusion with $a_1 = a_2 = 3a_3$ is located at depth $c = a_3$. The dimensionless stress distributions are shown in Figs. 15.3 ~ 15.4 by solid curves. For comparison, the results for a spherical inclusion ($a_1 = a_2 = a_3$) are shown by dotted curves. Figure 15.3 shows σ_{11} , σ_{33} components along the x_3 -axis. For the ellipsoidal inclusion, σ_{11} is compressive at interior points and increases with x_3 . It changes discontinuously to tension for

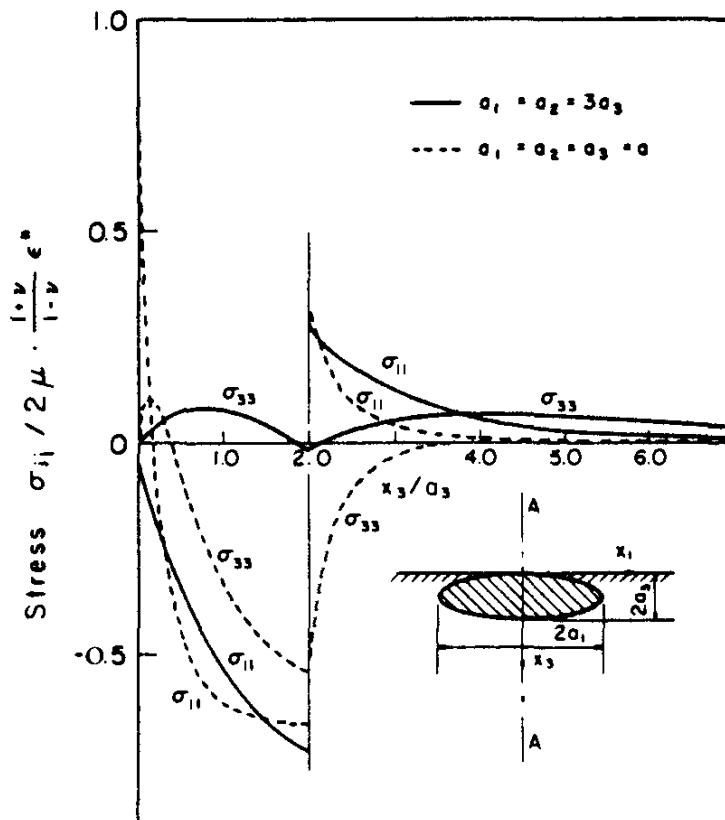


Fig. 15.3. Stress distributions on A-A section.

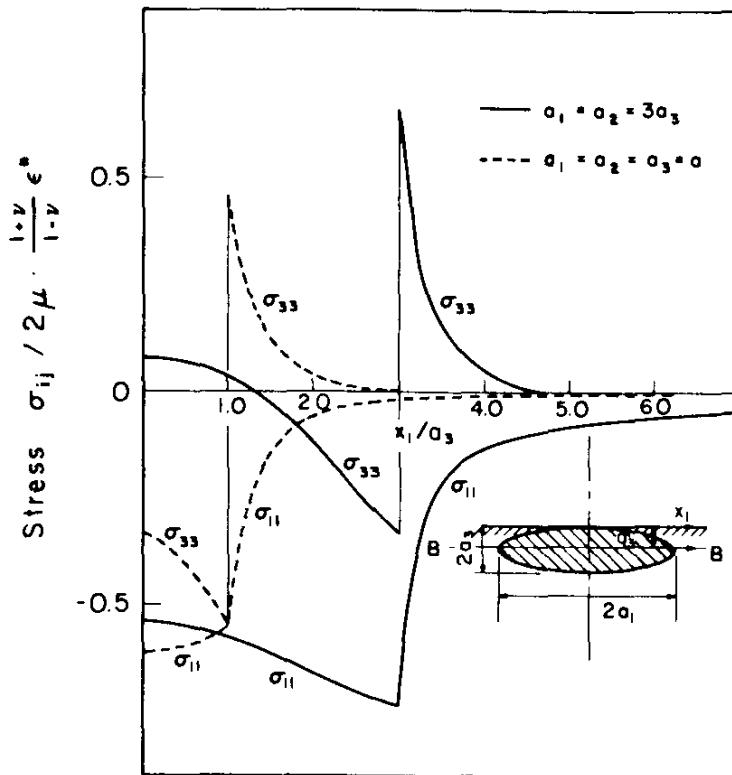


Fig. 15.4. Stress distributions on B-B section.

exterior points and decays to zero at infinity. The spherical inclusion, however, causes tension for the interior points near the free surface. The stress component σ_{33} is continuous and takes a maximum tension at an interior point. The maximum compression appears at the interface point. Its magnitude is quite different for the two cases, and the compression region is broader in the ellipsoidal case than in the spherical. To see the effect of the free boundary surface, the stress distributions for an infinite medium are shown in Fig. 15.5. It can be seen that the stress uniformity inside the inclusion for the infinite medium is diminished by the existence of the free surface and some region near the surface undergoes tension. Note also that component σ_{33} in the matrix may be positive (tension in regions) far away from the inclusion, while σ_{33} for the infinite medium is compressive everywhere.

In Fig. 15.4 the stress distributions on the B-B section are shown. The stress jump at the interface is observed as expected for each component of the stress. The effect of the free boundary surface appears in the σ_{33} component showing a tension region around the center of the ellipsoid. The effect of the inclusion shape on the stress distribution can be seen by comparing the interface point $x_1/a_3 = 1$ for the sphere with $x_1/a_3 = 3$ for the ellipsoid: $\sigma_{11} = \sigma_{22} = \sigma_{33}$ for the sphere, but $\sigma_{11} = \sigma_{22} \neq \sigma_{33}$ for the ellipsoid.

The effect of depth c is shown by curves in Figs. 15.6 ~ 15.7. σ_{11} is tensile at the boundary surface and decreases with x_3 . It jumps to compression for

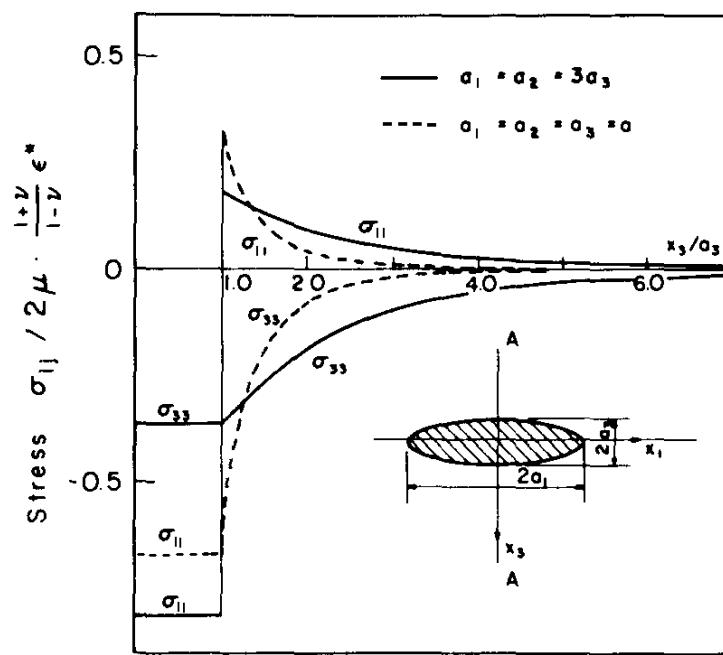


Fig. 15.5. Stress distributions on A-A section in case of infinite solid.

interior points of the inclusion and changes to tension again in the matrix. Due to the existence of the free surface, the compressive stress components for interior points of the inclusion increase with depth from the surface. This effect is less prevalent for the spherical inclusion. It may be said that the effect of the free surface disappears for the spherical inclusion, when $c > 2a$.

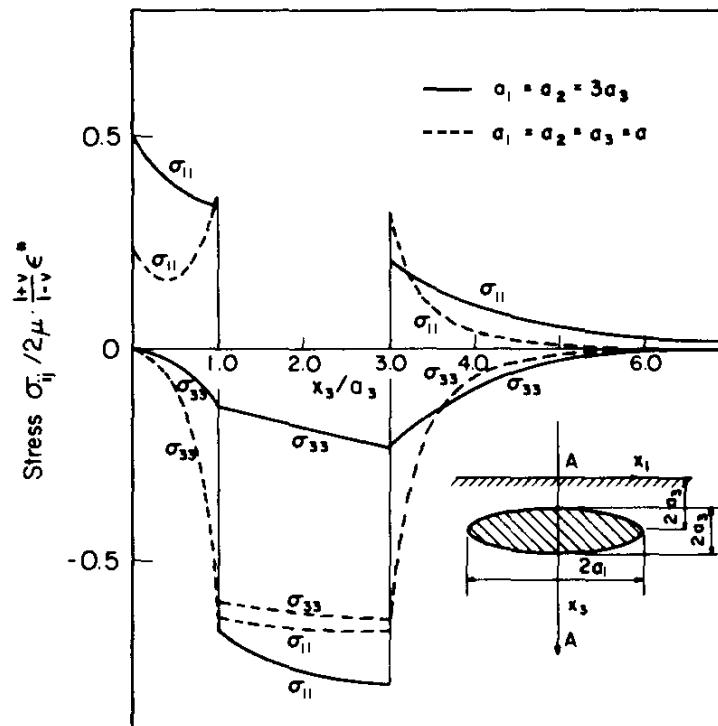


Fig. 15.6. Stress distributions on A-A section.

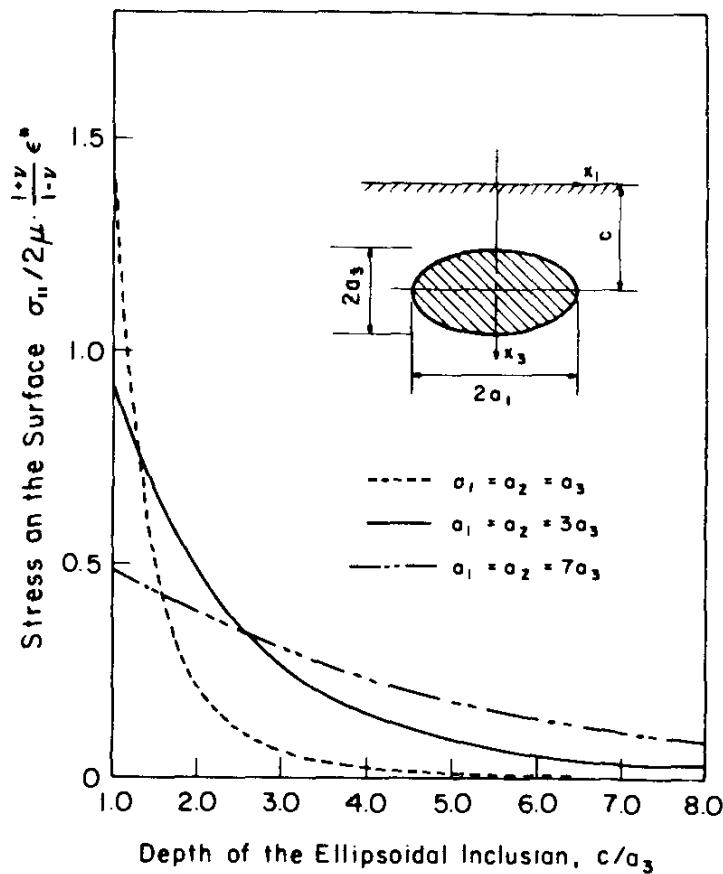


Fig. 15.7 Effect of the inclusion depth on the surface stress

In Fig. 15.7 the effect of c on the stress component σ_{11} at $x_1 = x_2 = x_3 = 0$ is shown for various aspect ratios of the ellipsoid. For the spherical inclusion, σ_{11} shows large tension at the surface that decreases with c . It becomes almost zero when $c > 3a$. σ_{11} at the surface decreases when the ellipsoid becomes flatter. However, the effect of c is less prevalent in the flatter inclusion. It is seen that the eigenstress at the surface depends not only on the depth of the inclusion from the surface but also on the shape of the inclusion.

The elastic strain energy is, from (13.3),

$$W^* = -\frac{1}{2} \int_{\Omega} \sigma_{ii} \epsilon^* dx. \quad (15.25.1)$$

Substituting from (15.24), we have

$$W^* = 2\mu \frac{1+\nu}{1-\nu} (\epsilon^*)^2 V \left(1 - \frac{1+\nu}{2\pi V} \int_{\Omega} \phi_{,33}(z) dx \right), \quad (15.25.2)$$

where V is the volume of the inclusion. The factor $2\mu(1+\nu)(\epsilon^*)^2V/(1-\nu)$ is

the elastic strain energy (13.21) for the case of an infinite space. The second term in (15.25.2) is the correction factor due to the free surface.

If the inclusion is a sphere, we have

$$\phi(z) = 4\pi a^3/3r = V/r, \quad (15.25.3)$$

where $r = (z_1^2 + z_2^2 + z_3^2)^{1/2}$. Since

$$\int_{\Omega} \left(\frac{1}{r} \right)_{,33} dx = 2\pi a^3/6c^3, \quad (15.25.4)$$

the correction term becomes $-(1+\nu)a^3/6c^3$. Eshelby (1961) has given this correction term as $-(1+\nu)a^3/4c^3$ which involves a trivial numerical error.

The force acting on the inclusion is defined as

$$F = -\frac{\delta W^*}{\delta c} = -\mu \frac{(1+\nu)^2}{1-\nu} (\epsilon^*)^2 V a^3 / c^2. \quad (15.25.5)$$

Thus, the free surface attracts the inclusion.

Tsuchida and Mura (1983) solved the case when the inclusion is spheroidal and the eigenstrain is symmetric about the axis of the spheroid.

* Cuboidal inclusion with uniform eigenstrains

When a cuboidal inclusion with a uniform eigenstrain is located in a half-space, the elastic field can be obtained from (15.12) ~ (15.13) by taking Ω as the cuboidal domain. The surface integrals (on $|\Omega|$) in (15.13) have been explicitly performed by Lin and Tung (1962) in their calculation of the stress field produced by a localized plastic slip at a free surface. Chiu (1978, 1980) has extended his solution for an infinite body to the case of a half-space by applying on the boundary plane equal and opposite normal and shear stresses in order to obtain the free boundary conditions. Lee and Hsu (1985) have calculated the thermal stress when the cuboidal domain peeps out at the free surface.

* Periodic distribution of eigenstrains

When eigenstrain $\epsilon_{ij}^*(x)$ is given by

$$\epsilon_{ij}^*(x) = \bar{\epsilon}_{ij}^*(\xi) \exp(i\xi \cdot x) \quad (15.26)$$

in a half-space, the displacement field is the sum of (3.10) and $u_i^s(x)$, where

$u_i^s(\mathbf{x})$ is the displacement caused by applying on the boundary plane $x_3 = 0$ equal and opposite normal and shear stresses which would render this boundary stress-free. Since the stress field for the full space problem is (3.12), the x_i component of the force to be applied on the boundary $x_3 = 0$ is

$$F_i = -C_{ijkl} \left\{ C_{pqmn} \bar{\epsilon}_{mn}^*(\xi) \xi_q \xi_l N_{kp}(\xi) D^{-1}(\xi) - \bar{\epsilon}_{kl}^* \right\} n_j \exp(i\xi \cdot \mathbf{x}^0) \\ \equiv \bar{F}_i \exp(i\xi \cdot \mathbf{x}^0), \quad (15.27)$$

where \mathbf{x}^0 is a point on the boundary, $\xi \cdot \mathbf{x}^0 = \xi_1 x_1^0 + \xi_2 x_2^0$, and \mathbf{n} is the outward unit normal on the boundary. If F_i is a unit force, the induced displacement field is given by (15.10) with $x'_3 = 0$. Thus

$$u_i^s(\mathbf{x}) = \iint_{-\infty}^{\infty} \bar{F}_j G_{ij}(\mathbf{x}, \mathbf{x}^0) \exp(i\xi \cdot \mathbf{x}^0) dx_1^0 dx_2^0. \quad (15.28)$$

The stress field of the problem is the sum of (3.12) and σ_{ij}^s , where

$$\sigma_{ij}^s(\mathbf{x}) = C_{ijkl} \frac{\partial}{\partial x_l} \iint_{-\infty}^{\infty} \bar{F}_m G_{km}(\mathbf{x}, \mathbf{x}^0) \exp(i\xi \cdot \mathbf{x}^0) dx_1^0 dx_2^0. \quad (15.29)$$

Owen and Mura (1967) have performed the above integrations and obtained

$$\sigma_{ij}^s(\mathbf{x}) = \frac{\bar{F}_m C_{ijkl}}{2\xi} \frac{\partial}{\partial x_l} [H_{km} \exp\{i(\xi_1 x_1 + \xi_2 x_2) - \xi x_3\}], \quad (15.30)$$

where

$$\xi = (\xi_1^2 + \xi_2^2)^{1/2},$$

$$H_{km} = \mu^{-1} \left[\delta_{km} - ix_3 \{ \delta_{3k} \xi_m + \delta_{3m} \xi_k - \delta_{3k} \delta_{3m} (\xi_K + \xi_M + i\xi) \} \right. \\ - i(\delta_{K1} + \delta_{K2})(\delta_{M1} + \delta_{M2}) \xi_k \xi_m / \xi \\ \left. + i(\xi_2 \delta_{k1} - \xi_1 \delta_{k2})(\xi_2 \delta_{m1} - \xi_1 \delta_{m2}) / \xi \right] \\ + (\lambda + \mu)^{-1} \left\{ \delta_{k3} \delta_{m3} + (1 - \delta_{K3} \delta_{M3}) \xi_k \xi_m / \xi^2 \right. \\ \left. + i(\delta_{m3} \xi_k - \delta_{k3} \xi_m) / \xi \right\}. \quad (15.31)$$

The solution in this subsection has many applications when the Fourier series and integral methods are employed. When eigenstrains are given by

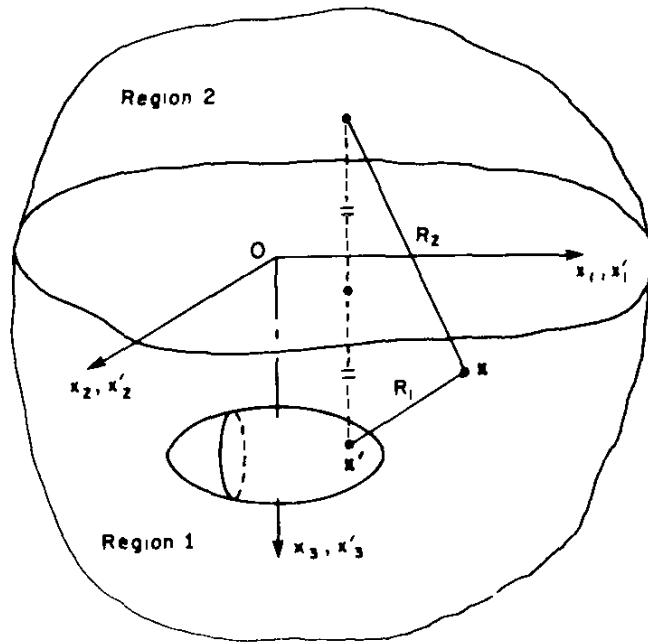


Fig. 15.8. Joined half spaces.

summations or integrations of (15.26), the corresponding solutions can be constructed from summations or integrations (with respect to ξ) of the solution given here. Owen and Mura have calculated the stress field of a Frank dislocation network located near a free surface. Owen (1971) has further calculated by the present method the stress disturbance near the free surface of a fiber-reinforced material subjected to tensile traction. Saito, Bozkurt, and Mura (1972) have considered the case of a periodic distribution of dislocations in a thin film.

Joined half spaces

Let us consider the case when an infinite body consists of two half-spaces with different shear moduli and Poisson's ratios (see Fig. 15.8). When one of the half-spaces, e.g. region 1, contains an inclusion Ω with eigenstrain ϵ_{ij}^* , the displacement field can be expressed in a form similar to (15.5),

$$u_i(x) = \int_{-\infty}^{\infty} C_{jlmn} \epsilon_{mn}^*(x') \frac{\partial}{\partial x'_l} G_{ij}(x, x') dx'. \quad (15.32)$$

It is easily proved that (15.32) satisfies (15.6); this is done in the manner outlined in (15.8). Since ϵ_{mn}^* is zero outside of Ω ,

$$u_i(x) = \int_{\Omega} C_{jlmn} \epsilon_{mn}^*(x') \frac{\partial}{\partial x'_l} G_{ij}(x, x') dx'. \quad (15.33)$$

$G_{ij}(x, x')$ in the above expressions is Green's function in the two semi-infinite solids; it is the x_i -direction displacement at point x caused by the x_j -direction unit body force applied at point x' .

The elastic moduli in regions 1 and 2 are denoted by subscripts 1 and 2, respectively.

When the two half-spaces are perfectly bonded, continuity of all components of displacements and tractions at the interface is required. The Green's function for this case has been found by Rongved (1955). The solution is given in terms of the Papkovich-Neuber displacement functions (see Sokolnikoff 1956),

$$G_{ij}(x, x') = \left(\frac{1}{2}G\right) \left\{ (\kappa + 1)\Phi_{ij} - (x_k \Phi_{kj} + \psi_j)_{,i} \right\}, \quad (15.34)$$

where $\kappa = 3 - 4\nu$ and point x' is in region 1. For x in region 1,

$$\Phi_{11} = \Phi_{22} = \frac{1}{2\pi(\kappa_1 + 1)} (1/R_1 + S/R_2),$$

$$\Phi_{21} = \Phi_{12} = 0,$$

$$\begin{aligned} \Phi_{3j} &= \frac{1}{2\pi(\kappa_1 + 1)} \left\{ (A\kappa_1 - S)(x_j - x'_j)/R_2(R_2 + x_3 + x'_3) \right. \\ &\quad \left. - 2Ax'_3(x_j - x'_j)/R_2^3 \right\}, \\ \psi_j &= \frac{1}{2\pi(\kappa_1 + 1)} \left\{ -(A\kappa_1^2 + B - 2S\kappa_1)(x_j - x'_j)/2(R_2 + x_3 + x'_3) \right. \\ &\quad \left. + (A\kappa_1 - S)x'_3(x_j - x'_j)/R_2(R_2 + x_3 + x'_3) \right\}, \end{aligned} \quad (15.35)$$

$$\Phi_{13} = \Phi_{23} = 0,$$

$$\Phi_{33} = \frac{1}{2\pi(\kappa_1 + 1)} \left\{ 1/R_1 + A\kappa_1/R_2 + 2Ax'_3(x_3 + x'_3)/R_2^3 \right\},$$

$$\begin{aligned} \psi_3 &= \frac{1}{2\pi(\kappa_1 + 1)} \left\{ -x'_3/R_1 - A\kappa_1 x'_3/R_2 \right. \\ &\quad \left. + (A\kappa_1^2 - B) \log(R_2 + x'_3 + x_3)^{1/2} \right\}. \end{aligned}$$

For x in region 2,

$$\begin{aligned}\Phi_{11} &= \Phi_{22} = \frac{T}{2\pi(\kappa_1 + 1)R_1}, \\ \Phi_{21} &= \Phi_{12} = 0, \\ \Phi_{3j} &= -\frac{(1-B-T)}{2\pi(\kappa_1 + 1)} \frac{x_j - x'_j}{R_1(R_1 - x_3 + x'_3)}, \\ \psi_j &= \frac{1}{2\pi(\kappa_1 + 1)} \left[-\{(1-A)\kappa_1 + (1-B-2T)\kappa_2\} \right. \\ &\quad \times (x_j - x'_j)/2(R_1 - x_3 + x'_3) \\ &\quad \left. + (1-A-T)x'_3(x_j - x'_j)/R_1(R_1 - x_3 + x'_3) \right], \\ \Phi_{13} &= \Phi_{23} = 0, \\ \Phi_{33} &= \frac{(1-B)}{2\pi(\kappa_1 + 1)R_1}, \\ \psi_3 &= \frac{1}{2\pi(\kappa_1 + 1)} \left[-(1-A)x'_3/R_1 \right. \\ &\quad \left. + \{(1-A)\kappa_1 - (1-B)\kappa_2\} \log(R_1 - x_3 + x'_3)^{1/2} \right],\end{aligned}\tag{15.36}$$

where

$$i, j = 1, 2,$$

$$R_1^2 = (x_1 - x'_1)^2 + (x_2 - x'_2)^2 + (x_3 - x'_3)^2,$$

$$R_2^2 = (x_1 - x'_1)^2 + (x_2 - x'_2)^2 + (x_3 + x'_3)^2,\tag{15.37}$$

$$\Gamma = G_2/G_1,$$

$$A = (1 - \Gamma)/(1 + \kappa_1 \Gamma), \quad B = (\kappa_2 - \kappa_1 \Gamma)/(\kappa_2 + \Gamma),$$

$$S = (1 - \Gamma)/(1 + \Gamma), \quad T = 2(\kappa_1 + 1)\Gamma/(\kappa_2 + 1)(1 + \Gamma).$$

When the two half-spaces have smooth contact, the boundary conditions for Green's function are continuity of the normal components of displacement and stress and a condition for vanishing shear components of stress at the interface. The Green function for this case has been found by Dundurs and Hetényi (1965). For x in region 1, (x' is in region 1),

$$\Phi_{11} = \Phi_{22} = \frac{1}{2\pi(\kappa_1 + 1)} (1/R_1 + 1/R_2),$$

$$\Phi_{21} = \Phi_{12} = 0,$$

$$\Phi_{3j} = \frac{1 - D}{2\pi(\kappa_1 + 1)} \left\{ (\kappa_1 - 1)(x_j - x'_j)/R_2(R_2 + x_3 + x'_3) - 2x'_3 x_3/R_2^3 \right\},$$

$$\begin{aligned} \psi_j = \frac{(1 - D)(\kappa_1 - 1)}{2\pi(\kappa_1 + 1)} & \left\{ -(\kappa_1 - 1)(x_j - x'_j)/2(R_2 + x_3 + x'_3) \right. \\ & \left. + x'_3(x_j - x'_j)/R_2(R_2 + x_3 + x'_3) \right\}, \end{aligned} \quad (15.38)$$

$$\Phi_{13} = \Phi_{23} = 0,$$

$$\begin{aligned} \Phi_{33} = \frac{1}{2\pi(\kappa_1 + 1)} & \left\{ 1/R_1 + [\kappa_1 - D(\kappa_1 + 1)]/R_2 \right. \\ & \left. + 2(1 - D)x'_3(x_3 + x'_3)/R_2^3 \right\}, \end{aligned}$$

$$\begin{aligned} \psi_3 = \frac{1}{2\pi(\kappa_1 + 1)} & \left\{ -x'_3/R_1 - [\kappa_1 - D(\kappa_1 - 1)]x'_3/R_2 \right. \\ & \left. + (1 - D)(\kappa_1^2 - 1) \log(R_2 + x_3 + x'_3)^{1/2} \right\}. \end{aligned}$$

For x in region 2,

$$\Phi_{11} = \Phi_{22} = \Phi_{12} = \Phi_{21} = 0,$$

$$\begin{aligned} \Phi_{3j} = \frac{D}{2\pi(\kappa_1 + 1)} & \left\{ (\kappa_1 - 1)(x_j - x'_j)/R_1(R_1 - x_3 + x'_3) \right. \\ & \left. - 2x'_3(x_j - x'_j)/R_1^3 \right\}, \end{aligned}$$

$$\begin{aligned}\psi_j &= \frac{D(\kappa_2 - 1)}{2\pi(\kappa_1 + 1)} \left\{ (\kappa_1 - 1)(x_j - x'_j)/2(R_1 - x_3 + x'_3) \right. \\ &\quad \left. - x'_3(x_j - x'_j)/R_1(R_1 - x_3 + x'_3) \right\}, \\ \Phi_{13} &= \Phi_{23} = 0, \\ \Phi_{33} &= \frac{D}{2\pi(\kappa_1 + 1)} \left\{ (\kappa_1 + 1)/R_1 - 2x'_3(x_3 - x'_3)/R_1^3 \right\}, \\ \psi_3 &= \frac{D(\kappa_2 - 1)}{2\pi(\kappa_1 + 1)} \left\{ x'_3/R_1 - (\kappa_1 + 1) \log(R_1 - x_3 + x'_3)^{1/2} \right\},\end{aligned}\tag{15.39}$$

where

$$\begin{aligned}i, j &= 1, 2, \\ D &= (\kappa_1 + 1)\Gamma / \{(\kappa_1 + 1)\Gamma + \kappa_2 + 1\}.\end{aligned}\tag{15.40}$$

Special cases $\Gamma = 0, 1$ and ∞ correspond to a single half-space, a homogeneous whole space, and the smooth rigid base contact, respectively.

It should be noted from (15.35) ~ (15.40) that (15.34) can be derived from Green's function for a homogeneous infinite medium. The terms containing R_2 and $R_1 - x_3 + x'_3$ in (15.35) ~ (15.39) can be constructed from R_1 by proper elementary operations (changing arguments, multiplying by $(x_j - x'_j)$, differentiating, and integrating). Aderogba (1977) has pointed out that an elastic solution for dissimilar joined half-spaces is constructed from the solution for a homogeneous whole space.

Dundurs and Guell (1965) and Guell and Dundurs (1967) (see also Aderogba 1972) have calculated the stress field for a center of dilatation where

$$\epsilon_{ij}^*(x) = \delta_{ij}\epsilon^*\delta(x_1, x_2, x_3 - c).\tag{15.41}$$

From (15.33) this case gives

$$u_i(x) = C_{Jlmn}\epsilon^* \left[\frac{\partial}{\partial x'_l} G_{ij}(x, x') \right]_{x'_1=0, x'_2=0, x'_3=c}\tag{15.42}$$

According to Dundurs and Guell, the normal traction transmitted by the interface is

$$(\sigma_{33})_{x_3=0} = \frac{4DG_1}{c^3} \left(\frac{c}{R_1} \right)^3 \left\{ 1 - 3 \left(\frac{c}{R_1} \right)^2 \right\}\tag{15.43}$$

for the smooth interface, and it is

$$(\sigma_{33})_{x_3=0} = \frac{2(1-A)G_1}{c^3} \left(\frac{c}{R_1} \right)^3 \left\{ 1 - 3 \left(\frac{c}{R_1} \right)^2 \right\} \quad (15.44)$$

for the perfectly bonded interface. It is interesting to compare the above results with the solution for the whole space (a center of dilatation is placed at point $(0, 0, c)$),

$$(\sigma_{33})_{x_3=0} = \frac{2G_1}{c^3} \left(\frac{c}{R_1} \right)^3 \left\{ 1 - 3 \left(\frac{c}{R_1} \right)^2 \right\} \equiv \sigma_{33}^*. \quad (15.45)$$

Then we have

$$\begin{aligned} (\sigma_{33})_{x_3=0} &= 2D\sigma_{33}^* \quad \text{for smooth surface,} \\ &= (1-A)\sigma_{33}^* \quad \text{for perfect bond.} \end{aligned} \quad (15.46)$$

Moon and Pao (1967) have calculated the interaction energy between a spherical inhomogeneity and a center of dilatation. The center of dilatation may be attracted to or repelled by the inhomogeneity, depending on the rigidity of the inhomogeneity and the matrix material. Recently, Zhang and Chou (1985) have obtained explicit expressions for the stress field and strain energy for an antiplane eigenstrain problem of an elliptic inclusion in a two-phase anisotropic medium.

Anisotropic inclusions

In this chapter we investigate elastic fields due to ellipsoidal inclusions in anisotropic materials. Since explicit expressions for anisotropic Green's functions are not available, the technique used in the last chapter is not applicable. We start from the result in Section 6 and discuss how to integrate the integrals involved in the Fourier space and physical space.

The results are presented in general forms, which are applicable to any distribution of eigenstrains given in an ellipsoidal inclusion. Known solutions can be derived as special cases from these general results.

16. Elastic field of an ellipsoidal inclusion

When eigenstrain $\epsilon_{ij}^*(x)$ is distributed in an ellipsoidal sub-domain Ω of an infinitely extended anisotropic material, the displacement is expressed by (6.2),

$$u_i(x) = -(2\pi)^{-3} \frac{\partial}{\partial x_i} \int_{\Omega} dx' \int_{-\infty}^{\infty} C_{jlmn} \epsilon_{nm}^*(x') N_{lj}(\xi) D^{-1}(\xi) \times \exp\{i\xi \cdot (x - x')\} d\xi \quad (16.1)$$

or, changing ξ to $-\xi$, by

$$u_i(x) = -(2\pi)^{-3} \frac{\partial}{\partial x_i} \int_{\Omega} dx' \int_{-\infty}^{\infty} C_{jlmn} \epsilon_{nm}^*(x') N_{lj}(\xi) D^{-1}(\xi) \times \exp\{-i\xi \cdot (x - x')\} d\xi, \quad (16.2)$$

where the integral with respect to x'_1, x'_2, x'_3 is defined in domain Ω , which is the ellipsoid

$$x_1^2/a_1^2 + x_2^2/a_2^2 + x_3^2/a_3^2 \leq 1$$

(see Fig. 11.1).

The integration with respect to the ξ -space is considered first. The volume element in the ξ -space, $d\xi$, is

$$d\xi = d\xi_1 d\xi_2 d\xi_3 = \xi^2 d\xi dS(\bar{\xi}), \quad (16.3)$$

where

$$\begin{aligned} \xi &= (\xi_1^2 + \xi_2^2 + \xi_3^2)^{1/2}, \\ \bar{\xi} &= \xi/\xi, \end{aligned} \quad (16.4)$$

and $dS(\bar{\xi})$ is a surface element on the unit sphere S^2 in the ξ -space. Then, (16.1) is written as

$$\begin{aligned} u_i(x) &= -(2\pi)^{-3} \frac{\partial}{\partial x_i} \int_{\Omega} dx' \int_0^\infty \xi^2 d\xi \int_{S^2} C_{jlmn} \epsilon_{nm}^*(x') N_{ij}(\xi) D^{-1}(\xi) \\ &\quad \times \exp\{i\xi \cdot (x - x')\} dS(\bar{\xi}), \end{aligned} \quad (16.5)$$

and (16.2) leads to

$$\begin{aligned} u_i(x) &= -(2\pi)^{-3} \frac{\partial}{\partial x_i} \int_{\Omega} dx' \int_{-\infty}^0 \xi^2 d\xi \int_{S^2} C_{jlmn} \epsilon_{nm}^*(x') N_{ij}(\xi) D^{-1}(\xi) \\ &\quad \times \exp\{i\xi \cdot (x - x')\} dS(\bar{\xi}), \end{aligned} \quad (16.6)$$

where ξ is changed to $-\xi$. The sum of (16.5) and (16.6) is

$$\begin{aligned} u_i(x) &= -\frac{1}{2}(2\pi)^{-3} \frac{\partial}{\partial x_i} \int_{\Omega} dx' \int_{-\infty}^\infty d\xi \int_{S^2} C_{jlmn} \epsilon_{nm}^*(x') N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) \\ &\quad \times \exp\{i\xi \bar{\xi} \cdot (x - x')\} dS(\bar{\xi}). \end{aligned} \quad (16.7)$$

In the above integrals the homogeneity of degree zero of $\xi^2 N_{ij}(\xi) D^{-1}(\xi)$ has been used, namely,

$$\xi^2 N_{ij}(\xi) D^{-1}(\xi) = N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}). \quad (16.8)$$

Since $\int_{-\infty}^\infty \exp(i\xi \eta) d\xi = 2\pi\delta(\eta)$, (16.7) becomes

$$\begin{aligned} u_i(x) &= -\frac{1}{8\pi^2} \frac{\partial}{\partial x_i} \int_{\Omega} dx' \int_{S^2} C_{jlmn} \epsilon_{nm}^*(x') N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) \delta(\bar{\xi} \cdot (x - x')) dS(\bar{\xi}). \\ &\quad (16.9) \end{aligned}$$

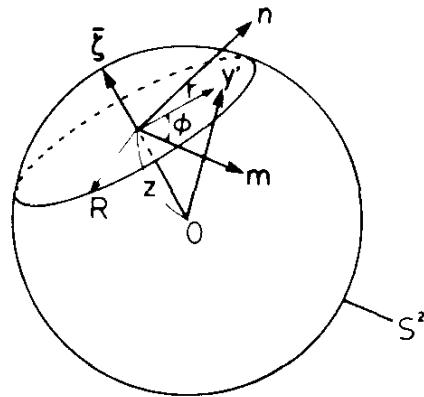


Fig. 16.1 Unit sphere and parameters

The following transformations of variables are used:

$$\begin{aligned} x_1/a_1 &= y_1, & x_2/a_2 &= y_2, & x_3/a_3 &= y_3, \\ x'_1/a_1 &= y'_1, & x'_2/a_2 &= y'_2, & x'_3/a_3 &= y'_3, \\ a_1 \xi_1 &= \xi_1, & a_2 \xi_2 &= \xi_2, & a_3 \xi_3 &= \xi_3, \\ \xi_1/\xi &= \bar{\xi}_1, & \xi_2/\xi &= \bar{\xi}_2, & \xi_3/\xi &= \bar{\xi}_3, \end{aligned}$$

$$\xi = (\xi_1^2 + \xi_2^2 + \xi_3^2)^{1/2} = (a_1^2 \xi_1^2 + a_2^2 \xi_2^2 + a_3^2 \xi_3^2)^{1/2}, \quad (16.10)$$

$$\bar{\xi} \cdot y = \bar{\xi} \cdot x / \xi.$$

We have also

$$dx' = dx'_1 dx'_2 dx'_3 = a_1 a_2 a_3 dy'_1 dy'_2 dy'_3 = a_1 a_2 a_3 r dr d\phi dz, \quad (16.11)$$

where r , ϕ and z are shown in Fig. 16.1 and

$$z = \bar{\xi} \cdot y' = \bar{\xi} \cdot x' / \xi. \quad (16.12)$$

Note that (16.10) transforms the ellipsoid into the unit sphere S^2 . Since

$$\partial/\partial x_i \delta(\bar{\xi} \cdot (x - x')) = \bar{\xi}_i \delta'(\bar{\xi} \cdot (x - x')), \quad (16.13)$$

where δ' is the derivative of δ with respect to the argument, (16.9) is written as

$$\begin{aligned} u_i(x) &= -\frac{a_1 a_2 a_3}{8\pi^2} \int_{-1}^1 dz \int_0^{2\pi} d\phi \int_0^R r dr \int_{S^2} C_{klmn} \epsilon_{nm}^*(x') N_{ik}(\bar{\xi}) \\ &\quad \times D^{-1}(\bar{\xi}) \bar{\xi}_i \delta'(\bar{\xi} \bar{\xi} \cdot y - \xi z) dS(\bar{\xi}), \end{aligned} \quad (16.14)$$

where

$$R = (1 - z^2)^{1/2} \quad (16.15)$$

must be a real number.

Differentiation of (16.14) with respect to x_j leads to

$$\begin{aligned} u_{i,j}(x) = & -\frac{a_1 a_2 a_3}{8\pi^2} \int_{-1}^1 dz \int_0^{2\pi} d\phi \int_0^R r dr \int_{S^2} C_{klmn} \epsilon_{nm}^*(x') N_{ik}(\xi) D^{-1}(\xi) \\ & \times \bar{\xi}_l \bar{\xi}_j \delta''(\xi \cdot y - z) dS(\xi). \end{aligned} \quad (16.16)$$

The argument $\xi(\bar{\xi} \cdot y - z)$ of the derivatives of Dirac's delta function becomes zero when

$$\bar{\xi} \cdot y = z. \quad (16.17)$$

When point y is inside the unit sphere, $z^2 < 1$ for any ξ , and R becomes a real number. When point y is outside the unit sphere, R is real for only certain ξ . In other words, for $x \in \Omega$, the integration domain for ξ is the whole surface of the unit sphere S^2 , and for $x \notin \Omega$ the integration domain becomes the subspace of S^2 satisfying the condition $z^2 < 1$. The subspace will be denoted by S^* , which is the unshaded domain seen in Fig. 16.2.

The expressions (16.14) and (16.16) and the other modified expressions appearing in later sections remain in the same form for any arbitrarily oriented coordinate system, except that ξ is taken as

$$\xi = \left\{ a_1^2 (a_{j1} \bar{\xi}_j)^2 + a_2^2 (a_{j2} \bar{\xi}_j)^2 + a_3^2 (a_{j3} \bar{\xi}_j)^2 \right\}^{1/2}, \quad (16.18)$$

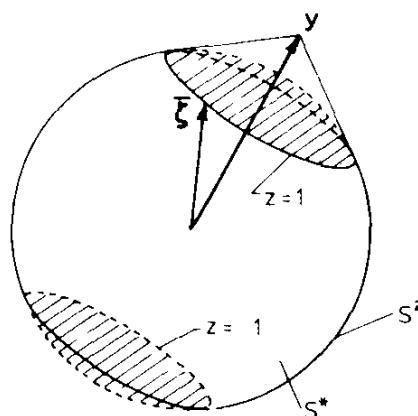


Fig. 16.2 S^* is a subspace of S satisfying condition $|\xi \cdot y| \leq 1$ for a given y

where a_{jl} is the direction cosine between the x_j -axis and the l^{th} principal direction of the ellipsoid.

17. Formulae for interior points

Let us consider the case when point x is inside the ellipsoid (interior point) or point y is inside the unit sphere. Since $\delta'(\xi\xi \cdot y - \xi z) = -\xi^{-1}(\partial/\partial z)\delta(\xi\xi \cdot y - \xi z)$, the integration of (16.14) by parts with respect to z leads to

$$\begin{aligned} u_i(x) &= -\frac{a_1 a_2 a_3}{8\pi^2} \int_{-1}^1 dz \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial}{\partial z} \epsilon_{nm}^*(x') - z \{ \epsilon_{nm}^*(x') \}_{r=R} \right] \\ &\quad \times \int_{S^2} C_{klmn} N_{ik}(\xi) D^{-1}(\xi) \bar{\xi}_l \xi^{-1} \delta(\xi\xi \cdot y - \xi z) dS(\xi), \end{aligned} \quad (17.1)$$

where R and x' are only functions of z for a fixed ξ through (16.15) and (16.12). The boundary values of the integration by parts vanish, since

$$\delta(\xi\xi \cdot y \pm \xi) = 0 \quad (17.2)$$

for interior points. Furthermore, for the interior points there exists a z satisfying the condition

$$\bar{\xi} \cdot y - z = 0, \quad -1 \leq z \leq 1, \quad (17.3)$$

for any ξ . Then, (17.1) can be integrated with respect to z ,

$$\begin{aligned} u_i(x) &= -\frac{a_1 a_2 a_3}{8\pi^2} \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial}{\partial z} \epsilon_{nm}^*(x') - (\bar{\xi} \cdot y) \{ \epsilon_{nm}^*(x') \}_{r=R} \right]_{z=\bar{\xi} \cdot y} \\ &\quad \times \int_{S^2} C_{klmn} N_{ik}(\xi) D^{-1}(\xi) \bar{\xi}_l \xi^{-2} dS(\xi), \end{aligned} \quad (17.4)$$

where

$$R = \{1 - z^2\}^{1/2}. \quad (17.5)$$

The points x' in (17.4) must satisfy the condition

$$\xi(\bar{\xi} \cdot y - \bar{\xi} \cdot y') = \bar{\xi} \cdot (x - x') = 0 \quad (17.6)$$

or

$$z = \bar{\xi} \cdot y. \quad (17.7)$$

Similarly, (16.16) becomes, after twice integrating by parts with respect to z ,

$$\begin{aligned} u_{i,j}(x) = & -\frac{a_1 a_2 a_3}{8\pi^2} \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial^2}{\partial z^2} \epsilon_{nm}^*(x') - z \left\{ \frac{\partial}{\partial z} \epsilon_{nm}^*(x') \right\}_{r=R} \right. \\ & \left. - z \frac{\partial}{\partial z} \{ \epsilon_{nm}^*(x') \}_{r=R} - \{ \epsilon_{nm}^*(x') \}_{r=R} \right]_{z=\bar{\xi} \cdot y} \\ & \times \int_{S^2} C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \bar{\xi}_j \bar{\xi}^{-3} dS(\bar{\xi}), \end{aligned} \quad (17.8)$$

where x' is subject to condition (17.6), and R is defined by (17.5). The above result can also be derived by differentiating (17.4), where $(\bar{\xi} \cdot y)_{,j} = \bar{\xi}_j \bar{\xi}^{-1}$ and $R_{,j} = -(\bar{\xi} \cdot y) \bar{\xi}_j \bar{\xi}^{-1} R^{-1}$ at $z = (\bar{\xi} \cdot y)$. It can be seen from (17.8) that if ϵ_{nm}^* is constant, $u_{i,j}$ is also constant, and if ϵ_{nm}^* is a linear function of coordinates, $u_{i,j}$ is also a linear function of coordinates.

The surface element $dS(\bar{\xi})$ can be transformed into a new surface element defined at point $\bar{\xi}$ by the use of (16.10). Let $dS(\bar{\xi})$ be a surface element constructed by the vector product of $d\bar{\xi}$ and $\delta\bar{\xi}$, and $dS(\bar{\xi})$, by the vector product of $d\bar{\xi}$ and $\delta\bar{\xi}$, where $d\bar{\xi}_1 = a_1 d\bar{\xi}_1/\bar{\xi}$, $\delta\bar{\xi}_1 = a_1 \delta\bar{\xi}_1/\bar{\xi}$, etc. (see Fig. 17.1). Then,

$$\begin{aligned} dS(\bar{\xi}) &= \begin{vmatrix} \bar{\xi}_1 & \bar{\xi}_2 & \bar{\xi}_3 \\ d\bar{\xi}_1 & d\bar{\xi}_2 & d\bar{\xi}_3 \\ \delta\bar{\xi}_1 & \delta\bar{\xi}_2 & \delta\bar{\xi}_3 \end{vmatrix}, \\ dS(\bar{\xi}) &= \begin{vmatrix} a_1 \bar{\xi}_1/\bar{\xi} & a_2 \bar{\xi}_2/\bar{\xi} & a_3 \bar{\xi}_3/\bar{\xi} \\ a_1 d\bar{\xi}_1/\bar{\xi} & a_2 d\bar{\xi}_2/\bar{\xi} & a_3 d\bar{\xi}_3/\bar{\xi} \\ a_1 \delta\bar{\xi}_1/\bar{\xi} & a_2 \delta\bar{\xi}_2/\bar{\xi} & a_3 \delta\bar{\xi}_3/\bar{\xi} \end{vmatrix}. \end{aligned} \quad (17.9)$$

Therefore, we have the relation

$$dS(\bar{\xi}) = a_1 a_2 a_3 \bar{\xi}^{-3} dS(\bar{\xi}). \quad (17.10)$$

Uniform eigenstrains

The displacement and strain fields for interior points become as follows when

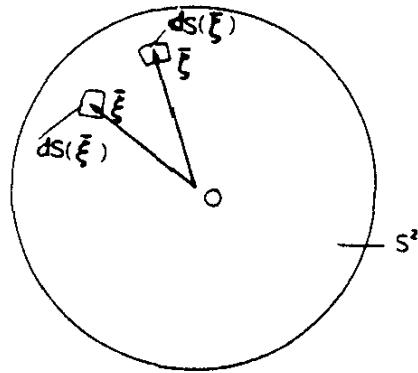


Fig. 17.1 Transformation of the surface element

ϵ_{ij}^* is uniform:

$$u_i(x) = \frac{a_1 a_2 a_3}{4\pi} C_{klmn} \epsilon_{nm}^* \int_{S^2} (\bar{\xi} \cdot x) N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \bar{\xi}_j \xi^{-3} dS(\bar{\xi}) \quad (17.11)$$

and

$$u_{i,j}(x) = \frac{a_1 a_2 a_3}{4\pi} C_{klmn} \epsilon_{nm}^* \int_{S^2} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \bar{\xi}_j \xi^{-3} dS(\bar{\xi}), \quad (17.12)$$

or, using (17.10),

$$u_i(x) = (1/4\pi) C_{klmn} \epsilon_{nm}^* \int_{S^2} (\bar{\xi} \cdot x) N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l dS(\bar{\xi}) \quad (17.13)$$

and

$$u_{i,j}(x) = (1/4\pi) C_{klmn} \epsilon_{nm}^* \int_{S^2} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \bar{\xi}_j dS(\bar{\xi}). \quad (17.14)$$

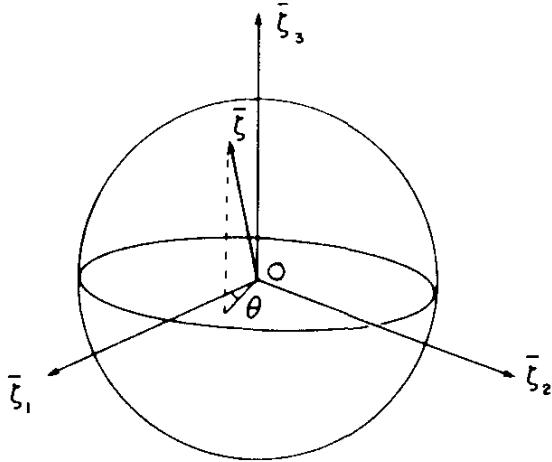
Equation (17.11) has been derived by Kinoshita and Mura (1971).

Furthermore, the surface element $dS(\bar{\xi})$ can be written as

$$dS(\bar{\xi}) = d\bar{\xi}_3 d\theta, \quad (17.15)$$

where θ is measured counter-clockwise from the $\bar{\xi}_1$ -axis as shown in Fig. 17.2. Then, (17.13) is written as

$$u_i(x) = (1/4\pi) C_{jlmn} \epsilon_{nm}^* x_k \int_{-1}^1 d\bar{\xi}_3 \int_0^{2\pi} G_{ijkl}(\bar{\xi}) d\theta, \quad (17.16)$$

Fig. 17.2 New coordinate system $\bar{\xi}_3$ and θ

where

$$G_{ijkl}(\bar{\xi}) = \bar{\xi}_k \bar{\xi}_l N_{ij}(\bar{\xi}) / D(\bar{\xi}). \quad (17.17)$$

If the notation introduced in (11.15) is used, we can write

$$\epsilon_{ij} = S_{ijmn} \epsilon_{mn}^*, \quad (17.18)$$

where

$$S_{ijmn} = (1/8\pi) C_{pqmn} \int_{-1}^1 d\bar{\xi}_3 \int_0^{2\pi} \{ G_{ipjq}(\bar{\xi}) + G_{jpqi}(\bar{\xi}) \} d\theta. \quad (17.19)$$

From (16.10)

$$\bar{\xi}_1 = \xi \bar{\xi}_1 / a_1, \quad \bar{\xi}_2 = \xi \bar{\xi}_2 / a_2, \quad \bar{\xi}_3 = \xi \bar{\xi}_3 / a_3; \quad (17.20)$$

however, since $G_{ijkl}(\bar{\xi}_1, \bar{\xi}_2, \bar{\xi}_3)$ are homogeneous polynomials of degree 0, we can use

$$\bar{\xi}_1 = \bar{\xi}_1 / a_1, \quad \bar{\xi}_2 = \bar{\xi}_2 / a_2, \quad \bar{\xi}_3 = \bar{\xi}_3 / a_3, \quad (17.21)$$

for $\bar{\xi}$ as the argument of $G_{ijkl}(\bar{\xi})$. The components of $\bar{\xi}$ are

$$\bar{\xi}_1 = (1 - \bar{\xi}_3^2)^{1/2} \cos \theta, \quad (17.22)$$

$$\bar{\xi}_2 = (1 - \bar{\xi}_3^2)^{1/2} \sin \theta, \quad (17.22)$$

$$\bar{\xi}_3 = \bar{\xi}_3;$$

see Fig. 17.2.

The integrals with respect to θ in (17.16) or (17.19) can be obtained by the residue calculation in a complex Z -plane, where

$$\begin{aligned}\cos \theta &= (Z + Z^{-1})/2, \\ \sin \theta &= (Z - Z^{-1})/2i, \\ d\theta &= dZ/iZ.\end{aligned}\tag{17.23}$$

Then,

$$\int_0^{2\pi} G_{ijkl}(\bar{\xi}) d\theta = 2\pi R [G_{ijkl}/Z],\tag{17.24}$$

where $R[G_{ijkl}/Z]$ is the sum of the residues of the function G_{ijkl}/Z existing within the unit circle $|Z| = 1$ under a fixed value of $\bar{\xi}_3$. Then (17.16) is written as

$$u_i(x) = \frac{1}{4\pi} C_{jlmn} \epsilon_{nm}^* x_k \bar{G}_{ijkl},\tag{17.25}$$

where

$$\bar{G}_{ijkl} = 2\pi \int_{-1}^1 R [G_{ijkl}/Z] d\bar{\xi}_3.\tag{17.26}$$

Also, we have, from (17.19),

$$S_{ijmn} = (1/8\pi) C_{pqmn} (\bar{G}_{ipjq} + \bar{G}_{jpqi}).\tag{17.27}$$

Some special cases are considered below; see Lin and Mura (1973).

Spheroid ($a_1 = a_2$, $a_1/a_3 = \rho$)

The crystalline directions are assumed to be coincident with the principal directions of the spheroid.

For cubic crystals, nonzero components of \bar{G}_{ijkl} are given below:

$$\begin{aligned}\bar{G}_{1111} &= \bar{G}_{2222} \\ &= \frac{2\pi}{a} \int_0^1 \frac{(1-x^2)}{pq} (1-x^2+\rho^2x^2) [\mu^2(1-x^2+\rho^2x^2) + \beta\rho^2x^2] dx \\ &\quad + \frac{\pi}{\alpha} \int_0^1 \frac{(1-x^2)^2}{p(p+q)} [\beta(1-x^2+\rho^2x^2) + \gamma\rho^2x^2] dx,\end{aligned}$$

$$\begin{aligned}
\bar{G}_{3333} &= \frac{4\pi}{a} \int_0^1 \frac{\rho^2 x^2}{pq} (1 - x^2 + \rho^2 x^2) [\mu^2(1 - x^2 + \rho^2 x^2) + \beta(1 - x^2)] dx \\
&\quad + \frac{\pi\gamma}{a} \int_0^1 \frac{\rho^2 x^2 (1 - x^2)^2}{p(p+q)} dx, \\
\bar{G}_{1122} &= \bar{G}_{2211} \\
&= \frac{2\pi}{a} \int_0^1 \frac{(1 - x^2)}{pq} \left\{ (1 - x^2 + \rho^2 x^2) [\mu^2(1 - x^2 + \rho^2 x^2) + \beta\rho^2 x^2] \right. \\
&\quad \left. + (1 - x^2) [\beta(1 - x^2 + \rho^2 x^2) + \gamma\rho^2 x^2] \right\} dx \\
&\quad - \frac{\pi}{a} \int_0^1 \frac{(1 - x^2)^2}{p(p+q)} [\beta(1 - x^2 + \rho^2 x^2) + \gamma\rho^2 x^2] dx,
\end{aligned} \tag{17.28}$$

$$\begin{aligned}
\bar{G}_{1133} &= \bar{G}_{2233} \\
&= \frac{2\pi}{a} \int_0^1 \frac{\rho^2 x^2}{pq} \left\{ 2(1 - x^2 + \rho^2 x^2) [\mu^2(1 - x^2 + \rho^2 x^2) + \beta\rho^2 x^2] \right. \\
&\quad \left. + (1 - x^2) [\beta(1 - x^2 + \rho^2 x^2) + \gamma\rho^2 x^2] \right\} dx,
\end{aligned}$$

$$\bar{G}_{1212} = -\frac{\pi(\lambda + \mu)}{a} \int_0^1 \frac{(1 - x^2)^2}{p(p+q)} [\mu(1 - x^2 + \rho^2 x^2) + \mu'\rho^2 x^2] dx,$$

$$\begin{aligned}
\bar{G}_{1313} &= \bar{G}_{2323} \\
&= -\frac{2\pi\mu(\lambda + \mu)}{a} \int_0^1 \frac{\rho^2 x^2 (1 - x^2)(1 - x^2 + \rho^2 x^2)}{pq} dx \\
&\quad - \frac{\pi\mu'(\lambda + \mu)}{a} \int_0^1 \frac{\rho^2 x^2 (1 - x^2)^2}{p(p+q)} dx,
\end{aligned}$$

$$\begin{aligned}
\bar{G}_{3311} &= \bar{G}_{3322} \\
&= \frac{2\pi}{a} \int_0^1 \frac{(1 - x^2)}{pq} (1 - x^2 + \rho^2 x^2) [\mu^2(1 - x^2 + \rho^2 x^2) + \beta(1 - x^2)] dx \\
&\quad + \frac{\pi\gamma}{2a} \int_0^1 \frac{(1 - x^2)^3}{p(p+q)} dx,
\end{aligned}$$

where

$$\rho = a_1/a_3,$$

$$a = \mu^2(\lambda + 2\mu + \mu'),$$

$$b = a^{-1}\mu\mu'(2\lambda + 2\mu + \mu'),$$

$$c = a^{-1}\mu'^2(3\lambda + 3\mu + \mu'),$$

$$\beta = \mu(\lambda + \mu + \mu'), \quad (17.29)$$

$$\gamma = \mu'(2\lambda + 2\mu + \mu'),$$

$$p = \left\{ (1 - x^2 + \rho^2 x^2)^3 + b\rho^2 x^2(1 - x^2)(1 - x^2 + \rho^2 x^2) \right. \\ \left. + \frac{1}{4}(1 - x^2)^2 [b(1 - x^2 + \rho^2 x^2) + c\rho^2 x^2] \right\}^{1/2}, \quad 0 < x < 1,$$

$$q = \left\{ (1 - x^2 + \rho^2 x^2)^3 + b\rho^2 x^2(1 - x^2)(1 - x^2 + \rho^2 x^2) \right\}^{1/2}, \quad 0 < x < 1.$$

It has been verified that p^2 and q^2 are positive for most of the cubic crystals. When $\rho = 1$, \bar{G}_{1111} and \bar{G}_{1212} agree with Kneer's results (1965). However, \bar{G}_{1122} of Kneer's is incorrect, since it does not reduce to the isotropic case.

For hexagonal crystals, the elastic moduli are denoted by

$$C_{11} = d,$$

$$\frac{1}{2}(C_{11} - C_{12}) = e,$$

$$C_{44} = f, \quad (17.30)$$

$$C_{13} + C_{44} = g,$$

$$C_{33} = h,$$

where C_{ij} are the Voigt constants. The nonzero components of \bar{G}_{ijkl} are given below:

$$\begin{aligned}
 \bar{G}_{1111} = \bar{G}_{2222} &= \frac{1}{2}\pi \int_0^1 \Delta(1-x^2) \left\{ [f(1-x^2) + h\rho^2x^2] \right. \\
 &\quad \times \left. [(3e+d)(1-x^2) + 4f\rho^2x^2] - g^2\rho^2x^2(1-x^2) \right\} dx, \\
 \bar{G}_{3333} &= 4\pi \int_0^1 \Delta \rho^2 x^2 [d(1-x^2) + f\rho^2x^2] [e(1-x^2) + f\rho^2x^2] dx, \\
 \bar{G}_{1122} = \bar{G}_{2211} &= \frac{1}{2}\pi \int_0^1 \Delta(1-x^2) \left\{ [f(1-x^2) + h\rho^2x^2] \right. \\
 &\quad \times \left. [(e+3d)(1-x^2) + 4f\rho^2x^2] - 3g^2\rho^2x^2(1-x^2) \right\} dx, \\
 \bar{G}_{1133} = \bar{G}_{2233} &= 2\pi \int_0^1 \Delta \rho^2 x^2 \left\{ [(d+e)(1-x^2) + 2f\rho^2x^2] \right. \\
 &\quad \times \left. [f(1-x^2) + h\rho^2x^2] - g^2\rho^2x^2(1-x^2) \right\} dx, \\
 \bar{G}_{3311} = \bar{G}_{3322} &= 2\pi \int_0^1 \Delta(1-x^2) [d(1-x^2) + f\rho^2x^2] [e(1-x^2) + f\rho^2x^2] dx, \\
 \bar{G}_{1212} &= \frac{1}{2}\pi \int_0^1 \Delta(1-x^2)^2 \left\{ g^2\rho^2x^2 - (d-e)[f(1-x^2) + h\rho^2x^2] \right\} dx, \\
 \bar{G}_{1313} = \bar{G}_{2323} &= (-2\pi) \int_0^1 \Delta g \rho^2 x^2 (1-x^2) [e(1-x^2) + f\rho^2x^2] dx,
 \end{aligned} \tag{17.31}$$

where

$$\begin{aligned}
 \Delta^{-1} &= [e(1-x^2) + f\rho^2x^2] \left\{ [d(1-x^2) + f\rho^2x^2] [f(1-x^2) + h\rho^2x^2] \right. \\
 &\quad \left. - g^2\rho^2x^2(1-x^2) \right\}. \tag{17.31.1}
 \end{aligned}$$

When $\rho = 1$, all integrals except \bar{G}_{3311} agree with Kneer's results (1965). Isotropic materials are obtained by a limiting process: $d \rightarrow \lambda + 2\mu$, $e \rightarrow \mu$, $f \rightarrow \mu$, $g \rightarrow \lambda + \mu$, and $h \rightarrow \lambda + 2\mu$.

Cylinder ($a_3 \rightarrow \infty$) (elliptic inclusion)

A rod or a needle-shaped inclusion can be approximated by a cylindrical inclusion. When $a_3 \rightarrow \infty$, (17.21) becomes

$$\bar{\xi}_1 = \bar{\xi}_1/a_1, \quad \bar{\xi}_2 = \bar{\xi}_2/a_2, \quad \bar{\xi}_3 = 0. \quad (17.32)$$

Since $G_{ijkl}(\bar{\xi}_1/a_1, \bar{\xi}_2/a_2, 0)$ are homogeneous functions of degree 0, the factor $(1 - \bar{\xi}_3^2)^{1/2}$ in (17.22) can be dropped. Then G_{ijkl} are independent of $\bar{\xi}_3$ and we have

$$u_i(x) = (1/2\pi) C_{jlmn} \epsilon_{mn}^* x_k \int_0^{2\pi} G_{ijkl}(\bar{\xi}_1/a_1, \bar{\xi}_2/a_2, 0) d\theta \quad (17.33)$$

with

$$\bar{\xi}_1 = \cos \theta, \quad \bar{\xi}_2 = \sin \theta. \quad (17.34)$$

When the coordinate axes x_i are not taken along the directions of the principal axes of the cylinder, we have

$$u_i(x) = (1/2\pi) C_{jlmn} \epsilon_{mn}^* x_k \int_0^{2\pi} G_{ijkl}(a_1 \bar{\xi}_p/a_p, a_2 \bar{\xi}_p/a_p, 0) d\theta, \quad (17.35)$$

where $p = 1, 2$ and a_{jp} are the direction cosines between the x_j -axis and the p^{th} principal directions of the ellipsoid.

If the cylinder has a circular cross section ($a_1 = a_2$), \bar{G}_{ijkl} are obtained from (17.28) by taking $\rho = 0$.

Jaswon and Bhargava (1961) and List (1969) were able to obtain explicit solutions for interior and exterior points by using complex representation. Jaswon and Bhargava's analysis has been extended by Willis (1964) to the anisotropic case with cubic symmetry. Subsequent anisotropic analyses have been performed by Bhargava and Radhakrishna (1963, 1964) for an orthotropic medium and by Chen (1967) for a medium with one plane of symmetry. These treatments of two-dimensional inclusion problems are restricted to the state of plane strain or plane stress, based on the classical formulation for the plane theory of elasticity (e.g., Green and Zerna 1954). However, for a general anisotropic medium it is not always possible to treat the plane strain and antiplane strain fields independently. Yang and Chou (1976) have presented a general method for solving the generalized plane problems of inclusions in anisotropic solids. They have used Green's function (5.84) for the generalized plane problems developed by Eshelby, Read, and Shockley (1953).

The following is the result obtained by Yang and Chou (1976) for orthorhombic materials with elastic moduli given by (A2.17), where eigenstrain ϵ_{ij}^* is assumed to be uniform in the elliptic domain with the half-axes a_1 and a_2 . For interior points,

$$\begin{aligned}\sigma_{11} &= -\frac{A^2(\bar{C}_{12}^2 - C_{12}^2)}{2\bar{C}_{12}Q} \left[2\epsilon_{11}^* + \frac{e}{A \sin \alpha} (A^2\epsilon_{11}^* + \epsilon_{22}^*) \right], \\ \sigma_{22} &= -\frac{(\bar{C}_{12}^2 - C_{12}^2)}{2\bar{C}_{12}Q} \left[\frac{e}{A \sin \alpha} (A^2\epsilon_{11}^* + \epsilon_{22}^*) + 2e^2\epsilon_{22}^* \right], \\ \sigma_{12} &= -\frac{A(\bar{C}_{12}^2 - C_{12}^2)}{\bar{C}_{12}Q \sin \alpha} e\epsilon_{12}^*,\end{aligned}\tag{17.35.1}$$

where

$$\begin{aligned}A &= (C_{11}/C_{22})^{1/4}, \quad \bar{C}_{12} = (C_{11}C_{22})^{1/2}, \quad e = a_2/a_1, \\ \alpha &= \cos^{-1}(\sqrt{-C}/2) \quad \text{for } -4 < C \leq 0, \\ &= \cos^{-1}(i\sqrt{C}/2) \quad \text{for } C > 0,\end{aligned}\tag{17.35.2}$$

$$C = (\bar{C}_{12} + C_{12})(\bar{C}_{12} - C_{12} - 2C_{66})/\bar{C}_{12}C_{66},$$

$$Q = 1 + e^2A^2 + 2eA \sin \alpha.$$

The component σ_{33} can be obtained from the condition $\epsilon_{33} = 0$. When isotropy is approached ($\lambda = 1$ and $C = 0$), equations (17.35.1) reduce to the results of Jaswon and Bhargava.

The strain energy is

$$\begin{aligned}W^* &= \frac{\pi a_1 a_2 (\bar{C}_{12}^2 - C_{12}^2)}{4\bar{C}_{12}Q} \left[2(A^2\epsilon_{11}^{*2} + e^2\epsilon_{22}^{*2}) + \frac{e}{A \sin \alpha} (A^2\epsilon_{11}^* + \epsilon_{22}^*)^2 \right] \\ &\quad + \frac{\pi a_1 a_2 A (\bar{C}_{12}^2 - C_{12}^2)}{\bar{C}_{12}Q \sin \alpha} \epsilon_{12}^{*2}.\end{aligned}\tag{17.35.3}$$

Yang and Chou (1977) further obtained solutions of antiplane strain problems of an elliptic inclusion in an anisotropic medium.

Flat ellipsoid ($a_3 \rightarrow 0$)

When $a_3 \rightarrow 0$, from (17.21),

$$\bar{\xi}_1, \bar{\xi}_2 \ll \bar{\xi}_3; \quad (17.36)$$

therefore, we can put $\bar{\xi}_1 = \bar{\xi}_2 = 0$ for the argument of $G_{ijkl}(\bar{\xi})$. Then $G_{ijkl}(0, 0, \bar{\xi}_3)$ are homogeneous functions of degree 0, and we set $\bar{\xi}_3 = 1$. Therefore, (17.16) becomes

$$u_i(x) = C_{jlmn} \epsilon_{nm}^* x_k G_{ijkl}(0, 0, 1). \quad (17.37)$$

When the coordinate axes are not taken along the principal directions of the flat ellipsoid, we have

$$u_i(x) = C_{jlmn} \epsilon_{nm}^* x_k G_{ijkl}(a_{13}, a_{23}, a_{33}), \quad (17.37.1)$$

where a_{jp} is the direction cosine between the x_j -axis and the p^{th} principal direction of the flat ellipsoid.

When the x -coordinate axes are taken in the principal directions of the flat ellipsoid, the displacement in the flat ellipsoid is identical to zero, since $G_{ijkl}(0, 0, 1)$ in (17.37) becomes $\delta_{k3} \delta_{l3} N_{ij}(0, 0, 1)/D(0, 0, 1)$ and, therefore, $x_k \delta_{k3} = x_3 = 0$ on the flat ellipsoid. Then we have

$$u_i = 0, \quad u_{i,1} = 0 \quad \text{and} \quad u_{i,2} = 0. \quad (17.38)$$

The stress becomes

$$\sigma_{pq} = C_{pqi3} C_{j3mn} \epsilon_{nm}^* N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) - C_{pqik} \epsilon_{ki}^*, \quad (17.39)$$

where $\bar{\xi} = (0, 0, 1)$. By definition, we have from (5.5)

$$C_{pqik} \bar{\xi}_q \bar{\xi}_k N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) = \delta_{pj}. \quad (17.40)$$

Since $\bar{\xi}_1 = 0$, $\bar{\xi}_2 = 0$, and $\bar{\xi}_3 = 1$, (17.40) is identical to

$$C_{p3i3} N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) = \delta_{pj}. \quad (17.41)$$

Then, (17.39) leads to

$$\sigma_{p3} = 0, \quad (17.42)$$

which should be expected for the case of a very flat inclusion. Furthermore, we have from (17.37) and (17.41)

$$u_{i,3} = C_{j3m1}\epsilon_{1m}^* N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) + C_{j3m2}\epsilon_{2m}^* N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) + \epsilon_{3i}^*. \quad (17.43)$$

If

$$\epsilon_{11}^* = \epsilon_{22}^* = \epsilon_{12}^* = 0, \quad (17.44)$$

it is shown that

$$\frac{1}{2}(u_{1,3} + u_{3,1}) = \epsilon_{13}^*, \frac{1}{2}(u_{2,3} + u_{3,2}) = \epsilon_{23}^*, u_{3,3} = \epsilon_{33}^* \quad (17.45)$$

and any stress component vanishes,

$$\sigma_{pq} = 0. \quad (17.46)$$

In short, when $\epsilon_{11}^* = \epsilon_{22}^* = \epsilon_{12}^* = 0$, all the stress components become zero, resulting in vanishing elastic energy. Further, an arbitrary line element parallel to the flat surface does not change, since $u_{i,1} = u_{i,2} = 0$ from (17.37).

If the coordinate system is taken arbitrarily, (17.39) can be written as

$$\sigma_{pq} = C_{pqik} C_{jlmn} \epsilon_{mn}^* N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_k \bar{\xi}_l - C_{pqik} \epsilon_{ki}^*, \quad (17.47)$$

where $\bar{\xi} = (a_{13}, a_{23}, a_{33})$.

Eigenstrains with polynomial variation

For this case the stresses at interior and exterior points are investigated simultaneously; see Section 19.

Eigenstrains with a periodic form

The elastic field inside an ellipsoidal inclusion is considered when the distribution of eigenstrains inside the inclusion is periodic,

$$\epsilon_{nm}^*(x') = \bar{\epsilon}_{nm}^* \exp(ic_p x'_p/a_p) = \bar{\epsilon}_{nm}^* \exp(ic_p y'_p), \quad (17.48)$$

where $\bar{\epsilon}_{nm}^*$ are constants, c is a given vector, and the summation convention defined in (11.39) is used. From Fig. 16.1 we write

$$y' = z\bar{\xi} + r \cos \phi \mathbf{m} + r \sin \phi \mathbf{n}, \quad (17.49)$$

and, therefore,

$$c_p y'_p = (\mathbf{c} \cdot \bar{\xi}) z + r c_0 \cos(\phi - \alpha), \quad (17.50)$$

where (see Fig. 17.3)

$$(\mathbf{c} \cdot \mathbf{m})/c_0 = \cos \alpha, (\mathbf{c} \cdot \mathbf{n})/c_0 = \sin \alpha, c_0 = \{ \mathbf{c} \cdot \mathbf{c} - (\mathbf{c} \cdot \bar{\xi})^2 \}^{1/2}. \quad (17.51)$$

Then we have

$$\begin{aligned} \frac{\partial}{\partial z} \epsilon_{nm}^*(x') &= i(\mathbf{c} \cdot \bar{\xi}) \bar{\epsilon}_{nm}^* \exp(i c_p y'_p), \\ \frac{\partial^2}{\partial z^2} \epsilon_{nm}^*(x') &= -(\mathbf{c} \cdot \bar{\xi})^2 \bar{\epsilon}_{nm}^* \exp(i c_p y'_p). \end{aligned} \quad (17.52)$$

Substituting (17.52) into (17.4), we obtain

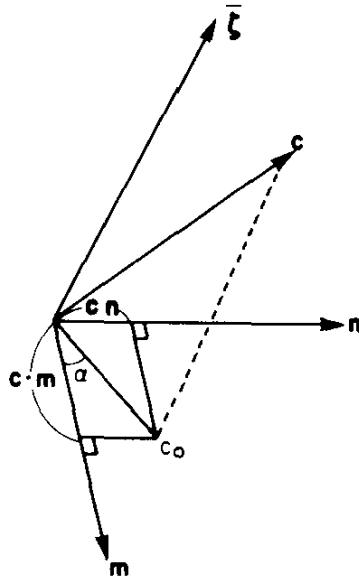
$$\begin{aligned} u_i(x) &= \frac{-a_1 a_2 a_3}{8\pi^2} \int_0^{2\pi} d\phi \left[\int_0^R r dr i(\mathbf{c} \cdot \bar{\xi}) \bar{\epsilon}_{nm}^* \right. \\ &\quad \times \exp\{i(\bar{\xi} \cdot \mathbf{y})(\mathbf{c} \cdot \bar{\xi}) + i r c_0 \cos(\phi - \alpha)\} \\ &\quad - (\bar{\xi} \cdot \mathbf{y}) \bar{\epsilon}_{nm}^* \exp\{i(\bar{\xi} \cdot \mathbf{y})(\mathbf{c} \cdot \bar{\xi}) \right. \\ &\quad \left. \left. + i R c_0 \cos(\phi - \alpha)\right\} \right] \\ &\quad \times \int_{S^2} C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \xi^{-2} dS(\bar{\xi}). \end{aligned} \quad (17.53)$$

From the properties of the Bessel functions,

$$\int_0^{2\pi} \exp\{i r c_0 \cos(\phi - \alpha)\} d\phi = 2\pi J_0(c_0 r), \{z J_1(z)\}' = z J_0(z), \quad (17.54)$$

and (17.53) is written as

$$\begin{aligned} u_i(x) &= \frac{-a_1 a_2 a_3}{4\pi} \int_{S^2} [i(\mathbf{c} \cdot \bar{\xi})(R/c_0) J_1(c_0 R) - (\bar{\xi} \cdot \mathbf{y}) J_0(c_0 R)] \bar{\epsilon}_{nm}^* \\ &\quad \times \exp\{i(\bar{\xi} \cdot \mathbf{y})(\mathbf{c} \cdot \bar{\xi})\} \bar{\xi}_l C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \xi^{-2} dS(\bar{\xi}). \end{aligned} \quad (17.55)$$

Fig. 17.3 Illustration of vector c

Similarly, (17.8) is written as

$$u_{i,j}(x) = \frac{a_1 a_2 a_3}{4\pi} \int_{S^2} \left[(R/c_0) J_1(c_0 R) (\mathbf{c} \cdot \bar{\xi})^2 + \{2i(\bar{\xi} \cdot \mathbf{y})(\mathbf{c} \cdot \bar{\xi}) + 1\} J_0(c_0 R) \right] \times \bar{\epsilon}_{nm}^* \exp\{i(\bar{\xi} \cdot \mathbf{y})(\mathbf{c} \cdot \bar{\xi})\} C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \bar{\xi}_j \bar{\xi}^{-3} dS(\bar{\xi}), \quad (17.56)$$

where

$$R = \left\{ 1 - (\bar{\xi} \cdot \mathbf{y})^2 \right\}^{1/2}, \quad \bar{\xi} \cdot \mathbf{y} = \bar{\xi} \cdot \mathbf{x} \bar{\xi}^{-1}. \quad (17.57)$$

The result (17.55) has been obtained by Mura, Mori and Kato (1976). This solution can be called the fundamental solution. When $\bar{\epsilon}_{nm}^*$ is given as an arbitrary function, it can be expressed by the Fourier series or integrals in terms of (17.48). The corresponding solution $u_{i,j}(x)$ is the sum or integral of (17.56) with respect to various values of c .

When the ellipsoid degenerates into a flat ellipsoid ($a_3 \rightarrow 0$), expression (17.55) simplifies. First the unit sphere S^2 is transformed into a cylinder as shown in Fig. 17.4, and the cylindrical surface element $d\Sigma$ reduces to $d\theta d\xi_3$, which is related to dS by

$$dS(\bar{\xi}) = d\theta d\xi_3 / (1 + \xi_3^2)^{3/2}. \quad (17.58)$$

Also, from (16.10),

$$\begin{aligned}\bar{\xi}_1 &= \cos \theta / (1 + \xi_3^2)^{1/2}, \quad \bar{\xi}_2 = \sin \theta / (1 + \xi_3^2)^{1/2}, \quad \bar{\xi}_3 = \xi_3 / (1 + \xi_3^2)^{1/2}, \\ \xi^{-3} dS(\bar{\xi}) &= (a_1^2 \cos^2 \theta + a_2^2 \sin^2 \theta + a_3^2 \xi_3^2)^{-3/2} d\theta d\xi_3,\end{aligned}\quad (17.59)$$

where $-\infty \leq \xi_3 \leq \infty$.

With use of the identity

$$\begin{aligned}&(a_1^2 \cos^2 \theta + a_2^2 \sin^2 \theta + a_3^2 \xi_3^2)^{-3/2} \\ &= (a_1^2 \cos^2 \theta + a_2^2 \sin^2 \theta)^{-1} \frac{\partial}{\partial \xi_3} \left\{ \xi_3 (a_1^2 \cos^2 \theta + a_2^2 \sin^2 \theta + a_3^2 \xi_3^2)^{-1/2} \right\},\end{aligned}\quad (17.60)$$

integration by parts with respect to ξ_3 is applied to (17.55). Then we have

$$\begin{aligned}u_i(x) &= (a_1 a_2 / 2\pi) C_{klmn} \bar{\epsilon}_{nm}^* \int_0^{2\pi} d\theta \left[\left\{ (x \cdot \bar{\xi}) J_0(c_0 R) - i c_p a_p \bar{\xi}_p R J_1(c_0 R) / c_0 \right. \right. \\ &\quad \times \exp \left\{ i(x \cdot \bar{\xi}) c_p a_p \bar{\xi}_p / \xi^2 \right\} N_{ik}(\bar{\xi}) \bar{\xi}_l D^{-1}(\bar{\xi}) \Big]_{\xi_3=\infty} \\ &\quad \times (a_1^2 \cos^2 \theta + a_2^2 \sin^2 \theta)^{-1} \\ &\quad - (a_1 a_2 a_3 / 4\pi) C_{klmn} \bar{\epsilon}_{nm}^* \int_0^{2\pi} d\theta \int_{-\infty}^{\infty} d\xi_3 (a_1^2 \cos^2 \theta + a_2^2 \sin^2 \theta)^{-1} \\ &\quad \times \xi_3 (a_1^2 \cos^2 \theta + a_2^2 \sin^2 \theta + a_3^2 \xi_3^2)^{-1/2} \\ &\quad \times \frac{\partial}{\partial \xi_3} \left[\left\{ (x \cdot \bar{\xi}) J_0(c_0 R) - i c_p a_p \bar{\xi}_p R J_1(c_0 R) / c_0 \right\} \right. \\ &\quad \times \exp \left\{ i(x \cdot \bar{\xi}) c_p a_p \bar{\xi}_p / \xi^2 \right\} N_{ik}(\bar{\xi}) \bar{\xi}_l D^{-1}(\bar{\xi}) \Big],\end{aligned}\quad (17.61)$$

where (16.10) has been used.

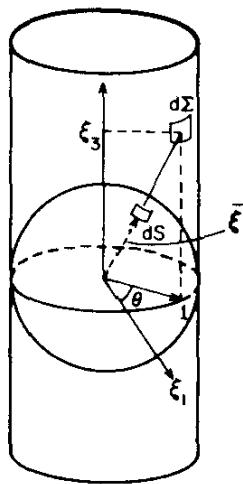


Fig. 17.4 The unit sphere S^2 is transformed into an infinitely long cylinder

For $\xi_3 \rightarrow \infty$, from (17.59),

$$\bar{\xi}_1 = 0, \quad \bar{\xi}_2 = 0, \quad \bar{\xi}_3 = 1; \quad (17.62)$$

therefore,

$$\begin{aligned} (\mathbf{x} \cdot \bar{\xi}) &= x_3, \quad \zeta = a_3, \\ c_0 &= (c_1^2 + c_2^2)^{1/2}, \quad R = (1 - x_3^2/a_3^2)^{1/2}. \end{aligned} \quad (17.63)$$

Furthermore, for $a_3 \rightarrow 0$, the second integral in the right-hand side of (17.61) vanishes. The factor [] in the integrand in the first integral is independent of θ and

$$\int_0^{2\pi} (a_1^2 \cos^2 \theta + a_2^2 \sin^2 \theta)^{-1} d\theta = 2\pi/a_1 a_2. \quad (17.64)$$

Equation (17.61) becomes, for $a_3 \rightarrow 0$,

$$u_i(\mathbf{x}) = C_{klmn} \bar{\epsilon}_{nm}^* x_3 J_0(c_0 R) \exp(ix_3 c_3/a_3) N_{ik}(\bar{\xi}) \bar{\xi}_l D^{-1}(\bar{\xi}), \quad (17.65)$$

where $\bar{\xi} = (0, 0, 1)$.

Since we are considering a point \mathbf{x} inside the inclusion, we set $x_3 = 0$ before

taking the limit $a_3 \rightarrow 0$. Then, (17.65) leads to

$$u_i(x) = 0 \quad (17.66)$$

for interior points. By taking the derivative of (17.65) with respect to x_j , we obtain the expressions for the displacement gradient and, therefore, the stress components throughout the inclusion,

$$\begin{aligned} u_{i,j}(x) &= C_{klmn} \bar{\epsilon}_{nm}^* J_0(c_0) \bar{\xi}_j \bar{\xi}_l N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}), \\ \sigma_{pq}(x) &= C_{pqij} \left[C_{klmn} \bar{\epsilon}_{mn}^* J_0(c_0) \bar{\xi}_j \bar{\xi}_l N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \right. \\ &\quad \left. - \bar{\epsilon}_{ij}^* \exp\{i(c_1 x_1/a_1 + c_2 x_2/a_2)\} \right], \end{aligned} \quad (17.67)$$

where $\bar{\xi}$ is the unit normal to the surface of the flat ellipsoid.

* 18. Formulae for exterior points

When point x is outside an ellipsoidal inclusion (point y is outside S^2), the boundary terms produced by the integration by parts with respect to z in (16.14) and (16.16) are not necessarily zero.

Since

$$\begin{aligned} \delta'(\xi \bar{\xi} \cdot y - \xi z) &= -\xi^{-1} \frac{\partial}{\partial z} \delta(\xi \bar{\xi} \cdot y - \xi z), \\ \delta''(\xi \bar{\xi} \cdot y - \xi z) &= \xi^{-2} \frac{\partial^2}{\partial z^2} \delta(\xi \bar{\xi} \cdot y - \xi z), \end{aligned} \quad (18.1)$$

(16.14) with (17.10) can be written as

$$\begin{aligned} u_i(x) &= (8\pi^2)^{-1} \int_{-1}^1 dz \int_0^{2\pi} d\phi \int_0^R r dr \int_{S^2} C_{klmn} \epsilon_{nm}^*(x') N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \xi^2 \\ &\quad \times \frac{\partial}{\partial z} \delta(\xi \bar{\xi} \cdot y - \xi z) dS(\bar{\xi}). \end{aligned} \quad (18.2)$$

Integration by parts with respect to z leads to

$$\begin{aligned}
u_i(x) = & \lim_{z \rightarrow 1} (8\pi^2)^{-1} \int_0^{2\pi} d\phi \int_0^R r dr \\
& \times \int_{S^2} C_{klmn} \epsilon_{nm}^*(x') N_{ik} D^{-1} \bar{\xi}_l \xi^2 \delta(\xi \bar{\xi} \cdot y - \xi z) dS(\bar{\xi}) \\
& - \lim_{z \rightarrow -1} (8\pi^2)^{-1} \int_0^{2\pi} d\phi \int_0^R r dr \int_{S^2} C_{klmn} \epsilon_{nm}^*(x') \\
& \times N_{ik} D^{-1} \bar{\xi}_l \xi^2 \delta(\xi \bar{\xi} \cdot y - \xi z) dS(\bar{\xi}) \\
& - (8\pi^2)^{-1} \int_{-1}^1 dz \int_0^{2\pi} d\phi \int_0^R r dr \int_{S^2} C_{klmn} \frac{\partial}{\partial z} \epsilon_{nm}^*(x') \\
& \times N_{ik} D^{-1} \bar{\xi}_l \xi^2 \delta(\xi \bar{\xi} \cdot y - \xi z) dS(\bar{\xi}) \\
& + (8\pi^2)^{-1} \int_{-1}^1 dz \int_0^{2\pi} d\phi \int_{S^2} C_{klmn} z \{ \epsilon_{nm}^*(x') \}_{r=R} \\
& \times N_{ik} D^{-1} \bar{\xi}_l \xi^2 \delta(\xi \bar{\xi} \cdot y - \xi z) dS(\bar{\xi}). \tag{18.3}
\end{aligned}$$

The first two integrals in (18.3) become zero, since $R \rightarrow 0$ for $z \rightarrow \pm 1$ and $\int_{S^2} \delta(\xi \bar{\xi} \cdot y - \xi z) dS(\bar{\xi})$ is finite. The integration with respect to z in the remaining integrals is performed explicitly by the use of the property of Dirac's delta function,

$$\int_{-1}^1 z \delta(\xi \bar{\xi} \cdot y - \xi z) dz = 1, \quad \int_{-1}^1 z \delta(\xi \bar{\xi} \cdot y - \xi z) dz = \bar{\xi} \cdot y, \tag{18.4}$$

if $|\bar{\xi} \cdot y| < 1$; otherwise, they are zero. Therefore, (18.3) becomes

$$\begin{aligned}
u_i(x) = & - (8\pi^2)^{-1} \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial}{\partial z} \epsilon_{nm}^*(x') - (\bar{\xi} \cdot y) \{ \epsilon_{nm}^*(x') \}_{r=R} \right]_{z=\bar{\xi} \cdot y} \\
& \times C_{klmn} \int_{S^2} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \xi^2 dS(\bar{\xi}), \tag{18.5}
\end{aligned}$$

where S^* is a subdomain of S^2 satisfying the condition

$$|\bar{\xi} \cdot \mathbf{y}| < 1. \quad (18.5.1)$$

S^* is shown in Fig. 16.2 by the unshaded surface on S^2 . The notations defined in (16.10) and Fig. 16.1 are used in (18.5), and $R = (1 - z^2)^{1/2}$.

Similarly, after repeating the integration by parts with respect to z , Mura and Cheng (1977) rewrote (16.16) as

$$\begin{aligned} u_{i,j}(\mathbf{x}) = & - (8\pi^2)^{-1} \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial^2}{\partial z^2} \epsilon_{mn}^*(\mathbf{x}') - z \left\{ \frac{\partial}{\partial z} \epsilon_{mn}^*(\mathbf{x}') \right\}_{r=R} \right. \\ & \left. - z \frac{\partial}{\partial z} \left\{ \epsilon_{mn}^*(\mathbf{x}') \right\}_{r=R} - \left\{ \epsilon_{mn}^*(\mathbf{x}') \right\}_{r=R} \right]_{z=\bar{\xi} \cdot \mathbf{y}} \\ & \times \int_{S^*} C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \bar{\xi}_j dS(\bar{\xi}) - (2\pi)^{-1} \\ & \times \oint_{L^+} \left\{ \epsilon_{mn}^*(\mathbf{x}') \right\}_{z=\bar{\xi} \cdot \mathbf{y}=1} y^{-1} C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \bar{\xi}_j d\theta(\bar{\xi}). \quad (18.6) \end{aligned}$$

The line integrals in (18.6) are defined on L^+ in Fig. 18.1 where $\bar{\xi} \cdot \mathbf{y} = 1$. The surface integrals on S^* in (18.6) can be obtained directly from (18.5) by differentiation with respect to x_j , $\partial/\partial x_j = \bar{\xi}_j \bar{\xi}^{-1} \partial/\partial z$ from (16.12).

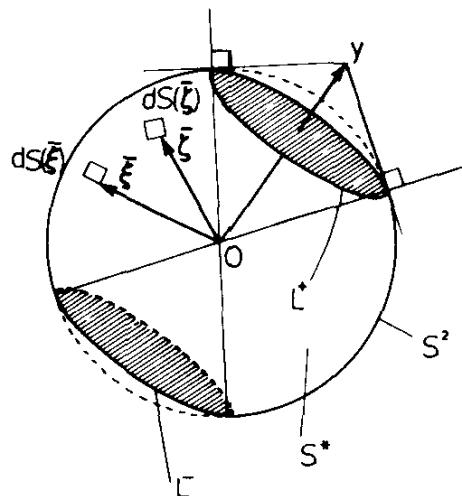


Fig. 18.1 L^+ is the boundary of S^* in the side of \mathbf{y}

The above result can be derived directly by differentiating (18.5) with respect to x_j . Since $R_{,j} = -(\xi \cdot y)\bar{\xi}_j/R\xi$ and $z_{,j} = \bar{\xi}_j/\xi$ at $z = \bar{\xi} \cdot y$, the differentiation gives

$$\begin{aligned}
 u_{i,j}(x) = & -(8\pi^2)^{-1} \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial^2}{\partial z^2} \epsilon_{nm}^*(x') - (\bar{\xi} \cdot y) \left\{ \frac{\partial}{\partial z} \epsilon_{nm}^*(x') \right\}_{r=R} \right. \\
 & \left. - (\bar{\xi} \cdot y) \frac{\partial}{\partial z} \{ \epsilon_{nm}^*(x') \}_{r=R} - \epsilon_{nm}^*(x') \right]_{z=\bar{\xi} \cdot y} \\
 & \times C_{klmn} \int_{S^*} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \bar{\xi}_j dS(\bar{\xi}) \\
 & - (8\pi^2)^{-1} \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial}{\partial z} \epsilon_{nm}^*(x') - (\bar{\xi} \cdot y) \{ \epsilon_{nm}^*(x') \}_{r=R} \right]_{z=\bar{\xi} \cdot y} \\
 & \times C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \xi_j \frac{\partial S}{\partial x_j}. \tag{18.6.1}
 \end{aligned}$$

The last term comes from the change in S^* when x changes by δx . It is clear that this term is considered only along L^+ and L^- , where L^- is the circle

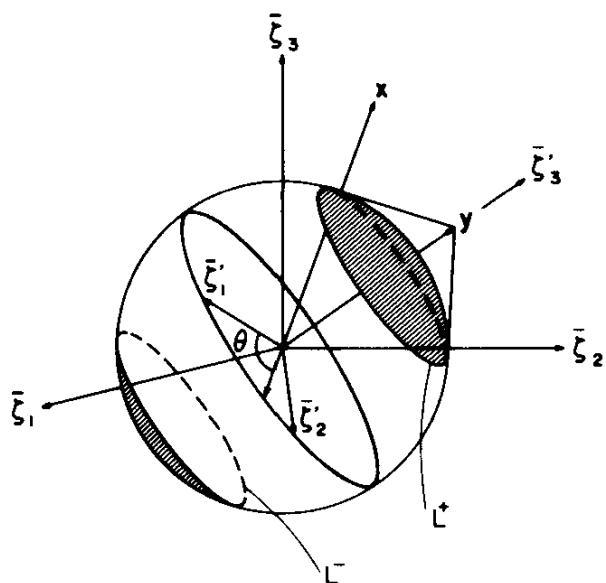


Fig. 18.2 New coordinate system $\bar{\xi}'$

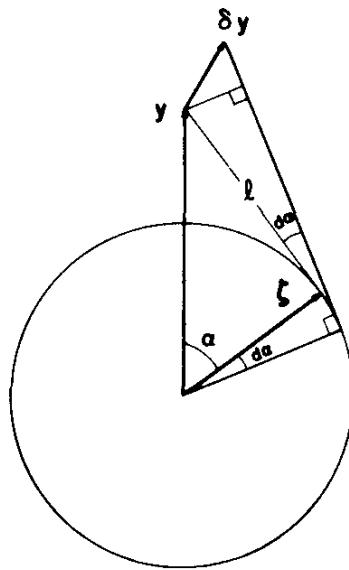


Fig. 18.3 Vectors in (18.6.3)

$\bar{\xi} \cdot y = -1$. From Figs. 18.2 and 18.3,

$$\frac{\partial S}{\partial x_j} = -\frac{\bar{\xi}_j}{\xi y} \oint_{L^+} d\theta \quad (18.6.2)$$

along L^+ . To understand this equation we note, in view of Fig. 18.3,

$$\begin{aligned} dS &= -\sin \alpha \, d\theta \, d\alpha, \\ \sin \alpha &= l/y, \\ l \, d\alpha &= \delta y \cdot \bar{\xi}, \\ \delta y \cdot \bar{\xi} &= (\delta x \cdot \bar{\xi})/\xi. \end{aligned} \quad (18.6.3)$$

The last relation is obtained from (16.10). Similarly, $\partial S / \partial x_j$ along L^- is given by

$$\frac{\partial S}{\partial x_j} = \frac{\bar{\xi}_j}{\xi y} \oint_{L^-} d\theta. \quad (18.6.4)$$

Thus, (18.6.1) is written as

$$\begin{aligned} u_{i,j}(x) &= -(8\pi^2)^{-1} \int_0^{2\pi} d\phi \left[\int_0^R r \, dr \frac{\partial^2}{\partial z^2} \epsilon_{nm}^*(x') - (\bar{\xi} \cdot y) \left\{ \frac{\partial}{\partial z} \epsilon_{nm}^*(x') \right\}_{r=R} \right. \\ &\quad \left. - (\bar{\xi} \cdot y) \frac{\partial}{\partial z} \left\{ \epsilon_{nm}^*(x') \right\}_{r=R} - \left\{ \epsilon_{nm}^*(x') \right\}_{r=R} \right]_{z=\bar{\xi} y} \end{aligned}$$

$$\begin{aligned}
& \times C_{klmn} \int_{S^*} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \bar{\xi}_j dS(\bar{\xi}) \\
& + (8\pi^2)^{-1} \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial}{\partial z} \epsilon_{nm}^*(x') \right. \\
& \quad \left. - (\bar{\xi} \cdot y) \{ \epsilon_{nm}^*(x') \}_{r=R} \right]_{z=\bar{\xi}, y=1} \\
& \times \oint_{L^+} d\theta(\bar{\xi}) C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \bar{\xi}_j y^{-1} \\
& - (8\pi^2)^{-1} \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial}{\partial z} \epsilon_{nm}^*(x') \right. \\
& \quad \left. - (\bar{\xi} \cdot y) \{ \epsilon_{nm}^*(x') \}_{r=R} \right]_{z=\bar{\xi}, y=-1} \\
& \times \oint_{L^-} d\theta(\bar{\xi}) C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \bar{\xi}_j y^{-1}, \tag{18.6.5}
\end{aligned}$$

when $z \rightarrow \pm 1$, $R \rightarrow 0$ and $\oint_0^{2\pi} d\phi \rightarrow 2\pi$. Furthermore, note from (17.49) that $[\epsilon_{nm}^*(x')]_{z=-1}$ for $[\bar{\xi}]_{L^-}$ on L^- is equal to $[\epsilon_{nm}^*(x')]_{z=1}$ for $[\bar{\xi}]_{L^+}$ ($= -[\bar{\xi}]_{L^-}$) on L^+ , and $N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \bar{\xi}_j$ is an even function of $\bar{\xi}$. Thus, one obtains

$$\begin{aligned}
u_{i,j}(x) & = - (8\pi^2)^{-1} \int_0^{2\pi} d\phi \left[\int_0^R r dr \frac{\partial^2}{\partial z^2} \epsilon_{nm}^*(x') - (\bar{\xi} \cdot y) \left\{ \frac{\partial}{\partial z} \epsilon_{nm}^*(x') \right\}_{r=R} \right. \\
& \quad \left. - (\bar{\xi} \cdot y) \frac{\partial}{\partial z} \{ \epsilon_{nm}^*(x') \}_{r=R} - \epsilon_{nm}^*(x') \right]_{z=\bar{\xi}, y} \\
& \times C_{klmn} \int_{S^*} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \bar{\xi}_j dS(\bar{\xi}) \\
& - (2\pi)^{-1} \oint_{L^+} \{ \epsilon_{nm}^*(x') \}_{z=\bar{\xi}, y=1} C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \bar{\xi}_j y^{-1} d\theta(\bar{\xi}),
\end{aligned}$$

which is identical to (18.6).

If point x is inside the inclusion, the line integrals in (18.6) can be dropped, and S^* in the surface integrals is replaced by S^2 . The resulting equation is identical to (17.8).

We note that the stress field has a discontinuity across the boundary of the inclusion. When x approaches the inclusion boundary from exterior points, the integral domain S^* in (18.6) becomes S^2 , and $\bar{\xi}$, satisfying the condition $\bar{\xi} \cdot y = 1$, approaches y ; that is, $a_1 \bar{\xi}_1 / \bar{\xi} \rightarrow x_1/a_1$, $a_2 \bar{\xi}_2 / \bar{\xi} \rightarrow x_2/a_2$, and $a_3 \bar{\xi}_3 / \bar{\xi} \rightarrow x_3/a_3$. It is easily shown that $\bar{\xi}_i \rightarrow n_i$, where n_i is the outward normal on the inclusion boundary. The integrals along L^+ in (18.6) are reduced to $-C_{klmn} \epsilon_{nm}^*(x) N_{ik}(n) D^{-1}(n) n_j n_l$. Thus, the stress jump on the inclusion boundary is

$$\begin{aligned} [\sigma_{ij}] &= \sigma_{ij}(\text{out}) - \sigma_{ij}(\text{in}) \\ &= C_{ijkl} \left\{ -C_{pqmn} \epsilon_{nm}^*(x) N_{kp}(n) D^{-1}(n) n_q n_l + \epsilon_{kl}^*(x) \right\} \end{aligned} \quad (18.7)$$

which agrees with (6.11).

The stress concentration of an inclusion or inhomogeneity is obtained from $\sigma_{ij}(\text{out})$ which can be evaluated from (18.7) when $\sigma_{ij}(\text{in})$ and $[\sigma_{ij}]$ are known (see Laws 1977). The stress concentration factor of a lens-shaped void and its relation to the stress intensity factor of a crack ($a_3 \rightarrow 0$) have been discussed by Mura and Cheng (1977). The dynamic stress concentration factor of a spherical cavity was investigated by Moon and Pao (1967).

The integrals on S^* in (18.6) can be further modified as follows: A new coordinate system $\bar{\xi}'$ is introduced as shown in Fig. 18.2, where the $\bar{\xi}'_3$ -axis is taken in the direction of y , and the $\bar{\xi}'_1$ - and $\bar{\xi}'_2$ -axes are lying on the plane perpendicular to y . Then we have

$$dS(\bar{\xi}) = d\bar{\xi}'_3 d\theta, \quad (18.8)$$

where θ is measured counter-clockwise from the $\bar{\xi}'_1$ -axis, and, therefore,

$$\int_{S^*} [] dS(\bar{\xi}) = \int_{-1/y}^{1/y} d\bar{\xi}'_3 \int_0^{2\pi} [] d\theta, \quad (18.9)$$

since S^* is determined from $\bar{\xi} \cdot y = \bar{\xi}'_3 y = \pm 1$.

The integrands in (18.6) are functions of $\bar{\xi}$ and therefore must be expressed in terms of $\bar{\xi}'_3$ and θ . The necessary transformation is as follows (see (16.10)):

$$\bar{\xi}_1 = \bar{\xi}'_1 \bar{\xi} / a_1, \quad \bar{\xi}_2 = \bar{\xi}'_2 \bar{\xi} / a_2, \quad \bar{\xi}_3 = \bar{\xi}'_3 \bar{\xi} / a_3, \quad \bar{\xi} \cdot y = \bar{\xi} \cdot x \bar{\xi}^{-1},$$

$$\bar{\xi}_i = a_{ij} \bar{\xi}'_j, \quad \bar{\xi} = (\bar{\xi}_i \bar{\xi}_i)^{1/2} = (a_1^2 \bar{\xi}_1^2 + a_2^2 \bar{\xi}_2^2 + a_3^2 \bar{\xi}_3^2)^{1/2},$$

$$\bar{\xi}'_1 = (1 - \bar{\xi}'_3^2)^{1/2} \cos \theta, \quad (18.10)$$

$$\bar{\xi}'_2 = (1 - \bar{\xi}'_3^2)^{1/2} \sin \theta,$$

$$\bar{\xi}'_3 = \bar{\xi}'_3,$$

where a_{ij} is the direction cosine between the $\bar{\xi}_i$ - and $\bar{\xi}_j$ -axes.

* Examples

It will now be shown that (18.5) and (18.6) lead to well-known solutions. Let us consider a sphere with radius a and with a uniform dilatational distribution of the eigenstrain,

$$\epsilon_{ij}^* = \delta_{ij} \epsilon^*. \quad (18.11)$$

Consider (18.5) with (18.9) at point $(x_1, 0, 0)$. Then, the $\bar{\xi}'_3$ -axis coincides with the $\bar{\xi}_1$ -axis, and $\bar{\xi}_i = \bar{\xi}_i$. Equation (18.5) becomes

$$u_1(x_1, 0, 0) = \frac{x_1 \epsilon^*}{4\pi} C_{klmn} \int_0^{2\pi} d\theta \int_{-a/x_1}^{a/x_1} G_{1kl}(\bar{\xi}) d\bar{\xi}_1, \quad (18.12)$$

where $G_{ijkl}(\bar{\xi}) = N_{ij}(\bar{\xi}) \bar{\xi}_k \bar{\xi}_l D^{-1}(\bar{\xi})$.

For an isotropic material, we have

$$u_1(x_1, 0, 0) = \frac{x_1 \epsilon^*}{4\pi} \frac{3\lambda + 2\mu}{\lambda + 2\mu} \int_0^{2\pi} d\theta \int_{-a/x_1}^{a/x_1} \bar{\xi}_1 d\bar{\xi}_1 = \frac{a^3 \epsilon^*}{3x_1^2} \frac{1+\nu}{1-\nu}. \quad (18.13)$$

Because of the symmetry of the problem, the above result can be written as $u = a^3 \epsilon^* (1+\nu) / 3r^2(1-\nu)$, where u is the displacement in the radial direction. Then $\epsilon_r = \partial u / \partial r = -2a^3 \epsilon^* (1+\nu) / 3r^3(1-\nu)$, which agrees with (11.45).

Next, consider the case when a centrally symmetric eigenstrain is prescribed in a sphere,

$$\epsilon_{ij}^*(x) = \delta_{ij} \epsilon^* |x|. \quad (18.14)$$

Then, the use of the notations in Fig. 16.1 and (16.10) leads to

$$\begin{aligned}\epsilon_{ij}^*(x') &= \delta_{ij} \epsilon^* a (z^2 + r^2)^{1/2}, \\ \left[\int_0^R r \frac{\partial}{\partial z} \epsilon_{nm}^*(x') dr \right]_{z=\xi y} &= a \delta_{nm} \epsilon^* (\bar{\xi} \cdot y) (1 - |\bar{\xi} \cdot y|).\end{aligned}\quad (18.15)$$

Consider point $(x_1, 0, 0)$. Equation (18.5) yields, for an isotropic medium,

$$\begin{aligned}u_1(x_1, 0, 0) &= \frac{x_1 \epsilon^*}{4\pi} \int_0^{2\pi} d\theta \int_{-a/x_1}^{a/x_1} \frac{1+\nu}{1-\nu} \bar{\xi}_1^2 |x_1 \cdot \bar{\xi}_1| d\bar{\xi}_1 \\ &= \frac{a^4 \epsilon^*}{4x_1^2} \frac{1+\nu}{1-\nu}\end{aligned}\quad (18.16)$$

or

$$u = \frac{a^4 \epsilon^*}{4r^2} \frac{1+\nu}{1-\nu} \quad (18.17)$$

which is identical to u obtained from (11.43) by taking $\alpha T = \epsilon^* r$. Equation (18.6) becomes

$$\begin{aligned}u_{1,1}(x_1, 0, 0) &= \frac{a \epsilon^*}{4\pi} \frac{1+\nu}{1-\nu} \left\{ \int_0^{2\pi} d\theta \int_{-a/x_1}^{a/x_1} 2 |\bar{\xi} \cdot y| \bar{\xi}_1^2 d\bar{\xi}_1 - 2 \right. \\ &\quad \left. \times \int_{L^+} [\bar{\xi}_1^2]_{\bar{\xi}_1=a/x_1} y^{-1} d\theta \right\} = -\frac{1}{2} \frac{1+\nu}{1-\nu} \frac{a^4 \epsilon^*}{x_1^3}\end{aligned}\quad (18.18)$$

or

$$\frac{\partial u}{\partial r} = -\frac{1}{2} \frac{1+\nu}{1-\nu} \frac{a^4 \epsilon^*}{r^3}. \quad (18.19)$$

It can also be shown that (18.6) leads to (11.33) when ϵ_{ij}^* is uniform and the material is isotropic. This result agrees with Eshelby's solution (1959).

In cases where analytical results are obtained from (18.5), equation (18.6) does not necessarily have to be used for $u_{i,j}$, as seen in the considered examples. However, (18.6) is necessary for numerical calculations when no analytical expression is available from (18.5).

19. Ellipsoidal inclusions with polynomial eigenstrains in anisotropic media

We consider the case when eigenstrains inside the inclusion are given in the following polynomial form of x' or y' :

$$\epsilon_{nm}^*(x') = \epsilon_{nm}(y'_1)^{\alpha_1}(y'_2)^{\alpha_2}(y'_3)^{\alpha_3}, \quad (19.1)$$

where ϵ_{nm} are constants and α_1 , α_2 and α_3 are integers. As shown in Fig. 16.1, we can write

$$y'_p = z\bar{\xi}_p + r(m_p \cos \phi + n_p \sin \phi), \quad (19.2)$$

where m , n , $\bar{\xi}$ are orthogonal unit vectors. The binomial theorem leads to

$$(y'_1)^{\alpha_1} = \sum_{\beta_1=0}^{\alpha_1} \frac{\alpha_1!}{\beta_1!(\alpha_1 - \beta_1)!} (z\bar{\xi}_1)^{\alpha_1 - \beta_1} r^{\beta_1} (m_1 \cos \phi + n_1 \sin \phi)^{\beta_1}. \quad (19.3)$$

The integrals with respect to ϕ in (18.5) have the form

$$\begin{aligned} & \int_0^{2\pi} (m_1 \cos \phi + n_1 \sin \phi)^{\beta_1} (m_2 \cos \phi + n_2 \sin \phi)^{\beta_2} \\ & \times (m_3 \cos \phi + n_3 \sin \phi)^{\beta_3} d\phi. \end{aligned} \quad (19.4)$$

A complex number $Z = \exp(i\phi)$ is introduced here. By defining

$$W_p = \frac{1}{2}(m_p + in_p), \quad \bar{W}_p = \frac{1}{2}(m_p - in_p), \quad (19.5)$$

where $i = \sqrt{-1}$, we can write (19.4) as

$$\begin{aligned} & \oint_{|Z|=1} \sum_{q_1=0}^{\beta_1} \frac{\beta_1!}{q_1!(\beta_1 - q_1)!} \bar{W}_1^{\beta_1 - q_1} W_1^{q_1} Z^{\beta_1 - 2q_1} \\ & \times \sum_{q_2=0}^{\beta_2} \frac{\beta_2!}{q_2!(\beta_2 - q_2)!} \bar{W}_2^{\beta_2 - q_2} W_2^{q_2} Z^{\beta_2 - 2q_2} \\ & \times \sum_{q_3=0}^{\beta_3} \frac{\beta_3!}{q_3!(\beta_3 - q_3)!} \bar{W}_3^{\beta_3 - q_3} W_3^{q_3} Z^{\beta_3 - 2q_3} \frac{dZ}{iZ}, \end{aligned} \quad (19.6)$$

where $|Z| = 1$ is a unit circle in the complex plane. The residue calculation is applied. The constant terms in the product of the power series of Z give the residues. Then, (19.6) is written as

$$2\pi \sum_{q_1+q_2+q_3=(\beta_1+\beta_2+\beta_3)/2}^{\beta_1 \beta_2 \beta_3} \frac{\beta_1!}{q_1!(\beta_1-q_1)!} \frac{\beta_2!}{q_2!(\beta_2-q_2)!} \frac{\beta_3!}{q_3!(\beta_3-q_3)!} \\ \times \bar{W}_1^{\beta_1-q_1} \bar{W}_2^{\beta_2-q_2} \bar{W}_3^{\beta_3-q_3} W_1^{q_1} W_2^{q_2} W_3^{q_3}. \quad (19.7)$$

The integral (19.6) becomes zero when $\frac{1}{2}(\beta_1 + \beta_2 + \beta_3)$ is not an integer; that is, (19.6) reduces to (19.7) when $\beta_1 + \beta_2 + \beta_3$ is even; otherwise it is zero.

Therefore equation (18.5), after integration with respect to r , becomes

$$u_i(x) = -(4\pi)^{-1} \int_{S^*} C_{klmn} \epsilon_{nm} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_i \zeta \alpha_1! \alpha_2! \alpha_3! \\ \times \sum_{\beta_1=0}^{\alpha_1} \sum_{\beta_2=0}^{\alpha_2} \sum_{\beta_3=0}^{\alpha_3} \left\{ (\bar{\xi} \cdot y)^{\alpha-\beta-1} (1 - (\bar{\xi} \cdot y)^2)^{(\beta+2)/2} \frac{\alpha-\beta}{\beta+2} \right. \\ \left. - (\bar{\xi} \cdot y)^{\alpha-\beta+1} (1 - (\bar{\xi} \cdot y)^2)^{\beta/2} \right\} \\ \times \frac{\bar{\xi}_1^{\alpha_1-\beta_1}}{(\alpha_1-\beta_1)!} \frac{\bar{\xi}_2^{\alpha_2-\beta_2}}{(\alpha_2-\beta_2)!} \frac{\bar{\xi}_3^{\alpha_3-\beta_3}}{(\alpha_3-\beta_3)!} \\ \times \sum_{q_1+q_2+q_3=\beta/2} \frac{\bar{W}_1^{\beta_1-q_1} W_1^{q_1}}{q_1!(\beta_1-q_1)!} \frac{\bar{W}_2^{\beta_2-q_2} W_2^{q_2}}{q_2!(\beta_2-q_2)!} \frac{\bar{W}_3^{\beta_3-q_3} W_3^{q_3}}{q_3!(\beta_3-q_3)!} dS(\bar{\xi}) \quad (19.8)$$

where $\alpha = \alpha_1 + \alpha_2 + \alpha_3$, $\beta = \beta_1 + \beta_2 + \beta_3$ and the summation with respect to β_1 , β_2 , β_3 is taken only when β is even. The x dependency of u_i can be seen from terms containing $(\bar{\xi} \cdot y) = (\bar{\xi} \cdot x)/\zeta$. If α equals N , then u_i is the sum of the polynomials of degrees $N+1$, $N-1$, $N-2, \dots$, for interior points, since $S^* = S^2$ for these points.

The displacement gradient can be obtained as the sum of the direct derivative of the integrand of (19.8) and the line integral on L^+ , as seen in (18.6), where

$$\{\epsilon_{nm}^*(x')\}_{z=1} = \epsilon_{nm} \bar{\xi}_1^{\alpha_1} \bar{\xi}_2^{\alpha_2} \bar{\xi}_3^{\alpha_3}, \quad (19.9)$$

since in Fig. 16.1 $y' \rightarrow \bar{\xi}$ when $z \rightarrow 1$. Equation (19.8) has been given by Mura and Kinoshita (1978).

Asaro and Barnett (1975) have obtained essentially the same expression.

The elastic field for exterior points of the inclusion becomes important when the interaction between two or more inclusions is considered. An approximation method has been proposed by Moschovidis and Mura (1975) for two isotropic inhomogeneous inclusions. The elastic field in the second inclusion caused by the first inclusion is approximated by its Taylor's expansion at the center of the second inclusion. The sum of the two elastic fields caused by individual inclusions can satisfy the required conditions for the equivalent inclusion problem at the second inclusion. A similar procedure is applied for points located in the first inclusion.

Special cases

When a given eigenstrain is linear,

$$\epsilon_{nm}^*(x') = \epsilon_{nm}^p x'_p / a_p = \epsilon_{nm}^p y'_p, \quad (19.10)$$

we have from (19.2)

$$\frac{\partial}{\partial z} \epsilon_{nm}^*(x') = \epsilon_{nm}^p \bar{\xi}_p, \quad (19.11)$$

and, therefore,

$$u_i(x) = -(4\pi)^{-1} \int_{S^*} \frac{1}{2} \left\{ 1 - 3(\bar{\xi} \cdot y)^2 \right\} \epsilon_{nm}^p \bar{\xi}_p C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \xi \, dS(\bar{\xi}). \quad (19.12)$$

When

$$\epsilon_{nm}^*(x') = \epsilon_{nm}^p (y'_p)^2, \quad (19.13)$$

we have, similarly,

$$u_i(x) = (4\pi)^{-1} \int_{S^*} \left[-(\bar{\xi} \cdot y) \bar{\xi}_p^2 + 2(\bar{\xi} \cdot y)^3 \bar{\xi}_p^2 + \{(\bar{\xi} \cdot y) - (\bar{\xi} \cdot y)^3\} \frac{1}{2} (1 - \bar{\xi}_p^2) \right] \times \epsilon_{nm}^p C_{klmn} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l \xi \, dS(\bar{\xi}). \quad (19.14)$$

When the ellipsoid degenerates into a flat ellipsoid ($a_3 \rightarrow 0$), an argument similar to (17.36) can be employed. For interior points we have, from (19.12),

$$u_{i,j}(x) = (4\pi)^{-1} C_{klmn} \epsilon_{nm}^p \int_{S^2} 3(\bar{\xi} \cdot y) N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_j \bar{\xi}_l \bar{\xi}_p dS(\bar{\xi}), \quad (19.15)$$

where $\bar{\xi}_1 = \xi \bar{\xi}_1 / a_1$, $\bar{\xi}_2 = \xi \bar{\xi}_2 / a_2$, $\bar{\xi}_3 = \xi \bar{\xi}_3 / a_3$, and $y_1 = x_1 / a_1$, $y_2 = x_2 / a_2$, $y_3 = x_3 / a_3$. $G_{ikjl}(\bar{\xi}_1, \bar{\xi}_2, \bar{\xi}_3) \equiv N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_j \bar{\xi}_l$ is a homogeneous function of $\bar{\xi}$ of degree zero and independent of the geometry of the ellipsoid. When $a_3 \rightarrow 0$, $\bar{\xi}_1, \bar{\xi}_2 \ll \bar{\xi}_3$, it can be written as $G_{ikjl}(0, 0, 1)$. The integral in (19.15), therefore, can be reduced to the integral

$$\int_{S^2} (\bar{\xi} \cdot y) \bar{\xi}_p dS(\bar{\xi}) = \int_{S^2} (\bar{\xi} \cdot y) n_p dS(\bar{\xi}) = \int_V y_p dV = (4\pi/3) y_p, \quad (19.16)$$

where V is the volume of S^2 and n_p is the normal vector on S^2 . Finally, (19.15) is written as

$$u_{i,j}(x) = C_{klmn} \epsilon_{nm}^p y_p G_{ikjl}(0, 0, 1). \quad (19.17)$$

Similarly, we have, from (19.14),

$$u_{i,j}(x) = C_{klmn} \epsilon_{mn}^p y_p^2 G_{ikjl}(0, 0, 1), \quad (19.18)$$

which is the displacement gradient inside a flat ellipsoid when an eigenstrain is given by (19.13).

* 20. Harmonic eigenstrains

Let us consider the case where the eigenstrain component ϵ_{ij}^* is a solid harmonic function of y in an ellipsoidal inclusion Ω ,

$$\epsilon_{ij}^*(x) = \bar{\epsilon}_{ij}^*(\omega) y^n P_n(y \cdot \omega / y), \quad (20.1)$$

where $y_1 = x_1 / a_1$, $y_2 = x_2 / a_2$, $y_3 = x_3 / a_3$, and $y = (y_i y_i)^{1/2}$; ω is an arbitrary vector on the unit sphere S^2 ; and P_n is the Legendre polynomial of degree n . It is known that any function $f(y)$ of class C^1 defined on S^2 can be expanded in a uniformly convergent series,

$$f(y) = \sum_{n=0}^{\infty} \frac{2n+1}{4\pi} \int_{S^2} f(\omega) P_n(y \cdot \omega) dS(\omega) \quad (20.2)$$

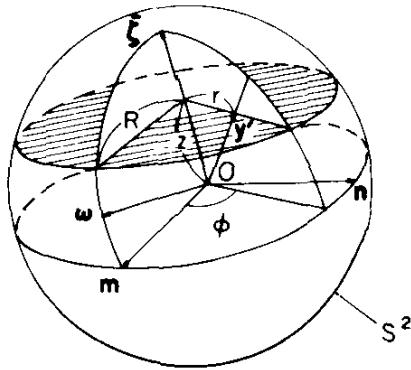


Fig. 20.1. Vectors in (20.3).

(see e.g., Hobson 1931). Therefore, the eigenstrain given by (20.1) may have a broad application, particularly in an interaction problem of two ellipsoidal inhomogeneities subjected to an applied stress. Note that (20.1) is not a solid harmonic function of \mathbf{x} .

When (20.1) is substituted into (16.14), we have

$$u_i(\mathbf{x}) = -\frac{a_1 a_2 a_3}{8\pi^2} \int_{-1}^1 dz \int_0^{2\pi} d\phi \int_0^R r dr \int_{S^2} C_{klpq} \bar{\epsilon}_{pq}^*(\omega) N_{ik}(\xi) \\ \times D^{-1}(\xi) \bar{\xi}_l(y')^n P_n(y' \cdot \omega/y') \delta'(\xi \bar{\xi} \cdot y - \xi z) dS(\xi), \quad (20.3)$$

where $y'_i = x'_i/a_i$, $y' = (y'_i y'_i)^{1/2}$, $\bar{\xi}_1 = a_1 \bar{\xi}_1/\xi$, $\xi = (a_1^2 \bar{\xi}_1^2 + a_2^2 \bar{\xi}_2^2 + a_3^2 \bar{\xi}_3^2)^{1/2}$, and z , ϕ , r and R are shown in Fig. 20.1. We have

$$y' = z\bar{\xi} + r(m \cos \phi + n \sin \phi). \quad (20.4)$$

The addition theorem of the Legendre polynomial leads to

$$P_n(y' \cdot \omega/y') = P_n(y' \cdot \bar{\xi}/y') P_n(\bar{\xi} \cdot \omega) \\ + 2 \sum_{m=1}^n \frac{(n-m)!}{(n+m)!} P_n^m(y' \cdot \bar{\xi}/y') P_n^m(\bar{\xi} \cdot \omega) \cos(m\phi). \quad (20.5)$$

Since $\int_0^{2\pi} \cos m\phi d\phi = 0$, only the first term in (20.5) is used for P_n in (20.3). Furthermore, we have

$$\int_0^{2\pi} d\phi \int_0^R r dr (y')^n P_n(y' \cdot \bar{\xi}/y') \\ = 2\pi \int_0^{\sqrt{1-z^2}} r dr (z^2 + r^2)^{n/2} P_n(z/\sqrt{z^2 + r^2}) \\ = 2\pi (1-z^2) P'_{n+1}(z)/(n+1)(n+2) \quad (20.6)$$

and

$$\begin{aligned}
& \int_{-1}^1 (1 - z^2) P'_{n+1}(z) \delta'(\xi \bar{\xi} \cdot y - \xi z) dz \\
&= \int_{-1}^1 \{(1 - z^2) P'_{n+1}(z)\}' \xi^{-1} \delta(\xi \bar{\xi} \cdot y - \xi z) dz \\
&= \int_{-1}^1 - (n+1)(n+2) P_{n+1}(z) \xi^{-1} \delta(\xi \bar{\xi} \cdot y - \xi z) dz \\
&= -(n+1)(n+2) P_{n+1}(\bar{\xi} \cdot y) \xi^{-2} \quad \text{for } |\bar{\xi} \cdot y| < 1 \\
&= 0 \quad \text{for } |\bar{\xi} \cdot y| > 1. \tag{20.7}
\end{aligned}$$

Therefore, (20.3) can be written as

$$\begin{aligned}
u_i(x) &= \frac{a_1 a_2 a_3}{4\pi} C_{klpq} \bar{\epsilon}_{pq}^*(\omega) \int_{S^*} N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l P_n(\bar{\xi} \cdot \omega) \\
&\quad \times P_{n+1}(\bar{\xi} \cdot y) \xi^{-2} dS(\bar{\xi}), \tag{20.8}
\end{aligned}$$

where S^* is the subspace of S^2 satisfying $|\bar{\xi} \cdot y| \leq 1$. For interior points of Ω , S^* is taken as S^2 . When $n = 0$, then $P_0 = 1$, $P_1 = \bar{\xi} \cdot y = \bar{\xi} \cdot x \xi^{-1}$, and (20.8) becomes identical to (17.11).

The above result, (20.8), can be further modified, since $N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l$ are homogeneous functions of degree -1 . Using (17.10), we have

$$u_i(x) = (1/4\pi) \int_{S^*} Q_{ni}(\bar{\xi}, \omega) P_{n+1}(\bar{\xi} \cdot y) dS(\bar{\xi}), \tag{20.9}$$

where

$$\begin{aligned}
Q_{ni}(\bar{\xi}, \omega) &= C_{klpq} \bar{\epsilon}_{pq}^*(\omega) N_{ik}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_l P_n(\bar{\xi} \cdot \omega), \\
\xi_1 &= \bar{\xi}_1/a_1, \quad \xi_2 = \bar{\xi}_2/a_2, \quad \xi_3 = \bar{\xi}_3/a_3. \tag{20.10}
\end{aligned}$$

Q_{ni} can be expanded in a series of surface harmonic functions,

$$Q_{ni}(\bar{\xi}, \omega) = \sum_{m=0}^{\infty} \frac{2m+1}{4\pi} \int_{S^2} Q_{ni}(\bar{\eta}, \omega) P_m(\bar{\xi} \cdot \bar{\eta}) dS(\bar{\eta}). \tag{20.11}$$

The above expression is substituted into (20.9) and the integration on S^* is performed. The addition theorem,

$$P_m(\bar{\xi} \cdot \bar{\eta}) = P_m(\bar{\xi} \cdot \bar{y}) P_m(\bar{\eta} \cdot \bar{y}) + 2 \sum_{l=1}^m \frac{(m-l)!}{(m+l)!} P_m^l(\bar{\xi} \cdot \bar{y}) P_m^l(\bar{\eta} \cdot \bar{y}) \cos(l\theta),$$

yields

$$\int_{S^*} P_m(\bar{\xi} \cdot \bar{\eta}) P_{n+1}(\bar{\xi} \cdot y) dS(\bar{\xi}) = 2\pi P_m(\bar{\eta} \cdot \bar{y}) \Phi_{nm}(y), \quad (20.12)$$

where $\bar{y} = y/y$, $y = |y|$, $dS(\bar{\xi}) = d\theta dt$, $t = \bar{\xi} \cdot \bar{y}$, and

$$\Phi_{nm}(y) = \int_{-1}^1 P_m(t) P_{n+1}(yt) dt \quad (20.13)$$

for interior points x of Ω , since $S^* = S^2$, and

$$\Phi_{nm}(y) = \int_{-1/y}^{1/y} P_m(t) P_{n+1}(yt) dt = \frac{1}{y} \int_{-1}^1 P_m(t/y) P_{n+1}(t) dt \quad (20.14)$$

for exterior points x , since S^* is determined from the condition

$$|\bar{\xi} \cdot y| = |ty| \leq 1. \quad (20.15)$$

The integrals (20.13) and (20.14) can be performed by the use of Rodrigues' formula (Whittaker and Watson, 1962),

$$P_m(t) = \frac{1}{2^m m!} \frac{d^m}{dt^m} (t^2 - 1)^m. \quad (20.16)$$

By repeated integration by parts, (20.13) becomes

$$gF_{nm}(y) = (-1)^{(n+1-m)/2} \times \sum_{l=0}^{(n+1-m)/2} \frac{(-1)^l \Gamma\left(\frac{n+m+2l+2}{2}\right)}{l! \Gamma\left(\frac{n-m-2l+3}{2}\right) \Gamma\left(\frac{2m+2l+3}{2}\right)} y^{m+2l}, \quad (20.17)$$

where $y \leq 1$, $m \leq n + 1$, and Γ is the Gamma-function. When $m > n + 1$, $\Phi_{nm} = 0$. Similarly, (20.14) becomes

$$\Phi_{nm}(y) = \sum_{l=0}^{(m-n-1)/2} \frac{(-1)^l \Gamma\left(\frac{2m-2l+1}{2}\right)}{l! \Gamma\left(\frac{m-n-2l+1}{2}\right) \Gamma\left(\frac{m+n-2l+4}{2}\right)} y^{-m-1+2l}, \quad (20.18)$$

where $y \geq 1$ and $n + 1 \leq m$. When $n + 1 > m$, $\Phi_{nm} = 0$. In addition,

$$\begin{aligned} \Phi_{nm}(1) &= 0 \quad \text{for } m \neq n + 1 \\ &= \frac{2}{2n+3} \quad \text{for } m = n + 1. \end{aligned} \quad (20.19)$$

Finally, (20.9) can be written as

$$u_i(x) = \frac{1}{2} \sum_{m=0}^{\infty} \frac{2m+1}{4\pi} \Phi_{nm}(y) \int_{S^2} Q_{ni}(\bar{\eta}, \omega) P_m(\bar{\eta} \cdot \bar{y}) dS(\bar{\eta}). \quad (20.20)$$

From the definition (20.10), terms for which $n + m$ is even, are zero. It should be noticed that (20.20) is a uniformly convergent series and the integral domain S^2 is independent of x or y . Therefore, the displacement gradient $u_{i,j}$ can be obtained directly from (20.20) by differentiation, where $\partial y/\partial x_1 = y_1/a_1 y$ and $\partial(\bar{\eta} \cdot \bar{y})/\partial x_1 = \bar{\eta}_1/a_1$, etc.

21. Periodic distribution of spherical inclusions

Let us consider a periodic distribution of spherical inclusions with a period $2L$ in all three directions in the Cartesian coordinates (Fig. 21.1(a)). The distribution of eigenstrains can be expressed in the Fourier series as

$$\epsilon_{ij}^*(x) = \sum_{\nu_1, \nu_2, \nu_3=-\infty}^{\infty} \sum' \sum' \bar{\epsilon}_{ij}^*(\xi) \exp(i\xi \cdot x), \quad (21.1)$$

where $\xi_1 = \pi\nu_1/L$, $\xi_2 = \pi\nu_2/L$, $\xi_3 = \pi\nu_3/L$ (excluding $\nu_1 = \nu_2 = \nu_3 = 0$, since the uniform eigenstrain in a whole space gives a divergent solution), and

$$\bar{\epsilon}_{ij}^*(\xi) = \frac{1}{8L^3} \iiint_{-L}^L \epsilon_{ij}^*(x) \exp(i\xi \cdot x) dx. \quad (21.2)$$

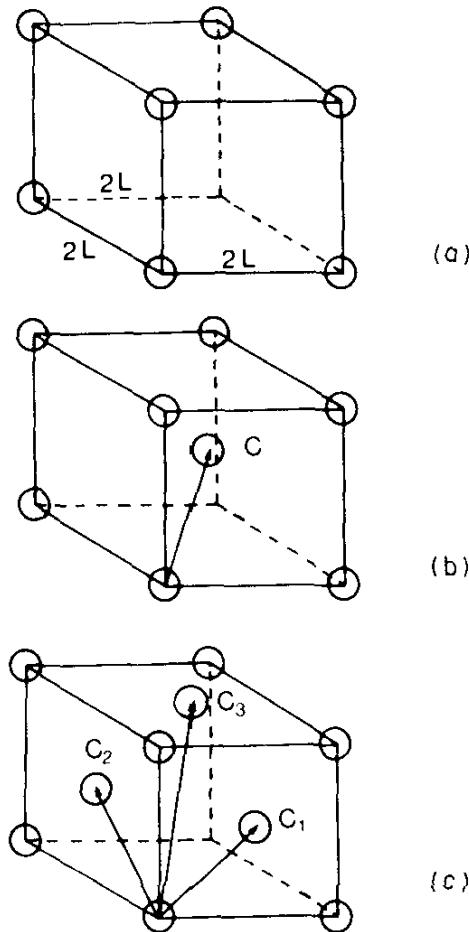


Fig. 21.1 Periodic distributions of spherical inclusions in a unit cell

Similarly, the displacement $u_i(x)$ can also be expressed in the Fourier series form

$$u_i(x) = \sum'_{\nu_1, \nu_2, \nu_3 = -\infty}^{\infty} \sum'_{\nu_1, \nu_2, \nu_3 = -\infty}^{\infty} \sum'_{\nu_1, \nu_2, \nu_3 = -\infty}^{\infty} \bar{u}_i(\xi) \exp(i\xi \cdot x). \quad (21.3)$$

According to (3.14), the solution can be written as

$$u_i(x) = -i \sum'_{\nu_1, \nu_2, \nu_3 = -\infty}^{\infty} \sum'_{\nu_1, \nu_2, \nu_3 = -\infty}^{\infty} \sum'_{\nu_1, \nu_2, \nu_3 = -\infty}^{\infty} C_{jlmn} \bar{\epsilon}_{mn}^* \xi_l N_{ij}(\xi) D^{-1}(\xi) \exp(i\xi \cdot x). \quad (21.4)$$

If $\epsilon_{ij}^*(x)$ is uniform inside the spherical domains and zero outside, then (21.2) becomes

$$\bar{\epsilon}_{ij}(\xi) = \frac{\epsilon_{ij}^*}{8L^3} \int_{\Omega} \exp(i\xi \cdot x) dx. \quad (21.5)$$

The integral has been carried out by Asaro (1975) and gives

$$\bar{\epsilon}_{ij}^* = (a/2L)^3 (2\pi)^{3/2} \epsilon_{ij}^* \eta^{-3/2} J_{3/2}(\eta), \quad (21.6)$$

where a is the radius of the sphere,

$$\eta = a\xi = a(\xi_1^2 + \xi_2^2 + \xi_3^2)^{1/2} = \frac{\pi a}{L} (\nu_1^2 + \nu_2^2 + \nu_3^2)^{1/2} = \frac{\pi a}{L} \nu, \quad (21.7)$$

and

$$J_{3/2}(\eta) = (2/\pi\eta)^{1/2} (\eta^{-1} \sin \eta - \cos \eta). \quad (21.8)$$

Finally, the displacement field $u_i(x)$ becomes

$$u_i(x) = -i \left(\frac{a}{2L} \right)^{3/2} C_{jlmn} \epsilon_{mn}^* \sum'_{\nu_1, \nu_2, \nu_3 = -\infty}^{\infty} \sum' N_{ij}(\xi) D^{-1}(\xi) \xi_l \nu^{-3/2} \\ \times J_{3/2} \left(\frac{\pi a \nu}{L} \right) \exp(i\xi \cdot x). \quad (21.9)$$

If the spherical inclusions are distributed in a body-centered cubic lattice (see Fig. 21.1(b)), the solution can be the sum of (21.9) and modified (21.9) where x is replaced by $x - c$. If the spherical inclusions are distributed in a face-centered cubic lattice (see Fig. 21.1(c)), the solution is obtained as the sum of four expressions similar to (21.9) having exponential factors $\exp(i\xi \cdot x)$, $\exp\{i\xi \cdot (x - c_1)\}$, $\exp\{i\xi \cdot (x - c_2)\}$ and $\exp\{i\xi \cdot (x - c_3)\}$. The distortions corresponding to these cases can be summarized by

$$u_{i,k}(x) = \left(\frac{a}{2L} \right)^{3/2} C_{jlmn} \epsilon_{mn}^* \sum'_{\nu_1, \nu_2, \nu_3 = -\infty}^{\infty} \sum' G_{ijkl}(\xi) \nu^{-3/2} \\ \times J_{3/2} \left(\frac{\pi a \nu}{L} \right) f(\nu) \exp(i\xi \cdot x) \quad (21.10)$$

where

$$f(\nu) = \begin{cases} 1 & \text{for simple cubic} \\ 1 + \exp(-i\xi \cdot c) & \text{for b.c.c.} \\ 1 + \exp(-i\xi \cdot c_1) + \exp(-i\xi \cdot c_2) + \exp(-i\xi \cdot c_3) & \text{for f.c.c.} \end{cases} \quad (21.11)$$

The elastic strain energy assigned to a single inclusion is obtained from (13.3) as

$$W^* = -\frac{1}{2} \int_{V_0} \sigma_{ij} \epsilon_{ij}^* dD, \quad (21.12)$$

where V_0 is the volume of a single inclusion, i.e., $V_0 = 4\pi a^3/3$. σ_{ij} in V_0 can be calculated from (21.10) and (2.4). Then, (21.12) becomes

$$\begin{aligned} W^* = & \frac{2}{3}\pi a^3 C_{ijkl} \epsilon_{kl}^* \epsilon_{ij}^* - \frac{1}{2} a^3 C_{ijkl} C_{pqmn} \epsilon_{mn}^* \epsilon_{ij}^* \\ & \times \sum'_{\nu_1, \nu_2, \nu_3 = -\infty}^{\infty} \sum' G_{kplq}(\nu) \nu^{-3} J_{3/2}^2 \left(\frac{\pi a \nu}{L} \right) F(\nu), \end{aligned} \quad (21.13)$$

where $F(\nu)$ is the real part of $f(\nu)$.

Khachaturyan (1969, 1970) and Khachaturyan and Airapetyan (1974) have studied a spatial distribution of the inclusions by a different method and concluded that the periodic distribution proves to be stable.

In some alloys, clusters or precipitates are formed in a periodic manner (Ardell, Nicholson, and Eshelby, 1966; and Eurin, Penisson, and Barnett, 1973), which is usually interpreted in terms of the interaction of two neighboring clusters or precipitates. However, the elastic strain energy in such a situation can be directly calculated from (21.13). Mori et al. (1978) have calculated the elastic strain energy for periodic arrangements of nitrogen atoms in iron. It is assumed that a nitrogen atom occupies a spherical region, V_0 , the volume of which is equal to that of an iron atom and has eigenstrains of $\epsilon_{33}^* = 0.86$ and $\epsilon_{11}^* = \epsilon_{22}^* = -0.08$ (Nowick and Berry 1975, misfitting sphere model). There are numerous ways of arranging the nitrogen atoms in periodic arrays. Mori et al. (1978) have calculated three cases: simple cubic, body-centered cubic, and face-centered cubic arrangements, all of which have symmetry identical to that of the host iron atoms. The elastic constants are assumed to be $C_{1111} = 2.37 \times 10^{11} \text{ N/m}^2$, and $C_{1122} = 1.41 \times 10^{11} \text{ N/m}^2$, $C_{1212} = 1.16 \times 10^{11} \text{ N/m}^2$. The calculated energy is plotted against $2L$ in Fig. 21.2. First, it is seen that the lowest elastic strain energy is attained when the nitrogen atoms are arranged in body-centered cubic arrays. Secondly, in the simple and face-centered cubic arrays, the elastic strain energy decreases monotonically to the asymptotic value as the period $2L$ increases. The difference between the asymptotic value and the elastic strain energy in any specified arrangement is the interaction energy among all the nitrogen atoms. Contrary to the cases of the simple cubic and face-centered cubic arrays the

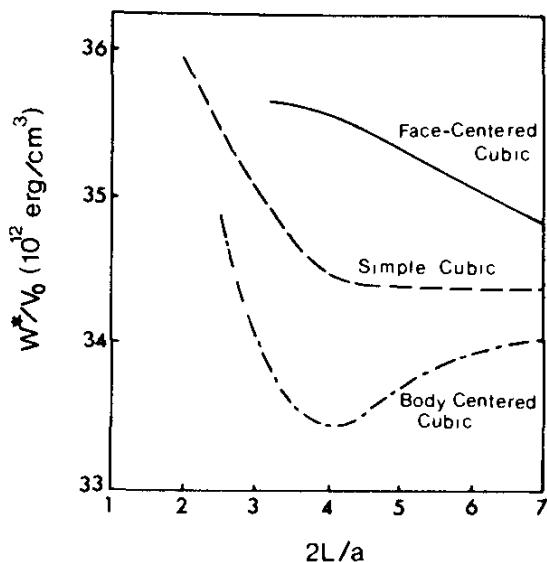


Fig. 21.2 Elastic energy per nitrogen atom

elastic strain energy in the body-centered cubic array first decreases and then increases toward the asymptotic value as the separation increases. The lowest energy state is attained when $2L/a \approx 4.0$. It is noted that this situation corresponds approximately to the arrangement of nitrogen atoms in the α'' precipitate, Fe_{16}N_2 , observed as an intermediate phase in supersaturated iron-nitrogen alloys. Although one cannot examine all the possible arrangements of the nitrogen atoms, it is clear from the above results that the formation of the α'' intermediate phase on tempering of supersaturated iron-nitrogen alloys can be understood on the basis of the elastic energy associated with the clustering of nitrogen atoms.

Instead of taking the approach described above, it is possible to use a direct method which utilizes the summation of the interaction energy between nitrogen atoms arranged in a periodic manner. Although Brown et al. (1973) reported that the summation converges very slowly, Furuhashi, Kinoshita, and Mura (1981) recently have proved that the summation of the stress fields of individual inclusions does not converge.

If a bounded inclusion Ω_p with eigenstrain ϵ_{mn}^{*p} is given in an infinite three-dimensional space D , the stress σ_{ij}^p due to the inclusion can be obtained from (6.1).

If an infinite number of inclusions Ω_p ($p = 1, 2, \dots$) are distributed in D , the stress σ_{ij} may be the sum of σ_{ij}^p ,

$$\sigma_{ij}(x) = \sum_{p=1}^{\infty} \sigma_{ij}^p(x). \quad (21.14)$$

The convergency of this series, however, is not obvious. Furuhashi, Kinoshita,

and Mura (1981) showed that the convergency depends on the dimensions of the distributions (line, plane, or three-dimensional) and the types of eigen-strains (uniform or linear function of coordinates, etc.). The summation (21.14) is replaced by a new summation when it does not converge.

Let us assume that the material is isotropic and $\epsilon_{ij}^{*p} = \delta_{ij}$ for all indices p . According to Timoshenko and Goodier (1951), σ_{ij}^p (σ_r^p or σ_t^p) has the form

$$1/\{(\bar{x}_1)^2 + (\bar{x}_2)^2 + (\bar{x}_3)^2\}^{3/2} \quad (21.15)$$

for exterior points of Ω_p , where \bar{x}_1 , \bar{x}_2 , \bar{x}_3 are the Cartesian coordinates measured from the center of spherical inclusion Ω_p .

The following Eisenstein's theorem (see Whittaker and Watson, 1962) is used:

$$\sum 1/(q_1^2 + q_2^2 + \dots + q_n^2)^s \quad (21.16)$$

in which the summation extends over all positive and negative integral values and zero values of q_1, q_2, \dots, q_n , (except the set of simultaneous zero values), is absolutely convergent if $s > \frac{1}{2}n$ and divergent if $s \leq \frac{1}{2}n$.

Suppose that the distribution of Ω_p is defined in the region $x_1 > 0$, $x_2 > 0$, $x_3 > 0$ as

$$\begin{aligned} \Omega_p = \{x; |x - 3p| < 1\}, \\ p = (p_1, p_2, p_3), p_1, p_2, p_3: \text{positive integers}. \end{aligned} \quad (21.17)$$

In order to investigate the convergency of (21.14), it is sufficient to investigate the convergency of the series

$$S = \sum_{p_1, p_2, p_3} 1/\{(x_1 - 3p_1)^2 + (x_2 - 3p_2)^2 + (x_3 - 3p_3)^2\}^{3/2}. \quad (21.18)$$

Since it is always possible for a given set of x_i and p_i to choose the smallest integers q_i such that

$$|x_i - 3p_i| \leq q_i, \quad (21.19)$$

then, for such q_i ,

$$S \geq \sum_{q_1, q_2, q_3} 1/(q_1^2 + q_2^2 + q_3^2)^{3/2}. \quad (21.20)$$

Since $n = 3$, $s = \frac{3}{2}$ in Eisenstein's theorem, the series on the right-hand side in (21.20) is divergent and, therefore, series (21.18) diverges.

Although q_1, q_2, q_3 in the series (21.20) do not take all integers used for the series (21.16), Eisenstein's theorem still holds. Consider the series $\sum_{p_i \geq n_i} 1/|p - \frac{1}{3}\mathbf{x}|^3$ instead of (21.18), where n_i are arbitrary positive integers such that $p_i - \frac{1}{3}x_i \geq 1$ for $p_i \geq n_i$, and let q_i be the smallest integers such as $p_i - \frac{1}{3}x_i \leq q_i$. Then the inequality

$$\sum_{p_i \geq n_i} 1/|p - \frac{1}{3}\mathbf{x}|^3 \geq \sum_{q_i \geq q'_i} 1/(q_1^2 + q_2^2 + q_3^2)^{3/2} \quad (21.21)$$

holds for some positive integers q'_i .

It is concluded that the resulting stress caused by a three-dimensional periodic distribution of spherical inclusions with uniform dilatational eigen-strains cannot be expressed as (21.14). The expression for the stress field must be modified, as will be shown later.

Let us consider the following plane distribution of inclusions:

$$\Omega_p = \{x; |x - 3\mathbf{p}| < 1\}, \quad (21.22)$$

$$\mathbf{p} = (p_1, p_2, 0), \quad p_1, p_2: \text{all positive and negative integers.}$$

In order to investigate the convergency of (21.14) it is sufficient to investigate the convergency of the series

$$S = \sum_{p_1, p_2} 1/\{(x_1 - 3p_1)^2 + (x_2 - 3p_2)^2 + x_3^2\}^{3/2}. \quad (21.23)$$

It is always possible to choose the largest integers q_1, q_2 for given values of x_i and p_i such that

$$|x_1 - 3p_1| \geq q_1, \quad |x_2 - 3p_2| \geq q_2. \quad (21.24)$$

Then

$$S \leq \sum_{q_1, q_2} 1/(q_1^2 + q_2^2)^{3/2}. \quad (21.25)$$

When Eisenstein's theorem is applied to the right-hand side in (21.25), the series converges because $n = 2$ and $s = \frac{3}{2}$. An argument similar to that following (21.20) can also be applied if one wants strictness of argument on the

convergency. It is concluded that for any plane or linear periodic distribution of inclusions, the resulting stress can be expressed as (21.14).

Now let us construct a suitable expression for the resulting stress caused by a three-dimensional periodic distribution of inclusions. In order to have a convergent series, we consider

$$\sum_p (\sigma_{ij}^p(x) - \sigma_{ij}^p(\mathbf{0})). \quad (21.26)$$

Since

$$|\sum_p (1/|x - p|^3 - 1/|p|^3)| \leq M \sum_p 1/|x - p|^3 |p| + C \sum_p 1/|x - p|^2 |p|^2, \quad (21.27)$$

where M and C are finite constants, the series (21.26) is convergent ($n = 3$, $s = \frac{4}{2}$). An arbitrary constant can be added to (21.26) when (21.14) is replaced by (21.26). The arbitrary constant must be determined from the condition of periodicity.

Let us define v_0 as a unit cell of the periodic distribution of inclusions. For distribution (21.17), v_0 is a hexahedron defined by $-\frac{3}{2} \leq x_i \leq \frac{3}{2}$, $i = 1, 2, 3$. The integration σ_{ij} over v_0 becomes

$$\begin{aligned} \int_{v_0} \sigma_{ij}(x) dx &= C_{ijkl} \int_{v_0} \{ u_{k,l}(x) - \epsilon_{kl}^*(x) \} dx \\ &= C_{ijkl} \int_{|v_0|} u_k(x) n_l dS - C_{ijkl} \int_{v_0} \epsilon_{kl}^*(x) dx, \end{aligned} \quad (21.28)$$

where $|v_0|$ is the boundary of v_0 and C_{ijkl} are the elastic moduli. The condition of periodicity gives

$$\int_{|v_0|} u_k(x) n_l dS = 0. \quad (21.29)$$

Then, (21.28) becomes

$$\int_{v_0} \sigma_{ij}(x) dx = -C_{ijkl} \int_{v_0} \epsilon_{kl}^*(x) dx. \quad (21.30)$$

In order to satisfy the above condition, the stress expression must have the form

$$\sigma_{ij}(x) = \sum_p (\sigma_{ij}^p(x) - \sigma_{ij}^p(0)) + \alpha_{ij}, \quad (21.31)$$

where

$$\alpha_{ij} = -(1/v_0) \left\{ C_{ijkl} \int_{v_0} \epsilon_{kl}^*(x) dx + \int_{v_0} \sum_p (\sigma_{ij}^p(x) - \sigma_{ij}^p(0)) dx \right\}. \quad (21.32)$$

Expression (21.14) can be recovered from (21.32) when the summation (21.14) is convergent. It can also be observed that expression (21.31) is independent of the choice of the origin of coordinates.

To generalize the last result for eigenstrains prescribed by polynomials of the coordinates, we start our discussion from equation (12.2). The displacement field for interior and exterior points of Ω is expressed as

$$u_i(x) = \frac{1}{8\pi(1-\nu)} \{ \Psi_{jl,jli} - 2\nu \Phi_{mm,i} - 4(1-\nu) \Phi_{il,l} \}, \quad (21.33)$$

where

$$\Psi_{ij}(x) = \int_{\Omega} |x - x'| \epsilon_{ij}^*(x') dx' \quad (21.34)$$

and

$$\Phi_{ij}(x) = \int_{\Omega} \frac{\epsilon_{ij}^*(x')}{|x - x'|} dx'. \quad (21.35)$$

$\Phi_{ij}(x)$ and $\Psi_{ij}(x)$ are the harmonic and biharmonic potentials due to a body Ω of density $\epsilon_{ij}^*(x')$; they have the properties described by (12.5) and (12.6).

Let $\epsilon_{ij}^*(x)$ be any homogeneous polynomial of degree l ,

$$\epsilon_{ij}^*(x) = B_{ijs_1 \dots s_l} x_{s_1} x_{s_2} \dots x_{s_l}. \quad (21.36)$$

Then, the structure of the elastic field can be determined by the property of the integral

$$\phi_{s_1 s_2 \dots s_l} = \int_{\Omega} \frac{x'_{s_1} x'_{s_2} \dots x'_{s_l}}{|x - x'|} dx'. \quad (21.37)$$

According to Dyson (1891), when Ω is a sphere with radius a , the integral (21.37) for exterior point x of Ω is explicitly written as

$$\phi_{s_1 s_2 \dots s_l} = \sum_{n=0}^{[l/2]} |x|^{-2(l-n)-1} \left\{ \left(\frac{\partial}{\partial x_j} \frac{\partial}{\partial x_j} \right)^n x_{s_1} x_{s_2} \dots x_{s_l} \right\}_{\alpha=0} \sum_{\alpha=0}^n C(n, \alpha) |x|^{2\alpha}, \quad (21.38)$$

where $[l/2]$ is Gauss' symbol (a maximum integer not exceeding $l/2$) and

$$C(n, \alpha) = \frac{\pi a^{l+3-2\alpha} (-1)^{\alpha+n+1}}{2^{2n} n! (n+1)!} \sum_{r=0}^{n+1} \frac{(-1)_n^r C_r}{(\alpha+n-l-r-\frac{1}{2})}. \quad (21.39)$$

The finite series (21.38) can be summarized in the form

$$\{ f_0^l(x)/|x|^l + f_1^l(x)/|x|^{l-2} + \dots + f_{(l+2)/2}^l(x) \}/|x|^{l+1} \quad (21.40)$$

if l is even, and in the form

$$\{ f_0^l(x)/|x|^{l-1} + f_1^l(x)/|x|^{l-3} + \dots + f_{(l+1)/2}^l(x) \}/|x|^{l+2} \quad (21.41)$$

if l is odd, where f_j^l are homogeneous polynomials of x of degree l for all indices j . Accordingly, the displacement expression (21.33) can be summarized in the form

$$\{ f_0^{l+3}(x)/|x|^{l+2} + f_1^{l+3}(x)/|x|^l + \dots + f_{(l+2)/2}^{l+3}(x) \}/|x|^{l+5} \quad (21.42)$$

if l is even, and in the form

$$\{ f_0^{l+3}(x)/|x|^{l+1} + f_1^{l+3}(x)/|x|^{l-1} + \dots + f_{(l+1)/2}^{l+3}(x) \}/|x|^{l+6} \quad (21.43)$$

if l is odd.

The corresponding expression for stress becomes

$$\{ f_0^{l+4}(x)/|x|^{l+2} + f_1^{l+4}(x)/|x|^l + \dots + f_{(l+2)/2}^{l+4}(x) \}/|x|^{l+7} \quad (21.44)$$

if l is even, and

$$\{ f_0^{l+4}(x)/|x|^{l+1} + f_1^{l+4}(x)/|x|^{l-1} + \dots + f_{(l+1)/2}^{l+4}(x) \}/|x|^{l+8} \quad (21.45)$$

if l is odd.

If an infinite number of inclusions are periodically distributed in the three-dimensional space as shown by (21.17), the resulting stress can be obtained by the sum of (21.43) or (21.44), depending on l . These summations are taken for all Ω_p ($p = 1, 2, \dots$). The coordinates appearing in (21.44) and (21.45) are measured from the center of each Ω_p . Convergency of the resulting series can be investigated by Eisenstein's theorem. The theory is applied to the last terms in (21.42) ~ (21.45), since they have the lowest $O(1/|\mathbf{x}|)$. Since $n = 3$ and $s = \frac{4}{2}$ for (21.45), the series constructed from (21.45) is convergent. It is concluded, therefore, that the stress field can be expressed as (21.14) when the eigenstrain is a homogeneous polynomial with degree of an odd number l . The other series representing displacement fields and the stress field for an even number of l are generally divergent. However, the series may converge when the last few terms in (21.42) ~ (21.44) are zero. It is always possible, however, to write the stress field in the form of (21.31).

We show that expression (21.31) with (21.32) is valid even for arbitrarily inclusions in an anisotropic medium with an arbitrary eigenstrain, where ϵ_{ij}^{*p} is integrable in Ω_p . Ω_p are identical for all p .

For Green's function of elasticity $G_{sk}(\mathbf{x})$, we have (5.61),

$$G_{sk,rl}(\mathbf{x}) = V(\mathbf{x})/|\mathbf{x}|^3, \quad (21.46)$$

where $V(\mathbf{x})$ is analytic for $\mathbf{x} \neq 0$ and positively homogeneous of degree 0. Then

$$V(\mathbf{x} + \mathbf{p}) = V(\mathbf{p}) + V_{,i}(\mathbf{p} + \theta\mathbf{x})x_i, \quad (0 < \theta < 1) \quad (21.47)$$

and

$$\frac{V(\mathbf{x} + \mathbf{p})}{|\mathbf{x} + \mathbf{p}|^3} - \frac{V(\mathbf{p})}{|\mathbf{x}|^3} = V(\mathbf{p}) \left(\frac{1}{|\mathbf{x} + \mathbf{p}|^3} - \frac{1}{|\mathbf{x}|^3} \right) + \frac{V_{,i}(\mathbf{p} + \theta\mathbf{x})x_i}{|\mathbf{x} + \mathbf{p}|^3}. \quad (21.48)$$

From (21.27) the series constructed from the first term in the right-hand side in (21.48) converges. Furthermore,

$$|V_{,i}(\mathbf{x})| \leq \frac{V_1}{|\mathbf{x}|} \quad (21.49)$$

and

$$|V_{,i}(\mathbf{p} + \theta\mathbf{x})| \leq \frac{V_1}{|\mathbf{p} + \theta\mathbf{x}|} \leq \frac{V_1}{|\mathbf{p}| - |\mathbf{x}|}, \quad (|\mathbf{p}| > |\mathbf{x}|). \quad (21.50)$$

Therefore, the series constructed from the last term in the right hand side in (21.48) is also convergent. From (6.1) we have

$$\sigma_{ij}^p(x) = -C_{ijrs} \int_{\Omega_p} G_{sk,rl}(x - p - \xi) C_{klmn} \epsilon_{mn}^*(\xi) d\xi - C_{ijmn} \epsilon_{mn}^*(x - p) \quad (21.51)$$

where ϵ_{mn}^* is integrable in Ω_p ; therefore, the series in (21.31) and (21.32) are convergent.

Ellipsoidal inhomogeneities

When the elastic moduli of an ellipsoidal subdomain of a material differ from those of the remainder (matrix), the subdomain is called an ellipsoidal inhomogeneity. Voids, cracks and precipitates are examples of the inhomogeneity which might also be called an inclusion. However, the term “inclusion” has been used in a different context in this book. Here, an inclusion has the same elastic moduli as the matrix, and it contains eigenstrains.

A material containing inhomogeneities is free from any stress field unless a load is applied. On the other hand, a material containing inclusions is subjected to an internal stress (eigenstress) field, even if it is free from all external tractions.

If an inhomogeneity contains an eigenstrain, it is called an inhomogeneous inclusion. Most of the precipitates in alloys and martensites in phase transformation are inhomogeneous inclusions. Eigenstrains inside these inhomogeneous inclusions are misfit and phase transformation strains.

Eshelby (1957) first pointed out that the stress disturbance in an applied stress due to the presence of an inhomogeneity can be simulated by an eigenstress caused by an inclusion when the eigenstrain is chosen properly. This equivalency will be called the equivalent inclusion method.

This book emphasizes the equivalent inclusion method, since it provides a consistent method whereby the results of inclusions in homogeneous media obtained in Chapters 2 and 3 are used. Exact methods of analysis for stress concentration around ellipsoidal cavities in isotropic media subjected to a uniform tension or torsion usually involve harmonic or bi-harmonic displacement potentials. Particular solutions of this kind have been developed by Lamé (1866), Boussinesq (1885), Galerkin (1930), Love (1927), Papkovich (1932), Neuber (1934) and Mindlin (1936). A brief survey of this approach has been given by Sternberg (1958). The solution for a spherical cavity under an arbitrary uniform field at infinity is attributed to Neuber (1937) and Leon (1908). It has also been given by Southwell and Gough (1926), Goodier (1933), Sternberg, Eubanks, and Sadowsky (1952), Sternberg and Sadowsky (1952), and Ling and Yang (1951). Sadowsky and Sternberg (1947), Shapiro (1947),

Miyamoto (1950, 1951, 1953), Edwards (1951) and Das (1954) have given solutions for a spheroidal cavity or inhomogeneity. For an ellipsoidal inhomogeneity or inclusion, solutions have been given by Sadowsky and Sternberg (1949), Lurie (1952), Robinson (1951) and Niesel (1953). The problem of a spheroidal inhomogeneity in a transversely isotropic material has been solved by Bose (1965) and Chen (1966, 1968). In connection with a geotechnical study, Selvadurai (1976, 1978) investigated the load-deflection characteristics of deep rigid anchors embedded in cohesive soil or rock media. The displacement potential functions used by Edwards (1951) were slightly modified by Tsuchida and Nakahara (1974) so that various half-space problems with a cavity could be solved (see, e.g., Tsuchida and Saito 1980). Tsuchida and Mura (1983) obtained the stress field in a half-space having a spheroidal inhomogeneity under tension.

22. Equivalent inclusion method

Consider an infinitely extended material with the elastic moduli C_{ijkl} , containing an ellipsoidal domain Ω with the elastic moduli C_{ijkl}^* . Ω is called an ellipsoidal inhomogeneity. We investigate the disturbance in an applied stress caused by the presence of this inhomogeneity. Let us denote the applied stress at infinity by σ_{ij}^0 and the corresponding strain by $\frac{1}{2}(u_{i,j}^0 + u_{j,i}^0)$. The stress disturbance and the displacement disturbance are denoted by σ_{ij} and u_i , respectively. The total stress (actual stress) is $\sigma_{ij}^0 + \sigma_{ij}$, and the total displacement is $u_i^0 + u_i$. Stress components σ_{ij} are in self-equilibrium; that is,

$$\sigma_{ij,j} = 0 \quad (22.1)$$

and $\sigma_{ij} = 0$ at infinity. When a finite body is considered,

$$\sigma_{ij} n_j = 0 \quad (22.2)$$

on the boundary, where n_i is the normal unit vector on the boundary.

Hooke's law is written as

$$\begin{aligned} \sigma_{ij}^0 + \sigma_{ij} &= C_{ijkl}^*(u_{k,l}^0 + u_{k,l}) \quad \text{in } \Omega, \\ \sigma_{ij}^0 + \sigma_{ij} &= C_{ijkl}(u_{k,l}^0 + u_{k,l}) \quad \text{in } D - \Omega. \end{aligned} \quad (22.3)$$

The equivalent inclusion method is used to simulate the stress disturbance using the eigenstress resulting from an inclusion which occupies the space Ω .

Consider an infinitely extended homogeneous material with the elastic moduli C_{ijkl} everywhere, containing domain Ω with an eigenstrain ϵ_{ij}^* . ϵ_{ij}^* has been introduced here arbitrarily in order to simulate the inhomogeneity problem by use of the inclusion method. Such an eigenstrain is called an equivalent eigenstrain. When this homogeneous material is subjected to the applied strain $\epsilon_{ij}^0 = \frac{1}{2}(u_{i,j}^0 + u_{j,i}^0)$ at infinity, the resulting total stress, distortion, and elastic distortion, respectively, are $\sigma_{ij}^0 + \sigma_{ij}$, $u_{i,j}^0 + u_{i,j}$, and $u_{i,j}^0 + u_{i,j} - \epsilon_{ij}^*$ in Ω . Then, Hooke's law yields

$$\begin{aligned}\sigma_{ij}^0 + \sigma_{ij} &= C_{ijkl}(u_{k,l}^0 + u_{k,l} - \epsilon_{kl}^*) \quad \text{in } \Omega, \\ \sigma_{ij}^0 + \sigma_{ij} &= C_{ijkl}(u_{k,l}^0 + u_{k,l}) \quad \text{in } D - \Omega,\end{aligned}\tag{22.4}$$

where $\sigma_{ij}^0 = C_{ijkl}u_{k,l}^0$.

The necessary and sufficient condition for the equivalency of the stresses and strains in the above two problems of inhomogeneity and inclusion is

$$C_{ijkl}^*(u_{k,l}^0 + u_{k,l}) = C_{ijkl}(u_{k,l}^0 + u_{k,l} - \epsilon_{kl}^*) \quad \text{in } \Omega\tag{22.5}$$

or

$$C_{ijkl}^*(\epsilon_{kl}^0 + \epsilon_{kl}) = C_{ijkl}(\epsilon_{kl}^0 + \epsilon_{kl} - \epsilon_{kl}^*) \quad \text{in } \Omega.\tag{22.5.1}$$

As mentioned in the preceding sections, ϵ_{kl} in the above equation can be obtained as a known function of ϵ_{kl}^* when the eigenstrain problem in the homogeneous material is solved. Thus, (22.5) determines ϵ_{kl}^* for a given ϵ_{kl}^0 , in such a manner that equivalency holds. After obtaining ϵ_{kl}^* , the stress $\sigma_{ij}^0 + \sigma_{ij}$ can be found from (22.3) or (22.4).

If σ_{ij}^0 is a uniform stress, ϵ_{ij}^* is also uniform in Ω . Then, from (11.15) or (17.18)

$$\epsilon_{kl} = \frac{1}{2}(u_{k,l} + u_{l,k}) = S_{klmn}\epsilon_{mn}^*,\tag{22.6}$$

where S_{klmn} are given by (11.16) for isotropic materials and by (17.27) for anisotropic materials.

Substitution of (22.6) into (22.5.1) leads to

$$C_{ijkl}^*(\epsilon_{kl}^0 + S_{klmn}\epsilon_{mn}^*) = C_{ijkl}(\epsilon_{kl}^0 + S_{klmn}\epsilon_{mn}^* - \epsilon_{kl}^*)\tag{22.7}$$

from which the six unknowns, ϵ_{ij}^* , are determined.

Sometimes the inhomogeneity may involve its own eigenstrain. Then, it is called the inhomogeneous inclusion. In other words, a domain which has

different elastic constants from those of the matrix may also have a given distribution of eigenstrain ϵ_{ij}^p . An example is the formation of martensite blades in quenched carbon steel and precipitations in alloys.

Let a material D , containing an ellipsoidal inhomogeneous inclusion Ω , be under a stress field $\sigma_{ij}^0 + \sigma_{ij}$. σ_{ij}^0 is the applied stress if the material is homogeneous, i.e. having no inclusions. σ_{ij} is the sum of the two stress disturbances, one caused by the inhomogeneity, and the other, by the eigenstress associated with eigenstrain ϵ_{ij}^p in Ω .

Denoting the elastic constants of $D - \Omega$ by C_{ijkl} and those of Ω by C_{ijkl}^* , we have Hooke's law,

$$\sigma_{ij}^0 + \sigma_{ij} = C_{ijkl}^*(u_{k,l}^0 + u_{k,l} - \epsilon_{kl}^p) \quad \text{in } \Omega, \quad (22.8)$$

$$\sigma_{ij}^0 + \sigma_{ij} = C_{ijkl}(u_{k,l}^0 + u_{k,l}) \quad \text{in } D - \Omega,$$

where

$$\begin{aligned} \sigma_{ij}^0 &= C_{ijkl}u_{k,l}^0 \quad \text{in } D, \\ \sigma_{ij,j}^0 &= 0 \quad \text{in } D, \\ \sigma_{ij}^0 n_j &= F_i \quad \text{on } |D|, \\ \sigma_{ij,j} &= 0 \quad \text{in } D, \\ \sigma_{ij} n_j &= 0 \quad \text{on } |D|. \end{aligned} \quad (22.9)$$

The inhomogeneous inclusion is simulated by an inclusion in the homogeneous material with eigenstrain ϵ_{ij}^p plus equivalent eigenstrain ϵ_{ij}^* ,

$$\begin{aligned} \sigma_{ij}^0 + \sigma_{ij} &= C_{ijkl}(u_{k,l}^0 + u_{k,l} - \epsilon_{kl}^p - \epsilon_{kl}^*) \quad \text{in } \Omega, \\ \sigma_{ij}^0 + \sigma_{ij} &= C_{ijkl}(u_{k,l}^0 + u_{k,l}) \quad \text{in } D - \Omega. \end{aligned} \quad (22.10)$$

Eigenstrain ϵ_{ij}^* is a fictitious one, introduced for this simulation. The equivalency between (22.8) and (22.10) holds when

$$C_{ijkl}^*(\epsilon_{kl}^0 + \epsilon_{kl} - \epsilon_{kl}^p) = C_{ijkl}(\epsilon_{kl}^0 + \epsilon_{kl} - \epsilon_{kl}^p - \epsilon_{kl}^*) \quad \text{in } \Omega. \quad (22.11)$$

If σ_{ij}^0 is a given uniform stress field and ϵ_{ij}^p is a given uniform eigenstrain in Ω , (22.11) is satisfied by taking ϵ_{ij} as the solution of the inclusion problem with a uniform eigenstrain $\epsilon_{ij}^p + \epsilon_{ij}^*$. Instead of (22.6), we now have

$$\epsilon_{kl} = \frac{1}{2}(u_{k,l} + u_{l,k}) = S_{klmn}(\epsilon_{mn}^p + \epsilon_{mn}^*) \equiv S_{klmn}\epsilon_{mn}^{**}, \quad (22.12)$$

where

$$\epsilon_{ij}^{**} = \epsilon_{ij}^p + \epsilon_{ij}^*. \quad (22.12.1)$$

Substituting (22.12) into (22.11), we have the equation to determine ϵ_{ij}^{**} ,

$$\begin{aligned} \sigma_{ij}^0 + \sigma_{ij} &= C_{ijkl}^*(\epsilon_{kl}^0 + S_{klmn}\epsilon_{mn}^{**} - \epsilon_{kl}^p) \\ &= C_{ijkl}(\epsilon_{kl}^0 + S_{klmn}\epsilon_{mn}^{**} - \epsilon_{kl}^{**}). \end{aligned} \quad (22.13)$$

In the absence of the applied stress, this becomes

$$\sigma_{ij} = C_{ijkl}^*(S_{klmn}\epsilon_{mn}^{**} - \epsilon_{kl}^p) = C_{ijkl}(S_{klmn}\epsilon_{mn}^{**} - \epsilon_{kl}^{**}). \quad (22.13.1)$$

Isotropic materials

Let us illustrate in more detail the equivalent inclusion method described above by assuming the material to be isotropic. When both the matrix and inhomogeneity are isotropic, (22.11) can be written as

$$\begin{aligned} 2\mu^*(\epsilon_{ij}^0 + \epsilon_{ij} - \epsilon_{ij}^p) + \lambda^*\delta_{ij}(\epsilon_{kk}^0 + \epsilon_{kk} - \epsilon_{kk}^p) \\ = 2\mu(\epsilon_{ij}^0 + \epsilon_{ij} - \epsilon_{ij}^{**}) + \lambda\delta_{ij}(\epsilon_{kk}^0 + \epsilon_{kk} - \epsilon_{kk}^{**}), \end{aligned} \quad (22.14)$$

where λ^* , μ^* and λ , μ are the Lamé constants in the inhomogeneity and the matrix, respectively, and

$$\begin{aligned} \epsilon_{ij}^0 &= \frac{1}{2}(u_{i,j}^0 + u_{j,i}^0), \\ \epsilon_{ij}^{**} &= \epsilon_{ij}^* + \epsilon_{ij}^p. \end{aligned} \quad (22.15)$$

It is convenient to introduce the following deviatoric (reduced) strains:

$$\begin{aligned}'\epsilon_{ij}^0 &= \epsilon_{ij}^0 - \delta_{ij}\epsilon_{kk}^0/3, \\ ''\epsilon_{ij}^p &= \epsilon_{ij}^p - \delta_{ij}\epsilon_{kk}^p/3, \\ ''\epsilon_{ij}^{**} &= \epsilon_{ij}^{**} - \delta_{ij}\epsilon_{kk}^{**}/3, \\ '\epsilon_{ij} &= \epsilon_{ij} - \delta_{ij}\epsilon_{kk}/3.\end{aligned}\tag{22.16}$$

Then (22.14) is equivalent to

$$\begin{aligned}2\mu^*('\epsilon_{ij}^0 + ''\epsilon_{ij}^p - ''\epsilon_{ij}^{**}) &= 2\mu(''\epsilon_{ij}^0 + ''\epsilon_{ij} - ''\epsilon_{ij}^{**}), \\ K^*(\epsilon_{kk}^0 + \epsilon_{kk} - \epsilon_{kk}^p) &= K(\epsilon_{kk}^0 + \epsilon_{kk} - \epsilon_{kk}^{**}),\end{aligned}\tag{22.17}$$

where

$$\begin{aligned}K &= \lambda + 2\mu/3 = E/3(1 - 2\nu), \\ E &= 2(1 + \nu)\mu, \\ \lambda &= 2\mu\nu/(1 - 2\nu),\end{aligned}\tag{22.18}$$

and K , E , and ν are the bulk modulus, Young's modulus, and Poisson's ratio of the matrix, respectively. Similar relations hold among the elastic constants of the inhomogeneity.

If σ_{ij}^0 is a given uniform stress field and ϵ_{ij}^p is a given uniform strain in Ω , we can use (22.12) for solving ϵ_{ij}^* from (22.17). The shear components of ϵ_{ij}^{**} are obtained directly from the first equation in (22.17):

$$\begin{aligned}\epsilon_{12}^{**} &= \{2(\mu - \mu^*)\epsilon_{12}^0 + 2\mu^*\epsilon_{12}^p\}/\{4(\mu^* - \mu)S_{1212} + 2\mu\}, \\ \epsilon_{23}^{**} &= \{2(\mu - \mu^*)\epsilon_{23}^0 + 2\mu^*\epsilon_{23}^p\}/\{4(\mu^* - \mu)S_{2323} + 2\mu\}, \\ \epsilon_{31}^{**} &= \{2(\mu - \mu^*)\epsilon_{31}^0 + 2\mu^*\epsilon_{31}^p\}/\{4(\mu^* - \mu)S_{3131} + 2\mu\}.\end{aligned}\tag{22.19}$$

The determination of ϵ_{11}^{**} , ϵ_{22}^{**} and ϵ_{33}^{**} is not a simple matter. After ϵ_{ij}^{**} are calculated, the stress field in Ω is obtained as

$$\sigma_{ij}^0 + \sigma_{ij} = 2\mu(\epsilon_{ij}^0 + S_{ijmn}\epsilon_{mn}^{**} - \epsilon_{ij}^{**}) + \lambda\delta_{ij}(\epsilon_{kk}^0 + S_{kkmn}\epsilon_{mn}^{**} - \epsilon_{kk}^{**}).\tag{22.20}$$

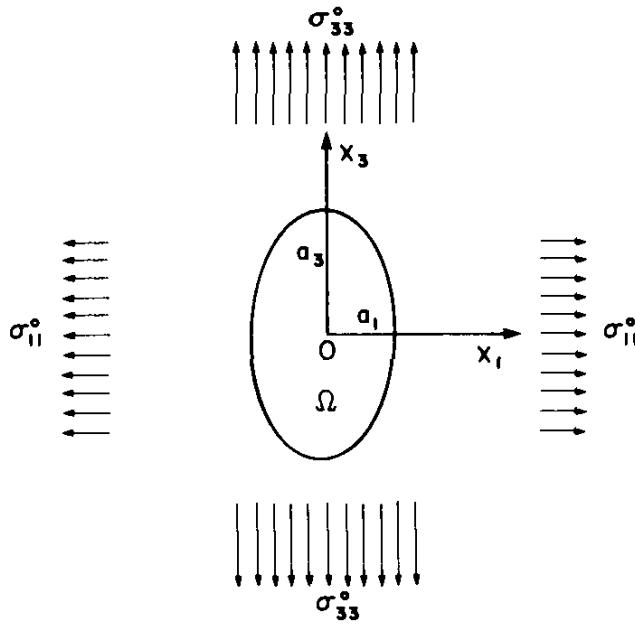


Fig. 22.1 An ellipsoidal inhomogeneity under stress σ_{ij}^0

The strain outside Ω is given by (11.33) and (11.41).

Let us consider the case where applied stress σ_{ij}^0 is uniform at infinity (see Fig. 22.1) and the inhomogeneity is an inhomogeneous inclusion with a uniform plastic strain field ϵ_{11}^p , ϵ_{22}^p , ϵ_{33}^p . When σ_{ij}^0 and ϵ_{ij}^p are shear, the solution has already been given by (22.19) and (22.20).

Sphere

From (11.15) and (11.21), ϵ_{ij} inside the spherical inclusion with uniform eigenstrains ϵ_{11}^{**} , ϵ_{22}^{**} and ϵ_{33}^{**} are given by

$$\begin{aligned} 15(1-\nu)\epsilon_{11} &= (7-5\nu)\epsilon_{11}^{**} + (5\nu-1)\epsilon_{22}^{**} + (5\nu-1)\epsilon_{33}^{**}, \\ 15(1-\nu)\epsilon_{22} &= (5\nu-1)\epsilon_{11}^{**} + (7-5\nu)\epsilon_{22}^{**} + (5\nu-1)\epsilon_{33}^{**}, \\ 15(1-\nu)\epsilon_{33} &= (5\nu-1)\epsilon_{11}^{**} + (5\nu-1)\epsilon_{22}^{**} + (7-5\nu)\epsilon_{33}^{**}, \end{aligned} \quad (22.21)$$

and

$$\epsilon_{kk} = \epsilon_{kk}^{**}(1+\nu)/3(1-\nu). \quad (22.22)$$

Then,

$$\epsilon'_{ij} = \epsilon_{ij}^{**}(8-10\nu)/15(1-\nu). \quad (22.23)$$

With the aid of this last result, it follows from the first equation in (22.17) that

$$'\epsilon_{ij}^{**} = 15 \left\{ ' \epsilon_{ij}^0 (\mu^* - \mu) - ' \epsilon_{ij}^p \mu^* \right\} (1 - \nu) / \{(5\nu - 7)\mu - (8 - 10\nu)\mu^*\}. \quad (22.24)$$

The second equations in (22.17) and (22.22) yield

$$\epsilon_{kk}^{**} = 3 \left\{ (K^* - K) \epsilon_{kk}^0 - K^* \epsilon_{kk}^p \right\} (1 - \nu) / \{(4\nu - 2)K - (1 + \nu)K^*\}. \quad (22.25)$$

Thus, we have

$$\begin{aligned} \epsilon_{ij}^{**} &= 15 \left\{ ' \epsilon_{ij}^0 (\mu^* - \mu) - ' \epsilon_{ij}^p \mu^* \right\} (1 - \nu) / \{(5\nu - 7)\mu - (8 - 10\nu)\mu^*\} \\ &\quad + \delta_{ij} \left\{ (K^* - K) \epsilon_{kk}^0 - K^* \epsilon_{kk}^p \right\} (1 - \nu) / \{(4\nu - 2)K - (1 + \nu)K^*\}. \end{aligned} \quad (22.26)$$

The stress components in Ω can be calculated from (22.20). For the case when tension σ is applied in the x_3 -direction, $'\epsilon_{11}^0 = ' \epsilon_{22}^0 = -\sigma/6\mu$, $'\epsilon_{33}^0 = \sigma/3\mu$ and $\epsilon_{kk}^0 = \sigma/3K$.

Penny shape

When Ω is a penny-shaped inhomogeneity, $a_1 = a_2 = a$ and $a_3 = 0$. From (11.23), ϵ_{ij} inside Ω , for given ϵ_{11}^{**} , ϵ_{22}^{**} and ϵ_{33}^{**} , are

$$\epsilon_{11} = \epsilon_{22} = 0, \quad \epsilon_{33} = \epsilon_{11}^{**}\nu/(1 - \nu) + \epsilon_{22}^{**}\nu/(1 - \nu) + \epsilon_{33}^{**}. \quad (22.27)$$

Then, (22.14) becomes

$$\begin{aligned} (2\mu^* - 2\mu) \epsilon_{11}^0 - 2\mu^* \epsilon_{11}^p + (\lambda^* - \lambda) \epsilon_{kk}^0 - \lambda^* \epsilon_{kk}^p + \lambda^* \epsilon_{33} \\ = -2\mu(\epsilon_{11}^{**} + \nu \epsilon_{22}^{**})/(1 - \nu), \\ (2\mu^* - 2\mu) \epsilon_{22}^0 - 2\mu^* \epsilon_{22}^p + (\lambda^* - \lambda) \epsilon_{kk}^0 - \lambda^* \epsilon_{kk}^p + \lambda^* \epsilon_{33} \\ = -2\mu(\epsilon_{22}^{**} + \nu \epsilon_{11}^{**})/(1 - \nu), \\ (2\mu^* - 2\mu) \epsilon_{33}^0 - 2\mu^* \epsilon_{33}^p + (\lambda^* - \lambda) \epsilon_{kk}^0 - \lambda^* \epsilon_{kk}^p \\ = -(\lambda^* + 2\mu^*) \epsilon_{33}. \end{aligned} \quad (22.28)$$

It should be noted that σ_{33} is zero. When (22.28) and (22.27) are solved for ϵ_{11}^{**} , ϵ_{22}^{**} and ϵ_{33}^{**} , one obtains

$$\begin{aligned}
 2\mu(1+\nu)\epsilon_{11}^{**} &= \{(2\mu - 2\mu^*)\epsilon_{11}^0 + 2\mu^*\epsilon_{11}^p\} - \nu\{(2\mu^* - 2\mu)\epsilon_{22}^0 + 2\mu^*\epsilon_{22}^p\} \\
 &\quad - \{(2\mu - 2\mu^*)\epsilon_{33}^0 + 2\mu^*\epsilon_{33}^p\}(1-\nu)\lambda^*/(\lambda^* + 2\mu^*) \\
 &\quad + \{(\lambda - \lambda^*)\epsilon_{kk}^0 + \lambda^*\epsilon_{kk}^p\}(1-\nu)2\mu^*/(\lambda^* + 2\mu^*), \\
 2\mu(1+\nu)\epsilon_{22}^{**} &= \{(2\mu - 2\mu^*)\epsilon_{22}^0 + 2\mu^*\epsilon_{22}^p\} - \nu\{(2\mu^* - 2\mu)\epsilon_{11}^0 + 2\mu^*\epsilon_{11}^p\} \\
 &\quad - \{(2\mu - 2\mu^*)\epsilon_{33}^0 + 2\mu^*\epsilon_{33}^p\}(1-\nu)\lambda^*/(\lambda^* + 2\mu^*) \\
 &\quad + \{(\lambda - \lambda^*)\epsilon_{kk}^0 + \lambda^*\epsilon_{kk}^p\}(1-\nu)2\mu^*/(\lambda^* + 2\mu^*), \quad (22.29) \\
 2\mu(1+\nu)\epsilon_{33}^{**} &= -\{(2\mu - 2\mu^*)\epsilon_{11}^0 + 2\mu^*\epsilon_{11}^p\}\nu - \{(2\mu - 2\mu^*)\epsilon_{22}^0 + 2\mu^*\epsilon_{22}^p\}\nu \\
 &\quad + \{(2\mu - 2\mu^*)\epsilon_{33}^0 + 2\mu^*\epsilon_{33}^p\}\{2\mu(1+\nu) + 2\lambda^*\nu\} \\
 &\quad /(\lambda^* + 2\mu^*) \\
 &\quad + \{(\lambda - \lambda^*)\epsilon_{kk}^0 + \lambda^*\epsilon_{kk}^p\}\{2\mu(1+\nu) - 4\mu^*\nu\}/(\lambda^* + 2\mu^*).
 \end{aligned}$$

The stress components are calculated from (22.20).

Rod

A rod-shaped geometry results when $a_1 = a_2 = a$ and $a_3 = \infty$. From (11.22), ϵ_{ij} inside Ω , for given ϵ_{11}^{**} , and ϵ_{22}^{**} , ϵ_{33}^{**} , are

$$\begin{aligned}
 8(1-\nu)\epsilon_{11} &= (5-4\nu)\epsilon_{11}^{**} + (4\nu-1)\epsilon_{22}^{**} + 4\nu\epsilon_{33}^{**}, \\
 8(1-\nu)\epsilon_{22} &= (4\nu-1)\epsilon_{11}^{**} + (5-4\nu)\epsilon_{22}^{**} + 4\nu\epsilon_{33}^{**}, \quad (22.30) \\
 \epsilon_{33} &= 0.
 \end{aligned}$$

Substitution of (22.30) into (22.14), after some modification, gives

$$\begin{aligned}
 & \{ \mu^* \left(\frac{5}{4} - \nu \right) + \mu \left(\frac{3}{4} - \nu \right) \} \epsilon_{11}^{**} + \{ \mu^* \left(\nu - \frac{1}{4} \right) + \mu \left(\frac{1}{4} - \nu \right) \} \epsilon_{22}^{**} \\
 & + \{ \mu^* \nu + \mu (\nu - 2) \} \epsilon_{33}^{**} \\
 & = \{ (2\mu - 2\mu^*) (\epsilon_{11}^0 - \epsilon_{33}^0) + 2\mu^* (\epsilon_{11}^p - \epsilon_{33}^p) \} (1 - \nu), \\
 & \{ \mu^* \left(\nu - \frac{1}{4} \right) + \mu \left(\frac{1}{4} - \nu \right) \} \epsilon_{11}^{**} + \{ \mu^* \left(\frac{5}{4} - \nu \right) + \mu \left(\frac{3}{4} - \nu \right) \} \epsilon_{22}^{**} \\
 & + \{ \mu^* \nu + \mu (\nu - 2) \} \epsilon_{33}^{**} \\
 & = \{ (2\mu - 2\mu^*) (\epsilon_{22}^0 - \epsilon_{33}^0) + 2\mu^* (\epsilon_{22}^p - \epsilon_{33}^p) \} (1 - \nu), \\
 & (\frac{1}{2}\lambda^* + \mu\nu) \epsilon_{11}^{**} + (\frac{1}{2}\lambda^* + \mu\nu) \epsilon_{22}^{**} + (\lambda^*\nu + 2\mu) \epsilon_{33}^{**} \\
 & = \{ (2\mu - 2\mu^*) \epsilon_{33}^0 + 2\mu^* \epsilon_{33}^p + (\lambda - \lambda^*) \epsilon_{kk}^0 + \lambda^* \epsilon_{kk}^p \} (1 - \nu). \tag{22.31}
 \end{aligned}$$

ϵ_{ij}^{**} are obtained from these linear algebraic equations.

If the problem is symmetric, so that $\epsilon_{11}^p = \epsilon_{22}^p$, $\epsilon_{11}^0 = \epsilon_{22}^0$ and $\epsilon_{11}^{**} = \epsilon_{22}^{**}$, we have

$$\begin{aligned}
 \epsilon_{11}^{**} &= \frac{1 - \nu}{\Delta} \left[(\lambda^*\nu + 2\mu) \{ (2\mu - 2\mu^*) (\epsilon_{11}^0 - \epsilon_{33}^0) + 2\mu^* (\epsilon_{11}^p - \epsilon_{33}^p) \} \right. \\
 &\quad \left. - \{ \mu^* \nu + \mu (\nu - 2) \} \right. \\
 &\quad \times \left. \{ (2\mu - 2\mu^*) \epsilon_{33}^0 + 2\mu^* \epsilon_{33}^p + (\lambda - \lambda^*) \epsilon_{kk}^0 + \lambda^* \epsilon_{kk}^p \} \right], \\
 \epsilon_{33}^{**} &= \frac{1 - \nu}{\Delta} \left[-(\lambda^* + 2\mu\nu) \{ (2\mu - 2\mu^*) (\epsilon_{11}^0 - \epsilon_{33}^0) + 2\mu^* (\epsilon_{11}^p - \epsilon_{33}^p) \} \right. \\
 &\quad \left. + \{ -\mu^* + \mu (1 - 2\nu) \} \right. \\
 &\quad \times \left. \{ (2\mu - 2\mu^*) \epsilon_{33}^0 + 2\mu^* \epsilon_{33}^p + (\lambda - \lambda^*) \epsilon_{kk}^0 + \lambda^* \epsilon_{kk}^p \} \right], \tag{22.32}
 \end{aligned}$$

where

$$\Delta = -2\lambda^*\mu^*\nu + 2(1 - \nu)\mu(\lambda^* + \mu) - 2\mu^*\mu(1 + \nu^2). \tag{22.33}$$

It should be noted that the stress components are independent of the size of the inhomogeneity, but are dependent on its shape.

Anisotropic inhomogeneities in isotropic matrices

When an anisotropic inhomogeneous inclusion is contained in an isotropic matrix, C_{ijkl} and S_{klmn} in (22.13.1) are taken for the isotropic material. The calculation then follows that of the isotropic inhomogeneity case.

Stress fields for exterior points

Displacement and stress fields outside of an ellipsoidal inhomogeneity, Ω , can be obtained from the formulae in Sections 12 and 18 when the equivalent eigenstrain is determined. However, these formulae are complicated in practice. Recently, Tanaka and Mura (1982) proposed an alternative method to evaluate the elastic fields for points exterior to Ω .

Consider an ellipsoidal inhomogeneity Ω with elastic moduli C_{ijkl}^* in a matrix with elastic moduli C_{ijkl} and an applied stress σ_{ij}^0 at infinity. First, obtain the stress field for the points interior to Ω and denote it by $\sigma_{ij}^0 + \sigma_{ij}(\text{in})$. Next, find the stress field for the points exterior to Ω if Ω is a void and the applied stress is $-\sigma_{ij}(\text{in})$. Denote the stress field by $\sigma_{ij}(\text{out})$. Then, the stress field for the points exterior to Ω in the original problem is the sum of $\sigma_{ij}^0 + \sigma_{ij}(\text{in})$ and $\sigma_{ij}(\text{out})$ (see Fig. 22.2). The proof is obvious. $\sigma_{ij}(\text{out})$ becomes $-\sigma_{ij}(\text{in})$ at infinity and zero in Ω . Therefore, $\sigma_{ij}^0 + \sigma_{ij}(\text{in}) + \sigma_{ij}(\text{out})$ is σ_{ij}^0 at infinity and $\sigma_{ij}^0 + \sigma_{ij}(\text{in})$ in Ω , which are the required conditions in the original problem. This method is useful since solutions for the void problem under various stress fields are usually available in the literature (e.g. Neuber 1937, Muskhelishvili 1953, Savin 1961, Peterson 1962, Miyamoto 1967).

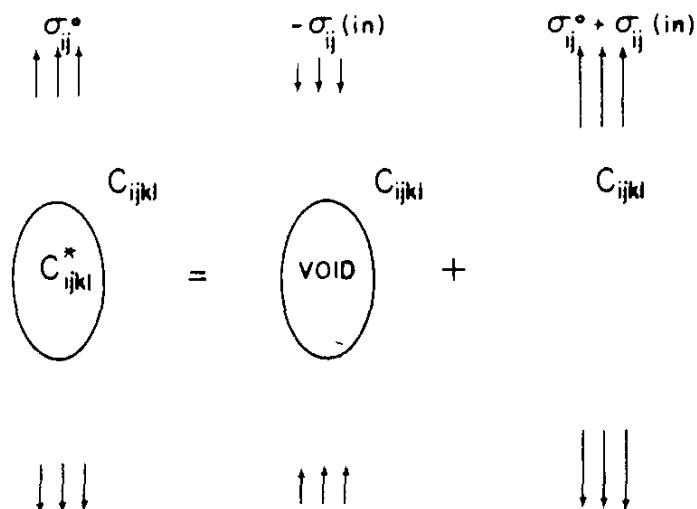


Fig. 22.2 The method for obtaining the exterior elastic field

23. Numerical calculations

The equivalent inclusion method can be extended to cases where applied stresses are not uniform. If an applied stress is linear with respect to the coordinates, the equivalent eigenstrain is also linear with respect to these coordinates. Generally, when the applied stress is a polynomial of degree n , the equivalent eigenstrain is also chosen as a polynomial of degree n . This arises from the fact that for a polynomial eigenstrain of degree n the corresponding eigenstress of the inclusion problem is the sum of polynomials of degrees $n, n - 2, n - 4, \dots$, for interior points (see the paragraph following (19.8)).

Let us consider an ellipsoidal inhomogeneity Ω in an isotropic infinite body (Fig. 23.1). The stress field is calculated when an applied strain (or stress) before the disturbance due to inhomogeneity is given by

$$\epsilon_{ij}^0(x) = E_{ij} + E_{ijk}x_k + E_{ijkl}x_kx_l + \dots, \quad (23.1)$$

where the coefficients are constants. The eigenstrain in (22.5.1) is assumed to be

$$\epsilon_{ij}^*(x) = B_{ij} + B_{ijk}x_k + B_{ijkl}x_kx_l + \dots. \quad (23.2)$$

The strain associated with (23.2) is given by (12.10) and (11.33),

$$\epsilon_{ij}(x) = D_{ijkl}(x)B_{kl} + D_{ijklq}(x)B_{klq} + D_{ijklqr}(x)B_{klqr} + \dots. \quad (23.3)$$

It should be remembered that for points interior to Ω , D_{ijkl} is constant, D_{ijklq} is linear in x , and D_{ijklqr} is the sum of the constants and quadratic functions

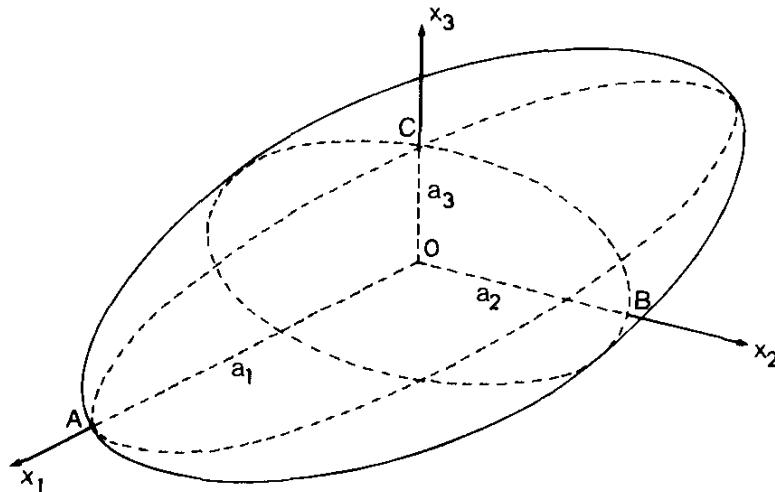


Fig. 23.1 An ellipsoidal inhomogeneity, indicating points A , B , and C

of x . Using expression (12.16), substituting (23.1), (23.2) and (23.3) into (22.5.1), and comparing coefficients in the power series, we obtain

$$\begin{aligned}
 & (C_{stmn} - C_{stmn}^*) \{ D_{mni_j}(0) B_{ij} + D_{mni_j k l}(0) B_{ijkl} + \dots \} - C_{stmn} B_{mn} \\
 & = (C_{stmn}^* - C_{stmn}) E_{mn}, \\
 & (C_{stmn} - C_{stmn}^*) D_{mni_j k, p}(0) B_{ijk} + \dots - C_{stmn} B_{mnp} \\
 & = (C_{stmn}^* - C_{stmn}) E_{mnp}, \\
 & (1/2!)(C_{stmn} - C_{stmn}^*) D_{mni_j k l, p q}(0) B_{ijkl} + \dots - C_{stmn} B_{mnpq} \\
 & = (C_{stmn}^* - C_{stmn}) E_{mnpq},
 \end{aligned} \tag{23.4}$$

etc.,

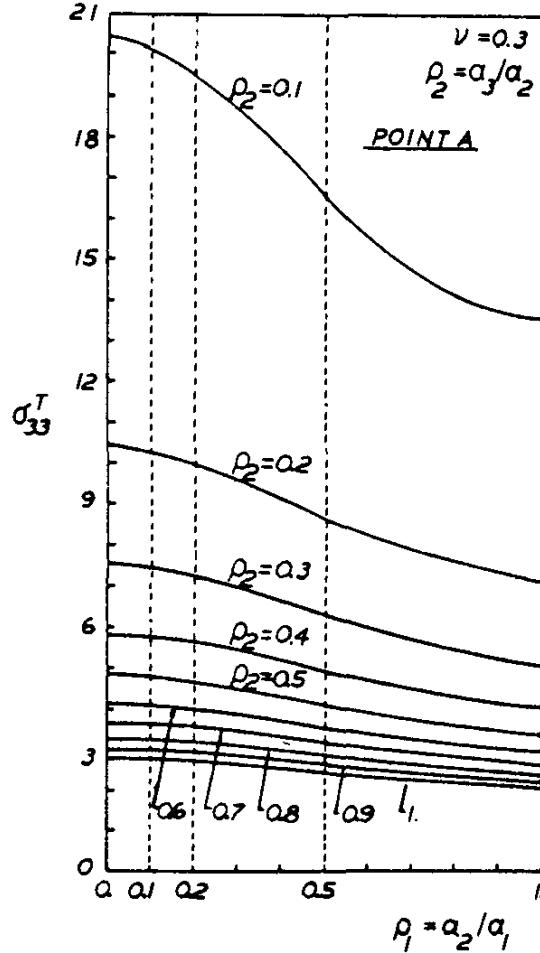


Fig. 23.2 Ellipsoidal void under $\sigma_{33}^0 = 1$, the stress component σ_{33}^T at point A in Fig. 23.1

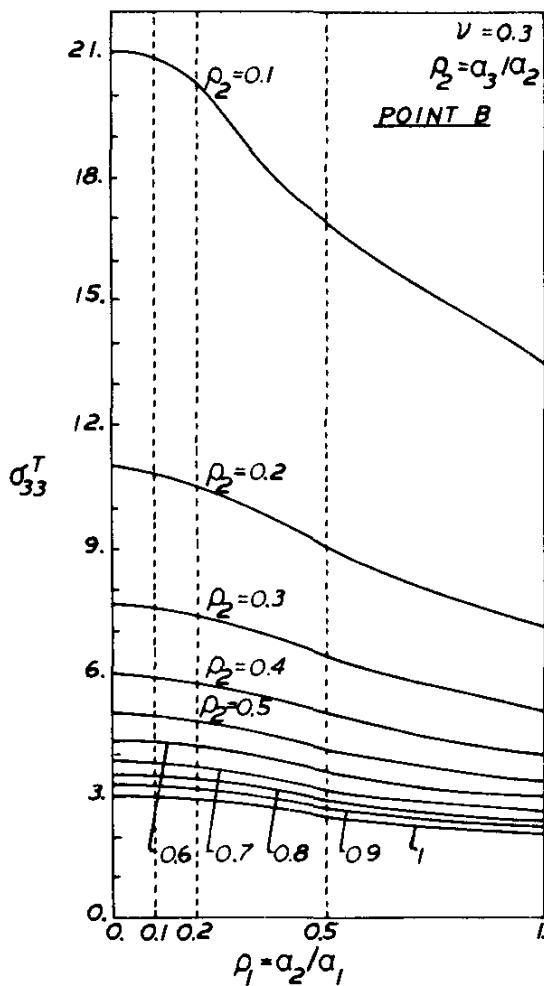


Fig. 23.3. Ellipsoidal void under $\sigma_{33}^0 = 1$, σ_{33}^T at point B

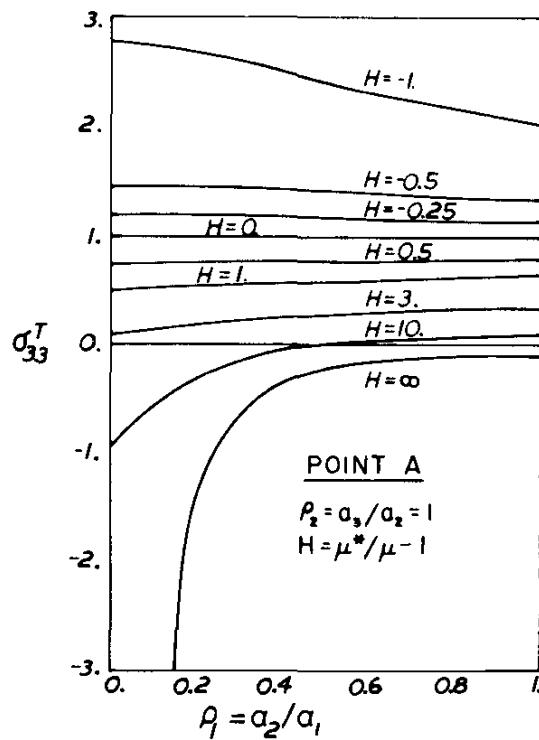


Fig. 23.4 Ellipsoidal inhomogeneity under $\sigma_{33}^0 = 1$, σ_{33}^T at point A, where μ^* is the shear modulus of the inhomogeneity

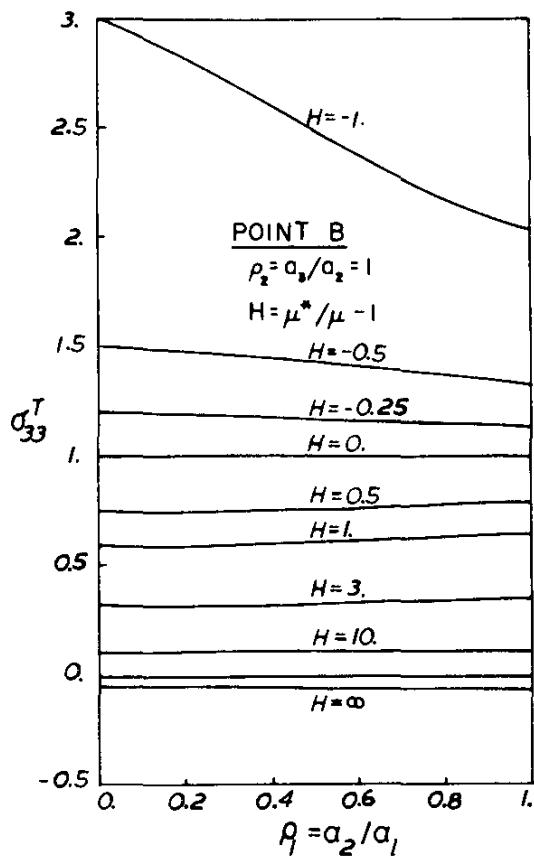


Fig. 23.5. Ellipsoidal inhomogeneity under $\sigma_{33}^0 = 1$, σ_{33}^T at point *B*.

where C_{ijkl}^* and C_{ijkl} are the elastic moduli of the inhomogeneity and the matrix, respectively. The system of equations (23.4) determines the unknown *B* constants. The stress and strain fields are determined from (23.3) and these constants.

Numerical calculations are performed by Moschovidis (1975). Figures 23.2 and 23.3 show $\sigma_{33}^T = \sigma_{33}^0 + \sigma_{33}$ at *A* and *B* in Fig. 23.1 when $\sigma_{33}^0 = 1$ is applied to an infinite isotropic body containing an ellipsoidal void. The curves in these figures show that the normal stress σ_{33}^T at point *B* is always larger than that at point *A* when $a_1 > a_2$.

Figures 23.4 and 23.5 show $\sigma_{33}^T = \sigma_{33}^0 + \sigma_{33}$ at *A* and *B* when $\sigma_{33}^0 = 1$ is applied to an infinite isotropic body containing a spheroidal inhomogeneity with shear modulus μ^* . These curves show the dependency of the normal stress on shear modulus and geometry. The shear modulus of the matrix is μ , and Poisson's ratios ν and ν^* are equal to 0.3. For large values of μ^* , σ_{33}^T shows large compression at point *A*. It is also found that σ_{11}^T and σ_{22}^T are everywhere much smaller than σ_{33}^T except at point *A* where σ_{22}^T is comparable to σ_{33}^T .

Figure 23.6 shows $\sigma_{23}^T = \sigma_{23}^0 + \sigma_{23}$ along the x_1 -axis when σ_{23}^0 is applied linearly (as shown by the dotted line) to an infinite isotropic body containing a

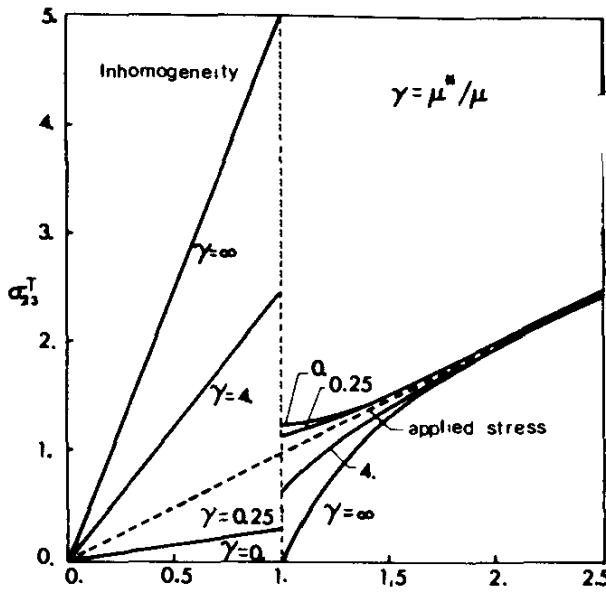


Fig. 23.6 Spherical inhomogeneity under linear torsion about the x_3 -axis

spherical inhomogeneity. The axis of torsion is the x_3 -axis, and $\gamma = \mu^*/\mu$. It is important to note that the system of algebraic equations (23.4) to be solved for B_{ijk} , becomes singular when the inhomogeneity reduces to a void ($\gamma = 0$). It is easy to see this from equation (22.5.1) which, for $\gamma = 0$, yields

$$C_{ijkl}(\epsilon_{kl} - \epsilon_{kl}^*) = -C_{ijkl}\epsilon_{kl}^0. \quad (23.5)$$

As a solution of the inclusion problem, $\sigma_{ij} = C_{ijkl}(\epsilon_{kl} - \epsilon_{kl}^*)$ always satisfies the equations of equilibrium. Therefore, the right-hand side in (23.5) must also satisfy the equations of equilibrium. In other words, the system of algebraic equations has a non-trivial solution only when terms in the right-hand side of the equations are proper. This situation leads to the conclusion that the system of algebraic equations (23.4) must be singular for $\gamma = 0$. This singularity will be discussed in Section 24.

Two ellipsoidal inhomogeneities

Let us consider two inhomogeneities, Ω_1 and Ω_2 , as shown in Fig. 23.7, under the applied stress σ_{ij}^0 . Applying the equivalent inclusion method in domains Ω_1 and Ω_2 , the equivalency equations become, from (22.5.1),

$$\begin{aligned} C_{ijkl}^I(\epsilon_{kl}^0 + \epsilon_{kl}) &= C_{ijkl}(\epsilon_{kl}^0 + \epsilon_{kl} - \beta_{kl}^I) \quad \text{in } \Omega_1, \\ C_{ijkl}^{II}(\epsilon_{kl}^0 + \epsilon_{kl}) &= C_{ijkl}(\epsilon_{kl}^0 + \epsilon_{kl} - \beta_{kl}^{II}) \quad \text{in } \Omega_2, \end{aligned} \quad (23.6)$$

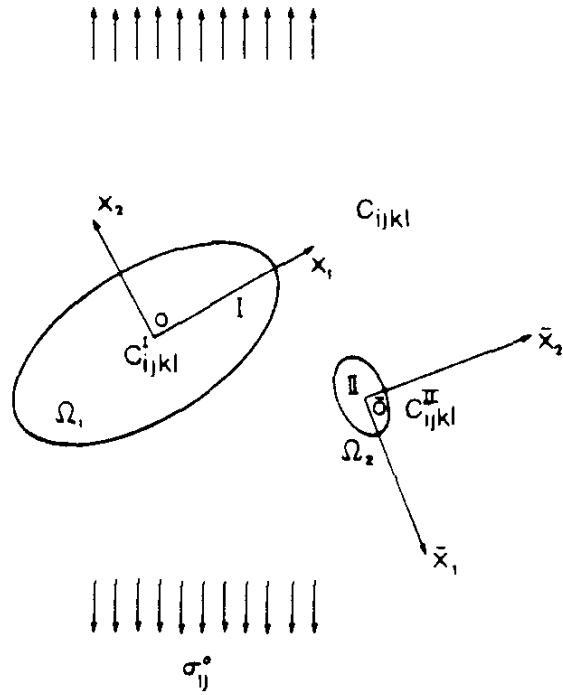


Fig 23.7 Two ellipsoidal inhomogeneities under applied stress σ_{ij}^0

where C_{ijkl}^I and β_{kl}^I represent, respectively, the elastic moduli and the equivalent eigenstrains defined in Ω_1 . The corresponding quantities in Ω_2 have the superscript II. It should be emphasized here that ϵ_{kl} is no longer uniform even if ϵ_{kl}^0 is uniform, since interior points of Ω_1 are exterior points of Ω_2 and the stress in Ω_1 is disturbed by Ω_2 .

Let us assume that the material is isotropic and infinitely extended. The applied strain before disturbance is given by

$$\epsilon_{ij}^0(x) = E_{ij} + E_{ijk}x_k + E_{ijkl}x_kx_l + \dots, \quad (23.7)$$

where the x_i coordinate system is taken at the center of Ω_1 . If the \bar{x}_i coordinate system is taken at the center of Ω_2 , the corresponding expression becomes

$$\bar{\epsilon}_{ij}^0(x) = \bar{E}_{ij} + \bar{E}_{ijk}\bar{x}_k + \bar{E}_{ijkl}\bar{x}_k\bar{x}_l + \dots. \quad (23.8)$$

Hereafter, tensor components referring to the \bar{x}_i -coordinate system are denoted by bars.

The two coordinate systems are related by

$$x_i - c_i = a_{ij}\bar{x}_j, \quad \bar{x}_i = a_{ji}(x_j - c_j), \quad (23.9)$$

where a_{ij} is the direction cosine between the x_i -axis and the \bar{x}_j -axis, and c_i is the x_i -coordinate of the origin $\bar{0}$ in Ω_2 .

We assume that the equivalent eigenstrains are

$$\begin{aligned}\beta_{ij}^I(x) &= B_{ij}^I + B_{ijk}^I x_k + B_{ijkl}^I x_k x_l + \dots, \\ \bar{\beta}_{ij}^{II}(\bar{x}) &= B_{ij}^{II} + B_{ijk}^{II} \bar{x}_k + B_{ijkl}^{II} \bar{x}_k \bar{x}_l + \dots.\end{aligned}\quad (23.10)$$

From (11.33) and (12.10) we can write

$$\begin{aligned}\epsilon_{ij}^I(x) &= D_{ijkl}^I(x) B_{kl}^I + D_{ijklq}^I(x) B_{klq}^I + D_{ijklqr}^I(x) B_{klqr}^I + \dots, \\ \bar{\epsilon}_{ij}^{II}(\bar{x}) &= D_{ijkl}^{II}(\bar{x}) B_{kl}^{II} + D_{ijklq}^{II}(\bar{x}) B_{klq}^{II} + D_{ijklqr}^{II}(\bar{x}) B_{klqr}^{II} + \dots,\end{aligned}\quad (23.11)$$

where ϵ_{ij}^I and $\bar{\epsilon}_{ij}^{II}$ are the respective strains caused by β_{ij}^I and $\bar{\beta}_{ij}^{II}$. The strain ϵ_{ij} in (23.6) is the sum of ϵ_{ij}^I and $\bar{\epsilon}_{ij}^{II}$. Interior points of Ω_1 are exterior points of Ω_2 , and functions D in (23.11) depend on position x as has been discussed following (12.15). As seen in (12.16), $D^I(x)$ are polynomials of x in Ω_1 ; however, $D^{II}(\bar{x})$ are not polynomials of x in Ω_1 . For x in Ω_1 , $D^{II}(\bar{x})$ are approximated by a Taylor expansion around point 0. Similarly, $D^I(x)$ are also expanded in a Taylor series of x in Ω_2 . $D^{II}(\bar{x})$ are polynomials of \bar{x} in Ω_2 , according to (12.16). After these considerations, the system of equations (23.6) is solved for B . The coefficients of the power series in the left- and right-hand sides in (23.6) are equated. Consequently, in Ω_1 ,

$$\begin{aligned}\Delta C_{stmn}^I \left\{ \left[D_{mni_j}^I[0] B_{ij}^I + D_{mni_j k l}^I[0] B_{ijkl}^I + \dots \right] \right. \\ \left. + a_{mc} a_{nh} \left[D_{chij}^{II}[0] B_{ij}^{II} + D_{chijk}^{II}[0] B_{ijk}^{II} \right. \right. \\ \left. \left. + D_{chijkl}^{II}[0] B_{ijkl}^{II} + \dots \right] \right\} - C_{stmn} B_{mn}^I \\ = -\Delta C_{stmn}^I E_{mn}, \\ \Delta C_{stmn}^I \left\{ \left[\frac{\partial}{\partial x_p} D_{mni_j k}^I[0] B_{ijk}^I + \dots \right] \right. \\ \left. + a_{mc} a_{nh} a_{pf} \left[\frac{\partial}{\partial \bar{x}_f} D_{chij}^{II}[0] B_{ij}^{II} + \frac{\partial}{\partial \bar{x}_f} D_{chijk}^{II}[0] B_{ijk}^{II} \right. \right. \\ \left. \left. + \frac{\partial}{\partial \bar{x}_f} D_{chijkl}^{II}[0] B_{ijkl}^{II} + \dots \right] \right\} - C_{stmn} B_{mnp}^I \\ = -\Delta C_{stmn}^I E_{mnp},\end{aligned}$$

$$\begin{aligned}
& \frac{1}{2!} \Delta C_{stmn}^I \left\{ \left[\frac{\partial^2}{\partial x_p \partial x_q} D_{mni j k l}^I [0] B_{ijkl}^I + \dots \right] \right. \\
& \quad + a_{mc} a_{nh} a_{pf} a_{qg} \left[\frac{\partial^2}{\partial \bar{x}_f \partial \bar{x}_g} D_{chi j}^{\text{II}} [0] B_{ij}^{\text{II}} + \frac{\partial^2}{\partial \bar{x}_f \partial \bar{x}_g} D_{chi j k}^{\text{II}} [0] B_{ijk}^{\text{II}} \right. \\
& \quad \left. \left. + \frac{\partial^2}{\partial \bar{x}_f \partial \bar{x}_g} D_{chi j k l}^{\text{II}} [0] B_{ijkl}^{\text{II}} + \dots \right] \right\} - C_{stmn} B_{mnpq}^I \\
& = -\Delta C_{stmn}^I E_{mnpq}, \tag{23.12}
\end{aligned}$$

etc.

Similarly, in Ω_2 ,

$$\begin{aligned}
& \Delta C_{stmn}^{\text{II}} \left\{ a_{cm} a_{hn} \left[D_{chi j}^I [\bar{0}] B_{ij}^I + D_{chi j k}^I [\bar{0}] B_{ijk}^I + D_{chi j k l}^I [\bar{0}] B_{ijkl}^I + \dots \right] \right. \\
& \quad \left. + \left[D_{mni j}^{\text{II}} [\bar{0}] B_{ij}^{\text{II}} + D_{mni j k l}^{\text{II}} [\bar{0}] B_{ijkl}^{\text{II}} + \dots \right] \right\} - C_{stmn} B_{mn}^{\text{II}} \\
& = -\Delta C_{stmn}^{\text{II}} \bar{E}_{mn}, \\
& \Delta C_{stmn}^{\text{II}} \left\{ a_{cm} a_{hn} a_{fp} \left[\frac{\partial}{\partial x_f} D_{chi j}^I [\bar{0}] B_{ij}^I + \frac{\partial}{\partial x_f} D_{chi j k}^I [\bar{0}] B_{ijk}^I \right. \right. \\
& \quad \left. \left. + \frac{\partial}{\partial x_f} D_{chi j k l}^I [\bar{0}] B_{ijkl}^I + \dots \right] \right. \\
& \quad \left. + \left[\frac{\partial}{\partial \bar{x}_p} D_{mni j k}^{\text{II}} [\bar{0}] B_{ijk}^{\text{II}} + \dots \right] \right\} - C_{stmn} B_{mnp}^{\text{II}} \\
& = -\Delta C_{stmn}^{\text{II}} \bar{E}_{mnp}, \tag{23.13}
\end{aligned}$$

$$\begin{aligned}
& \frac{1}{2!} \Delta C_{stmn}^{\text{II}} \left\{ a_{cm} a_{hn} a_{fp} a_{gq} \left[\frac{\partial^2}{\partial x_f \partial x_g} D_{chi j}^I [\bar{0}] B_{ij}^I + \frac{\partial^2}{\partial x_f \partial x_g} D_{chi j k}^I [\bar{0}] B_{ijk}^I \right. \right. \\
& \quad \left. \left. + \frac{\partial^2}{\partial x_f \partial x_g} D_{chi j k l}^I [\bar{0}] B_{ijkl}^I + \dots \right] \right\}
\end{aligned}$$

$$\begin{aligned}
& + \left[\frac{\partial^2}{\partial \bar{x}_p \partial \bar{x}_q} D_{mnijkl}^{\text{II}} [\bar{0}] B_{ijkl}^{\text{II}} + \dots \right] \Bigg) - C_{stmn} B_{mnpq}^{\text{II}} \\
& = -\Delta C_{stmn}^{\text{II}} \bar{E}_{mnpq},
\end{aligned}$$

etc.,

where

$$\Delta C_{ijkl}^{\text{I}} = C_{ijkl} - C_{ijkl}^{\text{I}}, \quad \Delta C_{ijkl}^{\text{II}} = C_{ijkl} - C_{ijkl}^{\text{II}}. \quad (23.14)$$

Equations (23.12) and (23.13) determine the unknowns B^{I} and B^{II} . The number of unknowns is the same as the number of equations.

Numerical calculations are performed by Moschovidis (1975). Table 23.1 shows the stresses at A , C' and C (see Fig. 23.8) for the two spherical cavities in the uniform tension field $\sigma_{11}^0 = \sigma_{22}^0 = \sigma_{33}^0 = 1$. The first row of the table shows the exact solution obtained by Sternberg and Sadowsky (1952). The second and third rows of the table show the current approximations calculated from the equivalent inclusion method when the equivalent eigenstrains are assumed to be polynomials of degree zero and of degree one in the position coordinates. Δ is the distance between 0 and $\bar{0}$.

It is found that the interaction between the two cavities becomes negligible when $\Delta > 4a_1$.

Figure 23.9 shows the stress distributions $\sigma_{ij}^T = \sigma_{ij}^0 + \sigma_{ij}$ for two spheroidal cavities ($a_1 = a_2 = \bar{a}_1 = \bar{a}_2 = 1$ and $a_3 = \bar{a}_3 = 0.125$) under $\sigma_{33}^0 = 1$. The distance between the cavities is relatively small ($\Delta = 2.5a_1$), and the interaction effect is apparent. The solutions have been approximated by using linear eigenstrains, and the cavities have been regarded as weak inhomogeneities ($\mu^{\text{I}} = \mu^{\text{II}} = 10^{-7} \mu$).

Table 23.1 Two spherical cavities of unit radii in uniform applied tension $\sigma_{11}^0 = \sigma_{22}^0 = \sigma_{33}^0 = 1$ at ∞ , stresses at equators and poles $\nu = 0.25$, $\Delta = 4$

	Point A			Point C'			Point C		
	σ_{11}^T	σ_{22}^T	σ_{33}^T	σ_{11}^T	σ_{22}^T	σ_{33}^T	σ_{11}^T	σ_{22}^T	σ_{33}^T
Sternberg and Sadowsky (1952)	0.0	1.50	1.47	1.51	1.51	0.0	1.57	1.57	0.0
Zeroth degree polynomial	0.011	1.508	1.467	1.519	1.519	0.01	1.534	1.534	-0.017
1st degree polynomial	0.001	1.5079	1.4676	1.503	1.503	-0.0008	1.5495	1.5495	-0.006

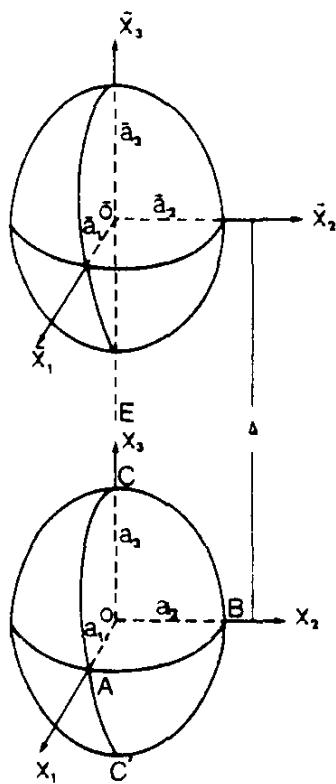


Fig. 23.8. Two ellipsoidal inhomogeneities under σ_{ij}^0 , indicating points *A*, *B* and *C*.

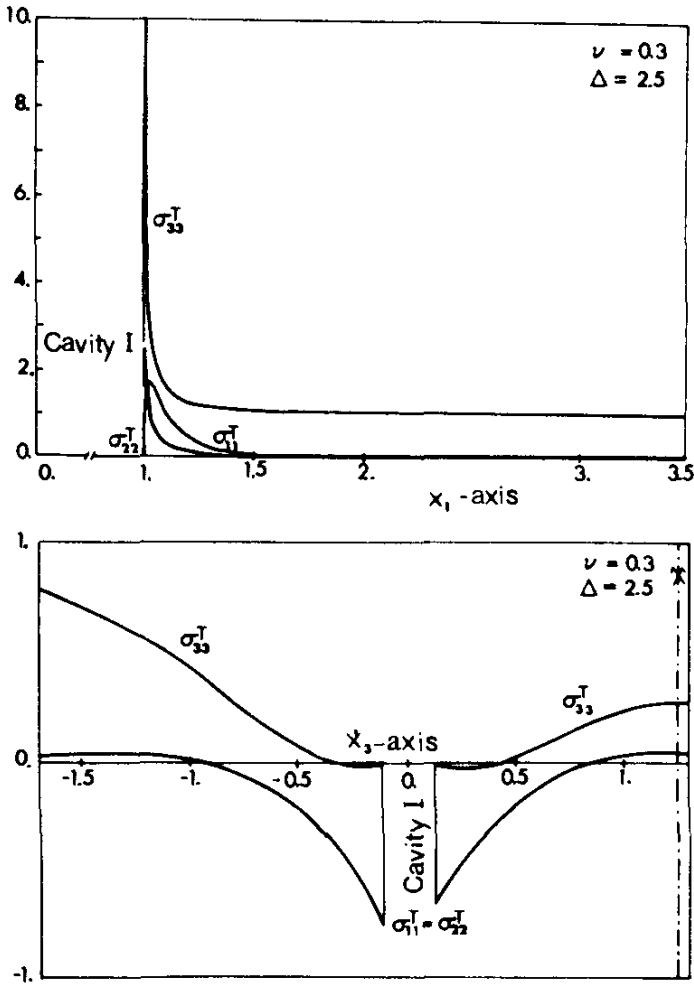


Fig. 23.9. Two spheroidal cavities ($a_1 = \bar{a}_1 = a_2 = \bar{a}_2 = 1$, $a_3 = \bar{a}_3 = 0.125$) under $\sigma_{33}^0 = 1$.

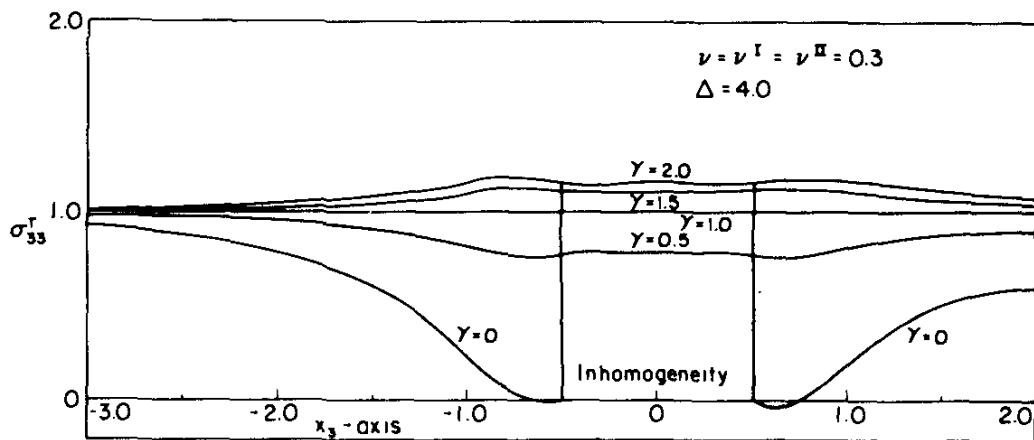


Fig. 23.10. Two spheroidal inhomogeneities ($a_1 = \bar{a}_1 = a_2 = \bar{a}_2 = 1$, $a_3 = \bar{a}_3 = 0.5$) under uniaxial tension $\sigma_{33}^0 = 1$ at ∞ , σ_{33}^T along x_3 -axis for various values of $\gamma = \mu^I/\mu = \mu^{II}/\mu$.

Figure 23.10 shows σ_{33}^T for two identical spheroidal inhomogeneities ($a_1 = a_2 = 1$, $a_3 = 0.5$, $\gamma = \mu^I/\mu = \mu^{II}/\mu$, $\nu = \nu^I = \nu^{II} = 0.3$) under an applied stress $\sigma_{33}^0 = 1$. It can be seen that the stress field inside the inhomogeneities is practically constant.

The accurate elastic interaction between two finite gas bubbles, each of arbitrary size under excess gas pressure, has been investigated by Willis and Bullough (1969). They found that two such bubbles always attract one another in an elastically isotropic medium. The Taylor-series method has been recently used by Johnson, Earmme, and Lee (1980) to study cuboidal precipitates.

* 24. Impotent eigenstrains

As mentioned in the paragraph preceding (23.5), the system of equations (22.5.1) or (23.4) becomes singular when the inhomogeneity is a void, and the applied stress, before the disturbance due to the inhomogeneity, is in the form of a degree-one polynomial. The solution of this system of equivalency equations is not unique. In other words, there are an infinite number of equivalent eigenstrains which do not generate any stress field within the material. Such eigenstrains are called impotent eigenstrains.

In this section analytical expressions for solutions of the system of homogeneous equivalency equations are presented for a spherical void in an isotropic material. The result of this special case is extended to the general case of arbitrarily shaped inclusions in anisotropic materials. Most parts of this section follow the work of Furuhashi and Mura (1979).

Let Ω be an inhomogeneity in D having elastic moduli C_{ijmn}^* , and let C_{ijmn} be the elastic moduli of the remainder $D - \Omega$. Let σ_{ij}^0 and ϵ_{ij}^0 be the stress and strain when the inhomogeneity is absent. Then we have $\sigma_{ij}^0 = C_{ijmn}\epsilon_{mn}^0$. Due to

the presence of the inhomogeneity, the stress and the strain are changed to $\sigma_{ij}^0 + \sigma_{ij}$ and $\epsilon_{ij}^0 + \epsilon_{ij}$, where

$$\epsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i}). \quad (24.1)$$

The equivalency equation which transforms an inhomogeneity problem into an equivalent inclusion problem is expressed by (22.5.1) or

$$C_{ijmn}\epsilon_{mn}^* - \Delta C_{ijmn}\epsilon_{mn} = \sigma_{ij}^0 - C_{ijmn}^*\epsilon_{mn}^0, \quad (24.2)$$

where ϵ_{mn}^* is the equivalent eigenstrain in Ω and

$$\Delta C_{ijmn} = C_{ijmn} - C_{ijmn}^*. \quad (24.2.1)$$

When Ω has an eigenstrain ϵ_{rq}^* , the displacement u_m is expressed by (6.1),

$$u_m(\mathbf{x}) = - \int_{\Omega} G_{mk,l}(\mathbf{x} - \mathbf{x}') C_{klrq} \epsilon_{rq}^*(\mathbf{x}') d\mathbf{x}'. \quad (24.3)$$

Substitution of (24.3) into (24.2) leads to the following fundamental integro-differential equation to give the equivalent eigenstrain ϵ_{ij}^* :

$$\sigma_{ij}^*(\mathbf{x}) + \Delta C_{ijmn} \frac{\partial^2}{\partial x_n \partial x_l} \int_{\Omega} G_{km}(\mathbf{x} - \mathbf{x}') \sigma_{kl}^*(\mathbf{x}') d\mathbf{x}' = q_{ij}(\mathbf{x}), \quad (24.4)$$

where

$$\sigma_{ij}^* = C_{ijmn}\epsilon_{mn}^*, \quad q_{ij} = \sigma_{ij}^0 - C_{ijmn}^*\epsilon_{mn}^0. \quad (24.5)$$

We can calculate the stress σ_{ij} from the formula

$$\sigma_{ij}(\mathbf{x}) = - C_{ijmn} \frac{\partial^2}{\partial x_n \partial x_l} \int_{\Omega} G_{mk}(\mathbf{x} - \mathbf{x}') \sigma_{kl}^*(\mathbf{x}') d\mathbf{x}' - \sigma_{ij}^*(\mathbf{x}). \quad (24.6)$$

Let us consider a special case in which the inhomogeneity Ω is a spherical isotropic inhomogeneity with shear modulus μ^* and Poisson's ratio ν^* and the matrix is an infinitely extended isotropic material with shear modulus μ and Poisson's ratio ν .

The applied stress field before the inhomogeneity disturbance is assumed to be a homogeneous polynomial of degree one,

$$q_{ij}(x) = B_{ijp}x_p, \quad B_{ijp} = B_{jip}, \quad (24.7)$$

where B_{ijp} are given constants.

It is well known that when Ω is an ellipsoid, the integral

$$\frac{\partial^2}{\partial x_n \partial x_l} \int_{\Omega} G_{km}(x - x') x'_p dx' \quad (24.7.1)$$

becomes a polynomial of degree one without the constant term for interior points, as discussed following (12.16). Therefore, we can write

$$\Delta C_{ijmn} \frac{\partial^2}{\partial x_n \partial x_l} \int_{\Omega} G_{km}(x - x') x'_p dx' = \omega_{ijq}^{klp} x_q. \quad (24.8)$$

We seek a solution of the integro-differential equation (24.4) in the forms,

$$\sigma_{ij}^*(x) = A_{ijp}x_p, \quad A_{ijp} = A_{jip}, \quad (24.9)$$

where A_{ijp} are constants to be determined. Substituting this expression into (24.5) and using (24.8), we obtain

$$A_{ijq}x_q + \omega_{ijq}^{klp}A_{klp}x_q = B_{ijq}x_q. \quad (24.9.1)$$

Hence, A_{ijq} must be a solution of the following linear system of equations:

$$A_{ijq} + \omega_{ijq}^{klp}A_{klp} = B_{ijq}. \quad (24.10)$$

The number of equations in (24.10) is 18. If Ω is not a void, the independent number of B_{ijq} is also 18. From (24.7) and (24.5), we have

$$q_{ij,j} = B_{ijj} = -C_{ijmn}^* \epsilon_{mn,j}^0, \quad (24.11)$$

since $\sigma_{ij,j}^0 = 0$.

When Ω is a void, we get the three relations $B_{ijj} = 0$. Therefore, when Ω is a void, the maximum number of independent coefficients B_{ijp} is 15. The system of 18 equations (24.10) consists of four groups of equations with coefficients $(A_{111}, A_{221}, A_{331}, A_{122}, A_{133})$, $(A_{222}, A_{112}, A_{332}, A_{121}, A_{233})$, $(A_{333}, A_{113}, A_{223}, A_{131}, A_{232})$ and $(A_{123}, A_{132}, A_{231})$. Each of the first three groups is made

up of a 5×5 matrix, and the last group is made up of a 3×3 matrix. If Ω is not a void, the solutions of these equations are uniquely determined.

When Ω is a void, the following matrix belongs to the first three groups:

$$\frac{1}{35(1-\nu)} \begin{pmatrix} 8 - 14\nu & 3 - 7\nu & 3 - 7\nu & -8 & -8 \\ 3 - 21\nu & 24 - 28\nu & 1 - 7\nu & -8 & 2 - 14\nu \\ 3 - 21\nu & 1 - 7\nu & 24 - 28\nu & 2 - 14\nu & -8 \\ -4 + 7\nu & -4 + 7\nu & 1 & 13 - 7\nu & -5 + 7\nu \\ -4 + 7\nu & 1 & -4 + 7\nu & -5 + 7\nu & 13 - 7\nu \end{pmatrix}. \quad (24.12)$$

Similarly, the matrix belonging to the last group is

$$\frac{1}{35(1-\nu)} \begin{pmatrix} 23 - 21\nu & -5 + 7\nu & -5 + 7\nu \\ -5 + 7\nu & 23 - 21\nu & -5 + 7\nu \\ -5 + 7\nu & -5 + 7\nu & 23 - 21\nu \end{pmatrix}. \quad (24.13)$$

The ranks of these two matrices are 4 and 3, respectively. Hence, we see that the rank of the matrix of coefficients in equation (24.10) is 15 when Ω is a void. Therefore, the homogeneous equations

$$A_{ijq} + \omega_{ijq}^{klp} A_{klp} = 0, \quad (24.14)$$

belonging to (24.10), have three independent non-trivial solutions A_{klp}^s ($s = 1, 2, 3$). Hence, if we denote by \check{A}_{klp} an arbitrary particular solution of the system of equations (24.10), we can express the general solution as

$$A_{klp} = \sum_{s=1}^3 C_s A_{klp}^s + \check{A}_{klp}, \quad (24.15)$$

where C_s are arbitrary constants. We can easily ascertain from (24.12) that A_{klp}^1 is the following vector:

$$A_{111}^1 = 2 + \frac{2\nu}{1-2\nu}, \quad A_{221}^1 = A_{331}^1 = \frac{2\nu}{1-2\nu}, \quad A_{122}^1 = A_{133}^1 = 1. \quad (24.16)$$

Similarly, A_{klp}^2 and A_{klp}^3 are obtained by the cyclic permutation of the indices. The eigenstrain ϵ_{ij}^{*1} corresponding to this solution is obtained from (24.5) and (24.9) as

$$\epsilon_{11}^{*1} = \frac{x_1}{\mu}, \quad \epsilon_{22}^{*1} = \epsilon_{33}^{*1} = 0, \quad \epsilon_{12}^{*1} = \frac{x_2}{2\mu}, \quad \epsilon_{13}^{*1} = \frac{x_3}{2\mu}, \quad \epsilon_{23}^{*1} = 0. \quad (24.17)$$

We obtain ϵ_{ij}^{*2} and ϵ_{ij}^{*3} by the cyclic permutation of the indices. It can be shown that the eigenstrain components in (24.17) do not introduce any stress field in D .

We conclude, therefore, that the eigenstrain (24.17) is impotent.

If the applied stress is the pure torsion around the x_3 -axis, i.e. $\sigma_{23}^0 = x_1$ and $\sigma_{13}^0 = -x_2$, it is easily verified from (24.13) that the following vector is the particular solution of equation (24.10):

$$\check{A}_{132} = -\frac{5}{4}, \quad \check{A}_{231} = \frac{5}{4}, \quad (24.18)$$

with all other components equal to zero.

We have considered a rather special case by choosing a sphere and an isotropic material. We can generalize the results for cases where the shape of the inclusion and the anisotropy of the material are completely arbitrary. The preceding example of an impotent inclusion suggests the following generalization.

Suppose that eigenstrain components ϵ_{ij}^* in Ω are derived from functions u_i^* by

$$\epsilon_{ij}^* = \frac{1}{2}(u_{i,j}^* + u_{j,i}^*). \quad (24.19)$$

It is assumed that u_i^* are continuous in D and sufficiently smooth in the interior of Ω . Then, if $u_i^* = 0$ on the boundary $|\Omega|$ of Ω , the stress components σ_{ij} vanish everywhere in D and this eigenstrain is impotent (Ω is an impotent inclusion).

Before considering the analytical proof of this theorem, we make an intuitive observation: Since the boundary displacement is zero, the remainder $D - \Omega$ is not disturbed by Ω ; hence, the displacement and stress in $D - \Omega$ vanish. On the other hand, the eigenstrain defined by (24.19) is compatible in Ω .

The analytical proof is as follows: When ϵ_{ij}^* is given by (24.19), the use of Gauss' theorem leads to

$$\begin{aligned} & \int_{\Omega} G_{mk,l}(x - x') C_{kldr} u_{r,p}^*(x') dx' \\ &= \frac{\partial}{\partial x_l} \int_{|\Omega|} C_{kldr} G_{mk}(x - x') u_r^*(x') n_p(x') ds \\ &+ C_{kldr} \frac{\partial^2}{\partial x_p \partial x_l} \int_{\Omega} G_{mk}(x - x') u_r^*(x') dx' \end{aligned} \quad (24.20)$$

where $n_p(x')$ stands for the outward normal at the point x' on $|\Omega|$. Because $u_i^* = 0$ on $|\Omega|$ and because of the definition (5.2) of Green's function G_{mk} , (24.20) becomes

$$\int_{\Omega} G_{mk,l}(x - x') C_{klr_p} u_{r,p}^*(x') dx' = \begin{cases} -u_m^*(x), & x \in \Omega, \\ 0, & x \in D - \Omega. \end{cases} \quad (24.21)$$

Hence, (24.6) leads to $\sigma_{ij} = 0$ everywhere.

When the boundary of Ω is expressed by an implicit function $f(x) = 0$, we construct a function u_i^* explicitly. Assume that f is sufficiently smooth. Define the function \hat{f} as

$$\hat{f}(x) = \begin{cases} f(x), & x \in \Omega, \\ 0, & x \in D - \Omega. \end{cases} \quad (24.22)$$

Let g_i be a sufficiently smooth arbitrary function and set

$$u_i^*(x) = g_i(x)\hat{f}(x). \quad (24.23)$$

$u_i^*(x)$ is continuous in D but, in general, is not differentiable on the boundary of Ω . If Ω is an ellipsoid defined by equation (11.2), then the function u_i^* has the form

$$u_i^*(x) = \begin{cases} g_i(x) \left(\frac{x_1^2}{a_1^2} + \frac{x_2^2}{a_2^2} + \frac{x_3^2}{a_3^2} - 1 \right), & x \in \Omega, \\ 0, & x \in D - \Omega, \end{cases} \quad (24.24)$$

where g_i is an arbitrary smooth function. Since g_i is arbitrary, we can construct an infinite number of impotent eigenstrains from (24.23) and (24.19).

If ϵ_{ij}^* is a polynomial eigenstrain of degree one, the function u_i^* corresponding to this eigenstrain is a polynomial of degree two and can be expressed as

$$u_i^*(x) = g_i(a_{jk}x_j x_k + a_j x_j + a_0), \quad (24.25)$$

where a_{jk} , a_j , a_0 , and g_i are constants. The eigenstrain derived from this function is impotent if, and only if, the domain Ω is bounded by the quadratic surface

$$a_{jk}x_j x_k + a_j x_j + a_0 = 0. \quad (24.26)$$

It is well known that there is no bounded quadratic surface except the ellipsoidal surface. Hence, if ϵ_{ij}^* is a polynomial impotent eigenstrain of degree one in a bounded domain Ω , then Ω is an ellipsoid.

Similarly, if ϵ_{ij}^* is a uniform impotent eigenstrain in a domain Ω , then Ω is a half-space.

There are not only smooth impotent eigenstrains, but also discontinuous ones. Let $\Omega_1, \dots, \Omega_J$ be subsets of D . Suppose that u_i^{*p} is continuous in D , equal to zero outside Ω_p , and sufficiently smooth in the interior of Ω_p . Since the eigenstrains ϵ_{ij}^{*p} derived from these functions are impotent in Ω_p , the eigenstrain $\epsilon_{ij}^* = \epsilon_{ij}^{*1} + \dots + \epsilon_{ij}^{*J}$ is also impotent in the union of $\Omega_1, \dots, \Omega_J$. If the interior of intersection $\Omega_r \cap \Omega_q$ for integers $r \neq q$ is not empty, this eigenstrain is discontinuous on the boundary of $\Omega_r \cap \Omega_q$. For example, if we define the function u_i^{*p} as

$$u_i^{*p}(x) = \begin{cases} \delta_{i1}(|px|^2 - 1), & |x| \leq \frac{1}{p}, \\ 0, & |x| > \frac{1}{p}, \end{cases} \quad (24.27)$$

then the impotent eigenstrain derived from this function has $J - 1$ discontinuous surfaces in the unit sphere $|x| < 1$.

25. Energies of inhomogeneities

The elastic strain energy, potential energy, interaction energy, and the Gibbs free energy will be discussed for a body which contains inhomogeneities.

Elastic strain energy

First we consider the case of a body D containing inhomogeneous inclusions and free from any external force or surface constraint. A given eigenstrain in Ω is denoted by ϵ_{ij}^p , where Ω is the subdomain of D occupied by the inhomogeneous inclusions. The (elastic) strain energy is the same as (13.1),

$$W^* = \frac{1}{2} \int_D \sigma_{ij} e_{ij} \, dD, \quad (25.1)$$

where $e_{ij} = \epsilon_{ij} - \epsilon_{ij}^p$ and $\epsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})$. Integrating by parts, we obtain from (25.1)

$$W^* = -\frac{1}{2} \int_{\Omega} \sigma_{ij} \epsilon_{ij}^p \, dD. \quad (25.2)$$

If Ω is an ellipsoid and ϵ_{ij}^p is uniform, (25.2) becomes

$$W^* = -\frac{1}{2} V \sigma_{ij} \epsilon_{ij}^p, \quad (25.3)$$

where V is the volume of Ω and σ_{ij} is given by (22.13.1).

If the elastic moduli C_{ijkl}^* in Ω are slightly different from C_{ijkl} of the matrix, (25.2) is approximated by the quantities defined in a homogeneous inclusion problem in which Ω has the elastic moduli C_{ijkl} and eigenstrain ϵ_{ij}^p . The displacement, elastic strain, stress, and elastic strain energy for the homogeneous inclusion problem are denoted by u'_i , e'_{ij} , σ'_{ij} and W' , respectively. Then, (13.3) is written as

$$\begin{aligned} W' &= -\frac{1}{2} \int_{\Omega} \sigma'_{ij} \epsilon_{ij}^p \, dD \\ &= -\frac{1}{2} \int_D \sigma'_{ij} (\epsilon_{ij}^p - u_{i,j} + u_{i,j}) \, dD, \end{aligned} \quad (25.3.1)$$

where $u_{i,j}$ is artificially subtracted and added, and the integral domain Ω is extended to D since $\epsilon_{ij}^p = 0$ in $D - \Omega$. u_i is the displacement in the inhomogeneous inclusion problem leading to (25.2). Since $\epsilon_{ij}^p - \frac{1}{2}(u_{i,j} + u_{j,i}) = -e_{ij}$ and $\int_D \sigma'_{ij} u_{i,j} \, dD = 0$,

$$W' = \frac{1}{2} \int_D \sigma'_{ij} e'_{ij} \, dD. \quad (25.3.2)$$

Similarly, (25.2) is rewritten as

$$W^* = \frac{1}{2} \int_D \sigma_{ij} e'_{ij} \, dD \quad (25.3.3)$$

by the use of relations $\epsilon_{ij}^p = \frac{1}{2}(u'_{i,j} + u'_{j,i}) - e'_{ij}$ and $\int_D \sigma_{ij} u'_{i,j} \, dD = 0$. Since

$$\sigma'_{ij} e_{ij} = \sigma_{ij} e'_{ij} \text{ in } D - \Omega,$$

$$W^* - W' = \frac{1}{2} \left(\int_{\Omega} \sigma_{ij} e'_{ij} \, dD - \int_{\Omega} \sigma'_{ij} e_{ij} \, dD \right). \quad (25.3.4)$$

In Ω ,

$$\begin{aligned} \sigma_{ij} &= C_{ijkl}^* e_{kl} = (C_{ijkl} + \Delta C_{ijkl}) e_{kl}, \\ \sigma'_{ij} &= C_{ijkl} e'_{kl}, \end{aligned} \quad (25.3.5)$$

where $\Delta C_{ijkl} = C_{ijkl}^* - C_{ijkl}$. Then

$$W^* - W' = \frac{1}{2} \int_{\Omega} \Delta C_{ijkl} e_{kl} e'_{kl} \, dD. \quad (25.3.6)$$

When $e_{kl} = e'_{kl} + \Delta e_{kl}$, and ΔC_{ijkl} and Δe_{kl} are small quantities with respect to C_{ijkl} and e_{kl} , (25.3.6) is approximated to

$$W^* - W' = \frac{1}{2} \int_{\Omega} \Delta C_{ijkl} e'_{kl} e'_{ij} \, dD \quad (25.3.7)$$

which was first proposed by Eshelby (1966). Since quantities e'_{ij} , σ'_{ij} and W' can easily be obtained from the homogeneous problem, (25.3.7) can be used for an approximation of W^* for the inhomogeneous inclusion problem. In the above equation, Ω can be the sum of many inhomogeneous inclusions, i.e. $\Omega = \Omega_1 + \Omega_2 + \dots$

If Ω consists of two ellipsoids, Ω_1 and Ω_2 , W^* has the same form as (25.2), $\Omega = \Omega_1 + \Omega_2$, and $\sigma_{ij} = \sigma_{ij}^{(1)} + \sigma_{ij}^{(2)}$, where $\sigma_{ij}^{(m)}$ is the stress caused by ϵ_{ij}^p in Ω_m . The interaction energy between Ω_1 and Ω_2 is $-\int_{\Omega_1} \sigma_{ij}^{(2)} \epsilon_{ij}^p \, dD$ and which is equal to $-\int_{\Omega_2} \sigma_{ij}^{(1)} \epsilon_{ij}^p \, dD$.

Next, consider the case when a body D , containing an inhomogeneous inclusion Ω with eigenstrain ϵ_{ij}^p , is subjected to an external surface force F_i . The displacement field is denoted by $u_i^0 + u_i$ and the stress field by $\sigma_{ij}^0 + \sigma_{ij}$. u_i^0 and σ_{ij}^0 are the displacement and stress when F_i acts in the absence of Ω . Then

$$W^* = \frac{1}{2} \int_D (\sigma_{ij}^0 + \sigma_{ij})(u_{i,j}^0 + u_{i,j} - \epsilon_{ij}^p) \, dD, \quad (25.4)$$

where

$$\sigma_{ij}^0 = C_{ijkl} u_{k,l}^0, \quad \sigma_{ij}^0 + \sigma_{ij} = C_{ijkl}^* (u_{k,l}^0 + u_{k,l} - \epsilon_{kl}^p). \quad (25.5)$$

Here C_{ijkl} and C_{ijkl}^* are the respective elastic moduli of the matrix and inhomogeneous inclusion.

The expression for W^* given in (25.4) may be further simplified. An equation similar to (13.6) holds,

$$\int_D \sigma_{ij} (u_{i,j}^0 + u_{i,j}) dD = 0. \quad (25.6)$$

Furthermore, we have

$$\begin{aligned} \sigma_{ij}^0 (u_{i,j} - \epsilon_{ij}^p) &= C_{ijkl} u_{k,l}^0 (u_{i,j} - \epsilon_{ij}^p) \\ &= C_{ijkl} u_{k,l}^0 (u_{i,j} - \epsilon_{ij}^p - \epsilon_{ij}^* + \epsilon_{ij}^*) \\ &= u_{k,l}^0 \sigma_{kl} + C_{ijkl} u_{k,l}^0 \epsilon_{ij}^* \\ &= \sigma_{ij} u_{i,j}^0 + \sigma_{ij}^0 \epsilon_{ij}^*, \end{aligned} \quad (25.7)$$

where ϵ_{ij}^* is the fictitious eigenstrain introduced in the equivalent inclusion method and

$$\sigma_{ij} = C_{ijkl} (u_{k,l} - \epsilon_{kl}^p - \epsilon_{kl}^*) \text{ in } \Omega. \quad (25.8)$$

Thus, we have

$$\int_D \sigma_{ij}^0 (u_{i,j} - \epsilon_{ij}^p) dD = \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* dD, \quad (25.9)$$

since $\int_D \sigma_{ij} u_{i,j}^0 dD = 0$. The elastic strain energy, therefore, becomes

$$W^* = \frac{1}{2} \int_D \sigma_{ij}^0 u_{i,j}^0 dD + \frac{1}{2} \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* dD - \frac{1}{2} \int_{\Omega} \sigma_{ij} \epsilon_{ij}^p dD. \quad (25.10)$$

If Ω is an inhomogeneity ($\epsilon_{ij}^p = 0$), the elastic strain energy is

$$W^* = \frac{1}{2} \int_D \sigma_{ij}^0 u_{i,j}^0 dD + \frac{1}{2} \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* dD. \quad (25.11)$$

On the other hand, when the applied stress σ_{ij}^0 is absent and Ω is an inhomogeneous inclusion with eigenstrain ϵ_{ij}^p , the elastic strain energy becomes

$$W^* = -\frac{1}{2} \int_{\Omega} \sigma_{ij} \epsilon_{ij}^p \, dD. \quad (25.11.1)$$

Therefore, the elastic strain energy (25.10) for the case when σ_{ij}^0 and ϵ_{ij}^p coexist is the sum of (25.11) and (25.11.1) because of Colonnetti's theorem (1921) or (25.28); namely

$$W^* = \frac{1}{2} \int_D \sigma_{ij}^0 u_{i,j}^0 \, dD + \frac{1}{2} \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* \, dD - \frac{1}{2} \int_{\Omega} \sigma_{ij} \epsilon_{ij}^p \, dD \quad (25.11.2)$$

The above expression has the same form as (25.10), but ϵ_{ij}^* and σ_{ij} in the two expressions are defined differently. ϵ_{ij}^* in (25.11.2) is the solution of (22.7), and σ_{ij} in (25.11.2) is the stress obtained from (22.13.1).

Interaction energy

When a body D contains an inhomogeneity Ω and is subjected to F_i on S , the total potential energy of the body (the Gibbs free energy) is defined by the sum of the elastic strain energy and the potential energy of F_i ,

$$\begin{aligned} W &= \frac{1}{2} \int_D (\sigma_{ij}^0 + \sigma_{ij}) (u_{i,j}^0 + u_{i,j}) \, dD - \int_S F_i (u_i^0 + u_i) \, dS \\ &= \frac{1}{2} \int_D \sigma_{ij}^0 u_{i,j}^0 \, dD + \frac{1}{2} \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* \, dD - \int_S F_i (u_i^0 + u_i) \, dS, \end{aligned} \quad (25.12)$$

from (25.11).

If the material is homogeneous, W becomes

$$W_0 = \frac{1}{2} \int_D \sigma_{ij}^0 u_{i,j}^0 \, dD - \int_S F_i u_i^0 \, dS. \quad (25.13)$$

The interaction energy between F_i and the inhomogeneity is defined by

$$\Delta W = W - W_0 = \frac{1}{2} \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* \, dD - \int_S F_i u_i \, dS. \quad (25.14)$$

It holds that

$$\begin{aligned} \int_S F_i u_i \, dS &= \int_S \sigma_{ij}^0 n_j u_i \, dS = \int_D \sigma_{ij}^0 u_{i,j} \, dD \\ &= \int_D \sigma_{ij}^0 (u_{i,j} - \epsilon_{ij}^*) \, dD + \int_D \sigma_{ij}^0 \epsilon_{ij}^* \, dD \end{aligned} \quad (25.15)$$

and

$$\int_D \sigma_{ij}^0 (u_{i,j} - \epsilon_{ij}^*) \, dD = \int_D C_{ijkl} u_{k,l}^0 (u_{i,j} - \epsilon_{ij}^*) \, dD = \int_D u_{k,l}^0 \sigma_{kl} \, dD = 0. \quad (25.16)$$

Therefore, we have

$$\Delta W = -\frac{1}{2} \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* \, dD. \quad (25.17)$$

If σ_{ij}^0 is uniform and Ω is an ellipsoidal inhomogeneity, ϵ_{ij}^* also becomes uniform and

$$\Delta W = -\frac{1}{2} V \sigma_{ij}^0 \epsilon_{ij}^*, \quad (25.18)$$

where V is the volume of Ω .

Let us consider ΔW when Ω is a crack. As shown in Chapter 5, (25.18) is used for the derivation of Griffith's fracture criterion, where $V \epsilon_{ij}^*$ is kept constant when $V \rightarrow 0$ and $\epsilon_{ij}^* \rightarrow \infty$. For later discussions on the crack opening displacement and crack growth rate, it is useful to derive an alternative expression for ΔW here.

When (25.13) is subtracted from the first expression in (25.12),

$$\begin{aligned} \Delta W &= \frac{1}{2} \int_D \sigma_{ij}^0 u_{i,j} \, dD + \frac{1}{2} \int_D \sigma_{ij}^0 u_{i,j}^0 \, dD \\ &\quad + \frac{1}{2} \int_D \sigma_{ij} u_{i,j} \, dD - \int_S \sigma_{ij}^0 n_j u_i \, dS. \end{aligned} \quad (25.18.1)$$

In D , $\sigma_{ij}^0 u_{i,j} = \sigma_{ij} u_{i,j}^0$ since domain Ω has no volume. Ω has only surface Σ where u_i is discontinuous. It holds that

$$\int_D \sigma_{ij} u_{i,j}^0 \, dD = \int_S \sigma_{ij} n_j u_i^0 \, dS - \int_D \sigma_{ij,j} u_i^0 \, dD = 0 \quad (25.18.2)$$

because u_i^0 is continuous on Σ , and

$$\begin{aligned} \int_S \sigma_{ij}^0 n_j u_i \, dS &= \int_D \sigma_{ij,j}^0 u_i \, dD + \int_D \sigma_{ij}^0 u_{i,j} \, dD - \int_\Sigma \sigma_{ij}^0 n_j [u_i] \, dS \\ &= - \int_\Sigma \sigma_{ij}^0 n_j [u_i] \, dS, \end{aligned} \quad (25.18.3)$$

where n_j is the outward normal to the upper surface of the crack ($\mathbf{n} = (0, 0, -1)$ when the crack is on the x_1x_2 -plane) and $[u_i]$ is the crack opening displacement, i.e.

$$\Delta u_i = [u_i] = u_i(\text{upper}) - u_i(\text{lower}). \quad (25.18.4)$$

We have also

$$\begin{aligned} \int_D \sigma_{ij} u_{i,j} \, dD &= \int_S \sigma_{ij} n_j u_i \, dS + \int_\Sigma \sigma_{ij} n_j [u_i] \, dS - \int_D \sigma_{ij,j} u_i \, dD \\ &= \int_\Sigma \sigma_{ij} n_j [u_i] \, dS = - \int_\Sigma \sigma_{ij}^0 n_j [u_i] \, dS \end{aligned} \quad (25.18.5)$$

since $(\sigma_{ij}^0 + \sigma_{ij}) n_j = 0$ on Σ . Finally, therefore, we have

$$\Delta W = \frac{1}{2} \int_\Sigma \sigma_{ij}^0 n_j [u_i] \, dS \quad (25.18.6)$$

where Σ is the crack surface.

When a body D contains an inhomogeneous inclusion Ω with eigenstrain ϵ_{ij}^p , and it is subjected to F_i , the Gibbs free energy of the body is

$$W = \frac{1}{2} \int_D (\sigma_{ij}^0 + \sigma_{ij}) (u_{i,j}^0 + u_{i,j} - \epsilon_{ij}^p) \, dD - \int_S F_i (u_i^0 + u_i) \, dS. \quad (25.19)$$

This total potential energy is compared with W_0 as defined by (25.13). The difference between W and W_0 is

$$\Delta W = W - W_0. \quad (25.20)$$

Since the first term on the right-hand side in (25.19) can be expressed by (25.10), we have

$$\Delta W = \frac{1}{2} \int_\Omega \sigma_{ij}^0 \epsilon_{ij}^* \, dD - \frac{1}{2} \int_\Omega \sigma_{ij} \epsilon_{ij}^p \, dD - \int_S F_i u_i \, dS. \quad (25.21)$$

It also holds that

$$\begin{aligned} \int_S F_i u_i \, dS &= \int_S \sigma_{ij}^0 n_j u_i \, dS = \int_D \sigma_{ij}^0 u_{i,j} \, dD \\ &= \int_D \sigma_{ij}^0 (u_{i,j} - \epsilon_{ij}^p - \epsilon_{ij}^*) \, dD + \int_D \sigma_{ij}^0 (\epsilon_{ij}^p + \epsilon_{ij}^*) \, dD \end{aligned} \quad (25.22)$$

and

$$\begin{aligned} \int_D \sigma_{ij}^0 (u_{i,j} - \epsilon_{ij}^p - \epsilon_{ij}^*) \, dD &= \int_D C_{ijkl} u_{k,l}^0 (u_{i,j} - \epsilon_{ij}^p - \epsilon_{ij}^*) \, dD \\ &= \int_D \sigma_{kl} u_{k,l}^0 \, dD = 0. \end{aligned} \quad (25.23)$$

Therefore, we have

$$\Delta W = -\frac{1}{2} \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* \, dD - \frac{1}{2} \int_{\Omega} \sigma_{ij} \epsilon_{ij}^p \, dD - \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^p \, dD. \quad (25.24)$$

Colonnetti's theorem

The variation of the Gibbs free energy associated with a change in a given physical system plays an important role in certain cases, as shown in Chapter 7.

Suppose that a body D containing inhomogeneities Ω is subjected to an applied force F_i on the boundary S of D . The Gibbs free energy of the body is expressed by (25.12) or

$$W = W^* + V^*, \quad (25.25)$$

with

$$\begin{aligned} W^* &= \frac{1}{2} \int_D \sigma_{ij}^A e_{ij}^A \, dD, \\ V^* &= - \int_S F_i u_i^A \, dS, \end{aligned} \quad (25.26)$$

where $\sigma_{ij}^A = \sigma_{ij}^0 + \sigma_{ij}$, $e_{ij}^A = \frac{1}{2}(u_{i,j}^A + u_{j,i}^A)$, and $u_i^A = u_i^0 + u_i$. e_{ij}^A is the elastic strain, W^* the elastic strain energy, and V^* is the potential energy of F_i . If the

inhomogeneities in D are further subjected to the eigenstrain ϵ_{ij}^p , the Gibbs free energy of the body changes to (25.19). The new stress, elastic strain, and displacement become $\sigma_{ij}^A + \sigma_{ij}^B$, $e_{ij}^A + e_{ij}^B$, and $u_i^A + u_i^B$, where σ_{ij}^B , e_{ij}^B , and u_i^B are increments (changes) in the fields due to ϵ_{ij}^p . The Gibbs free energy of the body (25.19) can now be written as

$$W = W^* + V^* \quad (25.27)$$

with

$$\begin{aligned} W^* &= \frac{1}{2} \int_D \sigma_{ij}^A e_{ij}^A \, dD + \frac{1}{2} \int_D \sigma_{ij}^B e_{ij}^B \, dD, \\ V^* &= - \int_S F_i u_i^A \, dS - \int_S F_i u_i^B \, dS. \end{aligned} \quad (25.28)$$

This result has been obtained from Colonnelli's theorem (1921) as shown below. The elastic strain energy is

$$\begin{aligned} W^* &= \frac{1}{2} \int_D (\sigma_{ij}^A + \sigma_{ij}^B)(e_{ij}^A + e_{ij}^B) \, dD \\ &= \frac{1}{2} \int_D \sigma_{ij}^A e_{ij}^A \, dD + \frac{1}{2} \int_D \sigma_{ij}^B e_{ij}^B \, dD + \int_D \sigma_{ij}^B e_{ij}^A \, dD, \end{aligned} \quad (25.29)$$

since $\sigma_{ij}^A e_{ij}^B = C_{ijkl}^{**} e_{kl}^A e_{ij}^B = \sigma_{kl}^B e_{kl}^A$, where C_{ijkl}^{**} is C_{ijkl} in $D - \Omega$ and C_{ijkl}^* in Ω . On the other hand,

$$\int_D \sigma_{ij}^B e_{ij}^A \, dD = \int_D \sigma_{ij}^B u_{i,j}^A \, dD = \int_S \sigma_{ij}^B n_j u_i^A \, dS - \int_D \sigma_{i,j,j}^B u_i^A \, dD = 0, \quad (25.30)$$

since $\sigma_{ij}^B n_j = 0$ on S and $\sigma_{i,j,j}^B = 0$ in D . Equation (25.30) is Colonnelli's theorem. The variations of W^* and V^* associated with the change from the condition corresponding to (25.26) to the condition corresponding to (25.28), are

$$\Delta W^* = \frac{1}{2} \int_D \sigma_{ij}^B e_{ij}^B \, dD = \frac{1}{2} \int_D \sigma_{ij}^B (u_{i,j}^B - \epsilon_{ij}^p) \, dD = -\frac{1}{2} \int_D \sigma_{ij}^B \epsilon_{ij}^p \, dD, \quad (25.31)$$

$$\begin{aligned} \Delta V^* &= - \int_S F_i u_i^B \, dS = - \int_S \sigma_{ij}^A u_i^B n_j \, dS = - \int_D \sigma_{ij}^A u_{i,j}^B \, dD \\ &= - \int_D \sigma_{ij}^A (e_{ij}^B + \epsilon_{ij}^p) \, dD = - \int_D \sigma_{ij}^A \epsilon_{ij}^p \, dD \end{aligned} \quad (25.32)$$

or

$$\begin{aligned}\Delta V^* &= - \int_S \sigma_{ij}^0 u_i^B n_j \, dS = - \int_D \sigma_{ij}^0 u_{i,j}^B \, dD \\ &= - \int_D \sigma_{ij}^0 (u_{i,j}^B - \epsilon_{ij}^{**}) \, dD - \int_D \sigma_{ij}^0 \epsilon_{ij}^{**} \, dD,\end{aligned}\quad (25.33)$$

where ϵ_{ij}^{**} has been defined in (22.15). Since

$$\int_D \sigma_{ij}^0 (u_{i,j}^B - \epsilon_{ij}^{**}) \, dD = \int_D u_{i,j}^0 \sigma_{ij}^B \, dD = 0, \quad (25.34)$$

(25.33) leads to

$$\Delta V^* = - \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^{**} \, dD. \quad (25.35)$$

The terms $\frac{1}{2} \int_D \sigma_{ij}^A e_{ij}^A \, dD - \int_S F_i u_i^A \, dS$ have been calculated from (25.12). Thus, we have

$$\Delta W = W - W_0 = - \frac{1}{2} \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^{*A} \, dD - \frac{1}{2} \int_{\Omega} \sigma_{ij}^B \epsilon_{ij}^p \, dD - \int_{\Omega} \sigma_{ij}^A \epsilon_{ij}^p \, dD, \quad (25.35.1)$$

where W_0 is defined by (25.13) and ϵ_{ij}^{*A} is ϵ_{ij}^* in (25.17) or (22.5), which is the equivalent eigenstrain for the inhomogeneity under applied stress σ_{ij}^0 . The above expression (25.35.1) is an alternative expression of (25.24).

The expressions of the interaction energies derived so far are valid also for the interaction between an inhomogeneous inclusion Ω and a different internal stress. σ_{ij}^0 and u_i^0 in the previous expressions are interpreted as the internal stress and displacement caused by the other internal stress source.

Uniform plastic deformation in a matrix

As an application of Colonnelli's theorem, let us consider a composite material under an applied force F_i . The material D consists of inhomogeneities Ω and matrix $D - \Omega$. The elastic stress is denoted by σ_{ij}^A . Assume that the inhomogeneities are stronger than the matrix and that a uniform plastic strain ϵ_{ij}^p is distributed in the matrix. Due to the plastic strain misfit with the

inhomogeneities, the stress state changes from σ_{ij}^A to $\sigma_{ij}^A + \sigma_{ij}^B$. Then, the Gibbs free energy of the system becomes

$$W = W^* + V^* \quad (25.36)$$

with

$$\begin{aligned} W^* &= \frac{1}{2} \int_D \sigma_{ij}^A e_{ij}^A \, dD + \frac{1}{2} \int_D \sigma_{ij}^B e_{ij}^B \, dD, \\ V^* &= - \int_S F_i u_i^A \, dS - \int_S F_i u_i^B \, dS, \end{aligned} \quad (25.37)$$

where e_{ij}^A and e_{ij}^B are the elastic strains associated with σ_{ij}^A and σ_{ij}^B , and u_i^A and u_i^B are the corresponding displacements. For the discussion in Section 43 we will investigate the dependency of W on ϵ_{ij}^p . The first terms in (25.37) are independent of ϵ_{ij}^p . The second terms can be rewritten as follows:

$$\begin{aligned} \frac{1}{2} \int_D \sigma_{ij}^B e_{ij}^B \, dD &= \frac{1}{2} \int_D \sigma_{ij}^B (u_{i,j}^B - \epsilon_{ij}^p) \, dD = -\frac{1}{2} \int_{D-\Omega} \sigma_{ij}^B \epsilon_{ij}^p \, dD, \\ - \int_S F_i u_i^B \, dS &= - \int_S \sigma_{ij}^A u_i^B n_j \, dS = - \int_D \sigma_{ij}^A (e_{ij}^B + \epsilon_{ij}^p) \, dD \quad (25.38) \\ &= - \int_{D-\Omega} \sigma_{ij}^A \epsilon_{ij}^p \, dD. \end{aligned}$$

Since $\epsilon_{ij}^p = 0$ in Ω and ϵ_{ij}^p is uniform in $D - \Omega$, the integrals in (25.38) are actually defined in $D - \Omega$. However, it is convenient to extend the integral domain into D and subtract Ω from D . Then, since $\int_D \sigma_{ij}^B \, dD = 0$ (see (42.3)), we can write

$$\begin{aligned} -\frac{1}{2} \int_{D-\Omega} \sigma_{ij}^B \epsilon_{ij}^p \, dD &= -\frac{1}{2} \epsilon_{ij}^p \int_{D-\Omega} \sigma_{ij}^B \, dD \\ &= -\frac{1}{2} \epsilon_{ij}^p \left(\int_D \sigma_{ij}^B \, dD - \int_\Omega \sigma_{ij}^B \, dD \right) \\ &= \frac{1}{2} \epsilon_{ij}^p \int_\Omega \sigma_{ij}^B \, dD. \end{aligned} \quad (25.39)$$

Since $\sigma_{ij}^A = \sigma_{ij}^0 + \sigma_{ij}$ and $\int_D \sigma_{ij} dD = 0$,

$$\begin{aligned} -\int_{D-\Omega} \sigma_{ij}^A \epsilon_{ij}^p dD &= -\epsilon_{ij}^p \int_{D-\Omega} \sigma_{ij}^A dD = -\epsilon_{ij}^p \left(\int_D \sigma_{ij}^A dD - \int_\Omega \sigma_{ij}^A dD \right) \\ &= -\epsilon_{ij}^p \sigma_{ij}^0 \int_D dD + \epsilon_{ij}^p \int_\Omega \sigma_{ij}^A dD. \end{aligned} \quad (25.40)$$

Thus, the part of W which depends on ϵ_{ij}^p becomes

$$\Delta W = \frac{1}{2} \epsilon_{ij}^p \int_\Omega \sigma_{ij}^B dD - \epsilon_{ij}^p \sigma_{ij}^0 \int_D dD + \epsilon_{ij}^p \int_\Omega \sigma_{ij}^A dD. \quad (25.41)$$

σ_{ij}^B can be calculated as the eigenstress by assuming that Ω has eigenstrain $-\epsilon_{ij}^p$ and that the matrix has no plastic strain, since the misfit strain is a relative quantity between the matrix and the inclusions.

Energy balance

In order to understand the physical meaning of the Gibbs free energy change ΔW , we consider the energy dissipated when ϵ_{ij}^p in (25.27) or (25.36) is changed by $\delta \epsilon_{ij}^p$. The work done by applied force F_i during the change is

$$\int_S F_i \delta u_i^B dS, \quad (25.42)$$

where F_i is kept constant during the variation. The change of elastic strain energy is

$$\int_D (\sigma_{ij}^A + \sigma_{ij}^B) \delta e_{ij}^B dD, \quad (25.43)$$

since e_{ij}^A does not change during the variation. σ_{ij}^A is the stress due to σ_{ij}^0 and the inhomogeneity, and σ_{ij}^B is the stress caused by ϵ_{ij}^p and the inhomogeneity. The energy δQ dissipated during the variation is the difference between (25.42) and (25.43),

$$\delta Q = \int_S F_i \delta u_i^B dS - \int_D (\sigma_{ij}^A + \sigma_{ij}^B) \delta e_{ij}^B dD, \quad (25.44)$$

where

$$\frac{1}{2}(\delta u_{i,j}^B + \delta u_{j,i}^B) = \delta e_{ij}^B + \delta \epsilon_{ij}^P. \quad (25.45)$$

Thus, we have

$$\delta Q = -\delta(\Delta W) \quad (25.46)$$

which means that the change in the Gibbs free energy is minus the energy dissipated during the variation $\delta \epsilon_{ij}^P$.

Expression (25.44) can be rewritten as

$$\delta Q = \int_D (\sigma_{ij}^A + \sigma_{ij}^B) \delta \epsilon_{ij}^P \, dD, \quad (25.47)$$

in which (25.45), $(\sigma_{ij}^A + \sigma_{ij}^B)n_j = F_i$ on S , and $\sigma_{ij,j}^A = 0$ and $\sigma_{ij,j}^B = 0$ in D are used. Expression (25.47) can also be obtained from (25.46) and (25.35.1), since σ_{ij}^B in (25.35.1) is a linear function of ϵ_{ij}^P as seen from (22.13.1).

When ϵ_{ij}^P is defined uniformly only in the matrix, (25.47) becomes

$$\begin{aligned} \delta Q &= \int_{D-\Omega} (\sigma_{ij}^A + \sigma_{ij}^B) \delta \epsilon_{ij}^P \, dD \\ &= \delta \epsilon_{ij}^P \left\{ \int_D (\sigma_{ij}^A + \sigma_{ij}^B) \, dD - \int_\Omega (\sigma_{ij}^A + \sigma_{ij}^B) \, dD \right\}. \end{aligned} \quad (25.48)$$

From (42.3) and the definitions of σ_{ij}^A and σ_{ij}^B , we have

$$\int_D \sigma_{ij}^B \, dD = 0, \quad \int_D \sigma_{ij}^A \, dD = \int_D \sigma_{ij}^0 \, dD. \quad (25.49)$$

Therefore,

$$\delta Q = \delta \epsilon_{ij}^P \left\{ \int_D \sigma_{ij}^0 \, dD - \int_\Omega (\sigma_{ij}^A + \sigma_{ij}^B) \, dD \right\}, \quad (25.50)$$

which can also be obtained from (25.46) and (25.41). $\sigma_{ij}^A + \sigma_{ij}^B = \sigma_{ij}^0 + \sigma_{ij}$ can be calculated from (22.13), where ϵ_{ij}^P is taken as $-\epsilon_{ij}^P$ in Ω , since σ_{ij} is equivalent to the eigenstress (stress disturbance) caused by the misfit $-\epsilon_{ij}^P$ in Ω and the applied stress σ_{ij}^0 .

Let us consider an example of the simple tension σ_{33}^0 . It is assumed that the matrix is a perfectly plastic solid with the yield stress σ_y and that the inhomogeneities $\Omega = \sum_p \Omega_p$ are perfectly elastic. The plastic strain has no dilatation and $\epsilon_{11}^p = \epsilon_{22}^p = -\epsilon_{33}^p/2$. The energy dissipated during $\delta\epsilon_{ij}^p$ is

$$\delta Q = \int_{D-\Omega} \sigma_y \delta\epsilon_{33}^p \, dD. \quad (25.51)$$

Then, (25.50) becomes

$$\sigma_y \delta\epsilon_{33}^p \int_{D-\Omega} \, dD = \delta\epsilon_{33}^p \left\{ \sigma_{33}^0 \int_{D-\Omega} \, dD - \int_{\Omega} (\sigma_{33} - \frac{1}{2}\sigma_{11} - \frac{1}{2}\sigma_{22}) \, dD \right\}. \quad (25.52)$$

The last terms are calculated from (22.13), resulting in

$$\sigma_{33} - \frac{1}{2}(\sigma_{11} + \sigma_{22}) = A\epsilon_{33}^p + B\sigma_{33}^0, \quad (25.53)$$

where

$$A = E^*/2(1 - \nu^*), \quad B = (E^*\nu - E\nu^*)/E(1 - \nu^*), \quad (25.54)$$

for penny-shaped particles (the disk plane is perpendicular to the x_3 -axis);

$$A = \frac{\frac{3}{2}E^*E(7 - 5\nu)}{E^*(1 + \nu)(8 - 10\nu) + E(7 - 5\nu)(1 + \nu^*)}, \quad (25.55)$$

$$B = \frac{(7 - 5\nu)\{E^*(1 + \nu) - E(1 + \nu^*)\}}{E^*(1 + \nu)(8 - 10\nu) + E(7 - 5\nu)(1 + \nu^*)},$$

for spherical particles; and

$$A = \frac{\frac{1}{2}E^*\{3E(1 - 2\nu^*) + 2E^*(1 + \nu)\}}{E^*(1 + \nu) + E(1 + \nu^*)(1 - 2\nu^*)}, \quad (25.56)$$

$$B = \frac{(E^*)^2(1 + \nu)/E - 2E^*\nu^*(1 + \nu) - E(1 + \nu^*)(1 - 2\nu^*)}{E^*(1 + \nu) + E(1 + \nu^*)(1 - 2\nu^*)},$$

for rod-shaped particles (the axis of the rod is parallel to the x_3 -axis). The values of A and B for anisotropic media have been calculated by Lin et al. (1973). When (25.53) is substituted into (25.52), we have

$$(1 - f)\sigma_y = (1 - f)\sigma_{33}^0 - f(A\epsilon_{33}^p + B\sigma_{33}^0) \quad (25.57)$$

or

$$\sigma_{33}^0 = \frac{fA\epsilon_{33}^P}{1-f(1+B)} + \frac{(1-f)}{1-f(1+B)}\sigma_y, \quad (25.58)$$

where f is the volume fraction of the particles. Equation (25.58) has been obtained by Tanaka and Mori (1970). The stress-strain relation of the composite material displays linear work-hardening as shown in Fig. 43.1. The details are presented in Section 43. The yield point also increases since $(1-f)/\{1-(1+B)f\} > 1$. It decreases, however, when $E^* < E$ and $\nu^* \approx \nu$.

Let us consider a creep problem under a constant applied stress σ_{33}^0 . If we assume that the matrix behaves as a Newtonian viscous solid (the stress is $\tilde{\mu} d\epsilon_{33}^P/dt$), δQ in (25.50) can be written as

$$\delta Q = (1-f)\tilde{\mu}\dot{\epsilon}_{33}^P\delta\epsilon_{33}^P; \quad (25.59)$$

then (25.50) becomes

$$(1-f)\tilde{\mu}\dot{\epsilon}_{33}^P = (1-f)\sigma_{33}^0 - f(A\epsilon_{33}^P + B\sigma_{33}^0). \quad (25.60)$$

The above differential equation has the solution

$$\epsilon_{33}^P = \frac{1-(1+B)f}{Af}\sigma_{33}^0[1 - \exp\{-fAt/(1-f)\tilde{\mu}\}] \quad (25.61)$$

when the initial condition $\epsilon_{33}^P = 0$ at $t = 0$ is used. The composite material, as a whole, behaves like Kelvin's material (Mura 1972). Cyclic creep problems have been investigated by Mura, Novakovic, and Meshii (1975) and Shetty, Mura, and Meshii (1975) by using equation (25.50).

26. Precipitates and martensites

Precipitates and martensites in alloys are typical examples of inhomogeneous inclusions. The elastic strain energy caused by these particular inclusions will be discussed in this section. If interfacial energy can be neglected, the morphology of precipitates and martensites can be determined by the condition that the elastic strain energy (25.2) take a minimum value. σ_{ij} in (25.2) or (25.3) can be evaluated by the equivalent inclusion method described by (22.13), where $\epsilon_{ij}^0 = 0$ and $\sigma_{ij}^0 = 0$, since no applied load is considered here.

Isotropic precipitates

Consider an infinite isotropic matrix with shear modulus μ and Poisson's ratio ν , containing an ellipsoidal precipitate whose elastic constants are μ^* and ν^* . Barnett et al. (1974) calculated the elastic strain energy per unit volume of the precipitate when

$$\epsilon_{ij}^p = \delta_{ij}\epsilon, \quad a_1 = a_2 = a, \quad 0 \leq \beta = a_3/a \leq \infty, \quad (26.1)$$

where ϵ is a constant misfit strain. Their results are

$$W^*/V = 3\epsilon^2\gamma^* \left[1 - \left\{ 1 + \frac{\gamma}{\gamma^*} \left(\frac{2K+1}{K+M-KM} \frac{1-\nu}{1+\nu} - 1 \right) \right\}^{-1} \right], \quad (26.2)$$

where

$$\gamma = \mu(1+\nu)/(1-2\nu), \quad \gamma^* = \mu^*(1+\nu^*)/(1-2\nu^*),$$

$$M = 1/(1-\beta^2) - \beta(1-\beta^2)^{-3/2} \cos^{-1}\beta, \quad \beta < 1 \\ = 1/3, \quad \beta = 1 \quad (26.3)$$

$$= 1/(1-\beta^2) + \beta(\beta^2-1)^{-3/2} \cosh^{-1}\beta, \quad \beta > 1,$$

$$K = \left(1 + (\mu^*/\mu - 1) \{ M(5-4\nu) - 3(3M-1)/2(1-\beta^2) \} / 2(1-\nu) \right) \\ \times \left(1 + (\mu^*/\mu - 1) \{ (1-2\nu) + 2M(1+\nu) \right. \\ \left. - 3(3M-1)/2(1-\beta^2) \} / 2(1-\nu) \right)^{-1}.$$

When $\mu^* = \mu$ and $\nu^* = \nu$, (26.2) reduces to (13.21),

$$W_0/V = 2\mu\epsilon^2(1+\nu)/(1-\nu). \quad (26.4)$$

The normalized strain energy W^*/W_0 is plotted in Fig. 26.1 against β for several values of μ^*/μ . Barnett et al. (1974) concluded that the minimum

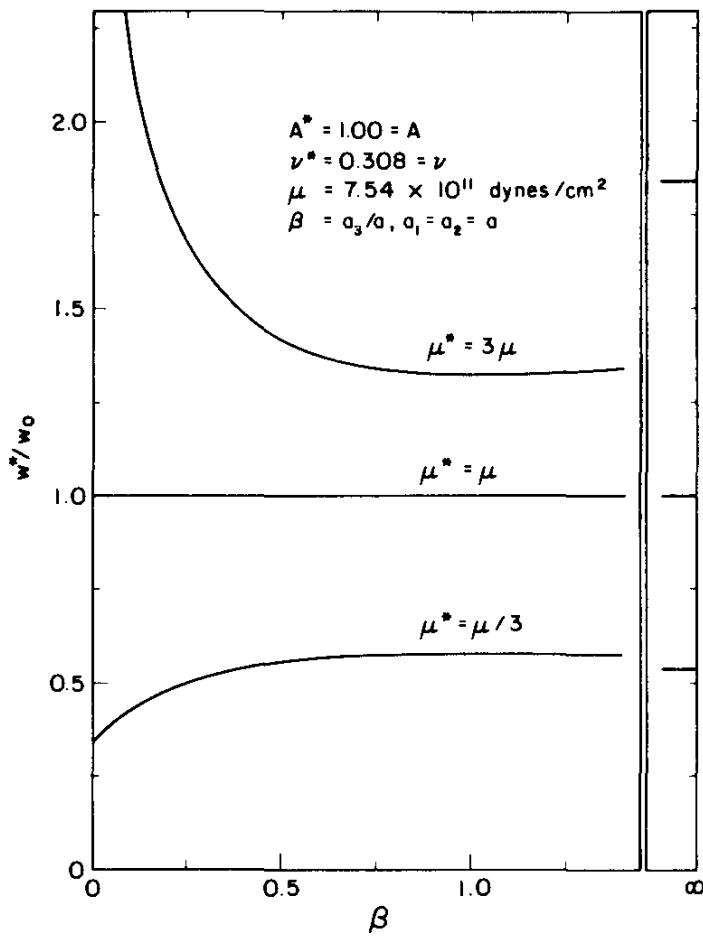


Fig. 26.1 Normalized strain energy of an isotropic coherent ellipsoidal precipitate in an isotropic matrix vs aspect ratios. A , ν , and μ are the anisotropy ratio, Poisson's ratio, and shear modulus, respectively

strain energy is achieved by a flat ellipsoid when $\mu^* < \mu$, and by a sphere when $\mu^* > \mu$. The same conclusion has been stated by László (1950).

Anisotropic precipitates

Here we calculate the elastic strain energy when an infinite anisotropic matrix contains an ellipsoidal anisotropic precipitate. The precipitate is an inhomogeneous inclusion with eigenstrain ϵ_{ij}^p . The dependency of the results on the orientation and shape of the precipitates is discussed.

Let us denote the elastic moduli of the matrix by C_{ijkl} and of the precipitate by C_{ijkl}^* . The x_i coordinate system is taken along the crystallographic directions of the matrix ([100], [010], [001]). The orientation of the precipitate is characterized by θ and ϕ (see Fig. 26.2).

Mori et al. (1978) have obtained contour lines of the elastic strain energy for a needle-shaped precipitate (Fig. 26.3) and those for a disk-shaped pre-

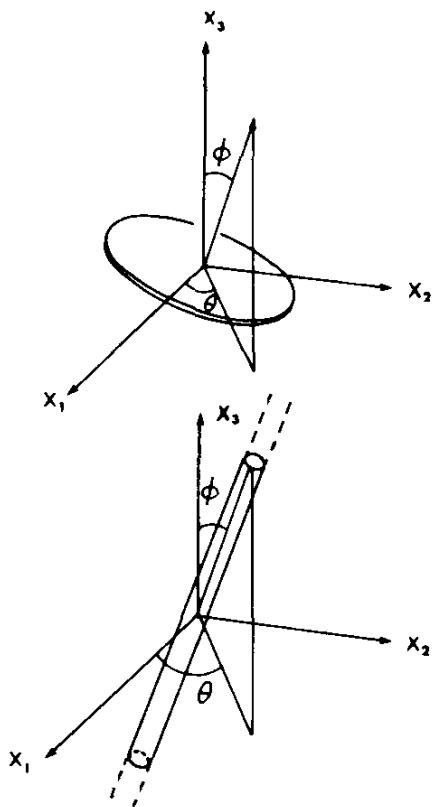


Fig. 26.2 Orientations of precipitates with respect to crystallographic directions x_i

cipitate (Fig. 26.4), when the matrix is copper ($C_{11} = 16.84 \times 10^{10}$ N/m 2 , $C_{12} = 12.14 \times 10^{10}$ N/m 2 , $C_{44} = 7.54 \times 10^{10}$ N/m 2) and

$$\epsilon_{ij}^p = \delta_{ij}\epsilon, \quad C_{ij}^* = fC_{ij}. \quad (26.5)$$

These conditions assume that the phase transformation strain is dilatational

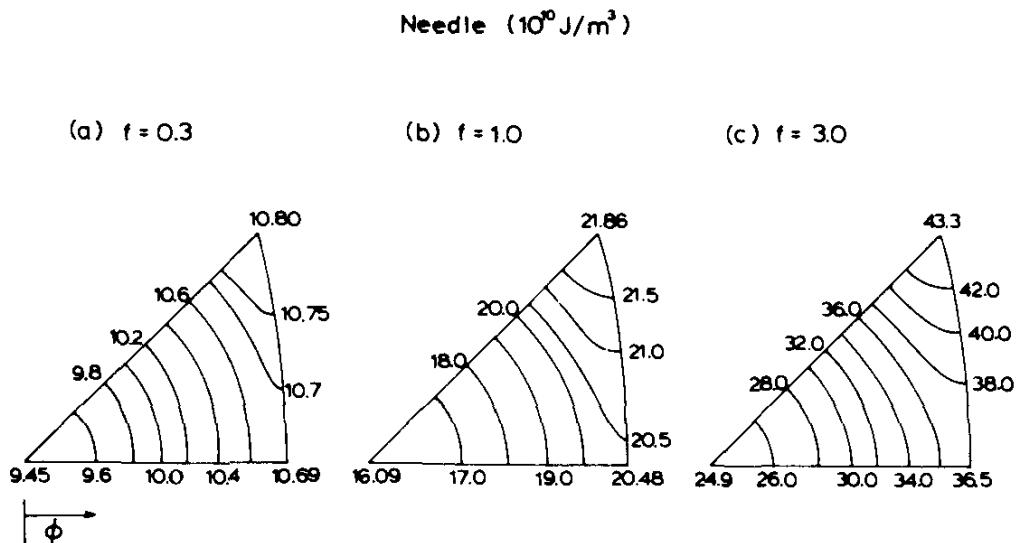


Fig. 26.3. Contour lines of energy; the matrix is copper and $C_{ij}^* = fC_{ij}$.

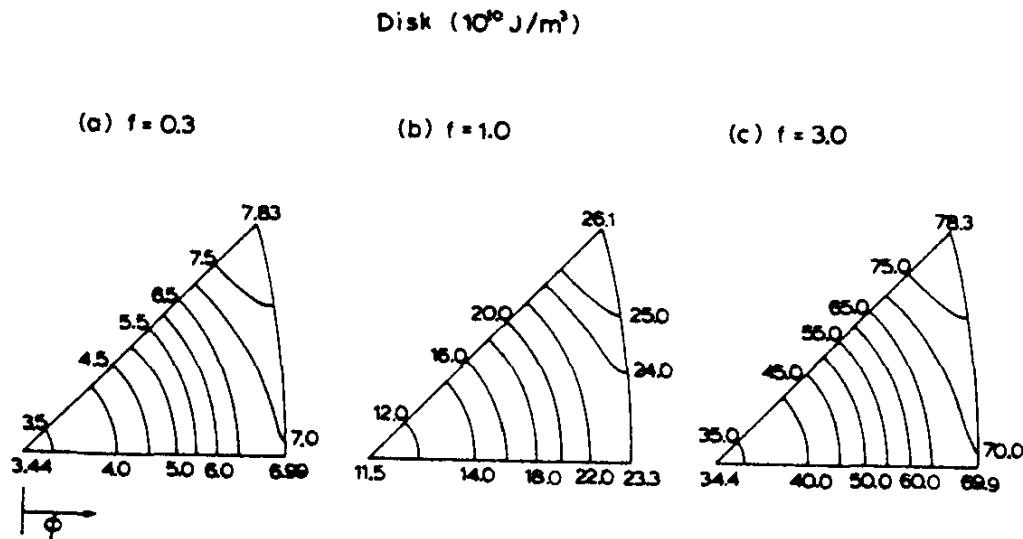


Fig. 26.4. Contour lines of energy; the matrix is copper and $C_{ij}^* = fC_{ij}$.

and the precipitate crystalline directions coincide with those of the matrix.

It can be seen that the minimum energy is achieved when the disk plane is perpendicular to [001]. This is due to the fact that the Zener anisotropy factor (Zener 1948)

$$A = 2C_{44}/(C_{11} - C_{12}) \quad (26.6)$$

is greater than 1 and C_{ij}^* is assumed to equal fC_{ij} in the energy calculation. The stress components in an ellipsoidal inhomogeneity containing eigenstrain ϵ_{ij}^p are expressed by (22.13.1). The right-hand side in (22.13.1) becomes (17.47) for a flat ellipsoidal inclusion, where ϵ_{ij}^{**} is used for ϵ_{ij}^* . As explained by (17.37.1) and (17.42), the displacement and the traction at the flat interface are zero. Using the principle of equivalent inclusion, we can find that the displacement and the traction at the flat interface of the inhomogeneous inclusion are also zero. This means that the inclusion is stressed but the matrix is not. The stress state is independent of the matrix moduli and remains the same even if the moduli of the matrix are equal to C_{ijkl}^* . Therefore, this inhomogeneous inclusion becomes the homogeneous inclusion with ϵ_{ij}^p . The stress is

$$\sigma_{pq} = C_{pqik}^* \left\{ C_{jlmn}^* \epsilon_{mn}^p N_{ij}(\bar{\xi}) D^{-1}(\bar{\xi}) \bar{\xi}_k \bar{\xi}_l - \epsilon_{ki}^p \right\}, \quad (26.7)$$

from (17.47), where $N_{ij}(\bar{\xi})$ and $D(\bar{\xi})$ are expressed in terms of C_{ijkl}^* , and $\bar{\xi}$ is the unit vector normal to the flat surface. The fact that the elastic stress state is independent of the matrix elastic moduli has been established by Mura and S.C. Lin (1974) and Lee, Barnett, and Aaronson (1977) who have pointed out that the elastic strain energy of a disk-shaped precipitate is independent of the

elastic moduli of the matrix. When C_{ijkl}^* has cubic symmetry and $\epsilon_{ij}^p = \delta_{ij}\epsilon^*$, the elastic strain energy W^* is calculated from (26.7) and (25.3),

$$W^* = \frac{1}{2}V(C_{11}^* + 2C_{12}^*)(\epsilon^*)^2 \{ 3 - (C_{11}^* + 2C_{12}^*)F \}, \quad (26.8)$$

where

$$\begin{aligned} F = & \left\{ C_{44}^{*2} + 2C_{44}^*(C_{11}^* - C_{12}^* - 2C_{44}^*)\Gamma + 3(C_{11}^* - C_{12}^* - 2C_{44}^*)^2\Delta \right\} \\ & \times \left\{ C_{11}^*C_{44}^{*2} + C_{44}^*(C_{11}^* + C_{12}^*)(C_{11}^* - C_{12}^* - 2C_{44}^*)\Gamma \right. \\ & \left. + (C_{11}^* - C_{12}^* - 2C_{44}^*)^2(C_{11}^* + 2C_{12}^* + C_{44}^*)\Delta \right\}^{-1}, \end{aligned} \quad (26.8.1)$$

$$\Gamma = \bar{\xi}_1^2\bar{\xi}_2^2 + \bar{\xi}_2^2\bar{\xi}_3^2 + \bar{\xi}_3^2\bar{\xi}_1^2,$$

$$\Delta = \bar{\xi}_1^2\bar{\xi}_2^2\bar{\xi}_3^2.$$

It can be seen that (26.8) has an extreme value when either one of $|\bar{\xi}_1|$, $|\bar{\xi}_2|$, or $|\bar{\xi}_3|$ is equal to 1 or $|\bar{\xi}_1| = |\bar{\xi}_2| = |\bar{\xi}_3| = 1/\sqrt{3}$. When the Zener factor $A^* = 2C_{44}^*/(C_{11}^* - C_{12}^*) > 1$, the minimum of W^* is found at $\bar{\xi} = (0, 0, 1)$, which is

$$W^* = V(\epsilon^*)^2(C_{11}^* + 2C_{12}^*)(C_{11}^* - C_{12}^*)/C_{11}^*; \quad (26.8.2)$$

this is in agreement with the result of Mura and S.C. Lin (1974). When $A^* < 1$, W^* takes a minimum at $\bar{\xi} = (1/\sqrt{3}, 1/\sqrt{3}, 1/\sqrt{3})$.

From Fig. 26.3, it is found that the elastic strain energy of a needle-shaped precipitate is a minimum when the needle is aligned parallel to one of the crystallographic directions $\langle 001 \rangle$. However, the minimum of the elastic strain energy of the needle can not be determined by A and A^* alone. In fact, Mori et al. (1978) have shown that the orientation of the needle (the axes direction) for minimum energy (for $A > 1$, $A^* < 1$) is [111] or [001], depending on the combination of C_{ij} and C_{ij}^* .

It is important to note from (26.7) that conclusions (17.42) and (17.46) with condition (17.44) are obtained even for an inhomogeneous, flat ellipsoidal (disk-shaped) inclusion, where ϵ_{ij}^* is replaced by ϵ_{ij}^p . The strain energy calculation of the spinodal alloy is essentially identical to that of a disk-shaped precipitate. The elastic strain energy of the spinodally decomposed cubic alloy has the same orientation dependence as (26.8) (see e.g. Hilliard 1970). For

example, when a solute concentration, c , fluctuates along the direction ξ , c and the corresponding eigenstrain are expressed by

$$\begin{aligned} c &= \sum_{n=1}^{\infty} c(n) \cos(2\pi nx/\lambda), \\ \epsilon_{ij}^p &= \epsilon_{ij}^0 \sum_{n=1}^{\infty} c(n) \cos(2\pi nx/\lambda), \end{aligned} \quad (26.8.3)$$

where λ is the wave length of the fluctuation and x is the distance along ξ . Applying (3.14) and (25.2) with $\epsilon_{ij}^0 = \epsilon^* \delta_{ij}$, we obtain the elastic strain energy,

$$W^* = \frac{1}{2} \langle c^2 \rangle (C_{11}^* + 2C_{12}^*) (\epsilon^*)^2 \{ 3 - (C_{11}^* + 2C_{12}^*) F \} \quad (26.8.4)$$

per unit volume of a cubic crystal. Here, $\langle c^2 \rangle$ is defined as

$$\langle c^2 \rangle = (1/\lambda) \int_0^\lambda c^2 dx \quad (26.8.5)$$

and F has been defined in (26.8.1). It has often been said that the coefficient $G^* = (C_{11}^* + 2C_{12}^*) \{ 3 - (C_{11}^* + 2C_{12}^*) F \}$ in equation (26.8.4) is the elastic modulus in the direction along which the concentration of solute atoms fluctuates. However, such a concept leads to a misunderstanding, since the stress components normal to the disk surface (σ_{3p}) are zero as is shown by (17.42), while G^* is simply evaluated from (26.8). This elastic strain energy is caused by the stress components parallel to the disk surface (σ_{11} , σ_{22} , σ_{12}). This energy is somewhat related to the elastic modulus perpendicular to the direction of solute atom fluctuation.

One can extend (26.8.3) to the situation where ϵ_{ij}^0 is not a pure dilatation. Such a case might occur when a carbon atom concentration fluctuates in martensitic steel that has a high carbon concentration. In this case, because of the interaction of the carbon atoms, almost all the carbon atoms are accommodated in a particular type of sublattice (Zener 1946, Mori and Mura 1976, Mori, Cheng, and Mura 1976). For example, ϵ_{ij}^0 in (26.8.3) has components of $\epsilon_{33}^0 \neq \epsilon_{11}^0 = \epsilon_{22}^0$. It is known that $\epsilon_{33}^0 = 0.93$ and $\epsilon_{11}^0 = \epsilon_{22}^0 = -0.08$. If the concentration of the carbon atoms fluctuates as expressed by (26.8.3), we obtain the elastic energy per unit volume,

$$W^* = \frac{1}{2} \langle c^2 \rangle \{ H - G \}, \quad (26.8.6)$$

from (3.14) and (25.2), where

$$H = \left\{ 2C_{11}(\sigma_{11}^0)^2 - 4C_{12}\sigma_{11}^0\sigma_{33}^0 + (C_{11} + C_{12})(\sigma_{33}^0)^2 \right\} \\ \times \{(C_{11} + 2C_{12})(C_{11} - C_{12})\}^{-1}, \quad (26.8.7)$$

$$G = \left[(\sigma_{11}^0)^2 \left\{ (C_{44})^2 (\bar{\xi}_1^2 + \bar{\xi}_2^2) + 2C_{44}(C_{11} - C_{12} - 2C_{44})\bar{\xi}_1^2\bar{\xi}_2^2 \right. \right. \\ \left. \left. + 2(C_{11} - C_{44})(C_{11} - C_{12} - 2C_{44})\bar{\xi}_1^2\bar{\xi}_2^2\bar{\xi}_3^2 + C_{44}(C_{11} - C_{44})(\bar{\xi}_1^2 + \bar{\xi}_2^2)\bar{\xi}_3^2 \right\} \right. \\ \left. - 2\sigma_{11}^0\sigma_{33}^0(C_{12} + C_{44}) \left\{ C_{44}(\bar{\xi}_1^2 + \bar{\xi}_2^2)\bar{\xi}_3^2 + 2(C_{11} - C_{12} - 2C_{44})\bar{\xi}_1^2\bar{\xi}_2^2\bar{\xi}_3^2 \right\} \right. \\ \left. + (\sigma_{33}^0)^2 \left\{ (C_{44})^2\bar{\xi}_3^2 + C_{44}(C_{11} - C_{44})(\bar{\xi}_1^2 + \bar{\xi}_2^2)\bar{\xi}_3^2 \right. \right. \\ \left. \left. + (C_{11} - C_{12} - 2C_{44})(C_{11} + C_{12})\bar{\xi}_1^2\bar{\xi}_2^2\bar{\xi}_3^2 \right\} \right] \\ \times \left\{ (C_{44})^2 C_{11} + C_{44}(C_{11} - C_{12} - 2C_{44})(C_{11} + C_{12})(\bar{\xi}_1^2\bar{\xi}_2^2 + \bar{\xi}_2^2\bar{\xi}_3^2 + \bar{\xi}_3^2\bar{\xi}_1^2) \right. \\ \left. + (C_{11} - C_{12} - 2C_{44})^2(C_{11} + 2C_{12} + C_{44})\bar{\xi}_1^2\bar{\xi}_2^2\bar{\xi}_3^2 \right\}^{-1},$$

with

$$\sigma_{11}^0 = (C_{11} + C_{12})\epsilon_{11}^0 + C_{12}\epsilon_{33}^0, \quad (26.8.8)$$

$$\sigma_{33}^0 = 2C_{12}\epsilon_{11}^0 + C_{11}\epsilon_{33}^0.$$

It can be shown that W^* has maxima at $\bar{\xi}_1^2 = 1$, $\bar{\xi}_2^2 = 1$ or $\bar{\xi}_3^2 = 1$ and minima at

$$\bar{\xi}_1^2 = \sin^2\theta, \quad \bar{\xi}_2^2 = 0 \quad \text{and} \quad \bar{\xi}_3^2 = \cos^2\theta \quad (26.8.9)$$

or

$$\bar{\xi}_1^2 = 0, \quad \bar{\xi}_2^2 = \sin^2\theta \quad \text{and} \quad \bar{\xi}_3^2 = \cos^2\theta \quad (26.8.10)$$

with θ given by

$$\theta = \cos^{-1}(\sqrt{A}), \quad (26.8.11)$$

where A is the root of

$$\begin{aligned}
 & (C_{11} - C_{12} - 2C_{44})(C_{11} + C_{12})C_{44}\left\{\left(\sigma_{11}^0\right)^2 - \left(\sigma_{33}^0\right)^2\right\}A^2 \\
 & - 2\left[C_{44}C_{11}\left\{(C_{11} - C_{44})\left(\sigma_{11}^0\right)^2 + (C_{11} - C_{44})\left(\sigma_{33}^0\right)^2 - 2(C_{12} + C_{44})\sigma_{11}^0\sigma_{33}^0\right\}\right. \\
 & \quad \left.- (C_{11} - C_{12} - 2C_{44})(C_{11} + C_{12})C_{44}\left(\sigma_{11}^0\right)^2\right]A + C_{44}^2C_{11}\left\{\left(\sigma_{11}^0\right)^2 + \left(\sigma_{22}^0\right)^2\right\} \\
 & - C_{44}C_{11}\left\{(C_{11} - C_{44})\left(\sigma_{11}^0\right)^2 + (C_{11} - C_{44})\left(\sigma_{33}^0\right)^2 - 2(C_{12} + C_{44})\sigma_{11}^0\sigma_{33}^0\right\} \\
 & + (C_{11} - C_{12} - 2C_{44})(C_{11} + C_{12})C_{44}\left(\sigma_{11}^0\right)^2 = 0 \tag{26.8.12} \\
 & (0 \leq A \leq 1).
 \end{aligned}$$

By putting $C_{11} = 2.37 \times 10^{11}$ N/m², $C_{12} = 1.41 \times 10^{11}$ N/m² and $C_{44} = 1.16 \times 10^{11}$ N/m², one obtains $\theta = 19.3^\circ$. This example has been treated by Toyoshima (1980) for a discussion of the modulated structure observed in iron-carbon martensites (Toyoshima and Nagakura, 1979). Toyoshima has found that the direction of the fluctuation of the carbon atom concentration is expressed by either [hol] or [ohl], and the angle θ depends on the amount of carbon in the martensite; as the carbon concentration goes to zero, θ approaches 22° .

Incoherent precipitates

The precipitates discussed in the last two subsections are coherent precipitates. When a second phase is nucleated, the interface between the precipitate and the matrix is classified as either coherent or incoherent. By coherent, it is meant that there is a one-to-one correspondence between atoms at the interface; an incoherent interface refers to a lack of conservation of lattice sites across the interface. For an incoherent precipitate, the stress is purely hydrostatic along the interface. The associated elastic strain energy has been obtained by Nabarro (1940) for the isotropic case and by Kröner (1954) and Lee and Johnson (1978) for the anisotropic case.

Let us consider an ellipsoidal incoherent precipitate in an infinite anisotropic medium. The elastic moduli of the precipitate and the matrix are

denoted by C_{ijkl}^* and C_{ijkl} , respectively. The equivalent inclusion method is employed. From (22.11) we have

$$C_{ijkl}^*(\epsilon_{kl} - \epsilon_{kl}^p) = C_{ijkl}(\epsilon_{kl} - \epsilon_{kl}^p - \epsilon_{kl}^*), \quad (26.9)$$

where no applied stress is considered. The stress components given by (26.9) must be hydrostatic,

$$C_{ijkl}(\epsilon_{kl} - \epsilon_{kl}^p - \epsilon_{kl}^*) = \delta_{ij}\sigma. \quad (26.10)$$

Furthermore, the incoherent precipitate is characterized by

$$\epsilon_{kk}^p = \epsilon. \quad (26.11)$$

This means that for the incoherent precipitate the six components of ϵ_{ij}^p are not given, but only ϵ_{kk}^p is prescribed to be a constant, ϵ . By assuming the relation (22.12), the equivalent eigenstrain ϵ_{ij}^* can be expressed in terms of ϵ_{kl}^p from (26.9), as in the coherent case. When the result, ϵ_{ij}^* , is substituted into (26.10), the six components of ϵ_{ij}^p are found to be linear functions of σ . The unknown σ is then determined from condition (26.11), while the associated elastic strain energy can be determined from (25.3).

Lee and Johnson (1978) have calculated W^*/V for the spheroidal precipitate ($a_1 = a_2 = a$, $a_3/a = \beta$), where the crystallographic directions of the precipitate and the matrix are parallel to the principal axes of the spheroid. Their result is

$$W^*/V = \frac{1}{2}\epsilon^2 K^* c^0 / (K^* + c^0), \quad (26.12)$$

where $K^* = \frac{1}{3}(C_{11}^* + 2C_{12}^*)$, which is the bulk modulus of the precipitate, and

$$\begin{aligned} c^0 &= (K \{(S_{1111} + S_{1122} - 1)(1 - S_{3333}) + 2S_{1133}S_{3311}\}) \\ &\times ((S_{1111} + S_{1122} - 1)(S_{3333} - \frac{2}{3}) \\ &+ \frac{2}{3}(S_{3333} - S_{1133} - S_{3311} - 1) - 2S_{1133}S_{3311})^{-1} \end{aligned} \quad (26.13)$$

with $K = \frac{1}{3}(C_{11} + 2C_{12})$. C_{ij}^* and C_{ij} are the elastic moduli of cubic crystals. The Eshelby tensor S_{ijkl} has been given by (17.27) and (17.28), where $\beta = 1/\rho$. Kröner (1954) has denoted by c^0 the compression modulus. Figure 26.5 shows the numerical results of $W^*/VC_{44}\epsilon^2$ calculated by Lee and Johnson (1978). The elastic strain energy reaches its maximum when the precipitate is a sphere, vanishing as the shape becomes a thin, flat ellipsoid.

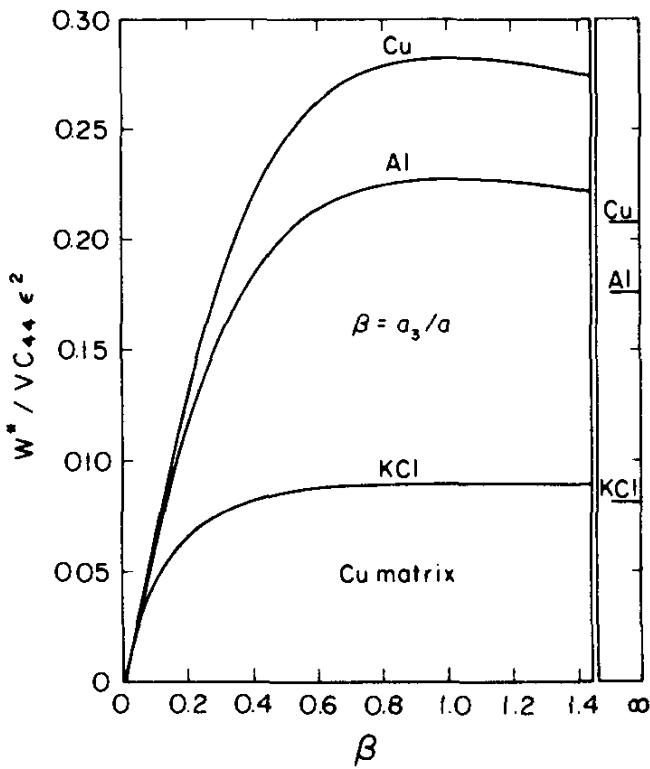


Fig. 26.5 Normalized elastic strain energy per unit volume of the incoherent precipitate vs aspect ratios for the Cu precipitate, Al precipitate and KCl precipitate in a Cu matrix

The incoherent state of the precipitates characterized by (26.10) and (26.11) can be interpreted as a free-energy minimum state of coherent precipitates with given ϵ_{ij}^p , when an additional inelastic deformation $\Delta\epsilon_{ij}^p$ is allowable. Consider a coherent precipitate with eigenstrain ϵ_{ij}^p . The corresponding stress field is denoted by σ_{ij} and is calculated from (26.9). Let us consider a relaxation which occurs inside or at the surface of the coherent precipitate. Dislocations punching out of a precipitate represent the relaxation process occurring outside, which may be caused by actual plastic deformation, by mass diffusion along the matrix-precipitate interface, or through the bulk of the precipitate as suggested by Christian (1965). This inelastic deformation due to relaxation is denoted by $\Delta\epsilon_{ij}^p$. Either case of diffusion or plastic deformation does not cause volume changes,

$$\Delta\epsilon_{ii}^p = 0. \quad (26.13.1)$$

The total stress in the precipitate is $\sigma_{ij} + \Delta\sigma_{ij}$, where $\Delta\sigma_{ij}$ is the eigenstress due to $\Delta\epsilon_{ij}^p$. The associated elastic strain energy W^* is

$$W^* = -\frac{1}{2}V(\sigma_{ij} + \Delta\sigma_{ij})(\epsilon_{ij}^p + \Delta\epsilon_{ij}^p) \quad (26.13.2)$$

from (25.3).

The minimum principle of the free energy W^* is introduced. $\Delta\epsilon_{ij}^P$ is created so that W^* becomes minimum for a given ϵ_{ij}^P . This extreme condition leads to

$$(\sigma_{ij} + \Delta\sigma_{ij})\delta(\Delta\epsilon_{ij}^P) = 0, \quad (26.13.3)$$

since $\sigma_{ij} + \Delta\sigma_{ij}$ is a linear function of $\epsilon_{ij}^P + \Delta\epsilon_{ij}^P$, which is the solution of (26.9) when ϵ_{ij}^P is replaced by $\epsilon_{ij}^P + \Delta\epsilon_{ij}^P$. Condition (26.13.3) is satisfied when

$$\sigma_{ij} + \Delta\sigma_{ij} = \delta_{ij}\sigma \quad (26.13.4)$$

because of (26.13.1); therefore, W^* becomes

$$W^* = -\frac{1}{2}V\sigma\epsilon_{ii}^P. \quad (26.13.5)$$

Condition (26.13.4) is no more than (26.10); that is, the situation considered above is identical to that of an incoherent precipitate.

Martensitic transformation

One of the important applications of the inclusion theory is in the area of martensitic transformation. Although various aspects of martensitic transformation have been investigated (Bilby and Christian 1956, Christian 1965, Wayman 1964, etc.), not until quite recently have the details of the application of the inclusion theory appeared in the literature.

The problem to be discussed here concerns the elastic field and elastic strain energy associated with the martensitic transformation. We assume that

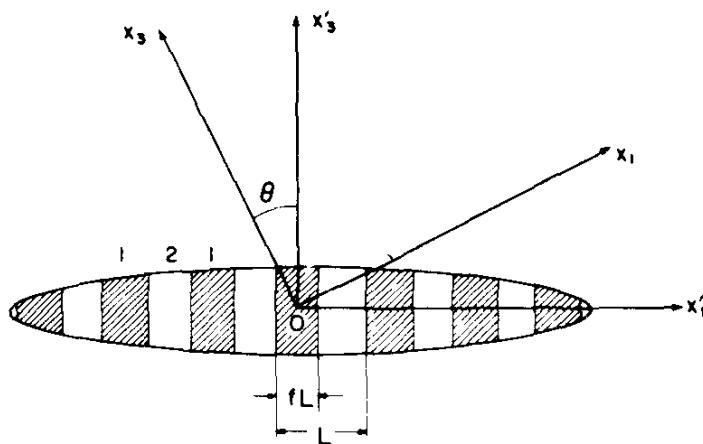


Fig. 26.6. Disk-shaped martensite blade, $a_1 = a_2 = a$, $\beta = a_3/a \ll 1$.

the orientation of martensite blades relative to the crystalline directions is determined by the principle of minimum elastic strain energy.

Mura, Mori, and Kato (1976) have considered a thin disk-shaped martensite blade ($a_1 = a_2 = a$, $\beta = a_3/a \ll 1$) as shown in Fig. 26.6. The x coordinate system is chosen parallel to certain crystallographic directions of the matrix, while the x' coordinate system coincides with the principal axes of the ellipsoid. In many ferrous alloys, martensites take the form of a thin, flat ellipsoid which consists of alternative twins, as shown schematically in Fig. 26.6. Each twin can be considered to be formed by the respective transformation strains (eigenstrains). In the case of martensitic transformation from a face-centered cubic to a body-centered tetragonal, the eigenstrain components in the x coordinate system are given by

$$\epsilon_{ij}^p = \begin{pmatrix} \epsilon_1 & 0 & 0 \\ 0 & \epsilon_2 & 0 \\ 0 & 0 & \epsilon_1 \end{pmatrix} \quad \text{in region 1,} \quad (26.14)$$

$$= \begin{pmatrix} \epsilon_2 & 0 & 0 \\ 0 & \epsilon_1 & 0 \\ 0 & 0 & \epsilon_1 \end{pmatrix} \quad \text{in region 2,}$$

and (see e.g. Wechsler, Lieberman and Read 1953)

$$\epsilon_1 = \sqrt{2} b/b_0 - 1, \quad \epsilon_2 = c/b_0 - 1, \quad (26.15)$$

where b_0 is the lattice constant for the f.c.c. phase and b and c are those for the b.c.t. phase. Region 1 is denoted by hatching in Fig. 26.6.

The Fourier series expression of (26.14) is

$$\epsilon_{ij}^p = \bar{\epsilon}_{ij}^p(0) + \sum_{n=1}^{\infty} \bar{\epsilon}_{ij}^p(n) \cos(2\pi n x'_1/L), \quad (26.16)$$

where

$$\bar{\epsilon}_{ij}^p(0) = (1/L) \int_{-L/2}^{L/2} \epsilon_{ij}^p dx'_1, \quad (26.17)$$

$$\bar{\epsilon}_{ij}^p(n) = (2/L) \int_{-L/2}^{L/2} \epsilon_{ij}^p \cos(2\pi n x'_1/L) dx'_1.$$

L is the wave-length of the alternating twins. $\bar{\epsilon}_{ij}^p(0)$ is the mean value of ϵ_{ij}^p . If the volume ratio of regions 1 and 2 is denoted by f , it holds that

$$\begin{aligned}\bar{\epsilon}_{11}^p(0) &= f\epsilon_1 + (1-f)\epsilon_2, \\ \bar{\epsilon}_{22}^p(0) &= f\epsilon_2 + (1-f)\epsilon_1, \\ \bar{\epsilon}_{33}^p(0) &= f\epsilon_1 + (1-f)\epsilon_1 = \epsilon_1.\end{aligned}\tag{26.18}$$

The eigenstrain components in the x' coordinate system are given by

$$\epsilon_{i'j'}^p = a_{ki'}a_{lj'}\epsilon_{kl}^p,\tag{26.19}$$

where $a_{ki'}$ is the direction cosine between the x_k - and the x'_i -axes.

As a first approximation, let us start with the case where $\epsilon_{ij}^p = \bar{\epsilon}_{ij}^p(0)$. This is similar to the approximation adopted by Shibata and Ono (1975, 1977) and Easterling and Thölén (1976). Equation (26.7) can be used for the elastic field. When

$$\epsilon_{1'1'}^p = \epsilon_{2'2'}^p = \epsilon_{1'2'}^p = 0,\tag{26.20}$$

the stress components vanish everywhere. Therefore, the associated elastic strain energy becomes zero. By solving (26.18) with (26.19), it is found that condition (26.20) can be satisfied if

$$a_{33'} = \cos \theta = (-\epsilon_1/\epsilon_2)^{1/2}, \quad a_{13'} = (1 + \epsilon_1/\epsilon_2)^{1/2}, \quad a_{23'} = 0,\tag{26.21}$$

and

$$f = \epsilon_1/(\epsilon_1 - \epsilon_2),\tag{26.21.1}$$

where θ is denoted in Fig. 26.6.

In the phenomenological crystallographic theory of martensitic transformation, the deformation in the martensite is assumed to be uniform, corresponding to the first term in (26.16). Furthermore, the phenomenological theory assumes that the magnitude and the angle between any two vectors lying parallel to the flat interface do not change (Wayman 1968). This is equivalent to (26.20). The postulates in the phenomenological crystallographic theory are interpreted in such a manner that the elastic strain energy vanishes when the uniform transformation is assumed. Thus, the prediction of f and the orientation of the martensite plate based on the phenomenological theory should agree with (26.21) and (26.21.1). In fact, this is the case when $|\epsilon_1|, |\epsilon_2| \ll 1$

(Wechsler, Lieberman, and Read 1953). The last restriction is that of the small deformation theory, while the calculations in the phenomenological theory are based on the finite deformation theory.

When we consider the effect of higher order terms in (26.16), the equivalent inclusion condition (22.11) is written as

$$\sigma_{ij} = C_{ijkl}^*(\epsilon_{kl} - \epsilon_{kl}^p) = C_{ijkl}(\epsilon_{kl} - \epsilon_{kl}^p - \epsilon_{kl}^{**}), \quad (26.22)$$

where ϵ_{ij}^{**} is the equivalent eigenstrain. The Fourier series expression is assumed for ϵ_{ij}^{**} ,

$$\epsilon_{ij}^{**} = \bar{\epsilon}_{ij}^{**}(0) + \sum_{n=1}^{\infty} \bar{\epsilon}_{ij}^{**}(n) \cos(2\pi n x_1 / L), \quad (26.23)$$

where

$$\epsilon_{ij}^{**} = \epsilon_{ij}^p + \epsilon_{ij}^*. \quad (26.24)$$

From (17.67), condition (26.22) is written as

$$\begin{aligned} & C_{ijkl}^* \left\{ C_{pqst} \bar{\epsilon}_{st}^{**}(0) \bar{\xi}_l \bar{\xi}_q N_{kp}(\bar{\xi}) D^{-1}(\bar{\xi}) - \bar{\epsilon}_{kl}^p(0) \right. \\ & + \sum_{n=1}^{\infty} C_{pqst} \bar{\epsilon}_{st}^{**}(n) \bar{\xi}_l \bar{\xi}_q J_0(2\pi n a / L) N_{kp}(\bar{\xi}) D^{-1}(\bar{\xi}) \\ & \left. - \sum_{n=1}^{\infty} \bar{\epsilon}_{kl}^p(n) \cos(2\pi n x'_1 / L) \right\} \\ & = C_{ijkl} \left\{ C_{pqst} \bar{\epsilon}_{st}^{**}(0) \bar{\xi}_l \bar{\xi}_q N_{kp}(\bar{\xi}) D^{-1}(\bar{\xi}) - \bar{\epsilon}_{kl}^{**}(0) \right. \\ & + \sum_{n=1}^{\infty} C_{pqst} \bar{\epsilon}_{st}^{**}(n) \bar{\xi}_l \bar{\xi}_q J_0(2\pi n a / L) N_{kp}(\bar{\xi}) D^{-1}(\bar{\xi}) \\ & \left. - \sum_{n=1}^{\infty} \bar{\epsilon}_{kl}^{**}(n) \cos(2\pi n x'_1 / L) \right\}, \end{aligned} \quad (26.25)$$

where $\bar{\xi}$ is the unit vector along the x'_3 -direction. When the orientation of the

flat ellipsoid and f are chosen as defined by (26.21) and (26.21.1), the first two terms on both sides of (26.25) disappear by taking

$$\bar{\epsilon}_{ij}^{**}(0) = \bar{\epsilon}_{ij}^p(0). \quad (26.26)$$

Further simplification of (26.25) can be made when the terms containing J_0 are neglected. Observation by electron microscope has shown that $a \approx 10 \mu\text{m}$ and $L \approx 100 \text{ \AA}$ (e.g., Tamura et al. 1964). Thus, $J_0(2\pi an/L) \approx J_0(\infty) \approx 0$. Finally, equation (26.25) becomes

$$-C_{ijkl}^* \sum_{n=1}^{\infty} \bar{\epsilon}_{kl}^p(n) \cos(2\pi nx'_1/L) = -C_{ijkl} \sum_{n=1}^{\infty} \bar{\epsilon}_{kl}^{**}(n) \cos(2\pi nx'_1/L) \quad (26.27)$$

which is also equal to σ_{ij} . Therefore, from (26.16),

$$\sigma_{ij} = -C_{ijkl}^* \{ \epsilon_{kl}^p - \bar{\epsilon}_{kl}^p(0) \}. \quad (26.28)$$

The elastic strain energy is

$$W^* = -\frac{1}{2} \int_V \sigma_{ij} \epsilon_{ij}^p \, dD, \quad (26.29)$$

where V is the volume of the flat ellipsoid. When (26.14) and (26.18) are used in the above two formulae, we have

$$W^*/V = f(1-f)(C_{11}^* - C_{12}^*)(\epsilon_1 - \epsilon_2)^2, \quad (26.30)$$

where f is given by (26.21.1). This is surprisingly large, presumably because a very thin, flat ellipsoid is assumed in the calculation.

Suezawa (1978) has used the method of Khachaturyan (1967) and evaluated the strain energy of martensitic transformation to be proportional to $(L/a_3)^2$ (see also a review paper by Christian 1979).

Shibata and Ono (1975, 1977) have considered the martensitic transformation of titanium. They assume elastic isotropy for both the matrix and the martensite. A homogeneous spheroidal inclusion ($a_1 = a_3 = a$, $k = a_2/a$) is subjected to a uniform transformation strain ϵ_{ij}^p (eigenstrain). The elastic strain energy is expressed by (13.20.1), where ϵ_{ij}^* is replaced by ϵ_{ij}^p (see Fig. 26.7).

The transformation of the b.c.c. phase of Ti and Zr alloys during quenching is martensitic and results in at least four structures: h.c.p., f.c.c., orthorhombic and f.c.c. orthorhombic (Williams 1973). The h.c.p. martensite is most com-

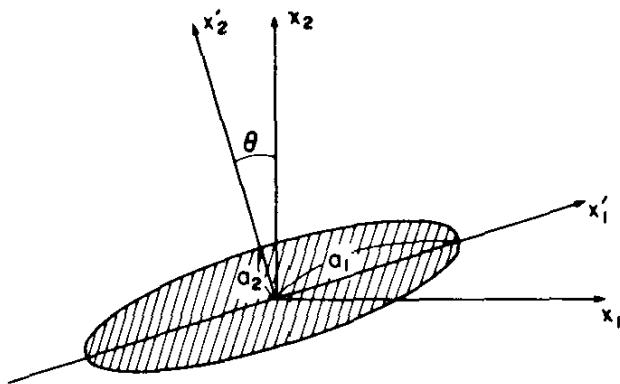


Fig. 26.7. Spheroidal martensite $a_1 = a_3 = a$, $k = a_2/a$.

mon and occurs in Ti, Zr, Ti-Mo alloys, Zr-Nb alloys and others. In pure metals and other dilute alloys, the lath martensite forms, whereas the plate martensite occurs with increasing solute content. The orientation relationship of the plate martensite to the matrix has been established by Burgers (1934),

$$\begin{aligned} (011)_B &\parallel (0001)_H, \\ [1\bar{1}1]_B &\parallel [11\bar{2}0]_H. \end{aligned} \quad (26.31)$$

By taking the x_1 -, x_2 -, and x_3 -directions as $[\bar{1}\bar{1}\bar{1}]_B$, $[\bar{2}\bar{1}1]_B$ and $[011]_B$, respectively, Shibata and Ono (1977) have used

$$\epsilon_{ij}^p = \begin{pmatrix} 0.0381 & -0.0884 & 0 \\ -0.0884 & -0.0434 & 0 \\ 0 & 0 & 0.0088 \end{pmatrix}, \quad (26.32)$$

where the ratios of the lattice parameters in b.c.c. and h.c.p. phases are taken as $a_H/a_B = 0.899$ and $c_H/a_H = 1.587$. Denoting the principal axes of the spheroid by x'_1 , x'_2 and x'_3 , we have the relation (26.19). The rotated coordinate system x' is obtained by rotating first about the x_3 -axis by θ , followed by the rotation about the x'_1 -axis by ψ (see Fig. 26.7). For a given ϵ_{ij}^p , the elastic strain energy per unit volume of martensite becomes a function of θ , ψ and $k = a_2/a$. Shibata and Ono have calculated θ , ψ and k , under which the elastic strain energy attains its minimum value. Their results indicate that the elastic strain energy is minimized when the morphology of h.c.p. Ti martensite is a thin disk shape lying on a plane close to $(11X)_B$, where X is equal to $1 \cdot 2 \sim 1 \cdot 3$. This habit plane agrees with experimental observations.

A spherical Fe precipitate, which is formed in a Cu matrix and is f.c.c. before martensitic transformation, becomes b.c.c. with the banded structure of alternating twins after transformation (Easterling and Miekkoja, 1967). The elastic strain energy of an Fe particle with the banded structure has recently

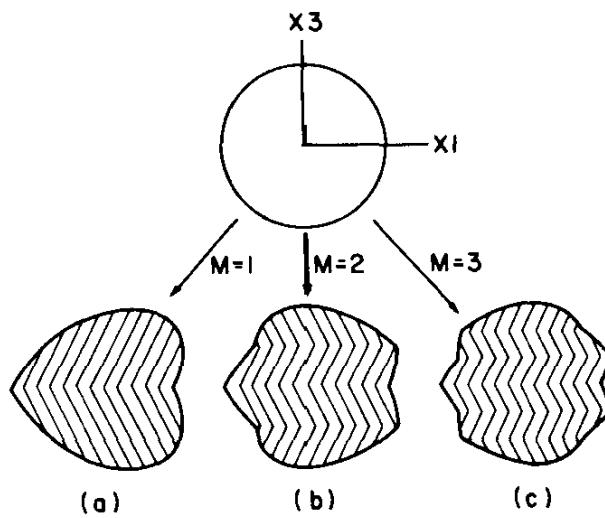


Fig. 26.8 Spherical precipitate undergoing martensitic transformation. When matrix constraint is absent, the spherical particle changes its shape successively to (a), (b) and (c) to form twinned martensite

been evaluated by Kinsman, Sprys, and Asaro (1975) using an unrealistic approximation based on a simplified model. The physical situation they have envisaged is a spherical region undergoing martensitic transformation, with the upper and the lower halves shearing in anti-parallel directions, as shown schematically in Fig. 26.8(a). Thus, the surface and the eigenstrain of the transformed particle Ω are

$$(x_1^2 + x_2^2 + x_3^2)/a^2 = 1, \quad (26.33)$$

$$\epsilon_{31}^p(x) = \pm \frac{1}{2}S, \quad x_3 \geq 0 \text{ in } \Omega,$$

where a is the radius of the spherical particle and S is a constant to be determined by the crystallographic information. However, Kinsman, Sprys, and Asaro (1975) approximated (26.33) with the eigenstrain

$$\epsilon_{31}^p(x) = tx_3/a \quad \text{in } \Omega, \quad (26.34)$$

with $t = S$, in order to utilize Asaro and Barnett's work (1975). Mura, Mori and Kato (1976) have calculated the elastic strain energy associated with situation (26.33) as follows: The Fourier series expression for the eigenstrain defined by (26.33) is

$$\epsilon_{31}^p(x) = (2S/\pi) \sum_{n=1}^{\infty} (2n-1)^{-1} \sin\{(2n-1)\pi x_3/a\}. \quad (26.35)$$

For simplicity, assume that the spherical precipitate and the matrix have the

same isotropic elastic constants. From (17.56), the only non-vanishing stress component σ_{31} pertinent to the strain energy evaluation is

$$\sigma_{31}(x) = \sum_{n=1}^{\infty} \bar{\sigma}_{31}(x, n), \quad (26.36)$$

with

$$\begin{aligned} \bar{\sigma}_{31}(x, n) = & (\mu S a^3 / \pi C_3) \int_{S^2} \left[J_0(C_0 R) \sin\{(x \cdot \bar{\xi}) C_3 \bar{\xi}_3 / a\} \right. \\ & + 2 J_0(C_0 R) C_3 \bar{\xi}_3 (x \cdot \bar{\xi}) \cos\{(x \cdot \bar{\xi}) C_3 \bar{\xi}_3 / a\} / a \\ & + J_1(C_0 R) \left\{ (x \cdot \bar{\xi})^2 C_0 / (a^2 R) + C_3^2 \bar{\xi}_3^2 R / C_0 \right\} \sin\{(x \cdot \bar{\xi}) C_3 \bar{\xi}_3 / a\} \Big] \\ & \times (\bar{\xi}_1^2 + \bar{\xi}_3^2 - 3\bar{\xi}_1^2 \bar{\xi}_3^2) dS(\bar{\xi}) - (4\mu S / C_3) \sin(C_3 x_3 / a), \end{aligned} \quad (26.37)$$

where $C_3 = (2n - 1)\pi$, μ is the shear modulus, and Poisson's ratio is taken to be $\frac{1}{3}$. Since the analytical integration involved in (26.37) is difficult, the stress and the elastic strain energy to be determined from (26.29) have been evaluated numerically on a computer. The result is

$$W^*/V = 0.182 \mu S^2. \quad (26.38)$$

The above value is a little larger than that of Kinsman, Sprys, and Asaro (1975).

Similar calculations have been performed for the cases where

$$\epsilon_{31}^p(x) = \begin{cases} \frac{1}{2}S, & 2Na/M < x_3 < (2N+1)a/M \\ -\frac{1}{2}S, & (2N+1)a/M < x_3 < (2N+2)a/M \end{cases} \quad \text{in } \Omega, \quad (26.39)$$

with $M = 2, 3$ (see Figs. 26.8(b), (c)). (The case $M = 1$ corresponds to (26.33).) In (26.39) N takes on integer values. The calculated elastic strain energies are plotted against M in Fig. 26.9, which also includes results for the case $M = 0$, corresponding to uniform shear and the value of Kinsman, Sprys, and Asaro (1975) for the case $M = 1$. It can be seen that as M increases, the elastic strain energy decreases. Thus, as far as the elastic strain energy is concerned, the alternating twin structure with the shorter interval is favored. This conclusion agrees with intuition, based on an examination of Fig. 26.8; as the interval

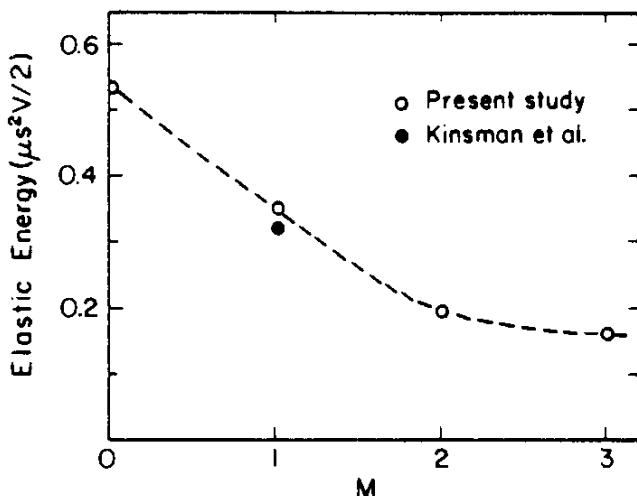


Fig. 26.9 Elastic strain energy of a martensite particle with M pairs of twins in Fig. 26.8

between the alternating twins decreases, the shape change of the particle caused by the transformation becomes smaller as a whole.

Stress orienting precipitation

As a typical example, we discuss the precipitation of α'' phases, $Fe_{16}N_2$, from a supersaturated Fe-N alloy. The eigenstrain (misfit) of this precipitate has the components

$$\epsilon_{11}^p = \epsilon_{22}^p = -0.0023, \quad \epsilon_{33}^p = 0.0971 \quad (26.40)$$

(Jack, 1951). There are two more types of the crystallographically equivalent $Fe_{16}N_2$, eigenstrains of which are obtained by the cyclic permutation of the index in (26.40). The type characterized by (26.40) is called the (001) variant, since the precipitates having (26.40) are formed in disk shapes, approximately parallel to the (001) plane. This can be understood by noting that $|\epsilon_{11}^p| = |\epsilon_{22}^p| \ll |\epsilon_{33}^p|$ and that the elastic energy of the disk-shaped precipitate is determined by the eigenstrains within the disk plane (see (17.42), (26.7) and the related discussions).

Statistically, the three types of the variants, (100), (010), and (001), are formed in an equal amount, when there is no external or internal stress, except the stress due to the precipitates. However, when an external stress is applied, a particular type of variant is preferentially found; e.g., the presence of the tensile stress along [001] favors the formation of the (001) variants against others (Nakada, Leslie and Chrav, 1967). This phenomenon can be interpreted by the interaction between the precipitates and the applied stress; the interaction energy (25.24) is far smaller for the (001) variant than for the (100) or

(010) variant, when the tensile stress is applied along the [001]. This effect, known as the stress orienting effect, is remarkable, and it has been shown that as the tensile stress increases, not only the relative number but also the absolute number of the (001) variants becomes larger (Tanaka, Sato and Mori, 1978).

Sato, Mori and Tanaka (1979) examined the precipitation orientations around a non-metallic inclusion subjected to a tensile stress as shown in Fig. 26.10. In the figure, the tensile stress $\sigma_{33}^0 = 98 \text{ MN/m}^2$ has been applied along the [001] (vertical) direction during the precipitation of Fe_{16}N_2 in a Fe-N alloy single crystal. It is noted that most of the precipitates are parallel to the (001) plane. With a close examination it can be seen that the density of the precipitates is larger in the close neighborhood of the inclusion (black contrasted spheroid) along the [010] (horizontal) direction and is small at the top or the bottom of the inclusion, the [001] direction. Sato, Mori, and Tanaka have calculated the stress outside the inclusion from (18.6) together with the method of the equivalent inclusion in Section 22. The elastic constants are $C_{1111} = 2.37 \times 10^{11} \text{ N/m}^2$, $C_{1122} = 1.41 \times 10^{11} \text{ N/m}^2$ and $C_{1212} = 1.16 \times 10^{11} \text{ N/m}^2$ for Fe. Since the non-metallic inclusion is identified as a slug particle, the isotropic elastic constants are assigned to the inclusion (the shear modulus



Fig. 26.10 Precipitates around a non-metallic inclusion subjected to a tensile stress (after Sato, Mori, and Tanaka 1979)

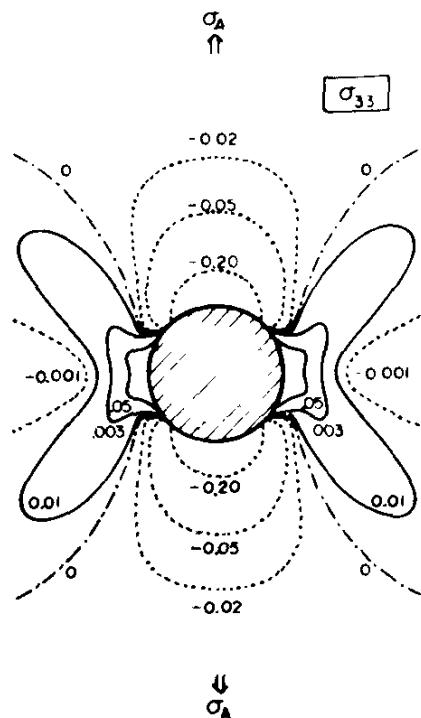


Fig. 26.11 Equi-contour lines of the stress disturbance around an inclusion (after Sato, Mori, and Tanaka 1979)

2.9×10^{10} N/m² and the Poisson ratio 0.22, for typical glass). Figure 26.11 shows the equi-contour lines of the stress disturbance which is expressed in units of σ_A . In agreement with Fig. 26.10 the stress at the [010] edges is significantly large while that at the [001] edges is smaller than σ_A .

Cracks

An elliptical crack is a special case of an ellipsoidal void where one of the principal axes of the ellipsoid becomes vanishingly small. The Griffith fracture criterion and stress intensity factors of cracks are the primary topics discussed in this chapter.

27. Critical stresses of cracks in isotropic media

Penny-shaped cracks

Cracks can be considered as a special case of voids when one of the principal axes, say a_3 , is infinitesimally small compared to the others. The simplest example, a penny-shaped crack ($a_1 = a_2 = a$, $a_3 = 0$), is considered first. Assume that an infinitely extended isotropic material containing a crack Ω is subjected to a uniform applied stress σ_{33}^0 at infinity. The equivalent inclusion method is employed for the analysis.

The crack is simulated by a penny-shaped inclusion with an eigenstrain ϵ_{ij}^* . The corresponding eigenstress σ_{ij} is given by (11.26). The necessary conditions for the application of the equivalent inclusion method are

$$\begin{aligned}\sigma_{33}^0 + \sigma_{33} &= 0 && \text{in } \Omega \\ \sigma_{31} = \sigma_{32} &= 0\end{aligned}\tag{27.1}$$

which correspond to traction-free conditions on the crack surface. It can be seen in (11.26) that no finite values of ϵ_{ij}^* satisfy the first condition in (27.1) for $a_3 \rightarrow 0$. Therefore, we allow ϵ_{33}^* to be unbounded in such a manner that

$$\lim_{a_3 \rightarrow 0} a_3 \epsilon_{33}^* = \text{finite} = \epsilon^*. \tag{27.2}$$

All other components of ϵ_{ij}^* are taken to be zero. Thus,

$$\sigma_{33} = -\frac{\mu\pi}{2(1-\nu)} \frac{\epsilon^*}{a}, \quad (27.3)$$

$$\sigma_{31} = \sigma_{32} = 0.$$

Conditions (27.1) are satisfied if ϵ^* is chosen as

$$\epsilon^* = \frac{2(1-\nu)a}{\mu\pi} \sigma_{33}^0. \quad (27.4)$$

The interaction energy (the decrease in the total potential energy of the body due to the crack) is given by (25.18),

$$\Delta W = -\frac{1}{2}(4\pi/3)a_1a_2a_3\sigma_{33}^0\epsilon_{33}^* \\ = -(2\pi/3)a^2\sigma_{33}^0\epsilon^* = -\frac{4(1-\nu)a^3}{3\mu}(\sigma_{33}^0)^2. \quad (27.5)$$

The Griffith (1921) fracture criterion is

$$\frac{\partial}{\partial a}(\Delta W + 2\pi a^2\gamma) = 0, \quad (27.6)$$

where $2\pi a^2$ is the total surface of the crack and γ is the surface energy per unit area. Condition (27.6) leads to the critical stress

$$\sigma_{33}^0 = \left\{ \frac{\pi\mu\gamma}{(1-\nu)a} \right\}^{1/2}. \quad (27.7)$$

A given penny-shaped crack with radius a becomes unstable (starts to grow) when the applied stress σ_{33}^0 reaches the critical value given by (27.7).

A penny-shaped crack under a uniform shear σ_{31}^0 at infinity can be treated in a similar manner. All components of ϵ_{ij}^* except ϵ_{31}^* are taken to be zero in (11.27), where the unbounded ϵ_{31}^* is such that

$$\lim_{a_3 \rightarrow 0} a_3\epsilon_{31}^* = \text{finite} = \epsilon^*. \quad (27.8)$$

We then have

$$\sigma_{31} = -\frac{\mu\pi(2-\nu)}{2(1-\nu)} \frac{\epsilon^*}{a}. \quad (27.9)$$

The traction-free conditions on the crack surface are

$$\begin{aligned} \sigma_{31}^0 + \sigma_{31} &= 0, \\ \sigma_{33} = \sigma_{32} &= 0, \end{aligned} \quad (27.10)$$

which are satisfied by

$$\epsilon^* = \frac{2(1-\nu)a}{\mu\pi(2-\nu)} \sigma_{31}^0. \quad (27.11)$$

The interaction energy thus becomes

$$\Delta W = -(4\pi/3)a^2\sigma_{31}^0\epsilon^* = -\frac{8(1-\nu)a^3}{3\mu(2-\nu)}(\sigma_{31}^0)^2, \quad (27.12)$$

and condition (27.6) leads to the critical stress

$$\sigma_{31}^0 = \left\{ \frac{\pi\mu(2-\nu)\gamma}{2(1-\nu)a} \right\}^{1/2}. \quad (27.13)$$

Results (27.7) and (27.13) have been obtained by Sack (1946) and Sneddon (1946). The interaction energy and the crack opening displacement for a pair of coplaner penny-shaped cracks are given by Fu and Keer (1969) for the shear case and by Fu (1973) for thermal cracks.

Slit-like cracks

When one of the principal axes of an ellipsoidal void, say a_3 , becomes infinite and the minor axis a_2 becomes infinitely small, the ellipsoid becomes a slit-like crack. When the unbounded solid which contains this crack is subjected to a uniform tension σ_{22}^0 at infinity, the stress disturbance due to the crack can be simulated by (11.22.1). In (11.22.1) we choose all $\epsilon_{ij}^* = 0$ except the unbounded ϵ_{22}^* which must satisfy

$$a_2\epsilon_{22}^* = \text{finite} = \epsilon^* \quad \text{for } a_2 \rightarrow 0. \quad (27.14)$$

Then we have

$$\sigma_{22} = -\frac{\mu}{1-\nu} \frac{\epsilon^*}{a_1}. \quad (27.15)$$

The traction-free conditions on the crack surface are

$$\begin{aligned} \sigma_{22}^0 + \sigma_{22} &= 0, \\ \sigma_{21} = \sigma_{23} &= 0, \end{aligned} \quad (27.16)$$

which are satisfied when

$$\epsilon^* = (1-\nu) a_1 \sigma_{22}^0 / \mu. \quad (27.17)$$

The interaction energy per unit length of the crack in the x_3 -direction becomes

$$\begin{aligned} \Delta W &= -\frac{1}{2} \pi a_1 a_2 \sigma_{22}^0 \epsilon_{22}^* = -\frac{1}{2} \pi a_1 \sigma_{22}^0 \epsilon^* \\ &= -\pi(1-\nu) a_1^2 (\sigma_{22}^0)^2 / 2\mu, \end{aligned} \quad (27.18)$$

and the Griffith fracture criterion

$$\frac{\partial}{\partial a_1} (\Delta W + 4a_1 \gamma) = 0 \quad (27.19)$$

leads to

$$\sigma_{22}^0 = \left\{ \frac{4\mu\gamma}{\pi(1-\nu)a_1} \right\}^{1/2}. \quad (27.20)$$

When the body containing this slit crack is subjected to a uniform applied stress σ_{12}^0 at infinity, a similar analysis leads to

$$\sigma_{12}^0 = \left\{ \frac{4\mu\gamma}{\pi(1-\nu)a_1} \right\}^{1/2}. \quad (27.21)$$

The above result, (27.20) and (27.21), is for the plane strain case. The corresponding result for the plane stress is obtained by replacing $E/(1-\nu^2)$

by E and $\nu/(1 - \nu)$ by ν . Then, we have the critical stresses

$$\sigma_{22}^0 = \left(\frac{2E\gamma}{\pi a_1} \right)^{1/2}, \quad \sigma_{12}^0 = \left(\frac{2E\gamma}{\pi a_1} \right)^{1/2}. \quad (27.22)$$

The above results have been obtained by Griffith (1921).

Flat ellipsoidal cracks

When a crack is a flat ellipsoid ($a_1 > a_2$, $a_3 = 0$), the stress disturbance can be simulated by (11.26) or (11.27) depending on the applied stress.

For simple tension σ_{33}^0 , we have

$$\lim_{a_3 \rightarrow 0} a_3 \epsilon_{33}^* = \sigma_{33}^0 a_2 (1 - \nu) / \mu E(k), \quad (27.23)$$

$$\Delta W = -\frac{2}{3}\pi (\sigma_{33}^0)^2 a_1 a_2^2 (1 - \nu) / \mu E(k),$$

where $k^2 = 1 - a_2^2/a_1^2$, and $E(k)$ and $F(k)$ in (27.24) are the complete elliptic integrals defined by (11.25).

For simple shear σ_{31}^0 , we have

$$\begin{aligned} \lim_{a_3 \rightarrow 0} a_3 \epsilon_{13}^* &= \sigma_{31}^0 / 2\mu \left[\frac{\nu}{1 - \nu} \frac{a_2 \{ F(k) - E(k) \}}{a_1^2 - a_2^2} + \frac{E(k)}{a_2} \right], \\ \Delta W &= -\frac{2}{3}\pi (\sigma_{31}^0)^2 a_1 a_2 / \mu \left[\frac{\nu}{1 - \nu} \frac{a_2 \{ F(k) - E(k) \}}{a_1^2 - a_2^2} + \frac{E(k)}{a_2} \right]. \end{aligned} \quad (27.24)$$

The expressions for ΔW in (27.23) and (27.24) have been obtained by Kassir and Sih (1967).

The critical stress can be obtained from

$$\begin{aligned} \frac{\partial}{\partial a_1} (\Delta W + 2\pi a_1 a_2 \gamma) &= 0, \\ \frac{\partial}{\partial a_2} (\Delta W + 2\pi a_1 a_2 \gamma) &= 0, \end{aligned} \quad (27.25)$$

if a_1 and a_2 are regarded as independent variables. The following formulae

are useful for calculation of (27.25):

$$\begin{aligned} dE(k)/dk &= \{E(k) - F(k)\}/k, \\ dF(k)/dk &= \{E(k)/(1 - k^2) - F(k)\}/k, \\ \partial k/\partial a_1 &= a_2^2/a_1^3 k, \quad \partial k/\partial a_2 = -a_2/a_1^2 k. \end{aligned} \quad (27.26)$$

For simple tension, the first equation in (27.25) yields

$$\sigma_{33}^0 = \left[\frac{3\mu\gamma k^2 E^2(k)}{(1 - \nu) a_2 \{ (1 - 2(k')^2) E(k) + (k')^2 F(k) \}} \right]^{1/2}, \quad (27.27)$$

and the second equation gives

$$\sigma_{33}^0 = \left[\frac{3\mu\gamma k^2 E^2(k)}{(1 - \nu) a_2 \{ (2 - (k')^2) E(k) - (k')^2 F(k) \}} \right]^{1/2}, \quad (27.27.1)$$

where

$$k^2 = 1 - a_2^2/a_1^2, \quad (k')^2 = a_2^2/a_1^2. \quad (27.27.2)$$

The solution of the simultaneous equations (27.25) leads to $a_1 = a_2$ and σ_{33}^0 given by (27.7). For a given set of a_1 and a_2 , where $a_1 \neq a_2$, the critical value of σ_{33}^0 is given as the smaller of (27.27) or (27.27.1). When $a_1 > a_2$, the smaller value is (27.27.1). The crack grows from the minor axis of the ellipse. This is consistent with the fact that the stress intensity factor at the minor axis is greater than that at the major axis.

For simple shear, the first equation in (27.25) yields

$$\begin{aligned} \sigma_{31}^0 &= \left[\frac{3\mu\gamma}{(1 - \nu) a_2} \right. \\ &\times \left. \frac{\{(k^2 - \nu) E(k) + \nu(k')^2 F(k)\}^2}{\{(1 - \nu) - (3 + \nu)(k')^2 + 2(k')^4\} E(k) + \{(2\nu + 1)(k')^2 - (k')^4\} F(k)} \right]^{1/2} \end{aligned} \quad (27.28)$$

and the second equation gives

$$\sigma_{31}^0 = \left[\frac{3\mu\gamma}{(1-\nu)a_2} \times \frac{\{(k^2 - \nu)E(k) + \nu(k')^2F(k)\}^2}{\{2 - 2\nu + (4\nu - 3)(k')^2 + (k')^4\}E(k) + \{(\nu - 1)(k')^2 + (1 - 3\nu)(k')^4\}F(k)} \right] \quad (27.28.1)$$

When $1 > a_2/a_1 > 0.7$, ($\nu = 0.3$), σ_{31}^0 given by (27.28) is smaller than σ_{31}^0 given by (27.28.1). Therefore, the crack starts to grow from the major axis of the ellipse. If, however, $a_2/a_1 < 0.7$, σ_{31}^0 given by (27.28.1) becomes smaller than σ_{31}^0 given by (27.28). Then, the crack starts to grow from the minor axis. When $a_2/a_1 > 1$, expressions (27.28) and (27.28.1) must be rewritten by using the following formulae:

$$E(k) = (a_2/a_1)E(\bar{k}),$$

$$F(k) = (a_1/a_2)F(\bar{k}),$$

$$\bar{k}^2 = 1 - a_1^2/a_2^2, \quad (27.28.2)$$

$$k^2 = \bar{k}^2/(\bar{k}^2 - 1),$$

$$(k')^2 = 1/(1 - \bar{k}^2).$$

Smith (1971) has investigated the case when δa_1 and δa_2 satisfy the condition $m = \delta a_2/\delta a_1$. He considered a penny-shaped crack which in its initial state is subjected to an applied stress field σ_{33}^0 and σ_{31}^0 . This penny-shaped crack may grow into an elliptical crack. The Griffith fracture criterion may be obtained from (27.23) and (27.24) as

$$\begin{aligned} \delta \left\{ -\frac{2}{3}\pi(\sigma_{33}^0)^2 a_1 a_2^2 (1 - \nu)/\mu E(k) \right. \\ - \frac{2}{3}\pi(\sigma_{31}^0)^2 a_1 a_2 / \mu \left[\frac{\nu}{1 - \nu} \frac{a_2 \{ F(k) - E(k) \}}{a_1^2 - a_2^2} + \frac{E(k)}{a_2} \right] \\ \left. + 2\pi a_1 a_2 \gamma \right\} = 0, \end{aligned} \quad (27.29)$$

where

$$\delta \{ \cdot \} = \left[\frac{\partial}{\partial a_1} \{ \cdot \} + m \frac{\partial}{\partial a_2} \{ \cdot \} \right] \delta a_1. \quad (27.30)$$

The limits $a_1 \rightarrow a$ and $a_2 \rightarrow a$ are taken in (27.29), since the crack is initially penny-shaped. Then, (27.29) and (27.30) lead to

$$\frac{\pi \mu \gamma}{(1-\nu)a} = (\sigma_{33}^0)^2 + \frac{(\sigma_{31}^0)^2 \{(4-\nu) + m(4-3\nu)\}}{(2-\nu)^2(1+m)}. \quad (27.31)$$

The right-hand side of (27.31) is a maximum at $m = 0$. It implies that for a penny-shaped crack with a given size the minimum σ_{33}^0 is obtained when $m = 0$ under a constant σ_{31}^0 , or the minimum σ_{31}^0 is obtained when $m = 0$ under a constant σ_{33}^0 . Therefore, it can be seen that a penny-shaped crack under σ_{31}^0 and σ_{33}^0 grows into an ellipse, and further, that there is no growth perpendicular to the direction of the applied shear stress. Generally it is believed that a penny-shaped crack extends to another penny-shaped crack under the action of σ_{33}^0 alone. According to Smith's theory, a penny-shaped crack extends to an elliptic crack under σ_{31}^0 when a non-zero σ_{31}^0 is also acting.

Crack opening displacement

Since a crack is regarded as an ellipsoidal void by letting $a_3 \rightarrow 0$, the crack opening displacement is obtained by equating the two expressions for the interaction energy (25.17) and (25.18.6),

$$-\frac{1}{2} \int_{\Omega} \sigma_{ij}^0 \epsilon_{ij}^* dD = \frac{1}{2} \int_{\Sigma} \sigma_{ij}^0 n_j [u_i] dS. \quad (27.32)$$

The volume element dD in Ω is written as $2h dS$, where h is the half-thickness of the ellipsoidal void in the x_3 -direction and

$$h = a_3 (1 - x_1^2/a_1^2 - x_2^2/a_2^2). \quad (27.33)$$

Thus, we have

$$-\sigma_{ij}^0 \epsilon_{ij}^* 2h = \sigma_{ij}^0 n_j [u_i] \quad (27.34)$$

since (27.32) holds for any Σ . For $\sigma_{ij}^0 = \sigma_{33}^0$, (27.34) gives $[u_3] = 2h \epsilon_{33}^*$ and

$[u_1] = [u_2] = 0$. For $\sigma_{ij}^0 = \sigma_{3i}^0$ ($i = 1$ or 2), (27.34) gives $[u_i] = 4h\epsilon_{3i}^*$ and $[u_3] = 0$. The normal on the crack n_i is defined on the upper surface before the opening. $a_3\epsilon_{ij}^*$ are given by (27.4), (27.11), and (27.17). This method is applicable for anisotropic materials.

28. Critical stresses of cracks in anisotropic media

Uniform applied stresses

Consider an elliptic crack ($x_1^2/a_1^2 + x_2^2/a_2^2 = 1$) in an anisotropic medium. The x coordinate axes are chosen parallel to the principal axes of the ellipsoid, $x_1^2/a_1^2 + x_2^2/a_2^2 + x_3^2/a_3^2 = 1$. The elliptic crack is degenerated from the ellipsoid by taking $a_3 \rightarrow 0$. Let σ_{ij}^0 be the applied stress at infinity.

The boundary conditions on the crack surface are

$$\begin{aligned}\sigma_{33}^0 + \sigma_{33} &= 0, \\ \sigma_{31}^0 + \sigma_{31} &= 0, \\ \sigma_{32}^0 + \sigma_{32} &= 0,\end{aligned}\tag{28.1}$$

where σ_{ij} is the stress disturbance due to the crack. This stress disturbance is simulated by the eigenstress derived from an equivalent ellipsoidal inclusion with eigenstrain ϵ_{ij}^* . From (17.12) we have

$$\sigma_{ij} = C_{ijkl} \left\{ \frac{a_1 a_2 a_3}{4\pi} C_{pqmn} \epsilon_{mn}^* \int_{S^2} G_{kpjq}(\bar{\xi}) \xi^{-3} dS(\bar{\xi}) - \epsilon_{kl}^* \right\},\tag{28.2}$$

where

$$\begin{aligned}\xi &= (a_1^2 \bar{\xi}_1^2 + a_2^2 \bar{\xi}_2^2 + a_3^2 \bar{\xi}_3^2)^{1/2}, \\ G_{kpjq}(\bar{\xi}) &= N_{kp}(\bar{\xi}) \bar{\xi}_l \bar{\xi}_q / D(\bar{\xi}).\end{aligned}\tag{28.3}$$

Since we are interested in the limiting case $a_3 \rightarrow 0$, the integral in (28.2) is modified. As shown in (17.42), σ_{3j} becomes zero for this case when ϵ_{ij}^* is finite. However, σ_{3j} becomes nonzero if ϵ_{ij}^* is infinite and $a_3\epsilon_{ij}^*$ is finite for $a_3 \rightarrow 0$. Nonzero values of σ_{ij} are necessary in order to satisfy the boundary conditions (28.1).

It holds that

$$\begin{aligned} & a_1 a_2 a_3 \int_{S^2} G_{kp\ell q}(\bar{\xi}) \xi^{-3} dS(\bar{\xi}) \\ &= \int_{S^2} \frac{G_{kp\ell q}(\bar{\xi}) dS(\bar{\xi})}{a_1^2 a_2^2 a_3^2 (\bar{\xi}_1^2/a_2^2 a_3^2 + \bar{\xi}_2^2/a_3^2 a_1^2 + \bar{\xi}_3^2/a_1^2 a_2^2)^{3/2}}. \end{aligned} \quad (28.4)$$

Introducing the following transformation:

$$\begin{aligned} \bar{\xi}_1 &= (1 - \bar{\xi}_3^2)^{1/2} \cos \theta, \\ \bar{\xi}_2 &= (1 - \bar{\xi}_3^2)^{1/2} \sin \theta, \\ t &= \bar{\xi}_3 / (1 - \bar{\xi}_3^2)^{1/2}, \\ dS(\bar{\xi}) &= d\bar{\xi}_3 d\theta = (1 - \bar{\xi}_3^2)^{3/2} dt d\theta, \end{aligned} \quad (28.5)$$

and since $G_{kp\ell q}(\bar{\xi})$ is a polynomial of degree zero, we express (28.4) as

$$\int_0^{2\pi} \frac{d\theta}{a_1^2 a_2^2 a_3^2} \int_{-\infty}^{\infty} \frac{G_{kp\ell q}(\cos \theta, \sin \theta, t) dt}{(\cos^2 \theta/a_2^2 a_3^2 + \sin^2 \theta/a_3^2 a_1^2 + t^2/a_1^2 a_2^2)^{3/2}}. \quad (28.6)$$

When integration by parts with respect to t is performed, using

$$\begin{aligned} & \frac{\partial}{\partial t} \left\{ t / (\cos^2 \theta/a_2^2 a_3^2 + \sin^2 \theta/a_3^2 a_1^2 + t^2/a_1^2 a_2^2)^{1/2} \right\} \\ &= (\cos^2 \theta/a_2^2 a_3^2 + \sin^2 \theta/a_3^2 a_1^2) \\ &\times (\cos^2 \theta/a_2^2 a_3^2 + \sin^2 \theta/a_3^2 a_1^2 + t^2/a_1^2 a_2^2)^{-3/2} \end{aligned} \quad (28.7)$$

and

$$(G_{kp\ell q})_{t=\infty} = -(G_{kp\ell q})_{t=-\infty} = G_{kp\ell q}(0, 0, 1), \quad (28.8)$$

(28.6) becomes

$$2G_{kp\ell q}(0, 0, 1) \int_0^{2\pi} \frac{d\theta}{a_1 a_2 (\cos^2 \theta/a_2^2 + \sin^2 \theta/a_1^2)} - a_3 \int_0^{2\pi} \frac{d\theta}{a_1^2 a_2^2} \\ \times \int_{-\infty}^{\infty} \frac{t \partial G_{kp\ell q}(\cos \theta, \sin \theta, t)/\partial t dt}{(\cos^2 \theta/a_2^2 + \sin^2 \theta/a_1^2)(\cos^2 \theta/a_2^2 + \sin^2 \theta/a_1^2 + a_3^2 t^2/a_1^2 a_2^2)^{1/2}}. \quad (28.9)$$

Noting that

$$\int_0^{2\pi} \frac{d\theta}{\cos^2 \theta/a_2^2 + \sin^2 \theta/a_1^2} = 2\pi a_1 a_2 \quad (28.10)$$

and that $a_3 \ll a_1, a_2$, we finally have

$$\sigma_{ij} = (1/4\pi) C_{ijk\ell} C_{pqmn} \epsilon_{mn}^* \{ 4\pi G_{kp\ell q}(0, 0, 1) - a_3 \Pi_{kp\ell q} \} - C_{ijk\ell} \epsilon_{kl}^*, \quad (28.11)$$

where

$$\Pi_{kp\ell q} = \int_0^{2\pi} \frac{d\theta}{a_1^2 a_2^2} \int_{-\infty}^{\infty} \frac{t \partial G_{kp\ell q}(\cos \theta, \sin \theta, t)/\partial t dt}{(\cos^2 \theta/a_2^2 + \sin^2 \theta/a_1^2)^{3/2}} \\ = - \int_0^{2\pi} \frac{d\theta}{a_1^2 a_2^2} \int_{-\infty}^{\infty} \frac{G_{kp\ell q}^*(\cos \theta, \sin \theta, t)}{(a_2^{-2} \cos^2 \theta + a_1^{-2} \sin^2 \theta)^{3/2}} dt \quad (28.12)$$

after integrating by parts, where $G_{kp\ell q}^*$ is chosen so that $[t G_{kp\ell q}^*]_{t=-\infty}^{t=\infty} = 0$ and $G_{kp\ell q} - G_{kp\ell q}^* = \text{constant}$, namely, the quotient of two polynomials of degree 6 divided by a polynomial of degree 6 is rewritten in a constant plus a polynomial of degree 4 divided by a polynomial of degree 6. The constant is ignored when the integration by parts is performed. $G_{kp\ell q} = G_{kp\ell q}^*$ when ℓ and q are not 3 at the same time.

As has been shown already by (17.42), the first and third terms in (28.11) cancel each other out on the crack surface. The boundary conditions (28.1) therefore become

$$\sigma_{3j}^0 - (1/4\pi) C_{3jk\ell} C_{pqmn} a_3 \epsilon_{mn}^* \Pi_{kp\ell q} = 0 \quad (j = 1, 2, 3) \quad (28.13)$$

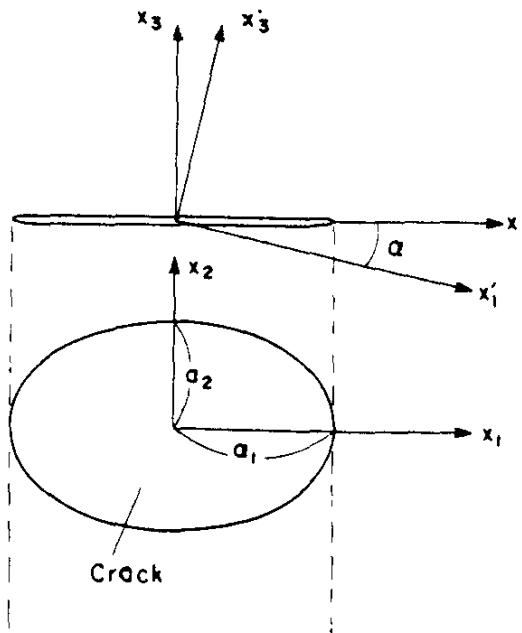


Fig. 28.1. The x' coordinate system coincides with the crystallographic directions of a crystal.

or

$$\sigma_{3j}^0 + L_{3jmn} a_3 \epsilon_{mn}^* = 0 \quad (j = 1, 2, 3), \quad (28.14)$$

where

$$L_{ijmn} = -(1/4\pi) C_{ijkl} C_{pqmn} \Pi_{kplq}. \quad (28.15)$$

The set of three equations (28.14) can be solved for the remaining eigenstrains.

Mura and S.C. Lin (1974) and Cheng (1977) have considered (28.14) in some detail when the crack is oriented as shown in Fig. 28.1, where the x_2 -axis coincides with one of the crystallographic directions, say the x'_2 -axis. The other two crystallographic directions are taken as the x'_1 - and x'_3 -axes.

For a simple tension case ($\sigma_{3j}^0 = \sigma_{33}^0$, normal to the crack surface), they have found that

$$a_3 \epsilon_{33}^* = -\sigma_{33}^0 L_{3131}/\Delta, \quad a_3 \epsilon_{31}^* = \sigma_{33}^0 L_{3133}/2\Delta, \quad (28.16)$$

and for a simple shear case ($\sigma_{3j}^0 = \sigma_{31}^0$, tangential to the crack surface),

$$a_3 \epsilon_{33}^* = \sigma_{31}^0 L_{3331}/\Delta, \quad a_3 \epsilon_{31}^* = -\sigma_{31}^0 L_{3333}/2\Delta, \quad (28.17)$$

where

$$\Delta = L_{3333} L_{3131} - L_{3133} L_{3331}. \quad (28.18)$$

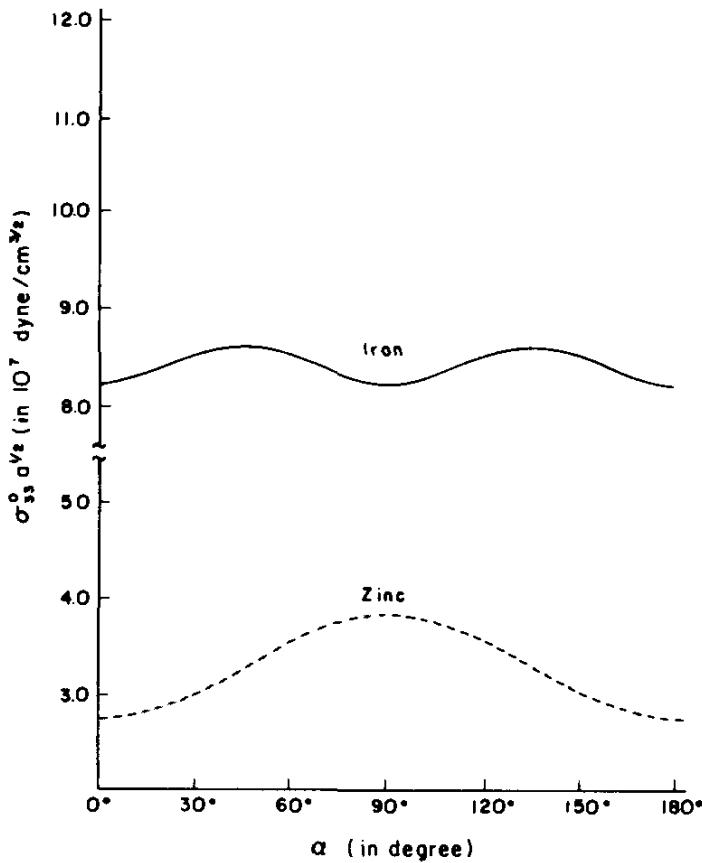


Fig. 28.2. The critical stress in simple tension.

All other components of ϵ_{ij}^* can be taken as zero. It is found that $L_{3133} = 0$, $L_{3331} = 0$ for $\alpha = 0$.

The interaction energy is

$$\Delta W = -\frac{1}{2}(4\pi/3)a_1 a_2 \sigma_{3j}^0 a_3 \epsilon_{3j}^*. \quad (28.19)$$

For a penny-shaped crack ($a_1 = a_2 = a$), the critical stress is obtained from

$$\partial(\Delta W + 2\pi a^2 \gamma)/\partial a = 0, \quad (28.20)$$

where γ is the surface energy. Numerical results obtained by Mura and S.C. Lin (1974) and Cheng (1977) are shown in Fig. 28.2 and Fig. 28.3 for simple tension and shear, respectively. Two materials, iron and zinc, are examined. In the case of zinc, the x'_1 , x'_2 -plane is taken as the basal plane. The physical constants are taken from the American Institute of Physics Handbook (1972). For iron, $\lambda = 14.1$, $\mu = 11.6$, and $\mu' = -13.6$ with units of 10^{11} dyne/cm 2 , and $\gamma = 2.0 \times 10^3$ dyne/cm. For zinc, $C_{11} = 16.1$, $C_{33} = 6.1$, $C_{44} = 3.83$, $C_{12} = 3.42$, and $C_{13} = 5.01$ with units of 10^{11} dyne/cm 2 , and $\gamma = 708$ dyne/cm. It is interesting to observe that the critical σ_{33}^0 of iron takes a minimum value at

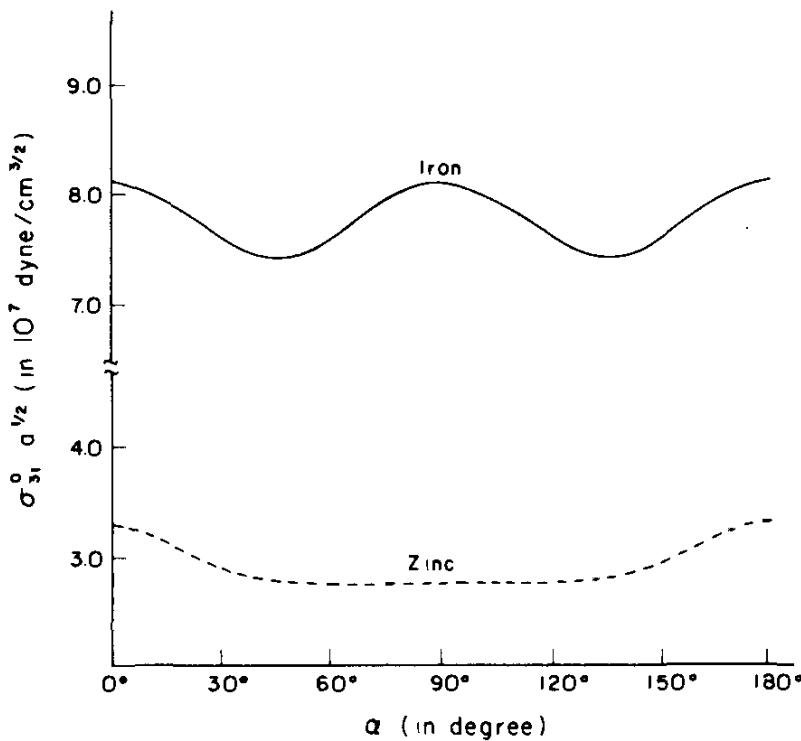


Fig. 28.3. The critical stress in simple shear.

$\alpha = 90^\circ$, while the corresponding quantity for zinc obtains its maximum at $\alpha = 90^\circ$. The critical value of σ_{31}^0 for iron reaches a maximum value at $\alpha = 90^\circ$, but that of zinc takes a minimum value at $\alpha = 90^\circ$.

Non-uniform applied stresses

If the applied stress is given in the form of

$$\sigma_{3j}^0 = \sigma_{3j}^p x_p / a_p, \quad (28.21)$$

the eigenstrain used in the equivalent inclusion method becomes

$$\epsilon_{ij}^* = \epsilon_{ij}^p x_p / a_p. \quad (28.22)$$

The eigenstress inside the equivalent inclusion Ω can be obtained from (19.12), where $S^* = S^2$. When $a_3 \ll a_1, a_2$, we have, as seen in (19.17),

$$\begin{aligned} \sigma_{ij} &= C_{ijkl} \left\{ C_{pqmn} \epsilon_{mn}^s (x_s / a_s) G_{kplq}(0, 0, 1) - \epsilon_{kl}^p x_p / a_p \right\} \\ &\quad + a_3 L_{ijmn}^{st} \epsilon_{mn}^s x_t / a_t, \end{aligned} \quad (28.23)$$

where

$$L_{ijmn}^{st} = - (3/4\pi) C_{ijkl} C_{pqmn} \int_{S^2} \frac{a_1 a_2 \xi_s \xi_t \bar{\xi}_3 \partial G_{kplq}(\bar{\xi}) / \partial \bar{\xi}_3}{(a_1^2 \cos^2 \theta + a_2^2 \sin^2 \theta)^{5/2} (1 - \bar{\xi}_3^2)^{1/2}} dS(\bar{\xi}), \quad (28.24)$$

and

$$\xi_1 = a_1 \xi_1, \quad \xi_2 = a_2 \xi_2, \quad \xi_3 = 0,$$

$$\xi = (\cos \theta, \sin \theta, 0),$$

$$\bar{\xi}_1 = (1 - \bar{\xi}_3^2)^{1/2} \cos \theta, \quad (28.25)$$

$$\bar{\xi}_2 = (1 - \bar{\xi}_3^2)^{1/2} \sin \theta,$$

$$dS(\bar{\xi}) = d\bar{\xi}_3 d\theta.$$

The boundary conditions are

$$\sigma_{3j}^0 + \sigma_{3j} = 0 \quad (j = 1, 2, 3). \quad (28.26)$$

It can also be shown that the first two terms in (28.23) cancel each other for σ_{3j} . However, the above boundary conditions can only be satisfied by taking $a_3 \epsilon_{mn}^s = \text{finite}$ for $a_3 \rightarrow 0$. The applied stress (28.21) cannot be given arbitrarily. Since $\sigma_{3j,j}^0 = 0$ must be satisfied, we restrict attention to the case where

$$\sigma_{31}^1 = \sigma_{32}^2 = \sigma_{33}^3 = 0. \quad (28.27)$$

For simplicity, the crack plane is assumed to be one of the crystallographic planes ($\alpha = 0$ in Fig. 28.1). Then, for simple tension, $\sigma_{33}^0 = \sigma_{33}^1 x_1/a_1 + \sigma_{33}^2 x_2/a_2$,

$$\begin{aligned} a_3 \epsilon_{33}^1 &= -\sigma_{33}^1 / L_{3333}^{11}, \\ a_3 \epsilon_{33}^2 &= -\sigma_{33}^2 / L_{3333}^{22}, \end{aligned} \quad (28.28)$$

and for simple shear, $\sigma_{31}^0 = \sigma_{31}^2 x_2/a_2$,

$$a_3 \epsilon_{31}^2 = -\sigma_{31}^2 / L_{3131}^{22}. \quad (28.29)$$

If the material is isotropic,

$$\begin{aligned} L_{3333}^{11} &= -\mu \left\{ (k')^2 F(k) + (2k^2 - 1) E(k) \right\} / (1 - \nu) a_2 k^2, \\ L_{3333}^{22} &= \mu \left\{ (k')^2 F(k) - (k^2 + 1) E(k) \right\} / (1 - \nu) a_2 k^2, \\ L_{3131}^{22} &= \mu \left\{ (k')^2 F(k) - (k^2 + 1) E(k) \right\} / a_2 k^2, \end{aligned} \quad (28.30)$$

where $k^2 = 1 - a_2^2/a_1^2$, $(k')^2 = a_2^2/a_1^2$, and $E(k)$ and $F(k)$ are defined by (11.25).

* *Π integrals for a penny-shaped crack ($a_1 = a_2$)*

The integrals defined by (28.12) are given below for cubic and hexagonal crystals when the crystallographic directions coincide with the x coordinate system ($α = 0$ in Fig. 28.1). The crack plane is the basal plane for hexagonal crystals.

* *Π integrals for cubic crystals*

$$\begin{aligned} \Pi_{1111} = \Pi_{2222} &= -\frac{\pi}{aa_1} \int_0^1 \frac{(1-x^2)^{1/2}(\kappa + \gamma x^2)}{p(p+q)} dx \\ &\quad - \frac{2\pi}{aa_1} \int_0^1 \left[\frac{\mu^2 + \kappa x^2}{pq(1-x^2)^{1/2}} - \frac{a}{\mu\sqrt{2}(1-x)^{1/2}} \right] dx - \frac{2\pi\sqrt{2}}{\mu a_1}, \\ \Pi_{3333} &= -\frac{\pi\gamma}{aa_1} \int_0^1 \frac{x^2(1-x^2)^{1/2}}{p(p+q)} dx \\ &\quad - \frac{\pi}{aa_1} \int_0^1 \frac{\mu^2 x^4 (1-x^2)^{1/2} (c - 2b - 3cx^2)}{p^3 q} dx \\ &\quad - \frac{4\pi}{aa_1} \int_0^1 \left[\frac{x^2(\kappa - 2\mu^2)}{pq(1-x^2)^{1/2}} - \frac{\kappa - 2\mu^2}{\sqrt{2}(1-x)^{1/2}} \right] dx - \frac{4\pi\sqrt{2}}{aa_1} (\kappa - 2\mu^2) \\ &\quad - \frac{4\pi}{aa_1} \int_0^1 \left[\frac{(p^2 + q^2)\mu^2 b x^4 (1-2x^2)}{(pq)^3 (1-x^2)^{1/2}} + \frac{\mu^2 b}{\sqrt{2}(1-x)^{1/2}} \right] dx \\ &\quad + \frac{4\pi\sqrt{2}}{aa_1} \mu^2 b, \end{aligned}$$

$$\begin{aligned}
\Pi_{3311} = \Pi_{3322} &= -\frac{\pi\gamma}{2aa_1} \int_0^1 \frac{(1-x^2)^{3/2}}{p(p+q)} dx \\
&\quad - \frac{2\pi}{aa_1} \int_0^1 \left[\frac{\mu^2 + \kappa - \kappa x^2}{pq(1-x^2)^{1/2}} - \frac{\mu^2}{\sqrt{2}(1-x)^{1/2}} \right] dx - \frac{2\pi\sqrt{2}\mu^2}{aa_1}, \\
\Pi_{1122} = \Pi_{2211} &= \frac{\pi}{aa_1} \int_0^1 \frac{(\kappa + \gamma x^2)(1-x^2)^{1/2}}{p(p+q)} dx \\
&\quad - \frac{2\pi}{aa_1} \int_0^1 \frac{(\kappa + \gamma x^2)(1-x^2)^{1/2}}{pq} dx \\
&\quad - \frac{2\pi}{aa_1} \int_0^1 \left[\frac{\mu^2 + \kappa x^2}{pq(1-x^2)^{1/2}} - \frac{a}{\mu\sqrt{2}(1-x)^{1/2}} \right] dx - \frac{2\pi\sqrt{2}}{a_1\mu}, \\
&\tag{28.31}
\end{aligned}$$

$$\begin{aligned}
\Pi_{1133} = \Pi_{2233} &= -\frac{\pi}{aa_1} \int_0^1 \frac{x^4(1-x^2)^{1/2}(\mu^2 + \kappa x^2)(c - 2b - 3cx^2)}{p^3q} dx \\
&\quad - \frac{4\pi}{aa_1} \int_0^1 \left[\frac{(\mu^2 + \kappa x^2)bx^4(1-2x^2)(\mu^2 + \kappa x^2)}{(pq)^3(1-x^2)^{1/2}} \right. \\
&\quad \left. + \frac{2ba}{\mu\sqrt{2}(1-x)^{1/2}} \right] dx + \frac{8\pi\sqrt{2}b}{a_1\mu} \\
&\quad + \frac{2\pi}{aa_1} \int_0^1 \left[\frac{(8\kappa - \gamma)x^4 + (4\mu^2 - \kappa)x^2}{pq(1-x^2)^{1/2}} - \frac{7\kappa - \gamma + 4\mu^2}{\sqrt{2}(1-x)^{1/2}} \right] dx \\
&\quad + \frac{2\pi\sqrt{2}}{aa_1} (7\kappa - \gamma + 4\mu^2), \\
\Pi_{1212} &= \frac{\pi(\lambda + \mu)}{aa_1} \int_0^1 \frac{(1-x^2)^{1/2}(\mu + \mu'x^2)}{p(p+q)} dx,
\end{aligned}$$

$$\begin{aligned}\Pi_{1313} = \Pi_{2323} &= \frac{\pi\mu'(\lambda + \mu)}{aa_1} \int_0^1 \frac{x^2(1-x^2)^{1/2}}{p(p+q)} dx \\ &+ \frac{2\pi\mu(\lambda + \mu)}{aa_1} \int_0^1 \left[\frac{x^2}{pq(1-x^2)^{1/2}} - \frac{1}{\sqrt{2}(1-x)^{1/2}} \right] dx \\ &+ \frac{2\pi\sqrt{2}\mu(\lambda + \mu)}{aa_1},\end{aligned}$$

where

$$\begin{aligned}a &= \mu^2(\lambda + 2\mu + \mu'), \\ b &= a^{-1}\mu\mu'(2\lambda + 2\mu + \mu'), \\ c &= a^{-1}\mu'^2(3\lambda + 3\mu + \mu'), \\ \kappa &= \mu(\lambda + \mu + \mu'), \\ \gamma &= \mu'(2\lambda + 2\mu + \mu'), \\ p &= [1 + bx^2(1-x^2) + (1-x^2)^2(b+cx^2)/4]^{1/2}, \\ q &= [1 + bx^2(1-x^2)]^{1/2}.\end{aligned}\tag{28.32}$$

* *Π integrals for transversely isotropic materials*

$$\Pi_{1111} = \Pi_{2222}$$

$$\begin{aligned}&= -\frac{\pi}{2a_1} \int_0^1 \left\{ \frac{[f(1-x^2) + hx^2][((d+3e)(1-x^2) + 4fx^2) - g^2x^2(1-x^2)]}{\Delta^{-1}(1-x^2)^{1/2}} \right. \\ &\quad \left. - \frac{4}{\sqrt{2}f(1-x)^{1/2}} \right\} dx - \frac{2\sqrt{2}\pi}{a_1f},\end{aligned}$$

$$\begin{aligned}
\Pi_{3333} &= -\frac{4\pi}{a_1} \int_0^1 \Delta ex^2 (1-x^2)^{1/2} (d-4fx^2) dx \\
&\quad - \frac{4\pi}{a_1} \int_0^1 \left[\frac{\Delta fx^4 (d-4fx^2)}{(1-x^2)^{1/2}} - \frac{f^3 h (d-4f)}{\sqrt{2} (1-x)^{1/2}} \right] dx - \frac{4\pi\sqrt{2}}{a_1} f^3 h (d-4f) \\
&\quad + \frac{8\pi}{a_1} \int_0^1 \left\{ (1-x^2)^{-1/2} \Delta^2 [e(1-x^2) + fx^2]^2 fx^6 \right. \\
&\quad \times \{ g^2(1-2x^2) + (f-h)[d(1-x^2) + fx^2] \\
&\quad \left. + (d-f)[f(1-x^2) + hx^2] \} \right. \\
&\quad \left. - \frac{(f^2 - g^2 - 2fh + dh)}{\sqrt{2} (1-x)^{1/2} fh^2} \right\} dx + \frac{8\pi\sqrt{2}}{a_1} \frac{f^2 - g^2 - 2fh + dh}{fh^2},
\end{aligned}$$

$$\Pi_{1122} = \Pi_{2211}$$

$$\begin{aligned}
&= -\frac{\pi}{2a_1} \int_0^1 \left\{ \frac{[f(1-x^2) + hx^2][(d+3e)(1-x^2) + 4fx^2] - 3g^2x^2(1-x^2)}{\Delta^{-1}(1-x^2)^{1/2}} \right. \\
&\quad \left. - \frac{4}{f\sqrt{2}(1-x)^{1/2}} \right\} dx - \frac{2\pi\sqrt{2}}{a_1 f},
\end{aligned}$$

$$\begin{aligned}
\Pi_{3311} = \Pi_{3322} &= -\frac{\pi}{2a_1} \int_0^1 \left\{ \Delta(1-x^2)^{-1/2} [d(1-x^2) + fx^2] [e(1-x^2) + fx^2] \right. \\
&\quad \left. - \frac{1}{\sqrt{2} h (1-x)^{1/2}} \right\} dx - \frac{\sqrt{2}\pi}{2a_1 h}, \quad (28.33)
\end{aligned}$$

$$\Pi_{1133} = \Pi_{2233}$$

$$\begin{aligned}
&= -\frac{2\pi}{a_1} \int_0^1 \Delta x^2 f (d+e)(1-x^2)^{1/2} dx \\
&\quad - \frac{2\pi}{a_1} (2f^2 + hd + he - g^2) \int_0^1 \left[\frac{\Delta x^4}{(1-x^2)^{1/2}} - \frac{1}{\sqrt{2} (1-x)^{1/2} f^2 h} \right] dx
\end{aligned}$$

$$\begin{aligned}
& - \frac{2\pi\sqrt{2}}{a_1} \frac{(2f^2 + hd + he - g^2)}{f^2 h} \\
& + \frac{24\pi}{a_1} fh \int_0^1 \left[\frac{\Delta x^6}{(1-x^2)^{1/2}} - \frac{1}{f^2 h \sqrt{2} (1-x)^{1/2}} \right] dx + \frac{24\pi\sqrt{2}}{a_1 f} \\
& - \frac{2\pi}{a_1} \int_0^1 4\Delta^2 x^8 fh (1-x^2)^{-1/2} \left\{ (f-e) [df(1-x^2)^2 \right. \\
& \quad \left. + (f^2 + dh - g^2)x^2(1-x^2) + fhx^4] + [e(1-x^2) + fx^2] \right. \\
& \quad \left. \times [(f^2 - 2fd + dh - g^2)(1-x^2) + (2fh - dh - f^2 + g^2)x^2] \right\} dx \\
& + \frac{8\pi}{a_1} \int_0^1 \frac{3fh - dh - eh - f^2 + g^2}{f^2 h \sqrt{2} (1-x)^{1/2}} dx \\
& - \frac{8\pi\sqrt{2}}{a_1 f^2 h} (3fh - dh - eh - f^2 + g^2), \\
\Pi_{1212} &= - \frac{\pi}{2a_1} \int_0^1 \Delta (1-x^2)^{1/2} \left\{ g^2 x^2 - (d-e) [f(1-x^2) + hx^2] \right\} dx, \\
\Pi_{1313} = \Pi_{2323} &= \frac{2\pi}{a_1} \int_0^1 \left\{ \Delta g x^2 (1-x^2)^{-1/2} [e(1-x^2) + fx^2] \right. \\
& \quad \left. - \frac{g}{fh \sqrt{2} (1-x)^{1/2}} \right\} dx + \frac{2\pi\sqrt{2} g}{a_1 fh},
\end{aligned}$$

where $\Delta = 1/\Delta^{-1}$,

$$\begin{aligned}
\Delta^{-1} &= [e(1-x^2) + fx^2] \\
&\times \{ [d(1-x^2) + fx^2] [f(1-x^2) + hx^2] - g^2 x^2 (1-x^2) \}, \quad (28.34)
\end{aligned}$$

and d, h, f, g , and e are defined in (17.30).

29. Stress intensity factors for a flat ellipsoidal crack

Stress intensity factors of cracks as well as critical stresses are important quantities in fracture mechanics, as has been pointed out by Irwin (1948). The stress intensity factors are also important in the study of crack growth rates of fatigue cracks in connection with the Paris law (1964). Stress fields around cracks and their stress intensity factors are well documented in the literature, e.g., Sneddon and Lowengrub (1969), Sneddon (1961), Irwin (1958), Sih and Liebowitz (1968), Paris and Sih (1964), and Kassir and Sih (1975), among others. However, little is known about the stress intensity factors of materials which are anisotropic or when crack shapes are arbitrary.

In this section we follow the work of Willis (1968) on the stress field around a flat ellipsoidal crack in general anisotropic elastic media. The applied stress, σ_{ij}^0 , is assumed to be a polynomial function of the coordinates. The x coordinate system is chosen as shown in Fig. 28.1. The stress disturbance is denoted by σ_{ij} . The total stress field is $\sigma_{ij}^0 + \sigma_{ij}$. The displacement disturbance u_i is related to σ_{ij} through

$$\sigma_{ij} = C_{ijkl} u_{k,l}. \quad (29.1)$$

The boundary conditions are

$$\begin{aligned} \sigma_{3i}^0 + \sigma_{3i} &= 0 \quad \text{when } x_3 = 0, & x_1^2/a_1^2 + x_2^2/a_2^2 &\leq 1, \\ u_i(x) &\rightarrow 0 \quad \text{when } |x| \rightarrow \infty \end{aligned} \quad (29.2)$$

where σ_{3i}^0 is a given polynomial function of x_1 and x_2 .

It can be expected that the displacement field u_i and the corresponding stress σ_{ij} have properties similar to those derived from eigenstrains ϵ_{ij}^* . From (16.2) we have

$$\begin{aligned} u_i(x) &= i(2\pi)^{-3} \iint_{-\infty}^{-\infty} C_{jlmn} \epsilon_{nm}^*(x') \xi_l N_{ij}(\xi) D^{-1}(\xi) \\ &\quad \times \exp\{-i\xi \cdot (x - x')\} d\xi dx'. \end{aligned} \quad (29.3)$$

When

$$\epsilon_{3m}^*(x') dx' = -b_m(x') dS_3(x') = b_m(x') dx'_1 dx'_2 \quad (29.4)$$

is defined on the crack surface, (29.3) becomes

$$u_i(x) = i(2\pi)^{-3} \int_{-\infty}^{\infty} \int_S C_{jlm} b_m(x') \xi_l N_{ij}(\xi) D^{-1}(\xi) \times \exp\{-i\xi \cdot (x - x')\} d\xi dx'_1 dx'_2, \quad (29.5)$$

where $S = S_3$ is the crack surface and \mathbf{b} is called the Burgers vector of the Somigliana dislocation (1914). Eshelby (1957) has found that $|\mathbf{b}|$ is proportional to $(1 - x_1^2/a_1^2 - x_2^2/a_2^2)^{1/2}$ when the applied stress at infinity is uniform. In the present case where the applied stress is polynomial, $\mathbf{b}(x)$ is chosen as

$$\mathbf{b}(x) = \bar{\mathbf{b}}(x_1/a_1 + ix_2/a_2)^p (x_1/a_1 - ix_2/a_2)^q (1 - x_1^2/a_1^2 - x_2^2/a_2^2)^{1/2}, \quad (29.6)$$

where $\bar{\mathbf{b}}$ is a constant vector of a complex number which will be determined later together with p and q , and $p \geq q$. The ξ_3 -integration can be performed by using Cauchy's theorem. Then, (29.5) becomes

$$u_i(x) = (2\pi)^{-2} \sum_{N=1}^3 \iint_{-\infty}^{\infty} d\xi_1 d\xi_2 \frac{C_{jlm} \xi_l^N N_{ij}(\xi^N)}{\partial D(\xi^N)/\partial \xi_3} \iint_S b_m(x') \times \exp\{-i\xi^N \cdot (x - x')\} dx'_1 dx'_2, \quad (29.7)$$

where

$$\xi^N = (\xi_1, \xi_2, \xi_3^N(\xi_1, \xi_2)) \quad (29.8)$$

and $\xi_3 = \xi_3^N(\xi_1, \xi_2)$ are the roots of the equation

$$D(\xi) = 0 \quad (29.9)$$

with negative imaginary parts for $x_3 > 0$. For isotropic materials there is only one double root with a negative imaginary part ($N = 1$). For a double root the residual is obtained from $\partial \{(\xi_q \xi_l N_{kp}(\xi) D^{-1}(\xi))(\xi_3 - \xi_3^N)^2\} / \partial \xi_3$.

The stress components are

$$\sigma_{ij}(x) = \frac{-i}{4\pi^2} \sum_{N=1}^3 \iint_{-\infty}^{\infty} d\xi_1 d\xi_2 \frac{C_{ijk} C_{pqm} \xi_q^N \xi_l^N N_{kp}(\xi^N)}{\partial D(\xi^N)/\partial \xi_3} \iint_S b_m(x') \times \exp\{-i\xi^N \cdot (x - x')\} dx'_1 dx'_2. \quad (29.10)$$

The functions $\xi_3^N(\xi_1, \xi_2)$ are homogeneous of degree 1. It should be noted that $N_{k,p}(\xi)$ and $D(\xi)$ are homogeneous of degrees 4 and 6, respectively.

The complicated integrations in (29.7) and (29.10), when $b(x)$ is given by (29.6), are carried out by Willis (1968). The results are

$$\begin{aligned}
 u_i(x) = & \frac{(-1)^{q+1}}{2\pi} \bar{b}_m \int_0^{2\pi} d\phi \sum_{N=1}^3 F_{im}(\eta_1/a_1, \eta_2/a_2, \xi_3^N(\eta_1/a_1, \eta_2/a_2)) e^{i(p-q)\phi} \\
 & \times \frac{\partial}{\partial g} \int_1^{\{(g+1)/(g-1)\}^{1/2}} \left\{ \left[-g + \frac{\sqrt{g^2-1}}{2} \left(w + \frac{1}{w} \right) \right]^p \right. \\
 & \quad \times \left[g + \frac{\sqrt{g^2-1}}{2} \left(w + \frac{1}{w} \right) \right]^q \\
 & \quad + \left[-g + \frac{\sqrt{g^2-1}}{2} \left(w + \frac{1}{w} \right) \right]^{p+1} \\
 & \quad \left. \times \left[g + \frac{\sqrt{g^2-1}}{2} \left(w + \frac{1}{w} \right) \right]^{q+1} \right\} \frac{dw}{w}, \\
 \end{aligned} \tag{29.11}$$

$$\begin{aligned}
 \sigma_{ij}(x) = & \frac{(-1)^{q+1}}{2\pi} \bar{b}_m \int_0^{2\pi} d\phi \\
 & \times \sum_{N=1}^3 F_{ijm}(\eta_1/a_1, \eta_2/a_2, \xi_3^N(\eta_1/a_1, \eta_2/a_2)) e^{i(p-q)\phi} \\
 & \times \frac{\partial^2}{\partial g^2} \int_1^{\{(g+1)/(g-1)\}^{1/2}} \left\{ \left[-g + \frac{\sqrt{g^2-1}}{2} \left(w + \frac{1}{w} \right) \right]^p \right. \\
 & \quad \times \left[g + \frac{\sqrt{g^2-1}}{2} \left(w + \frac{1}{w} \right) \right]^q \\
 & \quad + \left[-g + \frac{\sqrt{g^2-1}}{2} \left(w + \frac{1}{w} \right) \right]^{p+1} \\
 & \quad \left. \times \left[g + \frac{\sqrt{g^2-1}}{2} \left(w + \frac{1}{w} \right) \right]^{q+1} \right\} \frac{dw}{w}. \\
 \end{aligned} \tag{29.12}$$

where

$$\begin{aligned} F_{im}(\xi_1, \xi_2, \xi_3^N) &= C_{jlm} \xi_l^N N_{ij}(\xi^N) / \partial D(\xi^N) / \partial \xi_3, \\ F_{jm}(\xi_1, \xi_2, \xi_3^N) &= C_{ijkl} C_{pqm} \xi_q^N \xi_l^N N_{kp}(\xi^N) / \partial D(\xi^N) / \partial \xi_3, \end{aligned} \quad (29.13)$$

$$\begin{aligned} \eta_1 &= \cos \phi, \quad \eta_2 = \sin \phi, \quad \eta_1 + i\eta_2 = \exp(i\phi), \\ y_1 &= x_1/a_1, \quad y_2 = x_2/a_2, \quad y = (y_1^2 + y_2^2)^{1/2}, \\ g &= -x_3 \xi_3^N (\eta_1/a_1, \eta_2/a_2) - \eta \cdot y, \\ D(\xi_1, \xi_2, \xi_3^N) &= 0. \end{aligned} \quad (29.14)$$

When $x_3 = 0$,

$$\begin{aligned} \sigma_{ij}(x_1, x_2, 0) &= \frac{1}{4} i \bar{b}_m \int_0^{2\pi} \sum_{N=1}^3 F_{ijm}(\eta_1/a_1, \eta_2/a_2, \xi_3^N(\eta_1/a_1, \eta_2/a_2)) \\ &\times e^{i(p-q)\phi} \frac{\partial^2}{\partial(\eta \cdot y)^2} [K_{pq}(\eta \cdot y) - K_{p+1,q+1}(\eta \cdot y)] d\phi, \end{aligned} \quad (29.15)$$

where

$$\begin{aligned} K_{pq}(\eta \cdot y) &= \sum_{m=0}^p \sum_{\substack{n=0 \\ m+n \text{ even}}}^q (-1)^{(m-n)/2} \binom{p}{m} \binom{q}{n} \left(\frac{m+n}{2} \right) \left(\frac{1 - (\eta \cdot y)^2}{4} \right)^{(m+n)/2} \\ &\times (\eta \cdot y)^{p+q-m-n}. \end{aligned} \quad (29.16)$$

It is seen that $\sigma_{ij}(x_1, x_2, 0)$ is a polynomial of degree $(p+q)$ in y_1, y_2 when $|y| < 1$. The boundary conditions (29.2) are satisfied by choosing suitable constants $\bar{b}_1, \bar{b}_2, \bar{b}_3$, when σ_{3i}^0 is a polynomial of degree $(p+q)$ in y_1, y_2 .

The relative displacement of the two crack surfaces is

$$u_i(x_1, x_2, +0) - u_i(x_1, x_2, -0) = b_i(x_1, x_2). \quad (29.17)$$

This is based upon the fact that the Burgers vector \mathbf{b} (or dislocation density on a plane) is defined by the multiple-valuedness of the displacement across the

crack plane. Since the displacement is symmetric about the crack plane, $u_i(x_1, x_2, +0) = \frac{1}{2}b_i(x_1, x_2)$. When \bar{b} is determined by the boundary conditions, the elastic field is completely determined by (29.11) and (29.12).

The interaction energy (25.17) becomes

$$\Delta W = -\frac{1}{2} \int_V \sigma_{m3}^0 \epsilon_{3m}^* dx' = \frac{1}{2} \int_S \sigma_{3m}^0 b_m(x') dx'_1 dx'_2 \quad (29.18)$$

which is used for the Griffith fracture criterion calculation.

In (29.16) we have $K_{00} = 1$, $K_{11} = \frac{1}{2}(\eta \cdot y)^2 + \frac{1}{2}$, $K_{10} = (\eta \cdot y)$, and $K_{21} = \frac{1}{2}(\eta \cdot y)^3 + \frac{1}{2}(\eta \cdot y)$. These values are used for a constant applied stress $p = q = 0$ and a linear applied stress $p = 1$, $q = 0$.

Uniform applied stresses

The integral with respect to w in (29.12) is denoted by I . Then, for $p = q = 0$,

$$\frac{\partial^2 I}{\partial g^2} = -\frac{1}{2} \log \frac{g+1}{g-1} + \frac{g}{g^2-1}. \quad (29.19)$$

Since the imaginary part of g is positive, $-\pi \leq \arg(g+1)/(g-1) \leq 0$. When $x_3 = 0$ and $y < 1$, we have $g = -(\eta \cdot y)$ and $\log(g+1)/(g-1) = \log|(1+g)/(1-g)| - i\pi$. Furthermore, $\log|(1+g)/(1-g)|$ and $g/(g^2-1)$ are odd functions of η for $x_3 = 0$. On the other hand, $\sum_{N=1}^3 F_{ijm}$ is an even function of η . Therefore, for $x_3 = 0$ and $y < 1$, $-\frac{1}{2}\log|(1+g)/(1-g)|$ and $g/(g^2-1)$ do not contribute to (29.12) and we have

$$\sigma_{ij} = -\frac{1}{4}i \int_0^{2\pi} \sum_{N=1}^3 F_{ijm}(\eta_1/a_1, \eta_2/a_2, \xi_3^N(\eta_1/a_1, \eta_2/a_2)) \bar{b}_m d\phi. \quad (29.20)$$

The boundary conditions (29.2) determine the unknown constants \bar{b}_1 , \bar{b}_2 , and \bar{b}_3 .

The stress intensity factor at a crack tip is defined by

$$k_{ij} = \lim_{r \rightarrow 0} (2\pi r)^{1/2} \sigma_{ij}, \quad (29.21)$$

where r is the normal distance from the edge of the crack. For a flat ellipsoidal crack,

$$r = (y-1)/\left(x_1^2/a_1^4 + x_2^2/a_2^4\right)^{1/2}. \quad (29.22)$$

The stress components σ_{ij} in (29.21) can be evaluated from (29.12) in the neighborhood of the crack edge. When x is close to the crack edge, the second term in (29.19) is dominant. Accordingly, we consider the integral

$$J(x) = \frac{1}{2\pi} \int_0^{2\pi} \frac{f(\eta)}{g^2 - 1} d\phi, \quad (29.23)$$

where $g = -(\eta \cdot y)$. We have

$$J(x) = \frac{1}{2\pi} \int_0^{2\pi} \frac{f(\eta) - f(\bar{y})}{g^2 - 1} d\phi + \frac{f(\bar{y})}{2\pi} \int_0^{2\pi} \frac{d\phi}{g^2 - 1}, \quad (29.24)$$

where $\bar{y} = y/y$. The first term in (29.24) remains bounded as $y \rightarrow 1$, so that

$$J(x) = \frac{f(\bar{y})}{2\pi} \int_0^{2\pi} \frac{d\phi}{g^2 - 1} + 0(1) = if(\bar{y})/(y^2 - 1)^{1/2} + 0(1). \quad (29.25)$$

Thus,

$$\begin{aligned} \sigma_{ij} &= \frac{-1}{2\pi} \int_0^{2\pi} \sum_{N=1}^3 F_{ijm} \bar{b}_m \frac{g}{g^2 - 1} d\phi + 0(1) \\ &= \left[\frac{i(\eta \cdot y)}{(y^2 - 1)^{1/2}} \sum_{N=1}^3 F_{ijm} (\eta_1/a_1, \eta_2/a_2, \xi_3^N(\eta_1/a_1, \eta_2/a_2)) \bar{b}_m \right]_{y \rightarrow 1} + 0(1) \end{aligned} \quad (29.26)$$

The stress intensity factor (29.21) becomes

$$\begin{aligned} k_{ij} &= i\sqrt{\pi} \left(x_1^2/a_1^4 + x_2^2/a_2^4 \right)^{-1/4} \\ &\times \sum_{N=1}^3 F_{ijm} (x_1/a_1^2, x_2/a_2^2, \xi_3^N(x_1/a_1^2, x_2/a_2^2)) \bar{b}_m. \end{aligned} \quad (29.27)$$

The stress intensity factor of a flat ellipsoidal crack in transversely isotropic media has been investigated by Shield (1951), Kassir and Sih (1968) and Hoenig (1978). Their methods differ from the Willis approach (1968). Explicit expressions for k_1 , k_{II} , and k_{III} are presented by Hoenig.

For isotropic materials, (29.9) has two roots. The root with a negative imaginary part is

$$\xi_3(\xi_1, \xi_2) = -i(\xi_1^2 + \xi_2^2)^{1/2}. \quad (29.28)$$

Using (3.34), we can write (29.13) as

$$\begin{aligned} F_{311}(\xi_1, \xi_2, \xi_3) &= \frac{-i\mu(\lambda + \mu)}{\lambda + 2\mu} \left(\xi_1^2 + \frac{1}{2} \frac{\lambda + 2\mu}{\lambda + \mu} \xi_2^2 \right) / (\xi_1^2 + \xi_2^2)^{1/2}, \\ F_{312} = F_{321}(\xi_1, \xi_2, \xi_3) &= \frac{-i\mu(\lambda + \mu)}{\lambda + 2\mu} \frac{\lambda}{2(\lambda + \mu)} \xi_1 \xi_2 / (\xi_1^2 + \xi_2^2)^{1/2}, \\ F_{313} = F_{323} = F_{331} = F_{332} &= 0, \end{aligned} \quad (29.29)$$

$$\begin{aligned} F_{322}(\xi_1, \xi_2, \xi_3) &= \frac{-i\mu(\lambda + \mu)}{\lambda + 2\mu} \left(\frac{1}{2} \frac{\lambda + 2\mu}{\lambda + \mu} \xi_1^2 + \xi_2^2 \right) / (\xi_1^2 + \xi_2^2)^{1/2}, \\ F_{333}(\xi_1, \xi_2, \xi_3) &= \frac{-i\mu(\lambda + \mu)}{\lambda + 2\mu} (\xi_1^2 + \xi_2^2)^{1/2}; \end{aligned}$$

and (29.20) becomes

$$\begin{aligned} \sigma_{31} &= -\frac{\mu}{4} \frac{\lambda + \mu}{\lambda + 2\mu} \int_0^{2\pi} \left(\frac{\eta_1^2}{a_1^2} + \frac{1}{2} \frac{\lambda + 2\mu}{\lambda + \mu} \frac{\eta_2^2}{a_2^2} \right) \bar{b}_1 \frac{d\phi}{(\eta_1^2/a_1^2 + \eta_2^2/a_2^2)^{1/2}}, \\ \sigma_{32} &= -\frac{\mu}{4} \frac{\lambda + \mu}{\lambda + 2\mu} \int_0^{2\pi} \left(\frac{1}{2} \frac{\lambda + 2\mu}{\lambda + \mu} \frac{\eta_1^2}{a_1^2} + \frac{\eta_2^2}{a_2^2} \right) \bar{b}_2 \frac{d\phi}{(\eta_1^2/a_1^2 + \eta_2^2/a_2^2)^{1/2}}, \\ \sigma_{33} &= -\frac{\mu}{4} \frac{\lambda + \mu}{\lambda + 2\mu} \int_0^{2\pi} \bar{b}_3 (\eta_1^2/a_1^2 + \eta_2^2/a_2^2)^{1/2} d\phi. \end{aligned} \quad (29.30)$$

If $\sigma_{ij}^0 = \sigma_{33}^0$, boundary conditions (29.2) yield $\bar{b}_1 = \bar{b}_2 = 0$, and

$$\bar{b}_3 = \frac{2(1-\nu)}{\mu E(k)} a_2 \sigma_{33}^0. \quad (29.31)$$

$E(k)$ is defined by (11.25). Result (29.31) is consistent with (27.23), since

$2a_3\epsilon_{33}^* = \bar{b}_3$. The stress intensity factor k_{33} in (29.27) is denoted by k_1 for $\sigma_{ij}^0 = \sigma_{33}^0$, i.e.

$$k_1 = \sigma_{33}^0 a_2 \sqrt{\pi} (x_1^2/a_1^4 + x_2^2/a_2^4)^{1/4} / E(k). \quad (29.32)$$

For a penny-shaped crack where $a_1 = a_2 = a$ and $E(k) = \frac{1}{2}\pi$,

$$k_1 = \sigma_{33}^0 (\pi a)^{1/2} / \frac{1}{2}\pi. \quad (29.32.1)$$

The result (29.32) has been obtained by Sadowsky and Sternberg (1949) while the special case of a penny-shaped crack has been considered by Sneddon (1946).

In a similar manner, we obtain the stress intensity factor for shear stresses. When $\sigma_{ij}^0 = \sigma_{31}^0$, boundary conditions (29.2) and (29.30) lead to

$$\bar{b}_1 = \sigma_{31}^0 a_2 k^2 / \frac{\mu}{2(1-\nu)} \{ (k^2 - \nu) E(k) + \nu(1-k^2) F(k) \} \quad (29.33)$$

and $\bar{b}_2 = \bar{b}_3 = 0$, where $F(k)$ and $E(k)$ are defined in (11.25). The stress intensity factors (29.27) become

$$k_{31} = \sigma_{31}^0 a_2 \sqrt{\pi} k^2 \frac{(x_1^2/a_1^4 + x_2^2/a_2^4)^{-3/4} \{ x_1^2/a_1^4 + (1-\nu)x_2^2/a_2^4 \}}{(k^2 - \nu) E(k) + \nu(1-k^2) F(k)}, \quad (29.34)$$

$$k_{32} = \sigma_{31}^0 a_2 \sqrt{\pi} k^2 \frac{(x_1^2/a_1^4 + x_2^2/a_2^4)^{-3/4} \nu(x_1/a_1^2)(x_2/a_2^2)}{(k^2 - \nu) E(k) + \nu(1-k^2) F(k)}. \quad (29.34.1)$$

Denoting the normal vector along the ellipse by ν , we define the stress intensity factor of the second kind k_{II} by

$$k_{II} = k_{31}\nu_1 + k_{32}\nu_2, \quad (29.35)$$

where

$$\begin{aligned} \nu_1 &= (x_1/a_1^2) / (x_1^2/a_1^4 + x_2^2/a_2^4)^{1/2}, \\ \nu_2 &= (x_2/a_2^2) / (x_1^2/a_1^4 + x_2^2/a_2^4)^{1/2}. \end{aligned} \quad (29.36)$$

From this, we have

$$k_{11} = \sigma_{31}^0 a_2 \sqrt{\pi} k^2 \frac{(x_1^2/a_1^4 + x_2^2/a_2^4)^{-1/4} (x_1/a_1^2)}{(k^2 - \nu) E(k) + \nu(1 - k^2) F(k)}. \quad (29.37)$$

In the derivation, the following integrals are used:

$$\begin{aligned} \int_0^{\pi/2} \left(\frac{\cos^2 \phi}{a_1^2} + \frac{\sin^2 \phi}{a_2^2} \right)^{1/2} d\phi &= E(k)/a_2, \\ \int_0^{\pi/2} \frac{\sin^2 \phi}{a_2} \left(\frac{\cos^2 \phi}{a_1^2} + \frac{\sin^2 \phi}{a_2^2} \right)^{-1/2} d\phi &= (1 - 1/k^2) F(k) + E(k)/k^2. \end{aligned} \quad (29.38)$$

Expression (29.37) is consistent with Kassir and Sih's result (1966).

Non-uniform applied stresses

Let us consider the case where the applied stress before the disturbance due to the crack is a linear function of x_1 and x_2 . Then, $p = 1$ and $q = 0$ are chosen in (29.6). When the integral with respect to w in (29.12) is denoted by I , we have

$$\frac{\partial^2 I}{\partial g^2} = \frac{3}{2}g \log \frac{g+1}{g-1} + \frac{2-3g^2}{g^2-1}. \quad (29.39)$$

For $x_3 = 0$ and $y < 1$, $\log(g+1)/(g-1) = \log|(1+g)/(1-g)| - i\pi$, $g \log|(1+g)/(1-g)|$ and $(2-3g^2)/(g^2-1)$ are even functions of η . Since $\sum F_{ijm} e^{i\phi}$ is an odd function of η , (29.12) becomes

$$\begin{aligned} \sigma_{ij} &= -\frac{3i}{4} \int_0^{2\pi} \sum_{N=1}^3 F_{ijm}(\eta_1/a_1, \eta_2/a_2, \xi_3^N(\eta_1/a_1, \eta_2/a_2)) \\ &\quad \times (\eta_1 + i\eta_2) \bar{b}_m(\eta \cdot y) d\phi. \end{aligned} \quad (29.40)$$

The boundary conditions (29.2) determine the unknown constants.

For a stress near the crack edge, the second term in (29.39) is dominant.

Thus, for $x_3 = 0$ and $y \rightarrow 1$,

$$\begin{aligned}\sigma_{ij} &= \frac{-1}{2\pi} \int_0^{2\pi} \sum_{N=1}^3 F_{ijm} \bar{b}_m e^{i\phi} \frac{2 - 3g^2}{g^2 - 1} d\phi + 0(1) \\ &= \left[\frac{-i \{2 - 3(\eta \cdot y)^2\}}{(y^2 - 1)^{1/2}} \sum_{N=1}^3 F_{ijm} (\eta_1/a_1, \eta_2/a_2, \xi_3^N(\eta_1/a_1, \eta_2/a_2)) \right. \\ &\quad \times (\eta_1 + i\eta_2) \bar{b}_m \Bigg]_{\substack{y \rightarrow 1 \\ \eta \rightarrow y}} + 0(1). \quad (29.41)\end{aligned}$$

The stress intensity factor becomes

$$\begin{aligned}k_{ij} &= i\sqrt{\pi} \left(x_1^2/a_1^4 + x_2^2/a_2^4 \right)^{-1/4} \\ &\quad \times \sum_{N=1}^3 F_{ijm} (x_1/a_1^2, x_2/a_2^2, \xi_3^N(x_1/a_1^2, x_2/a_2^2)) \\ &\quad \times (x_1/a_1 + ix_2/a_2) \bar{b}_m. \quad (29.42)\end{aligned}$$

If the material is isotropic and the applied stress is given by

$$\sigma_{33}^0 = \frac{3\mu}{4} \frac{\lambda + \mu}{\lambda + 2\mu} (A_1 y_1 + A_2 y_2), \quad (29.43)$$

we choose

$$\bar{b}_3 = b + ib', \quad (29.44)$$

where b and b' are real numbers. Then, condition $\sigma_{33} = -\sigma_{33}^0$ applied to (29.40) leads to

$$\begin{aligned}b &= A_1 / \int_0^{2\pi} \eta_1^2 (\eta_1^2/a_1^2 + \eta_2^2/a_2^2)^{1/2} d\phi, \\ b' &= -A_2 / \int_0^{2\pi} \eta_2^2 (\eta_1^2/a_1^2 + \eta_2^2/a_2^2)^{1/2} d\phi.\end{aligned} \quad (29.45)$$

The stress intensity factor k_{33} is denoted by k_I ,

$$k_I = a_2 (\sqrt{\pi} / 3) (x_1^2/a_1^4 + x_2^2/a_2^4)^{1/4} \left(x_1 \frac{\partial \sigma_{33}^0}{\partial x_1} / E_1(k) + x_2 \frac{\partial \sigma_{33}^0}{\partial x_2} / E_2(k) \right), \quad (29.46)$$

where

$$\begin{aligned} E_1(k) &= \int_0^{\pi/2} \sin^2 \phi (1 - k^2 \sin^2 \phi)^{1/2} d\phi, \\ E_2(k) &= \int_0^{\pi/2} \cos^2 \phi (1 - k^2 \sin^2 \phi)^{1/2} d\phi, \\ k^2 &= (a_1^2 - a_2^2)/a_1^2. \end{aligned} \quad (29.47)$$

The stress intensity factor k_{II} defined by (29.35) is derived in a similar manner, while the stress intensity factor k_{III} of the third kind is defined by

$$k_{III} = k_{31} t_1 + k_{32} t_2, \quad (29.48)$$

where vector t is the tangential unit vector along the ellipse.

Kassir and Sih (1968) have found that k_I under a uniform σ_{33}^0 is independent of the elastic moduli of a transversely isotropic solid. Barnett and Asaro (1972) have found that for a slit-like crack the stress intensity factor K_{ij} is independent of the elastic moduli for any arbitrarily anisotropic material.

Sekine and Mura (1979) have modified Willis' method (1968) and proved that if the displacement discontinuity of the elliptical Somigliana dislocation (1914) has the form $P_N(x_1, x_2)(1 - x_1^2/a_1^2 - x_2^2/a_2^2)^{1/2}$, where P_N is a homogeneous polynomial of degree N , the stresses on the plane of the Somigliana dislocation are inhomogeneous polynomials of the coordinates, whose terms are of degree $N, (N - 2), (N - 4), \dots$. The same subject has been discussed by Kassir and Sih (1967) under the more restrictive condition $P_N = P_N(x_1^2, x_2^2)$.

Certain special cases relating to elliptical disk-shaped cracks have been examined in the literature. Chen (1966) has solved the problem of an elliptical crack subjected to linearly varying loads in a transversely isotropic body. Stress intensity factors for an embedded elliptical crack or a semi-elliptical crack, which approach the free surface of the semi-infinite solid under uniform tension perpendicular to the plane of crack, have been investigated by Miyamoto and Miyoshi (1971), Shah and Kobayashi (1973), and Nisitani and Murakami (1974). No analytical expressions are available for the stress intensity factors.

30. Stress intensity factors for a slit-like crack

The stress intensity factors of a slit-like crack in anisotropic media can be obtained from (29.27) and (29.42) as a special case when $a_1 \rightarrow \infty$ and $x_2 \rightarrow a_2 = a$. However, more explicit expressions can be obtained by the method of continuously distributed dislocations (Bilby and Eshelby 1968). We follow the work of Barnett and Asaro (1972) in this section.

Consider a slit-like crack in an infinite anisotropic elastic medium (Fig. 30.1). σ_{ij}^0 denotes the applied stress at infinity. The stress disturbance induced by the crack is denoted by σ_{ij} , so that the total stress field is $\sigma_{ij}^0 + \sigma_{ij}$. The boundary conditions are

$$\sigma_{2i}^0 + \sigma_{2i} = 0 \quad \text{for } x_2 = 0, \quad |x_1| \leq c. \quad (30.1)$$

σ_{ij} is simulated by the stress field due to continuously distributed dislocations on $x_2 = 0$, $|x_1| \leq c$, where the dislocation line direction is parallel to the x_3 -axis. From the elastic theory of dislocations (Willis 1970 and Barnett and Swanger 1971), the stress at x_1 , $x_2 = 0$, due to a single dislocation located at $x_1 = t$, $x_2 = 0$ with the Burgers vector b_j , is expressed as

$$\sigma_{2i} = K_{ij} b_j / (x_1 - t). \quad (30.2)$$

The boundary conditions (30.1) may be satisfied if the dislocations are continuously distributed with strength $b_j(t)$ (Burgers vector times distribution density). Then, (30.1) becomes

$$K_{ij} \int_{-c}^c b_j(t) (x_1 - t)^{-1} dt = -\sigma_{2i}^0 \quad (30.3)$$

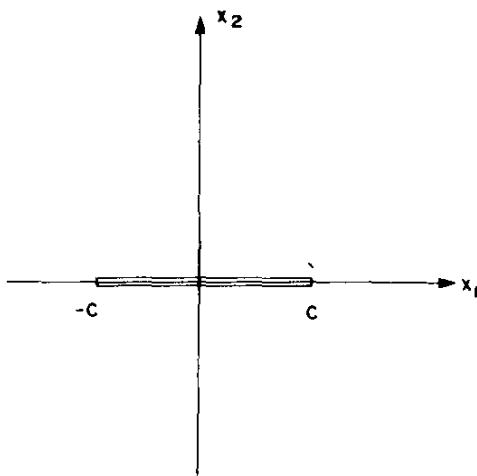


Fig. 30.1. Slit-like crack with length $2c$.

or

$$\int_{-c}^c b_j(t)(x_1 - t)^{-1} dt = -K_{ji}^{-1}\sigma_{2i}^0. \quad (30.4)$$

The inverse matrix components K_{ji}^{-1} are given by

$$K_{ji}^{-1} = K_{ij}^{-1} = \epsilon_{jmn}\epsilon_{irs}K_{mr}K_{ns}/2\epsilon_{abc}K_{1a}K_{2b}K_{3c}, \quad (30.5)$$

so that

$$K_{ji}^{-1}K_{is} = K_{ji}K_{is}^{-1} = \delta_{js}. \quad (30.6)$$

Equation (30.4) is the Hilbert integral equation for $b_j(t)$.

Uniform applied stresses

For constant σ_{2i}^0 , as seen in Appendix 4, the solution of (30.4) is easily obtained as

$$b_j(t) = K_{ji}^{-1}\sigma_{2i}^0 t / \pi(c^2 - t^2)^{1/2}, \quad (30.7)$$

where Q_0 in (A.4.5) of Appendix 4 is taken to be zero in view of condition

$$\int_{-c}^c b_j(t) dt = 0. \quad (30.8)$$

This condition is satisfied, since there is no relative displacement of the crack surface at $x_1 = \pm c$. The relative displacement (crack opening displacement) Δu_j at x_1 is given by

$$\Delta u_j = \int_{x_1}^c b_j(t) dt \quad (30.9)$$

from the dislocation theory.

The stress traction acting on the plane of the crack ($|x_1| > c$, $x_2 = 0$) becomes

$$\begin{aligned} & \sigma_{2i}^0 + K_{ij} \int_{-c}^c K_{jm}^{-1} \sigma_{2m}^0 (t/\pi) (c^2 - t^2)^{-1/2} (x_1 - t)^{-1} dt \\ &= \sigma_{2i}^0 + \int_{-c}^c \sigma_{2i}^0 (t/\pi) (c^2 - t^2)^{-1/2} (x_1 - t)^{-1} dt \\ &= \sigma_{2i}^0 |x_1| / (x_1^2 - c^2)^{1/2} \quad (|x_1| > c). \end{aligned} \quad (30.10)$$

The stress intensity factor defined by (29.21) leads to

$$k_{2i} = \sigma_{2i}^0 \sqrt{\pi c} \quad (30.11)$$

where $r = (x_1 - c)$. The last equation is Barnett and Asaro's remarkable result (1972). The stress intensity factor k_{2i} due to σ_{2i}^0 is independent of elastic moduli. Result (30.11) is a special case of (29.32) and (29.34) for $a_1 \rightarrow \infty$, $a_2 \rightarrow c$.

From (30.7) and (30.9), the crack opening displacement is

$$\Delta u_j = K_{ji}^{-1} \sigma_{2i}^0 (c^2 - x_1^2)^{1/2} / \pi. \quad (30.12)$$

The interaction energy ΔW is expressed from (25.18.6) as

$$\Delta W = -\frac{1}{2} \int_{-c}^c \sigma_{2j}^0 \Delta u_j \, dx_1 \quad (30.13)$$

which becomes

$$\Delta W = -\frac{1}{4} K_{ji}^{-1} \sigma_{2i}^0 \sigma_{2j}^0 c^2. \quad (30.14)$$

The critical applied stress can be obtained by the use of (27.19) where $a_1 = c$, and

$$-\partial(\Delta W)/\partial c = \frac{1}{2} K_{ji}^{-1} \sigma_{2i}^0 \sigma_{2j}^0 c. \quad (30.15)$$

When the material is isotropic, $K_{11} = K_{22} = \mu/2\pi(1-\nu)$, $K_{33} = \mu/2\pi$ and $K_{ij} = 0$ ($i \neq j$) (see Read, 1953). The critical stresses obtained from (30.15) and (27.19) agree with (27.20) and (27.21).

The tensor components appearing in (30.14) and (30.15) are referred to the x coordinate system shown in Fig. 30.1. Since the form of (30.14) and (30.15) is invariant of coordinate systems, we can choose a new coordinate system in the crystallographic directions in which the elastic moduli appear in their simplest form.

The crack opening displacement near the crack tip is obtained from (30.12) with (30.11). For an isotropic material, $K_{22} = \mu/2\pi(1-\nu)$ and $(c^2 - x_1^2)^{1/2} \approx (2cr)^{1/2}$, where r is the distance from the crack tip, and therefore

$$[u_2] = 4k_I(1-\nu)(r/2\pi)^{1/2}/\mu. \quad (30.16)$$

This result is applicable to any plane crack since the crack opening displacement near the crack edge is a local quantity. On the other hand, $[u_i]$ is

obtained from (27.34). When the two expressions for the crack opening displacement are put to be equal, we have an equation to determine the stress intensity factor.

An example is shown for an elliptic crack. From (27.34) and (27.4) we have

$$\begin{aligned}[u_3] &= 2a_3\epsilon_{33}^*(1 - x_1^2/a_1^2 - x_2^2/a_2^2)^{1/2} \\ &= 2a_2(1 - \nu)\sigma_{33}^0(1 - x_1^2/a_1^2 - x_2^2/a_2^2)^{1/2}/\mu E(k) \\ &= 2a_2(1 - \nu)\sigma_{33}^0(2r)^{1/2}(x_1^2/a_1^4 + x_2^2/a_2^4)^{1/4}/\mu E(k),\end{aligned}\quad (30.17)$$

where r is the normal distance from the crack edge defined by (29.22). On the other hand, we have from (30.16)

$$[u_3] = 4k_I(1 - \nu)(r/2\pi)^{1/2}/\mu \quad (30.18)$$

where the normal direction to the crack surface is taken as the x_3 -direction. Equating (30.17) and (30.18), we have

$$k_I = a_2\sqrt{\pi}\sigma_{33}^0(x_1^2/a_1^4 + x_2^2/a_2^4)^{1/4}/E(k) \quad (30.19)$$

which is identical to (29.32).

Non-uniform applied stresses

When an applied stress σ_{2i}^0 is a function of x_1 on $x_2 = 0$, $|x_1| \leq c$, the integral equation (30.4) still holds. The solution has been obtained by Barnett and Asaro (1972). Since the involved analysis is essentially the same as that in the following section for the Dugdale crack, the calculation is not reported here.

Isotropic materials

The stress intensity factors of a slit-like crack in an infinitely extended isotropic material are the same as (30.11). These factors, $k_{22} = k_I$, $k_{21} = k_{II}$, and $k_{23} = k_{III}$, have been obtained by Westergaard (1939). Irwin (1948) pointed out the relation between the critical stress and the stress intensity factor (see also Irwin 1957, 1958). The associated stress intensity factors for cases where an infinitesimally thin notch (crack) is introduced into a semi-infinite slab have been investigated by many researchers (see Wigglesworth 1957, Lachenbruch 1961, Koiter 1965, Nisitani and Murakami 1972, and others).

For a strip with a center notch (crack), numerical calculations for the stress intensity factors have been performed by Isida (1971). The stress intensity factor for an inner or edge crack in arbitrarily shaped plates has been investigated by Murakami (1978) by using the Green function for the traction-free crack in an infinite plate (see also Erdogan 1962). Yamamoto and Sumi (1978) recently reported that a slit-like crack is not a two-dimensional problem (neither plane strain nor plane stress), since for moderate values of the ratio of crack length to the plate thickness and because of the effect of Poisson's ratio, the necessary conditions for neither plane stress nor plane strain can be satisfied. They evaluated the variation of the stress intensity factor along the crack front.

Stress intensity factors of notches and cracks in various standard test specimens and machine elements under various applied loads have been investigated by a number of researchers, and many of the results are listed in a handbook by Tada (1973) and in a special publication by Kamei and Yokobori (1974).

A recent development is the use of the finite element method in the evaluation of the stress intensity factors (see Zienkiewicz, 1971). Early original work in this area is found in papers by Watwood (1969), Chan, Tuba and Wilson (1970), and Dixson and Pook (1969). Since the stress field in the neighborhood of the crack tip is proportional to $1/\sqrt{r}$ (r is the distance from the crack tip), the proportionality constant, i.e., the stress intensity factor, can be obtained by plotting the finite element solution as a function r . The stress intensity factor can also be found by plotting the crack opening displacement versus r . These two methods are called the stress and displacement methods. A third method, an energy method, has been proposed, based on the fact that the energy release rate due to the crack extension is proportional to the square of the stress intensity factor. The first two methods do not give satisfactory results unless very fine meshes are employed, while moderate accuracy can be obtained by the energy method. The energy method, however, cannot be applied to mixed mode or three-dimensional problems. Recently, the stress method has been reconsidered by Miyata and Kusumoto (1975) and Murakami (1976, 1978). They have found empirical relations between stress intensity factors and stress solutions obtained by the finite element method, and they have obtained stress intensity factors for several three-dimensional cracks with relatively coarse meshes.

Byskov (1970), Rao, Raju, and Krishna Murty (1971) and Pian, Tong, and Luk (1971) proposed special elements by using the hybrid vibrational method or the collocation method. These methods introduce additional freedom in conjunction with Williams' coefficients (1957) for stress singularities. Another interesting approach, proposed by Yamamoto (1971), is illustrated in Fig. 30.2.

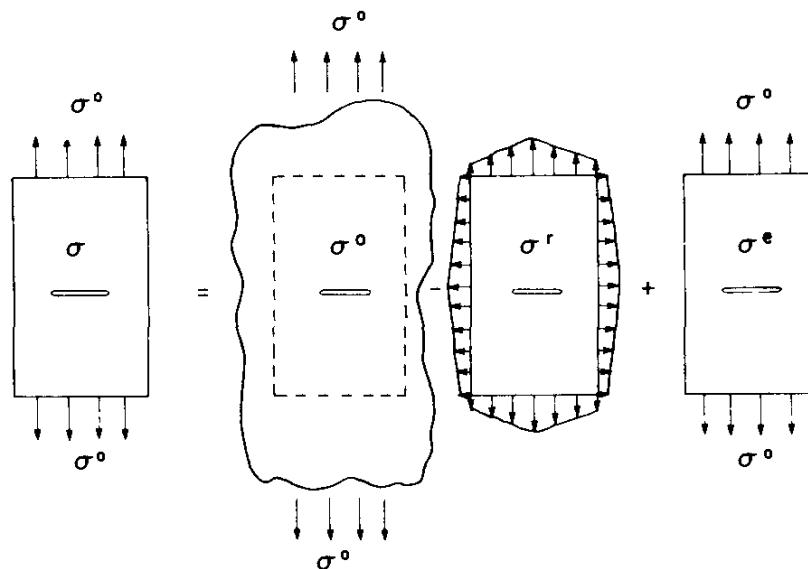


Fig. 30.2. Superposition method.

The exact stress field σ is $\sigma \approx \sigma^a - \sigma^r + \sigma^e$, where σ^a is an analytical solution for an infinite body, σ^r is a solution by the finite element method in the finite body subjected to a boundary force with the same traction as at the corresponding imaginary boundary in an infinite body, and σ^e is a solution of the finite body with the originally given load by the finite element method. Yamamoto postulated that, in the neighborhood of the crack tip, $\sigma = \alpha(\sigma^a - \sigma^r) + \sigma^e = \alpha\sigma^a$, where α is introduced as a Lagrangian multiplier which coincides with the stress intensity factor. Thus, we have $\alpha = \sigma^e/\sigma^r$. This method has been further extended by Yamamoto and Sumi (1978, 1979) to include three-dimensional crack problems.

The theory of stability of interacting tension cracks in brittle solids has been recently developed by Nemat-Nasser (1978), Nemat-Nasser, Keer, and Parihar (1978), Keer, Nemat-Nasser, and Oranratnachai (1979), Sumi, Nemat-Nasser, and Keer (1980), and Horii and Nemat-Nasser (1986), in connection with thermally induced edge cracks in plane strain conditions. These authors show that as a half-space is cooled on its free surface, tension cracks develop and penetrate the half-space. However, a critical state is then reached at which some cracks stop, as others grow at a faster rate. Moreover, depending on the cooling mechanism, another critical state may be attained at which the cracks which had stopped may actually snap closed as the remaining ones jump into a finitely longer length. The growth process is highly imperfection sensitive, as shown by Nemat-Nasser and Shokooh (1980). In a series of experiments on glass plates, Geyer and Nemat-Nasser (1981) obtained good qualitative and quantitative verification of the theory.

31. Stress concentration factors

Let us consider an ellipsoidal inhomogeneity in an infinite medium (see Fig. 31.1). σ_{ij}^0 denotes the applied stress, and σ_{ij} the stress disturbance due to the inhomogeneity. σ_{ij} can be simulated by the eigenstress associated with eigenstrain ϵ_{ij}^* of an equivalent inclusion in a homogeneous medium. The stress immediately outside the inclusion can be written as

$$\sigma_{ij}^T = [\sigma_{ij}] + \sigma_{ij}(\text{in}) + \sigma_{ij}^0, \quad (31.1)$$

where

$$[\sigma_{ij}] = \sigma_{ij}(\text{out}) - \sigma_{ij}(\text{in}); \quad (31.2)$$

see (6.11). The equivalent eigenstrain ϵ_{ij}^* in (6.11) must be determined from the equivalency condition (22.7).

The stress concentration factor κ of σ_{ij}^T for a given $\sigma_{kl}^0 = S$ is defined by

$$\kappa = \sigma_{ij}^T / S. \quad (31.3)$$

If the inhomogeneity is a void, $\sigma_{ij}(\text{in}) + \sigma_{ij}^0 = 0$, and

$$\kappa = [\sigma_{ij}] / S. \quad (31.4)$$

Mura and Cheng (1977) have calculated κ for a three-dimensional, lens-shaped crack, where a_3 is much smaller than a_1 or a_2 . Since $a_3 \ll a_1, a_2$, the results (28.16) and (28.17) can be used. For simplicity the case $\alpha = 0$ in Fig. 28.1 is

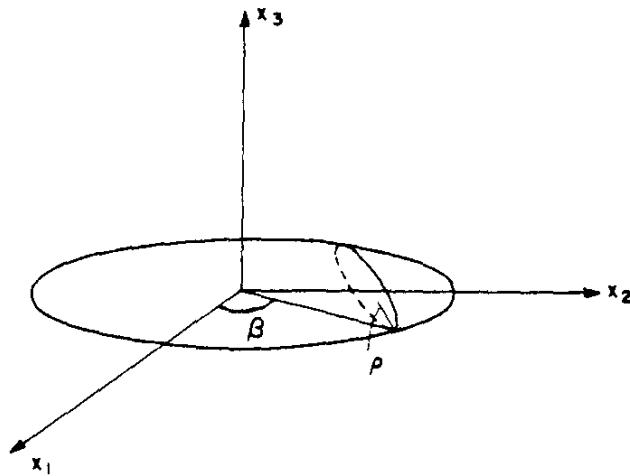


Fig. 31.1. A lens-shaped crack.

considered. The definition (31.3) does not mean that κ is the same for all stress components.

Simple tension

Let σ_{33}^0 be prescribed with all other components of σ_{ij}^0 being zero. Combining (31.4), (6.11), and (28.16), we have the stress concentration factor,

$$\kappa = (4\pi/a_3) \{ C_{3333} - C_{33kl} C_{pq33} G_{kplq}(\mathbf{n}) \} / C_{33mn} C_{st33} \Pi_{msnt}, \quad (31.5)$$

where $G_{kplq}(\mathbf{n}) = N_{kp}(\mathbf{n}) n_l n_q / D(\mathbf{n})$ and Π_{msnt} is defined by (28.12). The interesting points to be investigated are on the crack boundary with $x_3 = 0$. The radius ρ at a point corresponding to angle β in Fig. 31.1 is

$$\rho = (a_3^2/a_2) \{ \sin^2\beta + (a_2/a_1)^2 \cos^2\beta \}^{1/2}. \quad (31.6)$$

Equation (31.5) is then written as

$$\kappa = \frac{4\pi}{(\rho a_2)^{1/2}} \frac{\{ C_{3333} - C_{33kl} C_{pq33} G_{kplq}(\mathbf{n}) \}}{C_{33mn} C_{st33} \Pi_{msnt}} \left(\sin^2\beta + \frac{a_2^2}{a_1^2} \cos^2\beta \right)^{1/4}. \quad (31.7)$$

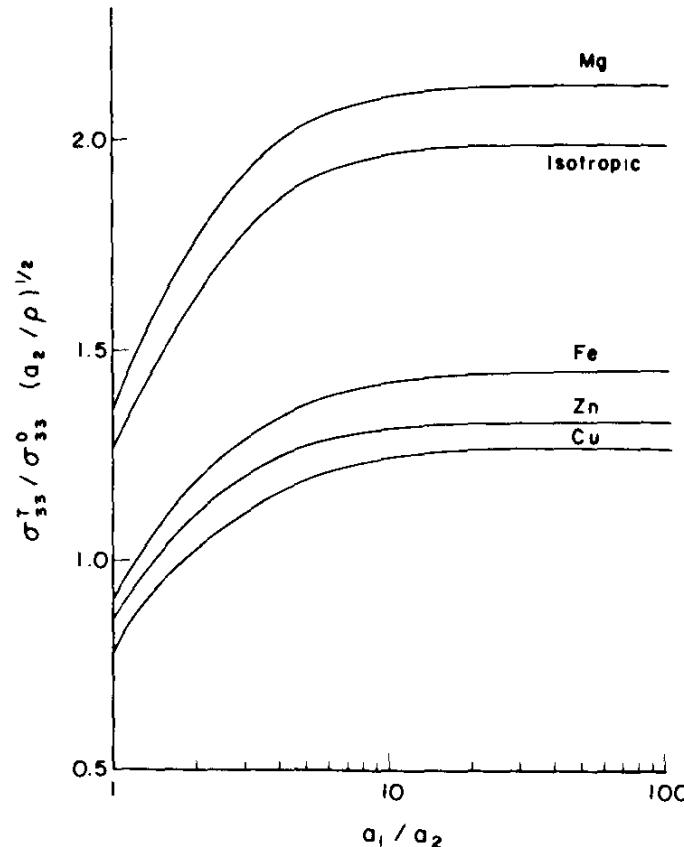


Fig. 31.2. Maximum tensile stress concentration factor divided by $(a_2/\rho)^{1/2}$ for various ratios of a_1/a_2 .

For isotropic materials it becomes

$$\kappa = 2(a_2/\rho)^{1/2} \left(\sin^2\beta + \frac{a_2^2}{a_1^2} \cos^2\beta \right)^{1/4} / E(k), \quad (31.8)$$

where $E(k)$ is defined in (11.25). When κ is considered at the point $\beta = 90^\circ$ with $a_1 \rightarrow \infty$, it agrees with the well-known solution given by Inglis (1913).

Numerical values of $\kappa/(a_2/\rho)^{1/2}$ at $\beta = 90^\circ$, where the stress concentration factor becomes maximum, are plotted in Fig. 31.2.

Pure shear

Let σ_{31}^0 be prescribed with all other components of σ_{ij}^0 being zero. From (31.4), (6.11), (28.17), and (31.6) the stress concentration factor of σ_{31}^T is

$$\kappa = \frac{4\pi}{(\rho a_2)^{1/2}} \frac{\{C_{3131} - C_{31kl}C_{pq31}G_{kplq}(n)\}}{C_{31mn}C_{st31}\Pi_{msnt}} \left(\sin^2\beta + \frac{a_2^2}{a_1^2} \cos^2\beta \right)^{1/4}. \quad (31.9)$$

For isotropic materials and $\beta = 90^\circ$, it becomes

$$\kappa = (a_2/\rho)^{1/2} k^2 / \left[\frac{k^2}{1-\nu} \{ F(k) - E(k) \} + \{ E(k) - (k')^2 F(k) \} \right], \quad (31.10)$$

where $F(k)$ is defined in (11.25) and $k'^2 = 1 - k^2$.

Numerical values of $\kappa/(a_2/\rho)^{1/2}$ at $\beta = 90^\circ$ are plotted in Fig. 31.3. The elastic moduli in Figs. 31.2 and 31.3 are taken from the American Institute of Physics Handbook (1972). It is observed that when a_1/a_2 approaches infinity, the values of $\kappa/(a_2/\rho)^{1/2}$ approach constants which agree with the values obtained from Lekhnitskii's method (1968). The resulting displacement field agrees with Chen's solution (1966) for an elliptical disk-shaped crack in transversely isotropic materials.

The state of stress due to an ellipsoidal cavity in an infinite isotropic medium has been given by Sadowsky and Sternberg (1949) and Miyamoto (1953) for the case of three constant normal stresses, applied at infinity, having their directions aligned with the principal axes of the ellipsoid. Earlier solutions obtained by Neuber (1937) consider loading in applied shear as well as normal stresses for the special case of the spheroidal cavity. Mirandy and Paul (1976) also employ the equivalent inclusion method. They investigate the effect of Poisson's ratio on the stress concentration factors of an ellipsoidal cavity subjected to a completely arbitrary constant state of stress at infinity.

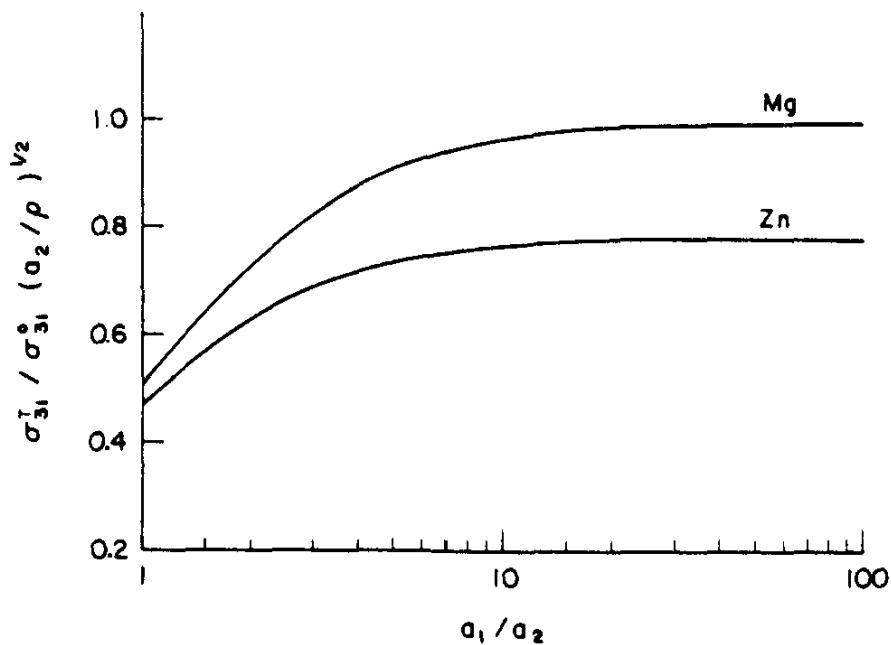


Fig. 31.3. Maximum shear stress concentration factor divided by $(a_2/\rho)^{1/2}$ for various ratios of a_1/a_2

It should be pointed out that the stress intensity factors generally cannot be derived by the limiting process

$$k_I = \lim \sqrt{2\pi\rho} \kappa \sigma_{ij}^0. \quad (31.11)$$

32. Dugdale-Barenblatt cracks

In the absence of plastic deformations around crack tips (or notches), some stress components become infinite there. In reality, however, the stress field

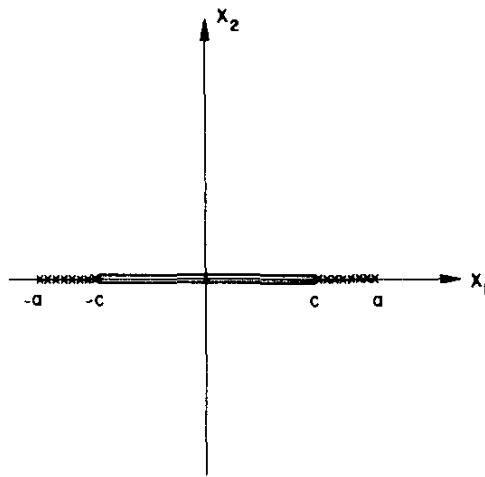


Fig. 32.1. Dugdale-Barenblatt crack.

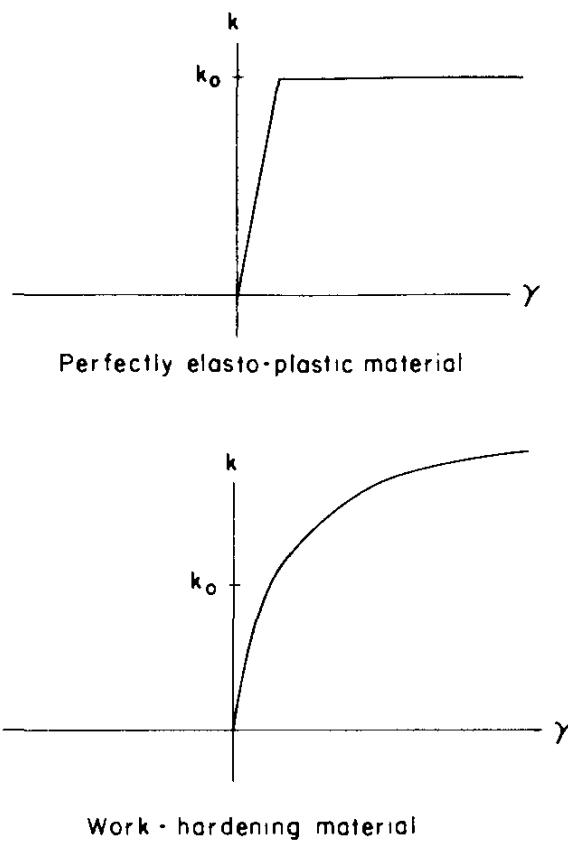


Fig. 32.2. Stress-strain curves.

cannot exceed a certain limit because of the plastic deformation in the neighborhood of the crack tips. Dugdale (1960) has simplified the situation and has assumed that the plastic region is one-dimensional, being confined on (c, a) and $(-a, -c)$, as shown in Fig. 32.1. He has related the size of the plastic domain to the applied stress and crack length $2c$ by assuming the material to be elastic-perfectly plastic (Fig. 32.2a). One method of coping with this problem of stress singularity is that of Barenblatt (1960). He postulates a system of cohesive stresses in the corresponding region (a, c) and $(-a, -c)$ in Fig. 32.1. The magnitude of these stresses is just sufficient to cause the usual stress singularity to vanish.

The boundary conditions for a Dugdale crack under a simple tension, $\sigma_{22}^0 = S$, are

$$\begin{aligned}
 \sigma_{2j} &= 0 \quad (j = 1, 2) \quad \text{at } x_2 = 0 \quad |x_1| < c, \\
 \sigma_{22} &= k_0 \quad \text{at } x_2 = 0 \quad c < |x_1| < a, \\
 \sigma_{22} &= S \quad \text{at } x_2 = \infty, \\
 u_2 &= 0, \quad \sigma_{21} = 0 \quad \text{at } x_2 = 0 \quad a < |x_1|,
 \end{aligned} \tag{32.1}$$

where σ_{22}^0 is the applied tensile stress and k_0 is the tensile yield stress. The

following stress analysis shows that $\sigma_{11} = \sigma_{22} = \sigma_{33}$ for the plane strain and $\sigma_{11} = \sigma_{22}, \sigma_{33} = 0$ for the plane stress at the crack tips. The boundary condition $\sigma_{22} = k_0$ is therefore valid only for the plane stress assumption, since $\sigma_{22} = k_0$ comes from the maximum shear theory $\frac{1}{2}(\sigma_{22} - \sigma_{33}) = \frac{1}{2}k_0$.

The differential equations to be solved are, for the isotropic materials,

$$\mu u_{i,jj} + (\mu + \lambda) u_{k,ki} = 0. \quad (32.2)$$

When the Papkovich–Neuber potential Ψ is used,

$$\begin{aligned} 2\mu u_1 &= \frac{1-\kappa}{2} \Psi_{,1} - x_2 \Psi_{,12}, \\ 2\mu u_2 &= \frac{1+\kappa}{2} \Psi_{,2} - x_2 \Psi_{,22}, \end{aligned} \quad (32.3)$$

where

$$\Psi_{,11} + \Psi_{,22} = 0, \quad (32.4)$$

$\kappa = (3 - \nu)/(1 + \nu)$ for the plane stress, and $\kappa = 3 - 4\nu$ for the plane strain, where ν is Poisson's ratio. The stress components derived from (32.3) are

$$\begin{aligned} \sigma_{22} &= \Psi_{,22} - x_2 \Psi_{,222}, \\ \sigma_{12} &= -x_2 \Psi_{,122}, \\ \sigma_{11} &= -\Psi_{,11} - x_2 \Psi_{,112}. \end{aligned} \quad (32.5)$$

Before we apply (32.5) to (32.1), it is convenient to rewrite (32.1) as

$$\begin{aligned} \sigma_{22} &= 0 && \text{at } x_2 = \infty, \\ \sigma_{12} &= 0 && \text{at } x_2 = \infty, \\ \sigma_{12} &= 0 && \text{at } x_2 = 0, \\ \sigma_{22} &= -S && \text{at } x_2 = 0, \quad |x_1| < c, \\ \sigma_{22} &= k_0 - S && \text{at } x_2 = 0, \quad c < |x_1| < a, \\ u_2 &= 0 && \text{at } x_2 = 0, \quad a < |x_1|. \end{aligned} \quad (32.6)$$

The solution for (32.1) is obtained from the solution for (32.6) by superimposing S onto σ_{22} .

From the symmetry of the problem with respect to x_1 , Ψ is chosen as

$$\Psi = - \int_0^\infty \xi^{-1} A(\xi) e^{-\xi x_2} \cos \xi x_1 d\xi, \quad (32.7)$$

which satisfies (32.4). The first three conditions in (32.6) are automatically satisfied. With (32.5), the fourth and fifth conditions yield

$$\int_0^\infty \xi A(\xi) \cos \xi x_1 d\xi = \sigma(x_1), \quad (32.8)$$

where

$$\begin{aligned} \sigma(x_1) &= S && \text{for } |x_1| < c \\ &= S - k_0 && \text{for } c < |x_1| < a. \end{aligned} \quad (32.9)$$

Assuming

$$A(\xi) = \int_0^a q(t) J_0(\xi t) dt, \quad (32.10)$$

where J_0 is the Bessel function of zero order, from (32.3) and (32.7) we have, at $x_2 = 0$,

$$\begin{aligned} \frac{4\mu}{1+\kappa} u_2 &= \int_0^a q(t) dt \int_0^\infty J_0(\xi t) \cos \xi x_1 d\xi \\ &= \int_0^a q(t) \frac{H(t - x_1)}{(t^2 - x_1^2)^{1/2}} dt \\ &= \int_{x_1}^a \frac{q(t)}{(t^2 - x_1^2)^{1/2}} dt && \text{for } |x_1| < a \\ &= 0 && \text{for } a < |x_1|. \end{aligned} \quad (32.11)$$

The last condition in (32.6) is thereby satisfied. The unknown function q is determined from (32.8). Substituting (32.10) into (32.8), we obtain

$$\begin{aligned} \sigma(x_1) &= \frac{d}{dx_1} \int_0^a q(t) dt \int_0^\infty J_0(\xi t) \sin \xi x_1 d\xi = \frac{d}{dx_1} \int_0^a q(t) \frac{H(x_1 - t)}{(x_1^2 - t^2)^{1/2}} dt \\ &= \frac{d}{dx_1} \int_0^{x_1} \frac{q(t)}{(x_1^2 - t^2)^{1/2}} dt && \text{for } |x_1| < a. \end{aligned} \quad (32.12)$$

Integrating with respect to x_1 we have Abel's integral equation

$$\int_0^{x_1} \frac{q(t)}{(x_1^2 - t^2)^{1/2}} dt = P(x_1), \quad (32.13)$$

where

$$P(x_1) = \int_0^{x_1} \sigma(s) ds. \quad (32.13.1)$$

The solution is

$$q(t) = \frac{2}{\pi} \frac{d}{dt} \int_0^t \frac{x_1 P(x_1) dx_1}{(t^2 - x_1^2)^{1/2}} \quad (32.13.2)$$

or, integrating by parts,

$$q(t) = \frac{2}{\pi} t \int_0^t \frac{\sigma(x_1)}{(t^2 - x_1^2)^{1/2}} dx_1. \quad (32.14)$$

When $\sigma(x_1)$ is given as (32.9), (32.14) becomes

$$\begin{aligned} q(t) &= St && \text{for } t < c, \\ q(t) &= \frac{2}{\pi} t \left\{ (S - k_0) \cos^{-1} \frac{c}{t} + S \sin^{-1} \frac{c}{t} \right\} && \text{for } c < t < a. \end{aligned} \quad (32.15)$$

All quantities of the elastic field can be obtained from (32.15) by using (32.7) and (32.10). In order to have a finite value of stress, however, we must have

$$q(a) = 0, \quad (32.16)$$

because, at $x_2 = 0$, $a < |x_1|$, we have

$$\begin{aligned} \sigma_{22} &= - \frac{d}{dx_1} \int_0^a q(t) \frac{H(x_1 - t)}{(x_1^2 - t^2)^{1/2}} dt = - \frac{d}{dx_1} \int_0^a q(t) d \sin^{-1} \frac{t}{|x_1|} \\ &= \frac{aq(a)}{x_1 (x_1^2 - a^2)^{1/2}} + \frac{1}{x_1} \left[q'(t) (x_1^2 - t^2)^{1/2} \right]_0^a - \int_0^a q''(t) (x_1^2 - t^2)^{1/2} dt. \end{aligned}$$

From (32.15) and (32.16) we have

$$\cos^{-1} \frac{c}{a} = \frac{\pi}{2} \frac{S}{k_0}. \quad (32.17)$$

The above equation gives the relationship between the applied stress and the plastic zone size. The relation has been found by Dugdale (1960).

Of particular interest is the displacement component u_2 in the plastic domain. It is obtained from (32.11) and (32.15) for $x_2 = 0^+$:

$$\begin{aligned} u_2(x_1, c) &= \frac{(1 + \kappa)k_0}{4\pi\mu} \left[(x_1 + c) \cosh^{-1} \left| \frac{m}{c + x_1} + n \right| \right. \\ &\quad \left. - (x_1 - c) \cosh^{-1} \left| \frac{m}{c - x_1} + n \right| \right] \\ &= \frac{(1 + \kappa)k_0}{4\pi\mu} \left[x_1 \log \frac{x_1(a^2 - c^2)^{1/2} - c(a^2 - x_1^2)^{1/2}}{x_1(a^2 - c^2)^{1/2} + c(a^2 - x_1^2)^{1/2}} \right. \\ &\quad \left. + c \log \frac{(a^2 - c^2)^{1/2} + (a^2 - x_1^2)^{1/2}}{(a^2 - c^2)^{1/2} - (a^2 - x_1^2)^{1/2}} \right], \end{aligned} \quad (32.18)$$

where $m = (a^2 - c^2)/a$ and $n = c/a$. The value at $x_2 = 0^-$ is the negative of (32.18). The displacement at the upper tip of the crack is obtained from (32.18) by setting $x_1 = c$,

$$u_2 = \frac{(1 + \kappa)k_0 c}{2\pi\mu} \cosh^{-1} \left\{ \frac{1}{2} \left(\frac{c}{a} + \frac{a}{c} \right) \right\}. \quad (32.19)$$

When $a/c \approx 1$ and $S/k_0 \ll 1$, (32.17) and (32.19) are approximated as

$$\begin{aligned} 1 - c/a &= \frac{\pi^2}{8} (S/k_0)^2, \\ u_2 &= \frac{(1 + \kappa)k_0}{2\pi\mu} c (1 - c/a) = \frac{(1 + \kappa)}{16\mu} \pi c \frac{S^2}{k_0}, \end{aligned} \quad (32.20)$$

and we have

$$u_2(x_1, c)/u_2(c, c) = (1 - \xi)^{1/2} - \frac{\xi}{2} \log \frac{1 + (1 - \xi)^{1/2}}{1 - (1 - \xi)^{1/2}}, \quad (32.20.1)$$

where $\xi = (x_1 - c)/(a - c)$.

Orowan (1945, 1949) and Irwin (1948, 1957) have proposed that the applied critical stress S required to propagate a crack of length $2c$ in a semi-brittle solid of elastic constant μ should be written in the form of a Griffith equation, i.e.,

$$S \simeq (\mu\gamma/c)^{1/2}, \quad (32.21)$$

in which γ represents the sum of the true surface energy and the plastic work required to increase the area of the crack by a unit amount. This Orowan-Irwin formula can be obtained from (32.20). We postulate that the crack becomes unstable when the crack opening displacement $2u_2$ given in (32.20) reaches a critical value Φ . Then,

$$S = \left(\frac{8\mu k_0 \Phi}{\pi(1 + \kappa)c} \right)^{1/2}. \quad (32.22)$$

The strain hardening effect can be taken into account in this result if an approximation is employed as done by Theocaris and Gdoutos (1974). Alternative interpretations of (32.21) are given by Weertman (1978) and Vilmann and Mura (1979).

A similar result can be obtained for the anti-plane problem. A shear stress $\sigma_{32}^0 = S$ is applied at infinity. The boundary conditions are

$$\begin{aligned} \sigma_{32} &= S && \text{at } x_2 = \infty, \\ \sigma_{32} &= 0 && \text{at } x_2 = 0, \quad |x_1| < c, \\ \sigma_{32} &= k_0 && \text{at } x_2 = 0, \quad c < |x_1| < a, \\ w &= 0 && \text{at } x_2 = 0, \quad a < |x_1|, \end{aligned} \quad (32.23)$$

where w is the displacement in the x_3 -direction. Again we rewrite (32.23) as

$$\begin{aligned} \sigma_{32} &= 0 && \text{at } x_2 = \infty, \\ \sigma_{32} &= -S && \text{at } x_2 = 0, \quad |x_1| < c, \\ \sigma_{32} &= k_0 - S && \text{at } x_2 = 0, \quad c < |x_1| < a, \\ w &= 0 && \text{at } x_2 = 0, \quad a < |x_1|. \end{aligned} \quad (32.24)$$

The solution for (32.23) is obtained from the solution for (32.24) by superimposing S onto 0. Hooke's law yields $\sigma_{31} = \mu w_{,1}$ and $\sigma_{32} = \mu w_{,2}$, and the equation of equilibrium $\sigma_{31,1} + \sigma_{32,2} = 0$ leads to

$$w_{,11} + w_{,22} = 0. \quad (32.25)$$

We assume w has the form

$$w = (1/\mu) \int_0^\infty A(\xi) e^{-\xi x_2} \cos \xi x_1 d\xi. \quad (32.26)$$

Then,

$$\sigma_{32} = - \int_0^\infty \xi A(\xi) e^{-\xi x_2} \cos \xi x_1 d\xi \quad (32.27)$$

which satisfies the first condition in (32.24). The second and third conditions in (32.24) yield

$$\int_0^\infty \xi A(\xi) \cos \xi x_1 d\xi = \sigma(x_1), \quad (32.28)$$

where

$$\begin{aligned} \sigma(x_1) &= S && \text{for } |x_1| < c, \\ \sigma(x_1) &= S - k_0 && \text{for } c < |x_1| < a. \end{aligned} \quad (32.29)$$

The above result is the same as (32.8) with (32.9). Taking

$$A(\xi) = \int_0^\infty q(t) J_0(\xi t) dt, \quad (32.30)$$

we have, from (32.15),

$$\begin{aligned} q(t) &= St && \text{for } t < c, \\ q(t) &= \frac{2}{\pi} t \left\{ (S - k_0) \cos^{-1} \frac{c}{t} + S \sin^{-1} \frac{c}{t} \right\} && \text{for } c < t < a. \end{aligned} \quad (32.31)$$

w at $x_2 = 0$ is calculated from (32.26) and (32.11) as

$$\begin{aligned}\mu w &= \int_x^a \frac{q(t)}{(t^2 - x_1^2)^{1/2}} dt \quad \text{for } |x_1| < a, \\ \mu w &= 0 \quad \text{for } a < |x_1|.\end{aligned}\tag{32.32}$$

Substituting (32.31), we have, similar to (32.18),

$$w = \frac{k_0}{\pi\mu} \left[(x_1 + c) \cosh^{-1} \left| \frac{m}{c + x_1} + n \right| - (x_1 - c) \cosh^{-1} \left| \frac{m}{c - x_1} + n \right| \right],\tag{32.33}$$

where $m = (a^2 - c^2)/a$ and $n = c/a$.

The condition for the finite value of σ_{32} at $x_1 = a$ is (32.16); therefore,

$$\cos^{-1} \frac{c}{a} = \frac{\pi}{2} \frac{S}{k_0}.\tag{32.34}$$

When $a/c \approx 1$ and $S/k_0 \ll 1$, (32.34) and w at $x_1 = c$ are written approximately as

$$\begin{aligned}1 - c/a &= \frac{\pi^2}{8} (S/k_0)^2, \\ w &= \frac{2}{\pi\mu} k_0 (a - c).\end{aligned}\tag{32.35}$$

We can see from this result that the critical stress for the anti-plane problem also has the form given by (32.21).

BCS model

Bilby, Cottrell and Swinden (1963) have solved the anti-plane problem given by (32.23) or (32.24) using a completely different method. The stress disturbance caused by the crack and plastic region is simulated by the dislocation stress. The dislocations are distributed continuously in the domain $(-a, a)$. The idea that cracks can be simulated by arrayed dislocations has originated in the work of Eshelby (1957).

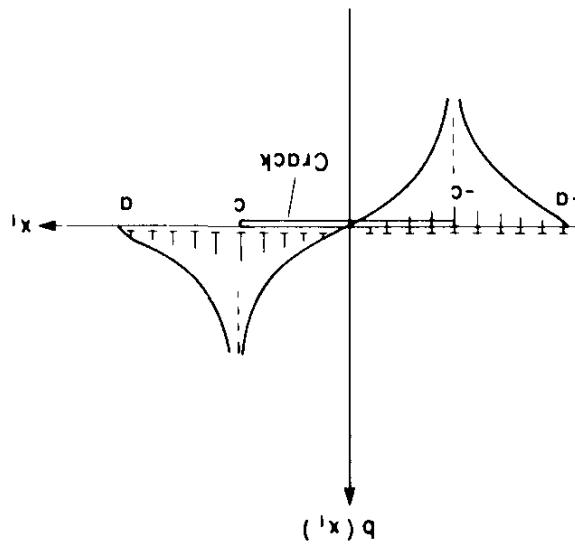


Fig. 32.3 Distribution of dislocations.

Consider a crack as shown in Fig. 32.3. $\sigma_{23}^0 = S$ is an applied stress. The dislocation stress σ_{23} at $x_2 = 0$ due to a screw dislocation at $x_1 = t$, $x_2 = 0$ is $(\mu b / 2\pi) / (x_1 - t)$. The boundary conditions corresponding to (32.23) can be reduced to the Hilbert integral equation

$$S + \int_{-a}^a (\mu / 2\pi) b(t) (x_1 - t)^{-1} dt = \begin{cases} 0 & \text{for } |x_1| < c, \\ k_0 & \text{for } c < |x_1| < a, \end{cases} \quad (32.36)$$

where $b(t)$ is the strength of the Burgers vector (Burgers vector times the density of the dislocation distribution). As seen in Appendix 4, the work of Head and Louat (1955) leads to the solution for (32.36),

$$\begin{aligned} b(x_1) &= \frac{2k_0}{\pi\mu} \log \frac{x_1(a^2 - c^2)^{1/2} + c(a^2 - x_1^2)^{1/2}}{x_1(a^2 - c^2)^{1/2} - c(a^2 - x_1^2)^{1/2}} && \text{for } c < |x_1| < a, \\ b(x_1) &= \frac{2k_0}{\pi\mu} \log \frac{x_1(a^2 - c^2)^{1/2} + c(a^2 - x_1^2)^{1/2}}{c(a^2 - x_1^2)^{1/2} - x_1(a^2 - c^2)^{1/2}} && \text{for } |x_1| < c, \end{aligned} \quad (32.37)$$

provided that condition (A.4.7) is satisfied. This condition gives (A.4.9) or

$$c/a = \cos(\pi S / 2k_0) \quad (32.38)$$

which coincides with (32.34).

The relative displacement Δu_3 at $x_2 = 0$ (crack opening displacement in $|x_1| < c$ and plastic displacement in $c < |x_1| < a$) is obtained from

$$\Delta u_3 = \int_{x_1}^a b(t) dt. \quad (32.39)$$

It becomes

$$\begin{aligned} \Delta u_3 = \frac{2k_0}{\pi\mu} & \left\{ x_1 \log \frac{x_1(a^2 - c^2)^{1/2} - c(a^2 - x_1^2)^{1/2}}{x_1(a^2 - c^2)^{1/2} + c(a^2 - x_1^2)^{1/2}} \right. \\ & \left. + c \log \frac{(a^2 - c^2)^{1/2} + (a^2 - x_1^2)^{1/2}}{(a^2 - c^2)^{1/2} - (a^2 - x_1^2)^{1/2}} \right\}. \end{aligned} \quad (32.40)$$

The relative displacement at $x_1 = c$ is obtained by the limiting process,

$$\frac{(x_1/c)(a^2 - c^2)^{1/2} - (a^2 - x_1^2)^{1/2}}{(a^2 - c^2)^{1/2} - (a^2 - x_1^2)^{1/2}} \rightarrow \frac{a^2}{c^2}, \text{ becoming}$$

$$\Delta u_3 = (4k_0/\pi\mu)c \log(a/c). \quad (32.41)$$

For small plastic zones, $(a - c)/c \ll 1$, (32.38) and (32.41) become, approximately,

$$\begin{aligned} c/a &= 1 - \pi^2 S^2 / 8k_0^2, \\ \Delta u_3 &= (4k_0/\pi\mu)(a - c) = \pi c S^2 / 2\mu k_0, \end{aligned} \quad (32.42)$$

which agree with (32.35) where $\Delta u_3 = 2w$. The critical stress is similar to (32.22). The distribution of dislocations (32.37) is sketched in Fig. 32.3.

The BCS model has been extended by many researchers. In order to investigate plastic yielding from sharp notches, Bilby et al. and (1964) Bilby and Swinden (1965) have considered an infinite array of relaxed cracks. Fracture characteristics of solids containing doubly periodic arrays of cracks have been investigated by Karihaloo (1978, 1979).

Barnett and Asaro (1972) have considered the BCS model in an arbitrarily anisotropic elastic medium. As shown by (30.2), the dislocation stress at

$x_1 = x_1$, $x_2 = 0$ due to a single dislocation located at $x_1 = t$, $x_2 = 0$ with the Burgers vector \mathbf{b} is

$$\sigma_{2i} = K_{ij} b_j / (x_1 - t). \quad (32.43)$$

Assuming a continuous distribution of dislocations with the strength $\mathbf{b}(t)$, we write the boundary conditions given in the domain $x_2 = 0$, $|x_1| < a$, as

$$\begin{aligned} \sigma_{2i}^0 + \int_{-a}^a K_{ij} b_j(t) (x_1 - t)^{-1} dt &= 0 \quad \text{for } |x_1| < c, \\ \sigma_{2i}^0 + \int_{-a}^a K_{ij} b_j(t) (x_1 - t)^{-1} dt &= k_i(x_1) \quad \text{for } c < |x_1| < a, \end{aligned} \quad (32.44)$$

where σ_{2i}^0 ($i = 1, 2, 3$) are components of the applied stress at infinity and k_i is the component of yielding shear stress. It is convenient to rewrite (32.44) as

$$\int_{-a}^a b_j(t) (x_1 - t)^{-1} dt = K_{ij}^{-1} (k_i(x_1) - \sigma_{2i}^0), \quad (32.45)$$

where $k_i(x_1) = 0$ for $|x_1| < c$. When k_i is constant, the solution of (32.45) has the same form as (A.4.8), where k is replaced by $K_{ij}^{-1} k_i$.

The BCS model has been further extended to cracks with inclined slip planes (Fig. 32.4) by Atkinson and Kay (1971), Vitek (1976), Riedel (1976), Atkinson and Kanninen (1977), Karihaloo (1979), Takeuchi (1979), Miyamoto et al. (1981), Hayashi and Nemat-Nasser (1981), and Chang and Ohr (1984). A further extension to the two-dimensional dislocation distribution is presented in Section 41.

The Dugdale model has been used to obtain a dynamic solution by Goodier and Field (1963) and Kanninen et al. (1968). The dislocation distribution predicted by the BCS model was observed by an electron microscope by Kobayashi and Ohr (1979) and Ohr and Narayan (1980).

In the Dugdale-Barenblatt or BCS crack, the stress at the crack tip has no singularity. On the other hand, Thomson (1976, 1978) and Weertman (1978, 1980) have proposed a crack model in which a small enclave that is surrounded by a plastic region is considered to exist at the crack tip. This small enclave is a dislocation-free zone and the stress at the crack tip has the same singularity as the elastic crack. The stress intensity factor, however, is shielded by the dislocation distribution in the surrounding plastic zone. Further extensions of the theory have been made by Chang and Ohr (1985) who considered inclined dislocation emission, by Li (1985) who numerically calculated the interaction between a crack and dislocations to investigate the rate of dislo-

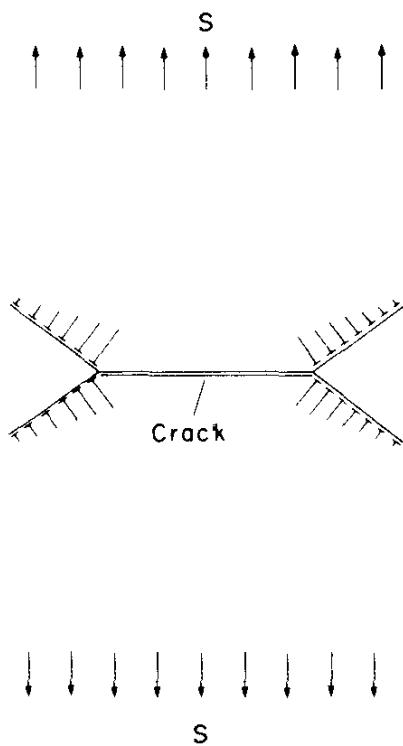


Fig. 32.4. Crack with inclined slip plane.

tion emission during loading and unloading, and by Weertman (1983, 1984) and Weertman, Lin, and Thomson (1983) who proposed a double-slip plane model to obtain crack growth equations for Mode II or III under a monotonically increasing stress.

Penny-shaped crack

Let us consider a penny-shaped crack with radius c as shown in Fig. 32.5. The material is assumed to be isotropic and infinitely extended. When a uniform

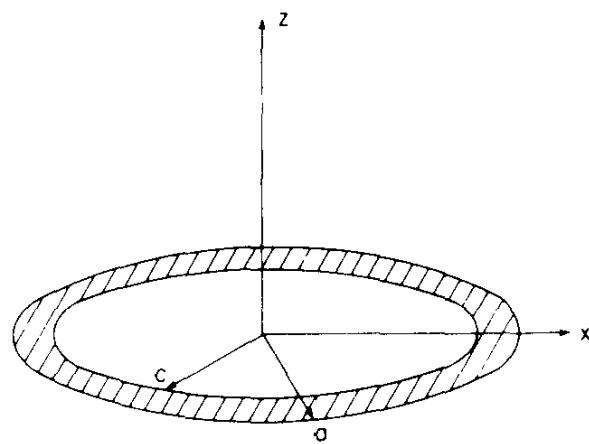


Fig. 32.5. Penny-shaped Dugdale-Barenblatt crack.

tension S is applied at infinity, the tip of the crack is subjected to an infinitely large stress even for a small S . The stress, however, is relaxed if a plastic zone at the neighborhood of the crack tip is developed, as indicated by the hatched area $c < r < a$ in Fig. 32.5. When the crack surface is normal to the tensile direction and the maximum shear theory of yielding is employed for the plastic domain, Dugdale's hypothesis requires the following boundary conditions for an axially symmetric problem:

$$\begin{aligned}\sigma_2 &= S && \text{at } z = \infty, \\ \sigma_{zr} &= \sigma_{z\theta} = 0 && \text{at } z = 0, \\ \sigma_z &= 0 && \text{at } z = 0, \quad 0 < r < c, \\ \frac{1}{2}(\sigma_z - \sigma_\theta) &= k_0 && \text{at } z = 0, \quad c < r < a, \\ w &= 0 && \text{at } z = 0, \quad a < r,\end{aligned}\tag{32.46}$$

where w is the displacement in the z -direction and k_0 is a point on the stress-strain curve as shown in Fig. 32.2. Since the material in the plastic zone is work-hardening, the stress state in this zone is not uniform. However, for mathematical simplicity we assume that the stress state is uniform in the plastic zone. It is convenient to consider, instead of (32.46), the following boundary conditions:

$$\begin{aligned}\sigma_z &= 0 && \text{at } z = \infty, \\ \sigma_{zr} &= \sigma_{z\theta} = 0 && \text{at } z = 0, \\ \sigma_z &= -S && \text{at } z = 0, \quad 0 < r < c, \\ \frac{1}{2}(\sigma_z - \sigma_\theta) &= k_0 - \frac{1}{2}S && \text{at } z = 0, \quad c < r < a, \\ w &= 0 && \text{at } z = 0, \quad a < r.\end{aligned}\tag{32.47}$$

Proof that $\frac{1}{2}(\sigma_z - \sigma_\theta)$ gives the maximum stress in Tresca's yield condition will be given after the solution has been obtained. Keer and Mura (1966) have solved this problem as described below.

According to Inglis (1913), Collins (1962), and England (1963), the solution for the axially symmetric problems can be expressed as

$$\begin{aligned}\sigma_z &= -\frac{\partial^2 \phi}{\partial z^2} + z \frac{\partial^3 \phi}{\partial z^3}, \\ \sigma_\theta &= -\frac{\partial^2 \phi}{\partial z^2} - (1 - 2\nu) \frac{\partial^2 \phi}{\partial r^2} + \frac{z}{r} \frac{\partial^2 \phi}{\partial r \partial z}, \\ \sigma_{zr} &= z \frac{\partial^3 \phi}{\partial r \partial z^2},\end{aligned}$$

$$\sigma_{z\theta} = \frac{z}{r} \frac{\partial^3 \phi}{\partial \theta \partial z^2}, \quad (32.48)$$

$$\sigma_r = \frac{\partial^2 \phi}{\partial r^2} + \frac{2\nu}{r} \frac{\partial \phi}{\partial r} + z \frac{\partial^3 \phi}{\partial r^2 \partial z},$$

$$2\mu w = -2(1-\nu) \frac{\partial \phi}{\partial z} + z \frac{\partial^2 \phi}{\partial z^2},$$

where

$$\begin{aligned} \frac{\partial \phi}{\partial z} &= \frac{1}{2i} \int_{-a}^a f(t) [r^2 + (z+it)^2]^{-1/2} dt, \\ \frac{\partial \phi}{\partial r} &= \frac{r}{2i} \int_{-a}^a f(t) \left\{ z+it + [r^2 + (z+it)^2]^{1/2} \right\}^{-1} [r^2 + (z+it)^2]^{-1/2} dt, \end{aligned} \quad (32.49)$$

$$f(t) = -f(-t).$$

The unknown function $f(t)$ is determined from the third and fourth conditions in (32.47), using (32.48). Since we have

$$\lim_{z \rightarrow 0} [r^2 + (z+it)^2]^{1/2} = (r^2 - t^2)^{1/2} \quad (r > t)$$

and

$$\lim_{z \rightarrow 0} [r^2 + (z+it)^2]^{1/2} = i(t^2 - r^2)^{1/2} \quad (t > r),$$

the derivatives of ϕ as $z \rightarrow 0$ become (when $f(0) = 0$)

$$\begin{aligned} \left(\frac{\partial \phi}{\partial z} \right)_{z=0} &= - \int_r^a \frac{f(t) dt}{(t^2 - r^2)^{1/2}} \quad (0 \leq r \leq a) \\ &= 0 \quad (a < r < \infty), \end{aligned}$$

$$\left(\frac{\partial^2 \phi}{\partial z^2} \right)_{z=0} = \frac{1}{r} \frac{d}{dr} \int_0^r \frac{tf(t) dt}{(r^2 - t^2)^{1/2}} = \int_0^r \frac{f'(t) dt}{(r^2 - t^2)^{1/2}} \quad (0 \leq r \leq a), \quad (32.50)$$

$$\left(\frac{\partial \phi}{\partial r} \right)_{z=0} = -\frac{1}{r} \int_0^r \frac{tf(t) dt}{(r^2 - t^2)^{1/2}} \quad (0 \leq r \leq a).$$

Conditions (32.47), therefore, lead to

$$\begin{aligned} -\frac{1}{r} \frac{d}{dr} \int_0^r \frac{tf(t) dt}{(r^2 - t^2)^{1/2}} &= -S & (r < c), \\ -\frac{(1-2\nu)}{2} \frac{d}{dr} \left\{ \frac{1}{r} \int_0^r \frac{tf(t) dt}{(r^2 - t^2)^{1/2}} \right\} &= k - \frac{1}{2}S & (c < r < a). \end{aligned} \quad (32.51)$$

The function $f(t)$ can be determined from (32.51) by the method described by Green and Zerna (1954),

$$\begin{aligned} \frac{1}{2}\pi f(t) &= St & (0 < t < c), \\ \end{aligned} \quad (32.52)$$

$$\frac{1}{2}\pi f(t) = St - \frac{4k_0 - (1+2\nu)S}{1-2\nu} \left\{ (t^2 - c^2)^{1/2} - \frac{c}{2} \sin^{-1} \frac{c}{t} \right\} \quad (c < t < a).$$

It is seen that $\frac{1}{2}(\sigma_z - \sigma_\theta)$ is the maximum shear stress.

Using (32.50) and (32.48), we have

$$\sigma_z - \sigma_\theta = 2k_0 - S,$$

$$\sigma_z - \sigma_r = 2k_0 - S - \left\{ 2k_0 - (1+2\nu)\frac{1}{2}S \right\} \frac{c}{r}, \quad (32.53)$$

$$\sigma_r - \sigma_\theta = \left\{ 2k_0 - (1+2\nu)\frac{1}{2}S \right\} \frac{c}{r},$$

at $z = 0$, $c < r < a$. Furthermore, we have

$$(\sigma_z)_{z=0, r=a} = -\lim_{t \rightarrow a} \frac{f(t)}{(a^2 - t^2)^{1/2}} + \int_0^a \frac{tf(t) dt}{(a^2 - t^2)^{3/2}} \quad (32.54)$$

by applying integration by parts to the second formula in (32.50). The requirement that the stress singularity vanish at $r = a$ leads to

$$f(a) = 0 \quad (32.55)$$

which becomes, from (32.52),

$$\frac{(1-2\nu)\left(\frac{S}{2k_0}\right)}{2 - (1+2\nu)\left(\frac{S}{2k_0}\right)} = \left\{ 1 - \left(\frac{c}{a}\right)^2 \right\}^{1/2} - \frac{c}{2a} \cos^{-1} \frac{c}{a}. \quad (32.56)$$

The displacement w can be easily obtained from (32.48) as

$$\begin{aligned} \frac{\pi}{4} \frac{2\mu(w)_{z=0^+}}{(1-\nu)} &= -\frac{\pi}{4} \frac{2\mu(w)_{z=0^-}}{(1-\nu)} \\ &= S(a^2 - r^2)^{1/2} - \frac{4k_0 - (1 + 2\nu)S}{(1 - 2\nu)} \left[\int_r^a \frac{(t^2 - c^2)^{1/2}}{(t^2 - r^2)^{1/2}} dt \right. \\ &\quad \left. - \frac{c}{2} \int_r^a \cos^{-1} c/t \frac{dt}{(t^2 - r^2)^{1/2}} \right] \quad (c \leq r \leq a). \quad (32.57) \end{aligned}$$

The value at the tip of crack is

$$\begin{aligned} \frac{2\mu(w)_{z=0,r=c}}{(1-\nu)ak_0} &= \frac{16}{\pi} \left[\left(1 - \frac{c^2}{a^2}\right)^{1/2} \left\{ \left(1 - \frac{c^2}{a^2}\right)^{1/2} - \frac{c}{2a} \cos^{-1} \frac{c}{a} \right\} \right. \\ &\quad \left. - 1 + \frac{c}{a} + \frac{c}{2a} \int_{c/a}^1 \frac{\cos^{-1}(c/at) dt}{(t^2 - c^2/a^2)^{1/2}} \right] \\ &\quad \times \left\{ (1 + 2\nu) \left[\left(1 - \frac{c^2}{a^2}\right)^{1/2} - \frac{c}{2a} \cos^{-1} \frac{c}{a} \right] + (1 - 2\nu) \right\}^{-1}. \quad (32.58) \end{aligned}$$

Equation (32.56) can be approximated as

$$1 - c/a = \frac{1}{8}(1 - 2\nu)^2 (S/k_0)^2 \quad (32.59)$$

and equation (32.58) as

$$2w \approx \frac{24(1-\nu)k_0}{\pi(1-2\nu)\mu} c(1 - c/a), \quad (32.60)$$

when $c/a \approx 1$ and $S/k_0 \ll 1$.

Eliminating $(1 - c/a)$ from (32.59) and (32.60), we have

$$S \approx \{\pi k_0 \mu 2w / 3(1 - \nu)(1 - 2\nu)c\}^{1/2}. \quad (32.61)$$

If it is postulated that the crack becomes unstable when the crack opening displacement $2w$ reaches a critical value Φ , then the critical applied stress can be expressed as

$$S = \left\{ \frac{\pi \mu k_0 \Phi}{3(1-\nu)(1-2\nu)c} \right\}^{1/2}. \quad (32.62)$$

The ratio of (32.22) to (32.62) is about 0.42 when $\nu = \frac{1}{3}$ and the κ for the plane stress case is used. This means that the slit-like crack is 0.42 times weaker than the penny-shaped crack for the same crack length c .

It will be shown that the approach using the dislocation distribution and that using the Dugdale plasticity hypothesis are equivalent for the case of the penny-shaped crack.

According to Kroupa (1960), the stress σ_z due to a circular dislocation located on the plane $z = 0$ is

$$\sigma_z = \frac{b\mu}{2(1-\nu)} \int_0^\infty s \xi J_0(\xi r) J_1(\xi s) d\xi, \quad z = 0, \quad (32.63)$$

where b is the Burgers vector in the z -direction and s is the radius of the dislocation. Keer and Mura (1966) have shown that the equivalent stress and displacement fields can be obtained when the distribution of dislocations is chosen as

$$\alpha_{\theta z} = -\frac{2(1-\nu)}{\mu} \frac{d}{dr} \int_r^a \frac{f(t) dt}{(t^2 - r^2)^{1/2}}, \quad (32.64)$$

where $f(t)$ is defined by (32.52).

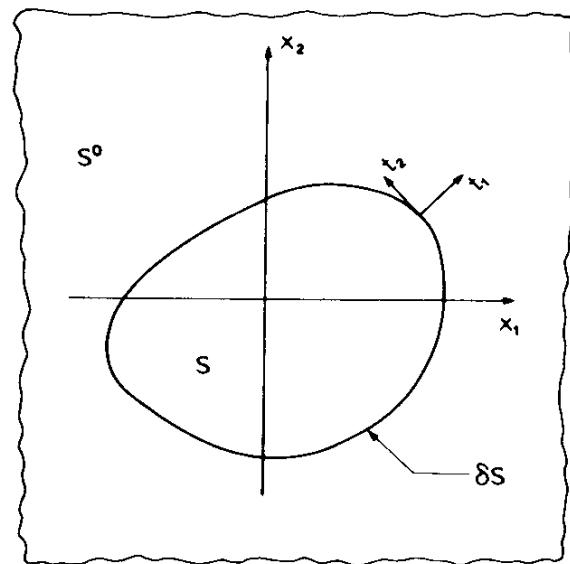
* 33. Stress intensity factors for an arbitrarily shaped plane crack

The three-dimensional elasticity problem of finding the stress distribution near a flat crack embedded in an infinitely extended homogeneous, isotropic solid, and opened up under prescribed internal normal pressure, has been solved analytically by a number of investigators for cracks having circular and elliptical contours. The problem becomes much more difficult, and analytical solutions are not available when the crack contour is other than circular or elliptical. For such cases, attempts have been made to formulate the problem in terms of integral equations which might be solved by numerical techniques.

An integral equation formulation of the general three-dimensional elasticity problem has been proposed by Kupradze (1953, 1965), Kinoshita and Mura (1956) and Mikhlin (1965). This formulation is known as the boundary integral equation (BIE) method. The method is derived from Green's formula (48.21). One obtains the limiting form of Somigliana's identity for displacements inside the body due to known surface tractions and displacements, when the point at which the displacement is evaluated approaches the surface of the body. This results in a system of coupled integral equations for the unknown surface tractions and displacements, as shown by equations (48.27) and (48.28) for the elasto-dynamics. The idea is equally applied to the elasto-statics (see Kitahara 1985).

Numerical approaches of the boundary integral equation method have been developed by Rizzo (1967), Cruse (1969), Lacht and Watson (1976), Banerjee and Butterfield (1981), and Brebbia, Futagami, and Tanaka (1983), among others. Cruse (1973) has reported an application of the numerical method to the solution of the penny-shaped crack problem. Bui (1975, 1977) obtained a singular integral equation for the crack displacement discontinuity. The shape of the crack can be rather general and the domain of integration is the surface of the crack. Bui has numerically solved the integral equation for the penny-shaped, elliptical and square crack under constant normal pressure. Weaver (1977) has derived from Somigliana's equation a singular integral equation for the unknown dislocations which are defined over the crack area only. He has numerically solved the problem of a crack in the form of a rectangle under constant normal pressure.

Considerable work in this area has been done by Soviet researchers. One formulation is given by Panasyuk (1968), who has reduced the crack problem to the solution of a singular integral equation for the crack displacement, but has not elaborated on the development of a solution technique for an arbitrarily shaped crack. He also discusses in some detail an approximate solution for an almost circular crack. Martynenko (1970), using Green's functions of a Riemannian space, gives the exact solution for a circular crack. He also obtains an approximate solution for a crack in the form of a strip which becomes exact when the strip becomes a half-plane. Andreykiv and Stadnik (1974) consider a plane crack whose contour is a convex, closed piece-wise smooth line, consisting of n ($n = 1, 2, \dots$) circular arcs forming sections of the circumferences of circles, each of which encloses the entire crack domain, and m ($m = 1, 2, \dots$) straight-line segments. They reduce the problem to the solution of a system of two-dimensional Fredholm integral equations with the stresses unknown outside the crack domain. They numerically solve this system for semi-circular and square cracks under constant pressure.

Fig. 33.1. An arbitrary plane crack S on $x_3 = 0$ plane.

What follows is a formulation of the plane crack problem in the work of Mastrojannis, Keer, and Mura (1979). To test the formulation, a numerical analysis is given involving plane cracks of various contour shapes embedded in an infinite isotropic solid and subjected to uniform and linearly varying internal normal pressure.

Consider an unbounded homogeneous elastic solid weakened by a plane crack S having a smooth contour ∂S . Let x_i ($i = 1, 2, 3$) be the Cartesian coordinates of the point x , and let the plane $x_3 = 0$ be the plane of the crack (Fig. 33.1). The faces of the crack are loaded symmetrically with respect to the $x_3 = 0$ plane with an arbitrary normal pressure of intensity $P(x_1, x_2)$; at infinity, the solid is stress-free. It is necessary to determine the stress $\sigma_{33}(x_1, x_2, x_3 = 0)$ in the region S^0 outside the crack domain, the normal displacement $u_3(x_1, x_2, x_3 = 0)$ of points inside the crack, and the stress intensity factor k_1 (Mode I) along the crack edge.

The determination of the stress $\sigma_{33}(x_1, x_2, 0)$ and the displacement $u_3(x_1, x_2, 0)$ reduces to a problem for the elastic half-space $x_3 \geq 0$ with the following boundary conditions (Sneddon 1966):

$$\begin{aligned} u_3(x_1, x_2, 0) &= 0 && \text{for } (x_1, x_2) \in S^0, \\ \sigma_{33}(x_1, x_2, 0) &= -P(x_1, x_2) && \text{for } (x_1, x_2) \in S, \\ \sigma_{31}(x_1, x_2, 0) &= \sigma_{32}(x_1, x_2, 0) = 0 && \text{for } (x_1, x_2) \in (S + S^0). \end{aligned} \quad (33.1)$$

The equations of equilibrium

$$u_{i,jj} + \frac{1}{1-2\nu} u_{j,ji} = 0 \quad (i, j = 1, 2, 3) \quad (33.2)$$

and the conditions of zero tangential stresses on the plane $x_3 = 0$ are satisfied by taking the displacement vector (e.g., Panasyuk, 1968) in the form

$$u_i = \Phi_i - \frac{x_3}{2(1-\nu)} \Phi_{3,i}, \quad (33.3)$$

where ν is Poisson's ratio and Φ_1 , Φ_2 , and Φ_3 are harmonic functions of x such that

$$\Phi_{i,3} = -\frac{1-2\nu}{2(1-\nu)} \Phi_{3,i} \quad (i = 1, 2). \quad (33.4)$$

The stresses σ_{33} , σ_{31} , and σ_{32} can be expressed in terms of Φ_3 as follows:

$$\begin{aligned} \sigma_{33}(x) &= \frac{E}{2(1-\nu^2)} [\Phi_{3,3} - x_3 \Phi_{3,33}], \\ \sigma_{31}(x) &= -\frac{E}{2(1-\nu^2)} x_3 \Phi_{3,31}, \\ \sigma_{32}(x) &= -\frac{E}{2(1-\nu^2)} x_3 \Phi_{3,32}, \end{aligned} \quad (33.5)$$

where E is Young's modulus. Thus, the elastic problem posed by equations (33.1) reduces to that of finding a harmonic function $\Phi_3(x)$ satisfying the mixed boundary conditions

$$\begin{aligned} \Phi_3(x_1, x_2, 0) &= 0 && \text{for } (x_1, x_2) \in S^0, \\ \left. \frac{\partial \Phi_3(x_1, x_2, x_3)}{\partial x_3} \right|_{x_3=0} &= -\frac{2(1-\nu^2)}{E} P(x_1, x_2) && \text{for } (x_1, x_2) \in S. \end{aligned} \quad (33.6)$$

To solve this problem, the harmonic function $\Phi_3(x)$ is expressed as a Fourier integral (Andreykiv and Stadnik 1974),

$$\Phi_3(x) = - \iint_{-\infty}^{+\infty} \frac{\exp \left[-x_3 (\xi_1^2 + \xi_2^2)^{1/2} + i(x_1 \xi_1 + x_2 \xi_2) \right]}{(\xi_1^2 + \xi_2^2)^{1/2}} A(\xi_1, \xi_2) d\xi_1 d\xi_2, \quad (33.7)$$

where $A(\xi_1, \xi_2)$ is an unknown function. Letting

$$q(x_1, x_2) = \begin{cases} -\frac{2(1-\nu^2)}{2\pi E} P(x_1, x_2) & \text{for } (x_1, x_2) \in S \\ \frac{2(1-\nu^2)}{2\pi E} \sigma_{33}(x_1, x_2, 0) & \text{for } (x_1, x_2) \in S^0, \end{cases} \quad (33.8)$$

we obtain from (33.6) and (33.7)

$$q(x_1, x_2) = \frac{1}{2\pi} \iint_{-\infty}^{\infty} \exp[+i(x_1\xi_1 + x_2\xi_2)] A(\xi_1, \xi_2) d\xi_1 d\xi_2 \quad (33.9)$$

which, upon application of the inverse Fourier transform theorem, gives

$$A(\xi_1, \xi_2) = \frac{1}{2\pi} \iint_{-\infty}^{\infty} \exp[-i(\xi_1\eta_1 + \xi_2\eta_2)] q(\eta_1, \eta_2) d\eta_1 d\eta_2. \quad (33.10)$$

Using (33.10) in (33.7) and noting that

$$\begin{aligned} & \frac{1}{2\pi} \iint_{-\infty}^{\infty} \frac{\exp[i(x_1 - \eta_1)\xi_1 + i(x_2 - \eta_2)\xi_2]}{(\xi_1^2 + \xi_2^2)^{1/2}} d\xi_1 d\xi_2 \\ &= \frac{L1}{[(x_1 - \eta_1)^2 + (x_2 - \eta_2)^2]^{1/2}}, \end{aligned} \quad (33.11)$$

we have

$$\begin{aligned} u_3(x_1, x_2, 0) &= \frac{(1-\nu^2)}{\pi E} \left\{ \iint_S \frac{P(\xi_1, \xi_2) d\xi_1 d\xi_2}{[(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2]^{1/2}} \right. \\ &\quad \left. - \iint_{S^0} \frac{\sigma_{33}(\xi_1, \xi_2, 0) d\xi_1 d\xi_2}{[(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2]^{1/2}} \right\}. \end{aligned} \quad (33.12)$$

Finally, from the first condition of (33.1), one obtains the following integral equation for the stress $\sigma_{33}(x_1, x_2, 0)$ in S^0 :

$$\begin{aligned} & \iint_S \frac{P(\xi_1, \xi_2) d\xi_1 d\xi_2}{[(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2]^{1/2}} = \iint_{S^0} \frac{\sigma_{33}(\xi_1, \xi_2, 0) d\xi_1 d\xi_2}{[(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2]^{1/2}} \\ & \text{for } (x_1, x_2) \in S^0. \end{aligned} \quad (33.13)$$

Once the stress $\sigma_{33}(x_1, x_2, 0)$ acting on the plane of the crack is known, the stress intensity factor k_I (Mode I) for a plane crack can be determined from the equation

$$k_I = \lim_{t_1 \rightarrow 0} (2\pi t_1)^{1/2} \sigma_{33}(t_1, t_2 = 0, t_3 = 0), \quad (33.14)$$

where t_1 , t_2 , and t_3 constitute a local orthogonal coordinate system on the crack contour, as shown in Fig. 33.1.

The analytical solution of (33.13) for an arbitrarily shaped crack appears to be beyond the present mathematical techniques. For this reason, numerical methods must be used in order to obtain specific results. It is well known that the stress $\sigma_{33}(\xi_1, \xi_2, 0)$ vanishes at infinity and has a square-root singularity on the smooth contour ∂S of the crack. Therefore, we assume that the stress distribution $\sigma_{33}(\xi_1, \xi_2, 0)$ in (33.13) has the form

$$\sigma_{33}(\xi_1, \xi_2, 0) = B(\xi_1, \xi_2) / [2\pi w(\xi_1, \xi_2)]^{1/2} \quad \text{for } (\xi_1, \xi_2) \in S^0, \quad (33.15)$$

where $B(\xi_1, \xi_2)$ is an unknown bounded function, and the function $w(\xi_1, \xi_2)$ represents the shortest distance from the point (ξ_1, ξ_2) to the crack contour. Thus, $w(\xi_1, \xi_2)$ behaves like t_1 in (33.14) near the crack neighborhood, and, at a point at infinity, $w(\xi_1, \xi_2) \rightarrow \infty$. Rvachev (1975) and Goncharyuk (1972) have given a method for the construction of $w(\xi_1, \xi_2)$.

Let the smooth crack contour ∂S be approximated by a set of n straight line segments with end-points $M_i(x_1, y_1)$, $N_i(x_2, y_2)$ ($i = 1, 2, 3, \dots, n$), where x_1 , y_1 are the coordinates of point M_i . When the crack contour is convex, the shortest distance of point (ξ_1, ξ_2) from the i^{th} straight line can be written as

$$f_i(\xi_1, \xi_2) = d_i - \xi_1 \cos \theta_i - \xi_2 \sin \theta_i, \quad (33.16)$$

where d_i is the normal distance from the origin of the ξ_1 , ξ_2 -coordinate system to the straight line $f_i(\xi_1, \xi_2) = 0$, and $(\cos \theta_i, \sin \theta_i)$ is a unit vector along d_i . Then, we write

$$w(\xi_1, \xi_2) = |f_i(\xi_1, \xi_2)\Lambda_1 f_2(\xi_1, \xi_2) \cdots \Lambda_n f_n(\xi_1, \xi_2)|, \quad (33.17)$$

where

$$\begin{aligned} f_i(\xi_1, \xi_2) \Lambda_1 f_j(\xi_1, \xi_2) \\ = \frac{1}{2} [f_i(\xi_1, \xi_2) + f_j(\xi_1, \xi_2) - |f_i(\xi_1, \xi_2) - f_j(\xi_1, \xi_2)|]. \end{aligned} \quad (33.18)$$

The right-hand side in (33.17) has been called the R_1 -conjunction by Rvachev (1975). It defines analytically the minimum value among $|f_1(\xi_1, \xi_2)|$, $|f_2(\xi_1, \xi_2)|, \dots, |f_n(\xi_1, \xi_2)|$. Furthermore, we assume that the function $B(\xi_1, \xi_2)$ has the following form:

$$B(\xi_1, \xi_2) = G(\xi_1, \xi_2) \iint_S \frac{[w(\eta_1, \eta_2)]^{1/2} P(\eta_1, \eta_2) d\eta_1 d\eta_2}{(\xi_1 - \eta_1)^2 + (\xi_2 - \eta_2)^2}$$

for $(\xi_1, \xi_2) \in S^0$, (33.19)

where $G(\xi_1, \xi_2)$ is a new unknown function to be determined.

With the above assumption, (33.15) becomes

$$\sigma_{33}(\xi_1, \xi_2, 0) = \frac{G(\xi_1, \xi_2)}{[2\pi w(\xi_1, \xi_2)]^{1/2}} \iint_S \frac{[w(\eta_1, \eta_2)]^{1/2} P(\eta_1, \eta_2) d\eta_1 d\eta_2}{(\xi_1 - \eta_1)^2 + (\xi_2 - \eta_2)^2}$$

for $(\xi_1, \xi_2) \in S^0$, (33.20)

and (33.14) leads to

$$k_1(\xi_1, \xi_2) = G(\xi_1, \xi_2) \iint_S \frac{[w(\eta_1, \eta_2)]^{1/2} P(\eta_1, \eta_2) d\eta_1 d\eta_2}{(\xi_1 - \eta_1)^2 + (\xi_2 - \eta_2)^2}$$

for $(\xi_1, \xi_2) \in \partial S$. (33.21)

The integral equation (33.13) is reduced to the following one which defines $G(\xi_1, \xi_2)$ in S^0 :

$$\begin{aligned} & \iint_S \frac{P(\xi_1, \xi_2) d\xi_1 d\xi_2}{[(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2]^{1/2}} \\ &= \iint_{S^0 - \Delta T} \frac{G(\xi_1, \xi_2) Q(\xi_1, \xi_2) d\xi_1 d\xi_2}{[2\pi w(\xi_1, \xi_2)]^{1/2} [(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2]^{1/2}} \\ &+ \iint_{\Delta T} \left[\frac{G(\xi_1, \xi_2) Q(\xi_1, \xi_2)}{[2\pi w(\xi_1, \xi_2)]^{1/2}} - \frac{G(x_1, x_2) Q(x_1, x_2)}{[2\pi w(x_1, x_2)]^{1/2}} \right] \end{aligned}$$

$$\begin{aligned}
& \times \frac{d\xi_1 d\xi_2}{[(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2]^{1/2}} \\
& + \frac{G(x_1, x_2)Q(x_1, x_2)}{[2\pi w(x_1, x_2)]^{1/2}} \iint_{\Delta T} \frac{d\xi_1 d\xi_2}{[(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2]^{1/2}} \\
& \text{for } (x_1, x_2) \in S^0, \tag{33.22}
\end{aligned}$$

where ΔT is a small region of the domain S^0 , inside which point (x_1, x_2) is located, and

$$Q(\xi_1, \xi_2) = \iint_S \frac{[w(\eta_1, \eta_2)]^{1/2} P(\eta_1, \eta_2) d\eta_1 d\eta_2}{(\xi_1 - \eta_1)^2 + (\xi_2 - \eta_2)^2} \quad \text{for } (\xi_1, \xi_2) \in S^0. \tag{33.23}$$

The sum of the third and fourth terms on the right-hand side in (33.22) are zero, but these terms are artificially added to avoid numerical integration at the singularity $\xi = x$. The fourth integral is analytically performed as shown below.

When ΔT is taken as a quadrilateral with the centroidal (x_1, x_2) , we have

$$\begin{aligned}
& \iint_{\Delta T} \frac{d\xi_1 d\xi_2}{[(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2]^{1/2}} \\
& = R_1 \log(C_1) + R_2 \log(C_2) + R_3 \log(C_3) + R_4 \log(C_4), \tag{33.24}
\end{aligned}$$

where

$$\begin{aligned}
R_1 &= [(Y_2 - Y_1)x_1 + (X_1 - X_2)x_2 + (X_2Y_1 - Y_2X_1)] \\
&/[(X_1 - X_2)^2 + (Y_1 - Y_2)^2]^{1/2}, \tag{33.25}
\end{aligned}$$

$$\begin{aligned}
C_1 &= \left\{ [(X_1 - x_1)^2 + (Y_1 - x_2)^2]^{1/2} + [(X_1 - x_1)^2 + (Y_1 - x_2)^2 - R_1^2]^{1/2} \right\} \\
&/\left\{ [(X_2 - x_1)^2 + (Y_2 - x_2)^2]^{1/2} - [(X_2 - x_1)^2 + (Y_2 - x_2)^2 - R_1^2]^{1/2} \right\},
\end{aligned}$$

and where (X_1, Y_1) , (X_2, Y_2) , (X_3, Y_3) , and (X_4, Y_4) are the coordinates of the corner points of the quadrilateral. The expressions for R_2 , C_2 , etc., can be obtained from (33.25) by the cyclic permutation of (1, 2, 3).

To treat the integral equation (33.22) numerically, the infinite domain of integration, S^0 , is replaced by a finite domain, S_F^0 , enclosed between the crack contour ∂S and a similarly oriented and shaped contour ∂S_F^0 at some distance from ∂S . The finite domain S_F^0 is then divided into N quadrilaterals over each of which the value of the unknown function $G(\xi_1, \xi_2)$ is assumed to be constant, where (ξ_1, ξ_2) defines the centroid of the corresponding quadrilateral. Also, the crack domain S is divided into M triangles and M quadrilaterals, where M is equal to the number of the line segments that compose the crack contour. Replacing the integrations over S and S_F^0 by the sum of integrals over quadrilaterals and triangles, the solution of (33.22) reduces to the solution of a system of N algebraic equations with N unknowns (the values of $G(\xi_1, \xi_2)$ on each quadrilateral in S_F^0).

* Numerical examples

The integral equation (33.22) has been solved and the stress intensity factor k_I calculated for cracks subjected to uniform and linearly varying internal pressure, i.e.,

$$\begin{aligned} P(x_1, x_2) &= P_0, \\ P(x_1, x_2) &= P_0 + P_1 x_1, \end{aligned} \tag{33.26}$$

where P_0 and P_1 are given constants. The cracks have contours in the shape of a circle, an ellipse, an egg, a square, and a triangle. The contour of each crack is approximated by 48 straight line segments, and the corners of the square and triangular cracks are rounded. The outer contour ∂S_F^0 is taken at a distance from the crack edge 3.5 times the radius of the inscribed circle. Choosing the center and the orientation of the coordinate system properly and utilizing the resulting symmetry about the ξ_1 -axis, we reduce the solution of integral equation (33.22) to the solution of a system of algebraic equations with 144 unknowns. The running time for the solution of each crack problem with the two loading cases was 143 seconds on the CDC 6600 Computer, the only input data being the coordinates of the 48 points on the crack contour, the values of P_0 and P_1 , the normal distance between ∂S and ∂S_F^0 , and the number of unknowns.

Figures 33.2 and 33.3 are plots of $k_I/2P_0(R/\pi)^{1/2}$ against the polar angle θ , where R is the radius of the inscribed circle. The ratio P_1/P_0 is taken to be 0.2. The results for the square crack under constant normal pressure seem to

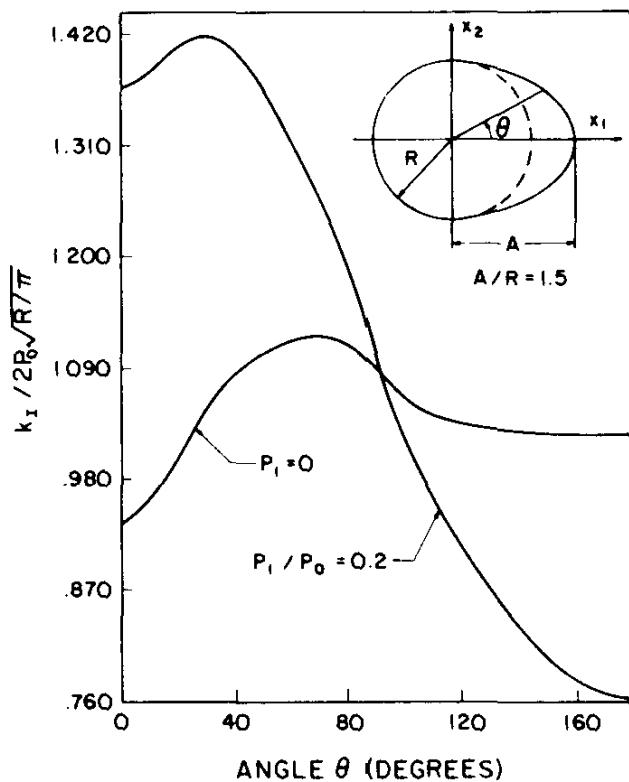


Fig. 33.2. The stress intensity factor along an egg-shaped plane crack.

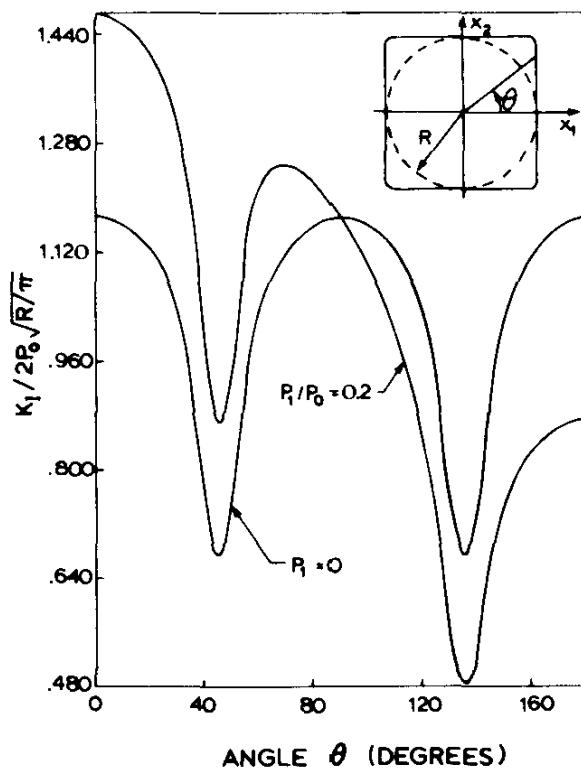


Fig. 33.3. The stress intensity factor along a rectangular plane crack.

be identical (excluding corners) to the results obtained numerically by Weaver (1977).

For polygonal plane-cracks, the stress singularity at points of abrupt changes in the slope of the crack contour is less severe than the square-root singularity along the smooth portions of the contour. For this reason, the stress intensity factor, as defined by (29.21), at those points assumes a zero value.

The method discussed here differs from those employed by Cruse (1969, 1973), Bui (1975, 1977), Weaver (1977), Andreykiv and Stadnik (1974), and Hayashi and Abe (1980). The present method results in a single integral equation for the function $G(\xi_1, \xi_2)$ characterizing the normal stresses outside the crack domain.

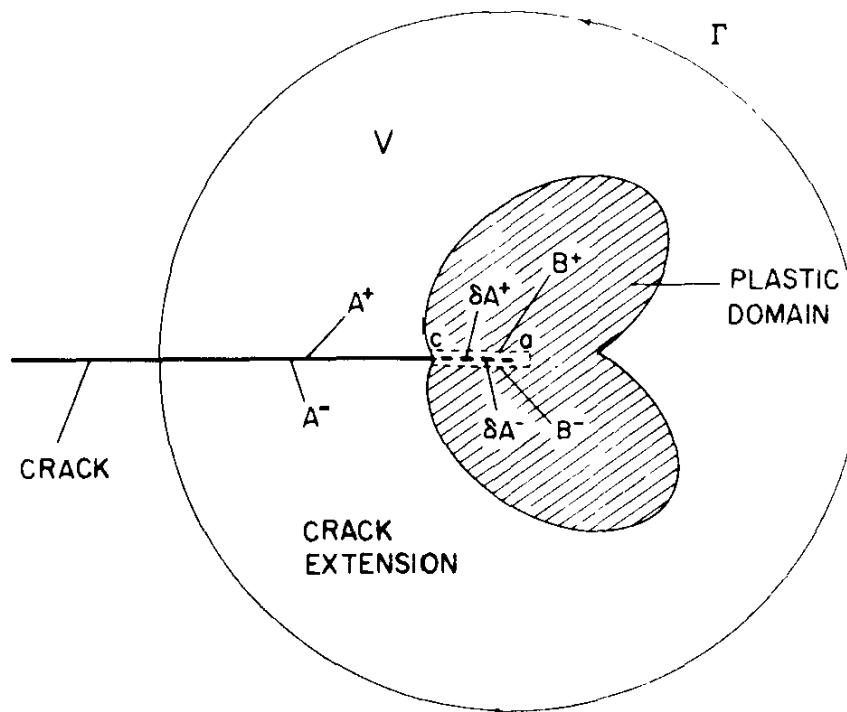
Although the support of the integral equation (33.22) is infinite and truncation of the domain of integration introduces some error, the numerical results obtained by the present method show only 0.5 per cent error for the circular crack and 1.85 per cent error for the elliptical crack compared with the known analytical results. This is extremely satisfactory compared with the results reported by Cruse (1973), 16 per cent in error, and by Bui (1977), 20 per cent in error, for the circular crack under constant pressure. In Bui's method, the support of the integral equation is finite (integration over the crack domain only), but the treatment of the singularity seems to be the main difficulty. The satisfactory results of the method offered here are attributed mainly to the choice of the stress distribution $\sigma_{33}(\xi_1, \xi_2, 0)$ in the form shown in (33.20) with the unknown function $G(\xi_1, \xi_2)$.

34. Crack growth

A crack tends to grow when an applied stress reaches a critical value. The crack propagation may be dynamic for brittle materials. If the material is ductile, the propagation may be quasi-static and the crack length may increase slowly with a load, maintaining at each instant a state of equilibrium. In the case of fatigue, cracks usually grow in the static fashion for any cyclic loading unless the tips of the cracks are extremely slanted or residual stresses are built up to oppose the motion of the cracks.

Energy release rate

Let us consider the static growth of a crack A in a body D subjected to the applied force F_i on its surface S . The displacement and stress fields are denoted by u_i and σ_{ij} , when the crack surface is A . We are interested in evaluating the change of energy during crack growth δA (Fig. 34.1) in order to

Fig. 34.1. Crack A grows by δA .

define the generalized force at the crack tip necessary for the crack extension. Changes of the displacement and stress fields due to the crack extension δA are denoted by δu_i and $\delta \sigma_{ij}$, yielding

$$\int_S F_i \delta u_i \, dS = \int_D \sigma_{ij} \delta u_{i,j} \, dD - \int_B \sigma_{ij} n_j [\delta u_i] \, dS, \quad (34.1)$$

where B is a surface of discontinuity of δu_i , and n is the normal vector on $B^+ \equiv B$. For most cases, B is δA . For the BCS model, B includes the plastic zone and $[\delta u_i = \delta u_i \text{ (on } B^+) - \delta u_i \text{ (on } B^-)]$. Equation (34.1) has been derived from $\int_S F_i \delta u_i \, dS = \int_S \sigma_{ij} n_j \delta u_i \, dS$ by applying Gauss' theorem, where the boundaries of D are S , A^+ , A^- , B^+ , and B^- (see Fig. 34.1) and where $\sigma_{ij,j} = 0$ in D and $\sigma_{ij} n_j = 0$ on $A^+ \equiv A$. The strain $\frac{1}{2}(u_{i,j} + u_{j,i})$ is the sum of elastic strain e_{ij} and plastic strain ϵ_{ij}^p ,

$$\frac{1}{2}(u_{i,j} + u_{j,i}) = e_{ij} + \epsilon_{ij}^p. \quad (34.2)$$

Therefore, (34.1) can be written

$$\int_S F_i \delta u_i \, dS - \int_D \sigma_{ij} \delta e_{ij} \, dD = \int_D \sigma_{ij} \delta \epsilon_{ij}^p \, dD - \int_B \sigma_{ij} n_j [\delta u_i] \, dS. \quad (34.3)$$

$\int_S F_i \delta u_i \, dS$ is the work done by applied force F_i during the change of crack surface δA and can be written as $\delta \int_S F_i u_i \, dS$ under the assumption that F does not change during the infinitesimal crack growth. $\int_D \sigma_{ij} \delta e_{ij} \, dD = \delta [\frac{1}{2} \int_D \sigma_{ij} e_{ij} \, dD]$ is the change of elastic strain energy. $\int_D \sigma_{ij} \delta \epsilon_{ij}^P \, dD$ is the plastic work done during the crack growth. Finally, $-\int_B \sigma_{ij} n_j [\delta u_i] \, dS$ is a positive work done at the crack tip during the crack opening $[\delta u_i]$.

The left-hand side in (34.3) is identical to $-\delta(\Delta W)$ when $\epsilon_{ij}^P = 0$, where ΔW has been defined by (25.14).

The expression

$$\Pi = \frac{1}{2} \int_D \sigma_{ij} e_{ij} \, dD - \int_S F_i u_i \, dS \quad (34.4)$$

is the mechanical energy portion of the Gibbs free energy. The Griffith fracture criterion can be written as

$$\delta G = \delta(\Pi + 2A\gamma) = 0, \quad (34.5)$$

where G is the Gibbs free energy and γ is the surface energy per unit area of the crack surface. Since the right-hand side in (34.3) expresses irreversible energies, $-\delta\Pi$ can be interpreted as the energy release during the crack growth δA . If a slit-like crack is changed by δA , the energy release rate, \mathcal{G}_i , is defined from

$$\mathcal{G}_i \delta \xi_i = -\delta\Pi = \int_D \sigma_{ij} \delta \epsilon_{ij}^P \, dD - \int_B \sigma_{ij} n_j [\delta u_i] \, dS, \quad (34.6)$$

where $\delta \xi_i$ is the displacement (change of position) of the crack tip due to the crack extension δA . For a three-dimensional crack, the left-hand side in (34.6) must be modified (see e.g. Palaniswamy and Knauss 1978).

For a slit-like crack, the generalized force acting on the tip of the crack is \mathcal{G}_i (crack extension force). For the slowly growing plastic crack, the second term in (34.6) can be neglected since it is of order $(\delta A)^2$. Then, (34.6) means that the plastic work during the crack growth $\delta \xi_1 = \delta A$ is proportional to δA . This result agrees with the experiment reported by Lee and Liebowitz (1978).

Irwin (1958) has calculated $-\int_{\delta A} \sigma_{ij} n_j [\delta u_i] \, dS$ for a slit-like elastic crack with length $2c$ under tension σ_{22}^0 in an elastic medium. According to the elasticity analysis, $\sigma_{22} = k_I / \{2\pi(x_1 - X)\}^{1/2}$ and $u_2 = (1 - \nu)k_I \{2(X - x_1)\}^{1/2} / \mu\sqrt{\pi}$, where X is the x_1 -coordinate of the crack tip and $k_I = \sigma_{22}^0 (\pi c)^{1/2}$.

Irwin assumes that $[\delta u_i]$ in (34.6) is half of $[\delta u_2] = 2u_2 = 2(1 - \nu)k_I\{2(X + \delta\xi_1 - x_1)\}^{1/2}/\mu\sqrt{\pi}$. Then,

$$\begin{aligned}\mathcal{G}_I \delta\xi_1 &= \int_X^{X+\delta\xi_1} (1 - \nu) k_I^2 \left(\frac{X + \delta\xi_1 - x_1}{x_1 - X} \right)^{1/2} (\mu\pi)^{-1} dx_1 \\ &= \frac{1 - \nu}{2\mu} k_I^2 \delta\xi_1.\end{aligned}\quad (34.7)$$

The crack extension force is obtained as

$$\mathcal{G}_I = (1 - \nu) k_I^2 / 2\mu. \quad (34.7.1)$$

The crack extension per unit length creates the surface energy 2γ . When 2γ is expressed in terms of k_I and 2μ from (27.20) with (30.11), it becomes the crack extension force (34.7.1). The same expression can be obtained from (30.15) by dividing by 2 since (30.15) is the energy increase caused by the crack extensions at the two tips of the crack. The same integral $-\int_{\delta A} \sigma_{ij} n_j [\delta u_i] dS$ can be calculated for the Dugdale crack. From (32.20), $[\delta u_2] \approx (1 + \kappa)\pi S^2 \delta\xi_1 / 8\mu k_0$ and $\sigma_{22} \approx k_0(\delta c = \delta\xi_1)$. Then, the integral becomes $(1 + \kappa)\pi S^2 (\delta\xi_1)^2 / 8\mu$, which becomes zero at the limit $\delta\xi_1 \rightarrow 0$. Yokobori and his co-authors (1966, 1968) and Rice (1966) pointed out this paradoxical result. This inconsistency with the Orowan-Irwin theory was also discussed later by Kfouri and Rice (1977). Wnuk (1972, 1974) has postulated that the integral domain in the second term in (34.6) is a material constant (process-zone size) and has derived an expression for a quasi-static extension of a tensile crack. Denoting the process zone by Δ , he has defined the crack separation energy by

$$\mathcal{G}_I^\Delta \delta\xi_1 = \int_c^{c+\Delta} k_0 [\delta u_2] dx_1, \quad (34.7.2)$$

where k_0 is defined in (32.1).

From (34.13.1),

$$\begin{aligned}\mathcal{G}_I^\Delta &= -k_0 \int_c^{c+\Delta} [u_{2,1}] dx_1 \\ &= -2k_0 \{u_2(c + \Delta, c) - u_2(c, c)\} \\ &\approx -2k_0 \{u_2(c + \Delta, c) - u_2(c + \Delta, c + \Delta)\}.\end{aligned}\quad (34.7.3)$$

For small-scale yielding, (32.18) can be approximated as

$$u_2(x_1, c) = \frac{4(1-\nu^2)k_0}{\pi E} \left\{ R - \frac{1}{2}\xi - \frac{1}{2}\xi \log \frac{4R}{\xi} \right\}, \quad (34.7.4)$$

where $a - c = R$ and $x_1 - c = \xi$. Then, (34.7.3) yields

$$\mathcal{G}_1^\Delta = \frac{8(1-\nu^2)k_0^2\Delta}{\pi E} \left(\frac{dR}{dc} + \frac{1}{2} + \frac{1}{2} \log \frac{4R}{\Delta} \right). \quad (34.7.5)$$

This is Wnuk's equation (1972) equation. A similar result is obtained by Rice and Sorensen (1976), Kfouri and Rice (1977), and Kfouri (1979).

From (32.20) we have, for small-scale yielding,

$$\begin{aligned} R &= \frac{1}{2}cQ^2, \\ Q &\equiv \pi S/2k_0, \end{aligned} \quad (34.7.6)$$

where Q is a loading parameter. If Δ and \mathcal{G}_1^Δ are constant, from (34.7.5) and (34.7.6) we obtain a relation, $Q = Q(c)$, which represents a stable crack growth. The catastrophic propagation is determined by the condition $dQ/dc = 0$. This idea has been developed by Cherepanov (1968), McClintock (1958), Rice (1968), and Wnuk (1979), among others.

The J-integral

The generalized force acting at the tip of a crack can be expressed in terms of a path-independent integral (*J*-integral) of a Maxwell tensor of elasticity taken over a surface. This fact has been developed by Eshelby (1951) and later by Rice (1968) and Eshelby (1970).

The three-dimensional expression of the *J*-integral is defined by

$$J_l = \int_{\Gamma} W n_l \, dS - \int_{\Gamma} \sigma_{ij} n_j u_{i,l} \, dS = \int_{\Gamma} P_{lj} n_j \, dS \quad (34.8)$$

with

$$P_{lj} = W \delta_{lj} - \sigma_{ij} u_{i,l}, \quad (34.8.1)$$

where Γ is an arbitrary closed surface (a closed curve for the two-dimensional

case) including the crack tip, \mathbf{n} is the outward unit normal on dS , P_{ij} is the energy-momentum tensor, and W is defined by

$$W_{,l} = \sigma_{ij}\epsilon_{ij,l}, \quad (34.9)$$

where $\epsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})$. It is easy to show that J_l is a path-independent integral. If another path (surface) Γ^* is taken, application of Gauss' theorem leads to $\int_{\Gamma - \Gamma^*} W \mathbf{n}_l \, dS - \int_{\Gamma - \Gamma^*} \sigma_{ij} n_j u_{i,l} \, dS = 0$.

For nonlinear elastic materials, (34.9) can be integrated as

$$W = \int_0^{\epsilon_{ij}} \sigma_{ij} \, d\epsilon_{ij}. \quad (34.9.1)$$

If the plastic unloading is involved, we cannot have (34.9.1) from (34.9).

Denoting by V the domain bounded by Γ , A^+ , B^+ , A^- , and B^- (Fig. 34.1), we write (34.8), by Gauss' theorem, as

$$J_l = \int_V W_{,l} \, dD - \int_V \sigma_{ij} u_{i,lj} \, dD + \int_{\pm B} \sigma_{ij} n_j u_{i,l} \, dS, \quad (34.9.2)$$

where $W_{,l} = \sigma_{ij} u_{i,jl}$, $\sigma_{ij,j} = 0$, and $\sigma_{ij} n_j = 0$ on A . It has been assumed in the derivation of (34.9.2) that W on $A^+ + B^+$ is equal to W on $A^- + B^-$. The first and second integrals in (34.9.2) cancel each other and $\sigma_{ij} n_j$ on B^+ is $-\sigma_{ij} n_j$ on B^- . Then, (34.9.2) becomes

$$J_l = \int_B \sigma_{ij} n_j [u_{i,l}] \, dS, \quad (34.10)$$

where $[u_{i,l}] = (u_{i,l} \text{ on } B^+ = B) - (u_{i,l} \text{ on } B^-)$.

On the crack surface, u_i is multiple-valued. After the crack tip has moved by $\delta\xi$, the change of displacement can be written as

$$\delta u_i = u_i(x - \delta\xi) - u_i(x) + \Delta u_i, \quad (34.11)$$

where Δu_i is a single-valued function. Equation (34.11) is Eshelby's basic assumption. Taylor's expansion of $u_i(x - \delta\xi)$ about point x is

$$u_i(x - \delta\xi) = u_i(x) - u_{i,l} \delta\xi_l. \quad (34.12)$$

Then, (34.11) becomes

$$\delta u_i = -u_{i,l} \delta\xi_l + \Delta u_i \quad (34.13)$$

or

$$[\delta u_i] = -[u_{i,l}] \delta \xi_l, \quad (34.13.1)$$

since Δu_i is a single-valued function. Therefore, we can write (34.10) in the form

$$J_l \delta \xi_l = - \int_B \sigma_{ij} n_j [\delta u_i] dS. \quad (34.14)$$

From (34.6) and (34.14) we have, for the slit-like crack,

$$\mathcal{G}_l \delta \xi_l = \int_D \sigma_{ij} \delta \epsilon_{ij}^P dD + J_l \delta \xi_l. \quad (34.15)$$

When $\delta \xi_l = \delta \xi_1 = \delta A$ and $\delta \epsilon_{ij}^P = 0$, we have

$$\mathcal{G}_1 = J_1. \quad (34.16)$$

The meaning of the J -integral in plane problems and in the absence of plastic deformations, is clearly shown by (34.16); however, the same is not the case when plastic deformations are involved. Miyamoto and Kageyama (1978) modified the J -integral for plastic deformations. It should be noted that J_l becomes zero when the material is perfectly plastic and $[\delta u_i]$ is of the order of $\delta \xi_1$, since $B = \delta \xi_1$ and $\sigma_{ij} n_j = k_0$ in (34.14). J_1 becomes $k_0 \delta_t$ for the BCS model, where δ_t is the crack opening displacement at the crack tip.

If dislocations in the neighborhood of the crack tip are displaced by the same amount as the displacement of the crack tip, we can write

$$\delta \epsilon_{ji}^P = -\epsilon_{jlh} \delta \xi_l \alpha_{hi}, \quad (34.17)$$

where α_{hi} is the dislocation density tensor defined by (7.33) [see (38.18)]. Therefore, Mura (1979) has written (34.15) as

$$\mathcal{G}_l = - \int_D \sigma_{ij} \epsilon_{jlh} \alpha_{hi} dD + J_l. \quad (34.18)$$

The integrand in the integral represents the Peach-Koehler (1950) force on dislocations.

Other path-independent integrals related to energy release rates have been investigated by Knowles and Sternberg (1972) and Budiansky and Rice (1973). The J -integral is also used for investigating singular behavior at the end of a

tensile crack in a hardening material (Hutchinson 1968, Rice and Rosengren 1968).

Eshelby (1970) has suggested a broader application of the energy-momentum tensor to phase transformation problems. A martensitic transformation region B undergoes a change of natural shape and size, that is, a change of inhomogeneous inclusion. He writes the change of the mechanical energy (34.4), caused by a migration $\delta\xi_i$ of the inclusion surface S , as

$$\delta\Pi = - \int_S [P_{lj}] \delta\xi_l n_j \, dS, \quad (34.18.1)$$

where $[P_{lj}] = P_{lj}(\text{out}) - P_{lj}(\text{in})$, as defined in (6.4). Since

$$[u_{i,l}] = \left[\frac{\partial u_i}{\partial n} \right] n_l, \quad (34.18.2)$$

where $\partial/\partial n$ denotes differentiation along the normal, (34.18.1) is equivalent to the statement that there is an effective normal force

$$F_l = \left([W] - \sigma_{ij} n_j \left[\frac{\partial u_i}{\partial n} \right] \right) n_l \quad (34.18.3)$$

per unit area of the interface. Equation (34.18.3) with $F_l = 0$ can be used to find the equilibrium position of phase and twin boundaries in the presence of stresses produced by the transformation itself, or applied externally, or both.

Fatigue

It is well known that materials fail under repeated (cyclic) loading and unloading at stresses smaller than the static fracture stresses. The magnitude of the stress required to produce failure decreases as the number of stress cycles increases. Micro-cracks are created in early cycles and they grow with a velocity according to a certain law; e.g. the Paris law (1964). Weertman (1966, 1973) has derived the Paris law by using the BCS model. A similar extension of the BCS model to fatigue is given by Lardner (1968), Bilby and Heald (1968), McCartney and Gale (1971), and Mura and C.T. Lin (1974). Recently, Budiansky and Hutchinson (1978) have used the Dugdale method for the analysis of fatigue crack closure. Tanaka and Nakai (1984) extended the analysis to a finite crack.

Let us consider a slit-like crack ($|x_1| < c, x_2 = 0$) as shown in Fig. 32.1, which may stand for an initial crack or a sharp notch. When $\sigma_{22}^0 = S$ is first attained at infinity, a plastic zone develops in the domain ($c < |x_1| < a, x_2 =$

0). The displacement is given by (32.20), where k_0 is the flow stress at this stage. The plastic zone is assumed to be small compared with c , and k_0 can be defined uniformly throughout the plastic zone. If the first unloading is $-\Delta S$ (the applied stress is reduced to $S - \Delta S$), the change of stress and displacement is defined by the boundary conditions similar to (32.1),

$$\begin{aligned}\sigma_{2j} &= 0 \quad (j = 1, 2) && \text{at } x_2 = 0 \quad |x_1| < c, \\ \sigma_{22} &= -k_0 - (k_0 + \Delta k_1) && \text{at } x_2 = 0 \quad c < |x_1| < a_1, \\ \sigma_{22} &= -\Delta S && \text{at } x_2 = \infty, \\ u_2 &= 0, \quad \sigma_{21} = 0 && \text{at } x_2 = 0, \quad a < |x_1|,\end{aligned}\tag{34.19}$$

where $c < |x_1| < a_1$ is a new plastic zone caused by the unloading. The stress and displacement in (34.19) are increments of the state described by (32.1). The real stress and displacement at the unloaded state are the sum of those corresponding to (32.1) and those corresponding to (34.19). The flow stress in the new plastic domain is $-(k_0 + \Delta k_1)$, under the assumption that there is no Bauschinger effect (see Fig. 34.2). The elasticity solution for the boundary conditions (34.19) is similar to (32.20). It is obtained from (32.20) by changing S to $-\Delta S$ and also changing k_0 to $-2k_0 - \Delta k_1$. It hence follows that

$$\begin{aligned}1 - c/a_1 &= \frac{1}{8}\pi^2 \left\{ \Delta S/(2k_0 + \Delta k_1) \right\}^2, \\ u_2 &= -\frac{(1 + \kappa)(2k_0 + \Delta k_1)}{2\pi\mu} c(1 - c/a_1).\end{aligned}\tag{34.20}$$

The quantities corresponding to the subsequent loading by ΔS (the applied

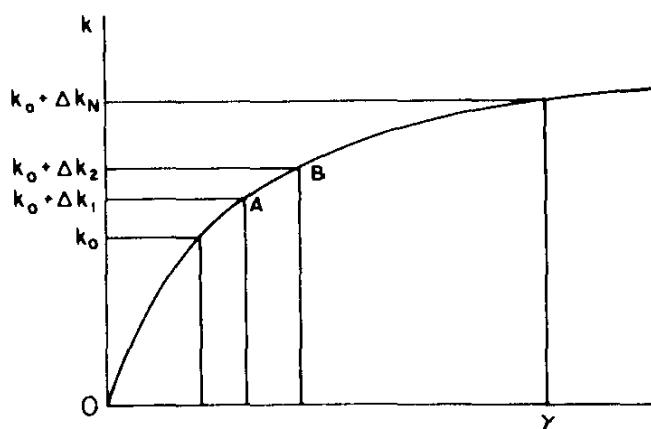


Fig. 34.2. Work-hardening stress-strain curve.

stress becomes S again) are obtained in a similar way: we use ΔS and $k_0 + \Delta k_1 + k_0 + \Delta k_2$ in place of $-\Delta S$ and $-2k_0 - \Delta k_1$ in (34.20). Then,

$$\begin{aligned} 1 - c/a_2 &= \frac{1}{8}\pi^2 \left\{ \Delta S/(2k_0 + \Delta k_1 + \Delta k_2) \right\}^2, \\ u_2 &= \frac{(1+\kappa)(2k_0 + \Delta k_1 + \Delta k_2)}{2\pi\mu} c(1 - c/a_2). \end{aligned} \quad (34.21)$$

After n cycles of loading and unloading, we have

$$\begin{aligned} 1 - c/a_n &= \frac{1}{8}\pi^2 \left\{ \Delta S/(2k_0 + \Delta k_{n-1} + \Delta k_n) \right\}^2, \\ u_2 &= \frac{(1+\kappa)(2k_0 + \Delta k_{n-1} + \Delta k_n)}{2\pi\mu} c(1 - c/a_n). \end{aligned} \quad (34.22)$$

It is noted that

$$a > a_1 > a_2 > \dots > a_n \quad (34.23)$$

for work-hardening materials, and

$$a > a_1 = a_2 = \dots = a_n \quad (34.24)$$

for perfectly plastic materials.

Suppose that the initial slit starts to grow after the critical number of cycles, N . We seek to establish some relation between the applied stress, the slit length, and N . For this purpose we eliminate $(1 - c/a_n)$ from the two equations in (34.22) to obtain

$$\begin{aligned} u_2 &= \frac{(1+\kappa)\pi}{16\mu} c(\Delta S)^2 / (2k_0 + \Delta k_{n-1} + \Delta k_n) \\ &\approx \frac{(1+\kappa)}{16\mu} \pi c (\Delta S)^2 / 2(k_0 + \Delta k_n). \end{aligned} \quad (34.25)$$

We assume that the crack starts to grow when the total plastic work done at the tip of the crack attains a critical value, U . The total plastic work is $2k_0 u_2 + \sum_{n=1}^N 2(k_0 + \Delta k_n) 2u_2$, where u_2 in the first term is (32.20) and the second u_2 is (34.25), and the factor 2 comes from the work done at the upper and lower surfaces at the crack tip. The other factor 2 in the second term

comes from the fact that N refers to both N loadings and N unloadings. Then, our energy criterion leads to

$$U = \frac{1+\kappa}{8\mu} \pi c S^2 + N \frac{1+\kappa}{8\mu} \pi c (\Delta S)^2 \quad (34.26)$$

or

$$N = \left(U - \frac{1+\kappa}{8\mu} K^2 \right) \frac{8\mu}{1+\kappa} (\Delta K)^{-2}, \quad (34.27)$$

where $\Delta K = (\pi c)^{1/2} \Delta S$ and $K = (\pi c)^{1/2} S$. K is called the stress intensity factor and ΔK is called the stress amplitude intensity factor. If the slit (length $2c$) is located at the center of a strip (width $2L$), $\Delta K = (\pi c)^{1/2} \Delta S [(2L/\pi c) \tan(\pi c/2L)]^{1/2}$. This ΔK is also used approximately for a double edge-notched strip (notch length a and strip width $2L$). For a single edge-notched strip (notch length c and strip width $2L$), the correction factor, to be multiplied to $(\pi c)^{1/2} \Delta S$, varies from 1.15 (for $c/L = 0.1$) to 2.86 (for $c/L = 1$).

From the first equation in (34.22), we have, approximately,

$$a_N - c = \frac{\pi}{32} \frac{(\Delta K)^2}{(k_0 + \Delta k_N)^2}. \quad (34.28)$$

The rate of crack growth is

$$\frac{dc}{dN} = (a_N - c)/N \quad (34.29)$$

which leads to

$$\frac{dc}{dN} = \frac{\pi(1+\kappa)}{256\mu k_N^2} (\Delta K)^4 \left/ \left(U - \frac{1+\kappa}{8\mu} K^2 \right) \right., \quad (34.30)$$

where $k_N = k_0 + \Delta k_N$. This is the fourth power law of Paris (1964). It has been assumed in the derivation that the crack grows in the same fashion after N cycles where the initial crack in the second step is the final crack in the first step. Experiments (e.g., Izumi and Fine, 1979) seem to support the tendency shown in (34.30).

A similar result can be obtained for the anti-plane problem where a shear stress $\sigma_{32}^0 = S$ is applied at infinity and σ_{32}^0 is cyclically changed with amplitude ΔS . Then, we have

$$\frac{dc}{dN} = \frac{\pi(\Delta K)^4}{64\mu k_N^2} / (U - K^2/2\mu). \quad (34.31)$$

If a penny-shaped crack is considered under cyclic tension with amplitude ΔS , we have, from (32.59) and (32.60),

$$\begin{aligned} a_N - c &= (1 - 2\nu)^2 c (\Delta S)^2 / 32(k_0 + \Delta k_N)^2, \\ w &= 3(1 - \nu)(1 - 2\nu)c(\Delta S)^2 / 4\pi\mu(k_0 + \Delta k_n), \end{aligned} \quad (34.32)$$

assuming $\Delta k_n \approx \Delta k_{n-1}$. Our crack growth assumption leads to

$$U = 2k_0w + \sum_{n=1}^N 2(k_0 + \Delta k_n)2w, \quad (34.33)$$

where w in the first term is (32.60) and w in the second term is given by (34.32). Then,

$$U = 3(1 - \nu)(1 - 2\nu)\{cS^2 + Nc(\Delta S)^2\}/\pi\mu, \quad (34.34)$$

and, therefore, (34.29) gives

$$\frac{dc}{dN} = \frac{3(1 - \nu)(1 - 2\nu)^3 \pi(\Delta K)^4}{512\mu k_N^2 \{U - 3(1 - \nu)(1 - 2\nu)K^2/4\mu\}}, \quad (34.35)$$

where $k_N = k_0 + \Delta k_N$, $K = 2(c/\pi)^{1/2}S$, and $\Delta K = 2(c/\pi)^{1/2}\Delta S$.

Comparing (34.31) and (34.35), it can be said that the crack growth rate of a slit-like crack is about 100 times that of a penny-shaped crack.

Weertman (1981) has derived a second-power, Paris fatigue crack growth equation for intrinsically ductile materials. Through repetition of forward and reversed crack tip shear sliding, a crack tip can actually advance an increment each stress cycle, as observed by Laird and Smith (1962) and Neuman (1974). Weertman's theory is supported by experiments done by Kobayashi and Mura (1983).

Dynamic crack growth

When a crack extends at speeds high enough to necessitate the inclusion of inertia effects, the associated kinetic energy must enter the energy argument of the previous subsections. In recent years a rather extensive body of literature has become available for the analysis of dynamic problems of crack propagation in homogeneous, isotropic, linearly elastic solids. Erdogan (1968), Achenbach (1974), Freund (1975) and Kanninen (1978) have presented review articles on this subject.

Let us consider the dynamic growth of a crack A in a body D subjected to an applied force F_i on the surface S of D . The rate of work done by F_i is

$$\int_S F_i \dot{u}_i \, dS = \int_D \sigma_{ij} \dot{u}_{i,j} \, dD + \int_D \rho \ddot{u}_i \dot{u}_i \, dD - \int_B \sigma_{ij} n_j [\dot{u}_i] \, dS, \quad (34.36)$$

where B is the domain where \dot{u}_i has a multiple value. The right-hand side in (34.36) has been derived from $\int_S \sigma_{ij} n_j \dot{u}_i \, dS$ by applying Gauss' theorem, where $\sigma_{ij,j} = \rho \ddot{u}_i$ in D , $\sigma_{ij,j} = 0$ on A , $[\dot{u}_i] = \dot{u}_i$ (on B^+) – \dot{u}_i (on B^-), and n is the unit normal on $B^+ \equiv B$. Since $\frac{1}{2}(\dot{u}_{i,j} + \dot{u}_{j,i}) = \dot{e}_{ij} + \dot{\epsilon}_{ij}^p$, (34.36) can be written as

$$\begin{aligned} & \int_S F_i \dot{u}_i \, dS - (d/dt) \left[\frac{1}{2} \int_D \sigma_{ij} e_{ij} \, dD + \frac{1}{2} \int_D \rho \dot{u}_i \dot{u}_i \, dD \right] \\ &= \int_D \sigma_{ij} \dot{\epsilon}_{ij}^p \, dD - \int_B \sigma_{ij} n_j [\dot{u}_i] \, dS = \mathcal{G}_i \dot{\xi}_i \end{aligned} \quad (34.37)$$

for a slit-like crack, where e_{ij} and $\dot{\epsilon}_{ij}^p$ are elastic and plastic strains, respectively, \mathcal{G}_i is the energy release rate, and $\dot{\xi}_i$ is the velocity of the crack. Achenbach (1974) further investigated \mathcal{G}_i for a slit-like crack in an elastic medium as Irwin (1958) did for a static crack [see (34.7)]. Suppose at time $t = t_0$ a slit-like crack of width $2c$ starts to grow and at time $t > t_0$ the position of the crack tip is $x_1 = X(t - t_0)$ (see Fig. 34.3), where $X(0) = 0$. The singular parts of the stresses in the plane of the crack are of the general form

$$\sigma_{2i}(x_1, 0, t) = T_{2i} [x_1 - X(t - t_0)]^{-1/2}, \quad (34.38)$$

and the singular parts of the particle velocities assume the forms

$$\begin{aligned} \dot{u}_i(x_1, 0^+, t) &= \dot{U}_i [X(t - t_0) - x_1]^{1/2}, \\ \dot{u}_i(x_1, 0^-, t) &= -\dot{U}_i [X(t - t_0) - x_1]^{1/2}. \end{aligned} \quad (34.39)$$

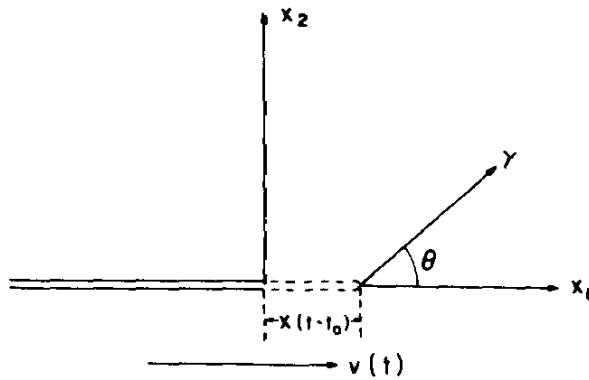


Fig. 34.3. Crack growth.

The functions T_{2i} and \dot{U}_i , which generally depend on time and on the geometrical and material parameters, are termed the stress intensity function and the velocity intensity function, respectively. Achenbach has evaluated

$$-\int_B \sigma_{ij} n_j [\dot{u}_i] dS = \int_{X(t-t_0)}^{X(t-t_0)+\epsilon} \sigma_{2i} [\dot{u}_i] dx_1 = \pi T_{2i} \dot{U}_i, \quad (34.40)$$

where ϵ is an arbitrarily small positive number. More specific expressions for T_{2i} and \dot{U}_i were obtained by Achenbach (1974) for the case of anti-plane shear. The case of plane strain is examined by Nuismer and Achenbach (1972) and Freund (1972). For the symmetric crack opening (Mode I), the instantaneous hoop stress near the crack tip may be expressed as

$$\sigma_\theta \approx (2\pi r)^{-1/2} k_I(t, v) T_\theta^I(\theta, v), \quad (34.40.1)$$

where $T_\theta^I(0, v) = 1$, $k_I(t, v)$ is the Mode I elastodynamic stress intensity factor, and $v = \dot{\xi}$ is the uniform velocity of the crack. The function $T_\theta^I(\theta, v)$ is universal because it is independent of the overall geometry and the loading.

The conditions governing crack motion can be expressed in terms of $k_I(t, v)$ and an experimentally determined critical value that is assumed to be a material property. According to a brief review by Achenbach and Kanninen (1978), the initiation of crack growth is determined by

$$k_I(t, 0) = K_d(\dot{\sigma}), \quad (34.40.2)$$

where $\dot{\sigma}$ represents the loading rate. For perfectly brittle fracture, $K_c = K_d$. Similarly, for a propagating crack,

$$k_I(t, v) = K_D(v), \quad (34.40.3)$$

where K_D is known as the dynamic fracture toughness. A crack is arrested after time t_a when $k_I < K_D$ for all $t > t_a$.

The J -integral (34.8) can also be modified for the dynamic case, where we add the kinetic energy density K to obtain

$$J_l = \int_{\Gamma} (W + K) n_l \, dS - \int_{\Gamma} \sigma_{ij} n_j u_{i,l} \, dS, \quad (34.41)$$

where

$$K = \frac{1}{2} \rho \dot{u}_i \dot{u}_i \quad (34.42)$$

and W is defined by (34.9). Multiplying (34.41) by $\dot{\xi}_l$ and applying Gauss' theorem, we have, for a slit-like crack,

$$\begin{aligned} J_l \dot{\xi}_l &= \int_V (\sigma_{ij} u_{i,jl} + \rho \dot{u}_i \dot{u}_{i,l}) \dot{\xi}_l \, dD - \int_V \sigma_{ij} u_{i,lj} \dot{\xi}_l \, dD \\ &\quad - \int_V \rho \ddot{u}_i u_{i,l} \dot{\xi}_l \, dD + \int_{B^+} \sigma_{ij} n_j u_{i,l} \dot{\xi}_l \, dS, \end{aligned} \quad (34.43)$$

where V is interpreted as being bounded by Γ , A^+ , B^+ , A^- and B^- , and W and K on $A^+ + B^+$ are assumed equal to W and K on $A^- + B^-$. $u_{i,l}$ on B^+ is not equal to $u_{i,l}$ on B^- as seen in (34.13.1) or

$$[\dot{u}_i] = -[u_{i,l}] \dot{\xi}_l \quad (34.44)$$

by dividing (34.13.1) by dt . The first and third terms on the right-hand side in (34.43) cancel each other. Furthermore, the second and fourth terms also cancel each other if V is sufficiently small, since from (34.13)

$$\begin{aligned} \dot{u}_i &\approx -u_{i,l} \dot{\xi}_l, \\ \ddot{u}_i &\approx -\dot{u}_{i,l} \dot{\xi}_l. \end{aligned} \quad (34.45)$$

Therefore, (34.43) becomes

$$J_l \dot{\xi}_l = - \int_B \sigma_{ij} n_j [\dot{u}_i] \, dS. \quad (34.46)$$

Then, the flux of energy release (34.37) can be written as

$$\mathcal{G}_i \dot{\xi}_i = \int_D \sigma_{ij} \epsilon_{ij}^p \, dD + J_i \dot{\xi}_i. \quad (34.47)$$

For the elastic crack of Mode I, we have

$$\mathcal{G}_1 = J_1 = \frac{1 - \nu^2}{E} A(v) k_1^2, \quad (34.47.1)$$

where E and ν are Young's modulus and Poisson's ratio, respectively, and A is a geometry-independent function of the crack speed given by

$$A(v) = \frac{(v/c_T)^2 (1 - v^2/c_L^2)^{1/2}}{(1 - \nu) [4(1 - v^2/c_L^2)^{1/2} (1 - v^2/c_T^2)^{1/2} - (2 - v^2/c_T^2)^2]}. \quad (34.47.2)$$

Here c_L and c_T are the longitudinal and shear wave speeds, respectively. The function $A(v)$ is unity at zero crack-tip speed, and it increases monotonically to become unbounded as $v \rightarrow c_R$, where c_R is the speed of Rayleigh waves in the material. As $v \rightarrow c_R$ the elastodynamic stress intensity factor vanishes, and we find $\mathcal{G}_1 \rightarrow 0$ as $v \rightarrow c_R$.

Contrary to the static case, the J -integral (34.41) is path-dependent. However, it can be path-independent if Γ is chosen as a small loop around the crack tip, or a rectangle made up of segments $|x_1 - X(t - t_0)| = \epsilon$ and $|x_2| = \delta$. An alternative expression for the energy flux,

$$J_i \dot{\xi}_i = \int_{\Gamma} (W + K) n_i \dot{\xi}_i \, dS + \int_{\Gamma} \sigma_{ij} n_j \dot{u}_i \, dS, \quad (34.48)$$

can be derived directly from (34.41) and (34.45). The above expression has been obtained by Atkinson and Eshelby (1968) and Freund (1972).

Several solutions of the equations of linear elasticity have been found for moving cracks. Yoffé (1951) has treated a crack of constant length being opened at one end and closed at the other end with a constant speed. Craggs (1960) has considered a semi-infinite crack extending at constant speed. Broberg (1960, 1967) has treated crack growth, starting from zero crack length. Baker (1962), on the other hand, considered a semi-infinite crack which suddenly appears in an elastic solid and then extends at constant speed under the action of a spatially uniform and time-independent normal pressure on the crack surfaces. In contrast to the work of Broberg and Baker, Freund (1972) has treated the extension of an existing equilibrium crack in a body subjected to general time-independent loads.

When a Dugdale crack with a constant length is propagating with a

constant speed v , the crack-tip opening displacement has been calculated by Kanninen (1968) and Kanninen et al. (1968). Their result can be written as

$$\delta(v) = A(v)\delta(0), \quad (34.49)$$

where $A(v)$ is the function given by (34.47.2) and $\delta(0)$ denotes the value of the crack opening displacement in the static case given by (32.20).

A more realistic solution has been obtained by Atkinson (1967) by considering a crack expanding at a uniform speed with collinear strip yield zones.

The order of stress singularity at the elastic crack is 0.5, as seen in (30.10). The order of stress singularity decreases when plastic deformation is allowed at the crack tip. It decreases with the decreasing of the work-hardening rate, as shown by Hutchinson (1968) and Rice and Rosengren (1968). It becomes zero for the perfectly plastic solid. Recently, Achenbach and Kanninen (1978) showed that, for a moving elasto-plastic crack, the order of the singularity decreases with increasing crack velocity.

Recent developments in experimentation, numerical methods, and elasto-plastic dynamic fracture are reviewed in a special issue of *Int. J. Fracture* edited by M.L. Williams and W.G. Knauss (1985).

Dislocations

Certain fundamental formulae for the elastic field caused by a dislocation loop have been presented in Section 7. In this chapter, the integrations appearing in these formulae are completed, and applications to the continuum plasticity are discussed.

An extensive literature is available on solutions of elastic fields caused by dislocations in various states of motion in bodies with various materials and geometries. The books by Hirth and Lothe (1982), Steeds (1973), and Lardner (1974) contain explicit expressions for the elastic fields. On the physics of dislocations, the following books are mentioned: Read (1953), Cottrell (1953), Weertman and Weertman (1964), Hull (1965), Friedel (1964), Suzuki (1967), and Nabarro (1967). Reviews and proceedings on dislocation theory include those by Seeger (1955), deWit (1960), Mura (1968), Kröner (1968), Simmons, deWit and Bullough (1970), Bacon, Barnett and Scattergood (1978), and Nabarro (1979). Continuum theories of continuous distribution of dislocations and plasticity are found in Kondo (1955), Bilby (1960), Kröner (1958), and Kunin (1975).

35. Displacement fields

The displacement field of a dislocation loop L is expressed by (7.6),

$$u_i(x) = \int_S C_{jlmn} b_m G_{ij,l}(x - x') n_n dS(x') \quad (35.1)$$

or, from (3.19),

$$\begin{aligned} u_i(x) &= i(2\pi)^{-3} \int_{-\infty}^{\infty} \int_S C_{jlmn} b_m \xi_l N_{ij}(\xi) D^{-1}(\xi) \\ &\quad \times \exp\{i\xi \cdot (x - x')\} d\xi n_n dS(x'), \end{aligned} \quad (35.2)$$

where S is the slip plane, L is the boundary of S , and n is the unit vector normal to S .

Parallel dislocations

Suppose that the slip plane $S(x)$ is defined by

$$-\infty < x < \infty, \quad -R < y < 0, \quad z = 0. \quad (35.3)$$

The plane S is a strip shown in Fig. 35.1. It should be noted that the parallel dislocations (bounding S) are not necessarily parallel to the crystalline directions x_1, x_2, x_3 . Vectors ξ and x have the respective components (ξ_1, ξ_2, ξ_3) and (x_1, x_2, x_3) , in the x_i -coordinate system, but have the components (ξ, η, ζ) and (x, y, z) in the x, y, z -coordinate system that is,

$$\begin{aligned} \xi \cdot (x - x') &= \xi_1(x_1 - x'_1) + \xi_2(x_2 - x'_2) + \xi_3(x_3 - x'_3) \\ &= \xi(x - x') + \eta(y - y') + \zeta(z - z'). \end{aligned} \quad (35.4)$$

The unit vectors in the x, y, z -directions are denoted by v, m and n , respectively.

The following calculations have been performed by Willis (1970). Equation (35.2) becomes

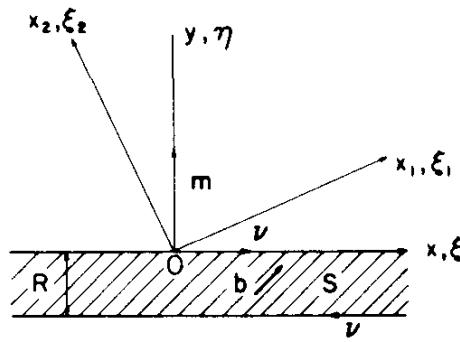
$$\begin{aligned} u_i(x) &= i(2\pi)^{-3} C_{jlmn} b_m n_n \int_{-R}^0 dy' \int_{-\infty}^{\infty} dx' \iiint_{-\infty}^{\infty} \frac{\xi_l N_{ij}(\xi)}{D(\xi)} \\ &\quad \times \exp[i\{\xi(x - x') + \eta(y - y') + \zeta z\}] d\xi d\eta d\zeta. \end{aligned} \quad (35.5)$$

Integration with respect to x' produces a factor $2\pi\delta(\xi)$, so that after integration with respect to ξ , (35.5) becomes

$$\begin{aligned} u_i(x) &= i(2\pi)^{-2} C_{jlmn} b_m n_n \int_{-R}^0 dy' \iint_{-\infty}^{\infty} \frac{\xi_l N_{ij}(\xi)}{D(\xi)} \\ &\quad \times \exp[i\{\eta(y - y') + \zeta z\}] d\eta d\zeta. \end{aligned} \quad (35.6)$$

Next, we integrate with respect to ζ , regarding it as a complex variable. If $z > 0$, we close the contour in the upper half of the complex ζ -plane (the lower half-plane for $z < 0$) and then use Cauchy's theorem to obtain

$$\begin{aligned} u_i(x) &= -(2\pi)^{-1} C_{jlmn} b_m n_n \sum_{N=1}^3 \int_{-R}^0 dy' \int_{-\infty}^{\infty} \frac{\xi_l^N N_{ij}(\xi^N)}{n_k \frac{\partial D}{\partial \xi_k}(\xi^N)} \\ &\quad \times \exp[i\{\eta(y - y') + \zeta^N z\}] d\eta, \end{aligned} \quad (35.7)$$

Fig. 35.1 Parallel dislocations bounding the slip plane S

where

$$\xi^N = \mathbf{m}\eta + \mathbf{n}\zeta^N(\eta), \quad (\xi_l^N = m_l\eta + n_l\zeta^N(\eta)), \quad (35.8)$$

and ζ^N are the three roots, with positive imaginary parts, of

$$D(\mathbf{m}\eta + \mathbf{n}\zeta) = 0. \quad (35.9)$$

v , m , and n are orthogonal unit vectors shown in Fig. 35.1. Since $\xi_l N_{ij}(\xi)/n_k \partial D/\partial \xi_k$ is a homogeneous algebraic equation of degree zero, we divide the numerator and the denominator by η^5 . Then (35.7) becomes, by dividing the domain of integration,

$$\begin{aligned}
 u_i(\mathbf{x}) = & - (2\pi)^{-1} C_{jlmn} b_m n_n \sum_{N=1}^3 \left[\frac{(m_l + n_l \bar{\zeta}^N) N_{ij}(\mathbf{m} + \mathbf{n} \zeta^N)}{n_k \frac{\partial D}{\partial \xi_k}(\mathbf{m} + \mathbf{n} \zeta^N)} \right. \\
 & \times \int_{-R}^0 dy' \int_0^\infty \exp\{i\eta(y - y' + \zeta^N z)\} d\eta \\
 & + \frac{(m_l + n_l \bar{\zeta}^N) N_{ij}(\mathbf{m} + \mathbf{n} \bar{\zeta}^N)}{n_k \frac{\partial D}{\partial \xi_k}(\mathbf{m} + \mathbf{n} \bar{\zeta}^N)} \\
 & \left. \times \int_{-R}^0 dy' \int_{-\infty}^0 \exp\{i\eta(y - y' + \bar{\zeta}^N z)\} d\eta \right], \quad (35.10)
 \end{aligned}$$

where ξ^N are the three roots of

$$D(\mathbf{m} + \mathbf{n}\xi^N) = 0 \quad (35.11)$$

with positive imaginary parts and $\bar{\xi}^N$ denotes the complex conjugate of ξ^N . The integrals can be easily evaluated, since ξ^N and $\bar{\xi}^N$ are independent of η , and the domain of integrals is divided to have convergency. The result is

$$u_i(x) = \frac{i}{2\pi} C_{jlmn} b_m n_n \sum_{N=1}^3 \left[\frac{(m_l + n_l \xi^N) N_{ij} (\mathbf{m} + \mathbf{n}\xi^N)}{n_k \frac{\partial D}{\partial \xi_k} (\mathbf{m} + \mathbf{n}\xi^N)} \log \frac{y + \xi^N z}{y + \xi^N z + R} \right. \\ \left. - \frac{(m_l + n_l \bar{\xi}^N) N_{ij} (\mathbf{m} + \mathbf{n}\bar{\xi}^N)}{n_k \frac{\partial D}{\partial \xi_k} (\mathbf{m} + \mathbf{n}\bar{\xi}^N)} \log \frac{y + \bar{\xi}^N z}{y + \bar{\xi}^N z + R} \right]. \quad (35.12)$$

A straight dislocation

The solution for a single dislocation along the x -axis is obtained by $R \rightarrow \infty$. The terms of $\log R$ are neglected, assuming that they are infinitely large constants. Then, we have

$$u_i(x) = \frac{i}{2\pi} C_{jlmn} b_m n_n \sum_{N=1}^3 \left[\frac{(m_l + n_l \xi^N) N_{ij} (\mathbf{m} + \mathbf{n}\xi^N)}{n_k \frac{\partial D}{\partial \xi_k} (\mathbf{m} + \mathbf{n}\xi^N)} \log(y + \xi^N z) \right. \\ \left. - \frac{(m_l + n_l \bar{\xi}^N) N_{ij} (\mathbf{m} + \mathbf{n}\bar{\xi}^N)}{n_k \frac{\partial D}{\partial \xi_k} (\mathbf{m} + \mathbf{n}\bar{\xi}^N)} \log(y + \bar{\xi}^N z) \right]. \quad (35.13)$$

36. Stress fields

The elastic distortion of a dislocation loop L is expressed by (7.16),

$$\beta_{ji}(x) = i(2\pi)^{-3} \int_L dx'_h \int_{-\infty}^{\infty} \epsilon_{jnh} C_{klmn} \xi_l N_{ik}(\xi) D^{-1}(\xi) b_m \\ \times \exp\{i\xi \cdot (x - x')\} d\xi. \quad (36.1)$$

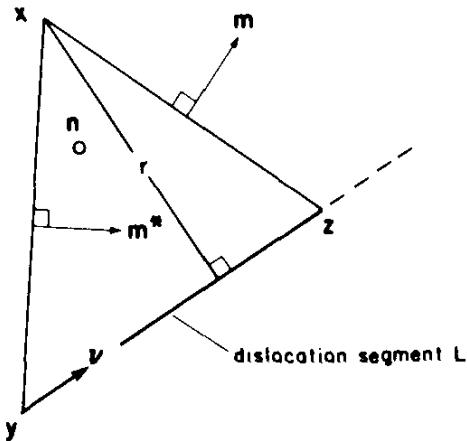


Fig. 36.1. Dislocation segment yz . Unit vector n is normal to the plane xyz

The stress field can be obtained from

$$\sigma_{ij} = C_{ijkl} \beta_{lk}. \quad (36.2)$$

Dislocation segments

Since an arbitrary dislocation loop can be approximated by the sum of a finite number of dislocation segments, the solution for a dislocation segment is fundamental for any shape of dislocation loop. The following calculation is based upon the work of Willis (1970), and Mura and Mori (1976).

Let us consider a dislocation segment yz shown in Fig. 36.1. An arbitrary point x' on the segment is expressed by

$$x' = y + l(z - y), \quad 0 \leq l \leq 1, \quad (36.3)$$

$$dx'_h = (z_h - y_h) dl,$$

and the volume element in the ξ -space is given by

$$d\xi = \xi^2 d\xi dS(\bar{\xi}), \quad (36.4)$$

where $dS(\bar{\xi})$ is the surface element of the unit sphere S^2 in the ξ -space and $\bar{\xi} = \xi/\xi$ and $\xi = |\xi|$. Willis has integrated (36.1) first with respect to l and then with respect to ξ , obtaining

$$\beta_{ji}(x) = \frac{-i}{(2\pi)^3} \epsilon_{jnh} C_{klmn} b_m (z_h - y_h) \int_{S^2} \frac{\bar{\xi}_l N_{ik}(\bar{\xi}) dS(\bar{\xi})}{D(\bar{\xi}) \{ \bar{\xi} \cdot (y - x) \} \{ \bar{\xi} \cdot (z - x) \}}. \quad (36.5)$$

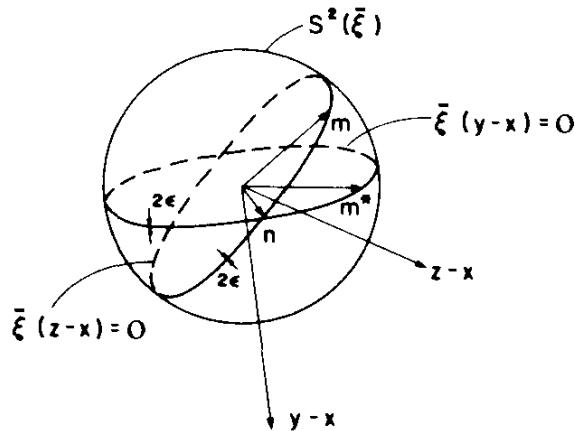


Fig. 36.2. Integral domain of the integral in (36.5).

In order to have a convergent integral, the exponential term in (36.1) has been written as $\exp[\xi\{-\epsilon + i\xi \cdot (x - x')\}]$, where $\epsilon > 0$. The limit $\epsilon \rightarrow 0$ has been taken after the integration.

$N_{ik}(\xi)$ and $D(\xi)$ are homogeneous polynomials of degrees 4 and 6 respectively, and the integrand in (36.5) is an odd function of ξ . Thus, the integral on the unit sphere S^2 vanishes almost everywhere except on the two unit circles satisfying $\xi \cdot (y - x) = 0$ and $\xi \cdot (z - x) = 0$; see Fig. 36.2. It is sufficient to reduce the integral domain in (36.5) into the narrow strips of width 2ϵ along these two unit circles.

Let us introduce three unit vectors n , m^* , and m , as seen in Fig. 36.1. n is normal to the plane containing points x , y , z ; m^* is orthogonal to n and $y - x$; and m is orthogonal to n and $z - x$. The vector products $m^* \times n$ and $m \times n$ are directed parallel to the $y - x$ and $z - x$ directions, respectively.

The principal part of the integrand in (36.5) can be written as

$$\begin{aligned} \frac{1}{\{\xi \cdot (y - x)\}\{\xi \cdot (z - x)\}} &= \frac{1}{\{\xi \cdot (z - y)\}\{\xi \cdot (y - x)\}} \\ &- \frac{1}{\{\xi \cdot (z - y)\}\{\xi \cdot (z - x)\}}. \end{aligned} \quad (36.6)$$

When the integral on the narrow strip along the circle $\xi \cdot (y - x) = 0$ is considered, the first term on the right-hand side in (36.6) is used; on the other hand, when the integral on the narrow strip along the circle $\xi \cdot (z - x) = 0$ is considered, the second term on the right-hand side in (36.6) is used. The plane of circle $\xi \cdot (y - x) = 0$ is perpendicular to $(y - x)$ and, therefore, contains

vectors \mathbf{n} and \mathbf{m}^* . The plane of circle $\bar{\xi} \cdot (z - x) = 0$ contains vectors \mathbf{n} and \mathbf{m} . On the narrow strip along the circle $\bar{\xi} \cdot (y - x) = 0$,

$$\bar{\xi} = -\mathbf{m}^* \sin \phi + \mathbf{n} \cos \phi + h(y - x)/|y - x|, \quad |h| \leq \epsilon \ll 1, \quad (36.7)$$

with parameters ϕ and h , where h is infinitesimally small. Then the integral in (36.5) defined on this narrow strip becomes

$$\begin{aligned} \int_{S^2} &= - \int_0^{2\pi} d\phi \\ &\times \int_{-\epsilon}^{\epsilon} dh \frac{(-m_i^* \sin \phi + n_i \cos \phi) N_{ik} (-\mathbf{m}^* \sin \phi + \mathbf{n} \cos \phi)}{D(-\mathbf{m}^* \sin \phi + \mathbf{n} \cos \phi) \sin \phi \{ \mathbf{m}^* \cdot (z - y) \} h |y - x|}, \end{aligned} \quad (36.8)$$

where

$$\begin{aligned} \bar{\xi} \cdot (z - y) &= -\sin \phi \{ \mathbf{m}^* \cdot (z - y) \}, \\ \bar{\xi} \cdot (y - x) &= h |y - x|, \end{aligned} \quad (36.9)$$

are used. Furthermore, we have

$$\begin{aligned} \{ \mathbf{m}^* \cdot (z - y) \} |y - x| &= r |z - y| = 2 \times \text{area of triangle } xyz, \\ (z_h - y_h)/|z - y| &= v_h, \end{aligned} \quad (36.10)$$

from Fig. 36.1, and

$$\int_{-\epsilon}^{\epsilon} \frac{dh}{h} = i\pi, \quad (36.10.1)$$

where r is the distance from x to the dislocation line, and Cauchy's principal value is taken for the last integral with respect to h .

A similar calculation is performed for the integral on the narrow strip along the circle $\bar{\xi} \cdot (z - x) = 0$ by putting

$$\bar{\xi} = -\mathbf{m} \sin \phi + \mathbf{n} \cos \phi + h(z - x)/|z - x|, \quad |h| \leq \epsilon \ll 1. \quad (36.11)$$

Then, (36.5) becomes

$$\begin{aligned} \beta_{ji}(x) = & \frac{1}{(2\pi)^2} \epsilon_{jnh} C_{klmn} b_m v_h r^{-1} \\ & \times \left[-\frac{1}{2} \int_0^{2\pi} \frac{(-m_l^* \sin \phi + n_l \cos \phi) N_{ik}(-m^* \sin \phi + n \cos \phi)}{\sin \phi D(-m^* \sin \phi + n \cos \phi)} d\phi \right. \\ & \left. + \frac{1}{2} \int_0^{2\pi} \frac{(-m_l \sin \phi + n_l \cos \phi) N_{ik}(-m \sin \phi + n \cos \phi)}{\sin \phi D(-m \sin \phi + n \cos \phi)} d\phi \right] \end{aligned} \quad (36.12)$$

or

$$\begin{aligned} \beta_{ji}(x) = & \frac{1}{(2\pi)^2} \epsilon_{jnh} C_{klmn} b_m v_h r^{-1} \\ & \times \left(\frac{1}{2} \int_0^{2\pi} \left[\frac{m_l^* N_{ik}(-m^* \sin \phi + n \cos \phi)}{D(-m^* \sin \phi + n \cos \phi)} \right. \right. \\ & \left. \left. - \frac{\cos \phi}{\sin \phi} n_l \left\{ \frac{N_{ik}(-m^* \sin \phi + n \cos \phi)}{D(-m^* \sin \phi + n \cos \phi)} - \frac{N_{ik}(n)}{D(n)} \right\} \right] d\phi \right. \\ & \left. - \frac{1}{2} \int_0^{2\pi} \left[\frac{m_l N_{ik}(-m \sin \phi + n \cos \phi)}{D(-m \sin \phi + n \cos \phi)} \right. \right. \\ & \left. \left. - \frac{\cos \phi}{\sin \phi} n_l \left\{ \frac{N_{ik}(-m \sin \phi + n \cos \phi)}{D(-m \sin \phi + n \cos \phi)} - \frac{N_{ik}(n)}{D(n)} \right\} \right] d\phi \right), \end{aligned} \quad (36.13)$$

where $\int_0^{2\pi} (\cos \phi / \sin \phi) n_l (N_{ik}(n) / D(n)) d\phi$ is added to the first integral in (36.12) and subtracted from the second integral in order to eliminate the singularities at $\phi = 0, \pi$, and 2π .

By introducing the definition

$$I_{lik}(\mathbf{m}^*, \mathbf{n}) = \int_0^{2\pi} \left[\frac{m_l^* N_{ik}(-\mathbf{m}^* \sin \phi + \mathbf{n} \cos \phi)}{D(-\mathbf{m}^* \sin \phi + \mathbf{n} \cos \phi)} - \frac{\cos \phi}{\sin \phi} n_i \right. \\ \times \left. \left\{ \frac{N_{ik}(-\mathbf{m}^* \sin \phi + \mathbf{n} \cos \phi)}{D(-\mathbf{m}^* \sin \phi + \mathbf{n} \cos \phi)} - \frac{N_{ik}(\mathbf{n})}{D(\mathbf{n})} \right\} \right] d\phi, \quad (36.14)$$

(36.13) can be written as

$$\beta_{ji}(x) = \frac{1}{(2\pi)^2} \epsilon_{jnh} C_{lkmn} b_m v_h r^{-1} \left\{ \frac{1}{2} I_{lik}(\mathbf{m}^*, \mathbf{n}) - \frac{1}{2} I_{lik}(\mathbf{m}, \mathbf{n}) \right\}. \quad (36.15)$$

For an infinitely long straight dislocation, $\mathbf{m}^* = -\mathbf{m}$, and \mathbf{m}^* and \mathbf{m} are perpendicular to \mathbf{v} . Then, $I_{lik}(\mathbf{m}^*, \mathbf{n}) = -I_{lik}(\mathbf{m}, \mathbf{n})$ and, therefore,

$$\beta_{ji}(x) = -(2\pi)^{-2} \epsilon_{jnh} C_{klmn} b_m v_h r^{-1} I_{lik}(\mathbf{m}, \mathbf{n}) \equiv v_h r^{-1} \sum_{jih} (\mathbf{m}, \mathbf{n}), \quad (36.16)$$

where $\mathbf{n} = \mathbf{v} \times \mathbf{m}$.

Figure 36.3 shows the atomic arrangement around the center of a straight edge dislocation in BCC Fe. This arrangement of atoms has been determined

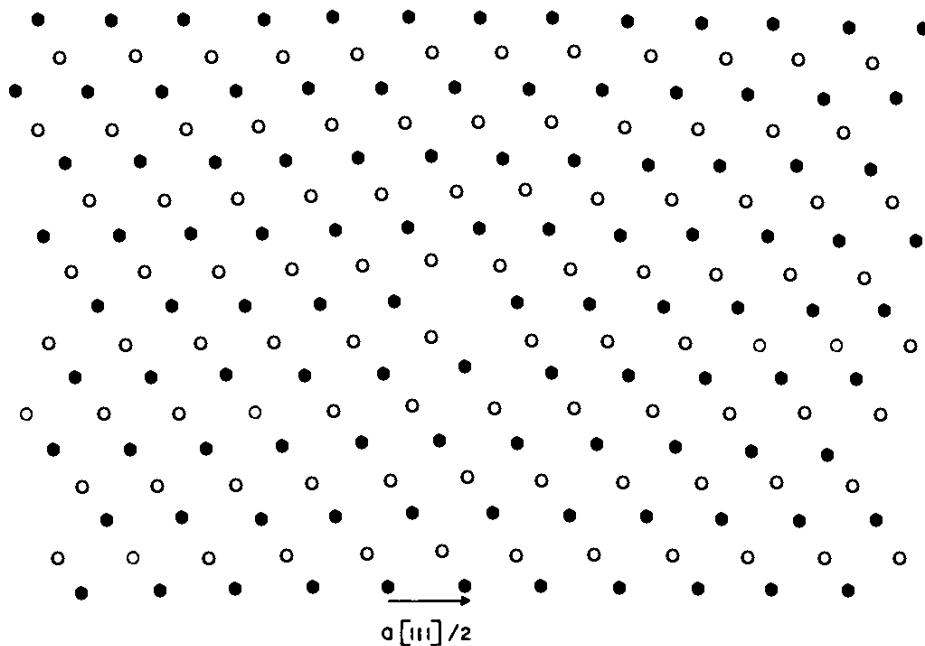


Fig. 36.3. Atomic arrangement around the center of an edge dislocation in BCC Fe.

from (36.16) with (36.14) or (36.24) when $\nu = [\bar{1}10]/\sqrt{2}$, $\mathbf{b} = \mathbf{a}[111]/2$, where a is the lattice constant of BCC Fe. Looking through the BCC structure along the $[\bar{1}10]$ direction, we notice two types of $(1\bar{1}0)$ planes, alternatingly stacked perpendicular to this direction. The atoms on these planes are denoted by the filled and open circles. In this calculation, the elastic moduli are taken as $C_{1111} = 2.37 \times 10^{11} \text{ N/m}^2$, $C_{1122} = 1.41 \times 10^{11} \text{ N/m}^2$, and $C_{1212} = 1.16 \times 10^{11} \text{ N/m}^2$.

Willis' formula

There are several alternative expressions of (36.15) which lead to Willis' result (1970) and the result of Asaro et al. (1973). Willis obtained the solution in terms of residues of a complex plane. Employing the transformation

$$p = -\cos \phi / \sin \phi, \quad dp = d\phi / \sin^2 \phi, \quad (36.17)$$

neglecting the terms which have been added and subtracted artificially, and considering the fact that $N_{ik}(\xi)/D(\xi)$ is a homogeneous function of ξ of degree -2 , we write (36.14) as

$$I_{lik}(\mathbf{m}^*, \mathbf{n}) = 2 \int_{-\infty}^{\infty} \frac{(m_l^* + pn_l) N_{ik}(\mathbf{m}^* + p\mathbf{n})}{D(\mathbf{m}^* + p\mathbf{n})} \, dp. \quad (36.18)$$

$I_{lik}(\mathbf{m}, \mathbf{n})$ is also transformed to a similar form. The integral in (36.18) can be expressed in terms of residues in the complex p -plane by Cauchy's theorem. Then, (36.15) becomes

$$\begin{aligned} \beta_{ji}(x) &= i(2\pi)^{-1} \epsilon_{jnh} b_m C_{klmn} v_h r^{-1} \\ &\times \sum_{N=1}^3 \left[\frac{(m_l^* + n_l \xi^{*N}) N_{ik}(\mathbf{m}^* + \mathbf{n} \xi^{*N})}{n_s \frac{\partial D}{\partial \xi_s}(\mathbf{m}^* + \mathbf{n} \xi^{*N})} - \frac{(m_l + n_l \xi^N) N_{ik}(\mathbf{m} + \mathbf{n} \xi^N)}{n_s \frac{\partial D}{\partial \xi_s}(\mathbf{m} + \mathbf{n} \xi^N)} \right] \end{aligned} \quad (36.19)$$

where ξ^N and ξ^{*N} are complex numbers with positive imaginary parts and are the roots of the following equations:

$$\begin{aligned} D(\mathbf{m} + \mathbf{n} \xi^N) &= 0, \\ D(\mathbf{m}^* + \mathbf{n} \xi^{*N}) &= 0. \end{aligned} \quad (36.20)$$

Since $D(\mathbf{m} + \mathbf{n}\zeta^N)$ is the sextic equation in ζ^N , considerable computation is required to find the roots of (36.20). Expression (36.13) is usually more convenient for numerical calculation than (36.19).

The Asaro et al. formula

The result of Asaro, Hirth, Barnett, and Lothe (1973) can also be obtained from (36.15). The following notation is introduced:

$$\begin{aligned} \mathbf{n}' &= -\mathbf{m}^* \sin \phi + \mathbf{n} \cos \phi, \\ \mathbf{m}' &= \mathbf{m}^* \cos \phi + \mathbf{n} \sin \phi, \\ (\mathbf{m}', \mathbf{n}')_{st} &= C_{sp tq} m'_p n'_q, \\ (\mathbf{n}', \mathbf{n}')_{ik}^{-1} &= N_{ik}(\mathbf{n}') / D(\mathbf{n}'). \end{aligned} \quad (36.21)$$

Then, we have

$$\begin{aligned} (\mathbf{n}', \mathbf{n}')(\mathbf{n}', \mathbf{n}')^{-1} &= I, \\ (\mathbf{n}', \mathbf{n}') &= \cos^2 \phi (\mathbf{n}, \mathbf{n}) - \cos \phi \sin \phi \{(\mathbf{m}^*, \mathbf{n}) + (\mathbf{n}, \mathbf{m}^*)\} \\ &\quad + \sin^2 \phi (\mathbf{m}^*, \mathbf{m}^*), \end{aligned} \quad (36.22)$$

$$\begin{aligned} (\mathbf{m}', \mathbf{n}') &= \cos^2 \phi \{(\mathbf{m}^*, \mathbf{n}) + (\mathbf{n}, \mathbf{m}^*)\} \\ &\quad - \cos \phi \sin \phi \{(\mathbf{m}^*, \mathbf{m}^*) - (\mathbf{n}, \mathbf{n})\} - (\mathbf{n}, \mathbf{m}^*), \end{aligned}$$

where I is the unit matrix. By substituting the second equation into the first and multiplying by $(\mathbf{n}, \mathbf{n})^{-1} \cos \phi / \sin \phi$, we have

$$\begin{aligned} \{I \cos \phi / \sin \phi - (\mathbf{n}, \mathbf{n})^{-1} (\mathbf{n}, \mathbf{m}^*) - (\mathbf{n}, \mathbf{n})^{-1} (\mathbf{m}', \mathbf{n}')\} (\mathbf{n}', \mathbf{n}')^{-1} \\ = (\mathbf{n}, \mathbf{n})^{-1} \cos \phi / \sin \phi, \end{aligned} \quad (36.23)$$

where the third equation in (36.22) has been used. The second integrand in (36.14) can be written as the ik component of $n_l \{(\mathbf{n}', \mathbf{n}')^{-1} - (\mathbf{n}, \mathbf{n})^{-1}\} \cos \phi / \sin \phi$, which becomes from (36.23) $n_l \{(\mathbf{n}, \mathbf{n})^{-1} (\mathbf{n}, \mathbf{m}^*) + (\mathbf{n}, \mathbf{n})^{-1} (\mathbf{m}^*, \mathbf{n}')\} (\mathbf{n}', \mathbf{n}')^{-1}$. Therefore, we have

$$\begin{aligned} I_{lik}(\mathbf{m}^*, \mathbf{n}) &= \int_0^{2\pi} \left[m_l^* (\mathbf{n}', \mathbf{n}')_{ik}^{-1} \right. \\ &\quad \left. - n_l (\mathbf{n}, \mathbf{n})_{is}^{-1} \{(\mathbf{n}, \mathbf{m}^*)_{st} + (\mathbf{m}', \mathbf{n}')_{st}\} (\mathbf{n}', \mathbf{n}')_{ik}^{-1} \right] d\phi. \end{aligned} \quad (36.24)$$

The expression (36.15) with (36.24) is the result obtained by Asaro et al (1973). These expressions can be represented in alternative ways, as given by Barnett (1972), Barnett and Swanger (1971), Barnett et al. (1972), Malén (1970, 1971), and Barnett and Lothe (1973). These results illustrate further development of the works of Eshelby, Read and Shockley (1953), Stroh (1958, 1962), and Bullough and Bilby (1954). On the other hand, Lothe (1967), Brown (1967), and Indenbom and Orlov (1968) have shown how the in-plane strain field of the general planar dislocation loop is determined directly from the strain field of a straight dislocation and the derivatives of these strains with respect to variables describing the direction of the line in the plane as shown by (7.17). The details of the stress analysis of dislocations, along with the work of Barnett et al. and Lothe and Brown, can be found in the review article by Bacon, Barnett and Scattergood (1978). The stress analysis of an infinite straight dislocation based upon the work of Eshelby et al. and Stroh can be found in the book by Steeds (1973).

Dislocation loops

The elastic distortion of a dislocation loop L is calculated by the integration of (36.15) along the dislocation line (see Fig. 36.4),

$$\beta_{ji}(x) = \frac{-1}{8\pi^2} \epsilon_{jnh} C_{klmn} b_m \oint_L \frac{\nu_h}{r} \frac{d}{dl} I_{lik}(\mathbf{m}, \mathbf{n}) dl, \quad (36.25)$$

where the difference of I_{lik} at two neighboring points in (36.15) has been expressed by the differential. The unit vectors \mathbf{m} , \mathbf{n} , and ν are shown in Fig. 36.4. Vector \mathbf{n} is normal to the plane containing dl and x , and vector \mathbf{m} is lying on the plane.

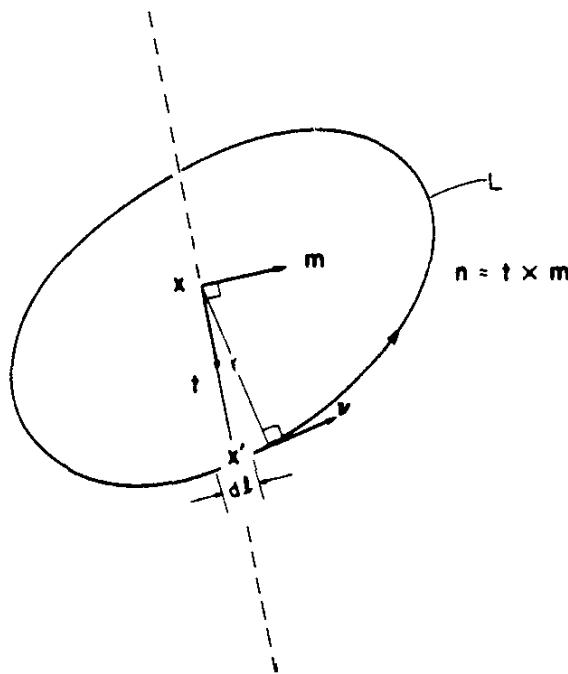
If the dislocation loop is a smooth curve, (36.25) is written as

$$\beta_{ji}(x) = \frac{1}{8\pi^2} \epsilon_{jnh} C_{klmn} b_m \oint_L \left(\frac{\nu_h}{r} \right) I_{lik}(\mathbf{m}, \mathbf{n}) dl \quad (36.26)$$

after integration by parts, or

$$\beta_{ji}(x) = -\frac{1}{2} \oint_L \frac{d}{dl} \left(\frac{\nu_h}{r} \right) \sum_{jih} (\mathbf{m}, \mathbf{n}) dl \quad (36.27)$$

with the use of the notation in (36.16). If a vector t is introduced along the line

Fig. 36.4. Dislocation loop L .

xx' , $t_h \Sigma_{jih}(\mathbf{m}, \mathbf{n})$ becomes the ji component of the elastic distortion caused by a fictitious infinite straight dislocation containing points x and x' with direction t . This distortion is measured at the end points of vector \mathbf{m} . The expression (36.27) is more convenient to use than the Indenbom and Orlov expression (7.17) or the Lothe (1967) and Brown (1967) expression, since they contain higher derivatives of the straight dislocation solution.

If the dislocation loop L is a smooth convex planar loop and point x is a field point inside L , then (36.27) can be reduced to a more convenient form. Let us denote by α the angle between the tangential direction at a point x' on the dislocation loop and some datum, and by θ the angle between $x - x'$ and the same datum (Fig. 36.5). Then, we have

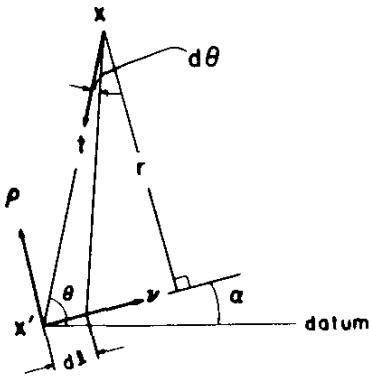
$$dl \sin(\theta - \alpha) = |x - x'| d\theta,$$

$$dl \cos(\theta - \alpha) = -d |x - x'|,$$

$$r = |x - x'| \sin(\theta - \alpha), \quad (36.28)$$

$$d\alpha = \kappa dl,$$

$$d\nu_h = \rho_h d\alpha,$$

Fig. 36.5. Geometry at dislocation line element dl .

where κ is the curvature of L at x' and ρ is the unit vector directed toward the center of curvature. Elementary calculus yields

$$\begin{aligned} \frac{d}{d\theta} \left(\frac{\nu_h}{r} \right) &= \frac{\kappa}{\sin^3(\theta - \alpha)} \{ \rho_n \sin(\theta - \alpha) + \nu_h \cos(\theta - \alpha) \} \\ &= -\kappa t_h \csc^3(\theta - \alpha), \end{aligned} \quad (36.29)$$

where (36.28) is used. Expression (36.27) is written, therefore, as

$$\beta_{ji}(x) = \frac{1}{2} \oint_L \kappa \sum_{ji}(\theta) \csc^3(\theta - \alpha) d\theta, \quad (36.30)$$

where

$$\sum_{ji}(\theta) = t_h \sum_{jih}(\mathbf{m}, \mathbf{n}) \quad (36.31)$$

is the ji component of the elastic distortion at the unit distance caused by the fictitious infinite straight dislocation containing x and x' . The last result (36.30) has been obtained by Asaro and Barnett (1976). When point x is located outside L , we can have a result similar to (36.30).

For a planar loop, the Indenbom and Orlov expression (7.17) reduces to Brown's formula (1967). Simple calculus gives for two dimensions (see Fig. 36.5),

$$\nu_p \frac{\partial}{\partial x_p} = \cos(\theta - \alpha) \frac{\partial}{\partial |x - x'|} - \frac{\sin(\theta - \alpha)}{|x - x'|} \frac{\partial}{\partial \theta}. \quad (36.32)$$

The elastic distortion of an infinite and straight dislocation is inversely proportional to the distance from the dislocation line to the field point. Therefore, (7.18) is written as

$$\beta_{ji}(\mathbf{v}; \mathbf{t}) = \left\{ 1/\sin(\theta - \alpha) \right\} \sum_{ji}(\theta). \quad (36.33)$$

Applying the operator of (36.32) twice to (36.33), we transform (7.17) to

$$\begin{aligned} \beta_{ji}(\mathbf{x}) &= \frac{1}{2} \oint_L \frac{\sin(\theta - \alpha)}{|x - x'|^2} \left(\sum_{ji}(\theta) + \sum''_{ji}(\theta) \right) dl(x') \\ &= \frac{1}{2} \oint_L \frac{1}{|x - x'|} \left(\sum_{ji}(\theta) + \sum''_{ji}(\theta) \right) d\theta. \end{aligned} \quad (36.34)$$

The above expression has been derived by Brown (1967) as a refinement of a similar expression by Lothe (1967).

Complicated configurations of dislocations can often be obtained by superposition of angular dislocations. Yoffé (1960) has calculated the elastic fields for angular dislocations in the unbounded material. Yoffé (1961), Comninou and Dundurs (1975), and Comninou (1977) have obtained similar results for a half-space.

Elastic solutions for a circular dislocation loop were obtained by Keller (1957), and Kroupa (1960) for the unbounded medium and by Salamon and Dundurs (1971) for a two-phase material. Saito (1979) has investigated the effect of a thin-layer on the semi-infinite medium. Elastic behaviors of disclination loops near a free surface and a two-phase material has been investigated by T.W. Chou (1971), Chou and Lu (1972), Chou and Pan (1973) and Chou and Lu (1972). Recently, Sekine and Mura (1979) have obtained the elastic field of a dislocation dipole segment. The elastic field of planar periodic dislocation networks has been obtained by Mura (1964), Owen and Mura (1967), Owen (1971), and Saada (1976). The solution for a helical dislocation has been obtained by Owen and Mura (1967).

37. Dislocation density tensor

The dislocation density tensor of Nye (1953) has already been introduced in (7.33). In this section, the tensor will be introduced by a different method.

First, consider a dislocation loop L which is the boundary of slip plane S , Fig. 7.1. The slip \mathbf{b} on S introduces a plastic distortion β_{ji}^P ,

$$\beta_{ji}^P \, d\mathbf{x} = -b_i n_j \, dS = -b_i \, dS_j, \quad (37.1)$$

where $d\mathbf{x} = dx_1 \, dx_2 \, dx_3$, dS is the surface element of S , and \mathbf{n} is the unit normal vector of S . Kroupa (1962) has called β_{ji}^P the dislocation loop density tensor. The Fourier integral representation of β_{ji}^P is

$$\beta_{ji}^P(\mathbf{x}) = \int_{-\infty}^{\infty} \bar{\beta}_{ji}^P(\xi) \exp(i\xi \cdot \mathbf{x}) \, d\xi, \quad (37.2)$$

where

$$\bar{\beta}_{ji}^P(\xi) = (2\pi)^{-3} \int_{-\infty}^{\infty} \beta_{ji}^P(\mathbf{x}) \exp(-i\xi \cdot \mathbf{x}) \, d\mathbf{x}. \quad (37.3)$$

Since b_i is constant on S , (37.3) becomes

$$\bar{\beta}_{ji}^P(\xi) = -(2\pi)^{-3} b_i \int_S n_j \exp(-i\xi \cdot \mathbf{x}) \, dS, \quad (37.4)$$

where (37.1) is used.

Multiplying (37.4) by $-i\epsilon_{hlj}\xi_l$ and applying the Stokes theorem, we have

$$-i\epsilon_{hlj}\xi_l \bar{\beta}_{ji}^P(\xi) = (2\pi)^{-3} b_i \int_L \exp(-i\xi \cdot \mathbf{x}) v_h \, dl, \quad (37.5)$$

where dl is the line element of L , and v_h is its unit tangent vector. The equality $-i\xi_l \exp(-i\xi \cdot \mathbf{x}) = (\partial/\partial x_l) \exp(-i\xi \cdot \mathbf{x})$ has been used in the derivation.

The dislocation density tensor is defined by

$$\alpha_{hi} \, d\mathbf{x} = b_i v_h \, dl = b_i \, dl_h. \quad (37.6)$$

Its Fourier integral expression is

$$\alpha_{hi}(\mathbf{x}) = \int_{-\infty}^{\infty} \bar{\alpha}_{hi}(\xi) \exp(i\xi \cdot \mathbf{x}) \, d\xi, \quad (37.7)$$

where

$$\bar{\alpha}_{hi}(\xi) = (2\pi)^{-3} \int_{-\infty}^{\infty} \alpha_{hi}(x) \exp(-i\xi \cdot x) dx \quad (37.8)$$

or

$$\bar{\alpha}_{hi}(\xi) = (2\pi)^{-3} b_i \int_L \exp(-i\xi \cdot x) v_h dl. \quad (37.9)$$

Comparing (37.5) and (37.9), we have

$$\bar{\alpha}_{hi} = -i\epsilon_{hli}\xi_l \bar{\beta}_{ji}^p. \quad (37.10)$$

When this is substituted into (37.7), we obtain (7.33) or

$$\alpha_{hi} = -\epsilon_{hli}\beta_{ji,l}^p, \quad (37.11)$$

which is Kröner's expression (1955, 1956).

Although the result (37.11) has been obtained for a single dislocation loop, it also holds for the case of continuously distributed dislocations, where α_{hi} and β_{ji}^p are spatial functions. In this case we can interpret

$$\alpha_{hi} dx = \sum b_i v_h dl, \quad (37.12)$$

where the summation is taken on all dislocation segments contained in the infinitesimal cube dx .

The dislocation density tensor α_{hi} expresses the x_i -component of the total Burgers vector of dislocations threading the unit surface perpendicular to the x_h -direction. Denoting the Burgers circuit by c (see Fig. 10.2), the total Burgers vector can be expressed as $\int_S \alpha_{hi} v_h dS$ which must be equal to the multiple value of displacement expressed by (10.11). It may also be expressed by $-\int_c \beta_{ji}^p dx_j$ or $-\int_S \epsilon_{hli} \beta_{ji,l}^p v_h dS$ after the use of Stokes' theorem (7.10).

The stress field due to a continuous distribution of dislocations can be obtained from (37.12) and (7.15),

$$\sigma_{ij}(x) = C_{ijkl} \int \epsilon_{lnh} C_{pqmn} G_{kp,q}(x - x') \alpha_{hm}(x') dx'. \quad (37.13)$$

An isolated dislocation line is a special case when the dislocation density tensor takes the form of Dirac's delta function. For instance, the dislocation line shown in Fig. 4.1 is expressed by $\alpha_{33} = b_3 \delta(x_1) \delta(x_2)$, $\beta_{23}^p =$

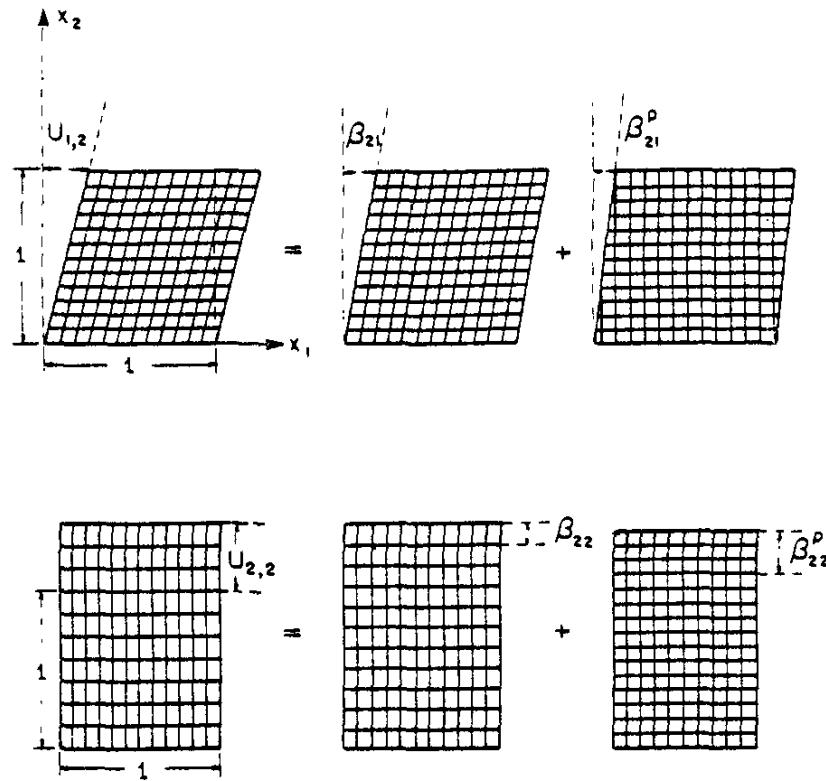


Fig. 37.1 Distortion is the sum of elastic (lattice) and plastic (non-lattice) distortions.

$b_3\delta(x_2)H(-x_1)$, and the dislocation line in Fig. 4.2 by $\alpha_{31} = b_1\delta(x_1)\delta(x_2)$, $\beta_{21}^P = b_1\delta(x_2)H(-x_1)$.

For the continuous distribution of dislocations, it holds that

$$u_{i,j} = \beta_{ji} + \beta_{ji}^P, \quad (37.14)$$

where β_{ji} is the elastic distortion; the total distortion is the sum of the elastic and plastic distortions. The plastic distortion does not produce any distortion among lattice points since it is caused by gliding (slip), while the elastic distortion is originated in an elastic deformation of the lattice as shown in Fig. 37.1.

Surface dislocation density

When β_{ji}^P jumps at a surface Σ , the surface dislocation density tensor is defined by

$$\begin{aligned} \alpha_{hi} &= -\epsilon_{hlj} \{ \beta_{ji}^P(\text{II}) - \beta_{ji}^P(\text{I}) \} n_l \\ &= -\epsilon_{hlj} [\beta_{ji}^P] n_l, \end{aligned} \quad (37.15)$$

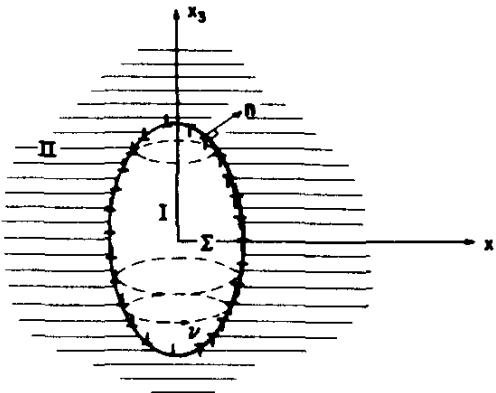


Fig. 37.2. Uniform plastic distortion around an inclusion. Orowan's dislocation loops α_{31} are created by the misfit.

where $\beta_{ji}^P(I)$ and $\beta_{ji}^P(II)$ are values of β_{ji}^P in domains I and II on Σ , respectively, and n is the normal vector on Σ directed toward domain II from domain I. The surface dislocation density tensor has been defined by Bullough and Bilby (1956). The formulation of (37.15) is a natural consequence of (37.11) for discontinuous quantities on an interface.

Figure 37.2 shows a uniform plastic distortion β_{31}^P in the matrix II around an inclusion. Equation (37.15) gives the surface dislocation density tensor,

$$\alpha_{11} = -\beta_{31}^P n_2, \quad \alpha_{21} = \beta_{31}^P n_1, \quad (37.16)$$

which is defined on the surface of the inclusion. These dislocations are Orowan's loops (1948, 1959), discussed in Section 43.

Another interesting example of the surface dislocations can be found in the crack problems discussed in Sections 27 and 32. A slit-like crack with length $2a_1$ along the x_1 -axis subjected to σ_{22}^0 at infinity was simulated by the infinitesimally thin elliptical inclusion ($a_2 \rightarrow 0$),

$$x_1^2/a_1^2 + x_2^2/a_2^2 = 1. \quad (37.17)$$

The equivalent eigenstrain ϵ_{22}^* is given by (27.14) and (27.17). This inclusion can be replaced by the surface dislocations (37.15) where $\beta_{ji}^P(II) = 0$ and $\beta_{ji}^P(I) = \epsilon_{22}^*$. Then, we have

$$\alpha_{32} = \epsilon_{22}^* n_1, \quad (37.18)$$

where n_1 is

$$n_1 = x_1 / (x_1^2 + x_2^2 a_1^4/a_2^4)^{1/2} \quad (37.19)$$

from (37.17). From (27.14) and (27.17),

$$\alpha_{32} = (1 - \nu) \sigma_{22}^0 (a_1/\mu a_2) n_1, \quad (37.20)$$

and, therefore,

$$2\alpha_{32} = 2(1 - \nu) \sigma_{22}^0 (x_1/a_1)/\mu (1 - x_1^2/a_1^2)^{1/2} \quad (37.21)$$

after using (37.17) and letting $a_2 \rightarrow 0$. The above dislocation distribution (37.21) defined in $|x_1| < a_1$ simulates the stress disturbance of the slit-like crack under σ_{22}^0 . The sum of the two surface dislocations defined on the upper and lower surfaces of the infinitesimally thin inclusion gives the factor of 2 in (37.21). The distribution of dislocations (37.21) has been suggested by Eshelby (1957); see also Bilby and Eshelby (1968).

In the above discussion we have derived the equivalent dislocation distribution from the equivalent inclusion for a given crack problem. It is also possible to find an equivalent inclusion from an equivalent dislocation distribution. For the Dugdale crack we have

$$\begin{aligned} 2\alpha_{32} &= \frac{2k_0}{\pi\mu} \log \frac{x_1(a_1^2 - c^2)^{1/2} + c(a_1^2 - x_1^2)^{1/2}}{x_1(a_1^2 - c^2)^{1/2} - c(a_1^2 - x_1^2)^{1/2}} \quad \text{for } c < |x_1| < a_1 \\ &= \frac{2k_0}{\pi\mu} \log \frac{x_1(a_1^2 - c^2)^{1/2} + c(a_1^2 - x_1^2)^{1/2}}{c(a_1^2 - x_1^2)^{1/2} - x_1(a_1^2 - c^2)^{1/2}} \quad \text{for } |x_1| < c \end{aligned} \quad (37.22)$$

from (32.37) in the BCS model. The equivalent eigenstrain ϵ_{22}^* is obtained from (37.18). It is found that $a_2\epsilon_{22}^*$ is zero at $x_1 = \pm a_1$, infinite at $x_1 = \pm c$, and finite at $x_1 = 0$.

Impotent distribution of dislocations

Mura (1968) has pointed out that if

$$\beta_{ji}^P = -\beta_{ij}^P, \quad (37.23)$$

the displacement and stress fields caused by α_{hi} as defined by (37.11) or (37.15) become identically zero. Such a distribution of dislocations is called impotent. It is obvious from (3.23) and (3.25) that $\epsilon_{ij}^* = \epsilon_{ij}^P = \frac{1}{2}(\beta_{ij}^P + \beta_{ji}^P) = 0$ leads to $u_i = \sigma_{ij} = 0$.

As an example, consider the case

$$\beta_{21}^P(x_1) = -\beta_{12}^P(x_1). \quad (37.24)$$

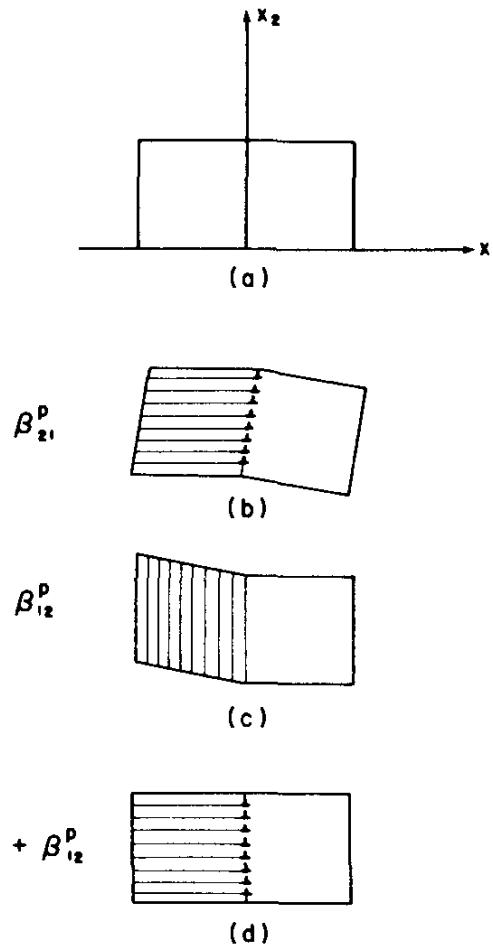


Fig. 37.3. Impotent dislocation distribution.

Equation (37.11) leads to an impotent distribution of dislocations $\alpha_{31}(x_1) = -\beta_{21,1}^P(x_1)$. If β_{21}^P and β_{12}^P are uniform in $x_1 < 0$ and zero in $x_1 > 0$, (37.15) leads to $\alpha_{31} = \beta_{21}^P$ along the surface $x_1 = 0$ as shown in Fig. 37.3(d). Figure 37.3(a) deforms to Fig. 37.3(b) by β_{21}^P , while Fig. 37.3(a) deforms to Fig. 37.3(c) by β_{12}^P . The superposition of the deformations due to β_{21}^P and β_{12}^P becomes Fig. 37.3(d). The elastic energy is zero and the distribution of dislocations in Fig. 37.3(d) results in an impotent distribution of dislocations. The dislocation distribution shown in Fig. 37.3(b) is the polygonization.

Another example of an impotent distribution of dislocations is shown in Fig. 37.4, where

$$\begin{aligned}\beta_{23}^P(x_1) &= -\beta_{32}^P, \\ \alpha_{33}(x_1) &= -\beta_{23,1}^P(x_1), \\ \alpha_{22}(x_1) &= \beta_{32,1}^P(x_1).\end{aligned}\tag{37.25}$$

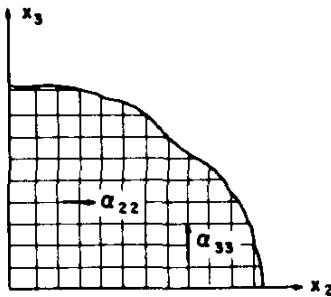


Fig. 37.4. Impotent dislocation network.

Planes perpendicular to the x_1 -axis rotate clockwise by the dislocation array α_{22} , while they rotate counterclockwise by α_{33} . The resulting deformation and stress fields vanish after the superposition of α_{22} and α_{33} . This impotent distribution of dislocations constructs a dislocation network of minimum strain energy.

38. Dislocation flux tensor

Using the analogy of heat conduction, we consider the growth rate of dislocations threading an arbitrary surface S ; see Fig. 38.1. Let the dislocation density tensor change by an amount $\dot{\alpha}_{hi}$ per unit time. The growth rate of the total Burgers vector of these dislocations is $\int_S \dot{\alpha}_{hi} n_h dS$, where n is the normal vector on surface S . The net transport of dislocations moving through the line element dl of the boundary S is $\nu \cdot (dl \times V)$ or $\epsilon_{lhi} V_l \nu_h dl_j$, where V is the dislocation velocity and ν is the dislocation direction. The growth rate of the total Burgers vector on S is equal to the flux of dislocations moving through L , namely,

$$\int_S \dot{\alpha}_{hi} n_h dS = \int_L \epsilon_{lhi} V_l \nu_h dl_j, \quad (38.1)$$

where

$$V_{lhi} = \sum_l V_l \nu_h b_i. \quad (38.2)$$

The Burgers vector for a single dislocation has been denoted by b in (38.2) and the summation applies to all dislocations. Mura (1963) has defined V_{lhi} as the dislocation flux (or velocity) tensor. Applying Stoke's theorem of integration to the right-hand side of (38.1), it follows that

$$\int_S \dot{\alpha}_{hi} dS_h = \int_S \epsilon_{hlj} (\epsilon_{mnj} V_{mni})_{,l} dS_h. \quad (38.3)$$

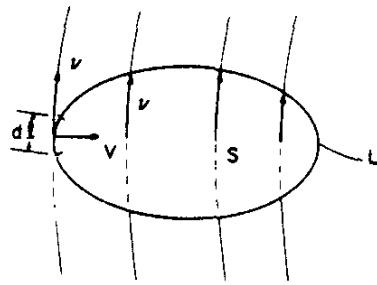


Fig. 38.1. Dislocations threading surface S increases by moving-in dislocations through L

Since this holds for any surface S , we have

$$\dot{\alpha}_{hi} = \epsilon_{hlij} \epsilon_{mni} V_{mni,l}. \quad (38.4)$$

On the other hand, the motion of dislocations causes a plastic distortion rate. For example, $\dot{\beta}_{21}^P$ is caused by the $+x_1$ -direction motion of α_{31} and by the $-x_3$ -direction motion of α_{11} , namely

$$\dot{\beta}_{21}^P = V_{131} - V_{311}. \quad (38.5)$$

More generally,

$$\dot{\beta}_{ji}^P = -\epsilon_{jmn} V_{mni}. \quad (38.6)$$

When it is substituted into (38.4), we have

$$\dot{\alpha}_{hi} = -\epsilon_{hlij} \dot{\beta}_{ji,l}^P \quad (38.7)$$

or

$$\alpha_{hi} = -\epsilon_{hlij} \beta_{ji,l}^P \quad (38.8)$$

by integrating with respect to time. Expression (38.8) is the same as (37.11).

We shall derive (38.6) by a different method, starting from a consideration of a single dislocation loop whose line element is moving with velocity V . The plastic distortion associated with this dislocation loop L can be expressed in the Fourier integral form,

$$\beta_{ji}^P(x, t) = \iint_{-\infty}^{\infty} \bar{\beta}_{ji}^P(\xi, \omega) \exp\{i(\xi \cdot x - \omega t)\} d\xi d\omega, \quad (38.9)$$

where

$$\bar{\beta}_{ji}^P(\xi, \omega) = (2\pi)^{-4} \iint_{-\infty}^{\infty} \beta_{ji}^P(x, t) \exp\{-i(\xi \cdot x - \omega t)\} dx dt. \quad (38.10)$$

Substituting (37.1) into (38.10), we have

$$\bar{\beta}_{ji}^p(\xi, \omega) = -(2\pi)^{-4} b_i \int_{-\infty}^{\infty} dt \int_{S(t)} \exp\{-i(\xi \cdot x - \omega t)\} dS_j(x). \quad (38.11)$$

When (38.11) is multiplied by $-i\omega$, we obtain

$$\begin{aligned} -i\omega \bar{\beta}_{ji}^p(\xi, \omega) &= (2\pi)^{-4} b_i \int_{-\infty}^{\infty} dt \int_{S(t)} \frac{\partial}{\partial t} \exp\{-i(\xi \cdot x - \omega t)\} dS_j(x) \\ &= (2\pi)^{-4} b_i \int_{-\infty}^{\infty} dt \left[\frac{\partial}{\partial t} \int_{S(t)} \exp\{-i(\xi \cdot x - \omega t)\} dS_j(x) \right. \\ &\quad \left. - \int_{\partial S/\partial t} \exp\{-i(\xi \cdot x - \omega t)\} dS_j(x) \right]. \end{aligned} \quad (38.12)$$

The first term in (38.12) vanishes by introducing proper initial conditions such as

$$S(t) = 0 \quad \text{at } t = \pm\infty. \quad (38.13)$$

The increment of the slip surface S swept by dl is determined by the vector product V and dl , that is,

$$\partial S_j / \partial t = \epsilon_{jnh} V_n dl_h. \quad (38.14)$$

Then, (38.12) can be written as

$$-i\omega \bar{\beta}_{ji}^p(\xi, \omega) = -(2\pi)^{-4} \epsilon_{jnh} \int_{-\infty}^{\infty} dt \int_L \exp\{-i(\xi \cdot x - \omega t)\} b_i V_n dl_h \quad (38.15)$$

or, from (37.6), as

$$\begin{aligned} -i\omega \bar{\beta}_{ji}^p(\xi, \omega) &= -(2\pi)^{-4} \epsilon_{jnh} \iint_{-\infty}^{\infty} \exp\{-i(\xi \cdot x - \omega t)\} V_n \alpha_{hi} dx dt. \\ \end{aligned} \quad (38.16)$$

On the other hand, we have from (38.10),

$$-i\omega \bar{\beta}_{ji}^p(\xi, \omega) = (2\pi)^{-4} \iint_{-\infty}^{\infty} \dot{\beta}_{ji}^p(x, t) \exp\{-i(\xi \cdot x - \omega t)\} dx dt. \quad (38.17)$$

Comparing (38.16) and (38.17), it holds that

$$\dot{\beta}_{ji}^p = -\epsilon_{jnh} V_n \alpha_{hi} \quad (38.18)$$

or

$$\dot{\beta}_{ji}^p = -\epsilon_{jnh} V_{nhi}. \quad (38.19)$$

Although all these formulae have been obtained by considering the motion of a single dislocation loop, the result (38.19) also holds for continuous distributions of dislocations by the principle of superposition.

The general theory of moving dislocations has been developed by Kosevich (1962–65), Mura (1963), Bross (1964, 1968), Zorski (1966), Amari (1968), Günther (1967, 1969), Kossecka (1969), Lardner (1969), Malén (1970), and Minagawa (1971). The earliest work on the corresponding field equations has been done by Hällander (1960), where Poisson's ratio has been assumed to be zero for mathematical simplicity.

Line integral expression of displacement and plastic distortion fields

The elastic distortion and stress fields of a dislocation line L (or loop) are expressed by the line integrals along L in (7.9) and (7.15), while the displacement field is expressed by the surface integral on the slip surface S in (7.6). The displacement field can also be expressed by a line integral, if a creation history of the dislocation line is prescribed. First, consider the displacement field (8.9) for a given $\beta_{ji}^p(x, t)$, where $\bar{\epsilon}_{mn}^*(\xi, \omega)$ is replaced by $\bar{\beta}_{nm}^p(\xi, \omega)$,

$$u_i(x, t) = - \iint_{-\infty}^{\infty} i C_{jlmn} \bar{\beta}_{nm}^p(\xi, \omega) \xi_l N_{ij}(\xi, \omega) D^{-1}(\xi, \omega) \\ \times \exp\{i(\xi \cdot x - \omega t)\} d\xi d\omega, \quad (38.20)$$

where

$$\bar{\beta}_{nm}^p(\xi, \omega) = (2\pi)^{-4} \iint_{-\infty}^{\infty} \beta_{nm}^p(x, t) \exp\{-i(\xi \cdot x - \omega t)\} dx dt. \quad (38.21)$$

If $\dot{\beta}_{ji}^p$ is a plastic distortion rate associated with $V_n \alpha_{hi}$ through (38.18), then (38.20) can be written in terms of $V_n \alpha_{hi}$. From (38.17) and (38.18),

$$\bar{\beta}_{nm}^p(\xi, \omega) = (2\pi)^{-4} (1/i\omega) \epsilon_{nkh} \iint_{-\infty}^{\infty} V_k(x, t) \alpha_{hm}(x, t) \\ \times \exp\{-i(\xi \cdot x - \omega t)\} dx dt. \quad (38.22)$$

When it is substituted into (38.20),

$$\begin{aligned} u_i(x, t) = & -(2\pi)^{-4} \epsilon_{nkh} \iiint_{-\infty}^{\infty} C_{jlmn} V_k(x', t') \alpha_{hm}(x', t') \\ & \times \frac{\xi_l}{\omega} \frac{N_{ij}(\xi, \omega)}{D(\xi, \omega)} \exp\{i\xi \cdot (x - x') - i\omega(t - t')\} d\xi d\omega dx' dt'. \end{aligned} \quad (38.23)$$

When a discrete (single) dislocation with Burgers vector b_i is considered, expression (37.6) is used and the integral with respect to x' in (38.23) becomes a line integral along the dislocation line L . The time derivative of (38.23) is

$$\dot{u}_i(x, t) = \epsilon_{nkh} \oint_L b_m dl_h(x') \int_{-\infty}^{\infty} dt' C_{jlmn} V_k(x', t') G_{ij,l}(x - x', t - t'), \quad (38.23.1)$$

where G_{ij} is Green's function defined by (8.10).

When a uniform motion of the dislocation distribution with a constant velocity is considered, we can write

$$\alpha_{hi}(x, t) = \alpha_{hi}(x - Vt). \quad (38.24)$$

Putting

$$x' - Vt' = y, \quad dy = dx', \quad (38.25)$$

the integration with respect to t' in (38.23) yields $2\pi\delta(\xi \cdot V - \omega)$. Then, (38.23) becomes

$$\begin{aligned} u_i(x, t) = & -(2\pi)^{-3} \epsilon_{nkh} \iint_{-\infty}^{\infty} C_{jlmn} V_k \alpha_{hm}(y) \frac{\xi_l}{\xi \cdot V} \frac{N_{ij}(\xi, \xi \cdot V)}{D(\xi, \xi \cdot V)} \\ & \times \exp\{i\xi \cdot (x - y - Vt)\} d\xi dy. \end{aligned} \quad (38.26)$$

The above result can be directly applied to problems of the static dislocation distribution by a limiting process $V \rightarrow 0$ and $V_k/V \rightarrow v_k$, where v is the

direction of motion when the dislocation α_{hm} is introduced into the material. Then (38.26) becomes

$$u_i(x) = -(2\pi)^{-3} \epsilon_{nkh} \iint_{-\infty}^{\infty} C_{jlmn} \alpha_{hm}(y) \frac{\xi_l v_k}{\xi \cdot v} \frac{N_{ij}(\xi)}{D(\xi)} \exp\{i\xi \cdot (x - y)\} d\xi dy. \quad (38.27)$$

It is interesting to note that the displacement field for a given static dislocation distribution $\alpha_{hm}(x)$ is not unique, depending on the choice of v .

For a single dislocation line L , using (37.6) we have

$$u_i(x) = -(2\pi)^{-3} \epsilon_{nkh} \int_{-\infty}^{\infty} \int_L C_{jlmn} b_m \frac{\xi_l v_k}{\xi \cdot v} \frac{N_{ij}(\xi)}{D(\xi)} \exp\{i\xi \cdot (x - y)\} d\xi dl_h. \quad (38.28)$$

This expression has been obtained by Mura (1971).

The plastic distortion as well as the displacement for a given static distribution of dislocations is not a state quantity. It is uniquely determined only when the history of creation of the dislocations is given. As with the displacement field, the history is assigned by a given uniform motion of dislocations in the v direction with an infinitely small velocity V .

Substituting (38.22) into (38.9), we have

$$\begin{aligned} \beta_{ji}^p(x, t) = & (2\pi)^{-4} \epsilon_{jkh} \iiint_{-\infty}^{\infty} \frac{V_k(x', t')}{i\omega} \alpha_{hi}(x', t') \exp\{i\xi \cdot (x - x') \\ & - i\omega(t - t')\} d\xi d\omega dx' dt'. \end{aligned} \quad (38.29)$$

For a uniformly moving dislocation distribution with a constant velocity V , (38.29) becomes

$$\begin{aligned} \beta_{ji}^p(x, t) = & (2\pi)^{-3} \epsilon_{jkh} \iint_{-\infty}^{\infty} \frac{V_k}{i\xi \cdot V} \alpha_{hi}(y) \exp\{i\xi \cdot (x - y - Vt)\} d\xi dy. \end{aligned} \quad (38.30)$$

For a static distribution of dislocations, after limiting process $V \rightarrow 0$, $V_k/V \rightarrow v_k/v$, (38.30) becomes

$$\beta_{ji}^p(x) = (2\pi)^{-3} \epsilon_{jkh} \iint_{-\infty}^{\infty} \frac{v_k}{i\xi \cdot v} \alpha_{hi}(y) \exp\{i\xi \cdot (x - y)\} d\xi dy. \quad (38.31)$$

It is easy to show that two β_{ji}^p with different v yield the same dislocation density tensor when (38.31) is substituted into (38.8) and the divergency law

$$\alpha_{hi,h}(x) = 0 \quad (38.32)$$

is taken into account. Equation (38.32) can be derived from (38.8). It implies that dislocation lines must close on themselves.

The elastic field of moving dislocations

We start with the solution (8.11), where ϵ_{mn}^* is taken as β_{nm}^p . The plastic distortion tensor β_{ji}^p is related to the dislocation density tensor through (38.8) and its time derivative is related to the dislocation velocity tensor through (38.6). Expression (8.11) is

$$u_i(x, t) = - \iint_{-\infty}^{\infty} C_{jlmn} \beta_{nm}^p(x', t') G_{ij,l}(x - x', t - t') dx' dt'. \quad (38.33)$$

The distortion is

$$\begin{aligned} u_{i,j}(x, t) &= - \iint_{-\infty}^{\infty} C_{kilmn} \beta_{nm}^p(x', t') G_{ik,lj}(x - x', t - t') dx' dt' \\ &= - \iint_{-\infty}^{\infty} (\epsilon_{jnh} \epsilon_{pqh} C_{kilmn} G_{ik,lp} \beta_{qm}^p + C_{kilmn} G_{ik,ln} \beta_{jm}^p) dx' dt', \end{aligned} \quad (38.34)$$

since $\epsilon_{jnh} \epsilon_{pqh} = \delta_{jp} \delta_{nq} - \delta_{iq} \delta_{np}$.

After integrating by parts the first term in (38.34) and applying (9.1) to the second term, we have

$$\begin{aligned} u_{i,j}(x, t) &= - \iint_{-\infty}^{\infty} (\epsilon_{jnh} \epsilon_{pqh} C_{kilmn} G_{ik,lp} \beta_{qm,p}^p - \delta_{mi} \delta(x - x') \delta(t - t') \beta_{jm}^p \\ &\quad + \rho \dot{G}_{mi} \dot{\beta}_{jm}^p) dx' dt'. \end{aligned} \quad (38.35)$$

Equations (38.8) and (38.6) are used in (38.35). Then, the elastic distortion $\beta_{ji} = u_{i,j} - \beta_{ji}^p$ is obtained as

$$\beta_{ji}(x, t) = \iint_{-\infty}^{\infty} (\epsilon_{jnh} C_{kilmn} G_{ik,l} \alpha_{hm} + \rho \dot{G}_{mi} \epsilon_{jnh} V_{nhm}) dx' dt'. \quad (38.36)$$

The corresponding stress is $\sigma_{ij} = C_{ijkl} \beta_{kl}$.

For a single dislocation loop L , $\alpha_{hm} dx' = b_m \nu_h dl$ and $V_{nhm} dx' = V_n b_m \nu_h dl$ and therefore,

$$\begin{aligned}\beta_{ji}(x, t) = & \int_{-\infty}^{\infty} dt' \int_L \left\{ \epsilon_{jnh} C_{klmn} G_{ik,l}(x - x', t - t') \right. \\ & \left. + \rho \dot{G}_{mi}(x - x', t - t') \epsilon_{jnh} V_n \right\} b_m \nu_h dl(x'),\end{aligned}\quad (38.37)$$

where ν is the direction of dislocation element dl .

Wave equations of tensor potentials

The following tensor potentials have been defined by Mura (1964):

$$\begin{aligned}B_{kmji}(x, t) &= \iint_{-\infty}^{\infty} G_{km}(x - x', t - t') \beta_{ji}^p(x', t') dx' dt', \\ A_{kmnhi}(x, t) &= \iint_{-\infty}^{\infty} G_{km}(x - x', t - t') V_{nhi}(x', t') dx' dt', \\ \phi_{kmhi}(x, t) &= \iint_{-\infty}^{\infty} G_{km}(x - x', t - t') \alpha_{hi}(x', t') dx' dt'.\end{aligned}\quad (38.38)$$

Then, the property of Green's function leads to the following wave equations,

$$\begin{aligned}C_{ijkl} B_{kpnm,lj} - \rho \ddot{B}_{ipnm} &= -\delta_{ip} \beta_{nm}^p, \\ C_{ijkl} \phi_{kphm,lj} - \rho \ddot{\phi}_{iphm} &= -\delta_{ip} \alpha_{hm}, \\ C_{ijkl} A_{kpshm,lj} - \rho \ddot{A}_{ipshm} &= -\delta_{ip} V_{shm}.\end{aligned}\quad (38.39)$$

From the relations (38.8) and (38.19), we have

$$\begin{aligned}\phi_{kphm} &= -\epsilon_{hsn} B_{kpnm,s}, \\ \epsilon_{nsh} A_{kpshm} &= -\dot{B}_{kpnm}.\end{aligned}\quad (38.40)$$

The second and third equations in (38.39) are similar to the wave equations of the potentials derived from the Maxwell equations in the vacuum,

$$\begin{aligned}\phi_{,jj} - (1/c^2) \ddot{\phi} &= -e, \\ A_{i,jj} - (1/c^2) \ddot{A}_i &= -ev_i/c,\end{aligned}\quad (38.41)$$

where c is the velocity of light, e the charge density, and v is the velocity of the charge. It is seen from (38.2) that the right-hand terms in (38.41) are similar to the right-hand terms in (38.39). The Lorentz condition

$$A_{l,l} + (1/c)\dot{\phi} = 0 \quad (38.42)$$

corresponds to the relation

$$\dot{\phi}_{kphm} = A_{kphlm,l} - A_{kplhm,l} \quad (38.43)$$

which is derived from (38.40) by eliminating B . $A_{kphlm,l}$ vanishes when V_{hlm} can be written as $V_h\alpha_{lm}$ with constant V_h , since $\alpha_{lm,l} = 0$, where differentiations with respect to x and t of the potentials in (38.38) are reduced to the differentiations of the source functions (the flux and density tensors) with respect to x' and t' after integration by parts.

When B and ϕ in (38.39) are known, the elastic distortion is evaluated from (38.36),

$$\beta_{ji} = \epsilon_{jnh}(C_{klmn}\phi_{ikhm,l} + \rho\dot{A}_{minhm}), \quad (38.44)$$

where ρ is assumed to be a constant.

The analogy between the dislocation theory and the electro-magnetic theory has been discussed by several researchers in different contexts, e.g. Holländer (1960), Schaefer (1969), Kröner (1958), Günther (1967), Minagawa (1971) among others. The gauge invariant associated with dislocations has been discussed by Golebiewska-Lasota (1979), Edelen (1980), and Günther (1984).

39. Energies and forces

In this section we consider the elastic strain energy of a static dislocation distribution, the interaction energy, and the forces acting on dislocations.

The elastic strain energy W^* caused by a plastic distortion β_{ji}^p has the same form as (13.3),

$$W^* = -\frac{1}{2} \int_{-\infty}^{\infty} \sigma_{ij} \beta_{ji}^p \, dx, \quad (39.1)$$

where β_{ji}^p is associated with α_{hi} through (38.8).

For a dislocation loop L , (37.1) leads to

$$W^* = \frac{1}{2} \int_S \sigma_{ij} b_i n_j \, dS, \quad (39.2)$$

where S is the slip plane bounded by L . For example, the straight screw dislocation, shown in Fig. 4.1, has an elastic strain energy per unit length of dislocation line,

$$W^* = \frac{\mu b}{4\pi} \log[x]_0^\infty = \frac{\mu b}{4\pi} \log(R/r_0), \quad (39.3)$$

where R is the size of material and r_0 is the atomic distance of lattice (dislocation core radius).

If the straight dislocation is in an anisotropic medium, σ_{ij} in (39.2) is calculated from (36.16). Then, the elastic strain energy per unit length of dislocation is

$$\begin{aligned} W^* &= \frac{1}{2} C_{ijkl} \int_{r_0}^R r^{-1} \, dr \nu_h \sum_{klh} (\mathbf{m}, \mathbf{n}) b_i n_j \\ &= \frac{1}{2} C_{ijkl} \nu_h \sum_{klh} (\mathbf{m}, \mathbf{n}) b_i n_j \log(R/r_0), \end{aligned} \quad (39.4)$$

where S in (39.2) has been taken as the plane with a normal vector \mathbf{n} as shown in Fig. 36.1.

The self-force on a dislocation loop is derived from the variation of W^* for an infinitesimal change of β_{ij}^p in (39.1). The variation becomes

$$\delta W^* = - \int_{-\infty}^{\infty} \sigma_{ij} \delta \epsilon_{ij}^p \, dx. \quad (39.5)$$

The factor $\frac{1}{2}$ disappears when a variation of (13.1) is taken and integration by parts is performed as in (13.2). The variation $\delta \epsilon_{ij}^p$ is caused by a vertical displacement $\delta \xi$ of a line element dl of the dislocation loop L . It holds from (38.2) and (38.6) that

$$\delta \beta_{ji}^p = -\epsilon_{jmni} \delta \xi_m \alpha_{ni}. \quad (39.6)$$

Since $\delta\epsilon_{ij}^p = \frac{1}{2}(\delta\beta_{ji}^p + \delta\beta_{ij}^p)$ and $\alpha_{ni} dx = v_n b_i dl$, (39.5) becomes

$$\delta W^* = \int_L \epsilon_{jmn} v_n b_i \sigma_{ij} \delta \xi_m dl, \quad (39.7)$$

where v is the direction of the line element.

The Peach-Koehler (1950) force acting on the dislocation line element is defined by

$$f_m = -\epsilon_{mnp} v_n \sigma_{ij} b_i. \quad (39.8)$$

Then,

$$\delta W^* = - \int_L f_m \delta \xi_m dl. \quad (39.9)$$

The interaction energy between the dislocation L and a stress field σ_{ij}^0 is defined in the same way as (13.13),

$$\Delta W = - \int_{-\infty}^{\infty} \sigma_{ij}^0 \epsilon_{ij}^p dx. \quad (39.10)$$

Its variation becomes

$$\begin{aligned} \delta(\Delta W) &= - \int_{-\infty}^{\infty} \sigma_{ij}^0 \delta \epsilon_{ij}^p dx \\ &= - \int_L f_m^0 \delta \xi_m dl, \end{aligned} \quad (39.11)$$

where f_m^0 has the same form as (38.9) with σ_{ij}^0 . The irreversibility of plastic work done on dl by σ_{ij} and σ_{ij}^0 is

$$(f_m + f_m^0)V_m > 0, \quad (39.12)$$

where V is the velocity of the dislocation line. The above expression can be extended to a continuous distribution of dislocations α_{ni} , moving with dislocation velocity tensor V_{mni} . The rate of plastic work per unit volume then becomes

$$-\epsilon_{mnp} (\sigma_{ij} + \sigma_{ij}^0) V_{mni} > 0. \quad (39.13)$$

The last result is important in investigating the mathematical theory of plasticity in the context of the dislocation theory, described in later sections.

Further investigations on the self-force f_m are presented by Asaro and Hirth (1973), Barnett (1976), Gavazza and Barnett (1976), Minagawa (1970), Das et al. (1973), and Golebiewska-Lasota (1978), among others. Line integral expressions of the interaction energy and self-energy of dislocation loops in anisotropic materials have been obtained by Mura (1969).

Dynamic consideration

Let us consider the following Lagrangian functional for a moving dislocation loop in an infinitely extended material:

$$\int_{t_0}^{t_1} \mathcal{L} dt = \int_{t_0}^{t_1} dt \int_D \left(\frac{1}{2} \rho \dot{u}_i \dot{u}_i - \frac{1}{2} \sigma_{ij} \beta_{ji} \right) dx, \quad (39.14)$$

where

$$\begin{aligned} \beta_{ji} &= u_{i,j}, \\ \sigma_{ij,j} &= \rho \ddot{u}_i, \\ \sigma_{ij} &= C_{ijkl} \beta_{lk}. \end{aligned} \quad (39.15)$$

The above formulae are defined everywhere except on the slip surface S . The boundary of S (which is the dislocation line) is moving with velocity ξ . The variation of (39.14) becomes

$$\begin{aligned} \delta \int_{t_0}^{t_1} \mathcal{L} dt &= \int_{t_0}^{t_1} dt \int_D (\rho \dot{u}_i \delta \dot{u}_i - \sigma_{ij} \delta \beta_{ji}) dx \\ &= \int_{t_0}^{t_1} dt \int_D \left[\rho \dot{u}_i \frac{\partial}{\partial t} (\delta u_i) - \sigma_{ij} \frac{\partial}{\partial x_j} (\delta u_i) \right] dx. \end{aligned} \quad (39.16)$$

Applying the Gauss theorem, we have

$$\begin{aligned} \int_D \sigma_{ij} \frac{\partial}{\partial x_j} (\delta u_i) dx &= \int_{S+\delta S} \sigma_{ij} n_j [\delta u_i] dS - \int_D \sigma_{ij,j} \delta u_i dx \\ &= \int_{\delta S} \sigma_{ij} n_j b_i \delta S - \int_D \sigma_{ij,j} \delta u_i dx, \end{aligned} \quad (39.17)$$

where δS is the variation of slip surface S , which is expressed by the vector product of a virtual displacement of the dislocation position $\delta \xi$ and the dislocation line element $v \, dl$,

$$n_j \delta S = \epsilon_{jlh} \delta \xi_l v_h \, dl. \quad (39.18)$$

In the above derivation, $[u_i]$ is the difference of u_i evaluated at the upper and lower surfaces of the slip plane: $[u_i] = b_i$ on S . After giving a vertical displacement $\delta \xi_l$ to the position of the dislocation, the variation $[\delta u_i] = \delta [u_i]$ is zero on S , and b_i on δS .

Furthermore, we have

$$\int_{t_0}^{t_1} \rho \dot{u}_i \frac{\partial}{\partial t} (\delta u_i) \, dt \, dx = - \int_{\delta S} \rho \dot{u}_i b_i \dot{\xi}_j n_j \delta S - \int_{t_0}^{t_1} \rho \ddot{u}_i \delta u_i \, dt \, dx. \quad (39.19)$$

Substituting (39.17), (39.19) into (39.16), we obtain

$$\delta \int_{t_0}^{t_1} \mathcal{L} \, dt = - \int_{t_0}^{t_1} dt \oint_L \left(\rho \dot{u}_i \dot{\xi}_j + \sigma_{ij} \right) b_i \epsilon_{jlh} \delta \xi_l v_h \, dl. \quad (39.20)$$

The term $\rho \dot{u}_i \dot{\xi}_j b_i \epsilon_{jlh} v_h$ is called the Lorentz force. Since $\dot{\xi}_j dt = \delta \xi_j$ and $\epsilon_{jlh} \delta \xi_j \delta \xi_l = 0$, (39.20) becomes

$$\delta \int_{t_0}^{t_1} \mathcal{L} \, dt = - \int_{t_0}^{t_1} dt \oint_L \sigma_{ij} b_i \epsilon_{jlh} \delta \xi_l v_h \, dl. \quad (39.21)$$

The Lorentz force does not contribute to the energy balance and the result is the same as the static case (39.7).

The elastic solution of a uniformly moving straight dislocation has been obtained by Frank (1949) and Eshelby (1949). Frank has shown that when a screw dislocation is moving with a constant velocity V , the total energy (the sum of the elastic strain energy and the kinetic energy) is given by $E_0/(1 - V^2/c^2)^{1/2}$, where E_0 is the elastic strain energy of the dislocation at rest, and c the shear wave velocity $(\mu/\rho)^{1/2}$. His calculation has been stimulated by the work done by Kontorova and Frenkel (1938) who studied a one-dimensional dislocation model and found a strikingly analogous form to that of a particle in special relativity. Frank's calculation was extended to an edge dislocation by Eshelby (1949). In this case no relativistic relation is found since the dilatational wave velocity is also involved.

The uniform motion of a straight dislocation is possible in the absence of an applied stress since $\sigma_{ij} b_i \epsilon_{jlh} v_h$ becomes zero at the dislocation position. It is not yet clear how the dislocation arrives at the constant velocity. Eshelby has extended his analysis to the Peierls dislocation (1940). No applied stress is necessary for uniform motion if the width of the dislocation is $2\bar{\xi} = \{b/(1 - v)\}D(V)/D(0)$ for an edge dislocation or $2\bar{\xi} = b\beta$ for a screw dislocation, where $D(V) = -2\mu(2c^2/V^2)(\gamma - a^4/\beta)$, $\gamma = (1 - V^2/a^2)^{1/2}$, $\beta = (1 - V^2/c^2)^{1/2}$, $a = (1 - V^2/2c^2)^{1/2}$, $c = (\mu/\rho)^{1/2}$ and $a = \{(\lambda + 2\mu)/\rho\}^{1/2}$. It is interesting to note that the width of the edge dislocation vanishes when $D(V) = 0$, the solution of which is the Rayleigh wave velocity. The width of the screw dislocation vanishes at $V = c$. Eshelby's calculation has been extended by Leibfried and Dietze (1949) to a dislocation moving in the middle of a plate (direction of dislocation parallel to the plate surface). Weertman (1960, 1961, 1966) has completed Eshelby's analysis (1949) by calculating the elastic strain energy and the kinetic energy of the edge dislocation. He has found that the shear stress on the slip plane (except at the dislocation position) decreases with increasing dislocation velocity, vanishes at the Rayleigh velocity, and changes signs at velocities higher than the Rayleigh velocity. Because of this, edge dislocations of like sign attract each other (contrary to the usual situation) when the dislocations are traveling at velocities above the Rayleigh velocity. Such a critical velocity of a dislocation is called the threshold velocity. The threshold velocities have been calculated for various crystals by Teutonico (1961, 1962, 1963) and Weertman (1962). They started their stress analysis from the general solution obtained by Bullough and Bilby (1954) for a uniformly moving straight dislocation in an anisotropic media. If a dislocation of general shape is considered, the formula (38.23.1) is recommended.

The effect of free surfaces and the inhomogeneity of materials on dislocation motion has not been fully investigated. Weertman (1963) considered a dislocation moving on the interface between two isotropic media of different elastic constants and densities. The case where a dislocation is moving parallel to the interface has been solved by M.S. Lee (1972).

Although the energy of a moving dislocation becomes infinite as its velocity approaches the sound velocity, there are dislocation solutions of the elastic equations for greater dislocation velocities than c . This dislocation-like solution was first demonstrated by Eshelby (1956). He suggests its possible application to the propagation of diffusionless transformations by dislocations; however, no successful application has been reported. Callias and Markenscoff (1980) analyzed the stress field due to the general transient motion of a supersonic screw dislocation and in particular the wave-front behaviour.

The applied stress required for the supersonic dislocation motion becomes

infinity. This difficulty is avoided by Weertman (1967, 1969) by considering a spread dislocation with a distribution function $B(x_1 - Vt)$. If B is a constant in the region $-A < (x_1 - Vt) < A$ and zero elsewhere, the equilibrium condition is satisfied for a uniformly applied stress. Weertman also has discussed atomic force law under which the equilibrium condition is satisfied without applied stress: see the review paper by Mura (1972). For a spread supersonic dislocation, Clifton and Markenscoff (1981) estimated that the required stress to sustain the supersonic velocity would exceed the theoretical strength of the material.

Almost all calculations for moving dislocations have been limited to straight infinite dislocations. Very few calculations have been done for circular dislocation loops. Günther (1968) has obtained the elastic solution for a circular edge dislocation moving uniformly in the direction of the normal to the circular plane. In the region $V < c < a$ the elastic field shows double Lorentz contractions and in the region $c < a < V$ there arise two Mach cones radiated from the dislocation loop with angles $\pm \sin^{-1}(V/c)$ and $\pm \sin^{-1}(V/a)$ with respect to the direction of the motion. No disturbance is seen outside the domain bounded by the cones and the circular plane, where c and a are the shear and dilatational velocities.

Unlike the uniform motion of a straight dislocation with a subsonic velocity, an oscillating straight dislocation generally requires the application of an oscillating stress (forces vibration). Free vibrations, however, are possible under special circumstances. By using Eshelby's elastic solution (1949) for an oscillating screw dislocation and the equilibrium condition, Nabarro (1951) has obtained the amplitude and phase-difference for the position of the oscillating dislocation as functions of the amplitude and wave number of an incident sound wave. He also has calculated the scattering cross-section (the ratio of the rate of radiation to the incident energy flux). Same calculation is done for an edge dislocation by Kiusalaas and Mura (1964). On the other hand, Leibfried (1950) has pointed out that a dislocation moving through a flux of sound waves (which are a natural consequence of the thermal energy of the crystal) experiences a retarding force which is proportional to the dislocation velocity. The numerical value of his result, however, is criticized by Nabarro (1951), Lothe (1960), and Eshelby (1962). Eshelby has used a kink model for mathematical simplicity and found that a kink moving through an isotropic flux of elastic waves has a scattering cross-section proportional to the square of its width, and it experiences a retarding force proportional to the product of its velocity and the energy density of the waves. Pegel (1966), Laub and Eshelby (1966), and Ninomiya and Ishioka (1967) have obtained dispersion curves for the free vibration of a straight dislocation. They found that free vibrations are possible for a certain range of frequencies and wave lengths, but that in general a suitable applied stress is required to maintain the vibrations.

Eshelby (1953) has considered several cases of the accelerated motion of a screw dislocation. The dislocation is at rest for negative t , and thereafter moves 1) with a velocity $c^2t/(x_0^2 + c^2t^2)^{1/2}$ and acceleration $c^2x_0^2/(x_0^2 + c^2t^2)^{3/2}$, 2) with a constant acceleration, 3) with a constant velocity. He has calculated the uniform applied stresses required for these motions. He also has calculated the velocity of the dislocation when a constant stress is applied at $t = 0$ and $t = t_1$ so that the total impulse is a constant value. Like Nabarro (1951), Eshelby also accounts for the Peierls stress law. According to the present author's opinion, no such attention is necessary as long as the equilibrium condition is being considered at $\bar{x}_1 = 0$, $x_2 = \frac{1}{2}b$. Some details of the elastic field caused by the uniform motion of a dislocation starting from rest are given by Ang and Williams (1959) and by Kiusalaas and Mura (1964). The velocity field has a singularity at the wave front. However, if the dislocation starts to move with a constant acceleration, the velocity at the wave front becomes zero. A solution for the general non-uniform motion has been obtained by Markenscoff (1980) for a screw dislocation and by Markenscoff and Clifton (1981) for an edge dislocation. The self-stress of a dislocation starting from rest and moving with a constant velocity has been obtained by Kiusalaas and Mura (1964) for a screw dislocation and Clifton and Markenscoff (1981) for an edge dislocation. Recently, Markenscoff and Li (1984) obtained the exact solution for the field radiated from a circular dislocation loop expanding in its own plane with constant velocity.

The interaction between a varying stress field and a moving screw dislocation has an electro-magnetic analogy as shown by Eshelby (1953) and Nabarro (1951). From this analogy, Nabarro predicts that a moving dislocation receives a Lorentz force (in addition to the Peach-Koehler force) similar to a moving-line density of charge. Since the Lorentz force has been predicted strictly from this analogy, its existence has been criticized by several authors. Although in some papers the Lorentz force is derived from a Lagrangian formalism, either the mathematics or the physics involved in these derivations is questionable.

The scattering of elastic waves by a dislocation is only one of the many energy dissipative mechanisms which have been suggested by a number of investigators. Eshelby (1949) has shown that the thermoelastic effect around a moving edge dislocation produces an irreversible heat flow causing energy dissipation. A more rigorous analysis of the thermoelastic dissipation due to an edge dislocation moving at an arbitrary speed has been performed by Weiner (1958). In order to produce a similar thermal effect for a moving screw dislocation, Mason (1960) has proposed a shear wave effect which would instantaneously raise the temperature of those phonons which have components along the compressed direction, while lowering the temperature of those which have components along the extended direction.

As an element of the dislocation line moves through the lattice, its potential energy varies in a roughly sinusoidal manner; Orowan (1940). Therefore, the traveling dislocation experiences an oscillatory force. The energy is dissipated through this oscillation by a mechanism similar to that mentioned in the last section. Hart (1955) has calculated the magnitude of this dissipation. Various other dissipative mechanisms have been studied by Lothe (1962), Koehler (1952) and Granato and Lücke (1956). Some atomistic aspects of thermally activated motions of dislocations have been investigated by Weiner (1969), Kröner (1955), Holländer (1960), Schaefer (1969), Günther (1967), Minagawa (1971), Golebiewska-Lasota (1979) among others discussed some analogies between the electromagnetic and moving dislocation fields. However, no practical applications have been found yet.

40. Plasticity

In this section the mathematical theory of plasticity is interpreted in terms of dislocation theory. The basic idea is that a stress field in continuum plasticity is the sum of the applied elastic stress (solution of a boundary value problem in elasticity) and the dislocation stress caused by dislocations created in the so-called plastic domains. The Mises yield criterion and the Prandtl–Reuss relation between stress and plastic strain rate are also derived from the theory of continuous distributions of dislocations.

Mathematical theory of plasticity

The fundamental equations of continuum plasticity are reviewed here before the dislocation theory is introduced. In the absence of body forces the equation of equilibrium is

$$\sigma_{ij,j} = 0. \quad (40.1)$$

The traction boundary conditions are

$$\sigma_{ij}n_j = F_i \text{ on } S, \quad (40.2)$$

where F_i is the applied force on surface S and n_i is the exterior unit normal on S . The strain ϵ_{ij} is the sum of the elastic strain e_{ij} and the plastic strain ϵ_{ij}^p ,

$$\epsilon_{ij} = e_{ij} + \epsilon_{ij}^p. \quad (40.3)$$

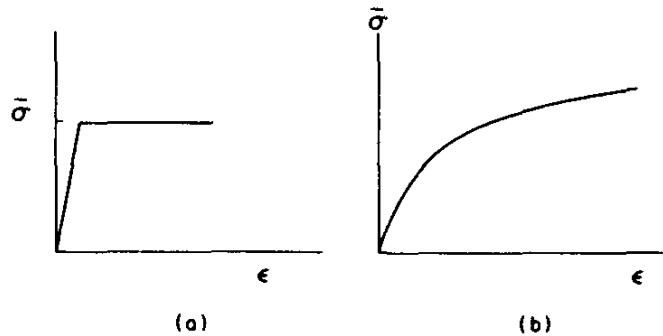


Fig. 40.1 Stress-strain curve in the simple tension test (a) is perfectly elasto-plastic material. (b) is work-hardening material

The strain ϵ_{ij} is compatible and expressed by

$$\epsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i}), \quad (40.4)$$

where u_i is the displacement component. The material is in a plastic state when

$$\frac{1}{2}s_{ij}s_{ij} = k^2 = \frac{1}{3}\bar{\sigma}^2, \quad (40.5)$$

where

$$s_{ij} = \sigma_{ij} - \frac{1}{3}\delta_{ij}\sigma_{kk} \quad (40.6)$$

is the reduced stress component (or deviatoric). k is the shearing yield stress for a perfectly elasto-plastic material and the shearing flow stress for a work-hardening material, while $\bar{\sigma}$ is the yield stress or flow stress in a simple tension test. $\bar{\sigma}$ (or k) is a constant for the perfectly elasto-plastic material (see Fig. 40.1a), while it is a function of plastic strain for the work-hardening material (see Fig. 40.1b). In this latter case we have

$$\bar{\sigma} = \bar{\sigma} \left(\int d\bar{\epsilon}^P \right), \quad (40.7)$$

where

$$d\bar{\epsilon}^P = \left\{ \frac{2}{3} d\epsilon_{ij}^P d\epsilon_{ij}^P \right\}^{1/2} \quad (40.8)$$

is the effective plastic strain increment. $\bar{\sigma}$ is also called the effective stress and, from (40.5),

$$\bar{\sigma} = \left\{ \frac{3}{2}s_{ij}s_{ij} \right\}^{1/2}. \quad (40.9)$$

The condition (40.5) is called the Mises yield criterion (1928). Finally, the constitutive equations are

$$\begin{aligned} \mathrm{d}\epsilon'_{ij} &= \mathrm{d}s_{ij}/2\mu, \\ \mathrm{d}\epsilon_{kk} &= \mathrm{d}\sigma_{kk}/3K, \\ \mathrm{d}\epsilon^P_{ij} &= s_{ij} \mathrm{d}\mu^*, \end{aligned} \quad (40.10)$$

where μ is the elastic shear modulus, and K the bulk modulus. $\mathrm{d}\mu^*$ is an unknown scalar quantity for the perfectly elasto-plastic material. For the workhardening material, it is

$$\mathrm{d}\mu^* = 3 \frac{\mathrm{d}\bar{\epsilon}^P}{2\bar{\sigma}} = 3 \left(\frac{\mathrm{d}\bar{\epsilon}^P}{\mathrm{d}\bar{\sigma}} \right) \frac{\mathrm{d}\bar{\sigma}}{2\bar{\sigma}}. \quad (40.11)$$

The last result has been obtained from $\mathrm{d}\epsilon^P_{ij} = (\mathrm{d}\mu^*)^2 s_{ij} s_{ij}$ and definitions (40.8) and (40.9). The slope $\mathrm{d}\bar{\sigma}/\mathrm{d}\bar{\epsilon}^P$ is obtained from the stress-strain curve in Fig. 40.1(b). The third equation in (40.10) has been proposed by Reuss (1930). It is a modification of the Lévy (1871) and Mises (1928) equation, $\mathrm{d}\epsilon_{ij} = \mathrm{d}\mu^* s_{ij}$. A special case of the Reuss equation has been obtained by Prandtl (1924) for the plane problems.

Dislocation theory

Using the dislocation flux tensor V_{nh_i} in (38.19), we write the plastic strain rate as

$$\dot{\epsilon}_{ij}^P = \frac{1}{2} \left(\dot{\beta}_{ji}^P + \dot{\beta}_{ij}^P \right), \quad (40.12)$$

where

$$\dot{\beta}_{ji}^P = -\epsilon_{jnh} V_{nh_i}. \quad (40.13)$$

The rate of plastic work per unit volume is

$$\delta W^P / \mathrm{d}t = \sigma_{ij} \dot{\epsilon}_{ij}^P = -\epsilon_{lhi} \sigma_{ji} V_{lhi}. \quad (40.14)$$

The plastic deformation does not cause any volume change, that is,

$$\dot{\epsilon}_{ii}^P = -\epsilon_{lhi} V_{lhi} = 0, \quad (40.15)$$

since the plastic work is irreversible,

$$-\epsilon_{lhj}\sigma_{ji}V_{lhi} > 0. \quad (40.16)$$

Mura (1965) has shown that the two conditions (40.15) and (40.16) are satisfied when

$$V_{lhi} = d\mu^* g_{lhi} \quad (40.17)$$

with

$$g_{lhi} = -\frac{1}{2}\epsilon_{lhj}s_{ji}, \quad (40.18)$$

where $d\mu^*$ is a positive scalar quantity. g_{lhi} is the x_i -component of the force acting on α_{hi} dislocations. The magnitude of the force must be a constant k for the perfectly elasto-plastic materials. Equation (40.13) leads to

$$g_{lhi}g_{lhi} = \frac{1}{2}s_{ij}s_{ij} = k^2 \quad (40.19)$$

which is the Mises yield criterion (40.5)

When (40.12) and (40.13) are used with (40.17), we have

$$\dot{\epsilon}_{ij}^p = \dot{\beta}_{ij}^p = \dot{\beta}_{ji}^p = d\mu^* s_{ij}, \quad (40.20)$$

which corresponds to the Reuss equation in (40.10). $d\mu^*$ may be determined from the simple tension test for work-hardening materials as seen in the mathematical theory of plasticity. $d\mu^*$ is a history-depending quantity. Kröner (1963) has assumed it to be a function of dislocation loop density. The constitutive equations in plasticity have been further studied by Eisenberg (1970), Hahn and Jaunzemis (1973), Lamberton (1974), Smith (1970), Karihaloo (1975), Bodner (1978), Weng and Phillips (1978), and Werne and Kelly (1978), among others, from the viewpoint of continuous distributions of dislocations. Gilman (1966) has discussed the stress-strain law in terms of multiplication and motion of dislocations.

After a series of original works by Nye (1953), Bilby, Gardner and Smith (1958) and Kröner (1963), Mura (1967) has postulated that the stress field in continuum plasticity is the sum of the applied elastic stress and the dislocation stress. He also has shown that the dislocation density tensor α_{hi} can be expressed in terms of the stress components if the impotent dislocations are excluded, and the material is infinitely extended.

We start our discussion with (37.14), that is,

$$u_{i,j} = \beta_{ji} + \beta_{ji}^p. \quad (40.21)$$

When substituted into (37.11), we have

$$\alpha_{hi} = \epsilon_{hlj}\beta_{ji,l} = \epsilon_{hlj}e_{ji,l} + \epsilon_{hlj}\omega_{ji,l}, \quad (40.22)$$

where

$$\beta_{ji} = e_{ji} + \omega_{ji}. \quad (40.23)$$

The first term in (40.23) is the elastic strain and the second term is the rotational tensor ($\omega_{ji} = -\omega_{ij}$). The part of α_{hi} contributed from the second term in (40.22) is impotent. The displacement and stress fields due to $\epsilon_{hlj}\omega_{ji,l}$ can be considered to be equivalent to those caused by eigenstrain $\beta_{ji}^* = -\omega_{ji}$ [see (37.23)]. Therefore, the potent part of α_{hi} can be written as

$$\alpha_{hi} = \epsilon_{hlj}e_{ji,l}. \quad (40.24)$$

Since $e_{ji} = e_{ij} = C_{ijmn}^{-1}\sigma_{mn}$, (40.24) becomes

$$\alpha_{hi} = C_{ijmn}^{-1}\epsilon_{hlj}\sigma_{mn,l}. \quad (40.25)$$

The above result indicates that the potent dislocation distribution can be obtained if the corresponding stress distribution is known.

The basic idea of the dislocation theory in connection with the continuum theory of plasticity is that a stress field in an elasto-plastic medium is the sum of the (potent) dislocation stress and the elastic applied stress. Several examples are shown in the following subsections.

Plane strain problems

Consider an isotropic material subjected to an applied boundary force. The material is deformed to an elasto-plastic state of plane strain. In the plastic domains, the sum of the applied elastic stress and the dislocation stress satisfies the Mises yield criterion,

$$\frac{1}{4}(\sigma_x - \sigma_y)^2 + \sigma_{xy}^2 = k^2, \quad (40.26)$$

When the material is perfectly elasto-plastic, the stress field satisfying the above condition may be written as (Hill 1960)

$$\begin{aligned}\sigma_x &= -p - k \sin 2\phi, \\ \sigma_y &= -p + k \sin 2\phi, \\ \sigma_{xy} &= k \cos 2\phi,\end{aligned}\tag{40.27}$$

where $p = -\frac{1}{2}(\sigma_x + \sigma_y)$ and ϕ is the angle between the direction of the first shear line and the x -axis measured counterclockwise. On the first and the second shear lines, the shear stress takes on the value of $\pm k$.

The stress is related to the elastic strain by Hooke's law in any domain (plastic or elastic),

$$\begin{aligned}e_x &= s_x/2\mu - p(1-2\nu)/E, \\ e_y &= s_y/2\mu - p(1-2\nu)/E, \\ e_{xy} &= s_{xy}/2\mu,\end{aligned}\tag{40.28}$$

where

$$\begin{aligned}s_x &= \frac{1}{2}(\sigma_x - \sigma_y), \\ s_y &= \frac{1}{2}(\sigma_y - \sigma_x), \\ s_{xy} &= \sigma_{xy}, \\ p &= -\frac{1}{2}(\sigma_x + \sigma_y).\end{aligned}\tag{40.29}$$

Equation (40.24) leads to

$$\begin{aligned}\alpha_{31} &= \partial e_{xy}/\partial x - \partial e_x/\partial y, \\ \alpha_{32} &= \partial e_y/\partial x - \partial e_{xy}/\partial y.\end{aligned}\tag{40.30}$$

The dislocation density is obtained from (40.30), (40.28), and (40.27). The components of α_{hi} in the directions of the shear lines 1 and 2 become

$$\begin{aligned}\alpha_1 &= \frac{k}{\mu} \frac{\partial \phi}{\partial s_2} + \frac{1-2\nu}{E} \frac{\partial p}{\partial s_2}, \\ \alpha_2 &= \frac{k}{\mu} \frac{\partial \phi}{\partial s_1} - \frac{1-2\nu}{E} \frac{\partial p}{\partial s_1},\end{aligned}\tag{40.31}$$

where ds_1 and ds_2 are line elements of the first and second shear lines. The above result has been obtained from (40.25) by assuming the x -direction to be the first shear line and the y -direction to be the second shear line. The Burgers vector of α_1 has the direction of the first shear line and that of α_2 is in the second shear line.

The equations of equilibrium with (40.27) lead to the Hencky relations,

$$\begin{aligned}\frac{\partial p}{\partial s_1} + 2k \frac{\partial \phi}{\partial s_1} &= 0, \\ \frac{\partial p}{\partial s_2} - 2k \frac{\partial \phi}{\partial s_2} &= 0.\end{aligned}\tag{40.32}$$

Then, (40.31) becomes

$$\begin{aligned}\alpha_1 &= \frac{2(2-\nu)}{E} k \frac{\partial \phi}{\partial s_2}, \\ \alpha_2 &= \frac{2(2-\nu)}{E} k \frac{\partial \phi}{\partial s_1}.\end{aligned}\tag{40.33}$$

The above result has been derived by Mura (1967). A similar result has been obtained by Eisenberg (1970) and Weng and Phillips (1976).

Lines of discontinuity in the continuum theory of plasticity are defined as those lines across which the stress varies in a discontinuous manner. In the dislocation theory, the lines of discontinuity are the lines along which surface dislocations are distributed. Excluding the impotent dislocations, we write the surface dislocation density tensor as,

$$\alpha_{3i} = \epsilon_{3ik} [e_{ki}] n_l, \tag{40.34}$$

where $[e_{ki}] = e_{ki}$ (in domain 2) – e_{ki} (in domain 1), and the normal n_l is directed toward domain 2 from domain 1. The above equation has been obtained from (37.15), (40.21), and (40.23). The traction force should be continuous on the line of discontinuity,

$$[\sigma_{ij}] n_j = 0. \tag{40.35}$$

Equations (40.34) and (40.35) lead to

$$\begin{aligned}\alpha_{31} &= \frac{2-\nu}{E} [p] n_2, \\ \alpha_{32} &= -\frac{2-\nu}{E} [p] n_1.\end{aligned}\tag{40.36}$$

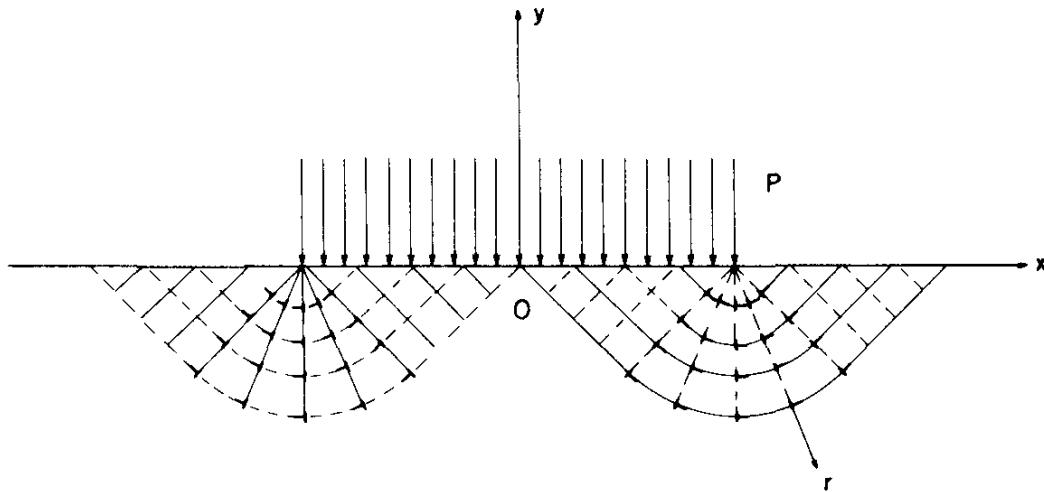


Fig. 40.2. Dislocation distributions in the fan-shaped plastic domains.

The resultant of α_{hi} along the line of discontinuity is

$$\alpha_s = \alpha_{31} \frac{dx_1}{ds} + \alpha_{32} \frac{dx_2}{ds} = -\frac{2-\nu}{E} [p], \quad (40.37)$$

where ds is the line element along the line of discontinuity, and $dx_1/ds = -n_2$, $dx_2/ds = n_1$. The Burgers vector of α_s is tangential to the line of discontinuity (see Fig. 40.7).

Consider a half-space with a uniform pressure $P = (2 + \pi)k$ along $|x| \leq \alpha$, $y = 0$. The first and second shear lines are shown by the solid and dashed lines in Fig. 40.2. The distribution of dislocations is easily found from (40.28). In the fan-shaped region in the right-hand side in Fig. 40.2, $\partial\phi/\partial s_1 = 1/r$, and

$$\alpha_1 = 0, \quad \alpha_2 = \frac{(2-\nu)}{E} \frac{2k}{r}. \quad (40.38)$$

Similarly,

$$\alpha_1 = \frac{(2-\nu)}{E} \frac{2k}{r}, \quad \alpha_2 = 0, \quad (40.39)$$

in the left-hand fan-shaped region. In the above equations, r is the distance measured from the edge of the indenter.

In order to demonstrate that the stress field is the sum of the applied elastic stress and the dislocation stress, we consider the simpler example shown in Fig. 40.3. The half-infinite domain is loaded by a semi-infinite band of

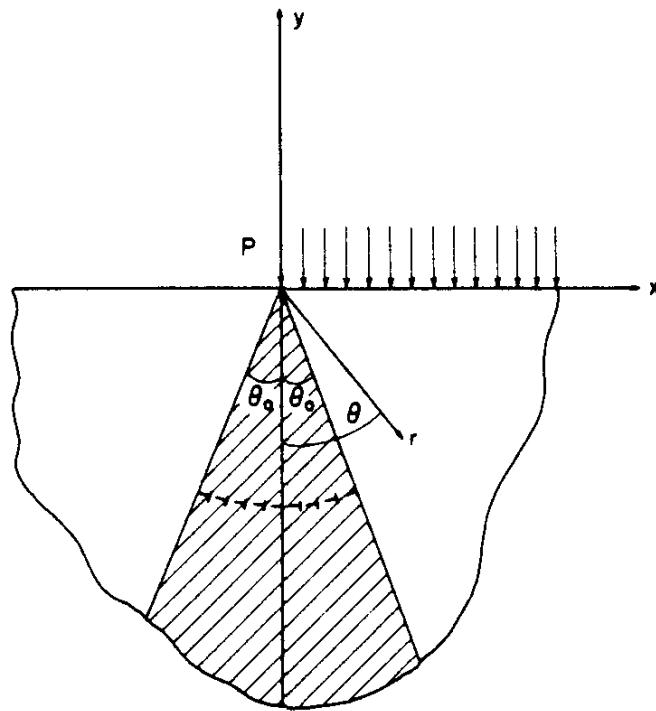


Fig. 40.3 A semi-infinite band of uniform pressure P on the surface of a half-space

uniform pressure P on the surface. The applied elastic stress components are

$$\begin{aligned}\sigma_r^A &= -\frac{P}{\pi} \left(\frac{1}{2}\pi + \theta - \frac{1}{2}\sin 2\theta \right), \\ \sigma_\theta^A &= -\frac{P}{\pi} \left(\frac{1}{2}\pi + \theta + \frac{1}{2}\sin 2\theta \right), \\ \sigma_{r\theta}^A &= \frac{P}{2\pi} (1 + \cos 2\theta),\end{aligned}\tag{40.40}$$

(see Timoshenko and Goodier 1951), where the polar coordinates (r, θ) are used. Let us assume that the plastic domain is $-\theta_0 \leq \theta \leq \theta_0$. Analogously with (40.39), the dislocations are distributed along the radii with the density inversely proportional to r ,

$$\alpha_{3r} = b/r.\tag{40.41}$$

The stress field of the distributed dislocations can be obtained by integrating over the plastic domain $|\theta| \leq \theta_0$ the solution for a single dislocation (Dun-

durs and Mura (1964)) multiplied by the density (40.39). The result is

$$\begin{aligned}\sigma_r^D &= \frac{\mu b}{2\pi} (2f + f''), \\ \sigma_\theta^D &= \frac{\mu b}{2\pi} 2f, \\ \sigma_{r\theta}^D &= -\frac{\mu b}{2\pi} f',\end{aligned}\tag{40.42}$$

where

$$\begin{aligned}f(\theta) &= (2\theta_0 + \sin 2\theta_0)(\sin 2\theta + 2\theta) - \pi \sin 2\theta_0 \cos 2\theta + 2\pi\theta_0 \\ &\quad \text{for } -\frac{1}{2}\pi \leq \theta \leq -\theta_0, \\ f(\theta) &= (2\theta_0 + \sin 2\theta_0)(\sin 2\theta + 2\theta) + \pi \cos 2\theta_0 \sin 2\theta - 2\pi\theta_0 \\ &\quad \text{for } -\theta_0 \leq \theta \leq \theta_0, \\ f(\theta) &= (2\theta_0 + \sin 2\theta_0)(\sin 2\theta + 2\theta) + \pi \sin 2\theta_0 \cos 2\theta - 2\pi\theta_0 \\ &\quad \text{for } \theta_0 \leq \theta \leq \frac{1}{2}\pi,\end{aligned}\tag{40.43}$$

and $f' = df/d\theta$.

The total stress field is

$$\begin{aligned}\sigma_r &= \sigma_r^A + \sigma_r^D, \\ \sigma_\theta &= \sigma_\theta^A + \sigma_\theta^D, \\ \sigma_{r\theta} &= \sigma_{r\theta}^A + \sigma_{r\theta}^D.\end{aligned}\tag{40.44}$$

The Mises yield criterion becomes

$$\frac{1}{2}(\sigma_r - \sigma_\theta)^2 + \sigma_{r\theta}^2 \leq k^2.\tag{40.45}$$

The equality should hold in $-\theta_0 \leq \theta \leq \theta_0$, and the inequality should hold in the rest of the body. The condition (40.45) is satisfied if (Mura 1968)

$$\begin{aligned}b &= k/\mu(1 + \cos 2\theta_0), \\ P &= 2k(2\theta_0 + \sin 2\theta_0 + \pi \cos 2\theta_0)/(1 + \cos 2\theta_0).\end{aligned}\tag{40.46}$$

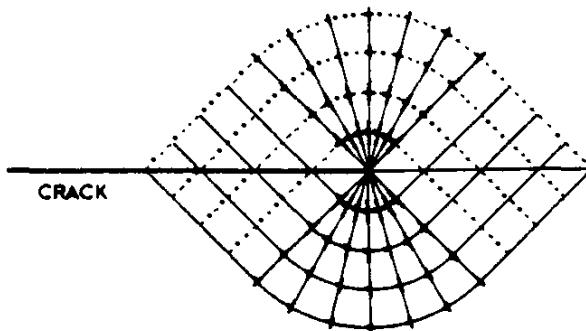


Fig. 40.4 Dislocation distributions at the crack tip.

The above relations determine the plastic domain and the necessary distribution of dislocations for a given P . The relation between p and θ_0 and the stress field (40.44) agree with the classical solution obtained by Sokolovsky (1950).

Vilmann and Mura (1979) have considered a two-dimensional distribution of dislocations as shown in Fig. 40.4, and have modified the Griffith fracture theory for application to ductile materials. From slip line theory, the stress field in the fan-shaped plastic domain is

$$\begin{aligned}\sigma_{11} &= k \{(1 + 3\pi/2 - 2\theta) - \sin 2\theta\}, \\ \sigma_{22} &= k \{(1 + 3\pi/2 - 2\theta) + \sin 2\theta\}, \\ \sigma_{12} &= k \cos 2\theta.\end{aligned}\tag{40.47}$$

From (40.24) and (40.47) the dislocation density in the radial direction is

$$\alpha_r = \frac{1-\nu}{\mu} \frac{2k}{r},\tag{40.48}$$

where r is the distance from the vertex. The Burgers vector of α_r is the radial direction. The application of this result to fatigue has been considered by Mura and Vilmann (1981).

When the applied load is reduced to zero, an additional dislocation distribution, Fig. 40.5(b), is superimposed on the old distribution, Fig. 40.5(a). When the load is applied again, the resulting dislocation distribution is the sum of those shown in Figs. 40.5 (a), (b), and (c). The sum of the dislocation distributions (b) and (c) is a dipole distribution which can be observed in fatigue tests. The second unloading adds the distribution (b) to the last one and the subsequent loading adds the distribution (c) to the preceding one and so on. Thus, the strength of the dipoles increases by the repeating loading. After some number of cyclic loadings, the crack tip advances by δa as shown in Fig. 40.6. The domain of the dipole distribution increases with the growth of the crack.

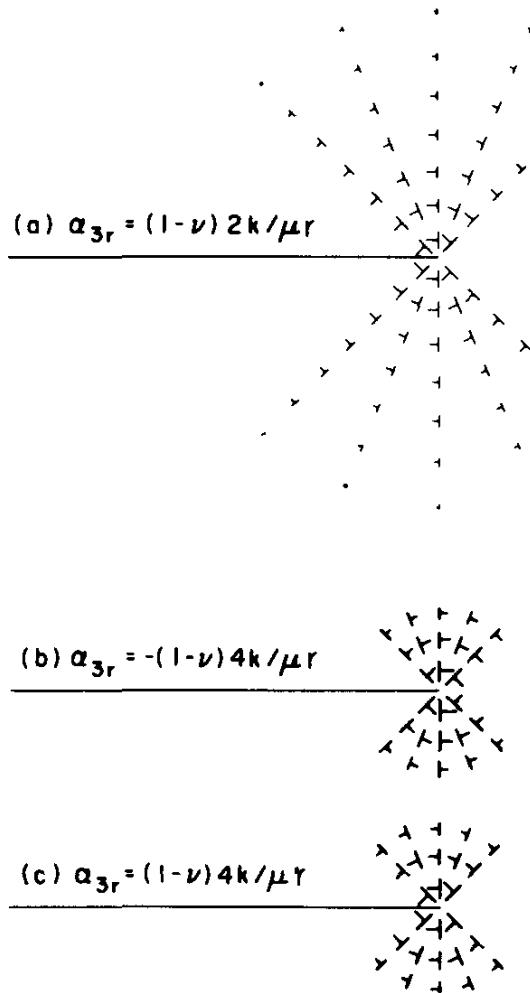


Fig. 40.5 Dislocations distributions at the crack tip. (a) At the maximum stress in the first loading (b) Additional dislocations created by the first unloading. The resulting distribution is the sum of (a) and (b). The dislocation distribution at the second loading is the sum of (a), (b) and (c). The sum of (b) and (c) is a dipole distribution.

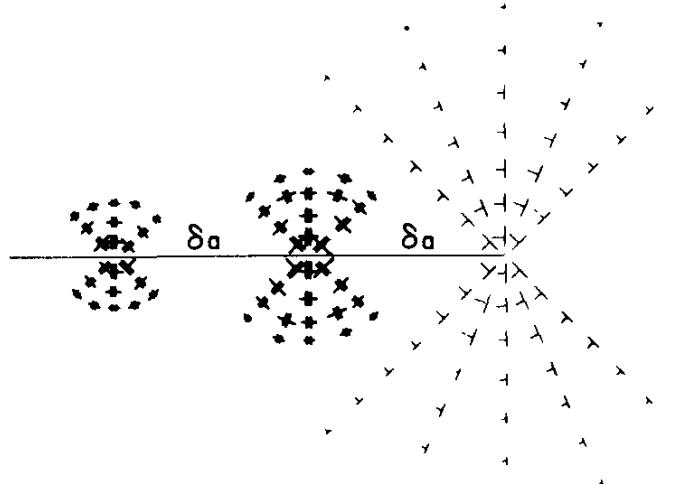


Fig. 40.6. Dislocation distributions of a propagating fatigue crack.

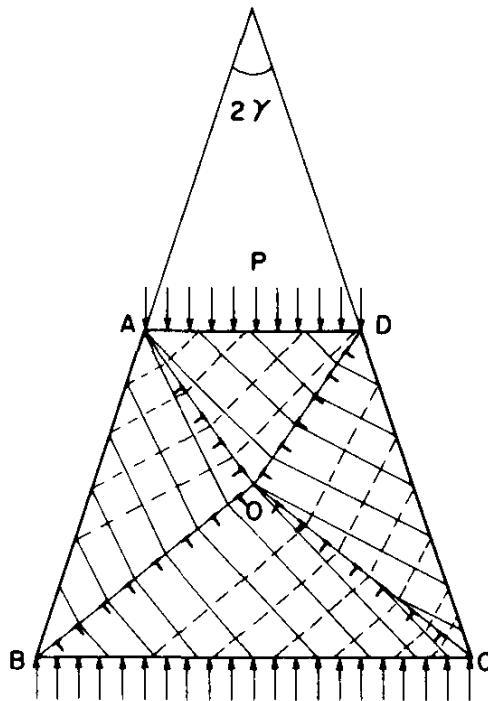


Fig. 40.7. Surface dislocations are on the lines of discontinuity.

Figure 40.7 shows surface dislocations along the lines of discontinuity when the wedge becomes fully plastic under pressure P . As seen in Prager and Hodge (1951), we have

$$[p] = -2k \sin \gamma \quad (40.49)$$

along AOC and DOB, where 2γ is the wedge angle. From (40.37) the density of the surface dislocations is obtained as

$$\alpha_s = \frac{2-\nu}{E} 2k \sin \gamma. \quad (40.50)$$

These distributions of dislocations shown in the above examples have been obtained from the statically admissible stress fields which are essentially independent of the displacement and strain fields (kinematically admissible fields). However, the present results of dislocation distributions give possible patterns of plastic deformation. Knowing the source of the dislocations (crack tips, for instance), one can evaluate possible plastic strains. These strains are the product of the dislocation density and the distance from the source.

Beams and cylinders

Nye (1953) has introduced the concept of a dislocation density and has expressed the curvature of a beam caused by dislocations in terms of the dislocation density tensor. He has considered the case where the dislocation

stress disappears. Read (1957), and Bilby, Gardner and Smith (1958) have dealt with the case where the stress does not vanish. They have obtained a relation among the stress gradient, curvature, and the dislocation density tensor. In this book, a somewhat different approach is used as illustrated by the following examples.

Consider an infinitely long beam in the x_1 -direction with a height of $2h$. Under pure bending, all quantities depend only on the x_2 -variable, and only the normal stress and strain in the x_1 -direction are considered. From (40.13)

$$\dot{\beta}_{11}^p = -V_{231}; \quad (40.51)$$

this shows that all dislocations are of the edge type with the Burgers vector in the x_1 -direction and that their motion takes place in the x_2 -direction, i.e., the plastic domain expands in the x_2 -direction. Although this motion of dislocations is of a climb type, it can be regarded as the sum of two slips of two dislocations whose Burgers vectors are $\pm 45^\circ$ with respect to the x_1 -direction. The sum of the two Burgers vectors is equal to the Burgers vector in the x_1 -direction.

When the edge dislocations with density b_1 are distributed uniformly in a strip domain $c < x_2 < h$ in the infinitely extended material, the dislocation stress is, from (4.13) and (4.14),

$$\begin{aligned} \sigma_{11} &= \frac{-\mu b_1}{2\pi(1-\nu)} \int_{-\infty}^{\infty} dx'_1 \int_c^h \frac{\bar{y}(3\bar{x}^2 + \bar{y}^2)}{(\bar{x}^2 + \bar{y}^2)^2} dx'_2 \\ &= -\frac{\mu b_1}{1-\nu} (h-c) \quad \text{for } x_2 \geq h, \\ &= -\frac{\mu b_1}{1-\nu} (2x_2 - h - c) \quad \text{for } h \geq x_2 \geq c, \\ &= \frac{\mu b_1}{1-\nu} (h-c) \quad \text{for } c \geq x_2, \end{aligned} \quad (40.52)$$

where $\bar{x} = x_1 - x'_1$ and $\bar{y} = x_2 - x'_2$. The other stress components disappear. If the dislocations are distributed in the two strips as shown in Fig. 40.8(a), the stress field becomes

$$\begin{aligned} \sigma_{11} &= \frac{-2\mu b_1}{1-\nu} (x_2 - c) \quad \text{for } h \geq x_2 \geq c, \\ \sigma_{11} &= 0 \quad \text{for } c \geq |x_2|, \\ \sigma_{11} &= \frac{-2\mu b_1}{1-\nu} (x_2 + c) \quad \text{for } -c \geq x_2 \geq -h. \end{aligned} \quad (40.53)$$

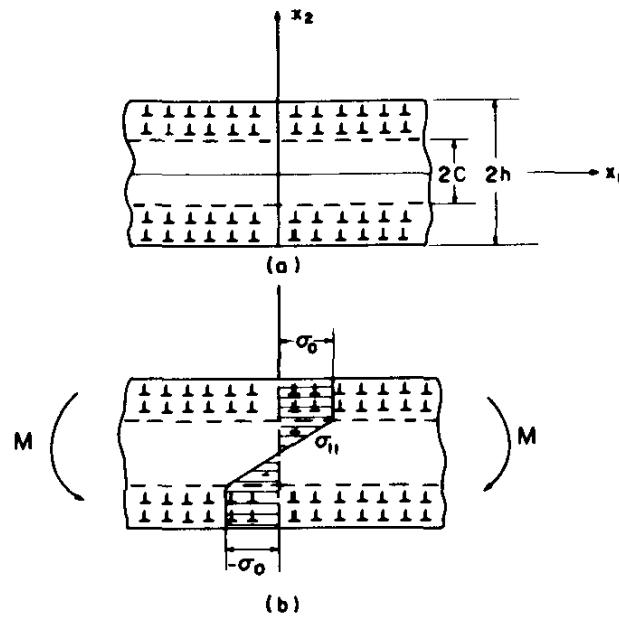


Fig. 40.8. Beam under plastic state.

Although (40.53) has been constructed from the solution for the infinite body, it can be used as the solution of the beam with the plastic domains $h \geq |x_2| \geq c$ if the linear stress

$$\sigma_{11} = M_0 x_2 / I \quad (40.54)$$

is added to (40.53), where

$$\begin{aligned} M_0 &= 2\mu b_1 (h - c)^2 (2h + c) / 3(1 - \nu), \\ I &= 2h^3 / 3. \end{aligned} \quad (40.55)$$

The stress (40.54) is determined such that the sum of (40.53) and (40.54) yields zero moment. This must be true since the beam is free from external tractions.

When the beam is perfectly elasto-plastic with the yield stress σ_0 and is bent by moment M (applied stress Mx_2/I), the total stress must be $\pm\sigma_0$ in the plastic domains, that is,

$$\frac{-2\mu b_1}{1 - \nu} (x_2 \mp c) + \frac{M_0 x_2}{I} + \frac{M x_2}{I} = \pm \sigma_0. \quad (40.56)$$

The above conditions are satisfied when

$$\begin{aligned} M_0/I + M/I &= 2\mu b_1/(1 - \nu), \\ 2\mu b_1 c / (1 - \nu) &= \sigma_0, \quad b_1 = \alpha_{31} = (1 - \nu^2) \sigma_0 / c E. \end{aligned} \quad (40.57)$$

Then, the total stress becomes the well-known distribution as shown in Fig. 40.8(b), and the moment necessary for this plastic state is

$$M = \frac{1}{3} \sigma_0 (3h^2 - c^2) \quad (40.58)$$

which agrees with the classical solution.

The dislocation distribution shown in this example can also be derived from (40.25). From the stress distribution in Fig. 40.8(b), α_{31} becomes zero in the plastic domains, and equals $-\sigma_0/cE$ in the elastic domain. If the uniform distribution σ_0/cE is superimposed everywhere, the resulting distribution becomes identical to the one shown in Fig. 40.8(b). The uniform distribution does not cause any stress field. E is Young's modulus. The factor $(1 - \nu^2)$ in the α_{31} -expression in (40.57) arises from the deviation of the beam theory from the plane strain state.

Similar calculations can be done for the torsion of a solid circular cylinder (Mura 1970). Assuming that all dislocations are of the same sign and parallel to the x_3 -axis (the axis of the cylinder), (40.13) leads to

$$\begin{aligned} \dot{\beta}_{13}^p &= -V_{233}, \\ \dot{\beta}_{23}^p &= V_{133}, \end{aligned} \quad (40.59)$$

or, equivalently, to

$$\dot{\beta}_{\theta 3}^p = V_{r33}. \quad (40.60)$$

This shows that the plastic deformation is caused by the radial motion of α_{33} . In Fig. 40.9 screw dislocations $\alpha_{33} = b_3$ are distributed uniformly in the plastic domain. The dislocation stress is obtained from (4.8). Integrating for this dislocation distribution we have

$$\begin{aligned} \sigma_{\theta z} &= -\mu b_3 (r^2 - c^2)/2r \quad \text{for } c \leq r \leq a, \\ &= 0 \quad \text{for } r \leq c. \end{aligned} \quad (40.61)$$

If the cylinder is free from external tractions, the following stress must be added to (40.61) so that the total twisting moment disappears:

$$\sigma_{\theta z} = M_0 r/J, \quad (40.62)$$

where

$$\begin{aligned} M_0 &= \frac{1}{4} \pi \mu b_3 (a^2 - c^2)^2, \\ J &= \frac{1}{2} \pi a^4. \end{aligned} \quad (40.63)$$

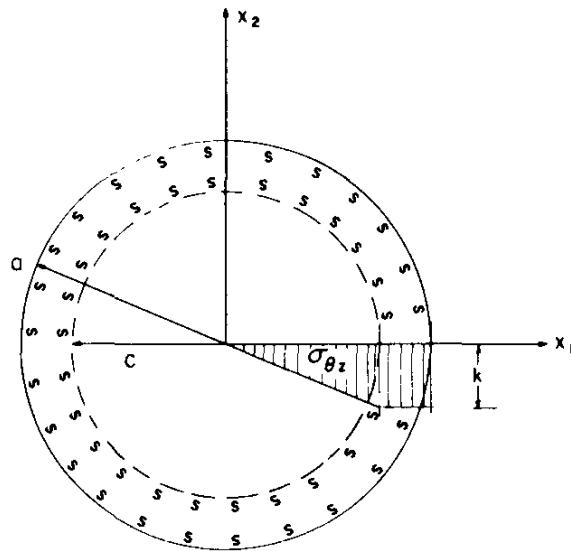


Fig. 40.9 A solid cylinder with radius a under torsion. Screw dislocations are distributed in plastic domain $c \leq r \leq a$.

When the cylinder is a perfectly elasto-plastic material with the yield shear stress k , twisted by moment M , the total stress must be equal to k in $c \leq r \leq a$, that is,

$$-\mu b_3(r - c) + M_0 r/J + Mr/J = k, \quad c \leq r \leq a, \quad (40.64)$$

where (40.61) has been approximated by $-\mu b_3(r - c)$ by assuming $c \approx a \approx r$. The above condition is satisfied when

$$\begin{aligned} M_0/J + M/J &= \mu b_3, \\ \mu b_3 c &= k. \end{aligned} \quad (40.65)$$

The second equation in (40.65) determines the dislocation density b_3 , and the first equation yields M necessary for having plastic domain $c \leq r \leq a$. The total stress distribution under M is shown in Fig. 40.9. According to the elementary strength of materials, the torsion of a circular cylinder is described by

$$\beta_{3\theta}^P = \theta r \quad (40.66)$$

or

$$\begin{aligned} \beta_{32}^P &= -\theta x_2, \\ \beta_{32}^P &= \theta x_1. \end{aligned} \quad (40.66.1)$$

Equation (37.11) leads to the dislocation distribution

$$\alpha_{11} = \alpha_{22} = \theta. \quad (40.67)$$

It can be proved that the dislocation distribution (40.67) is equivalent to $\alpha_{33} = -2\theta$ if the impotent distributions of dislocations are superimposed on (40.67). The impotent distributions are

$$\begin{aligned}\beta_{31}^P &= \theta x_2, \\ \beta_{13}^P &= -\theta x_2,\end{aligned} \quad (40.68)$$

and

$$\begin{aligned}\beta_{32}^P &= -\theta x_1, \\ \beta_{23}^P &= \theta x_1.\end{aligned} \quad (40.69)$$

Then, (40.68) gives

$$\begin{aligned}\alpha_{11} &= -\theta, \\ \alpha_{33} &= -\theta,\end{aligned} \quad (40.70)$$

and (40.69) leads to

$$\begin{aligned}\alpha_{22} &= -\theta, \\ \alpha_{33} &= -\theta.\end{aligned} \quad (40.71)$$

Therefore, the sum of (40.67), (40.70), and (40.71) yields

$$\alpha_{33} = -2\theta \quad (40.72)$$

which is the dislocation distribution assumed after (40.60).

When the screw dislocations distributed along a circle oscillate radially, all dynamic responses vanish. This was found by Mura (1972) and confirmed later by Parnes and Beltzer (1984).

Shioya and Shioiri (1976) have explained the non-uniform yield-zone pattern in twisted mild steel bars with circular cross-sections in terms of continuous distributions of dislocations. Flinn (1970) and Jimma and Kawai (1979) have investigated the deformation mechanism in fabrication processes by continuous distributions of dislocations. Jimma and Masuda (1977) have studied longitudinal and torsional waves in a linearly work-hardening material by using moving dislocations. Smith (1958) has proposed a model of a planar

network of moving dislocations at shock fronts in explosive metals. Ohnami et al. (1972) and Shiozawa and Ohnami (1974) have studied flow stresses of metals and X-ray diffraction by means of the geometrical property of continuous distributions of dislocations. An electron microscopic study of dislocation walls has been done by Yamamoto, Yagi and Honjo (1977). Owen (1971) has assumed the stress of a dislocation dipole to be the work-hardening in the material and has derived the corresponding stress-strain relations of composite materials.

41. Dislocation model for fatigue crack initiation

The initiation of fatigue cracks is one of the most important stages in the fatigue fracture process of metals. A large number of metallographic observations have been carried out to elucidate the micromechanisms responsible for crack initiation. The state of the art is found in the recent excellent review articles by Grosskrentz (1971) and Laird and Duquette (1972). The site of crack initiation varies depending on the microstructure of the material and the applied stresses. Among the possible sites of crack initiation, the persistent slip band is a preferential one for pure, single-phased metals and some poly-phased metals under a low-strain cycling. The cyclic strain is concentrated along the slip band, accompanied by extrusion or intrusion. Laird (1976), Brown (1977), Mughrabi (1980), Neumann (1983) among others have investigated the nature of microscopic mechanisms of persistent slip bands.

Most of the models proposed to account for the formation of extrusions and intrusions, are based upon the Mott assumption (1958) that dislocations move along different paths under forward and reverse loadings. These models, however, cannot explain the most important experimental fact in fatigue: the 'ratcheting' of plastic deformation by a cyclic loading. Lin and Ito (1969) have proposed a model for ratcheting in which a specimen is subjected to a special initial stress state. Zarka, Engel and Inglebert (1980) also have proposed a phenomenological model for the ratcheting. Recently, Tanaka and Mura (1981) have proposed a more reasonable model for ratcheting by considering two adjacent layers of dislocation pile-ups. Each layer has a different sign. Their dislocation dipole model can also yield the Coffin–Manson type law for the crack initiation and the Petch type equation for the grain size dependency of fatigue strength. This section will introduce the theory of Tanaka and Mura.

Let us consider an applied shear stress pattern as shown in Fig. 41.1. Figures 41.2 and 41.3 show models of dislocation pile-ups. Dislocations are piled up on layer I by the loading (forward) force and on layer II by the unloading (reverse) force. The signs of the dislocations are opposite in the two

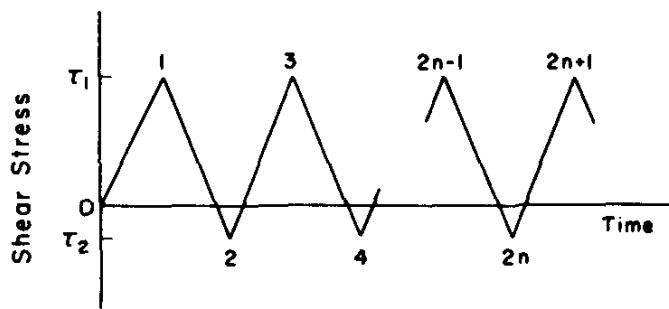


Fig. 41.1. Applied shear stress pattern.

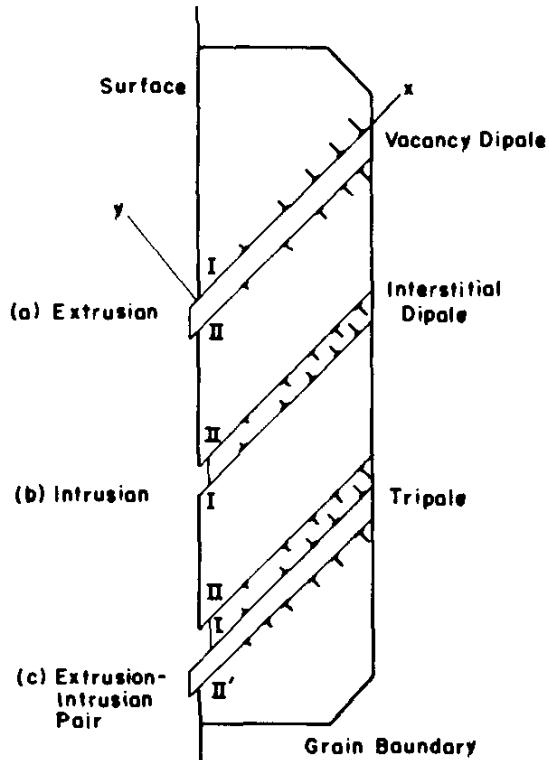


Fig. 41.2. Dislocation distribution in a most favorably oriented grain and the formation of extrusion and intrusion.

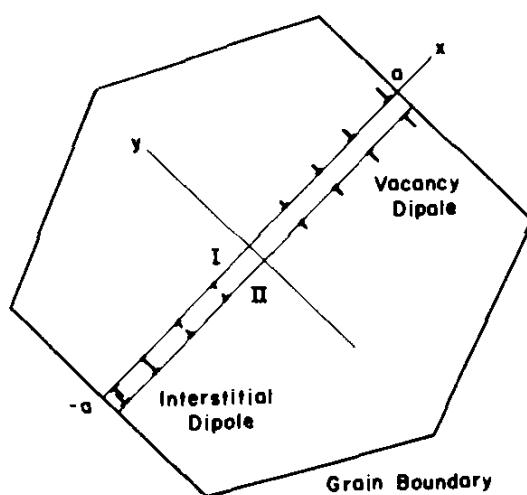


Fig. 41.3. Dislocation distribution in a most favorably oriented grain.

layers. The dislocations in Fig. 41.2 are created at the surface of the specimen and moved to the interior of a grain. The dislocations in Fig. 41.3 are created inside a grain and moved to the grain boundary.

We calculate the plastic strain, number of dislocations, the associated energy, and the stress field caused by the dislocation motion. Under the first loading τ_1 greater than the frictional stress k , the dislocation distribution with density $D_1(x)$ is produced on layer I as shown in Fig. 41.3. (The following analysis is developed for the dislocation pile-up in Fig. 41.3, but the result also applies to the case shown by Fig. 41.2 when the plane $x = 0$ is assumed to be a free surface.) By assuming k to be constant, we express the equilibrium condition of dislocations as

$$\tau_1^D + \tau_1 - k = 0, \quad (41.1)$$

where τ_1^D is the dislocation stress (the back stress) given by

$$\tau_1^D = A \int_{-a}^a \frac{D_1(\xi) d\xi}{x - \xi}. \quad (41.2)$$

The domain of the dislocation distribution is $-a < x < a$, and

$$A = \mu b / 2\pi(1 - \nu), \quad (41.3)$$

where b is the Burgers vector, μ the shear modulus, and ν Poisson's ratio. The solution of (41.1) is found by the method in Appendix 4. When the solution is assumed to be unbounded at $x = \pm a$, it becomes

$$D_1(x) = (\tau_1 - k)x / \pi A(a^2 - x^2)^{1/2}. \quad (41.4)$$

The total number of dislocations existing between $x = 0$ and a is

$$N_1 = \int_0^a D_1(x) dx = (\tau_1 - k)a / \pi A. \quad (41.5)$$

The average plastic strain caused by these dislocations is

$$\gamma_1 = \int_{-a}^a b D_1(x) x dx = (\tau_1 - k)ba^2 / 2A. \quad (41.6)$$

The multiplication of the number of pile-up layers in a unit area by the γ_1 value yields the macroscopic plastic strain. For simplicity, γ_1 is called the plastic strain.

The stored energy of dislocations is

$$U_1 = -\frac{1}{2} \int_{-a}^a \tau_1^D b D_1(x) x \, dx = \frac{1}{2} \gamma_1 (\tau_1 - k). \quad (41.7)$$

When the applied stress is reversed from τ_1 to τ_2 , the dislocations with opposite signs are piled up on layer II as shown in Figs. 41.2 and 41.3. We assume that the dislocations in layer I do not move by the unloading. This assumption of the irreversibility of dislocation motion will be discussed later. By denoting the density of dislocations on layer II by $D_2(x)$ and the back stress due to $D_2(x)$ by τ_2^D , we express the equilibrium condition in layer II as

$$\tau_2^D + \tau_1^D + \tau_2 + k = 0. \quad (41.8)$$

The distance between layers I and II is assumed to be very small compared with the pile-up length. Then, τ_1^D on layer II can be regarded as the same as on layer I. Equations (41.1) and (41.8) yield

$$\tau_2^D - (\Delta\tau - 2k) = 0, \quad (41.9)$$

where $\Delta\tau = \tau_1 - \tau_2$. Only when $\Delta\tau$ is larger than $2k$ can the dislocations on layer II start to move from $x = 0$ and pile up at $x = \pm a$. The dislocation density $D_2(x)$, the total number of dislocations between $x = 0$ and $x = a$ on layer II, and the plastic strain increment γ_2 are obtained as

$$D_2(x) = -(\Delta\tau - 2k)x/\pi A(a^2 - x^2)^{1/2},$$

$$N_2 = -(\Delta\tau - 2k)a/\pi A,$$

$$\gamma_2 = -(\Delta\tau - 2k)ba^2/2A.$$
(41.10)

The stored energy of dislocations $D_2(x)$ becomes

$$U_2 = -\frac{1}{2}\gamma_2(\Delta\tau - 2k). \quad (41.11)$$

The pile-up of opposite dislocations on layer II causes a positive back stress on layer I. This back stress enhances the pile-up of dislocations on layer I during the next loading. Similarly the dislocations on layer I enhance the pile-up of dislocations on layer II during the subsequent unloading. This is the ratcheting mechanism proposed by Tanaka and Mura (1981). After repeated loading and unloading, the extrusion or intrusion is created on the surface of the specimen as shown in Fig. 41.2.

The increment of dislocation $D_k(x)$, the dislocation number N_k , the plastic strain increment γ_k , the back stress increment τ_k^D , and the stored energy U_k at

the k -th step of the loading and unloading processes are obtained in a similar manner. They are

$$\begin{aligned} D_k(x) &= (-1)^{k+1} \Delta D(x), \quad N_k = (-1)^{k+1} \Delta N, \quad \gamma_k = (-1)^{k+1} \Delta \gamma, \\ \tau_k^D &= (-1)^{k+1} (2k - \Delta \tau), \quad U_k = \Delta U, \end{aligned} \quad (41.12)$$

where

$$\begin{aligned} \Delta \tau &= \tau_1 - \tau_2, \\ \Delta D(x) &= (\Delta \tau - 2k)x/\pi A(a^2 - x^2)^{1/2}, \\ \Delta N &= (\Delta \tau - 2k)a/\pi A, \\ \Delta \gamma &= (\Delta \tau - 2k)ba^2/2A, \\ \Delta U &= \frac{1}{2}\Delta \gamma(\Delta \tau - 2k). \end{aligned} \quad (41.13)$$

The index k takes $2n$ at the minimum stress after n cycles and $2n + 1$ at the maximum stress after n cycles. The total values of these quantities obtained in (41.12) or (41.13) are

$$\begin{aligned} D_I(x) &= \sum_{n=0}^n D_{2n+1}(x) = D_1(x) + n\Delta D(x), \\ N_I &= \sum_{n=0}^n N_{2n+1} = N_1 + n\Delta N, \\ U_I &= \sum_{n=0}^n U_{2n+1} = U_1 + n\Delta U, \end{aligned} \quad (41.14)$$

for the dislocation pile-up on layer I, and

$$\begin{aligned} D_{II}(x) &= \sum_{n=1}^n D_{2n}(x) = -n\Delta D(x), \\ N_{II} &= \sum_{n=1}^n N_{2n} = -n\Delta N, \\ U_{II} &= \sum_{n=1}^n U_{2n} = n\Delta U, \end{aligned} \quad (41.15)$$

for the dislocation pile-up on layer II. $D_I(x)$ is written as

$$D_I(x) = T_I x / \pi A (a^2 - x^2)^{1/2}, \quad (41.16)$$

where

$$T_I = \tau_1 - k + n(\Delta\tau - 2k). \quad (41.17)$$

The stress field due to the dislocation distribution $D_I(x)$ is the same as that for the shear crack under shear stress T_I (see Bilby and Eshelby, 1968). The stress intensity factor at the pile-up tip is

$$K_I = T_I (\pi a)^{1/2} = \{ \tau_1 - k + n(\Delta\tau - 2k) \} (\pi a)^{1/2}. \quad (41.18)$$

Similarly, the stress intensity factor at the tip of layer II becomes

$$K_{II} = T_{II} (\pi a)^{1/2} = -n(\Delta\tau - 2k) (\pi a)^{1/2}. \quad (41.19)$$

A tensile stress, built up at the tip between the two layers I and II, could become large enough to create the nucleus of a crack. Since the densities of pile-up dislocations on the two layers are about the same for long-life fatigue, except for the sign, the tensile stress σ_{xx} at $x = a$ and at half the distance between the two layers becomes

$$\sigma_{xx} = \sigma_{xx}^{D_I} + \sigma_{xx}^{D_{II}} = 3\sqrt{a} n(\Delta\tau - 2k) / \sqrt{2h}, \quad (41.20)$$

where h is the distance between the two layers. Embryonic cracks can be formed from this tensile stress as shown in Figs. 41.4 and 41.5. Formation of the intrusion causes the stress concentration and the intrusion can also be a crack embryo. The depth of the intrusion equals the total number of accumulated dislocations which is $N_I = N_{II} = n\Delta N$, multiplied by the Burgers vector.

The condition of the growth of the crack embryo will be treated from the viewpoint of energy balance. We assume that when the stored strain energy due to dislocations accumulated after n cycles becomes equal to the surface energy, the layers of dislocation dipoles are transformed into a free surface. The life of the crack initiation n_c is now defined as the number of stress cycles when the following energy condition is satisfied:

$$U = U_I + U_{II} = 2n_c \Delta U = 4aW_s, \quad (41.21)$$

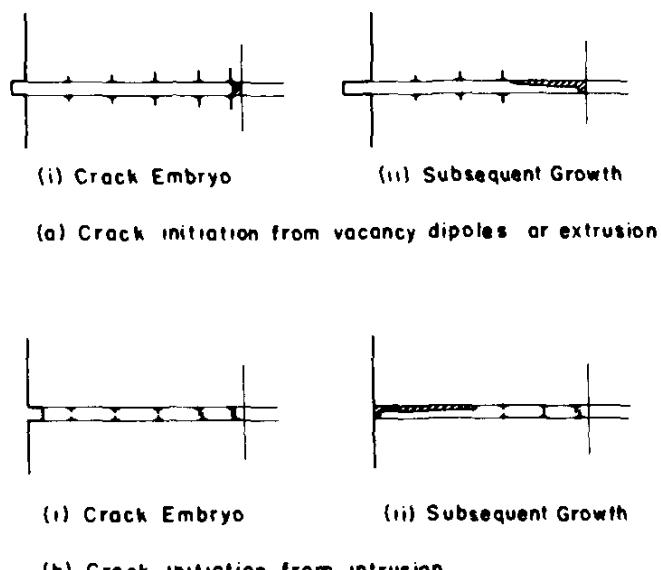


Fig. 41.4. Two types of crack initiation.

where W_s is the specific fracture energy for a unit area. The right-hand side of (41.21) is $2aW_s$ for the pile-up shown in Fig. 41.2. Equation (41.21) leads to

$$\begin{aligned} n_c &= 4aW_s/(\Delta\tau - 2k)\Delta\gamma \\ &= 8AW_s/ba(\Delta\tau - 2k)^2 \\ &= 2bW_s a^3/A(\Delta\gamma)^2. \end{aligned} \tag{41.22}$$

The last result,

$$n_c(\Delta\gamma)^2 = 2bW_s a^3/A, \quad (41.23)$$

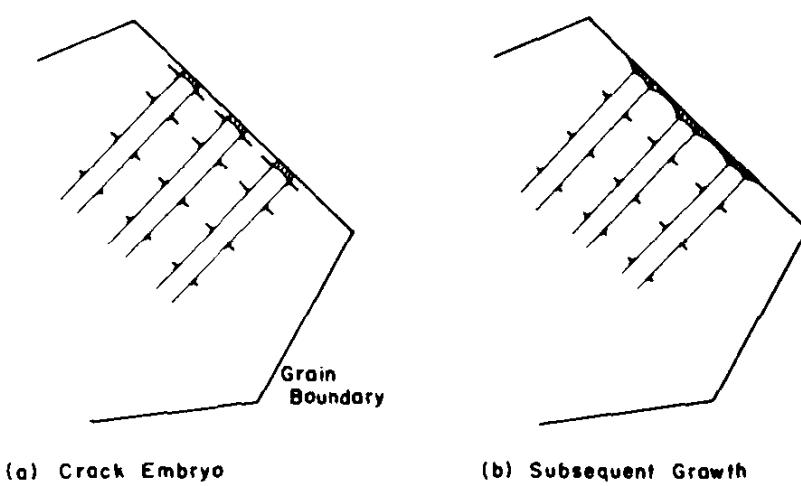


Fig. 41.5. Grain boundary crack initiated by stacked pile-ups of vacancy dipoles.

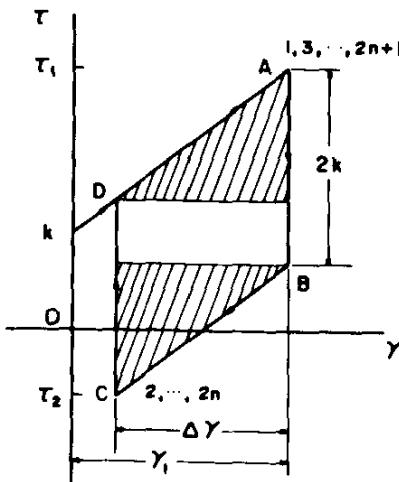


Fig. 41.6. Stress-strain hysteresis loop.

is similar to the Coffin (1954)–Manson (1954) law. They have found the life law for the complete fracture of smooth specimens under high strain cycling. Later, a similar law was confirmed by Lukás et al. (1974) to be valid for low-strain and long-life fatigue. The life up to the initiation of the crack on a grain size order is about fifty percent of the total life of fatigue (see Taira et al. 1979). Therefore, the Coffin–Manson relation seems to be valid for the initiation of cracks in long-life fatigue. The strain energy of dislocations is accumulated in the same amount in each loading and unloading except the first loading. The amount of stored energy does not correspond to the total area of the hysteresis loop, but only to the shaded area shown in Fig. 41.6. The energy corresponding to the remaining area of the loop is the work dissipated against the friction stress. Martin (1961) derived the Coffin–Manson relation by regarding the segment of the plastic work associated with work-hardening as an accumulating damage.

Tomkins (1968) also derived the Coffin–Manson law from a different viewpoint. He assumed that the crack growth per cycle is equal to the product of plastic strain and plastic zone size. Integrating the growth rate equation from an initial crack length to a final length, he obtained the Coffin–Manson law.

The second equation in (41.22) can be written as

$$\Delta\tau = 2k + (8AW_s/bn_c)^{1/2}a^{-1/2} \quad (41.24)$$

which is of the Petch-type equation for constant initial life. The Petch-type relation has been reported for the long-life fatigue of several metals (see Taira et al. 1979, Thompson and Backofen 1971, Ebner and Backofen 1959).

The most striking advantage of the Tanaka and Mura model based on the irreversibility assumption for dislocation motion is that the progress of the

ratcheting of a plastic deformation in the slip band can be calculated in each loading and unloading. Two neighboring layers, where the motion of dislocations with opposite signs takes place, can be regarded as the zone of strain localization akin to the persistent slip bands found in low-strain fatigue. Several experiments have been reported on the slip movement in such bands and most of them indicate a certain irreversibility of plastic deformation. Keith and Gilman (1959) observed in their etch pit study that dislocation movements were irreversible under cyclic stressing except for a small motion of dislocation loops in lithium fluoride crystals. More macroscopically, Charsley and Desvaux (1969), and Desvaux and Charsley (1969) found that a partial reversal of tensile slip steps occurred during compression, but no total reversal was seen in single crystals with wavy and planar slip modes.

The dislocation dipole model has been extended to fatigue crack initiations at inclusions and notches by Tanaka and Mura (1982), Mura and Tanaka (1981). The model also has been used for stress corrosion cracking in high strength steel by Hirose and Mura (1984), and Mura and Hirose (1984).

Recently, Kato et al. (1984) have evaluated the net sum γ_p of plastic strains accumulated after n cycles of loading by use of a random stochastic nature of slips. The possibility distribution function $f(\gamma_p)$ becomes the normal distribution and the expected value of $|\gamma_p|$ is obtained as $\sqrt{n} \Delta\gamma_p$, where n is the cycle number and $\Delta\gamma_p$ is the applied plastic strain amplitude. Their theory reaches a result similar to (41.23), but criticizes the energy expression (41.7).

Material properties and related topics

In this chapter, we demonstrate how inclusion problems are applied to estimate macroscopic mechanical properties of aggregates, i.e. particle bearing materials and polycrystalline materials. In these materials, internal stresses usually develop as a result of plastic deformation which is inherently heterogeneous due to the heterogeneity of the constituent medium. These internal stresses can be calculated in a straightforward manner by using the methods of Chapters 2 and 3. If one finds a way to connect these internal stresses with the applied stress needed to produce the plastic deformation, one can eventually predict the macroscopic plastic properties.

42. Macroscopic average

We begin with the somewhat general properties of the internal stresses of inclusions, not explicitly mentioned in the preceding chapters.

Average of internal stresses

$$\int_D \sigma_{ij} dD = \int_D \sigma_{ik} \delta_{jk} dD \quad (42.1)$$

is rewritten as

$$\int_D \sigma_{ij} dD = \int_D \sigma_{ik} x_{j,k} dD, \quad (42.2)$$

since $x_{j,k} = \delta_{jk}$. Integrating by parts and noting $\sigma_{ik,k} = 0$ in D and $\sigma_{jk} n_k = 0$

on its boundary $|D|$, one can see that the above equation is always equal to zero, that is,

$$\int_D \sigma_{ij} dD = \int_{|D|} \sigma_{ik} x_j n_k dS - \int_D \sigma_{ik,k} x_j dD = 0. \quad (42.3)$$

Note that this is valid without specifying the elastic constants or the origin of the internal stress.

Macroscopic strains

Suppose that the eigenstrain ϵ_{ij}^* is given to an aggregate Ω of inclusions, producing an internal stress field σ_{ij} and strain field ϵ_{ij} . Let us calculate the average of ϵ_{ij} over the whole body, $\langle \epsilon_{ij} \rangle_D$, defined by

$$\langle \epsilon_{ij} \rangle_D = (1/V) \int_D \epsilon_{ij} dD = (1/V) \int_D (e_{ij} + \epsilon_{ij}^*) dD, \quad (42.4)$$

where V is the volume of the body and e_{ij} the elastic strain. If the elastic constants are uniform, (42.4) is rewritten as

$$\langle \epsilon_{ij} \rangle_D = (1/V) C_{ijkl}^{-1} \int_D \sigma_{kl} dD + (1/V) \int_D \epsilon_{ij}^* dD, \quad (42.5)$$

which becomes

$$\langle \epsilon_{ij} \rangle_D = (1/V) \int_D \epsilon_{ij}^* dD \quad (42.6)$$

from (42.3). Since ϵ_{ij}^* is defined only in Ω , the integral domain in (42.6) can be changed to Ω . By defining the volume fraction f of the inclusions as Ω/V , (42.6) is written

$$\langle \epsilon_{ij} \rangle_D = f \langle \epsilon_{ij}^* \rangle_\Omega = f \epsilon_{ij}^*. \quad (42.7)$$

The last result applies when ϵ_{ij}^* is uniform in Ω . Expression (42.6) can be used to describe macroscopic strains induced by dislocation motions.

When the inclusions have elastic moduli different from those of the matrix, the inhomogeneous inclusions are simulated by the equivalent inclusions

defined in Section 22. That is, when the eigenstrain of the inclusion is ϵ_{kl}^P , we have

$$\sigma_{ij} = C_{ijkl}(\epsilon_{kl} - \epsilon_{kl}^{**}) = C_{ijkl}^*(\epsilon_{kl} - \epsilon_{kl}^P) \quad \text{in } \Omega, \quad (42.8)$$

where

$$\epsilon_{kl}^{**} = \epsilon_{kl}^P + \epsilon_{kl}^*, \quad (42.9)$$

$$\epsilon_{kl} = S_{klmn}\epsilon_{mn}^{**}, \quad (42.10)$$

and C_{ijkl}^* is the elastic moduli of the inhomogeneous inclusions. Then (42.6) and (42.7) are written, respectively, as

$$\langle \epsilon_{ij} \rangle_D = (1/V) \int_D (C_{ijkl}^{-1} \sigma_{kl} + \epsilon_{ij}^{**}) dD = (1/V) \int_{\Omega} \epsilon_{ij}^{**} dD \quad (42.11)$$

and

$$\langle \epsilon_{ij} \rangle_D = f \epsilon_{ij}^{**}. \quad (42.12)$$

Hence, the macroscopic strain is directly related to the eigenstrain of the equivalent inclusion.

In the above procedure, it is assumed that the stress due to a particular inclusion is not disturbed by other inclusions. This approximation is permitted when $f \ll 1$. However, it is a poor approximation when f is large.

Tanaka-Mori's theorem

For simplicity, let us consider a homogeneous body containing an inclusion Ω not necessarily ellipsoidal (see Fig. 42.1). At this stage of discussion, the shape of the inclusion is not specified. Give to the inclusion a uniform eigenstrain ϵ_{ij}^* . Let the internal stress due to ϵ_{ij}^* be σ_{ij}^∞ , implying that σ_{ij}^∞ is calculated by assuming the body to be infinitely extended. σ_{ij}^∞ outside Ω is safely given by

$$\sigma_{ij}^\infty(x) = -C_{ijkl} \int_{\Omega} C_{pqmn} \epsilon_{mn}^* G_{kp,qj}(x - x') dx' \quad (42.13)$$

from (3.25), where $\epsilon_{mn}^* = 0$ for points exterior to Ω . Let us integrate σ_{ij}^∞ over

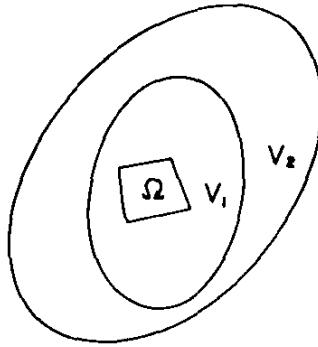


Fig. 42.1. Inclusion Ω with an arbitrary shape in an infinite domain.

the domain $V_2 - V_1$ as shown in Fig. 42.1. V_2 and V_1 are ellipsoidal and $V_2 \supset V_1 \supset \Omega$.

$$\begin{aligned} \int_{V_2 - V_1} \sigma_{ij}^{\infty} dx &= -C_{ijkl} \int_{V_2} dx \int_{\Omega} C_{pqmn} \epsilon_{mn}^* G_{kp,ql}(x - x') dx' \\ &\quad + C_{ijkl} \int_{V_1} dx \int_{\Omega} C_{pqmn} \epsilon_{mn}^* G_{kp,ql}(x - x') dx'. \end{aligned} \quad (42.14)$$

When we change the order of integration in (42.14),

$$\begin{aligned} \int_{V_2 - V_1} \sigma_{ij}^{\infty} dx &= - \int_{\Omega} dx' \left\{ C_{ijkl} \int_{V_2} C_{pqmn} \epsilon_{mn}^* G_{kp,ql}(x - x') dx \right\} \\ &\quad + \int_{\Omega} dx' \left\{ C_{ijkl} \int_{V_1} C_{pqmn} \epsilon_{mn}^* G_{kp,ql}(x - x') dx \right\}. \end{aligned} \quad (42.15)$$

Since

$$- \int_{V_2} C_{pqmn} \epsilon_{mn}^* G_{kp,ql}(x - x') dx$$

gives the total distortion at x' when the uniform eigenstrain is defined in V_2 and we have assumed V_2 to be an ellipsoid, this distortion is independent of x' and is determined by the shape of V_2 , as long as $x' \in V_2$. This condition is satisfied since $x' \in \Omega \subset V_2$. A similar remark applies to the second term in (42.15). Thus, (42.15) is rewritten as

$$\int_{V_2 - V_1} \sigma_{ij}^{\infty} dx = \Omega C_{ijkl} [S_{klmn}(V_2) \epsilon_{mn}^* - S_{klmn}(V_1) \epsilon_{mn}^*], \quad (42.16)$$

[see (11.1) and (11.15)], where Ω is the volume of the inclusion, and $S_{klmn}(V_2)$ and $S_{klmn}(V_1)$ are calculated by (17.14) as

$$S_{klmn} = (1/8\pi) C_{pqmn} \int_{S^2} \bar{\xi}_q \{ \bar{\xi}_l N_{kp}(\bar{\xi}) + \bar{\xi}_k N_{lp}(\bar{\xi}) \} D^{-1}(\bar{\xi}) dS(\bar{\xi}). \quad (42.17)$$

Equation (42.16) leads to interesting conclusions: The volume integral of σ_{ij}^∞ over $V_2 - V_1$ is proportional to Ω ; it is independent of the absolute position and size of V_2 and V_1 as long as $V_2 \supset V_1$; it depends only on the shape of V_2 and V_1 ; and it vanishes when V_2 and V_1 are similar in shape and have the same orientation, that is, when $a_1/b_1 = a_2/b_2 = a_3/b_3$, and

$$\begin{aligned} V_2: & x_1^2/a_1^2 + x_2^2/a_2^2 + x_3^2/a_3^2 \leq 1, \\ V_1: & (x_1 - x_1^0)^2/b_1^2 + (x_2 - x_2^0)^2/b_2^2 + (x_3 - x_3^0)^2/b_3^2 \leq 1. \end{aligned} \quad (42.18)$$

The last conclusion (42.16) is valid even when the eigenstrain in Ω is non-uniform and the elastic moduli of Ω differ from those of the matrix.

When Ω is an ellipsoidal inclusion we can have the limiting situation, $V_1 \rightarrow \Omega$. Then

$$\int_{V_2 - \Omega} \sigma_{ij}^\infty dx = \Omega C_{ijkl} [S_{klmn}(V_2) \epsilon_{mn}^* - \overbrace{S_{klmn}(\Omega) \epsilon_{mn}^*}], \quad V_2 \supset \Omega. \quad (42.19)$$

If V_2 and Ω are similar in shape, (42.19) vanishes,

$$\int_{V_2 - \Omega} \sigma_{ij}^\infty dx = 0, \quad V_2 \supset \Omega. \quad (42.20)$$

Similarly, for the corresponding strain,

$$\int_{V_2 - \Omega} \epsilon_{ij}^\infty dx = 0, \quad V_2 \supset \Omega. \quad (42.20.1)$$

The above properties of the integration in the domain around an ellipsoidal inclusion have been obtained by Tanaka and Mori (1972). Brown and Stobbs (1971) have noted (42.20) by using explicit form of σ_{ij}^∞ due to a spherical inclusion in an isotropic medium. It should be noted that (42.20) is valid for an inhomogeneous inclusion with arbitrary and non-uniform eigenstrains:

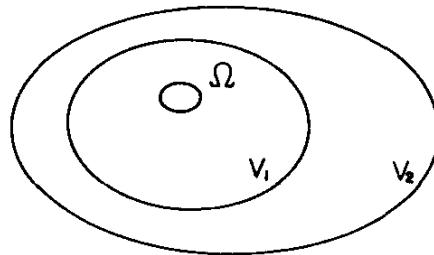


Fig 42.2. Domains Ω , V_1 and V_2 have the same shape and orientation.

(42.15) becomes zero even if $\epsilon_{mn}^* = \epsilon_{mn}^*(x')$ when V_2 and V_1 have the same ellipsoidal shape and orientation (see Fig. 42.2).

Image stresses

Let us consider a finite body D containing an ellipsoidal inclusion Ω with eigenstrain ϵ_{ij}^* . In order to utilize the property (42.20), we assume that D is similar to Ω in terms of its orientation and shape. This assumption does not restrict the problem when Ω is very small compared with D .

Let us find the internal stress σ_{ij} due to Ω . In the first approximation, σ_{ij} is taken as σ_{ij}^∞ defined previously. The integration of σ_{ij}^∞ over the whole body can be written as

$$\int_D \sigma_{ij}^\infty \, dD = \int_\Omega \sigma_{ij}^\infty \, dD + \int_{D - \Omega} \sigma_{ij}^\infty \, dD. \quad (42.21)$$

Because of (42.20), the second term vanishes; thus

$$\int_D \sigma_{ij}^\infty \, dD = \int_\Omega \sigma_{ij}^\infty \, dD. \quad (42.22)$$

Equation (42.22) shows that σ_{ij}^∞ does not possess the property (42.3) of internal stresses in a finite body and is based on the fact that σ_{ij}^∞ does not satisfy the traction free condition $\sigma_{ij}^\infty n_j = 0$ on $|D|$. Thus, one must add the correction term σ_{ij}^I to σ_{ij}^∞ to obtain the exact solution for the internal stress σ_{ij} ,

$$\sigma_{ij} = \sigma_{ij}^\infty + \sigma_{ij}^I, \quad (42.23)$$

where $\sigma_{ij,j}^I = 0$ in D and $\sigma_{ij}^I n_j + \sigma_{ij}^\infty n_j = 0$ on $|D|$. σ_{ij}^I is called the image stress. The determination of this image stress is generally a difficult problem.

However, one can estimate its average value from (42.3), (42.23), and (42.22). This average is

$$\langle \sigma_{ij}^I \rangle_D = (1/V) \int_D \sigma_{ij}^I \, dD = - (1/V) \int_D \sigma_{ij}^\infty \, dD = - (1/V) \int_\Omega \sigma_{ij}^\infty \, dD, \quad (42.24)$$

where V is the volume of D .

When the eigenstrain ϵ_{ij}^* is uniform in Ω , (42.24) becomes

$$\langle \sigma_{ij}^I \rangle_D = - (\Omega/V) \sigma_{ij}^\infty(\Omega) = - (\Omega/V) C_{ijkl} \{ S_{klmn}(\Omega) \epsilon_{mn}^* - \epsilon_{kl}^* \}. \quad (42.25)$$

$\langle \sigma_{ij}^I \rangle_D$ is small compared to the magnitude of σ_{ij}^∞ . Nevertheless, when σ_{ij}^I and σ_{ij}^∞ are integrated over the whole body, they attain values of the same order.

We can obtain a result similar to (42.25) for strain. Let

$$\epsilon_{ij} = \epsilon_{ij}^\infty + \epsilon_{ij}^I, \quad (42.26)$$

where ϵ_{ij}^I is the image strain due to the presence of the free surface. Equation (42.20.1) leads to

$$\langle \epsilon_{ij}^\infty \rangle_D = (1/V) \int_D \epsilon_{ij}^\infty \, dD = (1/V) \int_\Omega \epsilon_{ij}^\infty \, dD = (\Omega/V) S_{ijkl}(\Omega) \epsilon_{kl}^*, \quad (42.27)$$

while from (42.6) we have

$$\langle \epsilon_{ij} \rangle_D = \langle \epsilon_{ij}^\infty \rangle_D + \langle \epsilon_{ij}^I \rangle_D = (\Omega/V) \epsilon_{ij}^*. \quad (42.28)$$

Therefore, combining these results, we have

$$\langle \epsilon_{ij}^I \rangle_D = - (\Omega/V) \{ S_{ijkl}(\Omega) \epsilon_{kl}^* - \epsilon_{ij}^* \} \quad (42.29)$$

which can be obtained directly from (42.25) by inspection.

The importance of the image stress and strain has been discussed by Eshelby (1956) in connection with macroscopic deformations and average changes in the lattice constants.

Random distribution of inclusions—Mori and Tanaka's theory

Seldom is only a single precipitate formed in a material. For example, when the volume fraction of precipitates is 0.001 and their size is 100 Å, some 10^{15} precipitates are present in a unit volume. Even if the stress field due to an

individual precipitate were known, including the image stress term, it would take many years to add up the stresses due to all these precipitates. Moreover, the positions of individual precipitates are generally not known.

However, the average stress in the matrix and in the precipitates (inclusions) as well as the elastic energy are easily calculated when the precipitates are randomly distributed. Sometimes all these quantities are needed to discuss the macroscopic properties.

For simplicity, homogeneous ellipsoidal inclusions, with uniform eigen-strain ϵ_{ij}^* , are treated first. Let us formally define the average stresses in the matrix as $\langle \sigma_{ij} \rangle_M$ and those in the inclusions as $\langle \sigma_{ij} \rangle_I$. Equation (42.3) can then be expressed as

$$f\langle \sigma_{ij} \rangle_I + (1 - f)\langle \sigma_{ij} \rangle_M = 0, \quad (42.30)$$

where f is the volume fraction of the inclusions. Now let us introduce a new single inclusion. The number of inclusions is so large that this addition does not affect f . Since random distribution of the inclusions is assumed, we will insert the new inclusion randomly in the matrix. Then, within the inclusion (or within any inclusion) the stress becomes

$$\sigma_{ij} = \sigma_{ij}^\infty + \langle \sigma_{ij} \rangle_M, \quad (42.31)$$

where σ_{ij}^∞ is the stress calculated for a single inclusion present in an infinite medium:

$$\sigma_{ij}^\infty = C_{ijkl}(S_{klmn}\epsilon_{mn}^* - \epsilon_{kl}^*), \quad (42.32)$$

and $\langle \sigma_{ij} \rangle_M$ is the stress already present in the matrix. Since the new inclusion can be inserted at any place in the matrix, the average of σ_{ij} must be equal to $\langle \sigma_{ij} \rangle_I$. The average of (42.31) over all the positions available for inclusions is

$$\langle \sigma_{ij} \rangle_I = \sigma_{ij}^\infty + \langle \sigma_{ij} \rangle_M. \quad (42.33)$$

Substituting (42.33) into (42.30), the following equations are obtained:

$$\langle \sigma_{ij} \rangle_M = -f\sigma_{ij}^\infty, \quad (42.34)$$

$$\langle \sigma_{ij} \rangle_I = \sigma_{ij}^\infty - f\sigma_{ij}^\infty. \quad (42.35)$$

It should be noted that $-f\sigma_{ij}^\infty$ in the above equations is the average of the sum of σ_{ij}^∞ and σ_{ij}^I caused by all the inclusions.

The elastic strain energy per unit volume of the body is written as

$$\begin{aligned} W^* &= -\frac{1}{2}\epsilon_{ij}^*\int_{\Omega} \sigma_{ij} \, dD/V = -\frac{1}{2}\epsilon_{ij}^* \langle \sigma_{ij} \rangle_I f \\ &= -\frac{1}{2}f(1-f)\sigma_{ij}^\infty \epsilon_{ij}^* \end{aligned} \quad (42.36)$$

from (42.35) and (25.2) or (13.3). It is extremely useful that the average stress and the elastic energy can be written in such simple forms when the inclusions are randomly distributed. This fact has been found by Mori and Tanaka (1973). Brown (1973) has considered the same problem and has obtained a slightly different result.

A parallel discussion can be given in the situation when the eigenstrain is not uniform in an inclusion. In such a case, (42.33) can be written as

$$\langle \sigma_{ij} \rangle_I = \langle \sigma_{ij}^\infty \rangle_I + \langle \sigma_{ij} \rangle_M \quad (42.37)$$

with

$$\langle \sigma_{ij}^\infty \rangle_I = (1/\Omega) \int_{\Omega} \sigma_{ij}^\infty \, dD, \quad (42.37.1)$$

where σ_{ij}^∞ is the stress for a single inclusion present in an infinite medium and Ω is the volume of the inclusion. Then, (42.30) and (42.37) lead to

$$\langle \sigma_{ij} \rangle_M = -f\langle \sigma_{ij}^\infty \rangle_I, \quad (42.38)$$

$$\langle \sigma_{ij} \rangle_I = (1-f)\langle \sigma_{ij}^\infty \rangle_I. \quad (42.38.1)$$

From (42.31) the stress inside a representative inclusion can be written as

$$\sigma_{ij} = \sigma_{ij}^\infty - f\langle \sigma_{ij}^\infty \rangle_I. \quad (42.39)$$

When the inclusions are inhomogeneous with eigenstrains ϵ_{ij}^P , the treatment may be slightly modified. Let us assume, for simplicity, that ϵ_{ij}^P is uniform in the inhomogeneous inclusions. The equation (42.30) is still valid. The average stress in the matrix can be written as

$$\langle \sigma_{ij} \rangle_M = C_{ijkl} e_{kl}^0, \quad (42.40)$$

where e_{kl}^0 is to be determined. Let us introduce a new single inhomogeneous

inclusion into the matrix, as before. Then, the stress in this inclusion can be determined by the equivalent inclusion method (22.13),

$$\sigma_{ij} = C_{ijkl} (e_{kl}^0 + S_{klmn} \epsilon_{mn}^{**} - \epsilon_{kl}^{**}) = C_{ijkl}^* (e_{kl}^0 + S_{klmn} \epsilon_{mn}^{**} - \epsilon_{kl}^p), \quad (42.41)$$

where C_{ijkl}^* are the elastic moduli of the inclusions. The stresses σ_{ij} in (42.41) can be assumed to be $\langle \sigma_{ij} \rangle_I$. Then, (42.30) leads to

$$fC_{ijkl} (e_{kl}^0 + S_{klmn} \epsilon_{mn}^{**} - \epsilon_{kl}^{**}) + (1-f)C_{ijkl} e_{kl}^0 = 0 \quad (42.42)$$

or

$$fC_{ijkl} (S_{klmn} \epsilon_{mn}^{**} - \epsilon_{kl}^{**}) + C_{ijkl} e_{kl}^0 = 0. \quad (42.43)$$

Equations (42.41) and (42.43) are necessary and sufficient to determine 12 unknowns e_{ij}^0 and ϵ_{ij}^{**} .

If $f \ll 1$, the following approximation is admissible. Setting $\epsilon_{ij}^{**} = \epsilon_{ij}^* + \epsilon_{ij}'$,

$$(\sigma_{ij}^\infty \equiv) C_{ijkl} (S_{klmn} \epsilon_{mn}' - \epsilon_{kl}') = C_{ijkl}^* (S_{klmn} \epsilon_{mn}' - \epsilon_{kl}^p) \quad (42.44)$$

and

$$(\sigma_{ij}' \equiv) C_{ijkl} (e_{kl}^0 + S_{klmn} \epsilon_{mn}^* - \epsilon_{kl}^*) = C_{ijkl}^* (e_{kl}^0 + S_{klmn} \epsilon_{mn}^*), \quad (42.45)$$

where ϵ_{ij}' and ϵ_{ij}^* are the respective equivalent eigenstrains for ϵ_{ij}^p and e_{ij}^0 .

It is easy to solve (42.44) for ϵ_{ij}' under given ϵ_{ij}^p . We assume that $\epsilon_{ij}^* \ll \epsilon_{ij}'$. Then, (42.43) leads to

$$\langle \sigma_{ij} \rangle_M = C_{ijkl} e_{kl}^0 \approx -f\sigma_{ij}^\infty. \quad (42.46)$$

Substituting e_{ij}^0 obtained from (42.46) into (42.45), we can derive approximate values of ϵ_{ij}' . Then, (42.41) can be written as

$$\langle \sigma_{ij} \rangle_I = \sigma_{ij}^\infty + \sigma_{ij}'. \quad (42.47)$$

It is apparent that when $f \ll 1$, (42.38) is also a good approximation for the situation where ϵ_{ij}^p is not uniform in an inclusion. The discussion of this problem is similar to that developed above.

43. Work-hardening of dispersion hardened alloys

Metallurgists invented a dispersion strengthened alloy whose matrix is a plastically soft and ductile material and throughout which particles of plastically strong second phases are dispersed. Sometimes these particles are called nonshearable, implying that the particles cannot be cut by the crystal dislocations (glide dislocations). These particles thus hinder the dislocation motion and thereby raise the yield stress. Macroscopic yielding in a dispersion strengthened alloy is explained by the Orowan mechanism (see Ashby 1966); dislocations bowing out between particles. The critical stress for this bowing out can be calculated by using the results (36.15) or (36.25). For recent developments of this problem, the reader is referred to Bacon et al. (1973) and Scattergood and Bacon (1975). When dislocations bow out, dislocation loops, called Orowan loops, are left around the particles. The internal stresses from these Orowan loops affect the motion of the dislocations. Macroscopically, this effect is manifested by work-hardening. One approach used to estimate work-hardening is the balancing of forces acting on the glide dislocations bowing out between two neighboring particles, where one force comes from the applied stress, one from the glide dislocation itself, and one from the internal stresses caused by the Orowan loops. An approximate method using this assumption has been employed by (Hart 1972). However, this type of approach depends critically on the model adopted, the position of the loops around a particle, as well as the number of the particles considered. A particular loop may hinder or assist the motion of a dislocation, depending on the relative position of the loop and the dislocation. It is, therefore, desirable to take another approach, with which the estimation of work-hardening can be made independently of the choice of a detailed model. The following sections are devoted to this purpose and are mainly applications of the analysis of the elastic fields produced by an ellipsoidal inclusion.

Work-hardening in simple shear

Let us consider the situation where the matrix deforms plastically but the inclusions do not. For simplicity, the shape of the inclusions is assumed to be spherical and the plastic deformation in the matrix is assumed to be uniform. As an example, the plastic deformation is assumed to occur in simple shear; the slip plane is thus perpendicular to the x_3 -direction, while the slip direction is parallel to the x_1 -direction. Let the plastic strain in the matrix be $\epsilon_{31}^P = \frac{1}{2}\gamma_p$. (γ_p is the glide strain in the slip system.) Superimpose $\epsilon_{31}^P = -\frac{1}{2}\gamma_p$ uniformly throughout the specimen. Since the uniform plastic strain over the entire body does not produce internal stress, the internal stress induced by the prescribed

plastic deformation in the matrix is calculated by considering the situation where

$$\begin{aligned}\epsilon_{31}^p &= 0, && \text{in the matrix,} \\ \epsilon_{31}^p &= -\frac{1}{2}\gamma_p, && \text{in the inclusions.}\end{aligned}\quad (43.1)$$

From (22.13a) and (22.19), the stress in an inclusion is given by

$$\sigma_{31}^B = \mu\mu^*(1 - 2S)\gamma_p/g \quad (43.2)$$

when the inclusion is isolated in an infinite matrix. Here μ and μ^* are the shear moduli of the matrix and the inclusion, respectively,

$$g = 2(\mu^* - \mu)S + \mu, \quad (43.3)$$

and

$$S = S_{3131} = (4 - 5\nu)/\{15(1 - \nu)\}, \quad (43.4)$$

where ν is the Poisson ratio of the matrix. From (43.1), (43.2) and (25.3), the elastic strain energy due to an inclusion is calculated as

$$W^* = \frac{1}{2}V_0\mu\mu^*(1 - 2S)\gamma_p^2/g, \quad (43.5)$$

where V_0 is the volume of the inclusion. When many inclusions coexist with a volume fraction f , the elastic strain energy per unit volume can be approximated by

$$\bar{W}^* = \frac{1}{2}f\mu\mu^*(1 - 2S)\gamma_p^2/g, \quad (43.6)$$

provided f is much smaller than 1. The otherwise uniform applied stress, $\sigma_{31}^0 = \sigma^0$, is disturbed by the inhomogeneity in the elastic constant of the inclusions. The applied stress in the inclusion is calculated as

$$\sigma_{31}^A = \sigma^0\{1 + (\mu^* - \mu)(1 - 2S)/g\} \quad (43.7)$$

from (22.7). Thus, the change in the external potential per unit volume \bar{V}^* is given by (25.40),

$$\bar{V}^* = -\sigma^0\gamma_p + f\sigma^0\{1 + (\mu^* - \mu)(1 - 2S)/g\}\gamma_p. \quad (43.8)$$

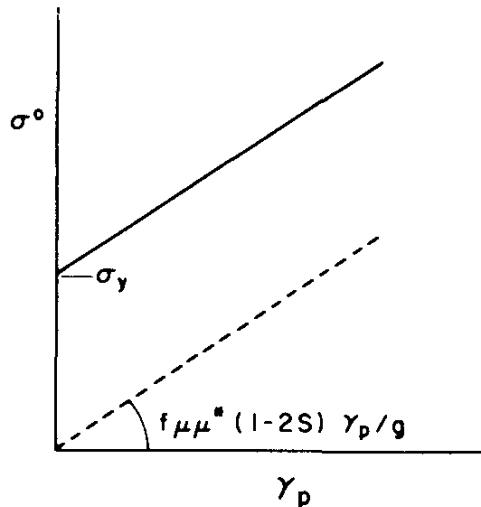


Fig. 43.1. Theoretical lines of linear work hardening due to particles with volume fraction f

The Gibbs free energy (25.41) per unit volume of the specimen, $\Delta\bar{W}$, is the sum of \bar{W}^* and \bar{V}^* ,

$$\Delta\bar{W} = \frac{1}{2}f\mu\mu^*(1 - 2S)\gamma_p^2/g - \sigma^0\gamma_p + f\sigma^0\{1 + (\mu^* - \mu)(1 - 2S)/g\}\gamma_p. \quad (43.9)$$

The specimen is in stable equilibrium when $\partial\Delta\bar{W}/\partial\gamma_p = 0$ and $\partial^2\Delta\bar{W}/\partial\gamma_p^2 > 0$. That is, the stable state is achieved by

$$\sigma^0\{(1 - f) - f(\mu^* - \mu)(1 - 2S)/g\} = f\mu\mu^*(1 - 2S)\gamma_p/g. \quad (43.10)$$

Since $f \ll 1$ is assumed, (43.10) can be written as

$$\sigma^0 = f\mu\mu^*(1 - 2S)\gamma_p/g, \quad (43.11)$$

which represents the equation relating the applied stress and the plastic strain (the constitutive equation) as is schematically shown by the dotted line in Fig. 43.1.

Since a finite stress is needed to start the plastic deformation, one has to raise the curve to the solid line,

$$\sigma^0 = \sigma_y + f\mu\mu^*(1 - 2S)\gamma_p/g. \quad (43.12)$$

The physical reason for this is as follows: When the increase in γ_p is $\delta\gamma_p$, the increase in \bar{W}^* is $f\mu\mu^*(1 - 2S)\gamma_p\delta\gamma_p/M$, and the work done by the applied stress (negative of the external potential increase, δU) is $\sigma^0\delta\gamma_p$. At the same time, the increase in γ_p in the matrix accompanies the energy dissipation

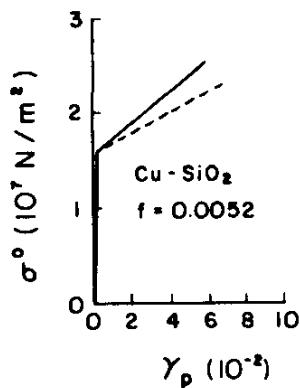


Fig. 43.2 The solid line is the experimentally observed stress-strain curve and the dotted line is calculated from (43.11) (after Tokushige 1976)

$\delta D^* \approx \sigma_y \delta \gamma_p$. The energy conservation law, $\delta U = \delta \bar{W}^* + \delta D^*$, yields the solid line in Fig. 43.2, which indicates linear work-hardening after yielding. Yielding by the Orowan mechanism is typical of energy dissipation. Thus, σ_y can be identified as the critical stress for the operation of the Orowan mechanism. The above method of treating the plastic deformation of dispersion-strengthened alloys appeared in the paper by Tanaka and Mori (1970). Experimentally, linear work-hardening is usually observed in a dispersion-hardened alloy when the strain is small and the deformation occurs at a low temperature, where the diffusion effect can be ignored. An example (Tokushige, 1976) is given in Fig. 43.2, in which the solid line is the experimentally observed stress-strain curve of a Cu–SiO₂ single crystal and the dotted line is calculated from (43.11). The SiO₂ particle is known to have a spherical shape in this alloy. Thus, $S = (4 - 5\nu)/\{15(1 - \nu)\}$. Here $f = 0.0052$, $\mu = 4.61 \times 10^{10}$ N/m², $\mu^* = 3.13 \times 10^{10}$ N/m² and $\nu = 0.33$. The calculation accounts for 65% of the observed hardening.

It is easy to extend the above calculation to other shapes of inclusions as well as other modes of plastic deformation such as uniaxial deformation as shown already by (25.58) (Tanaka and Mori, 1970). The elastic anisotropy of the matrix and the inclusions can also be taken into account (Lin et al., 1973). The anisotropy in plastic deformation induced by the asymmetrical arrangement of inclusions in a composite material has been discussed with an approach similar to the above method (Tanaka et al., 1973).

Brown and Stobbs (1971), and Brown and Clarke (1975) have taken a different approach to derive the hardening rate, a method essentially identical to (43.11), although their paper contains some misleading interpretations. These have been corrected later by Mori and Tanaka (1973). From (42.46) and (43.2), the average internal stress in the matrix is given as

$$\langle \sigma_{31} \rangle_M = -f\mu\mu^*(1 - 2S)\gamma_p/g, \quad (43.13)$$

after the matrix has been plastically deformed by γ_p . Thus, to continue the plastic deformation, one has to raise the applied stress by

$$\Delta\sigma_{31}^0 = -\langle\sigma_{31}\rangle_M \quad (43.14)$$

to overcome the internal stress. Note that (43.14) is identical to (43.11). Both the stress fields due to the inclusions in the infinite matrix and the image stresses due to the free surface effect contribute to $\langle\sigma_{31}\rangle_M$. $-\langle\sigma_{ij}\rangle_M$ is sometimes called the back stress. It is clear that $\langle\sigma_{ij}\rangle_M$ hinders the progress of plastic deformation, and aids the plastic flow in the reverse direction. Of course, one can see this from the preceding energy consideration. This property should be manifested in the Bauschinger effect. Conversely, the existence and magnitude of the back stress can be examined from the measurement of the Bauschinger effect. Some studies have been done along this line (Atkinson et al., 1974, Gould et al., 1974, and Mori and Narita, 1975). The magnitude of the back stress, $-\langle\sigma_{31}\rangle_M$, determined from the difference of the flow curves between forward and reverse strainings, is well accounted for by (43.13) when strain is small.

Let us examine (43.10) and (43.11) further. These equations account for the interaction among inclusions and for the free surface effect. The procedure mentioned in the preceding section, (42.40) through (42.47), can be used to improve these approximations. Then, the average internal stress in an inclusion is evaluated as

$$\langle\sigma_{31}\rangle_I = \mu\mu^*(1-2S)\gamma_p\{(1-f) - f(\mu^* - \mu)(1-2S)/g\}g, \quad (43.15)$$

and \bar{W}^* now is

$$\bar{W}^* = \frac{1}{2}f\mu\mu^*(1-2S)\gamma_p^2\{(1-f) - f(\mu^* - \mu)(1-2S)/g\}/g. \quad (43.16)$$

Writing $\Delta\bar{W}$ with (43.8) and (43.16), we obtain (43.11) instead of (43.10), as an equation satisfying the equilibrium condition.

Dislocations around an inclusion

Here we discuss the dislocation distribution around an inclusion when $\epsilon_{31}^p = \frac{1}{2}\gamma_p$ is uniform in the matrix and $\epsilon_{ij}^p = 0$ in the inclusion. For simplicity, assume

$$\begin{cases} \beta_{31}^p = \gamma_p & \text{in the matrix,} \\ \beta_{ji}^p = 0 & \text{in the inclusions.} \end{cases} \quad (43.17)$$

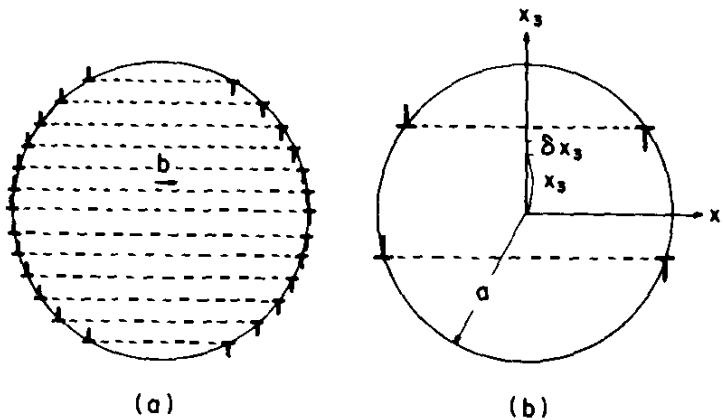


Fig. 43.3. Surface dislocations around a particle.

Since there is a jump at β_{ji}^P at the matrix-inclusion interface, the surface dislocations α_{hi} defined by (37.16) exist at the interface. Let n be the unit normal vector at the interface directed toward the matrix. Then,

$$\begin{aligned}\alpha_{11} &= -\gamma_p n_2, \\ \alpha_{21} &= \gamma_p n_1.\end{aligned}\tag{43.18}$$

All other components are zero. This means that the infinitesimal glide loops, which have the Burgers vectors parallel to the loop planes, are present at the interface (see Fig. 37.2 and Fig. 43.3(a)). The total Burgers vector of the surface dislocations crossing the unit length of the line element perpendicular to n and lying on the interface, becomes

$$B_1 = \alpha_{11} n_1 + \alpha_{21} n_2 = \gamma_p (1 - n_3^2)^{1/2},\tag{43.19}$$

since

$$\begin{aligned}n_1 &= -n_2 / (n_1^2 + n_2^2)^{1/2}, \\ n_2 &= n_1 / (n_1^2 + n_2^2)^{1/2}, \\ n_3 &= 0, \quad n_1^2 + n_2^2 + n_3^2 = 1.\end{aligned}\tag{43.20}$$

This result (43.19) implies that the density of the dotted lines in Fig. 43.3(a) is a constant equal to γ_p along the x_3 -axis.

With the dislocation distribution defined previously, another physical explanation can be given for the work-hardening derived in this section. Physically, the plastic deformation is caused by the motion of the crystal

dislocations with a finite Burgers vector. Thus, the situation described by (43.17) does not generally occur literally, say, at the atomic dimension. All that (43.17) implies in the case of crystal plasticity is that slip occurs randomly, and macroscopically uniformly. Here, “macroscopic” may be defined in such a manner that uniformity is discussed in comparison with the inclusion dimension. For example, (43.19) shows that the number n of the (crystal) Orowan loops surrounding a particle is, on the average, given by

$$n = 2a\gamma_p/b, \quad (43.21)$$

and that these loops are randomly distributed around the particle. Here, b is the magnitude of the crystal dislocations Burgers vector responsible for slip, and a is the radius of the spherical inclusion. This situation is schematically shown in Fig. 43.3(b). When the inclusions are randomly distributed, it is apparent that the position of the Orowan loops around a particular inclusion is also random. Let $N dx_3$ be the number of the Orowan loops per unit volume, situated between x_3 and $x_3 + dx_3$ with respect to the center of an inclusion (see Fig. 43.3). N is called the Orowan loop density. Then, uniformity of the plastic deformation in the matrix means that N is constant related to γ_p by

$$\int_{-a}^a N dx_3 = nN_p \quad (43.22)$$

or

$$\gamma_p = Nb/N_p, \quad (43.23)$$

where N_p is the number of inclusions per unit volume.

Now let us examine the stress field due to these Orowan loops. Consider the average of the internal stress in the matrix. Since there are many loops, this average remains almost unchanged when a loop is transferred from one particle to another. We repeat this until some particles are uniformly covered by the loops (other particles may not have any loop), then, divide these loops into the loops with an infinitesimal Burgers vector and redistribute these infinitesimal loops to all the particles uniformly. It is clear that, in the final stage, the infinitesimal loops are distributed uniformly as the surface dislocations described in (43.18) or (43.21), and that the average internal stress in the matrix is the same as that in the original distribution of the Orowan loops with a finite Burgers vector. In short, the work-hardening discussed in this section is that brought by the average internal stress due to the Orowan loops. For the discrete dislocation approach, it is difficult to conduct a quantitative calcula-

tion. Such difficulty can be avoided by using the stress field of an inclusion, as has been suggested by Mori and Tokushige (1977).

When the situation arises that the Orowan loop distribution density N is not constant but is a function of x_3 , the average internal stress in the matrix may still be calculated by the above procedure. The average internal stress in the matrix can be reproduced by giving the eigenstrain

$$\epsilon_{31}^p = -N(x_3)b/2N_p \quad (43.24)$$

to every particle. Then, the solutions of Sections 19 and 20 can be used together with (42.37.1). Such a situation will be discussed in the next section.

Uniformity of plastic deformation

One of the reasons that the straightforward evaluation of work-hardening in dispersion strengthened materials is possible is the assumption of uniform plastic deformation throughout the matrix. In the context of the approach taken in the present section, this assumption is justified, as shown below.

For simplicity, let us assume $f \ll 1$ and that the plastic strain in the matrix ϵ_{31}^p varies only along the x_3 -direction, that is,

$$\epsilon_{31}^p = \frac{1}{2}\gamma_p(x_3). \quad (43.25)$$

Neglecting the interaction among the inclusions and the disturbance of the applied stress $\sigma_{31}^0 = \sigma^0$ due to the inhomogeneity of the inclusions, one can write the Gibbs free energy per unit volume of the specimen as

$$\bar{W} = \frac{1}{2} \int_{D_3} fA\mu \{ \gamma_p(x_3) \}^2 dx_3 - \int_{D_3} \sigma^0 \gamma_p(x_3) dx_3 \quad (43.26)$$

with

$$A = \mu^*(1 - 2S)/g. \quad (43.27)$$

Here, the energy dissipation is not considered and D_3 indicates the integral domain with respect to x_3 . The first term in (43.26) represents the elastic strain energy (see (43.6)). The second term arises from the external potential. It is seen that (43.26) takes a minimum when

$$fA\mu \gamma_p(x_3) = \sigma^0. \quad (43.28)$$

That is, the stable state is achieved when the specimen is uniformly covered by the plastic domain in which the matrix deforms plastically with constant γ_p , determined by σ^0 in (43.28). As mentioned before, uniform slip cannot occur in a literal sense, since crystalline materials deform plastically by movement of crystal dislocations. Therefore, equation (43.28) should be interpreted in such a manner that the slip lines, each of which is produced by a single dislocation movement, cover the specimen uniformly, i.e., the slip line spacing becomes as small as possible. With this conclusion and the consideration of the dislocation configuration around the inclusion discussed previously, the procedure to evaluate the work-hardening is justified. The above procedure and conclusions are identical to those given by Mori and Mura (1976).

44. Diffusional relaxation of internal and external stresses

Let us consider the elastic deformation of an infinite body due to σ_{31}^0 . When the body contains an inhomogeneity, the stress field is disturbed. When the inhomogeneous particle is harder than the matrix, the interface boundary of the matrix (dotted line) and the particle (solid line) deform as shown in Fig. 44.1, if the two domains deform freely without any constraint from the surroundings. The internal stress (stress disturbance) can be interpreted as the eigenstress caused by the eigenstrain corresponding to the misfit (the difference between the dotted domain and solid domain in Fig. 44.1). If interfacial diffusion takes place on the particle surface, mass must be transported from region I to region V. The misfit would therefore disappear, and the internal stress and elastic energy would decrease; this is diffusional relaxation. Similarly, diffusional relaxation takes place when the matrix is subjected to plastic deformation but the inclusion is not.

In this section, some experimental evidence of diffusional relaxation will be mentioned. Next, the relaxation rate will be discussed and the creep deforma-

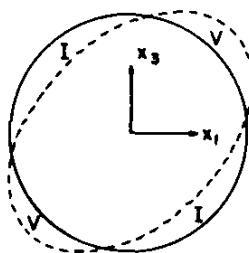


Fig. 44.1 When an inhomogeneous particle is harder than a matrix, the interface boundary of the matrix (dotted line) and the particle (solid line) deform as shown here by σ_{31}^0 if no constraint exists from the surroundings (%).

tion will be considered as a balance between the accumulation and the relaxation of the internal stress. The diffusional relaxation of the external stress will be also discussed.

Relaxation of the internal stress in a plastically deformed dispersion strengthened alloy

Suppose that plastic deformation $\epsilon_{31}^p = \frac{1}{2}\gamma_p$ occurs only in the matrix. The internal stress in this state is caused by the eigenstrain given in (43.1), which can be understood from Fig. 44.1. If a complete relaxation occurs by a diffusional process which transports mass from region I to region V in Fig. 44.1, then the eigenstrain in the inclusion would be reduced by

$$\Delta\epsilon_{31}^p = \frac{1}{2}\gamma_p \quad (44.1)$$

and the resulting eigenstrain in the inclusion would become zero. In that case, work-hardening is lost by the amount given by (43.11); the flow stress decreases correspondingly. Figure 44.2 shows the flow stress decrease of Cu–SiO₂ crystals ($f = 0.0052$) work-hardened at 77°K for 20 min. annealing at 473°K (Mori and Tokushige 1977). The dotted line is drawn according to (43.11). Except for the high strain region, the agreement between the prediction and the observation is good. Metallurgically, the deviation at the high strain region indicates that some relaxation processes which do not need diffusion have occurred.

The diffusional relaxation mentioned in this section can be further checked. The additional eigenstrain, (44.1), should be observed by the macroscopic deformation. By using (42.8), (42.12) and (22.19), the macroscopic strain increment $\langle\epsilon_{31}\rangle_D$ due to $\Delta\epsilon_{31}^p$ in the inclusions can be calculated as

$$\langle\epsilon_{31}\rangle_D = f\epsilon_{31}^{**} \quad (44.2)$$

with

$$\epsilon_{31}^{**} = \mu^* \Delta\epsilon_{31}^p / g, \quad (44.3)$$

where g is defined in (43.3). From (43.11) and (44.1) through (44.3), the loss of the work hardening, $\Delta\sigma^0$, is related to the macroscopic strain observed in relaxation annealing. It becomes

$$\Delta\sigma^0 = 2\mu(1 - 2S)\langle\epsilon_{31}\rangle_D. \quad (44.4)$$

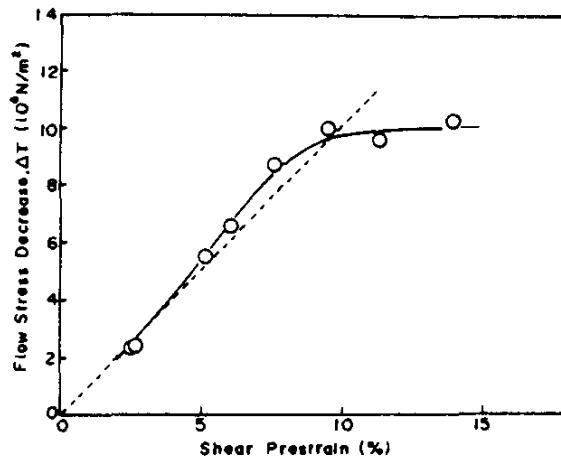


Fig. 44.2. The flow stress decreases by diffusion after prestraining (after Mori and Tokushige 1977).

This has been tested in Cu–SiO₂ crystals as shown in Fig. 44.3 (Mori and Narita 1975). The specimens were prestrained and annealed. The change in flow stress, $\Delta\sigma^0$, and the change in specimen length (corresponding to $\langle\epsilon_{31}\rangle_D$) due to annealing were measured. We assume that the annealing causes $\Delta\epsilon_{31}^P$ in (44.3). The theoretical values from (44.4) and the corresponding measured values are plotted by the filled circles and the triangles, respectively. These are in good agreement. The open circles represent the change in the back stress, $-\langle\sigma_{31}\rangle_M$, due to annealing. These changes were determined by Bauschinger tests before and after annealing. $-\langle\sigma_{31}\rangle_M$ is calculated as half of the difference of the flow stresses between forward and reverse straining. The changes in $-\langle\sigma_{31}\rangle_M$ also compare well with $\Delta\sigma^0$. The saturation tendency of $\Delta\sigma^0$ with prestraining is consistent with Fig. 44.2. Since the specimens were considered to have annealed fully, the change in back stress by annealing is equal to the back stress present before annealing. The saturation of the back stresses at high prestrains in Fig. 44.3 agrees with an observation by Atkinson et al. (1974).

Diffusional relaxation process, climb rate of an Orowan loop

There are three types of diffusion: volume, interfacial and pipe diffusion. The last one is caused by mass transport along the core of a dislocation, where diffusion is fast with a low activation energy because of the severely disturbed atomic arrangement. In this section we employ the pipe diffusion as a cause of climb of the Orowan loops. The interfacial diffusion will be examined in connection with stress relaxation in later sections.

The climb process of the Orowan loop is explained by Fig. 44.4. If atoms flow from the left-hand side of an Orowan loop to the right-hand side through

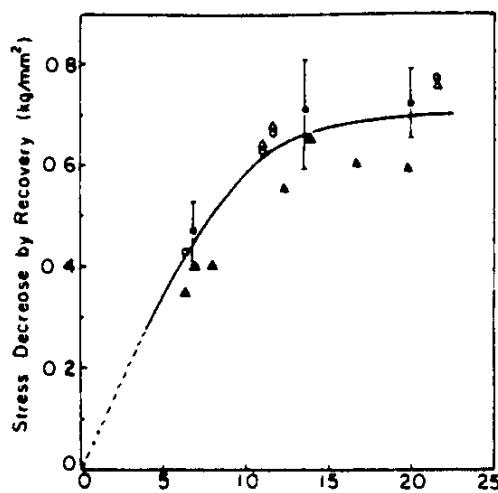


Fig. 44.3 The decrease in the back stress due to the recovery against the prestrain (after Mori and Narita 1975). The axis of abscissa is shear prestrain (%)

its own pipe, the Orowan loop will climb up. Let us calculate the number of atoms, δN , diffusing when an Orowan loop with the Burgers vector \mathbf{b} climbs from x_3 to $x_3 + \delta x_3$. The particles are assumed to be spherical with radius a . Since the thickness of an extra plane defined by the edge component of a dislocation is expressed by the magnitude of $\mathbf{v} \times \mathbf{b} = (b \sin \theta)$ (see Fig. 44.5), where \mathbf{v} is the unit vector along the dislocation, δN is calculated as

$$\delta N = (1/\Omega) \delta x_3 \int_0^\pi b \sin \theta (a^2 - x_3^2)^{1/2} d\theta = 2b(a^2 - x_3^2)^{1/2} \delta x_3 / \Omega, \quad (44.5)$$

where Ω is the atomic volume. The elastic energy of the Orowan loop W^* is

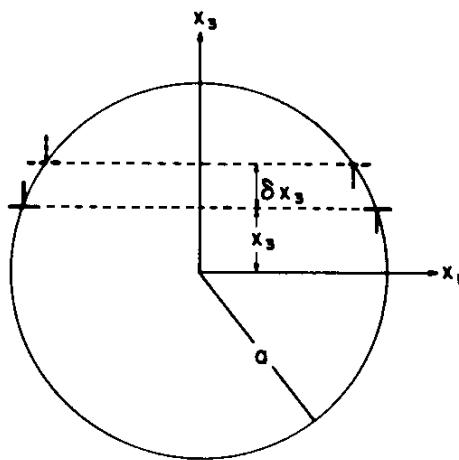


Fig. 44.4. The climb process of the Orowan loop.

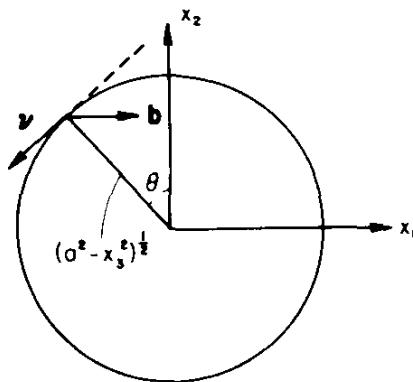


Fig. 44.5. Explanation for equation (44.5).

given by

$$W^* = \alpha \mu b^2 2\pi (a^2 - x_3^2)^{1/2} \quad (44.6)$$

(see, for example, Barnett 1976). Here, α is a constant of the order of unity and μ is the shear modulus. Thus, the change in the elastic energy δW^* is

$$\delta W^* = -2\pi\alpha\mu b^2 x_3 \delta x_3 / (a^2 - x_3^2)^{1/2} \quad (44.7)$$

when the loop climbs up by δx_3 . The diffusion distance of the atoms, l , is approximated to be a constant given by

$$l = \pi (a^2 - x_3^2)^{1/2} \quad (44.8)$$

which is the average over the possible travelling distance. One can define the force F acting on an atom as $-(\delta W^* / l \delta N)$. Thus,

$$F = \alpha \mu b \Omega x_3 (a^2 - x_3^2)^{-3/2}. \quad (44.9)$$

The Einstein relation gives the drift velocity v of an atom as

$$v = \alpha \mu b \Omega x_3 (a^2 - x_3^2)^{-3/2} D_p / kT, \quad (44.10)$$

where D_p is the pipe diffusion constant along the dislocation, k is the Boltzmann constant, and T is the temperature. Let us define by h^2 the cross section of the dislocation pipe. During the time interval δt , atoms $\delta N = 2h^2 v \delta t / \Omega$ flow and consequently the Orowan loop climbs by δx_3 related to δN by (44.5). Thus, the climb rate, $V = dx_3/dt$, of the Orowan loops is given by

$$V = \frac{dx_3}{dt} = Ax_3 (a^2 - x_3^2)^{-2} \quad (44.11)$$

with

$$A = \alpha\mu\Omega h^2 D_p / kT. \quad (44.12)$$

The above treatment of the climb rate of an Orowan loop is developed by Mori and Tokushige (1977). The treatment can be easily extended to the case when the inclusions are ellipsoidal (see Okabe and Mori 1979). Hazzledine and Hirsch (1974) have taken a different approach to analyze the climb process of the Orowan loops. Sato et al. (1984) have calculated the change of elastic distortion caused by annealing of Orowan loop and compared with experiments.

Suppose that the matrix deforms with a strain rate of $\dot{\epsilon}_{31}^p = \frac{1}{2}\dot{\gamma}_p$. If slip occurs uniformly, from (42.23) the Orowan loop density distribution N increases by $\dot{\gamma}_p N_p / b$ per unit time, N_p being the number of inclusions per unit volume. At the same time, the loop density decreases with a rate of $-\partial(NV)/\partial x_3$. Thus, in total, one has a differential equation,

$$\frac{\partial N}{\partial t} = \dot{\gamma}_p N_p / b - \frac{\partial(NV)}{\partial x_3}. \quad (44.13)$$

When $\dot{\gamma}_p = 0$,

$$\frac{\partial N}{\partial t} = -\frac{\partial(NV)}{\partial x_3} \quad (44.14)$$

corresponds to annealing. It can be shown that the solution of (44.14) with the initial condition of $N(x_3, 0) = N_0$ is

$$N(x_3, t) = N_0 \frac{z(a^2 - x_3^2)^2}{x_3(a^2 - z^2)^2}, \quad (44.15)$$

where z is the root of

$$At = a^4 \log(x_3/z) - \frac{1}{4}(x_3^2 - z^2)(4a^2 - x_3^2 - z^2). \quad (44.16)$$

Mori and Mura (1978) have solved (44.16) numerically to obtain $N(x_3, t)$ which may be approximated by $N = N_0(m_0 + m_2 x_3^2/a^2)$ if proper constants m_0 and m_2 are chosen. N_0 is related to the prestrain Y_p which has induced work-hardening.

From (43.24), one can express the eigenstrain ϵ_{31}^p needed to calculate the back stress $-\langle \sigma_{31} \rangle_M$ in a polynomial form. Thus, by using the result of

Section 19 and (22.13), one can calculate σ_{31}^∞ , by which $-\langle \sigma_{31} \rangle_M$ is obtained from (42.38) by assuming $f \ll 1$. Mori and Mura (1978) have numerically calculated the back stress decrease of a work-hardened Cu–SiO₂ alloy and compared it to the experimentally observed softening on annealing. Although the calculation reasonably accounts for the observation, it predicts the temperature range at which significant softening would occur to be narrower than the observed range. This has been corrected by Mori and Osawa (1979) who have taken the distribution of the particle size in an actual alloy into consideration. It is to be noted that the climb rate of the loop and thus the loop distribution brought about the relaxation are sensitive to the particle size.

Recovery creep of a dispersion strengthened alloy

If the Orowan loop density distribution is in balance between loss due to climb, and gain due to the accumulation by plastic deformation, one would expect stationary creep. The Orowan loop density distribution corresponding to a stationary creep rate $\dot{\gamma}_p$ is obtained from

$$\frac{\partial(NV)}{\partial x_3} = \dot{\gamma}_p N_p/b, \quad (44.17)$$

since $\partial N / \partial t = 0$ in (44.13). Since V is given by (44.11), it is clear that the following equation satisfies (44.17):

$$N = \dot{\gamma}_p (N_p/b) (a^2 - x_3^2)^2 / A \quad (44.18)$$

with A given by (44.12). From (43.24) and (44.18), the eigenstrain given to a particle for calculation of $\langle \sigma_{ij} \rangle_M$ is written as

$$\epsilon_{31}^p = -\dot{\gamma}_p (a^2 - x_3^2)^2 / 2A \quad (44.19)$$

which is in the form of (19.1). Thus, using the results of Section 19 (Asaro and Barnett 1975, Mura and Kinoshita 1978) and the equivalency condition developed in Section 22, one finds the stress in the inclusion as

$$\sigma_{31}^\infty = \dot{\gamma}_p a^4 (B_0 + B_2^{pq} x_p x_q / a^2 + B_4^{pqrs} x_p x_q x_r x_s / a^4) / 2A, \quad (44.20)$$

when the inclusion is in an infinite matrix. The coefficients B_0 , B_2^{pq} and B_4^{pqrs} are all computable and expressed in terms of the elastic constants of the

matrix and the inclusion. Thus, by (42.38) and (42.37.1) one obtains the average of the internal stress in the matrix as

$$\langle \sigma_{31} \rangle_M = -\mu f \dot{\gamma}_p a^4 (A_0 - 2A_2 + A_4)/2A, \quad (44.21)$$

where A_0 , A_2 and A_4 are given by Okabe et al. (1980). Therefore, the applied stress $\sigma_{31}^0 = \sigma^0$ needed to maintain stationary creep deformation is given by

$$\sigma^0 = \sigma^D + \mu f \dot{\gamma}_p a^4 (A_0 - 2A_2 + A_4)/2A, \quad (44.22)$$

where σ^D is the flow stress of the matrix (in absence of any particle).

The experimental observations relevant to (44.22), such as the existence of the stationary creep and the linear dependence of $\dot{\gamma}_p$ on σ^0 have recently been made (Mori and Osawa 1979, Okabe and Mori 1979, and Okabe et al. 1980). Figure 44.6 shows the observed stationary creep rate plotted against the applied stress in a Cu-SiO₂ alloy (single crystals, $f = 0.005$ [Okabe et al. 1980]). In a certain range of σ^0 , $\dot{\gamma}_p$ is linearly related to σ^0 , in accordance with (44.22). One can also examine the particle size dependence of creep rates. The

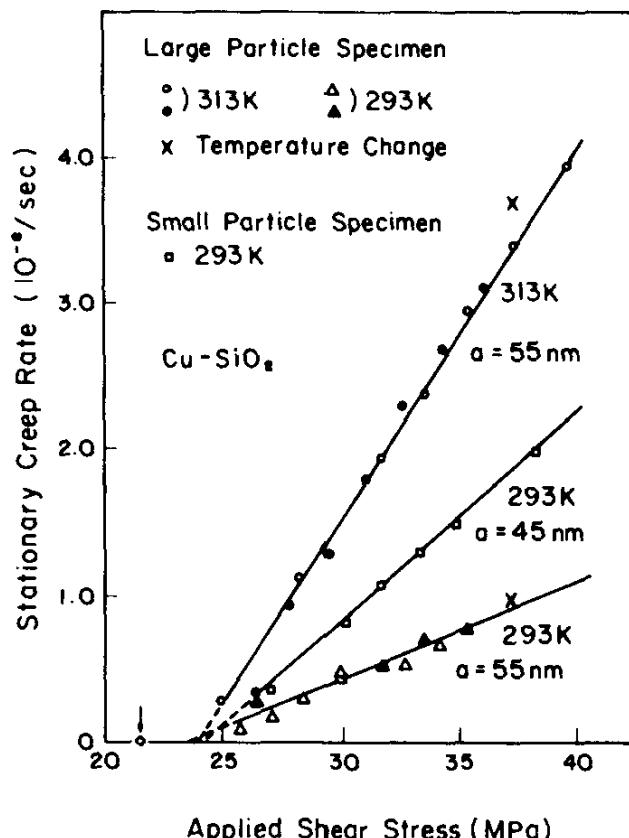


Fig. 44.6 Theoretical lines and experimental values of the stationary creep with parameters of particle sizes and testing temperatures.

ratio of the slope of the creep rate for the specimen with $a = 45$ nm to that for the specimen with $a = 55$ nm is obtained as 2.17, which is in excellent agreement with $(55)^4/(45)^4 = 2.23$, as required by (44.22). Finally, let us compare the calculation with the absolute magnitude of the slope in Fig. 44.6. By conducting numerical calculations, one obtains

$$\sigma^0 = \sigma^D + 0.5838\mu f\dot{\gamma}_p a^4/2A. \quad (44.23)$$

Here, μ is the shear modulus of the matrix (Cu, 4.61×10^{10} N/m²). In the calculation, Poisson's ratio of Cu has been taken to be 0.33, and the shear modulus and Poisson's ratio of SiO₂, 3.13×10^{10} N/m² and 0.18, respectively. By assuming $\alpha = \frac{1}{2}$, $\Omega = 11.8 \times 10^{-30}$ m², $h^2 = b^2$, $b = 2.56 \times 10^{-10}$ m, $D_p = b^2 \times 10^{12} \exp(-Q/kT)/s$, and $Q = 0.56$ eV, one obtains, from (44.23),

$$\dot{\gamma}_p/(\sigma^0 - \sigma^D) = 1.09 \times 10^{-7}/Mp_a s, \quad (44.24)$$

for $a = 55$ nm at $T = 293^\circ\text{K}$. The activation energy Q has been determined from Fig. 44.6. The experimental value of the slope corresponding to this situation is about $0.66 \times 10^{-7}/Mp_a s$. This argument is satisfactory. It has been similarly found that (44.22) accounts for the observations of stationary creep in Al-Si crystals (Mori and Osawa 1979). Okabe and Mori (1979) have extended the above analysis to the situation of rod shaped inclusions in conjunction with the experiment on a Cu-mullite alloy. Again, in this case a satisfactory agreement is found between the calculation and the observation.

Interfacial diffusional relaxation

Relaxation of the internal stress induced by plastic deformation in a material with plastically strong inclusions has been discussed on the basis of the diffusion of atoms through the cores of the dislocations at the matrix-inclusion interface. This is an adequate picture, since the existence of these dislocations at the interface is responsible for the internal stress. However, if the stress is brought about by some other reason, it is better to seek another path of diffusion such as interfacial diffusion. Interfacial diffusion especially applies in the situation where an external stress is present in a specimen which has a so-called incoherent inclusion. The origin of the stress is independent of the situation at the interface and the atomic arrangement at the matrix-inclusion interface is in disorder, so the diffusion along the interface is presumably fast. Even if an interface is described by an array of dislocations, such as a tilt or twist grain boundary, and atoms are considered to diffuse through pipes of these dislocations, the diffusional process is adequately treated as interfacial

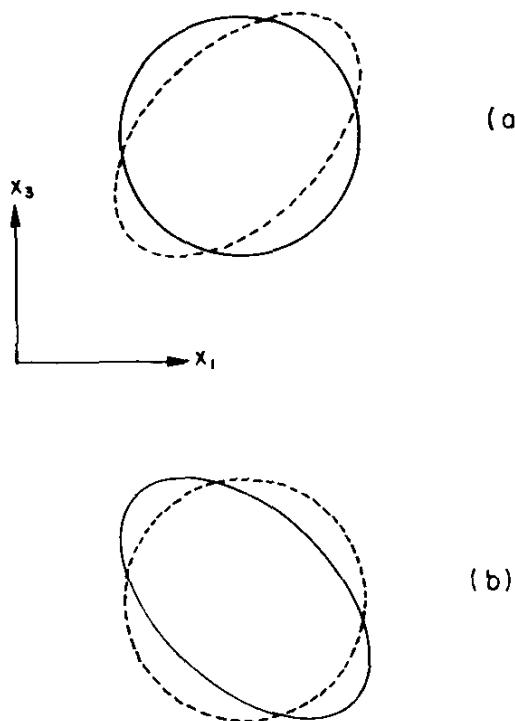


Fig. 44.7. Change of the misfit due to the interfacial diffusion.

diffusion. In the following, we will discuss the relaxation of an external stress due to diffusion at the matrix-inclusion interface.

For simplicity, a cylindrical inclusion of radius a lying parallel to the x_2 axis is considered. The matrix and the inclusion are elastically isotropic with the respective shear moduli of μ and μ^* . The Poisson ratio of the matrix is ν . Let the applied stress far from the inclusion be $\sigma_{31}^0 = \sigma^0$. Let atomic diffusion take place along the matrix-inclusion interface. After diffusion, the resulting stress field can be calculated by considering a uniform eigenstrain of $\Delta\epsilon_{31}^P$ in the inclusion. $\Delta\epsilon_{31}^P$ is a misfit strain caused by either an inclusion geometry change or a matrix geometry change due to the interfacial diffusion. These changes are illustrated by Figs. 44.7(a) and 44.7(b), both of which give the same misfit $\Delta\epsilon_{31}^P$. The dotted line in Fig. 44.7(a) is the shape of the inclusion in the stress free state, while the solid line in the figure is the void in the matrix after the inclusion is removed. This is the situation when the diffusing species are the material constituting the inclusion. Figure 44.7(b) applies to the situation when the diffusing species are the matrix material.

The eigenstress $\Delta\sigma_{ij}$ caused by $\Delta\epsilon_{ij}^P$ in a cylinder is calculated from (22.11) as

$$\Delta\sigma_{31} = -2\mu\mu^*(1 - 2S) \Delta\epsilon_{31}^P/g, \quad (44.25)$$

where

$$S = S_{3131} = (3 - 4\nu) / \{8(1 - \nu)\} \quad (44.26)$$

and

$$g = 2(\mu^* - \mu)S + \mu. \quad (44.27)$$

The increase in the elastic strain energy, ΔW^* , is given by (25.31) ($\sigma_{ij}^B = \Delta\sigma_{31}$):

$$\Delta W^* = -\frac{1}{2}\Delta\sigma_{ij} \Delta\epsilon_{ij}^P V_0 = 2\mu\mu^*(1 - 2S)(\Delta\epsilon_{31}^P)^2 V_0/g, \quad (44.28)$$

where V_0 is the volume of the inclusion per unit length, πa^2 . The applied stress in the inclusion σ_{ij}^A is calculated as

$$\sigma_{31}^A = \sigma^0\mu^*/g. \quad (44.29)$$

Thus, the change in the external potential due to $\Delta\epsilon_{31}^P$ is obtained from (25.32) as

$$\Delta V = -\sigma_{ij}^A \Delta\epsilon_{ij}^P V_0 = -2\sigma^0\mu^*\Delta\epsilon_{31}^P V_0/g. \quad (44.30)$$

The total energy change or the Gibbs free energy change ΔW due to $\Delta\epsilon_{31}^P$ is the sum of (44.28) and (44.30),

$$\Delta W = 2\mu\mu^*(1 - 2S)(\Delta\epsilon_{31}^P)^2 V_0/g - 2\sigma^0\mu^*\Delta\epsilon_{31}^P V_0/g. \quad (44.31)$$

Thus, the misfit strain $\overline{\Delta\epsilon_{31}^P}$ which minimizes the Gibbs free energy is found to be

$$\overline{\Delta\epsilon_{31}^P} = \sigma^0/\{2\mu(1 - 2S)\}. \quad (44.32)$$

In this situation, the internal stress $\overline{\Delta\sigma_{31}}$ is given by

$$\overline{\Delta\sigma_{31}} = -\sigma^0\mu^*/g \quad (44.33)$$

from (44.25). Thus, the total stress in the inclusion vanishes,

$$\sigma_{31}^A + \overline{\Delta\sigma_{31}} = 0 \quad (44.34)$$

from (44.29) and (44.33). It should be noted that the equilibrium condition,

(44.34), is a special case of more general conditions. Let the applied stress in an inclusion be σ_{ij}^A and the internal stress be $\Delta\sigma_{ij}$, which is brought about by the eigenstrain $\Delta\epsilon_{ij}^P$ due to the interfacial diffusion. Then, the Gibbs free energy is, generally,

$$\Delta W = -\frac{1}{2}\Delta\sigma_{ij}\Delta\epsilon_{ij}^P V_0 - \sigma_{ij}^A\Delta\epsilon_{ij}^P V_0 \quad (44.35)$$

which is minimized when

$$-(\Delta\sigma_{ij} + \sigma_{ij}^A)\delta(\Delta\epsilon_{ij}^P) = 0, \quad (44.36)$$

since $\Delta\sigma_{ij}$ is linearly related to $\Delta\epsilon_{ij}^P$. In diffusion, mass is conserved. It can be expressed by no-dilation,

$$\Delta\epsilon_{ii}^P = 0. \quad (44.37)$$

From (44.36) and (44.37), it is found that at equilibrium the total stress in the inclusion becomes hydrostatic,

$$\Delta\sigma_{ij} + \sigma_{ij}^A = \delta_{ij}\sigma \quad (44.38)$$

which includes (44.34) as a special case. It is further noted that the above discussion and conclusion are identical to those developed for a misfitting incoherent particle in Section 26.

One can discuss the kinetics of the above diffusional relaxation in a simple manner. Fig. 44.1, the number of atoms which have diffused to accomplish the formation of misfit $\Delta\epsilon_{31}^P$ is calculated as

$$N = 2V_0\Delta\epsilon_{31}^P/\pi\Omega \quad (44.39)$$

per unit length of the cylindrical inclusion. Here, Ω is the atomic volume. N is the volume of region I divided by Ω , where the major and minor axes of the ellipse in Fig. 44.1 are $a(1 + \Delta\epsilon_{31}^P)$ and $a(1 - \Delta\epsilon_{31}^P)$, respectively. When the relaxation is incomplete and the misfit strain changes by $\delta(\Delta\epsilon_{31}^P)$, the change in the Gibbs free energy is given by

$$\delta(\Delta W) = -(\Delta\epsilon_{31}^P - \Delta\epsilon_{31}^P)4\mu^*\mu(1 - 2S)V_0\delta(\Delta\epsilon_{31}^P)/g \quad (44.40)$$

from (44.31) and (44.32). The number of the transported atoms for this process is given from (44.39) as

$$\delta N = 2V_0\delta(\Delta\epsilon_{31}^P)/\pi\Omega. \quad (44.41)$$

Since the diffusion distance l is $\sim \frac{1}{2}\pi a$, the force F acting on an atom is calculated as

$$F = -\delta(\Delta W)/l\delta N = (\Delta\overline{\epsilon_{31}^p} - \Delta\epsilon_{31}^p)4\mu^*\mu(1-2S)\Omega/ag \quad (44.42)$$

from (44.40) and (44.41). The Einstein relation gives the drift velocity v of an atom as

$$v = FD_s/kT, \quad (44.43)$$

where D_s is the interfacial diffusion constant, k the Boltzmann constant and T the temperature. Let the thickness of the interfacial diffusional layer be h . Then, the number of the atoms which diffuse during the time interval δt is given by

$$\delta N = 4hv\delta t/\Omega, \quad (44.44)$$

since the cross section of the diffusion path is $4h$ per unit length of the cylinder. From (44.41) through (44.44), one obtains the rate of the misfit strain as

$$d(\Delta\epsilon_{31}^p)/dt = (\Delta\overline{\epsilon_{31}^p} - \Delta\epsilon_{31}^p)/\tau, \quad (44.45)$$

where the relaxation time τ is given by

$$\tau = \frac{kT}{hD_s} \frac{agV_0}{8\pi\mu^*\mu(1-2S)\Omega} \quad (44.46)$$

or by

$$\tau = \left\{ \frac{1}{2\mu^*} + \frac{(3-4\nu)}{2\mu} \right\} \frac{a^3 k T}{4h D_s \Omega} \quad (44.47)$$

from (44.26) and (44.27), and $V_0 = \pi a^2$. Since ϵ_{31}^p is related to the eigenstrain of the equivalent inclusion, ϵ_{31}^* , in a manner similar to (44.3), and since the macroscopic deformation is expressed by (44.2) when the volume fraction of the inclusion f is small, one expects the specimen to respond to the applied stress by

$$d\gamma/dt = (\bar{\gamma} - \gamma)/\tau \quad (44.48)$$

with τ given by (44.46) or (44.47). Here γ is the macroscopic shear strain caused by interfacial diffusion, and

$$\begin{aligned}\gamma &= f\mu^* \Delta\epsilon_{31}^p/g, \\ \bar{\gamma} &= f\mu^* \Delta\overline{\epsilon}_{31}^p/g = f\mu^*\sigma^0/\{2\mu(1-2S)g\},\end{aligned}\quad (44.49)$$

from (44.32). It is clear that, due to diffusional relaxation, the specimen behaves as a standard linear solid, (44.48) and (44.49), the relaxation strength of which is by definition (Zener 1948) given by

$$\Delta\mu = f\mu^*/\{(1-2S)g\}. \quad (44.50)$$

The preceding treatment of relaxation from interfacial diffusion followed Mori, Okabe and Mura (1980) who also have discussed spherical inclusions. Koeller and Raj (1978) have considered relaxation in a situation exactly the same as above. However, because they take a different approach, the result is not exactly the same, although the conclusions are similar in nature.

The interfacial diffusional relaxation has been successfully examined by internal friction measurements of Al-Si, Cu-SiO₂ and Cu-Fe alloys by

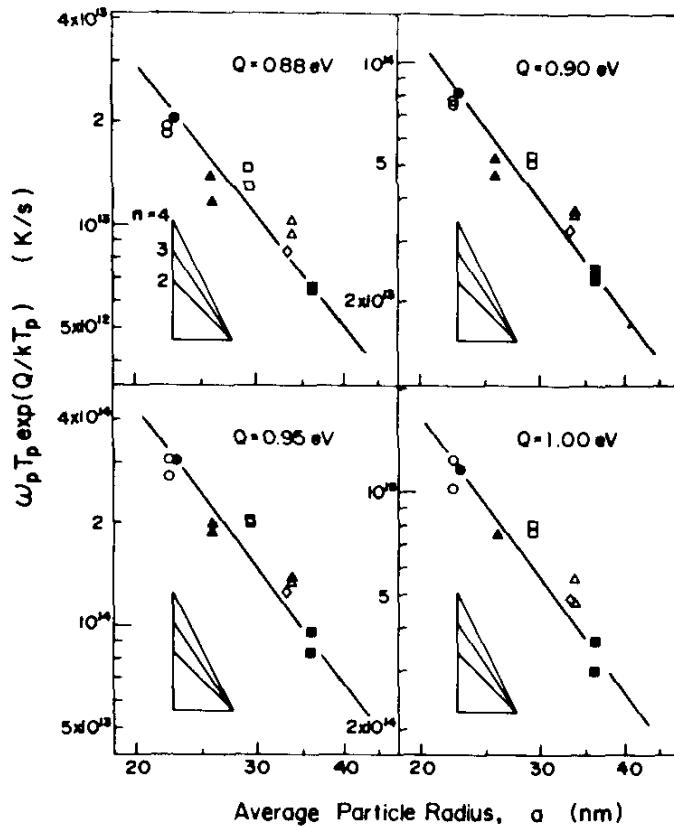


Fig. 44.8. Particle size dependency of peak temperature T_p .

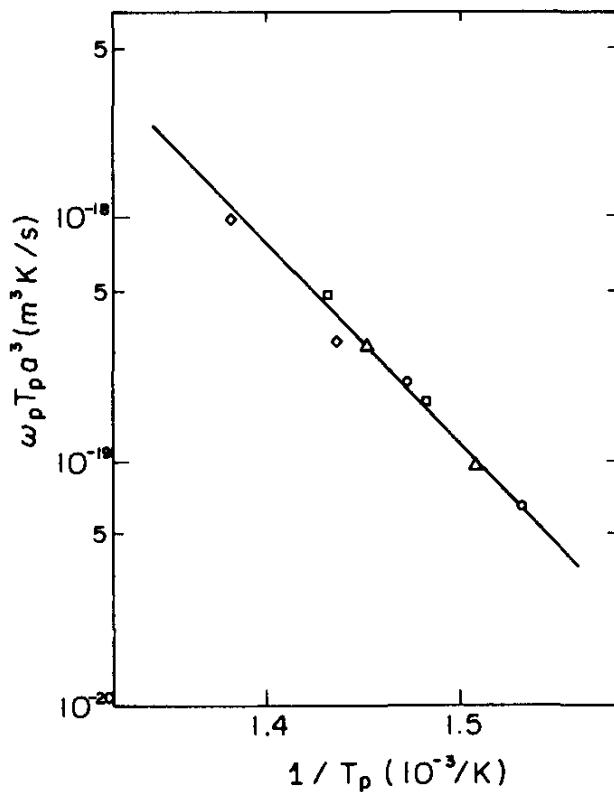


Fig. 44.8.1 Internal friction of a Cu alloy having Fe particles is measured as a function of temperature. The logarithm of the peak frequency ω_p times $T_p a^3$ is plotted against the reciprocal of the internal friction peak temperature T_p , confirming equation (44.47)

Okabe et al. (1981), Mori et al. (1983) and Monzen et al. (1983). The internal friction was measured as a function of temperature and showed a peak at a particular temperature T_p , depending on frequencies and particle sizes. The second phase particles (Si, SiO_2 and Fe) in these alloys are considered as spherical. The relaxation time has the same particle radius dependency as shown by (44.47). The maximum internal friction occurs when the frequency ω_p times the relaxation time τ is unity. τ is also a function of temperature T , where $D_s = D_0 \exp(-Q/kT_p)$. The relation $\tau\omega_p = 1$ is plotted by solid lines in Fig. 44.8 and Fig. 44.8.1 to show the particle size dependency. The slope of the lines in Fig. 44.8 is -3 since $\tau \propto a^3$. The experimental values plotted in these figures show a good agreement with the theory.

McClintock (1968) has suggested that cavities nucleate on interfaces by tearing the inclusion away from the ductile matrix or by the cracking of non-deformable inclusions. The incompatibility (misfit) between the inclusion and the matrix is expressed by the distribution of dislocations (43.18). These dislocations are dissipated by punching from the inclusion to initiate the interfacial cavities or cracks. This idea was first proposed by Ashby (1966) and further developed by Argon et al. (1975), Argon (1976), Raj and Ashby (1971, 1972, 1975), Raj (1975, 1976) and Chuang et al. (1979). It is interesting to note that condition (44.34) is equivalent to the condition of cavity formation.

45. Average elastic moduli of composite materials

The evaluation of average elastic moduli (the shear modulus and bulk modulus) of composite materials or polycrystals is one of the classical problems in micromechanics. The pioneer works on this subject have been done by Voigt (1889) and Reuss (1929). The Voigt approximation gives upper bounds and the Reuss approximation gives lower bounds of the average elastic moduli, as proved by Boas and Schmid (1934), Bishop and Hill (1951), and Hill (1952) (see also Bruggeman 1934, Huber and Schmid 1934, and Boas 1935).

The Voight approximation

Let us consider a composite material which consists of inhomogeneities with various shear moduli $\mu_1, \mu_2, \dots, \mu_n$ and a matrix with the shear modulus μ_0 (see Fig. 45.1). The average (or effective) shear modulus $\bar{\mu}$ of the composite material will be investigated here. The volume fractions of the inhomogeneities are denoted by c_1, c_2, \dots, c_n and that of the remainder (matrix) by c_0 . The shapes of the inhomogeneities are arbitrary.

Let us assume that the average elastic strain of the composite material is ϵ_{12}^0 when a shear stress is applied. Voigt (1889) approximated the average stress of the composite material as

$$\bar{\sigma} = \sum_{r=0}^n 2c_r \mu_r \epsilon_{12}^0 \quad (45.1)$$

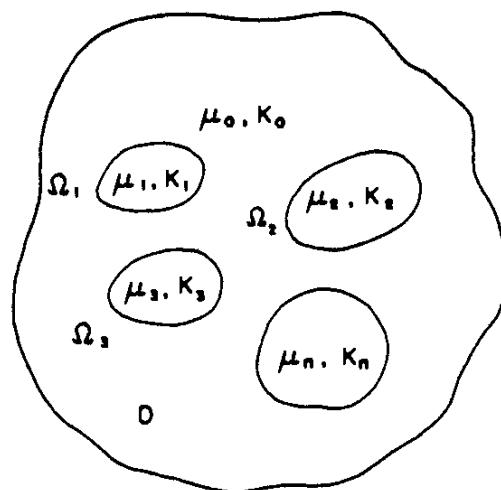


Fig. 45.1. Composite material containing inhomogeneities $\Omega_1, \Omega_2, \dots, \mu$: shear modulus K : bulk modulus.

which assumes that all the composite elements are subjected to the same uniform strain ϵ_{12}^0 . The definition of the average shear modulus is

$$\bar{\mu} = \bar{\sigma}/\bar{\gamma}, \quad (45.2)$$

$$\text{where } \bar{\gamma} = 2\epsilon_{12}^0. \quad (45.3)$$

Then, the Voigt average shear modulus is

$$\bar{\mu} = \sum_{r=0}^n c_r \mu_r. \quad (45.4)$$

The Voigt approximation can also be applied to anisotropic inhomogeneities. Since $\sigma_{12} = 2C_{1212}\epsilon_{12}^0$ and $\bar{\sigma} = \int_D \sigma_{12} dD$, where D is the unit volume, the average of C_{1212} becomes

$$\bar{C}_{1212} = \sum_{r=0}^n c_r C_{1212}^{(r)}, \quad (45.5)$$

where $C_{1212}^{(r)}$ is the elastic modulus in the r -th inhomogeneity with reference to the same coordinate system in which ϵ_{12}^0 is measured.

The average shear modulus $\bar{\mu}$ of a polycrystal can be evaluated from the anisotropic elastic moduli C_{ijkl} of constituent single crystals. The single crystals are assumed to be randomly oriented with respect to the x , y , z axes shown in Fig. 45.2. The average shear modulus is isotropic and becomes, from (45.4),

$$\bar{\mu} = (1/8\pi^2) \int_0^{2\pi} d\beta \int_0^\pi \sin \theta d\theta \int_0^{2\pi} C_{1212} d\phi, \quad (45.6)$$

where C_{1212} is referred to the x , y , z axes and (β, θ, ϕ) are the Euler angles. A random orientation of a single crystal is represented by a point on the unit sphere shown in Fig. 45.2. The orthogonal crystallographic directions of the crystal are expressed by the 1, 2 and 3 directions in the figure. The location of the point on the unit sphere indicates the 3-direction, and angle ϕ from the median indicates the 1-direction, where the plane containing the 1- and 2-directions is tangential to the unit sphere. The surface element of the unit sphere is $\sin \theta d\beta d\theta$ and the total area is 4π . Angle ϕ varies from 0 to 2π . Therefore, $(1/8\pi^2) \sin \theta d\beta d\theta d\phi$ in (45.6) corresponds to c_r in (45.5). The

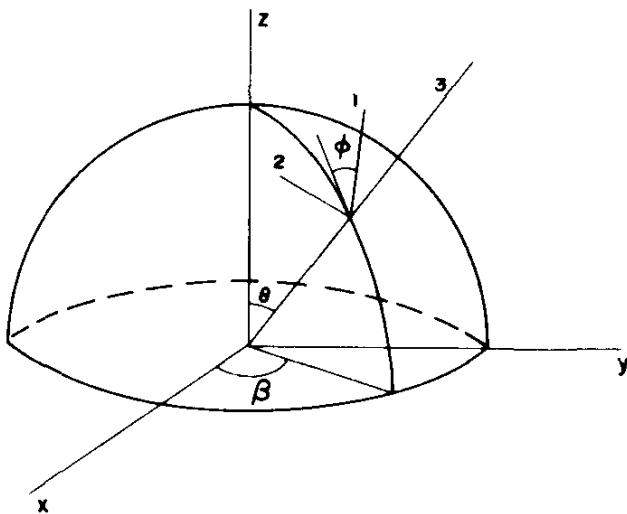


Fig. 45.2. A random orientation of a single crystal is represented by a point of the unit sphere. The crystalline directions are expressed by the axes 1, 2, 3.

elastic modulus C_{1212} in the x, y, z coordinate system and C_{IJKL} in the 1, 2, 3 coordinate system are related by

$$C_{1212} = a_{1I} a_{2J} a_{1K} a_{2L} C_{IJKL}, \quad (45.7)$$

where

$$a_{IJ} =$$

$$\begin{pmatrix} \cos \theta \cos \beta \cos \phi - \sin \beta \sin \phi, & -\cos \theta \cos \beta \sin \phi - \sin \beta \cos \phi, & \sin \theta \cos \beta \\ \cos \theta \sin \beta \cos \phi + \cos \beta \sin \phi, & -\cos \theta \sin \beta \sin \phi + \cos \beta \cos \phi, & \sin \theta \sin \beta \\ -\sin \theta \cos \phi, & \sin \theta \sin \phi, & \cos \theta \end{pmatrix} \quad (45.8)$$

The average bulk modulus \bar{K} can be obtained in a similar way. The bulk moduli of the inhomogeneities are denoted by K_1, K_2, \dots, K_n and that of the matrix by K_0 . The Voigt approximation is obtained by assuming that all the elements of the composite material have the same dilatation ϵ_{ii}^0 as the average. Then, the average hydrostatic stress $\bar{\sigma}$ is evaluated by

$$\bar{\sigma} = \sum_{r=0}^n c_r K_r \bar{\theta}, \quad (45.9)$$

where

$$\bar{\theta} = \epsilon_{ii}^0. \quad (45.10)$$

By definition, the average bulk modulus is

$$\bar{K} = \bar{\sigma}/\bar{\theta}. \quad (45.11)$$

Therefore, we have

$$\bar{K} = \sum_{r=0}^n c_r K_r. \quad (45.12)$$

The average bulk modulus of a polycrystal is evaluated in the same way. Since $\sigma_{ii} = \frac{1}{3} C_{iijj} \bar{\theta}$, and $\bar{\sigma} = \frac{1}{3} \int_D \sigma_{ii} dD$, we have

$$\bar{\sigma} = \frac{1}{9} \int_D C_{iijj} \bar{\theta} dD, \quad (45.13)$$

where D is the unit volume. The quantity C_{iijj} is an invariant and is independent of β , θ , ϕ in (45.6). Therefore, (45.13) is simply $\bar{\sigma} = \frac{1}{9} C_{iijj} \bar{\theta}$, which leads to

$$\bar{K} = \frac{1}{9} C_{iijj}. \quad (45.14)$$

The Reuss approximation

Reuss (1929) has assumed that all the elements of the composite material (Fig. 45.1) are subjected to a uniform stress equal to the average stress. According to Reuss' assumption, the average shear strain for a given shear stress $\bar{\sigma} = \sigma_{12}^0$ is expressed by

$$\bar{\gamma} = \sum_{r=0}^n c_r \bar{\sigma} / \mu_r. \quad (45.15)$$

By definition (45.2), the average shear modulus is obtained as

$$\bar{\mu} = \left(\sum_{r=0}^n c_r / \mu_r \right)^{-1}. \quad (45.16)$$

Similarly, the average bulk modulus becomes

$$\bar{K} = \left(\sum_{r=0}^n c_r / K_r \right)^{-1}. \quad (45.17)$$

Table 45.1 (after Hill 1952)

	C_{11}	C_{12}	C_{44}	G_R	G_V	$K_R = K_V$	E_R	E_V	ν_R	ν_V
Al	1.08	0.622	0.284	0.26	0.26	0.78	0.71	0.71	0.349	0.348
Cu	1.70	1.23	0.75	0.40	0.54	1.39	1.09	1.44	0.369	0.328
Au	1.86	1.57	0.42	0.24	0.31	1.67	0.69	0.87	0.431	0.413
α -Fe	2.37	1.41	1.16	0.74	0.89	1.73	1.93	2.29	0.313	0.280

Unit = 10^{12} dyn/cm².

For a polycrystal, we have

$$\bar{\mu} = \left[(1/2\pi^2) \int_0^{2\pi} d\beta \int_0^\pi \sin \theta \, d\theta \int_0^{2\pi} s_{1212} \, d\phi \right]^{-1}, \quad (45.18)$$

where s_{ijkl} is the elastic compliance and $\gamma = 2\epsilon_{12} = 4s_{1212}\sigma_{12}^0$. The average bulk modulus obtained by the Reuss method is identical to the Voigt result for the following reason: since $\theta = \epsilon_{ii} = s_{iijj}\bar{\sigma}$ for the hydrostatic stress $\bar{\sigma}$ and s_{iijj} is an invariant, it follows that $\bar{\theta} = s_{iijj}\bar{\sigma}$ and $\bar{K} = 1/s_{iijj} = C_{iijj}/9$.

Table 45.1 (Hill 1952) contains numerical results of $\bar{\mu}$ and \bar{K} for various polycrystal metals. The Voigt and Reuss approximations for $\bar{\mu}$ are denoted by G_V and G_R , respectively. The Voigt and Reuss approximations for \bar{K} are identical and are denoted by K_V or K_R . The average values of Young's modulus and Poisson's ratio are evaluated from μ and K by using the well known relations (A2.2). C_{ij} in the table are the elastic constants of the single crystals. Table 45.2 shows the experimental observation. According to Hill $\frac{1}{2}(G_R + G_V)$ or $(G_R G_V)^{1/2}$ are good approximations since

$$G_R < G < G_V, \quad (45.19)$$

where the true value of $\bar{\mu}$ is denoted by G .

Table 45.2 (after Hill 1952)

	G	K	E	ν
Al	0.265	0.74	0.71	0.34
Cu	0.436	1.33	1.18	0.35
Au	0.278	1.66	0.79	0.42
α -Fe	0.808	1.59	2.07	0.285

Unit = 10^{12} dyn/cm²

Hill's theory

Hill (1952) has shown that the Voigt approximation and the Reuss approximation are the upper and lower bounds of the true average elastic moduli.

Let the average stress and strain in an aggregate of single crystals under an applied force X_i be denoted by $\bar{\sigma}_{ij}$ and $\bar{\epsilon}_{ij}$. $\bar{\sigma}_{ij}$ and $\bar{\epsilon}_{ij}$ are uniform quantities which can be calculated from the boundary values. Namely, $\bar{\sigma}_{ij}$ is estimated from

$$\bar{\sigma}_{ij} n_j = X_i \quad \text{on } S, \quad (45.19.1)$$

where n_j is the unit exterior normal of the material surface S . $\bar{\epsilon}_{ij}$ is obtained from the equation

$$\begin{aligned} \bar{\epsilon}_{ij} &= \frac{1}{2}(\bar{u}_{i,j} + \bar{u}_{j,i}) \quad \text{in } D, \\ \bar{u}_i &= u_i^0 \quad \text{on } S, \end{aligned} \quad (45.19.2)$$

where \bar{u}_i is the average displacement which is a linear function of the space coordinates.

Denoting the average elastic moduli by \bar{C}_{ijkl} , we can write

$$\bar{\sigma}_{ij} = \bar{C}_{ijkl} \bar{\epsilon}_{kl} \quad (45.19.3)$$

or

$$\bar{\epsilon}_{ij} = \bar{s}_{ijkl} \bar{\sigma}_{kl}, \quad (45.19.4)$$

where \bar{s}_{ijkl} is the average elastic compliance tensor [see (A2.18)']. The actual stress, strain, and displacement are denoted by σ_{ij} , ϵ_{ij} and u_i , respectively. They must satisfy the same boundary conditions as in (45.19.1) and (45.19.2),

$$\begin{aligned} \sigma_{ij} n_j &= X_i \quad \text{on } S, \\ u_i &= u_i^0 \end{aligned} \quad (45.19.5)$$

Since $\sigma_{iJ,J} = 0$, $\bar{\sigma}_{iJ,J} = 0$, we have

$$\begin{aligned} \int_D \sigma_{ij} \epsilon_{ij} dD &= \int_D \sigma_{ij} u_{i,j} dD = \int_S X_i u_i^0 dS, \\ \int_D \bar{\sigma}_{ij} \bar{\epsilon}_{ij} dD &= \int_D \bar{\sigma}_{ij} \bar{u}_{i,j} dD = \int_S X_i u_i^0 dS. \end{aligned} \quad (45.19.6)$$

Therefore, it holds that

$$\bar{\sigma}_{ij}\bar{\epsilon}_{ij} = (1/V) \int_D \sigma_{ij}\epsilon_{ij} dD, \quad (45.19.7)$$

where V is the volume of D . New stress σ_{ij}^0 and strain ϵ_{ij}^0 are defined by the following: σ_{ij}^0 is the stress that would exist in a crystal with a local orientation and having the strain $\bar{\epsilon}_{ij}$. ϵ_{ij}^0 is the strain that would be produced in such a crystal by stress $\bar{\sigma}_{ij}$. Namely,

$$\begin{aligned} \sigma_{ij}^0 &= C_{ijkl}\bar{\epsilon}_{kl}, \\ \epsilon_{ij}^0 &= s_{ijkl}\bar{\sigma}_{kl}, \end{aligned} \quad (45.19.8)$$

where C_{ijkl} and s_{ijkl} represent the elastic modulus and the compliance of the single crystal. We have

$$\sigma_{ij} = C_{ijkl}\epsilon_{kl}, \quad \epsilon_{ij} = s_{ijkl}\sigma_{kl}. \quad (45.19.9)$$

From (45.19.8) and (45.19.9), $\sigma_{ij}^0\epsilon_{ij} = \sigma_{ij}\bar{\epsilon}_{ij}$ and $\sigma_{ij}\epsilon_{ij}^0 = \bar{\sigma}_{ij}\epsilon_{ij}$, and therefore,

$$\begin{aligned} \sigma_{ij}\epsilon_{ij} + (\sigma_{ij} - \sigma_{ij}^0)(\epsilon_{ij} - \bar{\epsilon}_{ij}) &= \sigma_{ij}^0\bar{\epsilon}_{ij} + 2(\epsilon_{ij} - \bar{\epsilon}_{ij})\sigma_{ij}, \\ \sigma_{ij}\epsilon_{ij} + (\sigma_{ij} - \bar{\sigma}_{ij})(\epsilon_{ij} - \epsilon_{ij}^0) &= \bar{\sigma}_{ij}\epsilon_{ij}^0 + 2(\sigma_{ij} - \bar{\sigma}_{ij})\epsilon_{ij}. \end{aligned} \quad (45.19.10)$$

The second terms in the left-hand sides in (45.19.10) are positive since they can be written as $C_{ijkl}(\epsilon_{kl} - \bar{\epsilon}_{kl})(\epsilon_{ij} - \bar{\epsilon}_{ij})$ and $s_{ijkl}(\sigma_{kl} - \bar{\sigma}_{kl})(\sigma_{ij} - \bar{\sigma}_{ij})$, respectively. It is also shown that

$$\int_D \sigma_{ij}(\epsilon_{ij} - \bar{\epsilon}_{ij}) dD = \int_S X_i(u_i^0 - u_i^0) dS = 0, \quad (45.19.11)$$

$$\int_D (\sigma_{ij} - \bar{\sigma}_{ij})\epsilon_{ij} dD = \int_S (X_i - X_i)u_i^0 dS = 0,$$

by integrating by parts. Then, (45.19.10) leads to

$$V\bar{\sigma}_{ij}\bar{\epsilon}_{ij} \leq \bar{\epsilon}_{ij} \int_D \sigma_{ij}^0 dD, \quad (45.19.12)$$

$$V\bar{\sigma}_{ij}\bar{\epsilon}_{ij} \leq \bar{\sigma}_{ij} \int_D \epsilon_{ij}^0 dD,$$

where (45.19.7) has been used for the derivation. Thus, we have, finally,

$$\begin{aligned}\bar{C}_{ijkl} &\leq (1/V) \int_D C_{ijkl} dD \\ \bar{s}_{ijkl} &\leq (1/V) \int_D s_{ijkl} dD.\end{aligned}\quad (45.19.13)$$

This result indicates that the Voigt approximations give upper bounds and the Reuss approximations yield lower bounds, since $4\bar{s}_{1212} = 1/\bar{C}_{1212}$, and $\bar{s}_{ijij} = 9/\bar{C}_{ijij}$.

Eshelby's method

Let us consider again the composite material shown in Fig. 45.1. We evaluate the average shear strain $\bar{\gamma}$ when an average shear stress is given as $\sigma_{12}^0 = \bar{\sigma}$. When the relation between $\bar{\gamma}$ and $\bar{\sigma}$ is obtained, the average shear modulus is determined by (45.2).

The stress in the composite material varies from place to place. Here it is denoted by $\sigma_{12}^0 + \sigma_{12}$, where σ_{12} is the disturbance due to inhomogeneities. Eshelby's equivalent inclusion method in Section 22 is employed. The composite material is simulated by a homogeneous material having the same elastic moduli as those of the matrix and containing inclusions Ω_r with eigenstrain ϵ_{12}^* , where $r = 1, 2, \dots, n$. Ω_r is the r -th inclusion with the same volume and location as the r -th inhomogeneity. The equivalency equation (22.5.1) is

$$\begin{aligned}\sigma_{12}^0 + \sigma_{12} &= 2\mu_0(\epsilon_{12}^0 + \epsilon_{12} - \epsilon_{12}^*) \quad \text{in } \Omega_r \\ &= 2\mu_r(\epsilon_{12}^0 + \epsilon_{12}),\end{aligned}\quad (45.20)$$

where

$$\epsilon_{12} = 2S_{1212}\epsilon_{12}^*, \quad \sigma_{12}^0 = 2\mu_0\epsilon_{12}^0, \quad (45.21)$$

and S_{ijkl} is Eshelby's tensor; $S_{1212} = (4 - 5\nu_0)/15(1 - \nu_0)$ for spherical inclusions. The equivalent eigenstrain ϵ_{12}^* in Ω_r is obtained from (45.20) as

$$\epsilon_{12}^* = (\mu_0 - \mu_r)\epsilon_{12}^0/g_r, \quad (45.22)$$

where

$$g_r = \mu_0 + 2S_{1212}(\mu_r - \mu_0). \quad (45.23)$$

The following quantities in Ω_r are easily calculated by substituting (45.22) into (45.21) and (45.20):

$$\begin{aligned}\epsilon_{12} &= 2S_{1212}(\mu_0 - \mu_r)\epsilon_{12}^0/g_r, \\ \sigma_{12} &= 2(2S_{1212} - 1)(\mu_0 - \mu_r)\mu_0\epsilon_{12}^0/g_r, \\ \epsilon_{12}^0 + \epsilon_{12} &= \mu_0\epsilon_{12}^0/g_r, \\ \sigma_{12}^0 + \sigma_{12} &= 2\mu_r\mu_0\epsilon_{12}^0/g_r.\end{aligned}\tag{45.24}$$

According to Tanaka-Mori's theorem (42.20), we have

$$\int_{D-\Omega_r} \sigma_{12} dD = 0\tag{45.25}$$

and, therefore,

$$\int_D \sigma_{12} dD = \sum_{r=1}^n c_r (2S_{1212} - 1)(\mu_0 - \mu_r) \sigma_{12}^0/g_r.\tag{45.26}$$

This does not, however, satisfy the condition for stress disturbance

$$\int_D \sigma_{12} dD = 0.\tag{45.27}$$

Therefore, we add the uniform stress

$$-\sum_{r=1}^n c_r (2S_{1212} - 1)(\mu_0 - \mu_r) \sigma_{12}^0/g_r,\tag{45.28}$$

to σ_{12} in (45.24). Consequently, the uniform strain

$$-\sum_{r=1}^n c_r (2S_{1212} - 1)(\mu_0 - \mu_r) \sigma_{12}^0/2\mu_0 g_r\tag{45.29}$$

is added to $\epsilon_{12}^0 + \epsilon_{12}$ in (45.24). The average strain is

$$\bar{\gamma} = 2 \int_D (\epsilon_{12}^0 + \epsilon_{12}) dD$$

$$= \sum_{r=1}^n c_r \bar{\sigma}/g_r - \sum_{r=1}^n c_r (2S_{1212} - 1)(\mu_0 - \mu_r) \bar{\sigma}/\mu_0 g_r + c_0 \gamma_0, \quad (45.30)$$

where γ_0 is the average strain in the matrix, and

$$\gamma_0 = \bar{\sigma}/\mu_0. \quad (45.31)$$

By using definition (45.2) and

$$c_0 + \sum_{r=1}^n c_r = 1, \quad (45.32)$$

we have

$$\bar{\mu} = \mu_0 / \left[1 + \sum_{r=1}^n c_r (\mu_0 - \mu_r) / \{ \mu_0 + 2S_{1212}(\mu_r - \mu_0) \} \right]. \quad (45.33)$$

Similarly, the average bulk modulus is obtained as

$$\bar{K} = K_0 / \left[1 + \sum_{r=1}^n c_r (K_0 - K_r) / \{ K_0 + \frac{1}{3} S_{iijj} (K_r - K_0) \} \right]. \quad (45.34)$$

The Reuss approximation is a special case, $2S_{1212} = 1$. The Voight approximation can also be obtained by $2S_{1212} = 0$ as well as by expanding the denominator, and assuming that the second term is much less than 1.

The results (45.33) and (45.34) can be obtained from the energy consideration of Section 25. The elastic strain energy of the composite material is written as (25.11). On the other hand, we can write it as

$$W^* = \frac{1}{2} \bar{\sigma}^2 / \bar{\mu}. \quad (45.34.1)$$

Substituting (45.22) and (45.34.1) into (25.11), we have

$$\frac{1}{2} \bar{\sigma}^2 / \bar{\mu} = \frac{1}{2} \bar{\sigma}^2 / \mu_0 + \frac{1}{2} \bar{\sigma}^2 \sum_{r=1}^n c_r (\mu_0 - \mu_r) / \mu_0 g_r. \quad (45.34.2)$$

Then, it leads to (45.33).

Self-consistent method

When the equivalent inclusion method is applied to obtain the stress disturbance of inhomogeneities, we assume the existence of a matrix which has

the shear and bulk moduli μ_0 and K_0 . This assumption, however, does not hold in some cases. Polycrystals, for instance, have no matrix, although they are aggregates of particles with different elastic moduli. In composite materials as shown in Fig. 45.1, the meaning of the matrix becomes rather vague when the volume fraction of inhomogeneities is increased, particularly when $c_0 \rightarrow 0$. Besides this, the interaction among inhomogeneities becomes more prominent as the number of particles increases. Kröner (1958) and Budiansky and Wu (1962) have proposed the self-consistent method in order to avoid the difficulties mentioned above.

Let us consider a composite material as shown in Fig. 45.1. The average strain and stress under an applied shear stress field are denoted by ϵ_{12}^0 and σ_{12}^0 , respectively. Hooke's law is

$$\sigma_{12}^0 = 2\bar{\mu}\epsilon_{12}^0, \quad (45.35)$$

where $\bar{\mu}$ is the average shear modulus. The stress disturbance due to the inhomogeneities is denoted by σ_{12} . The self-consistent method assumes that the composite material can be simulated by a homogeneous material which has a shear modulus $\bar{\mu}$, Poisson's ratio \bar{v} , and an eigenstrain ϵ_{12}^* in Ω_r , ($r = 1, 2, \dots, n$). The equivalency equation (22.5.1) becomes

$$\begin{aligned} \sigma_{12}^0 + \sigma_{12} &= 2\bar{\mu}(\epsilon_{12}^0 + \epsilon_{12} - \epsilon_{12}^*) \quad \text{in } \Omega_r, \\ &= 2\mu_r(\epsilon_{12}^0 + \epsilon_{12}), \end{aligned} \quad (45.36)$$

where

$$\epsilon_{12} = 2S_{1212}\epsilon_{12}^*. \quad (45.37)$$

The Poisson's ratio appearing in S_{1212} is the average Poisson's ratio \bar{v} . ϵ_{12}^* satisfying (45.36) is obtained as

$$\epsilon_{12}^* = (\bar{\mu} - \mu_r)\epsilon_{12}^0/\bar{g}_r, \quad (45.38)$$

where

$$\bar{g}_r = \bar{\mu} + 2S_{1212}(\mu_r - \bar{\mu}). \quad (45.39)$$

The strain and stress in Ω_r are obtained as

$$\begin{aligned} \epsilon_{12}^0 + \epsilon_{12} &= \bar{\mu}\epsilon_{12}^0/\bar{g}_r, \\ \sigma_{12}^0 + \sigma_{12} &= 2\mu_r\bar{\mu}\epsilon_{12}^0/\bar{g}_r. \end{aligned} \quad (45.40)$$

The average values are

$$\begin{aligned}\bar{\gamma} &= 2 \int_D (\epsilon_{12}^0 + \epsilon_{12}) dD = \sum_{r=1}^n c_r \bar{\mu} \bar{\gamma} / \bar{g}_r + c_0 \gamma_0, \\ \bar{\sigma} &= \int_D (\sigma_{12}^0 + \sigma_{12}) dD = \sum_{r=1}^n c_r \mu_r \bar{\mu} \bar{\gamma} / \bar{g}_r + c_0 \sigma_0,\end{aligned}\quad (45.41)$$

where σ_0 and γ_0 are the average shear stress and strain in the original matrix with shear modulus μ_0 :

$$\sigma_0 = \mu_0 \gamma_0. \quad (45.42)$$

Eliminating σ_0 from (45.41) and (45.42), we have

$$\bar{\sigma} = \mu_0 \bar{\gamma} + \sum_{r=1}^n c_r (\mu_r - \mu_0) \bar{\mu} \bar{\gamma} / \bar{g}_r, \quad (45.43)$$

and, therefore,

$$\bar{\mu} = \mu_0 + \sum_{r=1}^n c_r (\mu_r - \mu_0) \bar{\mu} / \{ \bar{\mu} + 2S_{1212}(\mu_r - \bar{\mu}) \}. \quad (45.44)$$

Similarly, the average bulk modulus is obtained as

$$\bar{K} = K_0 + \sum_{r=1}^n c_r (K_r - K_0) \bar{K} / \{ \bar{K} + \frac{1}{3} S_{iijj} (K_r - \bar{K}) \}. \quad (45.45)$$

In the case of spherical inclusions, we have

$$\begin{aligned}S_{1212} &= (4 - 5\bar{\nu}) / 15(1 - \bar{\nu}) \\ S_{iijj} &= (1 + \bar{\nu}) / (1 - \bar{\nu}).\end{aligned}\quad (45.46)$$

The relations in (A2.2) give

$$\bar{\nu} = (3\bar{K} - 2\bar{\mu}) / 2(\bar{\mu} + 3\bar{K}). \quad (45.47)$$

Equations (45.44) and (45.45) with (45.46) and (45.47) are simultaneous equations to determine $\bar{\mu}$ and \bar{K} for given c_r , μ_r , K_r ($r = 0, 1, \dots, n$). These

equations are valid even if $c_0 = 0$. In this case, μ_0 and K_0 are taken as zero in (45.44) and (45.45). Equations in (45.41) for $c_0 = 0$ lead to

$$1 = \sum_{r=1}^n c_r \mu_r / \bar{g}_r, \quad 1 = \sum_{r=1}^n c_r \bar{\mu} / \bar{g}_r. \quad (45.48)$$

The above relations were pointed out by Budiansky (1965).

T.T. Wu (1966) has solved simultaneously (45.44) and (45.45) and has investigated the shape effect of inclusions when Eshelby's tensor S_{ijkl} for spheroids is used. He concludes that the disk-shaped inclusions give by far the most significant increase in Young's modulus. Walpole (1967) has extended this theory to include the anisotropic case and has proposed a convergent series of successive approximations. Kneer (1965) and Morris (1970) have calculated elastic moduli of polycrystals where the orientation distribution of single crystals is expanded in a series of generalised spherical harmonics. Hershey's paper (1954) may be one of the oldest ones, after Voigt and Reuss, dealing with the elasticity of a polycrystalline in terms of the elasticity of the individual grains.

But the equivalent inclusion method, including the self-consistent method, is not accurate enough when one wants to know, for instance, the effect of the distribution pattern (morphology) of unidirectional fibers in a reinforced composite. In order to calculate the interaction among composite elements, boundary value problems of elasticity must first be solved. Exact analyses along this line have been attempted by Adams and Doner (1967), C.H. Chen (1970), Chen and Cheng (1967), and Sendeckyj (1970, 1971), among others.

Upper and lower bounds

Hashin and Shtrikman (1962, 1963) have proposed a variational principle for finding upper and lower bounds on the average elastic moduli of a composite material. Their method gives generally better bounds than the Voigt and Reuss bounds. Their method has been generalized and modified by Hill (1963), Walpole (1966, 1969, 1970), Willis (1977), Willis and Acton (1976), Kröner (1977), and Laws and McLaughlin (1979), among others.

Let us consider an inhomogeneous system with various elastic moduli C_{ijkl} . The overall (average) elastic moduli are denoted by \bar{C}_{ijkl} . The strain and stress state under an applied load is simulated by the equivalent inclusion method. The inhomogeneous system is simulated by a homogeneous system with eigenstrain ϵ_{ij}^* and elastic moduli C_{ijkl}^0 . The average strain is denoted by ϵ_{ij}^0 . The equivalency equation (22.5.1) becomes

$$C_{ijkl}^0 (\epsilon_{kl}^0 + \epsilon_{kl} - \epsilon_{kl}^*) = C_{ijkl} (\epsilon_{kl}^0 + \epsilon_{kl}), \quad (45.49)$$

where ϵ_{kl} is the strain disturbance due to the inhomogeneities, expressed by (3.24) in terms of $C_{klmn}^0 \epsilon_{mn}^*$. Let us define a polarization stress σ^* by

$$\sigma_{ij}^* \equiv C_{ijkl}^0 \epsilon_{kl}^*. \quad (45.50)$$

Expression (3.24) can be written symbolically as

$$\epsilon = \Gamma \sigma^*. \quad (45.51)$$

Then, (45.49) is written as

$$\sigma^* + (\mathbf{C} - \mathbf{C}^0)(\epsilon^0 + \Gamma \sigma^*) = 0 \quad (45.52)$$

or

$$(\mathbf{C} - \mathbf{C}^0)^{-1} \sigma^* + (\epsilon^0 + \Gamma \sigma^*) = 0. \quad (45.52.1)$$

By the use of definition

$$(f, g) = (1/V) \int_D f(x) g(x) \, dx, \quad (45.53)$$

Hashin and Shrikman (1962) have proposed the variational principle

$$\delta \left[\frac{1}{2} (\sigma^*, (\mathbf{C} - \mathbf{C}^0)^{-1} \sigma^*) + \frac{1}{2} (\sigma^*, \Gamma \sigma^*) + (\sigma^*, \epsilon^0) \right] = 0, \quad (45.54)$$

where the comparison function is σ^* . The stationary equation for the variation becomes (45.52.1) and its stationary value is

$$\frac{1}{2} (\sigma^*, \epsilon^0) = \frac{1}{2} \int_D C_{ijkl}^0 \epsilon_{kl}^* \epsilon_{ij}^0 \, dD = \frac{1}{2} \int_D \sigma_{kl}^0 \epsilon_{kl}^* \, dD, \quad (45.55)$$

where σ_{ij}^0 is defined by

$$\sigma_{ij}^0 = C_{ijkl}^0 \epsilon_{kl}^0. \quad (45.56)$$

The stationary value of the functional can be written, from (25.11), as

$$W^* - W^0, \quad (45.57)$$

where W^* is the elastic strain energy and W^0 is $\frac{1}{2} \int_D \sigma_{ij}^0 \epsilon_{ij}^0 \, dD$.

It can also be shown that the stationary value is a minimum when $\mathbf{C} - \mathbf{C}^0$ is positive definite, and a maximum when $\mathbf{C} - \mathbf{C}^0$ is negative. That is,

$$\begin{aligned} & \frac{1}{2}(\boldsymbol{\epsilon}^0, \bar{\mathbf{C}}\boldsymbol{\epsilon}^0) - \frac{1}{2}(\boldsymbol{\epsilon}^0, \mathbf{C}^0\boldsymbol{\epsilon}^0) \\ & \leq \frac{1}{2}(\boldsymbol{\sigma}^*, (\mathbf{C} - \mathbf{C}^0)^{-1}\boldsymbol{\sigma}^*) + \frac{1}{2}(\boldsymbol{\sigma}^*, \Gamma\boldsymbol{\sigma}^*) + (\boldsymbol{\sigma}^*, \boldsymbol{\epsilon}^0) \end{aligned} \quad (45.58)$$

for a positive definite $\mathbf{C} - \mathbf{C}^0$, and

$$\begin{aligned} & \frac{1}{2}(\boldsymbol{\epsilon}^0, \bar{\mathbf{C}}\boldsymbol{\epsilon}^0) - \frac{1}{2}(\boldsymbol{\epsilon}^0, \mathbf{C}^0\boldsymbol{\epsilon}^0) \\ & \geq \frac{1}{2}(\boldsymbol{\sigma}^*, (\mathbf{C} - \mathbf{C}^0)^{-1}\boldsymbol{\sigma}^*) + \frac{1}{2}(\boldsymbol{\sigma}^*, \Gamma\boldsymbol{\sigma}^*) + (\boldsymbol{\sigma}^*, \boldsymbol{\epsilon}^0) \end{aligned} \quad (45.59)$$

for a negative definite $\mathbf{C} - \mathbf{C}^0$. When $\boldsymbol{\sigma}^*$ is approximated as a linear functional of $\boldsymbol{\epsilon}^0$ in the variational method (45.54), the inequalities (45.58) and (45.59) can be used for estimating the lower and upper bounds of $\bar{\mathbf{C}}$ by choosing two sets of \mathbf{C}^0 so that $\mathbf{C} - \mathbf{C}^0$ becomes positive or negative definite, respectively. The minimum or maximum of the functional in (45.54) depends on the operator $(\mathbf{C} - \mathbf{C}^0)^{-1} + \Gamma$. Willis (1977) has shown that the operator is positive definite so long as $\mathbf{C} - \mathbf{C}^0$ is positive definite at each point of D , and is negative definite so long as $\mathbf{C} - \mathbf{C}^0$ is negative definite at each point of D , as follows. Suppose that $\mathbf{C} - \mathbf{C}^0$ is positive definite. Then,

$$(\mathbf{x}, (\mathbf{C} - \mathbf{C}^0)\mathbf{x}) > 0 \quad (45.60)$$

for any non-zero \mathbf{x} . Let $(\mathbf{C} - \mathbf{C}^0)\mathbf{x} = \mathbf{y}$. Then, $\mathbf{x} = (\mathbf{C} - \mathbf{C}^0)^{-1}\mathbf{y}$; therefore,

$$(\mathbf{x}, (\mathbf{C} - \mathbf{C}^0)\mathbf{x}) = (\mathbf{x}, \mathbf{y}) = (\mathbf{y}, \mathbf{x}) = (\mathbf{y}, (\mathbf{C} - \mathbf{C}^0)^{-1}\mathbf{y}) > 0. \quad (45.61)$$

This means that $(\mathbf{C} - \mathbf{C}^0)^{-1}$ is positive definite if $\mathbf{C} - \mathbf{C}^0$ is positive definite. We know that for any $\boldsymbol{\epsilon}^*$ on $\boldsymbol{\sigma}^*$, equation

$$\boldsymbol{\sigma} = \mathbf{C}^0(\boldsymbol{\epsilon} - \boldsymbol{\epsilon}^*) = \mathbf{C}^0\boldsymbol{\epsilon} - \boldsymbol{\sigma}^* \quad (45.62)$$

satisfies the condition $\sigma_{ij,j} = 0$ and $(\boldsymbol{\sigma}, \boldsymbol{\epsilon}) = 0$ as long as $\boldsymbol{\epsilon}$ is obtained from (45.51). We have, therefore,

$$(\boldsymbol{\sigma}^*, \Gamma\boldsymbol{\sigma}^*) = (\boldsymbol{\sigma}^*, \boldsymbol{\epsilon}) = ((\mathbf{C}^0\boldsymbol{\epsilon} - \boldsymbol{\sigma}^*), \boldsymbol{\epsilon}) = (\mathbf{C}^0\boldsymbol{\epsilon}, \boldsymbol{\epsilon}) > 0. \quad (45.63)$$

This means, then, that Γ is positive definite.

If $\mathbf{C} - \mathbf{C}^0$ is negative definite, we may consider the following approach. From (45.50) and (45.62),

$$\boldsymbol{\epsilon}^* = (\mathbf{C}^0)^{-1}\boldsymbol{\sigma}, \quad \boldsymbol{\epsilon} = (\mathbf{C}^0)^{-1}\boldsymbol{\sigma} + \boldsymbol{\epsilon}^*. \quad (45.64)$$

Hill (1963) calls $\boldsymbol{\epsilon}^*$ the strain polarization. It now follows that

$$\begin{aligned} (\boldsymbol{\sigma}^*, \Gamma\boldsymbol{\sigma}^*) &= (\mathbf{C}^0\boldsymbol{\epsilon}^*, \boldsymbol{\epsilon}) = \left(\mathbf{C}^0\boldsymbol{\epsilon}^*, \left((\mathbf{C}^0)^{-1}\boldsymbol{\sigma} + \boldsymbol{\epsilon}^* \right) \right) \\ &= (\boldsymbol{\epsilon}^*, \mathbf{C}^0\boldsymbol{\epsilon}^*) + \left(\mathbf{C}^0\boldsymbol{\epsilon}^*, (\mathbf{C}^0)^{-1}\boldsymbol{\sigma} \right) \\ &= (\boldsymbol{\epsilon}^*, \mathbf{C}^0\boldsymbol{\epsilon}^*) + \left((\mathbf{C}^0\boldsymbol{\epsilon} - \boldsymbol{\sigma}), (\mathbf{C}^0)^{-1}\boldsymbol{\sigma} \right) \\ &= (\boldsymbol{\epsilon}^*, \mathbf{C}^0\boldsymbol{\epsilon}^*) - \left(\boldsymbol{\sigma}, (\mathbf{C}^0)^{-1}\boldsymbol{\sigma} \right), \end{aligned} \quad (45.65)$$

since $(\boldsymbol{\epsilon}, \boldsymbol{\sigma}) = 0$. Consider the following identities:

$$\begin{aligned} \mathbf{C}^0(\mathbf{C}^0)^{-1} &= \mathbf{CC}^{-1}, \\ \mathbf{C}^0\mathbf{C}^{-1} &= \mathbf{C}^0\mathbf{C}^{-1}. \end{aligned} \quad (45.66)$$

The difference of the above two identities leads to

$$(\mathbf{C} - \mathbf{C}^0)^{-1}\mathbf{C}^0 = (\mathbf{C}^0)^{-1}\left((\mathbf{C}^0)^{-1} - \mathbf{C}^{-1}\right)^{-1} - \mathbf{I}, \quad (45.67)$$

where \mathbf{I} is the identity. Thus,

$$\begin{aligned} (\boldsymbol{\sigma}^*, (\mathbf{C} - \mathbf{C}^0)^{-1}\mathbf{C}^0\boldsymbol{\epsilon}^*) &= \left(\boldsymbol{\sigma}^*, (\mathbf{C}^0)^{-1}\left((\mathbf{C}^0)^{-1} - \mathbf{C}^{-1}\right)^{-1}\boldsymbol{\epsilon}^* \right) - (\boldsymbol{\sigma}^*, \boldsymbol{\epsilon}^*), \\ \end{aligned} \quad (45.68)$$

and, therefore,

$$(\boldsymbol{\sigma}^*, (\mathbf{C} - \mathbf{C}^0)^{-1}\boldsymbol{\sigma}^*) = \left(\boldsymbol{\epsilon}^*, \left((\mathbf{C}^0)^{-1} - \mathbf{C}^{-1}\right)^{-1}\boldsymbol{\epsilon}^* \right) - (\boldsymbol{\epsilon}^*, \mathbf{C}^0\boldsymbol{\epsilon}^*). \quad (45.69)$$

The sum of (45.65) and (45.69) provides

$$\begin{aligned} &(\boldsymbol{\sigma}^*, (\mathbf{C} - \mathbf{C}^0)^{-1}\boldsymbol{\sigma}^*) + (\boldsymbol{\sigma}^*, \Gamma\boldsymbol{\sigma}^*) \\ &= -\left(\boldsymbol{\epsilon}^*, \left(\mathbf{C}^{-1} - (\mathbf{C}^0)^{-1}\right)^{-1}\boldsymbol{\epsilon}^* \right) - \left(\boldsymbol{\sigma}, (\mathbf{C}^0)^{-1}\boldsymbol{\sigma} \right). \end{aligned} \quad (45.70)$$

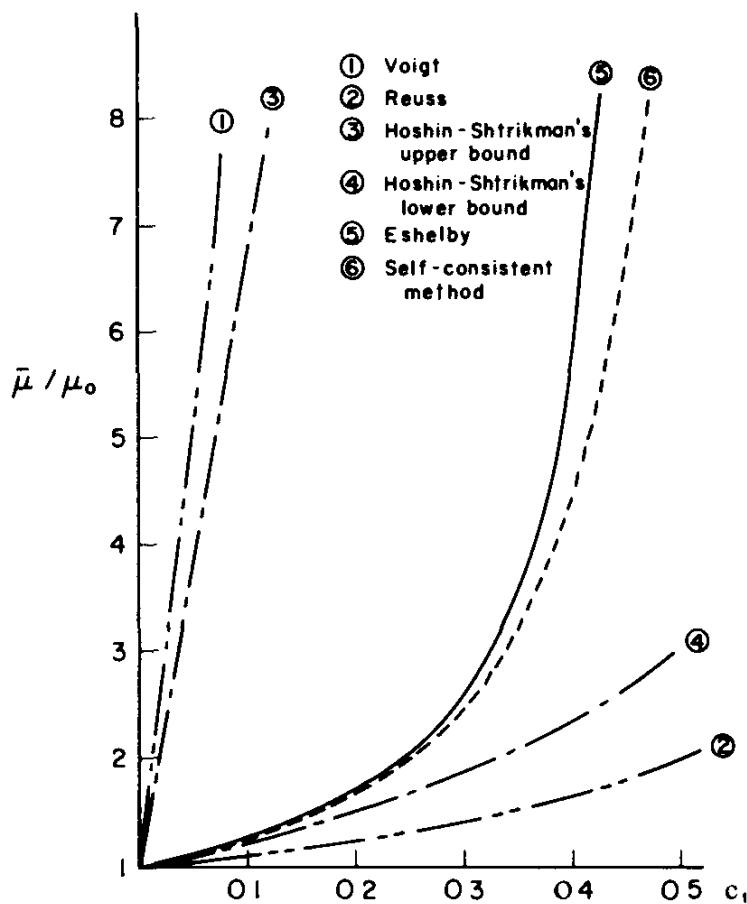


Fig. 45.3. The effective shear modulus versus the volume fraction of spherical inhomogeneities, $\mu_1/\mu_0 = 100$.

If $(C - C^0)$ is negative definite, then $(C^{-1} - (C^0)^{-1})^{-1}$ is positive definite. Therefore, the above equation (45.70) proves that the functional of σ^* in (45.59) is negative definite.

Some numerical examples are shown in Figs. 45.3 and 45.4 for a composite material containing spherical inhomogeneities. The ratios between the elastic isotropic moduli of the spheres and the matrix are taken as $\mu_1/\mu_0 = 100$, $K_1/K_0 = 100$. Poisson's ratio is taken as $\nu_1 = \nu_0 = 0.3$. The average shear modulus and bulk modulus are shown as functions of the volume fraction c_1 of spheres. For comparison, the Voigt bounds (45.4) and (45.12), the Reuss bounds (45.16) and (45.17), Eshelby's approximations (45.33) and (45.34), and the self-consistent approximations (45.44) and (45.45) are also shown in the figures.

Other related works

Many papers have been published for predicting elastic properties of fibrous composites. The review papers by Hashin (1964) and by Chamis and Sendekyj (1968), for instance, provide comprehensive lists of the literature. Various

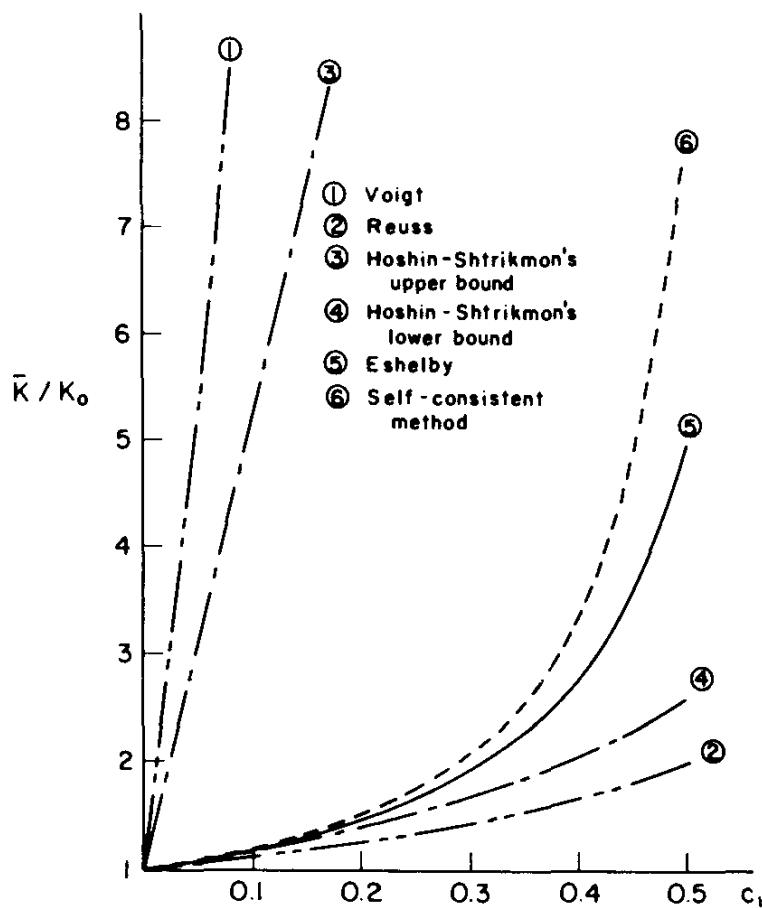


Fig. 45.4 The effective bulk modulus versus the volume fraction of spherical inhomogeneities,
 $K_1/K_0 = 100$

theories have been proposed which can be classified, according to Chamis and Sendeckyj, as netting analysis, mechanics of materials, self-consistent model, variational, exact, statistical, discrete element, semi-empirical models, and theories accounting for microstructure.

For many steady-state and transient dynamic problems of structural mechanics, it is helpful and sometimes required to know the dispersion characteristics of free harmonic waves, i.e. the dependence of the phase velocity as the wavelength. For waves propagating in the direction of the fibers, a fiber-reinforced composite is essentially a system of wave guides. Achenbach and Herrmann (1968) have derived the dispersion curves for transverse time-harmonic waves. Further dispersion curves are obtained by means of a smoothing operation (Sun, Achenbach and Herrmann 1968), and by variational methods (Kohn, Krumhansl and Lee 1972, Wheeler and Mura 1973, Nemat-Nasser and Minagawa 1975, and Nemat-Nasser and Horgan 1980).

The self-consistent imbedding approximation has been applied to rocks and porous media by many authors: Walsh (1968, 1969), Korringa (1973), Anderson et al. (1974), O'Connell and Budiansky (1974), Budiansky and O'Connell

(1976), Edil et al. (1975), and Korringa and Thompson (1977). When some cracks close and when some closed cracks undergo frictional sliding, then the overall response becomes anisotropic and dependent on the loading conditions, as well as on the loading path. The self-consistent method is used to estimate the overall moduli by Horii and Nemat-Nasser (1983).

Another important field of micromechanics is the mechanics of granular materials (soil, powder, rockfill). The works of Drucker and Prager (1952), Shield (1953, 1954), Mindlin (1954), and Mandl and Luque (1970), are phenomenological continuum theories. The microstructural continuum theories for granular materials have been developed by Mandel (1947), Spencer (1964), Horne (1965), Mogami (1965), Oda (1972), and Mehrabadi and Cowin (1978), Mróz (1980), among others (see Cowin and Satake 1978). An interesting application of the Cosserat continuum (1909) to granular materials has been given by Oshima (1953), Mindlin (1964), Eringen (1966), and Satake (1978). For finite plastic deformations of porous metals and geotechnical materials such as cohesionless or cohesive soils, Nemat-Nasser and Shokooh (1980) have developed a theory which accounts for plastic volumetric changes, pressure sensitivity, microscopic frictional effects, and a nonassociative flow rule.

46. Plastic behavior of polycrystalline metals and composites

Taylor (1938) has evaluated the flow stress of a polycrystal in tension from the critical shear stress required to cause slip in a single crystal. He regards the polycrystal as an aggregate of randomly oriented FCC crystals that are assumed to be rigid-plastic. Hershey (1954) and T.H. Lin (1957) have considered the influence of elastic strains on the Taylor theory. They have assumed that the total strain is the same in each grain (single crystal) and outlined a procedure which would predict slipping in different slip systems within a grain with an increase in the prescribed total strain. Later, Kröner (1961), Budiansky and Wu (1962), Lin (1964), Hill (1965), and Hutchinson (1970), among others, have abandoned the assumption of a homogeneous strain throughout the crystal and used instead the self-consistent method. These and related topics are reviewed in this chapter.

Taylor's analysis

The first realistic model for calculating the polycrystal uniaxial stress-strain relation from that of the single crystal was proposed by Taylor (1938). An aggregate of randomly oriented crystals under tension is assumed to be

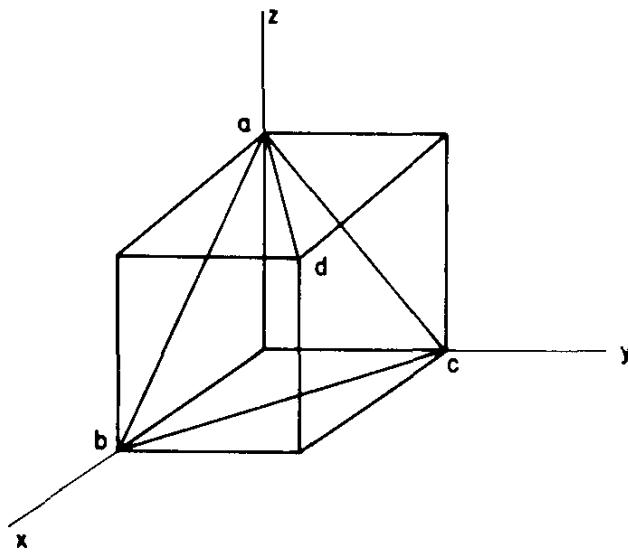


Fig. 46.1. FCC single crystal.

rigid-plastic. The associated average stress is denoted by $\bar{\sigma}_{ij}$ and the average plastic strain by $\bar{\epsilon}_{ij}^p$.

Suppose that grains of the polycrystal are FCC single crystals which have various orientations. Consider an arbitrary single grain. The relative orientations of the four slip planes of the FCC crystal are those found in the faces of a regular tetrahedron, and the three possible slip directions in each plane coincide with the edges of the triangular faces. The orientation of the tetrahedron $abcd$, relative to the cubic crystal axes x , y , z is shown in Fig. 46.1. Relative to an arbitrary set of Cartesian axes 1, 2, 3, the n^{th} slip system ($n = 1, 2, \dots, 12$) may be specified by the unit vector $m_i^{(n)}$ normal to the slip plane, and the unit vector $n_i^{(n)}$ in the slip direction.

Consider a uniform stress state σ_{ij} in a crystal of a given orientation. The resolved shear stress on the n^{th} slip system is

$$\begin{aligned}\tau^{(n)} &= \sigma_{ij} m_j^{(n)} n_i^{(n)} \\ &= \sigma_{ij} \alpha_{ij}^{(n)},\end{aligned}\tag{46.1}$$

and hence its rate is

$$\dot{\tau}^{(n)} = \dot{\sigma}_{ij} \alpha_{ij}^{(n)},\tag{46.1.1}$$

where

$$\alpha_{ij}^{(n)} = \frac{1}{2} (m_i^{(n)} n_j^{(n)} + m_j^{(n)} n_i^{(n)}).\tag{46.2}$$

The plastic strain in the grain due to the slips that have occurred along all the slip system is

$$\epsilon_{ij}^p = \sum_{n=1} \gamma^{(n)} \alpha_{ij}^{(n)}, \quad (46.3)$$

and its time derivative

$$\dot{\epsilon}_{ij}^p = \sum_{n=1} \dot{\gamma}^{(n)} \alpha_{ij}^{(n)}, \quad (46.4)$$

where $\gamma^{(n)}$ is the amount of plastic shear strain associated with $\tau^{(n)}$, the summation being taken over all active slip systems.

Taylor assumes that the plastic strain and its time rate are homogeneous and independent of the orientations of grains; in other words,

$$\epsilon_{ij}^p = \bar{\epsilon}_{ij}^p, \quad \dot{\epsilon}_{ij}^p = \dot{\bar{\epsilon}}_{ij}^p. \quad (46.5)$$

The plastic work done on all the crystals is

$$\bar{\sigma}_{ij} \dot{\bar{\epsilon}}_{ij}^p = \int_D \sigma_{ij} \dot{\epsilon}_{ij}^p dD = \int_D \sum_{n=1} \tau^{(n)} \dot{\gamma}^{(n)} dD, \quad (46.6)$$

where D is taken as a unit volume. Taylor further assumes that the strain hardening law chosen to represent the single crystal behavior, is

$$\tau_k = \tau^{(1)} = \tau^{(2)} = \dots = F\left(\sum_n |\gamma^{(n)}|\right); \quad (46.7)$$

that is, the flow stress on each system in a grain is equal and is a function of $\sum_n |\gamma^{(n)}|$. Taylor's analysis follows the following steps. For a given $\dot{\bar{\epsilon}}_{ij}^p$, $\dot{\gamma}^{(n)}$ are calculated from

$$\dot{\bar{\epsilon}}_{ij}^p = \sum_n \dot{\gamma}^{(n)} \alpha_{ij}^{(n)} \quad (46.8)$$

and

$$\sum_n |\dot{\gamma}^{(n)}| = \text{minimum}, \quad (46.9)$$

where $\alpha_{ij}^{(n)}$ varies from grain to grain. The orientations of the grains are equally distributed in a stereographic projection. After $\dot{\gamma}^{(n)}$ are found, they are substituted into (46.7) to evaluate $\tau^{(n)}$. The average stress $\bar{\sigma}_{ij}$ is calculated

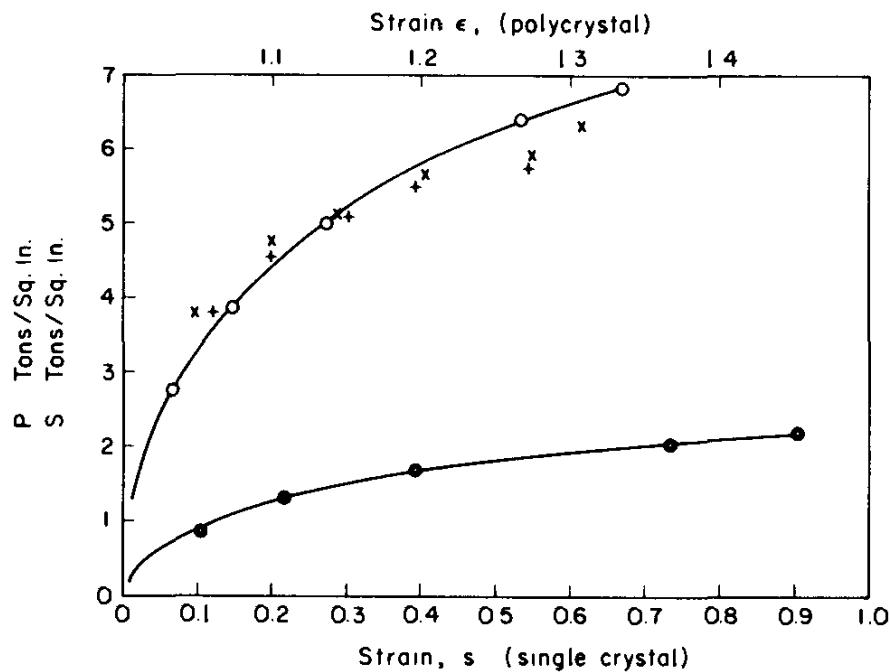


Fig. 46.2 Theoretical prediction of polycrystal stress-strain curve from a single crystal stress-strain curve (after G.I. Taylor 1938)

from (46.6) by using $\dot{\epsilon}_{ij}^p$, $\dot{\gamma}^{(n)}$ and $\tau^{(n)}$. The integral on the right-hand side in (46.6) is replaced by the integral shown in (45.6), since the orientations of the grains are equally distributed on the unit sphere associated with the Euler angles. A numerical example calculated by Taylor is shown in Fig. 46.2, where the load-extension curve ($P \sim \epsilon$) for a polycrystal aluminum is calculated from the shear stress-strain ($S \sim s$) curve of a single crystal aluminum. Points indicated by \odot are data for the single crystal, \times and $+$ show the experimental values of the polycrystal, and points \circ are the calculated values.

The minimum principle (46.9) has been proved by Bishop and Hill (1951) in the following manner. Let $d\epsilon_{ij}$ be a prescribed increment of strain in a single crystal and let σ_{ij} be a stress which will produce this strain by activating a set of shears $d\gamma^{(n)}$ with critical shear stress τ_k . Let $d\gamma^{*(n)}$ be any set of shears which are geometrically equivalent to the prescribed strain, but which are not necessarily operable by any stress satisfying the yield conditions; that is,

$$d\epsilon_{ij} = \sum_n d\gamma^{(n)} \alpha_{ij}^{(n)} = \sum_n d\gamma^{*(n)} \alpha_{ij}^{(n)} \quad (46.10)$$

and

$$\begin{aligned} \sum_n \tau^{(n)} d\gamma^{(n)} &= \tau_k \sum_n |d\gamma^{(n)}|, \\ \sum_n \tau^{(n)} d\gamma^{*(n)} &\leq \tau_k \sum_n |d\gamma^{*(n)}|. \end{aligned} \quad (46.11)$$

Multiplying (46.10) by σ_{ij} , we have, from (46.1),

$$\sum_n \tau^{(n)} d\gamma^{(n)} = \sum_n \tau^{(n)} d\gamma^{*(n)}. \quad (46.12)$$

Equations (46.11) and (46.12) lead to

$$\sum_n |d\gamma^{(n)}| < \sum_n |d\gamma^{*(n)}| \quad (46.13)$$

which is equivalent to the statement (46.9).

Taylor's hardening law (46.7) has been further studied and modified by Hutchinson (1964), Hill and Rice (1972), Havner and Shalaby (1977, 1978), Shalaby and Havner (1978), and Havner et al. (1979).

Applying the self-consistent method to finite elasto-plastic deformations, Iwakuma and Nemat-Nasser (1984) have estimated the overall moduli of polycrystalline solids. The method predicts a Bauschinger effect, hardening, and formation of vertex or corner on the yield surface. Their interesting conclusion is that small second-order quantities, such as rotations of grains and residual stresses have a first-order effect on the overall response, as they lead to a loss of the overall stability by localized deformations.

Self-consistent method

Taylor's analysis has been modified by using the idea of the self-consistent method explained in Section 45. The stress σ_{ij} and strain ϵ_{ij} in a grain are different from the average stress $\bar{\sigma}_{ij}$ and the average strain $\bar{\epsilon}_{ij}$. The difference of the plastic strain ϵ_{ij}^p and its average value $\bar{\epsilon}_{ij}^p$ is considered the cause of the non-uniformity. The misfit $\epsilon_{ij}^p - \bar{\epsilon}_{ij}^p$ is considered as the eigenstrain used for the spherical inclusion problem. The strain caused by the misfit can be written as $S_{ijkl}(\epsilon_{kl}^p - \bar{\epsilon}_{kl}^p)$, where S_{ijkl} is the Eshelby tensor. If the material has no grains and is homogeneous everywhere, the strain is $\epsilon_{ij} = C_{ijkl}^{-1}\bar{\sigma}_{kl} + \bar{\epsilon}_{ij}^p$. However, when the material has the misfit, we have

$$\epsilon_{ij} = C_{ijkl}^{-1}\bar{\sigma}_{kl} + \bar{\epsilon}_{ij}^p + S_{ijkl}(\epsilon_{kl}^p - \bar{\epsilon}_{kl}^p). \quad (46.14)$$

The first term on the right-hand side of (46.14) is the average elastic strain. The last term is composed of the elastic and plastic strain disturbances, the sum of which is the total strain disturbance due to the misfit between ϵ_{ij}^p and $\bar{\epsilon}_{ij}^p$. On the other hand, it must hold that

$$\epsilon_{ij} = C_{ijkl}^{-1}\sigma_{kl} + \epsilon_{ij}^p, \quad (46.15)$$

where σ_{ij} is the stress in a grain. The overall average of σ_{ij} is $\bar{\sigma}_{ij}$. The overall average of ϵ_{ij} is $\bar{\epsilon}_{ij}$, and that of ϵ_{ij}^p is $\bar{\epsilon}_{ij}^p$. Multiplying the elastic moduli to the equation resulting from (46.14) = (46.15), we have

$$\sigma_{ij} = \bar{\sigma}_{ij} + C_{ijkl} \{ S_{klmn} (\epsilon_{mn}^p - \bar{\epsilon}_{mn}^p) - (\epsilon_{kl}^p - \bar{\epsilon}_{kl}^p) \} \quad (46.16)$$

or its time derivative

$$\dot{\sigma}_{ij} = \dot{\bar{\sigma}}_{ij} + C_{ijkl} \{ S_{klmn} (\dot{\epsilon}_{mn}^p - \dot{\bar{\epsilon}}_{mn}^p) - (\dot{\epsilon}_{kl}^p - \dot{\bar{\epsilon}}_{kl}^p) \}. \quad (46.16.1)$$

The twelve slip systems are considered in a FCC single crystal of a given orientation. The resolved shear stress on the n^{th} slip system can be expressed by (46.1). The plastic strain in the grain can also be given by (46.3).

$\tau^{(n)}$ and $\gamma^{(n)}$ are related by the plastic shear stress-shear strain relation of the single crystal,

$$\tau = \tau(\gamma) \quad \text{or} \quad \dot{\tau} = \tau' \dot{\gamma}. \quad (46.17)$$

$\tau^{(n)}$ is the value of τ , and $\gamma^{(n)}$ is the value of γ for the n^{th} slip system. Equations (46.16.1), (46.1.1), (46.3), (46.17), and the following concept of average are fundamental ingredients in finding $\bar{\sigma}_{ij}$ as a function of $\bar{\epsilon}_{ij}^p$. The average of ϵ_{ij}^p for all possible orientations of single crystals (grains) is $\bar{\epsilon}_{ij}^p$. Namely,

$$\bar{\epsilon}_{ij}^p = (\epsilon_{ij}^p)_{\text{average}}. \quad (46.18)$$

$\bar{\sigma}_{ij}$ starts from zero. $\sigma_{ij} = \bar{\sigma}_{ij}$ until $\tau^{(n)}$ (resolved shear stress) in some grains reaches the elastic limit τ_y . If $\bar{\sigma}_{ij}$ is increased by $\dot{\bar{\sigma}}_{ij}$, then ϵ_{ij}^p is increased by $\dot{\epsilon}_{ij}^p$ from zero value. In order to find $\dot{\epsilon}_{ij}^p$ we must solve (46.16.1) by assuming $\dot{\epsilon}_{ij}^p$ from zero value. In order to find $\dot{\epsilon}_{ij}^p$ we must solve (46.16.1) by assuming $\dot{\epsilon}_{mn}^p = 0$ (initial value). Since $\dot{\sigma}_{ij}$ is also a function of $\dot{\epsilon}_{ij}^p$, it is convenient to rewrite (46.16.1) in another form. Multiplying (46.16.1) by $\alpha_{ij}^{(n)}$ (since we know which grains and which slip systems are plastically active at this moment, we know $\alpha_{ij}^{(n)}$) we have, from (46.1.1),

$$\dot{\tau}^{(n)} = \dot{\bar{\sigma}}_{ij} \alpha_{ij}^{(n)} + \alpha_{ij}^{(n)} C_{ijkl} \{ S_{klmn} (\dot{\epsilon}_{mn}^p - \dot{\bar{\epsilon}}_{mn}^p) - (\dot{\epsilon}_{kl}^p - \dot{\bar{\epsilon}}_{kl}^p) \}. \quad (46.19)$$

Furthermore, from (46.4) and (46.17) we have

$$\tau' \dot{\gamma}^{(n)} = \dot{\bar{\sigma}}_{ij} \alpha_{ij}^{(n)} + \alpha_{ij}^{(n)} C_{ijkl} \left\{ S_{klmn} \left(\sum_{n=1} \dot{\gamma}^{(n)} \alpha_{mn}^{(n)} - \dot{\bar{\epsilon}}_{mn}^p \right) - \left(\sum_{n=1} \dot{\gamma}^{(n)} \alpha_{kl}^{(n)} - \dot{\bar{\epsilon}}_{kl}^p \right) \right\}, \quad (46.20)$$

where $n = 1, 2, \dots, 12$ when all slip systems are plastically active. Some of the n slip systems are not active in the initial state after the elastic limit. The unknown quantities are $\dot{\gamma}^{(n)}$ in (46.20) ($\dot{\epsilon}_{ij}^p = 0$). The solutions of $\dot{\gamma}^{(n)}$ are substituted into (46.4) to obtain $\dot{\epsilon}_{ij}^p$. $\dot{\epsilon}_{ij}^p$ is not zero for the calculation of the second stage when $\bar{\sigma}_{ij}$ is further increased. $\dot{\epsilon}_{ij}^p$ is calculated from $\dot{\epsilon}_{ij}^p$ by

$$\dot{\epsilon}_{ij}^p = (1/8\pi^2) \int_0^{2\pi} d\beta \int_0^\pi \sin \theta d\theta \int_0^{2\pi} \dot{\epsilon}_{ij}^p d\phi, \quad (46.21)$$

where the Euler angles (β, θ, ϕ) (Fig. 45.2) relate the orientation of stress axes (1, 2, 3) to that of the crystal axes (x, y, z). The new increment $\dot{\epsilon}_{ij}^p$ provides an increment of $\dot{\gamma}^{(n)}$ in (46.20) by giving new $\bar{\sigma}_{ij}$ and $\dot{\epsilon}_{ij}^p$, where $\bar{\sigma}_{ij}$ is arbitrarily chosen and $\dot{\epsilon}_{ij}^p$ is calculated by (46.21) in the preceding calculation. The same step-by-step calculation is continued. $\bar{\epsilon}_{ij}$ is easily calculated by

$$\bar{\epsilon}_{ij} = C_{ijkl}^{-1} \bar{\sigma}_{kl} + \dot{\epsilon}_{ij}^p. \quad (46.22)$$

A more general plastic shear stress-strain relation is

$$\dot{\tau}^{(n)} = \sum_{m=1} h^{(nm)} \dot{\gamma}^{(m)} \quad (46.23)$$

which may be used for the left-hand side of (46.20). In this calculation, we must keep in mind the loading and unloading conditions as well as the condition that $\dot{\tau}^{(n)}$ and $\dot{\gamma}^{(n)}$ must have the same sign.

The method of solving (46.20) is essentially the one proposed by Kröner (1961), Budiansky and T.T. Wu (1962), T.H. Lin (1964) and Hutchinson (1970). The method proposed by Hill (1965) is slightly different. Equation (46.15) is written as

$$\sigma_{ij} = C_{ijkl} (\epsilon_{kl} - \epsilon_{kl}^p) \quad \text{or} \quad (46.24)$$

$$\dot{\sigma}_{ij} = C_{ijkl} (\dot{\epsilon}_{kl} - \dot{\epsilon}_{kl}^p).$$

When (46.24) is multiplied by $\alpha_{ij}^{(n)}$, we have

$$\dot{\tau}^{(n)} = \alpha_{ij}^{(n)} C_{ijkl} (\dot{\epsilon}_{kl} - \dot{\epsilon}_{kl}^p). \quad (46.25)$$

Substituting (46.23) and (46.4) into (46.25), we obtain

$$\sum_{m=1} h^{(nm)} \dot{\gamma}^{(m)} + \alpha_{ij}^{(n)} C_{ijkl} \sum_{m=1} \dot{\gamma}^{(m)} \alpha_{kl}^{(m)} = \alpha_{ij}^{(n)} C_{ijkl} \dot{\epsilon}_{kl}. \quad (46.26)$$

By solving (46.26) to express $\dot{\gamma}^{(n)}$ in terms of $\dot{\epsilon}_{kl}$, we have

$$\dot{\gamma}^{(n)} = f_{kl}^{(n)} \dot{\epsilon}_{kl}. \quad (46.27)$$

Then, (46.24) becomes

$$\dot{\sigma}_{ij} = C_{ijkl} \left(\dot{\epsilon}_{kl} - \sum_{n=1} f_{pq}^{(n)} \alpha_{kl}^{(n)} \dot{\epsilon}_{pq} \right). \quad (46.28)$$

If (46.28) is substituted into (46.16.1), we have

$$C_{ijkl} \left(\dot{\epsilon}_{kl} - \sum_{n=1} f_{pq}^{(n)} \alpha_{kl}^{(n)} \dot{\epsilon}_{pq} \right) = \dot{\bar{\sigma}}_{ij} + C_{ijkl} \left\{ S_{klmn} \left(\dot{\epsilon}_{mn}^p - \dot{\bar{\epsilon}}_{mn}^p \right) - \left(\dot{\epsilon}_{kl}^p - \dot{\bar{\epsilon}}_{kl}^p \right) \right\}. \quad (46.29)$$

From (46.29) we can calculate $\dot{\epsilon}_{ij}^p$ in terms of given values of $\dot{\bar{\sigma}}_{ij}$ and the initial value of $\dot{\bar{\epsilon}}_{ij}^p$ in the preceding calculation. The new $\dot{\bar{\epsilon}}_{ij}^p$ is calculated from $\dot{\epsilon}_{ij}^p$ by (46.21).

The early work by Lin (1957) used (46.20) without the second term, in which he assumed $\dot{\epsilon}_{ij}^p = \dot{\bar{\epsilon}}_{ij}^p$. Figure 46.3 shows the numerical example obtained by Hutchinson (1970), showing the comparison of various theoretical tensile stress-plastic strain curves for FCC polycrystals composed of isotropic and non-hardening single crystal grains. The Lin model refers to Lin's early work (1957). The K. B. W. model stands for the works by Kröner, Budiansky and Wu. These curves are obtained by assuming that the single crystals have no work-hardening. In this case the left-hand side in (46.20) is zero after yielding. The yield shear stress is denoted by τ_y .

Equation (46.20) can also be used for determining the yield surface of a polycrystal. Lin and Ito (1966) and Lin (1971) have assumed that a polycrystal consists of identical basic cubic blocks of 64 cube-shaped crystals having different orientations. The calculation is similar to the cubic inclusion problem. Further interesting studies on the yield surface have been done by Mandel (1966), Bui (1970), Zarka (1972, 1973), and Weng and Phillips (1977), among others.

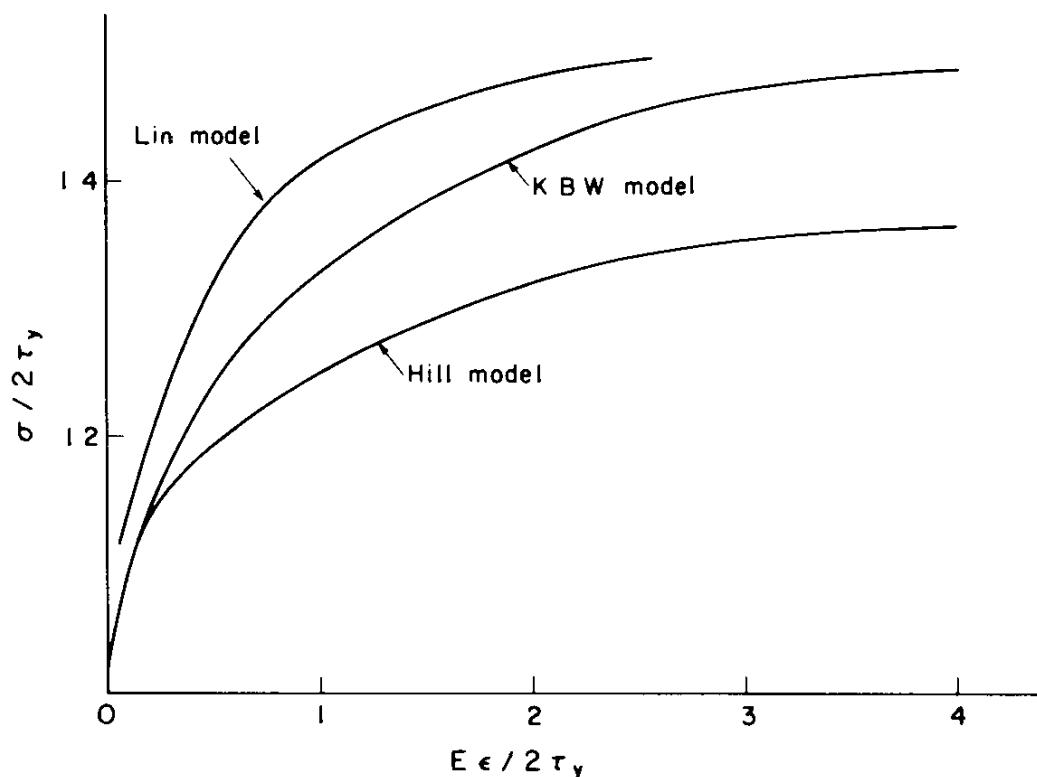


Fig. 46.3. Various theoretical predictions of stress-strain curves of a FCC polycrystal composed of isotropic and non-hardening single crystal grains (after Hutchinson 1970).

For the first original work to determine the yield function from multi-slip of an isotropic aggregate of FCC crystals the reader is referred to the two papers by Bishop and Hill (1951). Recently, Bassani (1977) has examined the yield function of FCC (and BCC) metals whose texture gives rise to transversely isotropic plastic properties.

Chen and Argon (1979) employed the self-consistent theory to consider the effect of grain boundary sliding in creeping polycrystals and composites where the sliding boundaries are treated as disk-shaped viscous inhomogeneities and the matrix (grain interior) follows a power law in creep. Raj and Ashby (1971) and Raj (1975) considered a similar problem and investigated the nucleation of intergranular cracks during steady state and cyclic creep. Their analysis is based upon stress concentrations at inclusions which are produced by grain boundary sliding.

Ghahremani (1980) has investigated the effect of grain boundary sliding on anelasticity of polycrystalline materials. Recently, Weng (1981) has employed the self-consistent method for predicting the creep and recovery strain of an aluminum alloy. Weng (1984) has given the stress and strain state of constituent phases for a general multiphase, anisotropic composite with arbitrarily oriented anisotropic inclusions. The self-consistent method has been applied to composite materials by various investigators (e.g. Taya and Mura 1981, Taya

and Chou 1981, 1982, Takao et al. 1982, other important references in Christensen 1979, T.W. Chou and M. Taya 1980, and Hashin 1983).

Embedded weakened zone

If a material contains a weakened zone, Ω , the zone can be simulated by an ellipsoidal inhomogeneous inclusion. For simplicity it is assumed that the principal axes of the ellipsoid are parallel to the coordinate axes. The two phases Ω and the matrix have different shear moduli μ^* and μ , respectively, and different resistances for plastic deformation. It is assumed that an applied stress σ_{12}^0 causes uniform plastic strain ϵ_{12}^α in Ω , and uniform plastic strain ϵ_{12}^β in the matrix. The stress disturbance σ_{ij} can be obtained from (22.19) where ϵ_{ij}^p is replaced by $(\epsilon_{12}^\alpha - \epsilon_{12}^\beta)$. For a single weakened zone, the stress in Ω is obtained as

$$\sigma_{12} = -(1 - 2S_{1212}) \frac{2\mu^*(\epsilon_{12}^\alpha - \epsilon_{12}^\beta) + (1 - \mu^*/\mu)\sigma_{12}^0}{1 + 2(\mu^*/\mu - 1)S_{1212}}. \quad (46.30)$$

The strain disturbance ϵ_{ij} in Ω is

$$\epsilon_{12} = 2S_{1212} \frac{(\mu^*/\mu)(\epsilon_{12}^\alpha - \epsilon_{12}^\beta) + (1 - \mu^*/\mu)\sigma_{12}^0/2\mu}{1 + 2(\mu^*/\mu - 1)S_{1212}}. \quad (46.31)$$

The total stress and strain in Ω are

$$\begin{aligned} \sigma'_{12} &= \sigma_{12}^0 + \sigma_{12}, \\ \epsilon'_{12} &= \epsilon_{12}^0 + \epsilon_{12}, \end{aligned} \quad (46.32)$$

where

$$\sigma_{12}^0 = 2\mu\epsilon_{12}^0. \quad (46.33)$$

Then, we have

$$\sigma'_{12} = -2\mu(1/2S_{1212} - 1)\epsilon'_{12} + \sigma_{12}^0/2S_{1212}. \quad (46.34)$$

Eshelby's tensor S_{1212} becomes $\frac{7}{30}$ for a spherical Ω and $\frac{15}{30}$ for a penny-shaped Ω . The above equation has been obtained by Rudnicki (1977 and 1979). σ'_{12} versus ϵ'_{12} is shown in Fig. 46.4 by the straight line ACDB for a given σ_{12}^0 . If the constitutive equation of the weakened zone is expressed by curve OCP,

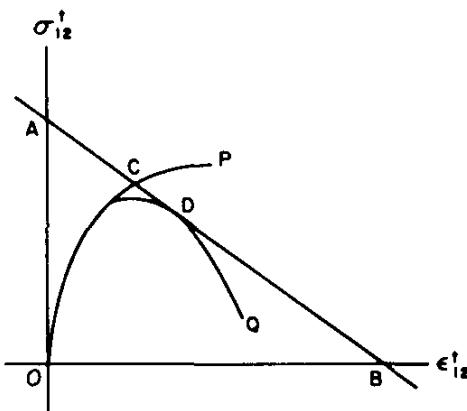


Fig 46.4 Stress-strain state inside an inclusion is represented by an intersection of AB and 0P (or 0Q).

point C is uniquely determined for a given σ_{12}^0 as the intersection of the straight line and the curve. On the other hand, if the constitutive equation is expressed by curve ODQ , for instance, the plastic flow inside Ω becomes unstable, perhaps cracks will be initiated to accommodate deformation. Rudnicki's theory is used by Rice (1979), Rice and Rudnicki (1979) in a study of earthquake premonitory processes. It is argued that the fluid coupling effects serve to stabilize the weakened rock against rapid fracture, and give rise instead to a precursory period of accelerating, but initially quasi-static straining which ultimately leads to dynamic instability (see also Rudnicki 1985 for a review).

When thin strips are pulled in tension, it is observed that the neck (shear-band bifurcation mode) forms across the specimen at an oblique angle which depends on the state of anisotropy. The general theory of bifurcation has been established by Thomas (1961) and Hill (1962) and further developments of the theory have been achieved by Hill and Hutchinson (1975), Stören and Rice (1975), Rudnicki and Rice (1975), Needleman (1979), Asaro and Rice (1977), and Iwakuma and Nemat-Nasser (1982) among others.

47. Viscoelasticity of composite materials

Eshelby (1957) has pointed out that the elastic theory of inclusions can be extended to viscoelastic materials. The viscoelastic creep of composite materials, for instance, can be evaluated by the viscoelastic theory of inclusions.

Homogeneous inclusions

Consider an infinitely extended viscoelastic material, and let an ellipsoidal sub-domain Ω of the material be subjected to an eigenstrain $\epsilon_{ij}^*(x, t)$. The inclusion Ω and the matrix $D - \Omega$ are assumed to have the same viscoelastic

properties. The stress and strain are denoted by σ_{ij} and ϵ_{ij} , respectively. Equations (2.1), (2.2) and (2.10) still hold,

$$\epsilon_{ij} = e_{ij} + \epsilon_{ij}^*, \quad (47.1)$$

where $\epsilon_{ij}^* = 0$ in $D - \Omega$. Instead of Hooke's law, we now consider

$$\sigma_{ij}(x, t) = L_{ijkl}(t)e_{kl}(x, 0) + \int_0^\infty L_{ijkl}(t-\tau) \frac{de_{kl}(x, \tau)}{d\tau} d\tau \quad (47.2)$$

or

$$e_{ij}(x, t) = M_{ijkl}(t)\sigma_{kl}(x, 0) + \int_0^\infty M_{ijkl}(t-\tau) \frac{d\sigma_{kl}(x, \tau)}{d\tau} d\tau \quad (47.3)$$

assuming that $e_{ij}(x, t)$, $\sigma_{ij}(x, t)$, $L_{ijkl}(t)$ and $M_{ijkl}(t)$ are all zero for $t < 0$. L_{ijkl} and M_{ijkl} are called the tensorial relaxation and creep functions, respectively, and have the same properties of symmetry with respect to the indices as the elastic moduli and compliances. Let the Laplace transform (A3.21) of a function $f(t)$ be indicated by a bar,

$$\bar{f}(s) = \int_0^\infty f(t) \exp(-st) dt, \quad s > 0. \quad (47.4)$$

The Laplace transform of (47.1) and (47.2) are obtained as

$$\bar{\epsilon}_{ij} = \bar{e}_{ij} + \bar{\epsilon}_{ij}^*, \quad (47.5)$$

and

$$\bar{\sigma}_{ij} = s\bar{L}_{ijkl}\bar{e}_{kl}, \quad (47.6)$$

where

$$\int_0^\infty M(t-\tau) \exp(-st) dt = \bar{M} \exp(-s\tau)$$

and

$$\int_0^\infty \frac{de}{d\tau} \exp(-s\tau) d\tau = -e(0) + s\bar{e}$$

are used. Equations (2.10) for equilibrium are transformed to

$$\bar{\sigma}_{ij,j} = 0 \quad (47.7)$$

and equations (2.2) become

$$\bar{\epsilon}_{ij} = \frac{1}{2}(\bar{u}_{i,j} + \bar{u}_{j,i}). \quad (47.8)$$

The system of equations (47.5) ~ (47.8) becomes completely equivalent to the elastic case, where the elastic moduli C_{ijkl} correspond to $s\bar{L}_{ijkl}$.

For isotropic materials, we have

$$L_{ijkl}(t) = \lambda(t)\delta_{ij}\delta_{kl} + \mu(t)(\delta_{ik}\delta_{jl} + \delta_{il}\delta_{jk}) \quad (47.9)$$

and

$$\bar{L}_{ijkl} = \bar{\lambda}\delta_{ij}\delta_{kl} + \bar{\mu}(\delta_{ik}\delta_{jl} + \delta_{il}\delta_{jk}) \quad (47.9.1)$$

(see Fung 1965). λ and μ in (A2.1) correspond to $s\bar{\lambda}$ and $s\bar{\mu}$ for the viscoelasticity. When an ellipsoidal inclusion has a uniform eigenstrain ϵ_{ij}^* , the Laplace transform of the stress in Ω is obtained from (11.20), by changing μ to $s\bar{\mu}$ and ν to $\bar{\nu} = \frac{1}{2}\bar{\lambda}/(\bar{\lambda} + \bar{\mu})$,

$$\begin{aligned} \bar{\sigma}_{11}/2s\bar{\mu} &= \left[\frac{a_1^2}{8\pi(1-\bar{\nu})} \left\{ \frac{1-\bar{\nu}}{1-2\bar{\nu}} 3I_{11} + \frac{\bar{\nu}}{1-2\bar{\nu}} (I_{21} + I_{31}) \right\} \right. \\ &\quad \left. + \frac{1-2\bar{\nu}}{8\pi(1-\bar{\nu})} \left\{ \frac{1-\bar{\nu}}{1-2\bar{\nu}} I_1 - \frac{\bar{\nu}}{1-2\bar{\nu}} (I_2 + I_3) \right\} - \frac{1-\bar{\nu}}{1-2\bar{\nu}} \right] \bar{\epsilon}_{11}^* \\ &\quad + \left[\frac{a_2^2}{8\pi(1-\bar{\nu})} \left\{ \frac{1-\bar{\nu}}{1-2\bar{\nu}} I_{12} + \frac{\bar{\nu}}{1-2\bar{\nu}} (3I_{22} + I_{32}) \right\} \right. \\ &\quad \left. - \frac{1-2\bar{\nu}}{8\pi(1-\bar{\nu})} \left\{ \frac{1-\bar{\nu}}{1-2\bar{\nu}} I_1 - \frac{\bar{\nu}}{1-2\bar{\nu}} (I_2 - I_3) \right\} - \frac{\bar{\nu}}{1-2\bar{\nu}} \right] \bar{\epsilon}_{22}^* \\ &\quad + \left[\frac{a_3^2}{8\pi(1-\bar{\nu})} \left\{ \frac{1-\bar{\nu}}{1-2\bar{\nu}} I_{13} + \frac{\bar{\nu}}{1-2\bar{\nu}} (3I_{33} + I_{23}) \right\} \right. \quad (47.10) \end{aligned}$$

$$\bar{\sigma}_{12}/2s\bar{\mu} = \left\{ \frac{a_1^2 + a_2^2}{8\pi(1-\bar{\nu})} I_{12} + \frac{1-2\bar{\nu}}{8\pi(1-\bar{\nu})} (I_1 + I_2) - 1 \right\} \bar{\epsilon}_{12}^*,$$

and other components are obtained by the cyclic permutation of (1, 2, 3). The inverse transform of $\bar{\sigma}_{ij}$ is, from (A3.22),

$$\sigma_{ij}(x, t) = (1/2\pi i) \int_{\gamma-i\infty}^{\gamma+i\infty} \bar{\sigma}_{ij}(x, s) \exp(st) ds. \quad (47.11)$$

The above result can be easily extended to more general eigenstrains and anisotropic materials. The Laplace transform of the displacement is obtained from (6.2) as

$$\bar{u}_i = -i(2\pi)^{-3} \int_{-\infty}^{\infty} \int_{\Omega} s \bar{L}_{jlmn} \bar{\epsilon}_{mn}^* \xi_l \left(\bar{N}_{ij}/\bar{D} \right) \exp\{i\xi \cdot (x - x')\} d\xi dx', \quad (47.12)$$

where \bar{N}_{ij} and \bar{D} are the cofactors and the determinant of the matrix having the ij element $s \bar{L}_{imjn} \xi_m \xi_n$. The above equation has been used by Kuo and Mura (1973) for solving a circular twist disclination in viscoelastic materials.

Inhomogeneous inclusions

The equivalent inclusion method in Section 22 can be used for the viscoelastic creep of composite materials. The constitutive equation of the matrix is assumed to be (47.2). On the other hand, the inhomogeneities behave as

$$\begin{aligned} \sigma_{ij}^0(t) + \sigma_{ij}(x, t) &= L_{ijkl}^*(t) \{ \epsilon_{kl}^0(0) + \epsilon_{kl}(x, 0) \} \\ &\quad + \int_0^{\infty} L_{ijkl}^*(t - \tau) \frac{d}{d\tau} \{ \epsilon_{kl}^0(\tau) + \epsilon_{kl}(x, \tau) \} d\tau, \end{aligned} \quad (47.13)$$

where

$$\sigma_{ij}^0(t) = L_{ijkl}(t) \epsilon_{kl}^0(0) + \int_0^{\infty} L_{ijkl}(t - \tau) \frac{d}{d\tau} \epsilon_{kl}^0(\tau) d\tau \quad (47.14)$$

is the applied stress at infinity, and $\epsilon_{ij}^0(t)$ is the corresponding strain. σ_{ij} in (47.2) and (47.13) is the stress disturbance due to the inhomogeneities. The equivalent inclusion method is to simulate σ_{ij} by the stress caused by homogeneous inclusions with proper ϵ_{ij}^* . The equivalency is obtained by the equation (47.13) = (47.14) + (47.2). The Laplace transform of this equation is

$$s \bar{L}_{ijkl}^*(\bar{\epsilon}_{kl}^0 + \bar{\epsilon}_{kl}) = s \bar{L}_{ijkl}(\bar{\epsilon}_{kl}^0 + \bar{\epsilon}_{kl} - \bar{\epsilon}_{kl}^*) \quad (47.15)$$

which can be obtained directly from (22.5). $\bar{\epsilon}_{kl}$ or $\bar{\sigma}_{ij}$ is a linear function of $\bar{\epsilon}_{ij}^*$. Equation (47.15) determines $\bar{\epsilon}_{ij}^*$ necessary for the equivalent inclusion. The stress state can be completely determined after ϵ_{ij}^* for a given $\sigma_{ij}^0(t)$ is evaluated.

Laws and McLaughlin (1978) have estimated the viscoelastic creep compliances of composite materials and have given numerical solutions for two common composite materials, namely an isotropic dispersion of spheres, and an uni-directional fibre reinforced material. Viscous liquids can be treated in the same manner as in viscoelastic materials if velocity is used in place of displacement. Bilby, Eshelby and Kundu (1975), and Howard and Brierley (1976) have found numerically the change of a viscous inhomogeneity inside a viscous liquid of different viscosity. Hashin (1965) has obtained the macroscopic viscoelastic heterogeneous media in terms of effective relaxation moduli and creep compliances. Recently, Budiansky and Hutchinson (1980) have determined the growth-rate and shape for an isolated void growing in a self-similar way in an infinite block of incompressible power-law, viscous material (see also Budiansky, Hutchinson and Lutsky 1980). Taya and Seidel (1981), Toon and Taya (1984) have obtained the void growth rate when a viscous material contains many voids. They consider the interaction among the voids by using Mori-Tanaka's method.

Waves in an infinite medium

Consider an infinite medium which consists of an isotropic matrix and ellipsoidal inhomogeneities Ω with volume fraction c . The matrix and the inhomogeneities are assumed to be different standard linear solids. Setting $D \equiv \partial/\partial t$, the stress-strain relations are written as

$$\begin{aligned} P_1(D)' \sigma_{ij} &= Q_1(D)' e_{ij} && \text{in the matrix} \\ P_2(D) \sigma_{kk} &= Q_2(D) e_{kk} \end{aligned} \quad (47.16)$$

and

$$\begin{aligned} P_1^*(D)' \sigma_{ij} &= Q_1^*(D)' e_{ij} && \text{in } \Omega, \\ P_2^*(D) \sigma_{kk} &= Q_2^*(D) e_{kk} \end{aligned} \quad (47.17)$$

where ' σ_{ij} ' and ' e_{ij} ' are the reduced stress and strain defined by (A2.6), and $P(D)$ and $Q(D)$ are polynomials of D .

Consider an applied strain $\epsilon_{ij}^0 = \bar{\epsilon}_{ij}^0 \exp(i\omega t)$. The strain and stress disturbances due to the inhomogeneities are also assumed to have the form

$\epsilon_{ij} = \bar{\epsilon}_{ij} \exp(i\omega t)$ and $\sigma_{ij} = \bar{\sigma}_{ij} \exp(i\omega t)$. If the inhomogeneities are simulated by inclusions with eigenstrain $\epsilon_{ij}^* = \bar{\epsilon}_{ij}^* \exp(i\omega t)$, the equivalency equations become

$$\begin{aligned}\{Q_1^*(i\omega)/P_1^*(i\omega)\}(\bar{\epsilon}_{ij}^0 + \bar{\epsilon}_{ij}) &= \{Q_1(i\omega)/P_1(i\omega)\}(\bar{\epsilon}_{ij}^0 + \bar{\epsilon}_{ij} - \bar{\epsilon}_{ij}^*), \\ \{Q_2^*(i\omega)/P_2^*(i\omega)\}(\bar{\epsilon}_{kk}^0 + \bar{\epsilon}_{kk}) &= \{Q_2(i\omega)/P_2(i\omega)\}(\bar{\epsilon}_{kk}^0 + \bar{\epsilon}_{kk} - \bar{\epsilon}_{kk}^*).\end{aligned}\quad (47.18)$$

The above equations determine $\bar{\epsilon}_{ij}^*$ for a given set of $\bar{\epsilon}_{ij}^0$ since $\bar{\epsilon}_{ij}$ is a linear function of $\bar{\epsilon}_{ij}^*$ as shown by (11.15), that is,

$$\bar{\epsilon}_{ij} = S_{ijkl} \bar{\epsilon}_{kl}^*. \quad (47.19)$$

Comparing with (22.17), the following correspondence holds:

$$\begin{aligned}2\mu^* &= Q_1^*(i\omega)/P_1^*(i\omega), & K^* &= Q_2^*(i\omega)/P_2^*(i\omega), \\ 2\mu &= Q_1(i\omega)/P_1(i\omega), & K &= Q_2(i\omega)/P_2(i\omega).\end{aligned}\quad (47.20)$$

Eshelby's tensor S_{ijkl} in (47.19) contains Poisson's ratio of the matrix, which is expressed as

$$\nu = (3K - 2\mu)/(2(3K + \mu)) \quad (47.21)$$

from (A2.2). K and μ are, of course, functions of $P(i\omega)$ and $Q(i\omega)$ through (47.20).

The average (or effective) shear modulus $\bar{\mu}$ and bulk modulus \bar{K} are determined from (45.33) and (45.34), where $\mu_0 = \mu$, $K_0 = K$, $\mu_r = \mu^*$, $K_r = K^*$ and $c_r = c$. In the above discussion we have assumed that the state of deformation is quasi-static. When a dynamic state of the wave propagation with angular frequency ω is considered, we assume that the material will respond by the complex moduli $\bar{\mu}$ and \bar{K} obtained in the above discussion. Then, the equations of motion become

$$\bar{\mu} \ddot{u}_{i,jj} + (\frac{1}{3} \bar{\mu} + \bar{K}) \ddot{u}_{j,ji} + \rho \omega^2 \ddot{u}_i = 0 \quad (47.22)$$

or

$$(\bar{\lambda} + 2\bar{\mu}) \ddot{u}_{j,ji} - \bar{\mu} \epsilon_{ijk} \epsilon_{kmn} \ddot{u}_{n,mj} + \rho \omega^2 \ddot{u}_i = 0, \quad (47.22.1)$$

where ρ is the average density of the material and $\bar{\lambda} + 2\bar{\mu} = \bar{K} + 4\bar{\mu}/3$. The solutions for (47.22) are the real parts of

$$\bar{u}_i = A n_i \exp\left(i\omega\left[t \pm \left\{\rho/(\bar{K} + 4\bar{\mu}/3)\right\}^{1/2} n_k x_k\right]\right) \quad (47.23)$$

and

$$\bar{u}_i = C_i \exp\left(i\omega\left[t \pm (\rho/\bar{\mu})^{1/2} n_k x_k\right]\right), \quad (47.24)$$

where A, C_i, n_i are arbitrary constants and $C_j n_j = 0$. Equation (47.23) represents a plane dilatational wave and equation (47.24) represents a plane shear wave. The real parts of the factors in front of $n_k x_k$ are the inverse of wave velocities, v_D, v_S ; whereas the imaginary parts are attenuation factors α_D, α_S . We have

$$v_D = \left[Re\left\{\rho/(\bar{K} + 4\bar{\mu}/3)\right\}^{1/2}\right]^{-1}, \quad \alpha_D = -\omega Im\left\{\rho/(\bar{K} + 4\bar{\mu}/3)\right\}^{1/2}, \quad (47.25)$$

for dilational waves, and

$$v_S = \left[Re\left(\rho/\bar{\mu}\right)^{1/2}\right]^{-1}, \quad \alpha_S = -\omega Im\left(\rho/\bar{\mu}\right)^{1/2}, \quad (47.26)$$

for shear waves.

Walsh (1968, 1969) has calculated the wave velocities and attenuations in a partially melted rock, where the melted zone is simulated by a penny-shaped inclusion of liquid. The seismic velocities of cracked rocks also are calculated by Anderson et al. (1974), O'Connell and Budiansky (1974), and Budiansky and O'Connell (1976), among others.

48. Elastic wave scattering

The subject of multiple scattering of waves is of interest in many fields of engineering and science. In acoustics, it has important practical applications in studies of the distribution of flaws in solids, fiber reinforced composites, porous media, rocks, earth, underwater signal transmissions, etc.

The basic scattering problem is shown in Fig. 48.1. An incident plane wave with angular frequency ω and wave vector k is expressed by

$$u_i^0(x, t) = u_i^0(x) \exp(-i\omega t), \quad (48.1)$$

$$u_i^0(x) = u_i^A \exp(ik \cdot x),$$

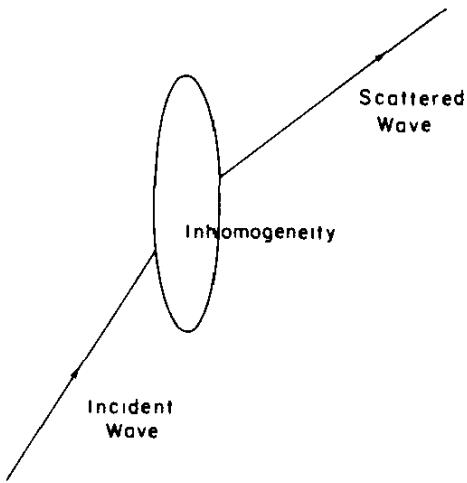


Fig. 48.1. Scattering of elastic wave due to an inhomogeneity.

the scattered wave by

$$u_i^s(x, t) = u_i^s(x) \exp(-i\omega t) \quad (48.2)$$

and the total wave amplitude by

$$u_i(x) = u_i^0(x) + u_i^s(x). \quad (48.3)$$

The equations of motion are

$$C_{ijkl} u_{k,lj}(x) + \rho \omega^2 u_i(x) = 0 \quad (48.4)$$

in the matrix and

$$C_{ijkl}^* u_{k,lj}(x) + \rho^* \omega^2 u_i(x) = 0 \quad (48.5)$$

in the inhomogeneity Ω , where the elastic moduli and the density in Ω are distinguished by an asterisk, *. Equation (48.5) can be written as

$$C_{ijkl} u_{k,lj}(x) + \rho \omega^2 u_i(x) + X_i(x) = 0, \quad (48.6)$$

where

$$X_i(x) = \Delta C_{ijkl} u_{k,lj}(x) + \Delta \rho \omega^2 u_i(x) \quad (48.7)$$

and

$$\begin{aligned} C_{ijkl}^* &= C_{ijkl} + \Delta C_{ijkl}, \\ \rho^* &= \rho + \Delta \rho. \end{aligned} \quad (48.8)$$

The quantity $X_i(x)$ is equivalent to a body force distributed in the domain Ω . The solution of a unit body force is known as the steady-state elastic wave Green's function (9.37). Then, for the distributed body force X_i we have

$$u_i(x) = u_i^0(x) + \int_D \left\{ \Delta C_{m_j k_l} u_{k,l}(x') + \Delta \rho \omega^2 u_m(x') \right\} g_{im}(x - x') dx' \quad (48.9)$$

or

$$\begin{aligned} u_i(x) &= u_i^0(x) + \Delta \rho \omega^2 \int_{\Omega} u_m(x') g_{im}(x - x') dx' \\ &\quad + \Delta C_{m_j k_l} \int_{\Omega} u_{k,l}(x') g_{im,j}(x - x') dx' \end{aligned} \quad (48.10)$$

after integration by parts.

For an isotropic material, $g_{im}(x - x')$ is known from (9.40). The far-field scattered amplitudes can be calculated by using the following asymptotic equalities:

$$\begin{aligned} 1/|x - x'| &\approx 1/x, \\ |x - x'| &\approx x - (x' \cdot \bar{x}), \end{aligned} \quad (48.11)$$

$$\frac{\partial^2}{\partial x'_i \partial x'_j} \frac{\exp(i\alpha|x - x'|)}{|x - x'|} \approx -\frac{\alpha^2 \bar{x}_i \bar{x}_j}{x} \exp(i\alpha x - i\alpha x' \cdot \bar{x}),$$

where $\bar{x} = x/x$.

From (9.40), we have

$$\begin{aligned} g_{ij}(x - x') &\approx \frac{1}{4\pi\rho\omega^2 x} \left[\beta^2 (\delta_{ij} - \bar{x}_i \bar{x}_j) \exp(i\beta x - i\beta x' \cdot \bar{x}) \right. \\ &\quad \left. + \alpha^2 \bar{x}_i \bar{x}_j \exp(i\alpha x - i\alpha x' \cdot \bar{x}) \right], \end{aligned} \quad (48.12)$$

$$\begin{aligned}
g_{ij,k}(x - x') &\approx -\frac{\partial}{\partial x'_k} g_{ij}(x - x') \\
&= \frac{i\bar{x}_k}{4\pi\rho\omega_x^2} \left[\beta^3 (\delta_{ij} - \bar{x}_i \bar{x}_j) \exp(i\beta x - i\beta x' \cdot \bar{x}) \right. \\
&\quad \left. + \alpha^3 \bar{x}_i \bar{x}_j \exp(i\alpha x - i\alpha x' \cdot \bar{x}) \right].
\end{aligned}$$

When the expressions in (48.12) are substituted into (48.10), we have

$$u_i^s(x) \approx (\delta_{im} - \bar{x}_i \bar{x}_m) f_m(\beta) x^{-1} \exp(i\beta x) + \bar{x}_i x_m f_m(\alpha) x^{-1} \exp(i\alpha x), \quad (48.13)$$

where

$$\begin{aligned}
f_m(\alpha) = \frac{a^2}{4\pi\rho\omega^2} &\left[\Delta\rho\omega^2 \int_{\Omega} u_m(x') \exp(-i\alpha x' \cdot \bar{x}) dx' \right. \\
&\quad \left. + i\alpha \bar{x}_j \Delta C_{m j k l} \int_{\Omega} u_{k,l}(x') \exp(-i\alpha x' \cdot \bar{x}) dx' \right] \quad (48.14)
\end{aligned}$$

which is called the *f*-vector. Since $u_m(x')$ and $u_{k,l}(x')$ in the integrands of the integrals in (48.14) are unknown, an iterative procedure is employed. The first approximation is obtained by substituting $u_m^0(x')$ and $u_{k,l}^0(x')$ for these unknown functions. This first approximation is called the first Born (or Rayleigh-Gauss) approximation (see Gubernatis et al., 1977).

The Born approximation becomes progressively worse as changes in the material properties become large, and is limited to $k|x - x'| < 1$ (long wavelength). This has been concluded by Gubernatis et al. (1977) by comparing the approximation with the exact solution for a spherical inhomogeneity obtained by Ying and Truell (1956), and Einspruch et al. (1960). More convenient perturbation methods, however, have been developed by Datta (1977), Gubernatis (1979), Mal and Knopoff (1967), and Gubernatis and Domany (1979), among others.

Another approximation method is based upon the eigenfunction expansions. According to Morse and Feshbach (1958), and Pao and Mow (1971), the dynamic displacement amplitude $u_i(x)$ for an isotropic material can be expressed as

$$u_i(x) = \Phi_{,i} - i\epsilon_{ijk} x_j \psi_{,k} - i(x_i \chi_{,j} - x_j \chi_{,i}), \quad (48.15)$$

When it is substituted into the wave equation (47.22.1) (u_i for \bar{u}_i), the wave equation is satisfied if

$$\begin{aligned}\Phi_{,ii} + \alpha^2 \Phi &= 0, \\ \psi_{,ii} + \beta^2 \psi &= 0, \\ \chi_{,ii} + \beta^2 \chi &= 0,\end{aligned}\tag{48.16}$$

are satisfied.

For the spherical coordinates the eigenfunctions of the scalar Helmholtz equations (48.16) are products of spherical Bessel functions and spherical harmonics $Y_{lm}(\theta, \phi)$. For non-spherical defects, the expansion method has been used in several different ways to obtain approximate solutions as seen in the papers by Waterman (1969, 1971), Varatharajulu and Pao (1976), Waterman (1976), and Visscher (1978).

Recently Gubernatis, Domany, and Krumhansl (1980) have published an excellent review paper on the elastic wave scattering theory with applications to non-destructive evaluation. For randomly distributed inhomogeneities we refer to the papers by Bose and Mal (1973), McCoy (1973), Sobezyk (1976), Datta (1977), and Varadan and Varadan (1979), among others.

Dynamic equivalent inclusion method

The equivalent inclusion method can also be applied to the dynamic response of an ellipsoidal inhomogeneity by a slight modification as suggested by Mura (1972c).

The equation of motion is

$$C_{ijkl}(u_{k,lj}^0 + u_{k,lJ}) = \rho(\ddot{u}_i^0 + \ddot{u}_i)\tag{48.17}$$

in the matrix and

$$C_{ijkl}^*(u_{k,lj}^0 + u_{k,lJ}) = \rho^*(\ddot{u}_i^0 + \ddot{u}_i)\tag{48.18}$$

in the inhomogeneity Ω , where u_i^0 is an incident wave and u_i is the disturbed displacement. The equation (48.18) is simulated by the equation of motion in the homogeneous material with a fictitious body force f_i and eigenstrain ϵ_{ij}^* in Ω ,

$$C_{ijkl}(u_{k,lj}^0 + u_{k,lJ} - \epsilon_{kl,j}^*) + f_i = \rho(\ddot{u}_i^0 + \ddot{u}_i)\tag{48.19}$$

The equations for the necessary equivalency are

$$\begin{aligned} C_{ijkl}^*(u_{k,l}^0 + u_{k,l}) &= C_{ijkl}(u_{k,l}^0 + u_{k,l} - \epsilon_{kl}^*), \\ \rho^*(\ddot{u}_i^0 + \ddot{u}_i) &= \rho(\ddot{u}_i^0 + \ddot{u}_i) - f_i. \end{aligned} \quad (48.20)$$

Willis (1980) and Fu and Mura (1983) used (48.20) for the analysis of dynamic response of a single ellipsoidal inhomogeneity subjected to plane time-harmonic waves and evaluated differential cross sections.

Green's formula

Due to the symmetry of C_{ijkl} , it holds

$$C_{ijkl}g_{km,l}(x - x')u_{i,j}(x') = C_{ijkl}u_{k,l}(x')g_{im,j}(x - x'), \quad (48.21)$$

where $g_{ij}(x - x')$ is defined by (9.36). The formula resulted from integrating (48.21) with respect to x' over any domain is called Green's formula. Green's formula has been used in the boundary integral equation (BIE) method developed by Kupradze (1953, 1965). Using this method, Kobayashi and Nishimura (1982) Niwa, Kitahara and Ikeda (1984), Kitahara (1985) among others have established numerical schemes to solve transient wave propagation problems around inhomogeneities.

Let us apply the BIE method to the problem shown by Fig. 48.1. Integrate (48.21) for the scattered wave amplitude (take u_i as u_i^s) in the matrix domain $(D - \Omega)$, and apply integration by parts. Then, we have

$$\begin{aligned} & - \int_{|\Omega|} C_{ijkl}g_{km,l}(x - x')u_i^s(x')n_j \, dS(x') \\ & + \int_{D - \Omega} C_{ijkl}g_{km,l}(x - x')u_i^s(x') \, dx' \\ & = \int_{|\Omega|} C_{ijkl}u_{k,l}^s(x')g_{im}(x - x')n_j \, dS(x') \\ & + \int_{D - \Omega} C_{ijkl}u_{k,l}^s(x')g_{im}(x - x') \, dx', \end{aligned} \quad (48.22)$$

where n_j is the outward unit normal on the boundary $|\Omega|$ of the domain Ω .

Since the scattered wave vanishes at infinity, the boundary integrals at infinity become zero (the radiation condition).

The relations $C_{ijkl}g_{km,lj}(x - x') = -\rho\omega^2 g_{im}(x - x') - \delta_{im}\delta(x - x')$ and $C_{ijkl}u_{k,lj}^s(x') = -\rho\omega^2 u_i^s(x')$ are substituted in (48.22) to yield

$$\begin{aligned} \int_{D-\Omega} \delta(x - x') u_m^s(x') dx' &= - \int_{|\Omega|} C_{ijkl}g_{km,l}(x - x') u_i^s(x') n_j dS(x') \\ &\quad - \int_{|\Omega|} C_{ijkl}u_{k,l}^s(x') g_{im}(x - x') n_j dS(x'). \end{aligned} \quad (48.23)$$

The left hand side in the above Green's formula is $u_m^s(x)$ when x is in the matrix $D - \Omega$, $\frac{1}{2}u_m^s(x)$ when x is on $|\Omega|$, and zero when x is in Ω , because of the definition of Dirac's delta function.

When the incident wave amplitude u_i^0 is used for u_i in (48.21) and the identity is integrated in the domain Ω , we have similarly

$$\begin{aligned} \int_{\Omega} \delta(x - x') u_m^0(x') dx' &= \int_{|\Omega|} C_{ijkl}g_{km,l}(x - x') u_i^0(x') n_j dS(x') \\ &\quad + \int_{|\Omega|} C_{ijkl}u_{k,l}^0(x') g_{im}(x - x') n_j dS(x'), \end{aligned} \quad (48.24)$$

since $C_{ijkl}u_{k,lj}^0(x') = -\rho\omega^2 u_i^0(x')$ in Ω and $D - \Omega$. When the formula $u_m^s = u_m - u_m^0$ is substituted into (48.23) and relation (48.24) is used, we have

$$\begin{aligned} \int_{D-\Omega} \delta(x - x') u_m(x') dx' &= - \int_{|\Omega|} C_{ijkl}g_{km,l}(x - x') u_i(x') n_j dS(x') \\ &\quad - \int_{|\Omega|} C_{ijkl}u_{k,l}(x') g_{im}(x - x') n_j dS(x') \\ &\quad + \int_D \delta(x - x') u_m^0(x') dx'. \end{aligned} \quad (48.25)$$

In order to apply (48.21) to u_i defined by (48.5), a new steady-state elastic wave Green's function $g_{km}^*(x - x')$ is defined to a homogeneous infinite

domain with the elastic moduli C_{ijkl}^* and the density ρ^* ,

$$C_{ijkl}^* g_{km,lj}^*(x - x') + \rho^* \omega^2 g_{im}^*(x - x') + \delta_{im} \delta(x - x') = 0 \quad (48.26)$$

The C_{ijkl} and g_{km} in (48.21) are replaced by C_{ijkl}^* and $g_{km}^*(x - x')$ and the resulting identity is integrated in domain Ω with respect to x' . Then, an expression similar to (49.24) is obtained,

$$\begin{aligned} \int_{\Omega} \delta(x - x') u_m(x') dx' &= \int_{|\Omega|} C_{ijkl}^* g_{km,l}^*(x - x') u_i(x') n_j dS(x') \\ &\quad + \int_{|\Omega|} C_{ijkl}^* u_{k,l}(x') g_{im}^*(x - x') n_j dS(x'). \end{aligned} \quad (48.26.1)$$

The left hand side of (48.26) is $u_m(x)$ when x is in Ω , $\frac{1}{2}u_m(x)$ when x on $|\Omega|$ and zero when x is in $D - \Omega$.

The boundary integral equations are obtained from (48.25) and (48.26) by taking x on $|\Omega|$ and by requiring the continuity conditions for the displacement and the force traction on $|\Omega|$. Writing the force traction as $t_i = C_{ijkl} u_{k,l} n_j = C_{ijkl}^* u_{k,l} n_j$, we have for x on $|\Omega|$

$$\begin{aligned} \frac{1}{2}u_m(x) &= - \int_{|\Omega|} C_{ijkl}^* g_{km,l}^*(x - x') u_i(x') n_j dS(x') \\ &\quad - \int_{|\Omega|} t_i(x') g_{im}^*(x - x') dS(x') + u_m^0(x) \end{aligned} \quad (48.27)$$

and

$$\begin{aligned} \frac{1}{2}u_m(x) &= \int_{|\Omega|} C_{ijkl}^* g_{km,l}^*(x - x') u_i(x') n_j dS(x') \\ &\quad + \int_{|\Omega|} t_i(x') g_{im}^*(x - x') dS(x') \end{aligned} \quad (48.28)$$

The above equations (48.27) and (48.28) are the integral equations to determine u_i and t_i on $|\Omega|$. After determination of these boundary values, the displacement u_m in $D - \Omega$ is evaluated by (48.25), and u_m in Ω by (48.26), where $t_i(x')$ is used for $C_{ijkl} u_{k,l}(x') n_j$ in (48.25) and $C_{ijkl}^* u_{k,l}(x') n_j$ in (48.26).

When the material contains many inhomogeneities, $\Omega_1, \Omega_2, \dots$ we have the same boundary integral equations (48.27) and (48.28) by taking $|\Omega| = |\Omega_1| + |\Omega_2| + \dots$. When the material is a half space, still the same equations hold if the other half space is taken as an infinity large void Ω_0 , and $|\Omega| = |\Omega_0| + |\Omega_1| + |\Omega_2| + \dots$.

When an incident wave is transient, it is expressed by the Fourier integral form with respect to ω . Then, the transient solution is obtained by superposing the steady-state solutions described here.

49. Interaction between dislocations and inclusions

The term inclusion is interpreted here in a broader sense to include inhomogeneity and crack. Dispersed second-phase particles and nonmetallic inclusions exert a large influence upon the mechanical properties of materials. In addition to increasing the yield strength by raising the stress necessary to move dislocations through the matrix, the presence of dispersed phases and inclusions affects the fracture behavior of these materials by providing sites for crack formation via particle or particle-interface cracking at the tip of blocked slip bands.

Inclusions and dislocations

Consider an infinite elastic medium containing a circular cylindrical inclusion of radius a (Fig. 49.1). The shear modulus and Poisson's ratio of the matrix are denoted by μ_1 and ν_1 and the corresponding constants of the inclusion by μ_2 and ν_2 . The stress field of a dislocation located at $(\xi, 0)$ is found by Dundurs (1967) for a screw dislocation and by Dundurs and Mura (1964) and Dundurs and Sendeckyj (1965) for an edge dislocation. The evaluation of the Peach-Koeler force requires the stress component σ_{yx} for the screw dislocation and σ_{yz} for the edge dislocation.

For the screw dislocation, the stress at $(x, 0)$ is

$$\sigma_{yz} = \frac{\mu_1 b}{2\pi} \left\{ \frac{1}{x - \xi} + \frac{\mu_2 - \mu_1}{\mu_2 + \mu_1} \left(\frac{1}{x - a^2/\xi} - \frac{1}{x} \right) \right\}, \quad (49.1)$$

where $x > a$.

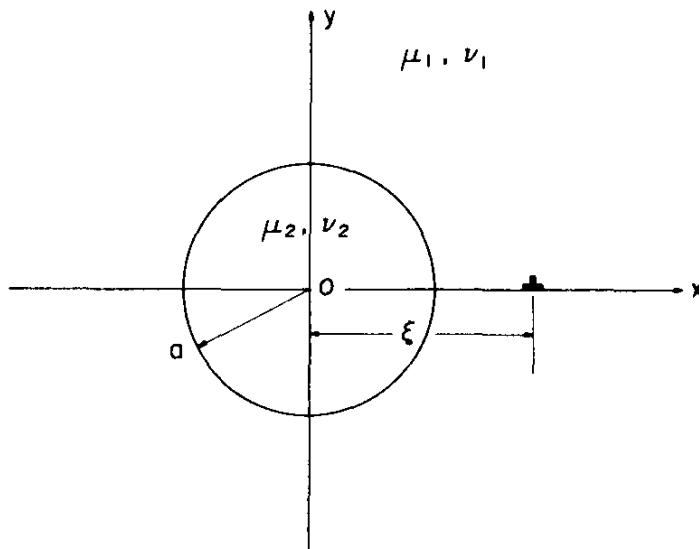


Fig. 49.1 A dislocation near a cylindrical inhomogeneity

For the edge dislocation, the stress at $(x, 0)$ becomes, for $x > a$,

$$\begin{aligned} \sigma_{yx} = & \frac{\mu_1 b}{2\pi(1-\nu_1)} \left[\frac{1}{x-\xi} - \frac{A+B}{2(x-a^2/\xi)} \right. \\ & - A \frac{(\xi/a)^2 - 1}{(\xi/a)^3} \left\{ 1 - \frac{(\xi/a)^2 - 1}{\xi/a} \frac{a}{x-a^2/\xi} \right\} \frac{a}{(x-a^2/\xi)^2} \\ & \left. + \frac{A+B}{2x} + (B-A) \frac{a}{2(\xi/a)x^2} - A \frac{a^2}{x^3} \right]. \end{aligned} \quad (49.2)$$

where b is the Burgers vector, and

$$\begin{aligned} A &= (1 - \mu_2/\mu_1)/(1 + \mu_2 \kappa_1/\mu_1), \\ B &= (\kappa_2 - \mu_2 \kappa_1/\mu_1)/(\kappa_2 + \mu_2/\mu_1), \\ \kappa_1 &= 3 - 4\nu_1, \quad \kappa_2 = 3 - 4\nu_2. \end{aligned} \quad (49.3)$$

Matsuoka et al. (1976) and Saito et al. (1977) have considered the effect of the free surface located near to the circular inclusion and a dislocation, more complicated cases have been considered by several authors. When a circular inclusion has a slipping interface, the elastic interaction between an edge dislocation and the inclusion is investigated by Dundurs and Gangadharan

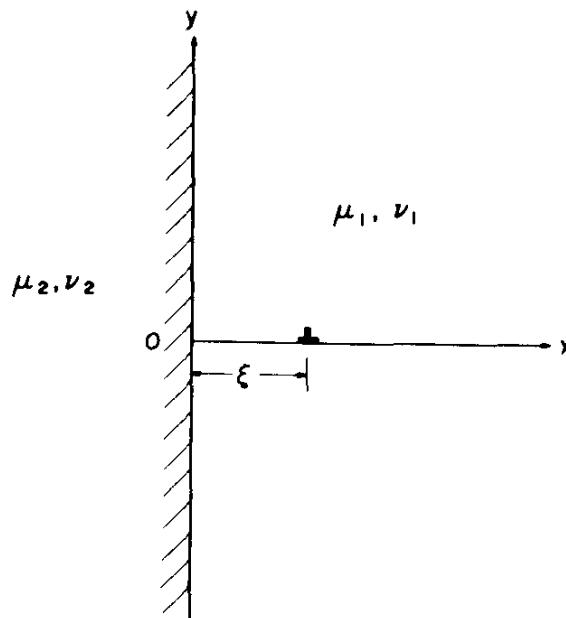


Fig. 49.2. A dislocation near a two-phase material.

(1969). The elastic interaction between a circular inclusion in a semi-infinite body and a screw dislocation is given by Tamate (1973). Sendeckyj (1970) has given the elastic field of a screw dislocation near an arbitrary number of circular inclusions. Tamate (1968) has investigated the behavior of a screw dislocation near a partially bonded bimetallic interface. The case for an edge dislocation is considered by Tamate and Kurihara (1970). A dislocation near or on a surface layer is treated by Weeks et al. (1968), Lee and Dundurs (1973), Masumura and Glicksman (1977) for the edge case. Gavazza and Barnett (1974), and Willis et al. (1972) have found the general solution for the interaction between a spherical inclusion and a screw dislocation. If the inclusion and a dislocation are far apart, the long-range interaction approximation is possible as shown by Barnett (1971), Comninou and Dundurs (1972), Yoo and Ohr (1972), Lin and Mura (1973), Yoo (1974), and Heinisch and Sines (1976). The interaction between a crack and a dislocation has been investigated by Tamate and Sekine (1972), and Rice and Thomson (1974). Hasebe et al. (1984) calculated the stress intensity factor of a kinked crack initiating from a rigid line inclusion.

If the inclusion is a half-space, we take $\xi/a \rightarrow 1$, $\{(\xi/a)^2 - 1\}a \rightarrow 2\xi$, $a - a^2/\xi \rightarrow \xi$ and $x - a^2/\xi \rightarrow x + \xi$ in (49.1) and (49.2) (see Fig. 49.2). Then, (49.1) and (49.2) become

$$\sigma_{yz} = \frac{\mu_1 b}{2\pi} \left(\frac{1}{x - \xi} + \frac{\mu_2 - \mu_1}{\mu_2 + \mu_1} \frac{1}{x + \xi} \right) \quad (49.4)$$

for the screw dislocation, and

$$\sigma_{yx} = \frac{\mu_1 b}{2\pi(1-\nu_1)} \left\{ \frac{1}{x-\xi} - \frac{A+B}{2(x+\xi)} - 2A\xi \frac{(x-\xi)}{(x+\xi)^3} \right\} \quad (49.5)$$

for the edge dislocation.

The above solutions agree with those obtained by Head (1953). The Peach-Koehler force can be obtained by $\sigma_{zy}b$ or $\sigma_{yx}b$ at $x = \xi$, where the term $1/(x - \xi)$ is omitted for the calculation.

The problem of a linear array of length L containing n dislocations piled up along the x -axis against the circular inclusion under the application of an applied stress σ_{zy}^0 (for screw) or σ_{yx}^0 (for edge) may be formulated as follows. For static equilibrium the force acting on any one dislocation in the pile-up due to all other pile-up dislocations must balance the force due to the applied stress. This requires that $\sigma_{zy} + \sigma_{zy}^0 = 0$ or $\sigma_{yx} + \sigma_{yx}^0 = 0$,

$$\frac{\mu_1 b}{2\pi} \left\{ \sum_{\substack{j=0 \\ j \neq i}}^{n-1} \frac{1}{x_i - x_j} + \frac{\mu_2 - \mu_1}{\mu_2 + \mu_1} \sum_{j=0}^{n-1} \left(\frac{1}{x_i - a^2/x_j} - \frac{1}{x_i} \right) \right\} + \sigma_{zy}^0 = 0, \quad (49.6)$$

for $i = 1, 2, \dots, n-1$.

for the screw dislocation array. A similar equation holds for the edge dislocation array.

When n is large, the discrete dislocations in the array can be replaced by a continuous distribution of dislocations with density $f(\xi)$. Equation (49.6) becomes

$$\frac{\mu_1 b}{2\pi} \int_a^{a+L} \left\{ \frac{1}{x-\xi} + \frac{\mu_2 - \mu_1}{\mu_2 + \mu_1} \left(\frac{1}{x-a^2/\xi} - \frac{1}{x} \right) \right\} f(\xi) d\xi + \sigma_{zy}^0 = 0 \quad (49.7)$$

for $a < x \leq L$.

Solving (49.6) yields values of x_1, x_2, \dots, x_{n-1} and n for a given constant σ_{zy}^0 . Equation (49.7) determines $f(x)$ and L for a given σ_{zy}^0 .

Solutions of these equations are obtained by many researchers (see a review paper by Chou and Li 1969). Eshelby, Frank and Nabarro (1951) have solved

the discrete dislocations pile-up problem in the homogeneous medium ($\mu_2 = \mu_1$, $\nu_1 = \nu_2$),

$$D \sum_{\substack{j=0 \\ j \neq i}}^{n-1} \frac{1}{x_i - x_j} - \sigma = 0 \quad \text{for } i = 1, 2, \dots, n-1, \quad (49.8)$$

where

$$\begin{aligned} D &= \mu_1 b / 2\pi, \\ \sigma &= -\sigma_{yz}^0, \end{aligned} \quad (49.9)$$

for the screw dislocation case, and

$$\begin{aligned} D &= \mu_1 b / 2\pi(1 - \nu_1), \\ \sigma &= -\sigma_{yx}^0, \end{aligned} \quad (49.10)$$

for the edge dislocation case. Equilibrium equation (49.8) holds for the $(n-1)$ locations. The leading dislocation at x_0 is a locked dislocation. Without losing generality, we can assume that $x_0 = 0$ and an obstacle is locking the leading dislocation. Eshelby et al. obtained x_1, x_2, \dots, x_{n-1} as the roots of the generalized Laguerre polynomial $L_{n-1}^{(1)}(x)$ by taking $D/2\sigma$ as a unit of length. The stress σ_{tip} acting on the locked dislocation is obtained as $(n-1)\sigma$, from the virtual work argument of Cottrell (1949). When the leading dislocation is displaced by δx_0 , the other $(n-1)$ dislocations are also displaced by δx_0 without losing the equilibrium since only the relative location of the dislocations is essential for the equilibrium. The work done by the applied stress σ on the $(n-1)$ dislocations during the virtual displacement δx_0 is $\sigma b(n-1)\delta x_0$ which must be equal to the work done by the locking force $\sigma_{tip}b$ on the leading dislocation. Namely, $\sigma b(n-1)\delta x_0 = \sigma_{tip}b\delta x_0$, which leads to $\sigma_{tip} = \sigma(n-1)$.

Stroh (1954, 1955) and Y.T. Chou (1967) have calculated details of the stress field and energy associated with the Eshelby, Frank and Nabarro problem. A modification of the Eshelby, Frank and Nabarro problem is made by Head and Thomson (1962) by considering a variation of the applied stress and by Y.T. Chou (1967) by taking a variation of the strength of the locked dislocation. J.C.M. Li (1969) has investigated the case of extended dislocations. The discrete dislocations in the Eshelby, Frank and Nabarro problem can also be replaced by a continuous distribution of dislocations as done by Leibfried

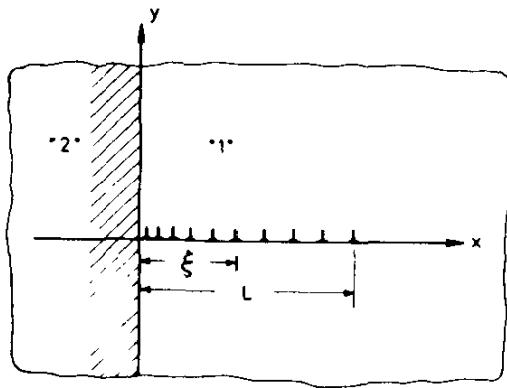


Fig. 49.3. Pile-up dislocations at the interface.

(1951). He has solved the integral equation

$$D \int_0^L \frac{f(\xi)}{x - \xi} d\xi - \sigma = 0 \quad \text{for } 0 < x \leq L, \quad (49.11)$$

and has obtained

$$f(x) = (\sigma/\pi D) \{ (L-x)/x \}^{1/2}. \quad (49.12)$$

The total number of dislocations is found to be

$$n = \int_0^L f(x) dx = \sigma L / 2D, \quad (49.13)$$

in agreement with the discrete dislocation case.

The pile-up problem in a two-phase medium (two half-spaces) requires solving an integral equation of the type (see Fig. 49.3)

$$D \int_0^L \left\{ \frac{1}{x - \xi} - \frac{\alpha}{x + \xi} - \frac{\beta \xi (x - \xi)}{(x + \xi)^3} \right\} f(\xi) d\xi - \sigma = 0 \quad \text{for } 0 < x \leq L \quad (49.14)$$

or

$$D \left\{ \sum_{\substack{j=0 \\ j \neq i}}^{n-1} \frac{1}{x_i - x_j} - \sum_{j=0}^{n-1} \frac{\alpha}{x_i + x_j} - \sum_{j=0}^{n-1} \frac{\beta x_j (x_i - x_j)}{(x_i + x_j)^3} \right\} - \sigma = 0 \quad (49.15)$$

for $i = 1, 2, \dots, n-1$,

where definitions (49.9) and (49.10) are used, and

$$\begin{aligned}\alpha &= (\mu_1 - \mu_2)/(\mu_1 + \mu_2), & \beta &= 0 && \text{(for screw),} \\ \alpha &= \frac{1}{2}(A + B), & \beta &= 2A && \text{(for edge).}\end{aligned}\quad (49.16)$$

The solution of (49.14) is found by Y.T. Chou (1965), Barnet and Tetelman (1966), Barnett (1967), and Smith (1967) for the screw dislocation case, and by Kuang and Mura (1968) for the edge dislocation case. The distribution density of the screw dislocations is

$$f(x) = \frac{2\sigma}{b\mu_1 \sin(\frac{1}{2}\pi s)} \sinh\left\{s \cosh^{-1}(L/x)\right\}, \quad 0 \leq x \leq L, \quad (49.17)$$

where

$$s = (2/\pi) \sin^{-1}\left\{\mu_1/(\mu_1 + \mu_2)\right\}^{1/2}. \quad (49.18)$$

The number of screw dislocations in the pile-up is then found to be

$$n = (\sigma L s / D) / \sin(s\pi). \quad (49.19)$$

The stress at the tip of the pile-up becomes

$$\sigma_{yz} = \frac{\sigma \cos(\frac{1}{2}s\pi)}{\sin^2(s\pi)} \left\{ \left(\frac{2L}{r} \right)^s - 2 \cos(\frac{1}{2}s\pi) \right\}, \quad (49.20)$$

where r is the distance from the tip in the x -direction. The order of stress singularity at $r = 0$ depends on the shear moduli of the two phases. The value of s is 1/2 for the homogeneous ($\mu_1 = \mu_2$) case.

The corresponding quantities for the case of edge dislocation pileup are too complicated to record here. Kuang and Mura (1968) showed that these quantities are depending on the roots s of the equation

$$\cos(\pi s) + \alpha - \beta(s - 1)^2 = 0. \quad (49.21)$$

Numerical examples of the edge dislocation distributions are shown in Fig. 49.4 where $\Gamma = \mu_2/\mu_1$. These distributions are very close to the screw dislocation distributions given by (49.17).

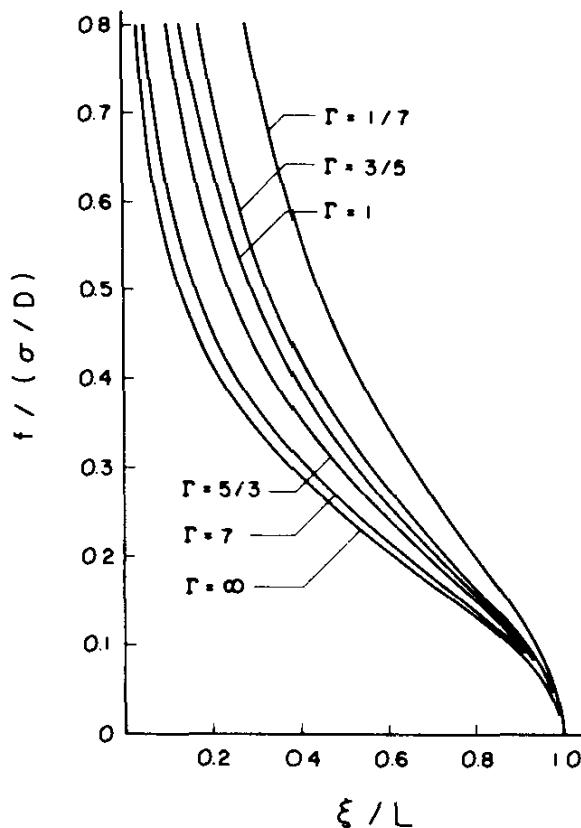


Fig. 49.4. Distribution of pile-up edge dislocations.

The nature of the stress singularity at the tip of the edge dislocation pile-up can be investigated by the following alternative method. Equation (49.14) is replaced by

$$D \int_0^\infty \left\{ \frac{1}{x-\xi} - \frac{\alpha}{x+\xi} - \frac{\beta \xi (x-\xi)}{(x+\xi)^3} \right\} f(\xi) d\xi - \sigma + H(x) = 0$$

for $0 < (x/L) < \epsilon$, (49.22)

where $\epsilon \ll 1$ and $H(x)$ is nonsingular as $x \rightarrow 0^+$.

Assuming that $f(\xi)$ is of the form

$$f(\xi) = f_0 / \xi^{1-k} \quad (0 < k \leq 1), (49.23)$$

we get

$$f_0 \pi D x^{k-1} \{ \cos(\pi k) - \alpha + \beta k^2 \} / \sin(\pi k) - \sigma + \bar{H}(x) = 0, (49.24)$$

where $\bar{H}(x)$ is not singular as $x \rightarrow 0^+$. Since only the first term in (49.24) is

singular as $x \rightarrow 0^+$, for identity to hold, the following relation must be satisfied:

$$-\cos(\pi k) + \alpha - \beta k^2 = 0. \quad (49.25)$$

The above equation is equivalent to (49.21) when $s - 1 = k$. The stress at the tip of the pile-up can be evaluated from the same integral in (49.22) except $x < 0$. The integral gives

$$\sigma_{12} = -\frac{(\mu_2)^2}{\mu_1} \frac{f_0}{\sin(\pi k)} \left\{ \frac{2k+1}{1+(3-4\nu_1)\mu_2/\mu_1} - \frac{2k-1}{(3-4\nu_2)+\mu_2/\mu_1} \right\} / r^{1-k}, \quad (49.26)$$

where $r = |x|$. The order of stress singularity at the tip is the same as that of dislocation distribution (49.25). Equations (49.22) ~ (49.24) are presented by Keer (1968) in a private communication.

Barnett (1969) has shown that

$$\begin{aligned} 0 < (1-k) \leq \frac{1}{2} & \quad \text{if } \mu_2/\mu_1 \geq (3-4\nu_2)/(3-4\nu_1), \\ \frac{1}{2} \leq (1-k) < 1 & \quad \text{if } \mu_2/\mu_1 \leq (3-4\nu_2)/(3-4\nu_1). \end{aligned} \quad (49.27)$$

The relation between k and Dundurs constants defined by (6.15) is discussed by Dundurs and Lee (1970).

The problem of screw dislocation pile-up against a circular cylinder requires solving (49.7). The exact solution is given by Barnett and Tetelman (1966, 1967). Toya (1976) has investigated the interfacial debonding caused by a screw dislocation pile-up at a circular cylindrical rigid inclusion.

Cracks in two-phase materials

The equilibrium conditions for pile-up dislocations can also be interpreted as the conditions for a fracture when the conditions for the free (crack) surface are identical to the equilibrium conditions for the dislocation array under an applied stress. Tucker (1973) has used the method of Kuang and Mura (1968, 1969) (the Melin transform and the Wiener-Hopf technique) and analyzed the behavior of Griffith, Zener-Stroh and Bullough-Gilman cracks in bi-metals. The effect of a circular inclusion on the stresses around a slit-like crack is investigated by Tamate (1968).

When a crack is located along the interface of a bi-metal, the well-known elastic solutions (e.g. Williams 1959, Erdogan 1963, England 1965, Rice and

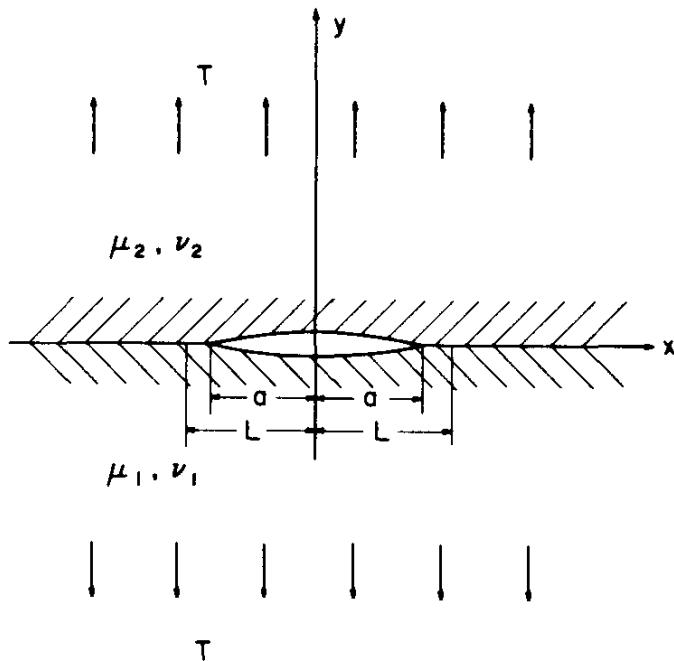


Fig. 49.5. A crack at an interface of a two-phase material.

Sih 1965, Clements 1971) characteristically involve oscillatory singularities. Comninou (1977) has reconsidered the problem using a continuous distribution of crack dislocations and found a non-oscillatory solution. She assumed that the crack is not completely open and that its faces are in frictionless contact near the crack tips (see also Comninou 1978, Dundurs and Comninou 1979, Comninou and Schmueser 1979). It has been shown by Achenbach et al. (1979) that the introduction of a small cohesive zone of a certain characteristic in the vicinity of the crack tip also serves to remove the corresponding oscillatory singularity. Recently, Hayashi and Nemat-Nasser (1981) have considered a branched crack at the interface of two dissimilar materials.

The analysis of Comninou is as follows. Consider a crack of length $2L$ lying in the interface of two elastic solids with shear moduli μ_1 and μ_2 and Poisson's ratio ν_1 , ν_2 as shown in Fig. 49.5. Under the action of uniform tension T applied at infinity, in the direction normal to the interface, the crack opens in the interval $(-a, a)$, where a is an unknown to be determined in the course of solution. We assume that a portion of the crack is closed and its two sides are in frictionless contact in the intervals $(-L, -a)$ and (a, L) . The boundary conditions are that the shear traction must vanish in $(-L, L)$ and that the normal traction must vanish in $(-a, a)$. To enforce these conditions we take advantage of the known stress fields for edge dislocations and consider a distribution $B_x(x)$ of glide dislocations in the interval $(-L, L)$, and a distribution $B_y(x)$ of climb dislocations in the interval $(-a, a)$.

When a discrete edge dislocation is located at point ξ on the interface between two solids, the induced interface tractions are (see Comninou 1977)

$$\begin{aligned}\sigma_{xy}(x, 0) &= Cb_x/\pi(x - \xi), \\ \sigma_{yy}(x, 0) &= -\beta Cb_x \delta(x - \xi),\end{aligned}\quad (49.28)$$

for a glide dislocation, and

$$\begin{aligned}\sigma_{xy}(x, 0) &= \beta Cb_y \delta(x - \xi), \\ \sigma_{yy}(x, 0) &= Cb_y/\pi(x - \xi),\end{aligned}\quad (49.29)$$

for a climb dislocation, where $\delta(x - \xi)$ is the Dirac delta function, and b_x and b_y are the components of the Burgers vector. Moreover,

$$C = \frac{2\mu_2(1 + \alpha)}{(\kappa_1 + 1)(1 - \beta^2)} = \frac{2\mu_2(1 - \alpha)}{(\kappa_2 + 1)(1 - \beta^2)}, \quad (49.30)$$

where α and β are the Dundurs constants (6.15).

The boundary conditions on the traction mentioned before lead to the integral equations which determine B_x and B_y . They are

$$C\beta B_y(x)[H(x + a) - H(x - a)] + \frac{C}{\pi} \int_{-L}^L \frac{B_x(\xi)}{x - \xi} d\xi = 0, \quad -L < x < L, \quad (49.31)$$

$$T - C \left[\beta B_x(x) - \frac{1}{\pi} \int_{-a}^a \frac{B_y(\xi)}{x - \xi} d\xi \right] = 0, \quad -a < x < a, \quad (49.32)$$

where $H(x)$ is the Heaviside unit step function. The condition for single-valued displacements for $|x| > L$ requires that

$$\int_{-L}^L B_x(\xi) d\xi = 0, \quad (49.33)$$

$$\int_{-a}^a B_y(\xi) d\xi = 0. \quad (49.34)$$

Denoting by $g(x)$ the gap between the solids and by $h(x)$ their relative slip, we have

$$\begin{aligned}g(x) &= u_y^{(2)}(x, 0) - u_y^{(1)}(x, 0), \\ h(x) &= u_x^{(2)}(x, 0) - u_x^{(1)}(x, 0).\end{aligned}\quad (49.35)$$

The definition of continuous distribution of dislocations (38.8) gives

$$B_x(x) = -dh/dx, \quad B_y(x) = -dg/dx. \quad (49.36)$$

Thus, we can assume that $B_y(x)$ is continuous in $(-a, a)$ and bounded at the ends $-a, a$. Then, (49.31) can be viewed as the Hilbert integral equation (A4.1) for the unknown function $B_x(x)$ and solved formally by treating $B_y(x)$ as known. Assuming $B_x(x)$ is unbounded at $-L$ and L , as the asymptotic analysis (Comninou 1977) indicates, $B_x(x)$ is obtained as (A.4.3), where $a_1 = -L, b_1 = L, c_{1+q} = -L, c_{2+q} = L, p = 1, q = 0$. The solution, using our condition (49.33), becomes

$$B_x(\bar{x}) = -\frac{(\beta/\pi)}{(1-\bar{x}^2)^{1/2}} \int_{-\bar{a}}^{\bar{a}} \frac{(1-t^2)^{1/2}}{(t-\bar{x})} B_y(t) dt, \quad -1 < \bar{x} < 1, \quad (49.37)$$

where

$$\bar{x} = x/L, \quad \xi/L = t, \quad a/L = \bar{a}. \quad (49.38)$$

Condition (49.34) is satisfied by symmetry. Since B_y is bounded, $B_x(x)$ is square root singular at L and $-L$.

Substituting $B_x(\bar{x})$ from (49.37) into (49.32), we have

$$-\frac{\beta^2}{(1-\bar{x}^2)^{1/2}} \int_{-\bar{a}}^{\bar{a}} \frac{(1-t^2)^{1/2}}{t-\bar{x}} B_y(t) dt + \int_{-\bar{a}}^{\bar{a}} \frac{1}{t-\bar{x}} B_y(t) dt = \pi T/C$$

$$-\bar{a} < \bar{x} < \bar{a}$$

$$(49.39)$$

which is a generalized Cauchy singular equation to determine $B_y(t)$. A numerical solution can be obtained by the method developed by Erdogan and Gupta (1972).

The singular behavior of the interface tractions can easily be established. The tractions at the interface are

$$\sigma_{xy} = \frac{C}{\pi} \int_{-L}^L \frac{B_x(\xi)}{x-\xi} d\xi, \quad |x| > L$$

$$= -\frac{\beta C}{\pi} \frac{\operatorname{sgn} \bar{x}}{(\bar{x}^2-1)^{1/2}} \int_{-\bar{a}}^{\bar{a}} \frac{B_y(t)(1-t^2)^{1/2}}{t-\bar{x}} dt, \quad \bar{x} > 1, \quad (49.40)$$

and

$$\begin{aligned}\sigma_{yy}(x, 0) &= T + \frac{C}{\pi} \int_{-a}^a \frac{B_y(\xi)}{x - \xi} d\xi, \quad |x| > L \\ &= T - C\beta B_x(x) + \frac{C}{\pi} \int_{-a}^a \frac{B_y(\xi)}{x - \xi} d\xi, \quad L > |x| > a.\end{aligned}\quad (49.41)$$

It can be seen that $\sigma_{xy}(x, 0)$ has square-root singularities at L and $-L$ and $\sigma_{yy}(x, 0)$ is bounded at L^+ , $-L^+$ but has square-root singularities at L^- , $-L^-$ because of the term $-C\beta B_x(x)$. Recently, Mak et al. (1980) have shown that both the normal and the shear stresses are singular at the crack tip when the interface adhesive zones have no-slip.

Finally, the solution of (49.39) must satisfy the following conditions: for no interpenetration,

$$g(x) > 0, \quad |x| < a, \quad (49.42)$$

and for contact,

$$\sigma_{yy}(x, 0) \leq 0, \quad a < |x| < L. \quad (49.42.1)$$

Comninou showed a numerical example for the case $a/L = 1 - 10^{-4}$ and $\beta = 0.4854$. Her result shows that $\sigma_{xy}(x, 0)(x^2/L^2 - 1)^{1/2}/T = 1.050$, $\sigma_{yy}(x, 0)/T = 23.36$ at $x/L = 1.0000$. These values cannot be obtained from the classical solution (e.g. England 1965) due to its oscillatory singularities. However, her solution is very close to England's solution when $x/L > 1.0002$. England's solution is

$$\sigma_{xy}(x, 0) = \frac{TL^{1/2}}{[2(x - L)]^{1/2}} (2\epsilon \cos \epsilon s - \sin \epsilon s), \quad x > L, \quad (49.43)$$

where

$$\epsilon = \frac{1}{2\pi} \log \frac{1 - \beta}{1 + \beta}, \quad s = \epsilon \log \frac{2L}{x - L}. \quad (49.44)$$

The crack extension force \mathcal{G}_1 defined by (34.6) becomes almost identical in both of the solutions and it can be approximated by

$$\mathcal{G}_1 \approx \pi K_2^2 / 4C, \quad (49.45)$$

where

$$K_2 = TL^{1/2}(1 + 4\epsilon^2)^{1/2}, \quad \epsilon \neq 0. \quad (49.46)$$

Comninou's consideration has been extended to a penny-shaped crack at the interface of two bonded dissimilar half-spaces by Keer et al. (1978). By considering contact zones, these solutions have no oscillatory singularities. The solutions obtained by Mossakovskii and Rykba (1964), Keer (1967), Willis (1972), and Lowengrub and Sneddon (1974) have the oscillatory singularities since their penny-shaped cracks are of Griffith type. However, for cracks in tension fields the oscillatory zone is very small compared to the radius of the crack and these solutions give good approximations outside the zone. The oscillatory zone is no longer small for a crack under shear fields. The solution of Lowengrub and Sneddon (1974) for a crack under internal pressure p gives

$$\sigma_{zz}(\rho, 0^+) = (K/\pi)\{S_1(\rho) + \frac{1}{2}\gamma S_2(\rho)\} \quad (49.47)$$

at the interface $z = 0$, where ρ is the dimensionless radius,

$$K = p/(\kappa_2 + \Gamma)^{1/2}(1 + \kappa_1\Gamma)^{1/2},$$

$$\Gamma = \mu_2/\mu_1, \quad \kappa = 3 - 4\nu, \quad (49.48)$$

$$\gamma = \frac{1}{2\pi} \log \frac{\kappa_1\Gamma + 1}{\Gamma + \kappa_2},$$

and

$$S_1(\rho) = -\frac{1}{\rho} \frac{\partial}{\partial\rho} \int_0^1 \frac{x^2 \cos(\gamma\theta)}{(\rho^2 - x^2)^{1/2}} dx,$$

$$S_2(\rho) = -\frac{1}{\rho} \frac{\partial}{\partial\rho} \int_0^1 \frac{x \sin(\gamma\theta)}{(\rho^2 - x^2)^{1/2}} dx, \quad (49.49)$$

$$\theta = \frac{1}{2} \log \frac{1+x}{1-x}.$$

50. Eigenstrains in lattice theory

In lattice theory (e.g. Born and Huang 1954, Maradudin 1958) a change of potential energy caused by a deformation is expressed by a function of atomic displacements measured from a perfect lattice state. For harmonic approximation the potential energy is expressed by a quadratic form of the atomic displacement components. The coefficients of the polynomial are chosen such that a uniform displacement (rigid motion) does not create any potential energy of deformation. The potential energy, however, does not vanish when a part of the material is displaced plastically and reformed into a perfect crystal. When a perfect crystal is deformed into another perfect crystal accompanying only a plastic shape change, no change of potential energy is expected. Therefore the classical lattice theory should be modified so that the potential energy does vanish for such a plastic deformation. A similar question about the incompleteness of the classical theory has been raised by Kuriyama (1967) from a different viewpoint.

It is shown that such a difficulty in the classical lattice theory simply can be eliminated when the concept of eigenstrains in continuum mechanics is introduced into the lattice theory. In continuum mechanics the eigenstrain has played an important role in analysis of elastic fields caused by line and point imperfections, inclusions, inhomogeneities, etc. The introduction of the concept of eigenstrains into lattice theory provides similar advantages and eliminates the above-mentioned difficulty in classical lattice theory.

The equation of motion of atom i in the classical lattice theory with the harmonic approximation is

$$M\ddot{u}_i(l) = - \sum_{l'} \phi_{ij}(ll') u_j(l'), \quad (50.1)$$

where the summation with respect to l' is referred to all atoms, M is the mass of an atom, $u_i(l)$ the displacement component from the perfect lattice state of atom i , indicating the position, and $\phi_{ij}(ll')$ are the force constants between i and l' atoms. The double indices appearing in the equations follow the summation convention from 1 to 3. The force constants have the following properties, since the right-hand side in equation (50.1) is zero for a constant displacement,

$$\sum_{l'} \phi_{ij}(ll') = \sum_l \phi_{ij}(ll') = 0. \quad (50.2)$$

Furthermore, the symmetry condition leads to

$$\phi_{ij}(ll') = \phi_{ji}(ll') = \phi_{ij}(l'l). \quad (50.3)$$

The summations with respect to l' in (50.1) and (50.2) are taken for all atoms including l . It is obvious from (50.2) that $\sum_{l'} \phi_{ij}(ll') u_j(l) = 0$. Then (50.1) can be written as

$$M\ddot{u}_i(l) = - \sum_{l'} \phi_{ij}(ll') [u_j(l') - u_j(l)]. \quad (50.4)$$

If we put

$$u_j(l') - u_j(l) = \beta_{kj}(ll') x_k(l'l), \quad (50.5)$$

$$x_k(l'l) = x_k(l') - x_k(l), \quad (50.6)$$

where $x_k(l)$ is a coordinate component of l atom in the perfect lattice state, we write (50.4)

$$M\ddot{u}_i(l) = - \sum_{l'} \phi_{ij}(ll') \beta_{kj}(ll') x_k(l'l). \quad (50.7)$$

It is important to keep in mind that (50.1) and (50.7) are equivalent only when $\beta_{kj}(ll')$ is compatible, namely, only when (50.5) holds. However, (50.5) does not always hold. If the material is involved in any inelastic phenomena, for instance, in plastic deformation, (50.5) does not hold and instead

$$u_j(l') - u_j(l) = [\beta_{kj}(ll') + \beta_{kj}^*(ll')] x_k(l'l). \quad (50.8)$$

Now we can call β_{kj} and β_{kj}^* the elastic distortion and eigendistortion, respectively, and their symmetric parts are called the elastic strain and eigenstrain, respectively. These definitions are strictly analogous to those of continuum mechanics, and the total strain is the sum of the elastic strain and eigenstrain. We assert that the fundamental equation of motion is (50.7) rather than (50.1), and condition (50.8) is subjected to equation (50.7). Equation (50.7) becomes (50.1) only when $\beta_{kj}^* = 0$. The plastic deformation which transforms a perfect crystal into another perfect crystal, accompanying a permanent shape change, can be described by zero elastic distortion (or strain), that is, $\beta_{kj} = 0$. In most inelastic problems u_i and β_{kj} are unknown quantities to be determined, while the components of β_{kj}^* are a priori given by the nature of the problems. For plastic deformations, $\beta_{kj}(ll') x_k(l'l)$ and $\beta_{kj}^*(ll') x_k(l'l)$ are the elastic and plastic displacements, respectively. The last displacement corresponds to the magnitude of glide which is a multiple of the lattice spacing. Analytically, however, it is expressed by a continuous function

of coordinates of a generalized function. $\beta_{kj}(ll')$ and $\beta_{kj}^*(ll')$ are defined between atoms l and l' and are equal to $\beta_{kj}(l'l)$ and $\beta_{kj}^*(l'l)$, respectively.

When $\beta_{kj}(ll')$ is eliminated from (50.7) and (50.8), we have

$$M\ddot{u}_i(l) = - \sum_{l'} \phi_{ij}(ll') u_j(l') + \sum_{l'} \phi_{ij}(ll') \beta_{kj}^*(ll') x_k(l'l). \quad (50.9)$$

Comparing (50.9) and (50.1) it can be seen that an extra term appears as a fictitious body force acting on atom l .

The potential energy in the classical lattice theory is within the harmonic approximation

$$\Phi = \frac{1}{2} \sum_{l,l'} \phi_{ij}(ll') u_i(l) u_j(l') \quad (50.10)$$

or

$$\Phi = -\frac{1}{2} \sum_{l' \neq l} \phi_{ij}(ll') [u_i(l') - u_i(l)] [u_j(l') - u_j(l)]. \quad (50.10.1)$$

The last expression (50.10.1) has been obtained by the use of (50.2) and the equality

$$\sum_{l' \neq l} \phi_{ij}(ll') u_i(l') u_j(l) + \sum_{l' \neq l} \phi_{ij}(ll') u_i(l) u_j(l') = \sum_{l,l'} \phi_{ij}(ll') u_i(l) u_j(l'), \quad (50.10.2)$$

where the summations $\sum_{l' \neq l}$ and $\sum_{l',l}$ are combinational and permutational, respectively.

Our modified theory asserts that the potential energy is not associated with the total displacement u_i but the elastic displacement defined by $\beta_{ki}(ll') x_k(l'l)$. Then, the new expression of the potential energy becomes, instead of (50.10),

$$\Phi = -\frac{1}{2} \sum_{l' \neq l} \phi_{ij}(ll') \beta_{ki}(ll') x_k(l'l) \beta_{mj}(ll') x_m(l'l). \quad (50.11)$$

If a component of stress tensor is defined as

$$\sigma_{ik}(ll') = -b^{-3} \phi_{ij}(ll') \beta_{mj}(ll') x_m(l'l) x_k(l'l) \quad (50.12)$$

(50.11) can be written as

$$\Phi = \frac{1}{2} b^3 \sum_{i,i'} \sigma_{ik}(ll') \beta_{ki}(ll'). \quad (50.13)$$

The definition of stress (50.12) has been chosen such that it agrees with one in the continuum mechanics for limiting case $b \rightarrow 0$. Similarly to continuum mechanics, the potential energy (the elastic strain energy caused by β_{ki}^*) can be written as

$$\Phi = -\frac{1}{2}b^3 \sum_{l'-l} \sigma_{ik}(ll') \beta_{ki}^*(ll') \quad (50.14)$$

for the static case. It is proved as follows. Equation (50.11) is written from (50.8) as

$$\Phi = -\frac{1}{2} \sum_{l'-l} \phi_{ij}(ll') \beta_{mj}(ll') x_m(l'l) [u_i(l') - u_i(l) - \beta_{ki}^*(ll') x_k(l'l)]. \quad (50.15)$$

For the static case (50.7) and (50.3) lead to

$$\sum_l \phi_{ij}(ll') \beta_{mj}(ll') x_m(l'l) = 0, \quad \sum_{l'} \phi_{ij}(ll') \beta_{mj}(ll') x_m(l'l) = 0 \quad (50.16)$$

and therefore (50.15) can be simplified to

$$\Phi = \frac{1}{2} \sum_{l'-l} \phi_{ij}(ll') \beta_{mj}(ll') x_m(l'l) x_k(l'l) \beta_{ki}^*(ll') \quad (50.17)$$

which is equivalent to (50.14) due to (50.12). Equation (50.14) has the same form as (13.3).

A uniformly moving screw dislocation

A straight screw dislocation along the x_3 axis is uniformly moving to the x_1 direction with a constant velocity v . At time $t = 0$, $l_{1/2,1/2}$, $l_{1/2,-1/2}$ atoms (see Figure 50.1) are in a quarter way jumping the next potential well. For mathematical simplicity, only the nearest neighborhood interaction is considered for a cubic crystal. Now we assume that

$$\begin{aligned} \phi_{33}(l_{m,n} l_{m+1,n}) &= -A, \\ \phi_{33}(l_{m,n} l_{m,n+1}) &= -B, \\ \phi_{33}(l_{m,n} l_{m,n}) &= 2(A + B). \end{aligned} \quad (50.18)$$

since the plastic displacement $\beta_{23}^*(ll')x_2(l'l)$ must be b , we take $\beta_{23}^*(ll') = 1$

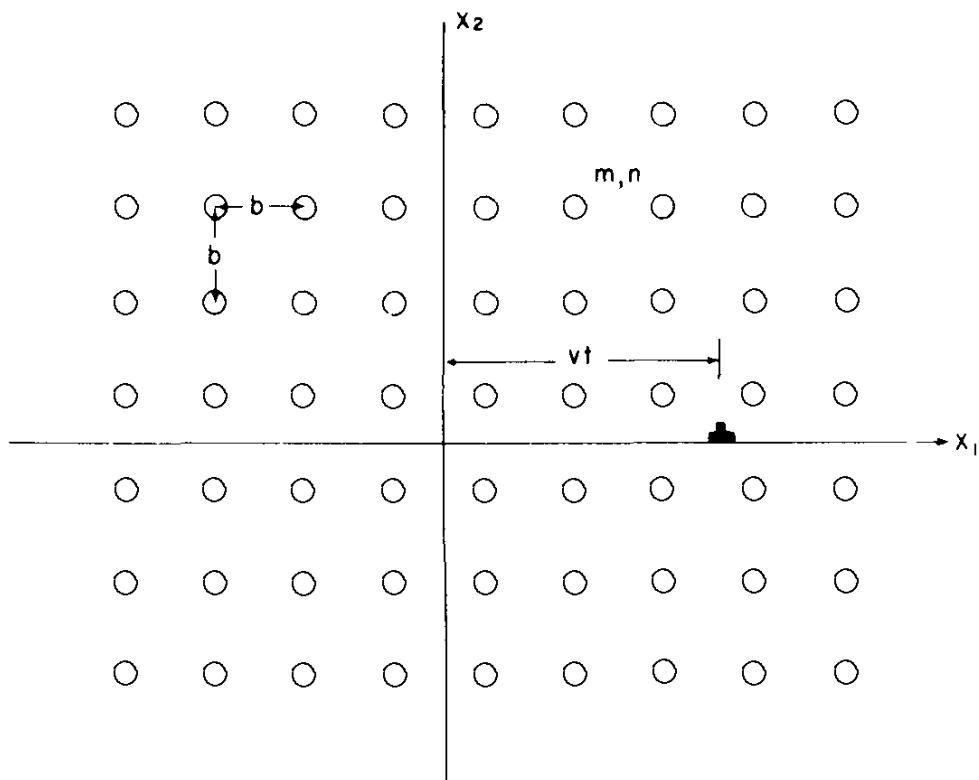


Fig. 50.1 A uniformly moving screw dislocation with velocity v . The atom positions are denoted by integers l , m and n

between $l = l_{k+1/2,1/2}$ and $l' = l_{k+1/2,-1/2}$ atoms (k = integer between $-\infty$ and vt/b). Then, our fundamental equation (50.9) becomes

$$\begin{aligned} M\ddot{u}_3(l_{m,n}) &= A[u_3(l_{m+1,n}) + u_3(l_{m-1,n})] \\ &\quad + B[u_3(l_{m,n+1}) + u_3(l_{m,n-1})] - 2(A+B)u_3(l_{m,n}) \end{aligned} \quad (50.19)$$

$$+ Bb \sum_{k=-\infty}^{vt/b} (\delta_{m,k+1/2}\delta_{n,1/2} - \delta_{m,k+1/2}\delta_{n,-1/2}),$$

where $\delta_{m,n}$ is the Kronecker delta. Although vt/b has been assumed as an integer, the analytic nature of our final solution can extend the solution to any value of vt/b .

Since

$$\delta_{m,k \pm 1/2} = \frac{1}{2\pi} \int_{-\pi}^{\pi} \exp\{i(m-k \mp \frac{1}{2})\xi_1\} d\xi_1, \quad (50.20)$$

the solution of (50.19) is assumed as

$$u_3(l_{m,n}) = \frac{1}{(4\pi)^2} \int_{-\pi}^{\pi} \int \bar{u}_3(\xi_1, \xi_2) \exp\left\{i\left(m - \frac{vt}{b}\right)\xi_1 + in\xi_2\right\} d\xi_1 d\xi_2. \quad (50.21)$$

$\bar{u}_3(\xi_1, \xi_2)$ is easily obtained as a solution of an algebraic equation when (50.20) and (50.21) are substituted into (50.19). Then, we have

$$u_3(l_{m,n}) = \pm \frac{b}{4} - \frac{b}{\pi^2} \int_0^{\pi/2} \int \frac{\sin \xi_2}{\sin \xi_1} \frac{\sin(X\xi_1) \sin(2n\xi_2)}{(A/B) \sin^2 \xi_1 + \sin^2 \xi_2 - V^2 \xi_1^2} d\xi_1 d\xi_2, \quad (50.22)$$

where

$$X = 2(m - vt/b), \quad V^2 = Mv^2/Bb^2 = v^2/c^2, \quad (50.23)$$

and c is the sound velocity of shear waves. The plus and minus signs of the first term in the right-hand side in (50.22) are taken for $n > 0$ and $n < 0$, respectively. When $V = 0$, (50.22) agrees with the result obtained by Maradudin (1958). When $A = B$ and $\xi_1/b = \bar{\xi}_1$, $\xi_2/b = \bar{\xi}_2$, $mb = x_1$, $nb = x_2$, $b \rightarrow 0$, (50.22) leads to the classical solution of continuum theory,

$$u_3(x_1, x_2) = (b/2\pi) \tan^{-1}(x_2/x'_1), \quad x'_1 = (x_1 - vt)/(1 - V^2)^{1/2}. \quad (50.24)$$

The displacement due to applied stress $\sigma_{23} = \sigma$ is obtained as

$$u_3(l_{m,n}) = nb^2\sigma/B. \quad (50.25)$$

The total displacement w is

$$w = (50.22) + (50.25). \quad (50.26)$$

The displacement field is anti-symmetric about $x_2 = 0$ plane. The displacement of atoms just above the slip plane is obtained by taking $n = \frac{1}{2}$. The numerical values are shown in Figure 50.2 for various values of V in case $A/B = 1$ and $B = \mu b$ (μ = shear modulus), where σ/μ is chosen such that

$$w/(\frac{1}{2}b) = 0.25 \quad \text{at } X = 1 \quad (50.27)$$

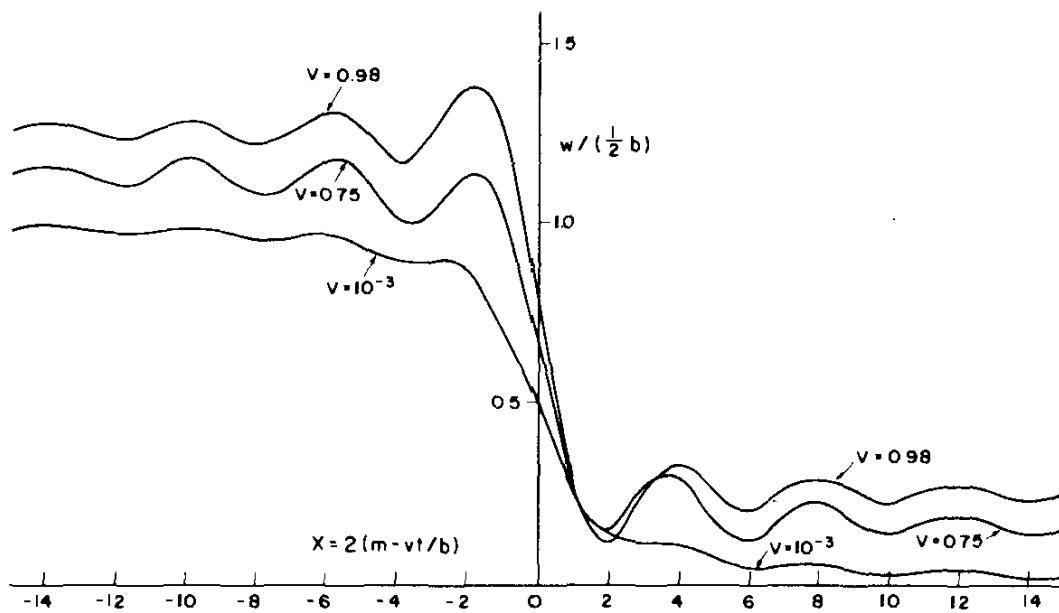


Fig. 50.2. The displacement of atoms just above the slip plane for various velocities of the dislocation, where $V = v/c$. The dislocation is in the position $X = 1$.

is satisfied. The last condition means that the atoms at the dislocation position are in a quarter way jumping to the next potential well. The applied stress σ satisfying condition (50.27) becomes a function of V as shown in Figure 50.3.

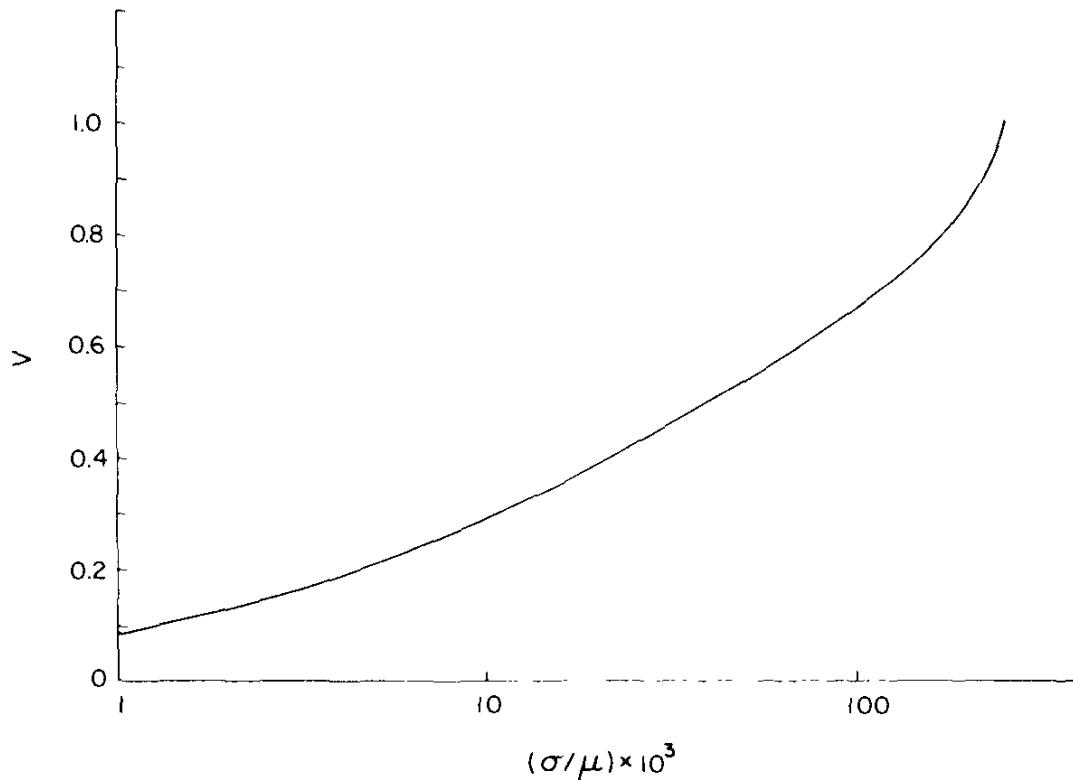


Fig. 50.3. Applied stress necessary to keep the dislocation in motion with velocity v .

In continuum theory no applied stress is necessary to maintain the dislocation in the uniform motion for $V < 1$ and the displacement profile of the upper slip plane is $w/\frac{1}{2}b = 1$ for $X < 0$ and $w = 0$ for $0 < X$.

Comparing the result shown in Figure 50.2 with the result of one-dimensional chain model obtained by Earmme and Weiner (1974), the present theory predicts the damping of oscillation of atoms in the trail of the dislocation and also predicts an oscillation of atoms ahead of the dislocation.

The theory in this section can be applied equally to vacancies and moving cracks as shown by Mura (1978).

There seems to exist a modern trend that the micromechanics goes to more microscopic scales as seen in recent publications, e.g. Kunin (1982), Weiner (1983), Yonezawa and Ninumiya (1983), Morinaga et al. (1984) and Eberhart et al., (1985).

51. Sliding inclusions

Inclusions and inhomogeneities considered hitherto are perfectly bonded to matrices. The condition of perfect bonding is sometimes inadequate in describing mechanical behavior of inclusions. Inclusions in high-strength steel, for instance, are easily debonded by a few cyclic loadings. Grain boundary sliding in polycrystals and granular media, debonding of fiber elements in composite materials are also common phenomena.

Consider an infinitely extended elastic body D , containing a uniform eigenstrain ϵ_{ij}^* in an ellipsoidal subdomain Ω . We investigate a solution of the elastic field when sliding takes place along the inclusion interface S . Although some laws of friction must be introduced on the inclusion interface, it is assumed, for simplicity, that the interface cannot sustain any shear traction. This free sliding inclusion is subjected to the following conditions:

$$\begin{aligned}\sigma_{ij,j} &= 0 && \text{in } D \\ \sigma_{ij} &= C_{ijkl}(u_{k,l} - \epsilon_{kl}^*) && \text{in } \Omega \\ \sigma_{ij} &= C_{ijkl}u_{k,l} && \text{in } D - \Omega \\ [\sigma_{ij}]n_j &= 0 && \text{on } S \\ [u_i]n_i &= 0 && \text{on } S \\ \sigma_{ij}n_i - \sigma_{jk}n_jn_kn_i &= 0, && \text{on } S\end{aligned}\tag{51.1}$$

where

$[] = [\text{out}] - [\text{in}]$, and n_i is the outward normal to S .

The last two conditions are different from the conditions for the perfect bonding. For the perfect bonding, $[u_i] = 0$ and the shear stress along the interface, $\sigma_{ij}n_j - \sigma_{jk}n_jn_kn_i$, has no restriction.

The shear stress on the inclusion interface in the perfect bonding case (Eshelby's inclusion case) is relaxed by the interface sliding (Somigliana's dislocation creation). Therefore, the solution for (51.1) is the sum of Eshelby's solution (6.1) and Volterra's solution (7.6) with Somigliana's dislocations b_i defined by

$$-[u_i] = b_i. \quad (51.2)$$

The b_i must be determined from the last two conditions in (51.1). Then, the displacement is

$$u_i = u_i^E + u_i^V, \quad (51.3)$$

where

$$u_i^E(x) = -\frac{\partial}{\partial x_k} \int_{\Omega} G_{ij}(x - x') C_{jkmn} \epsilon_{nm}^*(x') dx', \quad (51.4)$$

$$u_i^V(x) = \frac{\partial}{\partial x_k} \int_S G_{ij}(x - x') C_{jkmn} b_n(x') n_m dS(x'). \quad (51.5)$$

The corresponding stress is

$$\sigma_{ij} = \sigma_{ij}^E + \sigma_{ij}^V, \quad (51.6)$$

where

$$\begin{aligned} \sigma_{ij}^E &= C_{ijkl} (u_{k,l}^E - \epsilon_{kl}^*), \\ \sigma_{ij}^V &= C_{ijkl} u_{k,l}^V. \end{aligned} \quad (51.7)$$

Following Asaro (1975), \mathbf{b} defined on S is extended to the inside Ω . Then, Gauss' theorem is applied to (51.5),

$$u_i^V(x) = \frac{\partial}{\partial x_k} \int_{\Omega} G_{ij}(x - x') C_{jkmn} b_{n,m}(x') dx' + b_i(x). \quad (51.8)$$

Therefore, we have

$$\begin{aligned}\sigma_{ij}^E(x) &= -C_{ijkl} \left\{ \int_{\Omega} C_{pqmn} \epsilon_{nm}^*(x') G_{kp,ql}(x-x') dx' + \epsilon_{kl}^*(x) \right\} \\ \sigma_{ij}^V(x) &= -C_{ijkl} \left\{ \int_{\Omega} C_{pqmn} \epsilon_{nm}^{**}(x') G_{kp,ql}(x-x') dx' + \epsilon_{kl}^{**}(x) \right\},\end{aligned}\quad (51.9)$$

where

$$\epsilon_{ij}^{**} = -\frac{1}{2}(b_{i,j} + b_{j,i}) \quad (51.10)$$

Shearing Eigenstrains

Mura and Furuhashi (1984) have found that the stress σ_{ij} in (51.1) is identically zero when ϵ_{ij}^{**} is of the shear type, ϵ_{12}^* , ϵ_{23}^* , ϵ_{31}^* , where the coordinate system is taken in the principal axis directions of Ω .

Observing (51.9), we realize that $\sigma_{ij} = 0$ if

$$\epsilon_{ij}^* + \epsilon_{ij}^{**} = 0. \quad (51.11)$$

and all the conditions in (51.1) are satisfied except the condition $[u_i]n_i = 0$ on S . It is known that $[u_i] = -b_i$. The condition

$$b_i n_i = 0 \quad \text{on } S \quad (51.12)$$

is, however, satisfied when ϵ_{ij}^* is of the shear type. Because (51.10) and (51.11) lead to

$$b_i = \epsilon_{ij}^* x_j - \omega_{ij} x_j \quad (51.13)$$

and $\omega_{ij} = -\omega_{ji}$ is a rotation which is chosen so that the condition (51.12) is satisfied. Since ϵ_{ij}^* is of the shear type, when (51.13) is substituted into (51.12), we have

$$\begin{aligned}&\{\epsilon_{12}^*(1/a_1^2 + 1/a_2^2) - \omega_{12}(1/a_1^2 - 1/a_2^2)\}x_1 x_2 \\ &+ \{\epsilon_{31}^*(1/a_3^2 + 1/a_1^2) - \omega_{31}(1/a_3^2 - 1/a_1^2)\}x_3 x_1 \\ &+ \{\epsilon_{23}^*(1/a_2^2 + 1/a_3^2) - \omega_{23}(1/a_2^2 - 1/a_3^2)\}x_2 x_3 = 0\end{aligned}\quad (51.14)$$

where

$$\begin{aligned} n_1 &= (x_1/a_1^2)/(x_1^2/a_1^4 + x_2^2/a_2^4 + x_3^2/a_3^4)^{1/2}, \\ n_2 &= (x_2/a_2^2)/(x_1^2/a_1^4 + x_2^2/a_2^4 + x_3^2/a_3^4)^{1/2}, \\ n_3 &= (x_3/a_3^2)/(x_1^2/a_1^4 + x_2^2/a_2^4 + x_3^2/a_3^4)^{1/2}. \end{aligned} \quad (51.15)$$

When $a_1 \neq a_2 \neq a_3$, there exists a set of solutions of ω_{ij} satisfying (51.14). They are

$$\begin{aligned} \omega_{12} &= \epsilon_{12}^*(1/a_1^2 + 1/a_2^2)/(1/a_1^2 - 1/a_2^2), \\ \omega_{31} &= \epsilon_{31}^*(1/a_3^2 + 1/a_1^2)/(1/a_3^2 - 1/a_1^2), \\ \omega_{23} &= \epsilon_{23}^*(1/a_2^2 + 1/a_3^2)/(1/a_2^2 - 1/a_3^2). \end{aligned} \quad (51.15)$$

Therefore, it is concluded that for uniform shear eigenstrains a free-sliding ellipsoidal inclusion causes no stress field. This means that the misfit caused by ϵ_{ij}^* in Eshelby's inclusion is completely relaxed by the interface sliding given by (51.13) with (51.15). It means equivalently that the displacement $u_i = \epsilon_{ij}^* x_j - \omega_{ij} x_j$ transforms the ellipsoid $x_1^2/a_1^2 + x_2^2/a_2^2 + x_3^2/a_3^2 = 1$ into the identical ellipsoid $(x'_1)^2/a_1^2 + (x'_2)^2/a_2^2 + (x'_3)^2/a_3^2 = 1$ by the transformation

$$x'_i = x_i + b_i. \quad (51.16)$$

This is surprisingly true in the framework of small deformation theory.

When Ω is a sphere ($a_1 = a_2 = a_3 = a$), the terms in (51.14) containing ω_{ij} disappear and therefore b_i expressed by (51.13) does not satisfy the condition $b_i n_i = 0$. Mura and Furuhashi (1984) have found that when

$$b_i = B_{ij} x_j a^2 - B_{jk} x_j x_k x_i, \quad (51.17)$$

with

$$B_{ij} = \frac{14(7-5\nu)}{3(25-2\nu)a^2} \epsilon_{ij}^* \quad (51.18)$$

is chosen, u_i and σ_{ij} evaluated from (51.14) (51.15), (51.9), (51.10), satisfy all the conditions in (51.1). ν is Poisson's ratio and a is the radius of Ω . The expression (51.17) is found from the intuition that b_i must have the same form as the shear stress, $\sigma_{ij} x_j a^2 - \sigma_{jk} x_j x_k x_i$, at the interface. It is also suspected

from the solution of Ghahremani (1980) who has obtained the solution for an isotropic elastic medium, containing a sliding spherical inhomogeneity, subjected to uniform tension at infinity.

Spheroidal inhomogeneous inclusions

Consider a spheroidal inhomogeneous inclusion Ω in an infinite isotropic body, where $a_1 = a_2$ and the eigenstrain components in Ω are restricted to $\epsilon_{11}^* = \epsilon_{22}^* \neq \epsilon_{33}^*$ so that the symmetry about the x_3 axis is conserved. The body is subjected to all-around tension about the x_3 axis or anaxial tension in the x_3 direction in addition to the uniform eigenstrain ϵ_{ij}^* in Ω . The shear modulus and Poisson's ratio are μ and ν in the matrix and $\bar{\mu}$ and $\bar{\nu}$ in Ω . The inclusion can slide along the interface and no shear stress is allowed along the interface.

We follow the method developed by Edwards (1951). Assume that Ω is prolate as shown in fig. 51.1(a), where $a_1 = a_2 = a$, $a_3 = b$ and $b > a$. When Ω is oblate ($b < a$), its solution can be predicted from the solution for the prolate case according to Sadowsky and Sternberg (1947).

The prolate spheroidal coordinate system (α, β, γ) is introduced as shown in Fig. 51.1(b). The cartesian coordinates (x, y, z) are written as

$$\begin{aligned} x &= c \sinh \alpha \sin \beta \cos \gamma, \\ y &= c \sinh \alpha \sin \beta \sin \gamma, \\ z &= c \cosh \alpha \cos \beta, \end{aligned} \tag{51.19}$$

where c is the focal distance in the z axis. The surface of the ellipsoid is defined by $\alpha = \alpha_0$.

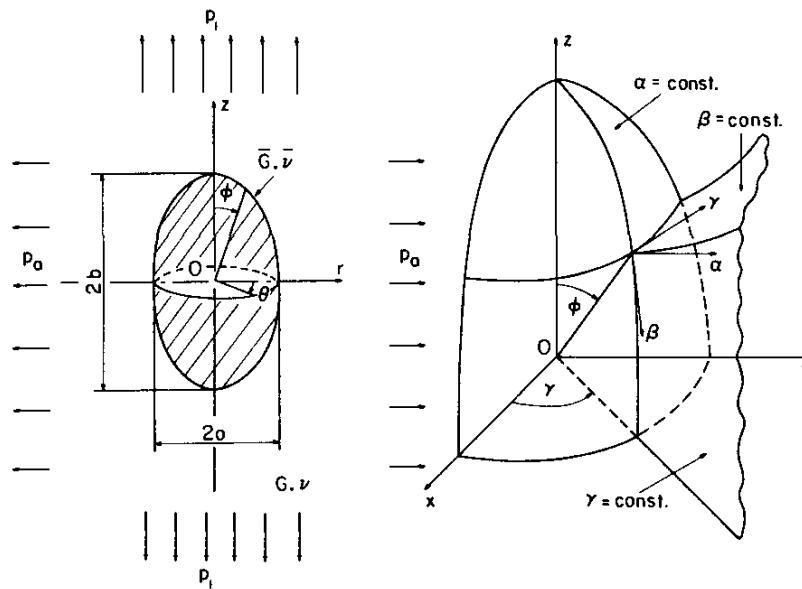


Fig. 51.1. (a) A spheroidal inhomogeneous inclusion subjected to the applied load. (b) The prolate spheroidal coordinate system.

The displacement components in the cartesian coordinate system are expressed by the Boussinesq's potential functions ψ , φ and λ ,

$$\begin{aligned}[u, v, w] &= \frac{1}{2\mu} \operatorname{grad} \psi, \\ [u, v, w] &= \frac{1}{\mu} \operatorname{curl}[0, 0, \varphi], \\ [u, v, w] &= \frac{z}{2\mu} \operatorname{grad} \lambda - \left[0, 0, \frac{3-4\nu}{2\mu} \lambda \right].\end{aligned}\tag{51.20}$$

The above formulae are for the matrix. The same formulae with bar-are used for inertia points in Ω . For this axial symmetry problem we can choose $\varphi = 0$.

The slip boundary conditions at the interface of the inclusion i.e. at $\alpha = \alpha_0$ are

$$\begin{aligned}u_\alpha &= \bar{u}_\alpha + u_\alpha^*, \quad \sigma_\alpha = \bar{\sigma}_\alpha \\ \tau_{\alpha\beta} &= 0, \quad \bar{\tau}_{\alpha\beta} = 0,\end{aligned}\tag{51.21}$$

where u_α^* is the displacement caused by $\epsilon_x^* = \epsilon_y^*$ and ϵ_z^* if the inclusion is free from the surrounding medium.

The boundary conditions at infinity are

$$\sigma_x = \sigma_y = p_0, \quad \alpha \rightarrow \infty\tag{51.22}$$

for the all-around tension case,

$$\sigma_z = p_1, \quad \alpha \rightarrow \infty\tag{51.23}$$

for the uniaxial tension along the z axis, and

$$\begin{aligned}\sigma_\alpha &= \sigma_\beta = \sigma_\gamma = 0 \\ \tau_{\alpha\beta} &= 0, \quad \alpha \rightarrow \infty\end{aligned}\tag{51.24}$$

for the eigenstrain problem, where the inclusion can be also an inhomogeneity. Using $P_n(p)$ and $Q_n(q)$, the Legendre functions of the first and second kinds, we write

$$\begin{aligned}\psi &= C_0 \sum_{n=0}^{\infty} A_n Q_n(q) P_n(p) \\ \lambda &= C_0 \sum_{n=0}^{\infty} B_n Q_n(q) P_n(p)\end{aligned}\tag{51.25}$$

for the matrix ($\alpha > \alpha_0$) and

$$\begin{aligned}\bar{\psi} &= C_0 \sum_{n=0}^{\infty} \bar{A}_n P_n(q) P_n(p) \\ \bar{\lambda} &= C_0 \sum_{n=0}^{\infty} \bar{B}_n P_n(q) P_n(p)\end{aligned}\quad (51.26)$$

for the inclusion ($\alpha < \alpha_0$), where

$$C_0 = \begin{cases} p_0 & \text{for all-around tension} \\ p_1 & \text{for uniaxial tension} \\ 2\mu\epsilon_x^* \text{ or } 2\mu\epsilon_z^* & \text{for eigenstrain case} \end{cases} \quad (51.27)$$

and

$$\begin{aligned}q &= \cosh \alpha, \quad \bar{q} = \sinh \alpha, \\ p &= \cos \beta, \quad \bar{p} = \sin \beta.\end{aligned}\quad (51.28)$$

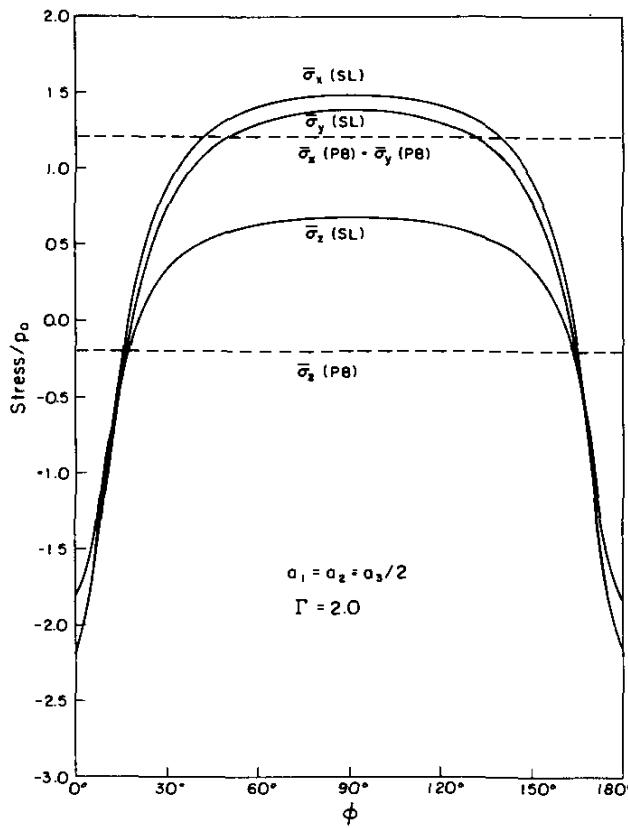


Fig 51.2 Variation of $\bar{\sigma}_x$, $\bar{\sigma}_y$ and $\bar{\sigma}_z$ (caused by all-around tension p_0) in the inhomogeneity on the spheroidal interface with ϕ for $s = 0.5$, $\Gamma = 0.5$, and $\gamma = \frac{1}{2}\pi$. PB stands for the perfect bonding and SL for the sliding inclusion.

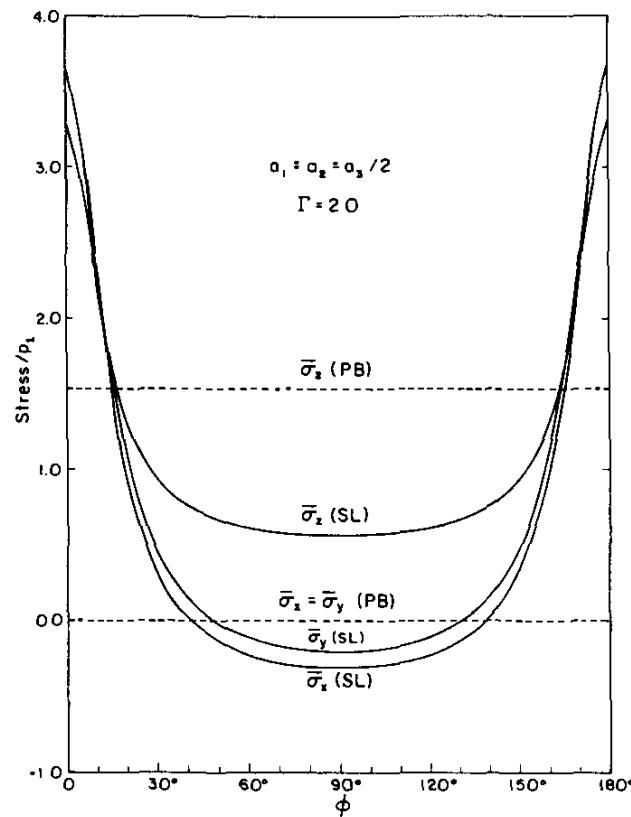


Fig. 51.3. Variation of the stresses in Ω (caused by tension p_1 in the z direction).

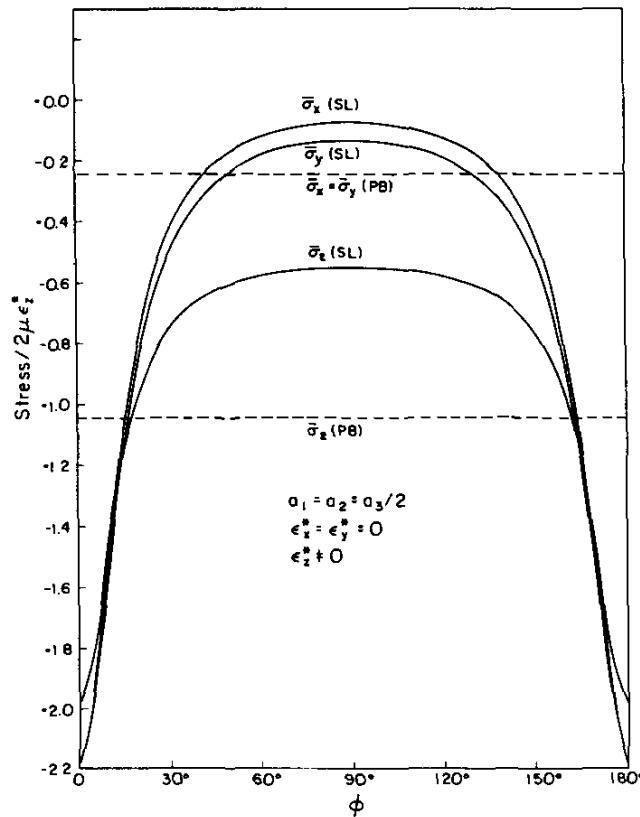


Fig. 51.4. Variation of $\bar{\sigma}_x$, $\bar{\sigma}_y$ and $\bar{\sigma}_z$ (caused by $\epsilon_x^* = \epsilon_y^* = 0$, $\epsilon_z^* \neq 0$) in the inclusion on the spheroidal interface with ϕ for $s = 0.5$, $\Gamma = 1$ and $\gamma = \frac{1}{2}\pi$

Numerical examples have been shown by Mura, Jasiuk and Tsuchida (1985). the solid curves in Fig. 51.2 ~ 4 are the stress components Ω along the interface and the dotted lines are the corresponding stress components for the perfect bonding case, where $\Gamma = \bar{\mu}/\mu$ and $\nu = \bar{\nu} = 0.3$. It is interesting to observe the deviation of the stresses from the uniformity.

52. Recent developments

Recent developments in inclusion problems can be reviewed from three points of view:

- 1) applications to composite materials, including precipitates and martensite problems in metallurgy,
- 2) a non-elastic matrix,
- 3) the fracture of composites and sliding and debonding.

Inclusions, precipitates, and composites

Average elastic moduli of composite materials, precipitates, and polycrystals have been investigated using the same method.

Walpole (1985) reviewed the problem of composite materials from the point of view which emphasizes the early and enduring influence of the papers of Eshelby (1957,1959). Recent papers on the subject are either improvements of approximations or slightly modified versions of approaches, e.g. Horii and Nemat-Nasser (1983,1985), Nunan and Keller (1984), Nomura and Chou (1984), Aleksandrov and Olegin (1985), Silovanyuk and Stadnik (1984), Mol'kov and Pebedrya (1985), Kantor and Bergman (1984), and Choi and Earmme (1986), among others.

For non-dilute concentrations of multiple constituents, the concept of an effective (fictitious) third phase surrounding the constituents, which was introduced by Christensen and Lo (1979), has been developed by Benveniste (1985), Nomura and Oshima (1985), Oshima and Nomura (1985), and Hlaváček (1986), among others.

Mori-Tanaka's method (1973) was written originally for the evaluation of the average stress field in the matrix which contains dilute composites. However, this method is also applicable to non-dilute concentrations of constituents with slight modifications. Hatta and Taya (1986) applied Eshelby's equivalent inclusion method to evaluate the average heat conductivity of composites with high concentrations of inhomogeneities. The average thermal expansion coefficients of an aligned short-fiber composite were also evaluated by Takao and Taya (1985) using the same method. Weng (1984) showed some

correlation between results obtained by Mori-Tanaka's method and Hashin-Shtrikman's method (1963).

Mori-Tanaka's method (1973) has been employed in a broad range of inclusion problems, e.g. by Noyan (1983) for equilibrium conditions for average stresses measured by X-rays, by Stobbs and Paetke (1985) for the Bauschinger effect in cold drawn patented wire, by Sarosiek, Grujicic, and Owen (1984) for the heterogeneity of deformation in the ferrite phase in steel, and by Pedersen (1983) for thermoelasticity and plasticity of composites, among others.

Pore compressibility of materials containing voids, as well as overall elastic moduli, is an important property. An analytical expression for the pore compressibility of an isolated spheroidal cavity was given by Zimmerman (1985). Hashin (1985) considered large isotropic deformations of composite materials with porous media consisting of a finitely deforming elastic matrix and spherical inclusions or voids; he showed that a very small number of pores have a significant effect on expansion, but not on contraction.

Plastic behavior of polycrystals has been investigated by many researchers after G.I. Taylor (1938). The most recent work of Walpole (1985) is on a transversely isotropic aggregate of cubic crystals. For large plastic deformations, Asaro and Needleman (1985) presented a review on the subject.

Metallurgical consideration of precipitations in alloys and martensite transformations is also an important consequence of Eshelby's papers (1957, 1959).

The biasing precipitates of Ni_4V were explained by the misfit of thermal expansions and transformation strains by Wakashima, Ishige, and Umekawa (1982). The morphology of crystalline cubic precipitates in amorphous solids was investigated by Schneck, Rokhlin, and Dariel (1984) in terms of elastic strain energy. The state and habit of Fe_{16}N_2 precipitates in b.c.c. iron were studied by Hong, Wedge, and Morris (1984). The habit plane for martensite formation was investigated by Hayakawa and Oka (1984), who evaluated the minimum strain energy for various shapes and the lattice invariant shear.

The equivalent eigenstrains for inhomogeneities are misfit strains in a homogeneous medium. The misfit strains have a tendency to decay when diffusional relaxation takes place. Srolovitz, Petkovic-Luton, and Luton (1983, 1984) and Srolovitz et al. (1984) published a series of papers along this line.

Coated nuclear fuel particles and duplex cylindrical fibers in composite materials are overlapping inhomogeneities. An overlapping inhomogeneity consists of two confocal spheroids, and it is embedded in an infinite body.

When the coated region is thin compared with the size of inclusion, the stresses in the overlapping inhomogeneity are approximated analytically as done by Walpole (1978). Such an assumption of the thickness has been removed by Mikata and Taya (1985, 1986) for a spheroidal inhomogeneity and

by Theocaris, Sideridis, and Papanicolaou (1985) for a cylindrical inhomogeneity. A four-concentric circular cylindrical model was used by Mikata and Taya (1985) for the stress analysis of coated fiber composites subjected to thermo-mechanical loadings.

Half-spaces

Masumura and Chou (1982) solved the antiplane eigenstrain problem of an elliptic inclusion in an anisotropic half-space, and Zhang and Chou (1985) extended the analysis to the case of joint half-spaces with different anisotropies. They obtained explicit expressions for the stress field and strain energy under a given symmetry of the anisotropy of materials and the orientation of the inclusion.

Selvadurai (1985) developed a set of bounds which can be used to estimate the asymmetric rotational stiffness of a rigid elliptical disc inclusion which is embedded in bonded contact at an isotropic bi-material elastic interface.

The thermoelastic stress generated by surface parallelepiped inclusions has been solved by Lee and Hsu (1985) by the use of Mindlin's Green's function for the half-space.

Loges, Michel, and Christ (1985) have dealt with the chemical interaction of a spheroidal inclusion with the surface of a half-space. They investigated the dependency of energy on the shape, size, and orientation of the detect.

Non-elastic matrices

Hitherto, plastic deformations in matrices have been assumed to be uniform in mathematical analysis when Eshelby's theory is applied to composites.

A numerical solution of local stress and strain fields near a spherical elastic inclusion was obtained by Thomson and Hancock (1984) when a matrix is subjected to large, remote strains associated with the constitutive laws of power hardening. Gilormini and Montheillet (1986) calculated the stress concentrations and deformations of an ellipsoidal inclusion when the matrix and inhomogeneity have different viscous laws and applied stresses are symmetrical with respect to the axes of the ellipsoidal inclusion. When the matrix is linearly viscoelastic, the stress field within the ellipsoidal inclusion is uniform. Budiansky, Hutchinson, and Slutsky (1982), Budiansky (1983), and Duva (1986) have presented some basic studies of void growth and collapse in non-elastic solids under biaxial loading at infinity. The inhomogeneity is a spherical void and the matrix is a nonlinear, incompressible viscous material. (The effective strain rate is proportional to the power of the effective stress.)

For polycrystalline metals undergoing creep at high temperatures, the nucleation, growth, and coalescence of grain boundary cavities become im-

portant. Tvergaard (1985) has shown a number of numerical solutions of axisymmetric model problems to study the combined influence of sliding and cavitation.

Grain boundary sliding versus intergranular plastic deformation requires strain compatibility conditions. Mussot, Rey, and Zaoui (1985) have shown a geometrical analysis for the compatibility conditions.

Material damping of randomly oriented, short-fiber composites was analyzed by Sun, Wu, and Gibson (1985). Nomura and Chou (1985) investigated the viscoelastic behaviour of short-fiber composite materials.

Dragon (1985) described void growth around a spherical inclusion with the use of the rate of a second-order damage tensor, whose definition supposes an appropriate averaging process over a statistically significant, representative volume. Descriptions of damage in materials by parameters in continuum mechanical terms have been proposed by many researchers, e.g. Kachanov (1958), Krajcinovic (1984), Jansson and Stigh (1985), and Onat (1984), among others. The influence of matrix degradation on fiber-mode mechanical properties of unidirectional composites was investigated by Gottesman and Mikulinsky (1984), who used the statistical parameters of the Weibull distribution of the fiber strength.

Taya, Hall, and Yoon (1985) gave experimental and analytical results of void growth in a single crystal subjected to high strain-rate impact.

Delamination buckling of fiber-reinforced and laminated composites subjected to compressive loads along the reinforcing direction has been investigated by Budiansky (1983) and Barber and Triantafyllidis (1985).

Cracks and inclusions

Budiansky, Hutchinson, and Lambropoulos (1983) introduced a continuum description for an elastic solid which contains particles that undergo an irreversible, stress-induced dilatent transformation. Stress and strain fields in the transformation zone at the tip of a macroscopic crack are determined for stationary and growing cracks. Detailed calculations for the toughening are made over a wide range of possible material parameters.

Rose (1986) and Rose and Swain (1986) investigated enhanced fracture toughness due to stress-induced transformation. Two viewpoints are proposed: 1) the increase can be attributed to the need to supply a work of transformation, and 2) the transformation can be considered to result in internal stresses which oppose the crack opening.

Budiansky, Hutchinson, and Evans (1986) investigated a fiber-reinforced ceramic subjected to tensile stress in the fiber direction. Extensive matrix cracking normal to the fibers is considered under two distinct situations

concerning the fiber-matrix interface: 1) unbounded fibers initially held in a matrix by thermal mismatches, but susceptible to frictional slip, and 2) fibers that initially are weakly bonded to the matrix, but may be debonded by the stresses near the tip of an advancing matrix crack.

A tensile crack bridged by fibers has been modeled in such a manner that it can be analyzed based on concepts from the mechanics of inclusion. Mori and Mura (1984) have analytical expressions for the energy release rate, stress intensity factor, and crack opening as functions of the size and spacing of the fibers.

Hashin (1985) employed the principle of minimum complimentary energy to analyze cross-ply laminates which contain distributions of intralaminar cracks within the 90° ply for tensile and shear membrane loading.

The energy release rates of various microcracks in short-fiber composites have been discussed by Taya and Chou (1983).

Characteristics of stress singularity at rigid inclusion tips have been investigated by Wang, Zhang, and Chou (1985) when the matrix is isotropically elastic. Hasebe, Nemat-Nasser, and Keer (1984) considered a kinked crack initiating from a rigid-line inhomogeneity.

When a ribbon-like rigid inclusion is embedded in a power-law hardening material, the order of the singularity at the ends of the inclusion under load is exactly the same as that at the crack tips, according to Hayashi (1982, 1984).

Elastic-plastic finite element analyses were used by Trantina and Barishpolsky (1984) to compute the crack driving force for cracks initiating at voids and inclusions.

Kunin and Gommerstadt (1985) have given a general theory for the interaction between a crack and an inhomogeneous inclusion by the use of the projection integral equation method developed by Kunin (1983).

Theocaris and Demakos (1985) considered an antiplane shear crack in an infinite plate with a circular inhomogeneity. A finite element analysis of the influence of an inhomogeneity or a hole on the stress intensity factor at the tip of an edge crack in a three-point bending specimen was presented by Sides, Perl, and Uzan (1984). The interaction problem between a planar crack and a flat inhomogeneity in an elastic solid was considered by Liu and Erdogan (1986). The stress intensity factors were calculated and tabulated for various crack-inhomogeneity geometries, the inhomogeneity to matrix modulus ratios, and general homogeneous loading conditions.

Expansion of a penny-shaped crack by an inclusion is a problem of overlapping inhomogeneities. Selvadurai and Singh (1984, 1986) evaluated the stress intensity factor at a crack tip when the inclusion is a rigid circular disc. When the inclusion is ellipsoidal, the contact area with the crack surface becomes an unknown quantity. Tsai (1984) solved the case when the inclusion is an oblate, rigid spheroid and the matrix is transversely isotropic.

Sliding and debonding inclusions

The analysis for sliding inclusions has been extended to the nonshear eigenstrain case by Mura, Jasiuk, and Tsuchida (1985). They used Boussinesq's potential functions and evaluated the displacement and stress fields when a spheroidal inhomogeneity is subjected to all-around tension or uniaxial tension. Tsuchida, Mura and Dundurs (1986) considered the two dimensional elliptic inclusion with a slipping interface, and Leo et al. (1985) evaluated elastic fields about a spherical inclusion with a greased interface. Kouris, Tsuchida, and Mura (1986) showed that the result of a sliding circular inclusion cannot be obtained as a limit of an elliptical sliding inclusion.

The problem of the stress field of an elliptic inhomogeneity which has debonded over an arc along the interface has been considered by Karihaloo and Viswanathan (1985). They used Eshelby's equivalent inclusion method.

A more interesting problem was solved in 1971 by Hussain and Pu (1971). They considered an unbonded circular inhomogeneity in an infinite medium subjected to uniaxial tension. The interface between the inclusion and the matrix is partitioned into regions of rigid linkage, slip, and separation. Solutions have been obtained by using variational method techniques developed by Noble and Hussain (1969). Mura and Taya (1985) introduced the concept of Somigliana dislocations to describe plastic deformations on the interface, and they evaluated residual stresses in metal composites due to temperature change.

The effect of sliding on the overall behavior of composite materials has been investigated by Benveniste (1984) and Benveniste and Aboudi (1984).

Second-phase particles or inclusions on a grain boundary suppress grain boundary sliding. Mori et al. (1983) modeled the grain boundary as a flat ellipsoidal inclusion and the particles as disc-shaped inclusions which have opposite eigenstrains from those given in the grain boundary, so that no plastic deformation exist in the particles. Their theory has been proven in experiments by Shigenaka, Monzen, and Mori (1983) and Suzuki, Tanaka, and Mori (1985), and it has been extended to a crack arrest mechanism in fiber-reinforced materials by Mori and Mura (1984). A review on the subject was given by Mori (1985).

Dynamic cases

Disturbances of elastic waves by an embedded cavity have been investigated by many authors. Boström and Karlsson (1985) evaluated point-force excitation of an elastic plate with an embedded cavity.

The equivalent inclusion method in the dynamic case was applied to the scattering fields of two ellipsoidal inhomogeneities by Li, Zhong, and Li (1985).

When the inhomogeneity is rigid and the zeroth and first-order low frequency approximations are used, the leading term of the scattering cross section was obtained explicitly by Dassios and Kiriaki (1986). Olsson (1985) used the null field approach for a star-shaped rigid inclusion.

Niwa, Hirose, and Kitahara (1986) used the boundary integral equation method for various inclusions in a half-space, such as a cavity, an elastic inhomogeneity, and a fluid inclusion.

A new version of the effective field approach was presented by Beltzer and Brauner (1986) with application to random fibrous composites, where wave velocities and attenuations are evaluated numerically. Elastic scattering problems have been investigated by Kinra and Li (1986) for a random distribution of inclusions and by Baik and Thompson (1985) for a planar distribution of inclusions. Coussy (1986) considered the scattering of SH-waves by a rigid elliptic fiber partially debonded from its surrounding matrix.

Miscellaneous

Eldiwany and Wheeler (1986) studied the three-dimensional problem of finding the shape of the minimum stress concentration for a rigid inhomogeneity embedded in an infinite elastic matrix under uniform applied stresses at infinity. The optimum inhomogeneities were found to be ellipsoidal in shape.

Castles and Mura (1985) found that the displacement and stress fields vanish in Ω when eigenstrains are distributed in the form of $1/|\tilde{x}|^p$ outside of Ω , where Ω is an ellipsoidal sub-domain in an infinite medium and $|\tilde{x}|$ is the distance from the center of Ω and $p > 0$.

Dryden, Deakin, and Shinozaki (1986) studied polymer spherulites which have crossed polar and spherically symmetric elastic constants. They presented a singular perturbation analysis of axisymmetric deformation.

When an inhomogeneity has the shape of an arc with a thin thickness, the state of stress by Bernar and Opanasovich (1984) is obtained as a solution of two singular integral-differential equations of the Prandtl type.

The plane elasticity problem of an edge dislocation located within an elliptical inhomogeneity was considered by Warren (1983). Santare and Keer (1986) solved the case when an edge dislocation is located near a rigid elliptical inhomogeneity. Contour plots for the glide component of the Peach-Koehler forces were presented. Their solution has a closed form. Stagni and Lizzio (1984) obtained an infinite series form of the solution when an edge dislocation is located near a general elliptic inhomogeneity.

Appendix 1

Einstein summation convention

If an index occurs twice in any one term, summation is taken from 1 to 3. For instance,

$$\begin{aligned}
 C_{ijk}e_{kl} &= C_{ij1l}e_{1l} + C_{ij2l}e_{2l} + C_{ij3l}e_{3l} \\
 &= (C_{ij11}e_{11} + C_{ij12}e_{12} + C_{ij13}e_{13}) \\
 &\quad + (C_{ij21}e_{21} + C_{ij22}e_{22} + C_{ij23}e_{23}) \\
 &\quad + (C_{ij31}e_{31} + C_{ij32}e_{32} + C_{ij33}e_{33}),
 \end{aligned} \tag{A1.1}$$

$$\sigma_{ij,j} = \sigma_{i1,1} + \sigma_{i2,2} + \sigma_{i3,3}. \tag{A1.2}$$

Kronecker delta

$$\delta_{ij} = \begin{cases} 1 & \text{for } i = j, \\ 0 & \text{for } i \neq j. \end{cases} \tag{A1.3}$$

Namely, $\delta_{11} = \delta_{22} = \delta_{33} = 1$, $\delta_{12} = \delta_{23} = \delta_{31} = \delta_{21} = \delta_{32} = \delta_{13} = 0$. We have

$$\delta_{ii} = 3. \tag{A1.4}$$

Permutation tensor

$$\epsilon_{ijk} = \begin{cases} 1 & \text{for the even permutation of 1, 2, 3,} \\ -1 & \text{for the odd permutation of 1, 2, 3,} \\ 0 & \text{for other cases.} \end{cases} \tag{A1.5}$$

Namely, $\epsilon_{123} = \epsilon_{231} = \epsilon_{312} = 1$, $\epsilon_{132} = \epsilon_{213} = \epsilon_{321} = -1$,
 $\epsilon_{112} = \epsilon_{223} = \epsilon_{333} = \dots = 0$.

We have

$$\epsilon_{ijk}\epsilon_{imn} = \delta_{jm}\delta_{kn} - \delta_{jn}\delta_{km}. \quad (\text{A1.6})$$

Appendix 2

The elastic moduli

The elastic moduli for isotropic materials are

$$C_{ijkl} = \lambda\delta_{ij}\delta_{kl} + \mu\delta_{ik}\delta_{jl} + \mu\delta_{il}\delta_{jk}, \quad (\text{A2.1})$$

where λ and μ are Lamé's constants. The following relations hold among Young's modulus E , Poisson's ratio ν , the bulk modulus K , and the shear modulus μ :

$$\begin{aligned} \lambda &= 2\mu\nu/(1 - 2\nu), & E &= 2(1 + \nu)\mu, & \nu &= \lambda/2(\lambda + \mu), \\ K &= \frac{1}{3}(2\mu + 3\lambda) = E/3(1 - 2\nu), & \nu &= (3K - 2\mu)/2(3K + \mu). \end{aligned} \quad (\text{A2.2})$$

Hooke's law becomes

$$\begin{aligned} \sigma_{11} &= 2\mu e_{11} + \lambda e_{kk}, \\ \sigma_{22} &= 2\mu e_{22} + \lambda e_{kk}, \\ \sigma_{33} &= 2\mu e_{33} + \lambda e_{kk}, \\ \sigma_{12} &= 2\mu e_{12}, \\ \sigma_{23} &= 2\mu e_{23}, \\ \sigma_{31} &= 2\mu e_{31}, \end{aligned} \quad (\text{A2.3})$$

or

$$\sigma_{ij} = 2\mu e_{ij} + \delta_{ij}\lambda e_{kk}. \quad (\text{A2.4})$$

The inverse of (A2.3) are

$$\begin{aligned} e_{11} &= \{\sigma_{11} - \nu(\sigma_{22} + \sigma_{33})\}/E, \\ e_{22} &= \{\sigma_{22} - \nu(\sigma_{33} + \sigma_{11})\}/E, \\ e_{33} &= \{\sigma_{33} - \nu(\sigma_{11} + \sigma_{22})\}/E, \end{aligned}$$

$$e_{12} = \sigma_{12}/2\mu, \quad (\text{A2.5})$$

$$e_{23} = \sigma_{23}/2\mu,$$

$$e_{31} = \sigma_{31}/2\mu.$$

When the reduced strain and stress are defined as

$$\begin{aligned} e'_{ij} &= e_{ij} - \delta_{ij} e_{kk}/3, \\ \sigma'_{ij} &= \sigma_{ij} - \delta_{ij} \sigma_{kk}/3. \end{aligned} \quad (\text{A2.6})$$

Hooke's law can be written as

$$\begin{aligned} \sigma'_{ij} &= 2\mu' e'_{ij}, \\ \sigma_{kk} &= 3K e_{kk}. \end{aligned} \quad (\text{A2.7})$$

The elastic moduli for cubic crystals are

$$C_{ijkl} = \lambda \delta_{ij} \delta_{kl} + \mu \delta_{ik} \delta_{jl} + \mu \delta_{il} \delta_{jk} + \mu' \delta_{ijkl}, \quad (\text{A2.8})$$

where all $\delta_{ijkl} = 0$, except $\delta_{1111} = \delta_{2222} = \delta_{3333} = 1$, and

$$\lambda = C_{12}, \quad \mu = C_{44}, \quad \mu' = C_{11} - C_{12} - 2C_{44}. \quad (\text{A2.9})$$

These C_{ij} are the Voigt constants and are related to C_{ijkl} as shown in Table A2.1 for general cases. For instance, $C_{1122} = C_{12}$, $C_{1123} = C_{14}$. Since $C_{ij} = C_{ji}$, the number of independent constants is generally 21.

Table A2.1 Relation between constant C_{ijkl} and the Voigt elastic constants C_{ij}

ij	kl	11	22	33	23	31	12
11	C_{11}	C_{12}	C_{13}	C_{14}	C_{15}	C_{16}	
22	C_{21}	C_{22}	C_{23}	C_{24}	C_{25}	C_{26}	
33	C_{31}	C_{32}	C_{33}	C_{34}	C_{35}	C_{36}	
23	C_{41}	C_{42}	C_{43}	C_{44}	C_{45}	C_{46}	
31	C_{51}	C_{52}	C_{53}	C_{54}	C_{55}	C_{56}	
12	C_{61}	C_{62}	C_{63}	C_{64}	C_{65}	C_{66}	

Let us make the notational changes,

$$e_{11} = e_1, \quad e_{22} = e_2, \quad e_{33} = e_3, \quad e_{23} = \frac{1}{2}e_4, \quad e_{31} = \frac{1}{2}e_5, \quad e_{12} = \frac{1}{2}e_6, \quad (\text{A2.10})$$

and

$$\sigma_{11} = \sigma_1, \quad \sigma_{22} = \sigma_2, \quad \sigma_{33} = \sigma_3, \quad \sigma_{23} = \sigma_4, \quad \sigma_{31} = \sigma_5, \quad \sigma_{12} = \sigma_6. \quad (\text{A2.11})$$

Hooke's law is written as

$$\begin{aligned} \sigma_1 &= C_{11}e_1 + C_{12}e_2 + C_{13}e_3 + C_{14}e_4 + C_{15}e_5 + C_{16}e_6, \\ \sigma_2 &= C_{21}e_1 + C_{22}e_2 + C_{23}e_3 + C_{24}e_4 + C_{25}e_5 + C_{26}e_6, \\ \sigma_3 &= C_{31}e_1 + C_{32}e_2 + C_{33}e_3 + C_{34}e_4 + C_{35}e_5 + C_{36}e_6, \\ \sigma_4 &= C_{41}e_1 + C_{42}e_2 + C_{43}e_3 + C_{44}e_4 + C_{45}e_5 + C_{46}e_6, \\ \sigma_5 &= C_{51}e_1 + C_{52}e_2 + C_{53}e_3 + C_{54}e_4 + C_{55}e_5 + C_{56}e_6, \\ \sigma_6 &= C_{61}e_1 + C_{62}e_2 + C_{63}e_3 + C_{64}e_4 + C_{65}e_5 + C_{66}e_6. \end{aligned} \quad (\text{A2.12})$$

For isotropic materials all $C_{ij} = 0$, except for

$$\begin{aligned} C_{11} &= C_{22} = C_{33}, \\ C_{12} &= C_{13} = C_{23}, \\ C_{44} &= C_{55} = C_{66} = \frac{1}{2}(C_{11} - C_{12}). \end{aligned} \quad (\text{A2.13})$$

For cubic crystals (Al, Cu, Au, Fe, Pb, Ni, Ag, W, UO₂, etc.) all $C_{ij} = 0$, except for

$$\begin{aligned} C_{11} &= C_{22} = C_{33}, \\ C_{12} &= C_{13} = C_{23}, \\ C_{44} &= C_{55} = C_{66}, \end{aligned} \quad (\text{A2.14})$$

when the coordinate system is chosen along the crystalline directions. For hexagonal crystals (Mg, Zn, Co, Cd, Al₂O₃, etc.) all $C_{ij} = 0$, except for

$$\begin{aligned} C_{11} &= C_{22}, \quad C_{33}, \\ C_{12}, \quad C_{13} &= C_{23}, \\ C_{44} &= C_{55}, \quad C_{66} = \frac{1}{2}(C_{11} - C_{12}), \end{aligned} \quad (\text{A2.15})$$

when the x_3 -direction is chosen along the hexad axis. This case is transversely isotropic.

For tetragonal crystals (Sn, Zr, MgCl, etc.) all $C_{ij} = 0$, except for

$$\begin{aligned} C_{11} &= C_{22}, \quad C_{33}, \\ C_{12}, \quad C_{13} &= C_{23}, \\ C_{44} &= C_{55}, \quad C_{66}, \end{aligned} \tag{A2.16}$$

when the x_3 -direction is chosen along the hexad axis. For orthorhombic crystals (S, U, etc.), all $C_{ij} = 0$, except for

$$\begin{aligned} C_{11}, \quad C_{22}, \quad C_{33}, \\ C_{12}, \quad C_{13}, \quad C_{23}, \\ C_{44}, \quad C_{55}, \quad C_{66}, \end{aligned} \tag{A2.17}$$

when the coordinate system is chosen along the crystalline directions.

The numerical values of C_{ij} for various materials are found in Simmons and Wang (1971).

The elastic strain can be expressed in the stress by

$$e_i = s_{ij} \sigma_j \tag{A2.18}$$

or

$$e_{ij} = s_{ijkl} \sigma_{kl}, \tag{A2.18.1}$$

where s_{ij} is called the elastic compliance. C_{ij} is sometimes called the elastic stiffness or the elastic constants.

There is a relation

$$s_{66} = 2(s_{11} - s_{12}) \tag{A2.19}$$

when

$$C_{66} = \frac{1}{2}(C_{11} - C_{12}). \tag{A2.20}$$

Appendix 3

Fourier series and integrals

Every function which is piecewise smooth in the interval $-L \leq x \leq L$ and periodic with the period $2L$ may be expanded in a Fourier series, that is,

$$f(x) = \frac{1}{2}a_0 + \sum_{n=1}^{\infty} \left(a_n \cos \frac{\pi n x}{L} + b_n \sin \frac{\pi n x}{L} \right), \quad (\text{A3.1})$$

where

$$\begin{aligned} a_0 &= \frac{1}{L} \int_{-L}^L f(x) dx, \\ a_n &= \frac{1}{L} \int_{-L}^L f(x) \cos \frac{\pi n x}{L} dx, \\ b_n &= \frac{1}{L} \int_{-L}^L f(x) \sin \frac{\pi n x}{L} dx. \end{aligned} \quad (\text{A3.2})$$

The convergence of the Fourier series is uniform in every closed interval in which the function is continuous.

Equations (A3.1) and (A3.2) may also be written in the form

$$f(x) = \sum_{n=-\infty}^{\infty} \alpha_n \exp(in\pi x/L), \quad (\text{A3.3})$$

$$\alpha_n = \frac{1}{2L} \int_{-L}^L f(x) \exp(-in\pi x/L) dx. \quad (\text{A3.4})$$

To prove this, we derive (A3.1) from (A3.3). Substituting (A3.4) into (A3.3), we have

$$\begin{aligned} f(x) &= \sum_{n=-\infty}^{\infty} \frac{1}{2L} \int_{-L}^L f(t) \exp(-in\pi t/L) dt \exp(in\pi x/L) \\ &= \sum_{n=-\infty}^{\infty} \left[\frac{1}{2L} \int_{-L}^L f(t) \cos(n\pi t/L) dt \cos(n\pi x/L) \right. \\ &\quad \left. + \frac{1}{2L} \int_{-L}^L f(t) \sin(n\pi t/L) dt \sin(n\pi x/L) \right] \end{aligned} \quad (\text{A3.5})$$

since the terms $\cos(n\pi t/L) \sin(n\pi x/L)$ cancel the terms $\cos(-n\pi t/L) \sin(-n\pi x/L)$. Furthermore, the identities $\cos(n\pi t/L) \cos(n\pi x/L) = \cos(-n\pi t/L) \cos(-n\pi x/L)$, etc. give the equivalency between (A3.5) and (A3.1).

It seems desirable to let L go to ∞ , since then it is no longer necessary to require that f be continued periodically. We assume that $f(x)$ is piecewise smooth in every finite interval and that at the discontinuities, the value of the function is the arithmetic mean of the right-hand and left-hand limits. The further assumption that the integral $\int_{-\infty}^{\infty} |f(x)| dx$ exists is added. Setting $\pi/L = \delta$, (A3.3) and (A3.4) become

$$f(x) = \sum_{n=-\infty}^{\infty} \alpha_n \exp(in\delta x), \quad (\text{A3.6})$$

$$\alpha_n = \frac{\delta}{2\pi} \int_{-L}^L f(x) \exp(-in\delta x) dx. \quad (\text{A3.7})$$

Furthermore, we set $n\delta = \xi$, $\delta = d\xi$ and let $L \rightarrow \infty$ and $\delta \rightarrow 0$. Then, (A3.6) can be written as

$$f(x) = \int_{-\infty}^{\infty} \bar{f}(\xi) \exp(i\xi x) d\xi, \quad (\text{A3.8})$$

where

$$\bar{f}(\xi) = \frac{1}{2\pi} \int_{-\infty}^{\infty} f(x) \exp(-i\xi x) dx. \quad (\text{A3.9})$$

The expression (A3.8) with (A3.9) is called the Fourier integral form of $f(x)$. $\tilde{f}(\xi)$ is called the Fourier transform of $f(x)$.

Dirac's delta function and Heaviside's unit function

Dirac's delta function $\delta(x)$ has the property

$$\int_{-\infty}^{\infty} \delta(x) F(x) dx = F(0) \quad (\text{A3.10})$$

for any suitably continuous function $F(x)$. $\delta(x)$ may be defined by

$$\lim_{n \rightarrow \infty} \frac{\sin nx}{\pi x} = \delta(x). \quad (\text{A3.11})$$

The Fourier transform of $\delta(x)$ is obtained from (A3.9) as

$$\tilde{\delta}(\xi) = (1/2\pi). \quad (\text{A3.12})$$

The Fourier integral form of $\delta(x)$ is, therefore,

$$\delta(x) = (1/2\pi) \int_{-\infty}^{\infty} \exp(i\xi x) d\xi. \quad (\text{A3.13})$$

Although the integral in (A3.13) does not converge, the formality (A3.13) holds. $\delta(x)$ is the generalized function (see Lighthill 1964).

Heaviside's unit (or step) function $H(x)$ is defined by

$$H(x) = \begin{cases} 1 & \text{for } x > 0, \\ 0 & \text{for } x < 0. \end{cases} \quad (\text{A3.14})$$

The Fourier transform of $H(x)$ becomes, from (A3.9),

$$\bar{H}(\xi) = 1/2\pi i\xi \quad (\text{A3.15})$$

if one can justify throwing out the term $\exp(-i\xi x)$ at $\xi = \infty$. According to Lighthill (p. 33, 1964), its correct expression is

$$\bar{H}(\xi) = \exp(-\frac{1}{2}\pi i \operatorname{sgn} \xi)/2\pi |\xi|, \quad (\text{A3.16})$$

where $\operatorname{sgn} \xi$ is defined as

$$\operatorname{sgn} \xi = \begin{cases} 1 & \text{for } \xi > 0, \\ -1 & \text{for } \xi < 0. \end{cases} \quad (\text{A3.17})$$

Laplace transform

Combining (A3.8) and (A3.9), we have

$$f(t) = \frac{1}{2\pi} \int_{-\infty}^{\infty} e^{i\xi t} d\xi \int_{-\infty}^{\infty} f(\tau) e^{-i\xi\tau} d\tau, \quad (\text{A3.18})$$

where t is used for x .

If

$$\begin{aligned} f(t) &= 0 && \text{for } t < 0, \\ &= e^{-\gamma t} F(t) && \text{for } t > 0, \end{aligned} \quad (\text{A3.19})$$

where $\gamma > 0$, (A3.18) becomes

$$\begin{aligned} F(t) &= \frac{e^{\gamma t}}{2\pi} \int_{-\infty}^{\infty} e^{i\xi t} d\xi \int_0^{\infty} F(\tau) e^{-(\gamma+i\xi)\tau} d\tau \\ &= \frac{1}{2\pi i} \int_{\gamma-i\infty}^{\gamma+i\infty} e^{zt} dz \int_0^{\infty} F(\tau) e^{-z\tau} d\tau, \end{aligned} \quad (\text{A3.20})$$

where $z = \gamma + i\xi$ is used.

The Laplace transform of a function $F(t)$ is defined as

$$\bar{F}(s) = \int_0^{\infty} F(t) e^{-st} dt, \quad s > 0. \quad (\text{A3.21})$$

When s is formally extended to a complex variable z , (A3.20) can be written as

$$F(t) = \frac{1}{2\pi i} \int_{\gamma-i\infty}^{\gamma+i\infty} e^{zt} \bar{F}(z) dz \quad (\text{A3.22})$$

which is called the inversion integral.

If $\bar{F}(s)$ is obtained as an analytic function in the semi-infinite plane $R(z) < \gamma$ except at poles $z_n (n = 1, 2, \dots)$ and

$$\begin{aligned} |\bar{F}(x \pm i\beta_N)| &< \delta_N & (-\beta_N \leq x \leq \gamma, |y| \leq \beta_N) \\ |\bar{F}(-\beta_N + iy)| &< M \end{aligned} \quad (\text{A3.23})$$

where $\delta_N \rightarrow 0$, $\beta_N \rightarrow \infty$ for $N \rightarrow \infty$, (A3.22) becomes

$$F(t) = \sum_{n=1} \rho_n(t), \quad (\text{A3.24})$$

where ρ_n is the residue of $e^{zt}\bar{F}(z)$ at z_n . All poles of $e^{zt}\bar{F}(z)$ are located in the plane $R(z) < \gamma$.

Appendix 4

Dislocation pile-ups

The Hilbert integral equation for an unknown $f(x)$,

$$\int_D \frac{f(t)}{x-t} dt + \sigma(x) = 0, \quad (\text{A4.1})$$

has been investigated by Muskhelishvili (1953) and illustrated for the dislocation problems by Head and Louat (1955). In the above equation $\sigma(x)$ is a given piecewise function. It is understood that the Cauchy principal value of the integral (see Whittaker and Watson (1962, p. 75) is to be taken to avoid divergence at $t = x$.

Let us assume that D consists of a finite number of intervals of the x -axis $(a_1, b_1), (a_2, b_2), \dots, (a_p, b_p)$. Suppose that at q of the end-points of the segments, denoted by c_1, c_2, \dots, c_q , $f(x)$ is to remain bounded, and that at the remaining $2p-q$ end-points, denoted by $c_{q+1}, c_{q+2}, \dots, c_{2p}$, $f(x)$ may be unbounded. Let

$$R_1(x) = \prod_{k=1}^q (x - c_k),$$

$$R_2(x) = \prod_{k=q+1}^{2p} (x - c_k), \quad (\text{A4.2})$$

where $R_1 = 1$ when $q = 0$, and $R_2 = 1$ when $2p < q + 1$. Then, if $p - q \geq 0$, solutions of (A4.1), bounded at c_1, \dots, c_q , always exist and are given by

$$f(x) = \frac{1}{\pi^2} \left\{ \frac{R_1(x)}{R_2(x)} \right\}^{1/2} \int_D \left\{ \frac{R_2(t)}{R_1(t)} \right\}^{1/2} \frac{\sigma(t)}{x-t} dt + \left\{ \frac{R_1(x)}{R_2(x)} \right\}^{1/2} Q_{p-q-1}(x), \quad (\text{A4.3})$$

where $Q_{p-q-1}(x)$ is an arbitrary polynomial of degree not greater than $p - q - 1$. It is identically zero for $p = q$ and $p = q + 1$.

If $p - q < 0$, a unique solution, bounded at c_1, \dots, c_q exists if and only if $\sigma(x)$ satisfies the conditions

$$\int_D \left\{ \frac{R_2(t)}{R_1(t)} \right\}^{1/2} t^n \sigma(t) dt = 0, \quad n = 0, 1, \dots, (q - p - 1), \quad (\text{A4.4})$$

and if this is so, the solution is given by (A4.3) with $Q_{p-q-1} = 0$. Moreover at a bounded end-point, $f(x)$ vanishes. The Cauchy principal values of the integrals are taken in all these formulae.

Let us consider an example, $\sigma = \sigma^0 = \text{constant}$, $D = (-c, c)$, and $f(x)$ is unbounded at $-c$ and c . Then, $a_1 = -c$, $b_1 = c$, $p = 1$, $q = 0$, $c_{q+1} = -c$, and $c_{q+2} = c$. We have $R_1 = 1$, and $R_2(x) = (x + c)(x - c)$. Since $p - q > 0$, (A4.3) becomes

$$\begin{aligned} f(x) &= \frac{\sigma^0}{\pi^2} \frac{1}{(c^2 - x^2)^{1/2}} \int_{-c}^c \frac{(c^2 - t^2)^{1/2}}{x - t} dt + \frac{Q_0}{(x^2 - c^2)^{1/2}} \\ &= \frac{\sigma^0}{\pi} \frac{x}{(c^2 - x^2)^{1/2}} + \frac{Q_0}{(x^2 - c^2)^{1/2}}. \end{aligned} \quad (\text{A4.5})$$

Q_0 is an arbitrary constant.

Another example will be considered: $D = (-a, a)$, $\sigma = \sigma^0$ in $|x| < c$, $\sigma = \sigma^0 - k$ in $c < |x| < a$, and $f(x)$ is bounded at $-a$ and a , where σ^0 and k are given constants. Then, $a_1 = -a$, $b_1 = a$, $p = 1$, $q = 2$, $c_1 = -a$, and $c_2 = a$, therefore $R_1(x) = (x + a)(x - a)$, $R_2 = 1$. Since $p - q < 0$,

$$\begin{aligned} f(x) &= (1/\pi^2)(a^2 - x^2)^{1/2} \int_{-a}^a \frac{\sigma dt}{(a^2 - t^2)^{1/2}(x - t)} \\ &= (1/\pi^2)(a^2 - x^2)^{1/2} \left\{ \int_{-a}^{-c} \frac{(\sigma^0 - k) dt}{(a^2 - t^2)^{1/2}(x - t)} \right. \\ &\quad + \int_{-c}^c \frac{\sigma^0 dt}{(a^2 - t^2)^{1/2}(x - t)} \\ &\quad \left. + \int_c^a \frac{(\sigma^0 - k) dt}{(a^2 - t^2)^{1/2}(x - t)} \right\} \end{aligned} \quad (\text{A4.6})$$

The condition (A4.4) becomes

$$\int_{-a}^a \frac{\sigma}{(a^2 - t^2)^{1/2}} dt = 0. \quad (\text{A4.7})$$

Further calculations for these integrals lead to

$$\begin{aligned} f(x) &= \frac{k}{\pi^2} \log \frac{x(a^2 - c^2)^{1/2} + c(a^2 - x^2)^{1/2}}{x(a^2 - c^2)^{1/2} - c(a^2 - x^2)^{1/2}} \quad \text{for } c < |x| < a \\ &= \frac{k}{\pi^2} \log \frac{x(a^2 - c^2)^{1/2} + c(a^2 - x^2)^{1/2}}{c(a^2 - x^2)^{1/2} - x(a^2 - c^2)^{1/2}} \quad \text{for } |x| < c \end{aligned} \quad (\text{A4.8})$$

and

$$c/a = \cos(\pi\sigma^0/2k). \quad (\text{A4.9})$$

The last result (A4.9) has been obtained from (A4.7).

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Author index

A

Aaronson, H.I., 110, 219, 222
Abe, H., 307
Aboudi, J., 497
Achenbach, J.D., 319, 320, 33, 438, 472
Acton, J.R., 433
Adachi, H., 484
Adams, D.F., 433
Aderogba, K., 127
Airapetyan, V.M., 168
Aleksandrov, A.Y., 492
Alexander, D., 497
Amari, S., 348
Amelinckx, S., 44
American Institute of Physics Handbook,
252, 279
Anderson, D.L., 438, 455
Andreykiv, A.E., 298, 300, 307
Ang, D.D., 360
Anthony, K.H., 53
Ardell, A.J., 168
Argon, A.S., 110, 420, 447
Armstrong, R.W., 52, 356
Asaro, R.J., 48, 49, 92, 160, 169, 234, 235,
236, 270, 271, 273, 274, 290, 333, 334,
335, 337, 356, 412, 449, 485, 493
Ashby, M.F., 398, 420, 447
Ati, A., 48
Atkinson, C., 291, 322
Atkinson, J.D., 323, 402, 408

B

Backofen, W.A., 386
Bacon, D.J., 13, 49, 324, 335, 398

Baik, J., 498
Baker, B., 322
Baker, G S., 443
Banerjee, P.K., 298
Barber, J., 495
Barenblatt, G.I., 281
Barishpolsky, H., 496
Barnett, D.M., 13, 32, 49, 92, 160, 219,
222, 235, 270, 271, 273, 274, 290, 324,
333, 334, 335, 337, 354, 410, 412, 465,
469, 471, 493
Bassani, J.L., 447
Bechtoldt, C.J., 50
Beevers, R B., 49
Beltzer, A I., 378, 498
Benveniste, Y., 492, 497
Bergman, D.J., 492
Bernar, I I., 498
Berry, B.S., 168
Bhargava, R.D., 141
Bilby, B A., 7, 34, 65, 73, 229, 271, 288,
290, 314, 324, 335, 342, 343, 358, 364,
374, 384, 453
Bishop, J.F.W., 421, 442, 447
Boas, W., 421
Bodner, S.R., 364
Bonnet, R., 48
Born, M., 477
Bornett, A., 168
Bose, S K., 178, 459
Boström, A., 497
Bouligand, Y., 50
Boussinesq, J., 177
Bozkurt, R.O., 123
Brauner, N., 498
Brebbia, G.A., 298

Brierley, P., 453
Broberg, K.B., 322
Bross, H., 26, 348
Brown, L.M., 49, 169, 335, 336, 337, 338,
 379, 392, 396, 401, 402, 408
Bruggeman, D A.G., 421
Budiansky, B., 313, 314, 431, 433, 438,
 439, 445, 446, 453, 455, 494, 495
Bui, H D , 298, 307, 446
Bullough, R., 34, 73, 198, 324, 335, 342,
 358, 465
Burgers, J.M., 13,17
Burgers, W.G., 234
Butterfield, R., 298
Byskov, E., 275

C

Callias, C., 358
Campbell, G.A., 17
Castles, R.R., 498
Césáro, E., 7, 65, 69
Chamis, C.C., 437, 438
Chan, S K., 275
Chang, S J , 291
Charsley, P., 387
Chen, C.H., 433
Chen, I.W., 447
Chen, S.H., 472, 475, 476
Chen, W.T., 141, 178, 270, 279
Cheng, D.H., 117
Cheng, P C., 151, 155, 168, 220, 223, 224,
 251, 252, 277
Cheng, S., 433
Cherepanov, G P , 34
Chiu, Y P , 104, 105, 107, 121
Choi, B I., 492
Chou, T W , 29, 31, 50, 53, 338, 448, 492,
 495, 496
Chou, Y T, 114, 128, 141, 142, 466, 467,
 469, 494, 496
Chrav, T.P., 237
Christ, A., 494
Christensen, R.M., 448, 492
Christian, J.W., 228, 229, 233
Chuang, T.J., 420
Cladis, P E., 49
Clarke, D.R., 401

Clements, D L., 472
Clifton, R.J., 359, 360
Coffin, L F , 386
Cohen, J B , 21
Cole, D , 438, 455
Collins, W.D., 293
Colonnetti, G , 208, 212
Comninou, M , 338, 465, 472, 473, 474,
 475, 476
Cook, R H , 169
Coussy, O., 498
Cottrell, A H., 288, 290, 324, 467
Cowin, S.C., 439
Craggs, J W., 322
Cruse, T.A., 298, 307

D

Dariel, M.P., 493
Das, E.S P., 53, 356
Das, S C., 178
Dassios, G , 498
Datta, S.K., 458, 459
Deakin, A S , 498
Demakos, C.B., 496
Desvaux, M P.E., 387
DeWit, R , 13, 52, 53, 324, 356
Dietze, H D , 368
Dixson, J.P., 375
Domany, E , 458, 459
Doner, D R., 433
Dragon, A., 495
Drucker, D C., 439
Dryden, J R , 498
Dugdale, D S , 281, 285
Dundurs, J., 43, 44, 53, 126, 127, 338, 370,
 463, 464, 465, 471, 472, 497
Duquette, D J., 379
Duva, J.M. 494
Dyson, F W., 85, 91, 95, 174

E

Earmme, Y Y , 198, 484, 492
Easterling, K E , 231, 234
Eberhart, M E., 484
Ebner, M L., 386
Edelen, D.G.B., 353

- Edil, T., 439
 Edwards, R.H., 178, 488
 Einspruch, N.G., 458
 Eisenberg, M.A., 364, 367
 Eldiwaný, B.H., 498
 Elhott, H.A., 29
 Engel, J.J., 379
 England, A.H., 293, 471, 475
 Erdogan, F., 275, 319, 471, 474, 496
 Eringen, A.C., 439
 Eshelby, J.D., 1, 7, 13, 26, 34, 36, 56, 74,
 92, 100, 121, 141, 157, 168, 177, 206,
 261, 271, 288, 311, 314, 322, 335, 343,
 357, 358, 359, 360, 394, 449, 453, 466,
 485, 492, 493
 Essmann, U., 49, 53
 Eubanks, R.A., 177
 Eurin, P.H., 168
 Evans, A.G., 495
 Ezaki, H., 484
- F**
- Faïvre, G., 104
 Feldman, E.P., 361
 Ferrers, N.M., 85, 95
 Feshbach, H., 458
 Field, F.A., 291
 Fine, M.E., 317
 Flinn, J.E., 378
 Foreman, A.J.E., 26
 Foster, R.M., 17
 Frank, F.C., 49, 57, 357, 466
 Fredholm, I., 13, 26
 Frenkel, Y.I., 358
 Freund, L.B., 319, 320, 322
 Friedel, J., 49, 324
 Fu, W.S., 242, 460
 Fung, Y.C., 451
 Furuhashi, R., 169, 170, 198, 486, 487
 Futagami, T., 298
- G**
- Gale, B., 314
 Galerkin, B., 177
 Gangadharan, A.C., 464
 Gardner, L.R.T., 364, 374
- Gavazza, S.D., 335, 356, 465
 Gdoutos, E.E., 286
 Gerstner, R.W., 44
 Geyer, J.F., 276
 Ghahremani, F., 447, 488
 Gibson, R.F., 495
 Gilman, J.J., 50, 364, 387
 Gilormini, P., 494
 Glicksman, M.E., 465
 Golebiewska-Lasota, A.A., 353, 356, 361
 Gommerstadt, B., 496
 Goncharyuk, I.V., 302
 Goodier, J.N., 40, 74, 88, 170, 177, 291,
 369
 Gottesman, T., 495
 Gough, H.J., 177
 Gould, D., 402
 Gradshteyn, I.S., 78
 Granato, A., 361
 Green, A.E., 141, 295
 Griffith, A.A., 241, 244
 Grosskreutz, J.C., 379
 Grujicic, M., 493
 Gubernatis, J.E., 458, 459
 Guell, D.L., 127
 Günther, H., 53, 348, 353, 359, 361
 Gupta, G., 474
- H**
- Hahn, G.T., 291, 323
 Hahn, H.T., 364
 Hall, I.W., 495
 Ham, R.K., 169
 Hancock, J.W., 494
 Hardiman, J., 41
 Harris, W.F., 49, 50
 Hart, E.W., 361, 398
 Hasebe, N., 465, 496
 Hashin, Z., 433, 434, 437, 448, 453, 493,
 496
 Hatta, H., 492
 Havner, K.S., 443
 Hayakawa, M., 493
 Hayashi, K., 291, 307, 472, 496
 Hayns, M.R., 465
 Hazzledine, P.M., 416
 Head, A.K., 289, 466, 467, 510

- Heald, P.T., 314
Heinisch, H.L., 465
Herrmann, G., 438
Hershey, A.V., 433, 439
Hetényi, M., 126
Hlaváček, H., 492
Hill, R., 39, 42, 366, 421, 425, 426, 433, 436, 439, 442, 443, 445, 447, 449
Hilliard, J.E., 223
Hirose, S., 498
Hirose, Y., 387
Hirsch, P.B., 44, 402, 411
Hirth, J P., 13, 44, 49, 324, 333, 334, 335, 356
Hobson, E.W., 31, 162
Hodge, P G.P., 373
Hoenig, A., 265
Hollander, E.F., 348, 353, 361
Hong, H., 493
Honjo, G., 379
Horgan, C.O., 438
Horii, H., 276, 439, 492
Horne, M.R., 439
Hoshina, M., 386
Howard, I C., 453
Howie, A., 44
Hsu, C C., 121, 494
Huang, K., 477
Huang, W., 53
Huber, A., 421
Huberman, M., 458
Hull, D., 324
Humphreys, F.J., 402
Hussain, M A., 497
Hutchinson, J W., 314, 323, 439, 443, 445, 446, 447, 449, 453, 494, 495
- I
Ibaraki, M., 233
Ichikawa, M., 50, 310
Ikeda, H., 460
Im, J., 420
Indenbom, V L., 13, 26, 48, 335
Inglebert, G., 379
Inglis, C E., 279, 293
Ioakimidis, N I., 41
Irwin, G.R., 260, 274, 286, 309, 319
- Ishige, K., 493
Ishioka, S., 360
Isida, M., 275
Ito, Y M., 379, 446
Iwakuma, T., 443, 449
Iwan, J., 497
Izumi, Y., 317
- J
Jack, K.H., 237
Jan, R.V., 26
Jansson, S., 495
Jasiuk, I., 492, 497
Jaswon, M A., 141
Jaunzemis, W., 364
Jimma, T., 378
Johnson, K H., 484
Johnson, W.C., 104, 108, 109, 198, 226, 227
- K
Kachanov, L M., 495
Kagawa, K I., 420
Kageyama, K., 291, 313
Kamei, A., 275, 310
Kamitani, A., 379
Kanninen, M.F., 291, 319, 320, 323
Kantor, Y., 492
Karhaloo, B L., 290, 291, 364, 497
Karlsson, A., 497
Kassir, M K., 244, 260, 265, 268, 270
Kato, M., 146, 168, 220, 223, 230, 235, 387
Kawai, K., 378
Kay, T R., 291
Keer, L M., 242, 276, 293, 297, 298, 465, 471, 472, 475, 476, 496, 498
Keith, R E., 387
Keller, J B., 492
Keller, J M., 338
Kelly, J M., 364
Lord Kelvin, 23
Kfouri, A P., 310, 311
Khachaturyan, A G., 9, 168, 233
Khetan, R P., 472
Kikuchi, R., 104
Kim, T J., 41

- Kinoshita, N , 26, 32, 135, 160, 169, 170, 298, 412
 Kinra, V K , 498
 Kinsman, K R., 110, 235, 236
 Kirchner, H.O K , 356
 Kiriakī, K., 498
 Kitagawa, K , 411
 Kitahara, M., 298, 460, 498
 Kiusalas, J , 359, 360
 Kléman, M , 49, 50
 Klesnil, M., 386
 Knauss, W.G , 309, 323
 Kneer, G , 139, 140, 433
 Knopoff, L , 458
 Knowles, J K 313
 Kobayashi, A S , 270
 Kobayashi, H., 318
 Kobayashi, S., 291, 460
 Kochs, U.F., 398
 Koda, M , 420, 497
 Koehler, J S , 19, 26, 313, 355, 361
 Koeller, R C , 419
 Kohn, W., 438
 Koiter, W T., 274
 Kondo, K., 7, 65, 73, 324
 Kontorova, T , 357
 Korringa, J., 438, 439
 Kosevich, A.M., 13, 348, 361
 Kossecka, E , 52, 348
 Kouris, D.A , 497
 Krajcinovic, D , 495
 Krishna Murty, A V., 274
 Krizek, R.J., 439
 Kröner, E , 1, 7, 13, 26, 29, 31, 32, 45, 53, 65, 73, 226, 227, 324, 340, 353, 361, 364, 431, 433, 439, 445, 446
 Kroupa, F., 297, 338, 339
 Krumhansl, J.A., 438, 458, 459
 Kuang, J.G , 467, 471
 Kundu, A.K., 453
 Kunin, I.A , 13, 324, 484, 496
 Kuo, H.H , 53, 452
 Kupradze, V.C., 298, 460
 Kurihara, T , 465
 Kuriyama, M., 477
 Kusumoto, S., 275
 Kuwata, T., 464
- L
 Lachenbruch, A.H , 274
 Lacht, J.C , 298
 Laird, C., 104, 110, 318, 379
 Lambermont, J H , 364
 Lambropolous, J.C., 495
 Lamé, M G , 177
 Langhaar, H.L., 42
 Lardner, R W , 314, 324, 348
 László, F , 220
 Latanision, R.M., 484
 Laub, T , 359
 Laws, N., 155, 433, 453
 Lee, E.H., 438
 Lee, J D , 309
 Lee, J.K., 104, 108, 109, 198, 214, 222, 226, 227
 Lee, M S., 358, 464, 471
 Lee, N G., 114
 Lee, S , 121, 494
 Leibfried, G., 13, 358, 359, 467
 Lejček, L , 29
 Lekhnitskii, S.G., 279
 Leo, P-H., 497
 Leon, A , 177
 Leslie, W C., 237
 Lévy, M , 363
 Lewis, J L , 475
 Li, G F., 498
 Li, H., 498
 Li, J C M , 50, 53, 291, 466, 467
 Li, P , 498
 Lie, K.H.C., 26
 Lieberman, D.S., 230, 232
 Liebowitz, H., 260, 309
 Lifshitz, I M , 26, 29
 Lighthill, M.J., 13, 507
 Lin, C.T., 314
 Lin, L.H., 292
 Lin, S C , 137, 217, 222, 223, 251, 252, 401, 465
 Lin, T H., 121, 379, 439, 445, 446
 Ling, C.B , 177
 List, R.D , 141
 Liu, G.C.T , 53
 Liu, X.H., 496
 Lizzio, R., 498

- Loges, F., 494
Lo, K H., 492
Lothe, J., 13, 44, 49, 324, 333, 334, 335, 336, 338, 359, 361
Love, A.E H., 177
Lowengrub, M., 260, 476
Lu, T.L., 338
Lucke, K., 361
Luk, C.H., 275
Lukás, P., 286
Luque, R F., 439
Lurie, A T., 178
Luton, M J., 493
- M
- MacMillan, W.D., 108
Mak, A.K., 475
Mal, A.K., 458, 459
Malén, K., 335, 348
Mandel, J., 439, 446
Mandl, G., 439
Mann, E., 20
Manson, S.S., 386
Maradudin, A.A., 477, 482
Marcoh, G., 48
Markenscoff, X., 359, 360
Martin, D E., 386
Martynenko, M.D., 298
Marukawa, K., 53
Marcinkowski, M J., 53, 356
Mason, W P., 360
Mastrojannis, E N., 299
Masuda, T., 378
Masumura, R.A., 465, 494
Matsuoka, A., 464
McCartney, L N., 314
McClintock, F.A., 311, 420
McCoy, J.J., 459
McLaughlin, R., 433, 453
Mehrabadi, M.M., 439
Meshii, M., 218
Michel, B., 494
Michell, J.H., 43
Miekkoja, H.M., 234
Mikata, Y., 42, 493, 494
Mikhlin, S.G., 298
Mikulinsky, M., 495
- Milne-Thomson, L.M., 41
Minagawa, S., 53, 348, 353, 356, 361, 438
Mindlin, R.D., 112, 113, 117, 177, 439, 494
Minster, B., 438, 455
Mirandy, L., 279
Mises, R von, 363
Miyamoto, H., 178, 187, 270, 279, 291, 313
Miyata, H., 275
Miyoshi, T., 270
Mochizuki, T., 413
Mogami, T., 439
Montheillet, F., 494
Monzen, R., 411, 420, 497
Moon, F C., 128, 155
Mor, T., 146, 168, 217, 218, 220, 223, 224, 230, 235, 238, 239, 328, 387, 392, 396, 401, 402, 405, 406, 407, 408, 411, 412, 413, 414, 419, 420, 492, 493, 496, 497
Moriguti, S., 7, 65
Morinaga, H., 484
Morita, H., 291
Morkov, V.A., 492
Morris, J.W., 493
Morris, P R., 433
Morse, P.M., 458
Moschovidis, Z A., 39, 160, 191, 196
Mossakovskii, V.I., 476
Mott, N F., 379
Mow, C.C., 458
Mroz, Z., 439
Mughrabi, H., 379
Mikherjee, A.K., 291, 323
Mura, T., 9, 11, 21, 26, 32, 47, 48, 51, 53, 78, 117, 121, 122, 123, 135, 137, 146, 151, 155, 160, 168, 169, 170, 178, 187, 198, 217, 218, 220, 222, 223, 224, 230, 235, 251, 252, 270, 277, 286, 293, 297, 298, 313, 314, 318, 324, 328, 338, 343, 345, 348, 350, 352, 356, 359, 360, 364, 367, 370, 371, 376, 378, 379, 382, 387, 401, 406, 411, 412, 419, 420, 438, 439, 447, 452, 459, 460, 463, 465, 469, 471, 484, 486, 487, 492, 496, 497, 498
Murakami, Y., 270, 275
Muskhelishvili, N I., 41, 187, 510
Mussot, P., 495

N

- Nabarro, F R N , 44, 49, 226, 324, 359, 360, 466
 Nagakura, S., 226
 Nakada, Y , 237
 Nakahara, I , 178
 Nakai, Y., 314
 Narayan, J , 291
 Narita, K., 402, 408
 Needleman, A., 449, 493
 Nemat-Nasser, S., 276, 291, 438, 439, 443, 449, 465, 472, 492, 496
 Neményi, P , 7, 45
 Neuber, H , 177, 187, 279
 Neuman, P., 318, 379
 Nicholson, R B , 168
 Niesel, W , 178
 Ninomiya, T , 359, 484
 Nishimura, N , 460
 Nisitani, H , 270, 274
 Niwa, Y., 460, 498
 Nix, W D , 493
 Noble, B , 497
 Nomura, S , 492, 495
 Novakovic, A , 218
 Nowick, A S , 168
 Noyan, I C , 493
 Nuismier, R J , 320
 Numan, K.C , 492
 Nye, J.F , 53, 338, 364, 373

O

- O'Connell, R J , 438, 455
 Oda, M , 439
 Ohnami, M , 379
 Ohr, S M , 291, 465
 Oka, H , 493
 Okabe, M , 411, 413, 414, 419
 Okamura, H , 291
 Olegin, I R , 492
 Olsson, P , 498
 Onat, E T , 495
 Onaka, S , 387, 411
 Ono, K , 102, 231, 233, 234
 Opanasovich, V K , 498
 Oranratnachai, A , 476

- Orlov, S S , 26, 48, 335
 Orowan, F , 44, 286, 342, 361
 Orsay Liquid Crystal Group, 49
 Osawa, T , 412, 413, 414
 Oshima, N , 439, 492
 Owen, D R J , 122, 123, 338, 379
 Owen, W S , 493

P

- Paetke, S , 493
 Palaniswamy, K , 309
 Pan, Y.C , 29, 31, 114, 338
 Panasyuk, V V , 298, 300
 Pao, Y H , 128, 155, 458
 Papanicolaou, G C , 494
 Papkovich, P F , 177
 Parihar, K S , 276
 Paris, P.C , 260, 314, 317
 Parnes, R., 378
 Pastur, L A., 361
 Paul, B , 279
 Peach, M.O , 313, 355
 Pebedrya, B E , 492
 Pedersen, O B , 493
 Pegel, B , 359
 Peierls, R.E , 358
 Penisson, J M , 168
 Perl, M , 495
 Peterson, R E , 187
 Petkovic-Luton, R A., 493
 Phillips, A., 364, 367, 446
 Pian, T H H , 275
 Pieranski, P , 49
 Polák, J , 386
 Polanyi, M., 44
 Pook, L P , 275
 Prager, W., 373, 439
 Prandtl, L , 363
 Pu, S.L., 497
 Purdy, G R., 169

R

- Radhakrishna, H C , 141
 Raj, R., 419, 420, 447
 Raju, I.S , 275
 Rao, A K , 275

- Read, T A , 230, 232
Read, W.T , 19, 26, 34, 36, 141, 273, 324, 335, 374
Reissner, H , 1, 7, 65
Reuss, A., 363, 421, 424
Rey, C., 495
Rice, J.R , 310, 311, 313, 314, 323, 420, 443, 449, 465, 471
Riedel, H , 291
Rieder, G , 73
Rizzo, F.J , 298
Robinson, C , 49
Robinson, K., 178
Rokhlin, S I , 493
Rongved, L , 124
Rose, L.R F , 495
Rosenfield, A R , 291, 323
Rosengren, G F , 314, 323
Routh, E J., 77
Rozenzweig, L N., 26, 29
Rudnicki, J.W , 448, 449
Russell, K.C , 219
Rvachev, V L., 302, 303
Rykba, M T , 476
Ryzhik, I M , 78
- S
- Saada, G , 338, 361
Sack, R A , 242
Sadowsky, M A , 177, 178, 196, 267, 279, 488
Safoglu, R , 420
Saito, K., 123, 338, 348
Saito, Y., 178
Salamon, N J., 338
Sankaran, R., 104, 110
Santare, M.H., 498
Sarosiek, A M., 493
Sass, S.L , 21
Satake, M., 439
Sato, A., 238, 239, 411, 420
Savin, G N , 187
Scattergood, R O , 13, 49, 324, 335, 398
Schaefer, H , 53, 353, 361
Schmid, E., 421
Schmueser, D., 472
Schneck, R , 493
Scriven, L E , 50
Seeger, A , 26, 53, 324
Sekerka, R F , 497
Seidel, E.D , 453
Sekine, H , 270, 338, 465
Selvadurai, A P.S , 178, 494, 496
Sendeckyj, G P , 41, 89, 433, 437, 438, 463, 465
Seo, K , 117
Shah, R C , 270
Shalaby, A H , 443
Shapiro, G S , 177
Sherman, D.I , 41
Shetty, D K , 218
Shibata, M., 102, 217, 231, 233, 234, 401
Shield, R T , 265, 439
Shigenaka, N , 497
Shinozaki, D M , 498
Shintani, K , 361
Shioiri, J., 378
Shioya, S , 464
Shioya, T., 378
Shiozawa, K , 379
Shockley, W., 26, 34, 36, 141, 335
Shokooh, A , 276, 439
Shtrikman, S., 433, 434, 493
Sideridis, E P., 494
Sides, A , 496
Sih, G C , 244, 260, 265, 268, 270, 472
Silovanyuk, V P , 492
Sills, L B , 420
Simmons, G , 504
Simmons, J.A., 324
Sines, G., 104, 465
Singh, B.M , 496
Slutsky, S , 453, 494
Smith, C S , 378
Smith, E., 73, 246, 290, 364, 374, 469
Smith, G C , 318
Smith, J , 50
Sneddon, I N , 242, 260, 267, 299, 476
Sobezyk, K , 459
Sokolnikoff, I S , 72
Sokolovsky, V V., 371
Somigliana, C , 13, 261, 270
Sorensen, E P., 311
Southwell, R V , 177
Spencer, A J M , 439

Sprys, J W , 234, 235, 236
 Srolovitz, D J., 493
 Stadnik, M M., 298, 300, 307, 492
 Stagni, I , 498
 Steeds, J W , 13, 324, 335
 Steketee, J.A , 13
 Sternberg, E , 177, 178, 196, 267, 279, 313,
 488
 Stigh, U , 495
 Stipps, M , 465
 Stobbs, W.M , 392, 401, 402, 408, 493
 Stokes, G.G , 64
 Storen, S., 449
 Stroh, A.N., 34, 37, 335, 467
 Suezawa, M , 233
 Sumi, Y , 275, 276
 Sun, C T., 438, 495
 Suzuki, H , 324
 Suzuki, K., 420, 497
 Sveklo, V A , 31
 Swain, M V , 495
 Swanger, L.A , 271, 335
 Swinden, K H , 288, 290
 Synge, J.L , 26

T

Tada, H., 275
 Tagaya, M , 233
 Taira, S , 386
 Takao, Y , 448, 492
 Takeuchi, T , 291
 Tamate, O , 465, 471
 Tamura, I , 233
 Tanaka, Ke , 88, 187, 314, 379, 382
 Tanaka, Ko., 218, 392, 396, 401, 492, 493
 Tanaka, M , 298
 Tanaka, R , 497
 Tanaka, Y., 238, 239
 Taya, M , 42, 48, 447, 448, 453, 492, 493,
 494, 495, 496, 497
 Taylor, G I , 19, 44, 439, 442, 493
 Teodosiu, C., 13, 47
 Tetelman, A.S , 469, 471
 Teutonico, L J., 358
 Theocaris, P S , 41, 286, 494, 496
 Thölen, A R., 231
 Thomas, S.L , 50

Thomas, T.Y., 449
 Thompson, A W , 386
 Thompson, D D., 439
 Thomson, P.F., 467
 Thomson, R., 291, 292, 465
 Thomson, R D., 494
 Timoshenko, S , 88, 170, 369
 Tokushige, H., 401, 405, 407, 408, 411
 Tomkins, B., 386
 Tong, P., 275
 Toyá, M., 471
 Toyoshima, M , 226
 Trantina, G G., 496
 Tráuble, H , 49, 53
 Triantafyllidis, N., 495
 Truell, R , 458
 Tsai, Y.M., 496
 Tsuchida, E , 121, 178, 492, 497
 Tuba, I.S , 275
 Tucker, M O., 471
 Tung, T K , 121
 Tvergaard, V., 495

U

Ukadgaonker, V G , 41
 Umekawa, S , 493
 Uzan, J., 496

V

Varadan, V K , 459
 Varadan, V V , 459
 Varatharajulu, V , 459
 Vause, R F., 443
 Vilmann, C., 286, 371
 Visscher, W M., 459
 Viswanathan, K , 497
 Vitek, V., 291
 Voigt, W , 421
 Volterra, V., 7, 13, 44, 46, 49, 65

W

Wakashima, K , 401, 493
 Walpole, L.J , 39, 42, 433, 492, 493
 Walsh, J.B., 438, 455
 Wang, H., 504

- Wang, Z.Y., 496
Ward, J.C., 49
Warren, W.E., 498
Waterman, P.C., 459
Watson, G.N., 164, 170, 510
Watson, J.O., 298
Watwood, V.B., 275
Wayman, C.M., 229, 231
Weaver, J., 298, 307
Wechsler, M.S., 230, 232
Wedge, D.E., 493
Weeks, R., 465
Weertman, J., 286, 291, 292, 314, 318, 324, 358, 359
Weertman, J.R., 324
Weiner, J.H., 360, 484
Weingarten, G., 7, 13, 44, 65
Weng, G.J., 364, 367, 446, 447, 492
Werne, R.W., 364
Westergaard, L.T., 274
Wheeler, L.T., 498
Wheeler, P., 438
Whelan, M.J., 41, 44
Whittaker, E.T., 164, 170, 510
Wigglesworth, L.A., 274
Williams, C.E., 49
Williams, J.C., 233
Williams, M.L., 275, 323, 360, 471
Willis, J.R., 18, 26, 32, 47, 55, 141, 198, 260, 262, 265, 270, 271, 325, 328, 333, 433, 435, 460, 465, 476
Wilson, W.K., 275
Witterholt, E.J., 458
Wnuk, M.P., 310, 311
Wu, J.K., 495
Wu, T.T., 431, 433, 439, 445, 446
- Y
- Yagi, K., 379
Yamamoto, N., 379
Yamamoto, Y., 275
Yang, H.C., 141, 142
Yang, K.L., 177
Ying, C.F., 458
Yoffé, E.H., 322, 338
Yakobori, T., 275, 310
Yonezawa, F., 484
Yoo, M.H., 465
Yoon, H.S., 453, 495
Yoshimura, H., 233
Yu, I.W., 41
- Z
- Zaoui, A., 495
Zarka, J., 379, 446
Zener, C., 222, 224, 419
Zerna, W., 141, 295
Zhang, H.T., 128, 494, 496
Zhong, W.F., 498
Zienkiewicz, O.C., 275
Zimmerman, R.W., 493
Zorski, H., 348

Subject index

A

Abel's integral equation, 284
accelerated motion of dislocation, 360
addition theorem, 164
analogy between electromagnetic- and moving dislocation fields, 361
angular dislocation, 338
anisotropic inclusion, 129
anisotropic precipitates, 220
annealing, 411
arbitrarily shaped plane crack, 297
Asaro and Barnett formula, 337
Asaro et al formula, 334
attenuation, 455
average elastic moduli of composite materials, 421
average of internal stress, 388

B

back stress, 402
Bauschinger effect, 402
BCS model, 288, 313
beam, 373
bifunction, 449
Born approximation, 458
boundary integral equation method, 460
Boussinesq's potential, 489
Brown and Lothe formula, 338
Bulk modulus, 4
Bullough-Gilman crack, 471
Burgers circuit, 45, 68
Burgers vector, 45, 73

C

catastrophic crack propagation, 311
Cauchy principal value, 510
Cauchy singular equation, 475
cavity, 494, 495
center of dilation, 127
Cesáro's integral, 69
chain model, 484
Coffin-Manson law, 379, 385
coherent precipitate, 227
cohesive zone, 281, 472
Colonnetti's theorem, 212
compatibility, 3, 6
complex potential method, 41
compliance, 453, 504
composite material, 213, 421, 433, 453, 492, 493, 494, 495
constitutive equation, 362, 364
continuous distribution of dislocation, 340, 466
Cosserat medium, 53, 439
crack, 240
crack branching, 472
crack closure, 314
crack embryo, 384
crack extension force, 309, 475
crack growth, 307
crack initiation, 379
crack opening displacement, 247, 273
crack tip dislocation, 371
cracks in two phase materials, 471
creep, 412, 447, 449, 494
-function, 450
critical stresses of cracks, 240, 248, 274

cubic crystal, 14, 137, 503
 cuboidal inclusion, 104, 121
 cuboidal precipitates, 20, 198
 cyclic creep, 218
 cyclic loading, 314, 379
 cylinder, 80, 141, 373

D

debonding, 471, 492, 497
 deviatoric strain, 182
 deviatoric stress, 362
 diffusional relaxation, 406
 diffusionless transformation, 359
 dilatational eigenstrain, 103, 114, 127, 156
 Dirac's delta function, 12, 21, 22, 46, 507
 direct observation of dislocation, 44
 disclination, 49, 50, 70
 direction of-, 50
 strain energy of-, 53
 -density tensor, 53
 -dipole, 338
 -loop, 3, 68
 -network, 338
 -of twist type, 50, 452
 -of wedge type, 50
 discontinuity, 39, 155
 dislocation, 44, 45, 70, 324
 direction of-, 45
 circular-, 338, 359, 360
 -core radius, 354
 density tensor, 52, 338
 -dipole, 371, 379
 -flux tensor, 345, 363
 -loop, 3, 68, 324
 -pile up, 466, 510
 -segment, 328
 -velocity tensor, 345
 helical-, 338
 imperfect-, 45
 moving-, 55, 57
 partial-, 45
 straight-, 327
 Volterra's-, 71
 dislocation loop density tensor, 339
 dispersion curve, 360, 438
 dispersion hardened alloys, 398
 displacement gradient, 45

dissipation, 361
 distant parallelism, 73
 distortion, 45, 478
 elastic-, 45
 plastic-, 45
 total-, 45
 divergency law, 351
 Dugdale-Barenblatt crack, 280, 343
 Dundurs' constant, 43, 471, 473
 dynamic crack growth, 319
 dynamic problems, 497
 dynamic fracture toughness, 321

E

earthquake, 449
 edge dislocation, 18
 effect of isotropic elastic moduli on stress, 42
 effective strain increment, 362
 effective stress, 362
 eigendistortion, 478
 eigenfunction expansion, 458
 eigenstrain, 1
 -problem, 3
 dilatational-, 103, 114, 156
 dynamic solution of-, 53
 harmonic-, 161
 periodic-, 7, 121, 144
 polynomial-, 89, 158, 173
 shear-, 486, 487
 uniform-, 74, 134
 eigenstress, 1
 eigenvalue, 59
 eigenvector, 69
 Einstein relation, 418
 Einstein summation convention, 499
 Eisenstein's theorem, 171
 elastic
 -compliance, 3, 504
 -constant, 3, 504
 -distortion, 341, 478
 -modulus, 3, 501
 -polarization, 1
 -stiffness, 504
 -strain, 3
 -strain energy, 97, 204, 353
 ellipsoid, 74

ellipsoidal cavity, 279
 elliptic cylinder, 80, 94, 141
 elliptic integral, 83
 elliptical plate, 95
 embedded weaken zone, 449
 embryonic crack, 384
 energy and force of dislocation, 353
 energy of inclusions, 97
 energy of inhomogeneities, 204
 energy balance, 215, 319
 energy dissipation, 216, 361
 energy momentum tensor, 312
 energy release rate, 307, 309, 319, 496
 England solution, 475
 equilibrium condition, 5
 equivalency equations, 179, 199
 homogeneous-, 199
 equivalent eigenstrain, 179
 equivalent inclusion method, 40, 178
 Eshelby's method, 428
 Eshelby's solution, 74
 Eshelby's tensor, 77, 443, 448
 Euler angle, 423, 445
 Euler-Schouten curvature tensor, 73
 exterior, 84, 88, 149, 187
 extrusion, 379

F

f-vector, 458
 fatigue, 314, 371, 379
 Ferrers and Dyson formula, 95
 finite element method, 275
 flat ellipsoidal, 83, 143, 161
 -crack, 244
 flow stress, 362
 force constant, 477
 Fourier
 -series, 9, 505
 -integral, 9, 505
 fracture criterion, 241
 fracture under non-uniform applied stress, 253
 Frank dislocation network, 123
 Fredholm integral equation, 298
 free body, 5
 free energy, 100
 fundamental equation of elasticity, 3
 fundamental metric tensor, 72

G

gamma-function, 165
 gange invariant, 353
 Gauss' symbol, 174
 generalized function, 13
 generalized plane problem, 34
 Gibbs free energy, 100, 204, 208, 309, 400, 416
 grain boundary sliding, 447
 granual material, 439
 Green's formula, 460
 Green's function, 11, 21
 -for anisotropic materials, 25
 -for transversely isotropic material, 26, 114
 -in half-space, 110, 112
 -in joint half-spaces, 123
 Mindlin's-, 494
 -of steady-state, 64
 derivatives of-, 32
 dynamic-, 55, 57
 two-dimensional-, 34
 Griffith equation, 286
 Griffith fracture criterion, 209, 241, 309

H

half-space, 110, 123, 464, 494
 harmonic approximation, 477
 harmonic eigenstrain, 161
 Hashin-Strikman's method, 433, 493
 heat conductivity, 492
 Heaviside step function, 16, 407
 Helmholtz equation, 459
 Helmholtz free energy, 100
 Hencky relation, 367
 hexagonal crystal, 14, 139, 503
 Hilbert integral equation, 272, 289, 510
 Hill's theory, 426
 homogeneous equivalency equations, 199
 Hooke's law, 3

I

I-integral, 77, 85, 92
 image stress, 393
 impotent eigenstrain, 199

- dislocation, 343, 365
- inclusion, 202
- inclusion, 2, 38, 74, 129, 177
- incoherent precipitate, 226, 414
- incompatibility, 1, 7, 65
 - tensor, 69
- Indenbom and Orlov formula, 48
- indenter, 368
- inhomogeneity, 2, 40, 177, 181
 - anisotropic-, 187
- inhomogeneous-, 177, 180, 452
 - inclusion, 179
- instability, 449
- interacting tension cracks, 276
- interaction between dislocations and inclusions, 463
- interaction energy, 99, 208, 353
- interface discontinuity, 39
- interior, 75, 92, 133
- internal friction, 419
- internal stress, 1
- intrusion, 379
- irreversibility of plastic work, 355
- isotropic, 13, 22
 - transversely-, 14, 26
- J**
- J-integral, 311
- jointed half-space, 123
- K**
- Kelvin's material, 218
- kinematically admissible field, 373
- Kronecker delta, 2, 481, 499
- Kröner's expression, 340
- Kröner's formula, 31
- L**
- Laguerre polynomial, 467
- Lamé constant, 4
- Laplace transform, 508
- lattice theory, 477
- Legendre polynomial, 31, 161
 - function, 489
- Levy and Mises equation, 303
- line integral expression
 - for displacement, 348
 - for strain, 47, 48
 - for velocity field, 349
 - plastic strain, 348
- line of discontinuity, 367
- linear connection, 73
- liquid crystal, 49
- locked dislocation, 467
- Lorentz force, 357, 360
 - condition, 353
 - contraction, 359
- M**
- Mach cone, 359
- macroscopic strain, 389
- martensite, 218, 492, 493
 - spherical-, 234
 - transformation, 229
- mass diffusion, 228
- Maxwell tensor of elasticity, 311
- Mellin transform, 471
- Mindlin's solution, 112
- minimum principle for strain, 441, 442
- minimum principle of free energy, 228
- minimum strain energy, 223
- Mises yield criterion, 362, 364, 365
- misfit strain, 1, 406
- modulated structure, 224
- Mori and Tanaka's theory, 394, 453, 480, 492
- motor, 53
- Mott assumption, 379
- moving dislocation, 55, 57, 355
- Mura's formula, 45, 47
- N**
- Newtonian viscous solid, 218
- non-elastic matrix, 492, 494
- nonelastic strain, 1
- non-metallic inclusion, 238
- non-uniform applied stress on a crack, 268
- Nye's tensor, 53

O

- oblate spheroid, 84, 94, 488
- Orowan mechanism, 398
- Orowan-Irwin formula, 286
 - theory, 310
- Orowan's loop, 342, 399, 404, 408
- orthorhombic material, 142, 504
- oscillating dislocation, 359

P

- Pankovich-Neuber function, 124, 282
- parallel dislocation, 325
- Paris law, 260, 314, 317
- path-independent integrals, 313
- Peach-Koehler force, 313, 355, 463, 466
- Peierls dislocation, 358
 - stress law, 360
- penny shape, 81, 184, 217
 - crack, 240, 252, 292, 318
- periodic eigenstrain, 7, 121, 144
 - distribution of spherical inclusions, 165
- permutation tensor, 6, 8, 499
- persistent slip band, 379
- Petch-type equation, 379, 387
- phase transformation, 1
- plane strain, 5, 25, 275
- plane strain problem, 365
- plane stress, 5, 25, 275
- plastic distortion, 339, 341
 - rate, 346
- plastic polycrystal, 439
- plastic rotation, 51
- plasticity, 361
- Poisson's ratio, 4
- polarization, 1
- polarization stress, 434
- polycrystal, 439
- polygonization, 344
- polynomial applied strain, 188
 - eigenstrain, 89
 - eigenstrains in anisotropic materials, 158
- porous media, 439
- potent dislocation, 365
- potential energy, 99, 479

- Prandtl-Reuss relation, 361, 363
- precipitation, 180, 218, 492, 493
- process zone, 310
- projection integral equation, 496
- prolate spheroid, 84, 488
- H integrals, 255

R

- R_1 -conjunction, 303
- ratcheting, 379
- Rayleigh-Gauss approximation, 458
- Rayleigh wave, 322, 358
- recovery strain, 412
- reduced strain, 182, 453, 502
 - stress, 362, 502
- relaxation, 406
 - function, 450
 - moduli, 453
 - strength, 419
 - time, 418
- residual stress, 1
- resolved shear stress, 440
- Reuss approximation, 424
- Reuss equation, 363, 364
- Ricci tensor, 71
- Riemann-Christoffel tensor, 71
- Riemannian space, 71, 72
- rod, 185, 217
- Rodrigues' formula, 164
- Rudnicki's theory, 449

S

- scattering cross-section, 359, 498
- Schouten's torsion tensor, 73
- screw dislocation, 15
 - moving-, 57
- seismic velocity, 455
- self-consistent method, 430, 443
- self-force, 354
- sgn ξ , 507
- shear line, 366
 - shear modulus, 4
- singular, 192, 198
- sink of dislocations, 53
- sliding inclusion, 484, 492, 497
- slip, 2

- plane, 45, 440
- slipping interface, 465
- slit-like crack, 242, 271
- Somigliana dislocation, 48, 485, 497
 - equation, 298
- source of dislocation, 53
- sphere, 79, 93, 183
- spheriod, 84, 137, 223, 488
- spherulite, 498
- standard linear solids, 419, 453
- state quantity, 51, 350
- Stokes' theorem, 47
- strain
 - elastic-, 46
 - total-, 46
- strain energy, 97, 204
 - of dilatational eigenstrain, 103
 - of elliptic cylinder, 101
 - of penny-shaped flat ellipsoid, 101
 - of spherical inclusion, 101
 - of spheroid, 102
- strain hardening, 441
- strain polarization, 436
- stress concentration factor, 40, 155, 277
- stress corrosion, 387
- stress-free transformation strain, 1
- stress intensity factor, 155, 245, 260, 264, 275, 297, 496
 - elastodynamic-, 320
- stress intensity function, 320
- stress jump, 40
- stress orienting precipitation, 237
- stress singularity, 323, 471
 - oscillatory-, 472, 475
- surface dislocation density, 341, 367, 373

- T
- Tanaka-Mori's theorem, 390, 429
- Taylor's analysis, 439
- Taylor's hardening law, 443
- tetragonal crystal, 504
- thermal crack, 276
- thermal expansion, 1, 88, 492
 - coefficient, 2
- thermal stress, 1, 88
- thermoelastic effect, 360, 493, 494
- thin film, 123
- threshold velocity, 358
- torsion, 192, 376
- total strain, 3

- potential energy, 99, 208
- toughening, 495
- transformation strain, 1
- transversely isotropic, 26, 503
- Tresca's yield condition, 293
- twin, 230, 234
- two ellipsoidal inhomogeneities, 192
- two gas bubbles, 198
- two spherical cavities, 196

- U
- uniform eigenstrain, 74, 134
- upper and lower bounds, 433

- V
- V-integral, 91
- variant, 237
- velocity intensity function, 320
- viscoelasticity, 449
- Voigt approximation, 421
- Voigt constants, 140, 502
- Volterra formula, 45

- W
- wave in viscoelasticity material, 453, 455
- wave equation, 362
- wave scattering, 455
- wave velocity
 - dilatational-, 56, 455
 - shear-, 56, 455
- Weibul distribution, 495
- Wiener-Hopf technique, 471
- William's coefficient, 275
- Willis' formula, 311
- Wnuk's equation, 311
- work-hardening, 362, 398, 404

- Y
- yield function, 447
- yield stress, 362
- yield surface, 446
- Young's modulus, 4, 501

- Z
- Zener anisotropic factor, 222
- Zener-Stroh crack, 471