Open Channel Flow with Manning's Equation

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Abstract

1 Theory

1.1 Manning's Equation

The Manning's equation for the velocity and discharge of uniform flow in open channels is [1, 2, 4, 3]:

$$v = \frac{K_u}{n} R^{\frac{2}{3}} S^{\frac{1}{2}},\tag{1}$$

$$Q = vA = \frac{K_u}{n} A R^{\frac{2}{3}} S^{\frac{1}{2}} = \frac{K_u}{n} A^{\frac{5}{3}} P^{-\frac{2}{3}} S^{\frac{1}{2}}, \tag{2}$$

where

v = Mean velocity, m/s (ft/s),

 $Q = \text{Discharge}, \, m^3/s \, (ft^3/s),$

n = Manning's coefficient of roughness,

R = Hydraulic radius, m (ft). R = P/A,

P = Wetted perimeter, m (ft),

A = Crossing-section area of flowing water perpendicular to the direction of flow, m^2 (ft^2) ,

 $S={\rm Energy}$ slope, m/m (ft/ft). For steady uniform flow $S=S_0$, and

 $K_u = \text{units conversion factor}$, 1 for SI, 1.486 for English units.

1.2 Normal Depth

Depending on the geometry of the channel, both A and P are depondent on normal depth y. To calculate y for a given Q, we solve

$$f_d(y_i) = \frac{K_u}{n} A^{\frac{5}{3}} P^{-\frac{2}{3}} S^{\frac{1}{2}} - Q \tag{3}$$

for $f_d(y_i) = 0$. Analytical solutions are available in some special cases, for example, triangular channels. In general, a numerical solution is used to solve the nonlinear equation iteratively. Using Newton's method [5], the iteration starts with an initial guess y_0 , and iterates with

$$y_{i+1} = y_i - \frac{f_d(y_i)}{f_d'(y_i)} \tag{4}$$

where

$$f'_d(y_i) = \frac{\partial f_d}{\partial y} = \frac{K_u}{n} S^{\frac{1}{2}} \left(\frac{5}{3} R^{\frac{2}{3}} \frac{\partial A}{\partial y} - \frac{2}{3} R^{\frac{5}{3}} \frac{\partial P}{\partial y} \right)$$
 (5)

until

$$|f_d(y_i)| \le TOLQ,\tag{6}$$

$$|y_{i+1} - y_i| \le TOLD,\tag{7}$$

or

$$i \ge MAXI$$
 (8)

with

 $TOLQ = discharge tolerance, m^3/s (ft^3/s),$

TOLD =depth tolerance, m(ft),

MAXI = maximum number of iteration.

When the iteration stops with criteria Eq.(8), an error is shown to notify the user. The user may adjust y_0 or t_0 , TOLQ, TOLD, and/or MAXI to obtain a satisfactory solution. For circular, elliptical, and arch pipes, a replacement of variable y with an alternative variable t (for example) is convient. These equations remain valid. An alternative tolerance TOLA (in lieu of TOLD) is specified when t is an angle.

For open channel flow in closed conduits such as circular, elliptical, and arch pipes, the calculated discharge peaks before the pipe is full [1, 2, 4]. The normal depth y_{max} or t_{max} at peak discharge Q_{max} can be calculated by solving

$$\frac{\partial Q}{\partial t} = \frac{K_u}{n} \left(\frac{5}{3} R^{\frac{2}{3}} \frac{\partial A}{\partial t} - \frac{2}{3} R^{\frac{5}{3}} \frac{\partial P}{\partial t} \right) S^{\frac{1}{2}} = 0.$$
 (9)

or

$$f(t) = 5P\frac{\partial A}{\partial t} - 2A\frac{\partial P}{\partial t} = 5PA' - 2AP' = 0.$$
 (10)

In absence of an analytical solution, Newton's method is used with

$$t_{i+1} = t_i - \frac{f(t_i)}{f'(t_i)},\tag{11}$$

$$f'(t_i) = 5A''P + 3A'P' - 2AP''. (12)$$

If $Q > Q_{max}$, an error is shown with y_{max} returned as normal depth.

In cases where Q is not a monotonically increasing function with y, multiple y values may result in the same Q value. The lowest value is returned as normal depth.

In summary, to calculate normal depth with Newton's method, we need A, P, A' and P'. A" and P" are needed when Q_{max} needs to be calculated using Newton's method.

1.3 Critical Depth

Critical flow occurs when the specific energy

$$E = \frac{v^2}{2g} + y = \frac{Q^2}{2gA^2} + y \tag{13}$$

reaches a minimum (g is acceleration of gravity, 32.17 ft/s^2 for US Customery units, 9.81 m/s^2 for SI). Namely,

$$\frac{\partial E}{\partial y} = -\frac{Q^2}{gA^3} \frac{\partial A}{\partial y} + 1 = 0. \tag{14}$$

To solve the equation with Newton's method, critical depth y_c or t_c is calculated by

$$t_{c,i+1} = t_{c,i} - \frac{f(t_{c,i})}{f'(t_{c,i})} \tag{15}$$

where

$$f_c(t) = gA^3 - Q^2 \frac{\partial A}{\partial y} \tag{16}$$

$$f'_c(t) = 3gA^2 \frac{\partial A}{\partial t} - Q^2 \frac{\partial}{\partial t} \left(\frac{\partial A}{\partial y} \right)$$
 (17)

The critical velocity v_c is

$$v_c = \frac{Q}{A_c} = \sqrt{g \frac{A}{\frac{\partial A}{\partial y}}} = \sqrt{gD}$$
 (18)

where $D=A/\frac{\partial A}{\partial y}$ is the hydraulic depth.

The Froude number, $F = v/v_c$. The critical slope S_c is backcalculated from Eq. (1).

2 Triangular

$$A = \frac{1}{2}(z_1 + z_2)y^2 \tag{19}$$

$$P = \left(\sqrt{1 + z_1^2} + \sqrt{1 + z_2^2}\right) y \tag{20}$$

$$R = \frac{1}{2} \frac{(z_1 + z_2)}{\sqrt{1 + z_1^2} + \sqrt{1 + z_2^2}} y \tag{21}$$

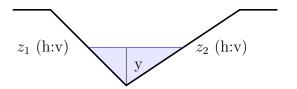


Figure 1: Triangular Section

Eq. (2) becomes

$$Q = \frac{K_u}{n} \frac{z_1 + z_2}{2} \left(\frac{1}{2} \frac{z_1 + z_2}{\sqrt{1 + z_1^2} + \sqrt{1 + z_2^2}} \right)^{\frac{2}{3}} y^{\frac{8}{3}} S^{\frac{1}{2}}.$$
 (22)

Note this equation can be used to backcalculate y from Q.

$$\frac{\partial A}{\partial y} = (z_1 + z_2)y\tag{23}$$

Eq. (16) becomes

$$f_c(y) = \frac{g}{8}(z_1 + z_2)^3 y^6 - Q^2(z_1 + z_2)y = 0.$$
 (24)

$$y_c^5 = \frac{8Q^2}{g(z_1 + z_2)^2} \tag{25}$$

3 Rectangular

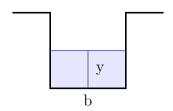


Figure 2: Rectangular Section

$$A = by (26)$$

$$P = b + 2y \tag{27}$$

$$R = \frac{by}{b+2y} \tag{28}$$

$$\frac{\partial A}{\partial y} = b \tag{29}$$

$$f_c(y) = gb^3y^3 - Q^2b = 0 (30)$$

$$y_c^3 = \frac{Q^2}{gb^2} \tag{31}$$

4 Trapezoidal

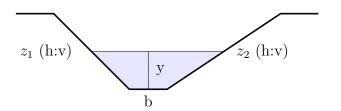


Figure 3: Trapezoidal Section

$$A = \frac{1}{2}(z_1 + z_2)y^2 + by \tag{32}$$

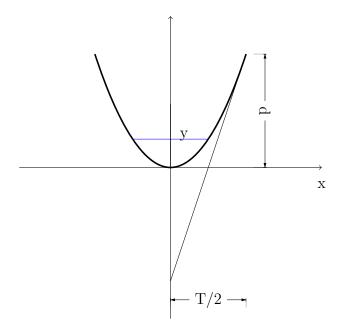
$$P = \left(\sqrt{1 + z_1^2} + \sqrt{1 + z_2^2}\right) y + b \tag{33}$$

$$\frac{\partial A}{\partial y} = (z_1 + z_2)y + b \tag{34}$$

$$\frac{\partial^2 A}{\partial y^2} = z_1 + z_2 \tag{35}$$

$$\frac{dP}{dy} = \sqrt{1 + z_1^2} + \sqrt{1 + z_2^2} \tag{36}$$

5 Parabolic



For a parabola with top width T and depth d,

$$y = 4\frac{d}{T^2}x^2\tag{37}$$

$$A = 2xy - \int_{-x}^{x} 4\frac{d}{T^{2}}x^{2}dx = \frac{16}{3}\frac{d}{T^{2}}x^{3} = \frac{2}{3}\frac{T}{\sqrt{d}}y^{\frac{3}{2}}$$
(38)

$$\frac{\partial A}{\partial y} = T\sqrt{\frac{y}{d}} \tag{39}$$

$$\frac{\partial^2 A}{\partial u^2} = \frac{T}{2\sqrt{ud}}\tag{40}$$

To calculate P and P',

$$P = 2 \int_0^y \sqrt{1 + (dx/dy)^2} dy \tag{41}$$

$$\frac{\partial y}{\partial x} = 8\frac{d}{T^2}x\tag{42}$$

$$\left(\frac{\partial y}{\partial x}\right)^2 = 16\frac{d}{T^2}y = \frac{y}{a} \tag{43}$$

with $a = T^2/16d$

$$\int \sqrt{1 + \frac{a}{t}} dt = \frac{a}{2} \ln \left(\sqrt{1 + \frac{a}{t}} + 1 \right) - \frac{a}{2} \ln \left(|\sqrt{1 + \frac{a}{t}} - 1| \right) + x \sqrt{1 + \frac{a}{t}} + c \tag{44}$$

$$\int \sqrt{1 + \frac{a}{t}} dx = \frac{a}{2} \ln \left| \frac{\sqrt{1 + \frac{a}{t}} + 1}{\sqrt{1 + \frac{a}{t}} - 1} \right| + \sqrt{t^2 + at} + c = \frac{a}{2} \ln \left| \frac{\sqrt{t^2 + at} + t}{\sqrt{t^2 + at} - t} \right| + \sqrt{t^2 + at} + c$$
(45)

$$P = 2\int_0^y \sqrt{1 + (dx/dy)^2} dy = 2\int_0^y \sqrt{1 + \frac{a}{y}} dy = a \ln \left| \frac{\sqrt{y^2 + ay} + y}{\sqrt{y^2 + ay} - y} \right| + 2\sqrt{y^2 + ay}$$
 (46)

$$\frac{dP}{dy} = a \frac{\frac{2y+a}{\sqrt{y^2+ay}} + 1}{\sqrt{y^2+ay} + y} - a \frac{\frac{2y+a}{\sqrt{y^2+ay}} - 1}{\sqrt{y^2+ay} - y} + \frac{2y+a}{\sqrt{y^2+ay}}$$
(47)

Let $z = \sqrt{y^2 + ay}$,

$$\frac{dP}{dy} = a\frac{\frac{2y+a}{z}+1}{z+y} - a\frac{\frac{2y+a}{z}-1}{z-y} + \frac{2y+a}{z} = a\frac{2y+a+z}{z^2+yz} - a\frac{2y+a-z}{z^2-yz} + \frac{2y+a}{z} \tag{48}$$

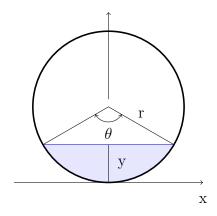


Figure 4: Circular Section

6 Circular

6.1 Basic Equations

$$y = r(1 - \cos\frac{\theta}{2})\tag{49}$$

$$A = \frac{1}{2}(\theta - \sin \theta)r^2 \tag{50}$$

$$P = \theta r \tag{51}$$

$$R = \frac{1}{2} \left(1 - \frac{\sin \theta}{\theta} \right) r \tag{52}$$

$$T = 2r\sin\frac{\theta}{2} \tag{53}$$

$$\frac{dy}{d\theta} = \frac{r}{2}\sin\frac{\theta}{2} \tag{54}$$

$$\frac{dA}{d\theta} = \frac{1}{2}(1 - \cos\theta)r^2\tag{55}$$

$$\frac{dP}{d\theta} = r \tag{56}$$

$$\frac{dT}{d\theta} = r\cos\frac{\theta}{2} \tag{57}$$

6.2 Calculate Maximum Discharge Using Manning's Equation

$$Q = \frac{K_u}{n} A R^{2/3} S^{1/2} = \frac{K_u}{n} A^{5/3} P^{-2/3} S^{1/2}, \tag{58}$$

$$\frac{dQ}{d\theta} = \frac{K_u}{n} \left(\frac{5}{3} R^{2/3} \frac{\partial A}{\partial \theta} - \frac{2}{3} R^{5/3} \frac{\partial P}{\partial \theta} \right) S^{1/2} = 0, \tag{59}$$

$$5\frac{\partial A}{\partial \theta} - 2R\frac{\partial P}{\partial \theta} = 0, (60)$$

$$3\theta - 5\theta\cos\theta + 2\sin\theta = 0, (61)$$

$$\theta_{max} = 5.27810713,\tag{62}$$

$$Q_{max} = 2.2189 \frac{K_u}{n} r^{8/3} S^{1/2}, (63)$$

$$y_{max} = 1.87636243r, (64)$$

6.3 Calculate Normal Depth

For Q less than Q_{max} ,

$$\theta_{i+1} = \theta_i - \frac{f_d(\theta_i)}{f_d'(\theta_i)} \tag{65}$$

where

$$f_d(\theta_i) = \frac{K_u}{n} A^{5/3} P^{-2/3} S^{1/2} - Q \tag{66}$$

$$f_d'(\theta_i) = \frac{K_u}{n} \left(\frac{5}{3} R^{2/3} \frac{\partial A}{\partial \theta} - \frac{2}{3} R^{5/3} \frac{\partial P}{\partial \theta} \right) S^{1/2}$$
 (67)

6.4 Calculate Critical Depth

$$\frac{\partial A}{\partial y}(\theta) = \frac{\frac{\partial A}{\partial \theta}}{\frac{\partial y}{\partial \theta}} = \frac{1 - \cos \theta}{\sin \theta/2} r = 2r \sin \frac{\theta}{2}$$
 (68)

$$\frac{\partial}{\partial \theta} \left(\frac{\partial A}{\partial y} \right) = r \cos \frac{\theta}{2} \tag{69}$$

$$f_c(\theta) = gA^3 - Q^2 \frac{\partial A}{\partial y} = gA^3 - 2rQ^2 \sin\frac{\theta}{2}$$
 (70)

$$f'_{c}(\theta) = 3gA^{2} \frac{\partial A}{\partial \theta} - Q^{2} \frac{\partial}{\partial \theta} \left(\frac{\partial A}{\partial y} \right)$$
 (71)

7 Elliptical

7.1 Basic Equations

$$u = b\cos\alpha$$

$$v = a\sin\alpha$$
(72)

$$\tan \beta = -\frac{a}{b} \tan \alpha \tag{73}$$

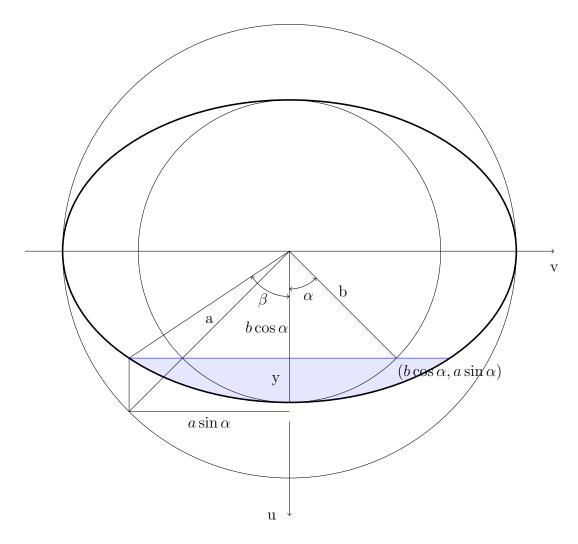


Figure 5: Elliptical Section $(a \ge b)$

$$\sin^2 \alpha = \frac{\tan^2 \alpha}{1 + \tan^2 \alpha} = \frac{b^2 \tan^2 \beta}{a^2 + b^2 \tan^2 \beta}$$
 (74)

$$\cos^2 \alpha = \frac{1}{1 + \tan^2 \alpha} = \frac{a^2}{a^2 + b^2 \tan^2 \beta} \tag{75}$$

$$r^2 = a^2 \sin^2 \alpha + b^2 \cos^2 \alpha \tag{76}$$

7.2 Wet Perimeter

7.2.1 $a \ge b$

In the case of $a \ge b$ (Figure 5), the wet perimeter is

$$P = 2\int_0^\alpha \sqrt{a^2 \cos^2 t + b^2 \sin^2 t} dt = 2a\int_0^\alpha \sqrt{1 - (1 - b^2/a^2)\sin^2 t} dt = 2aE(\alpha, \eta)$$
 (77)

with $\eta^2 = 1 - b^2/a^2$, $E(\alpha, \zeta)$ as the Legendre ellitipical integral of the second kind.

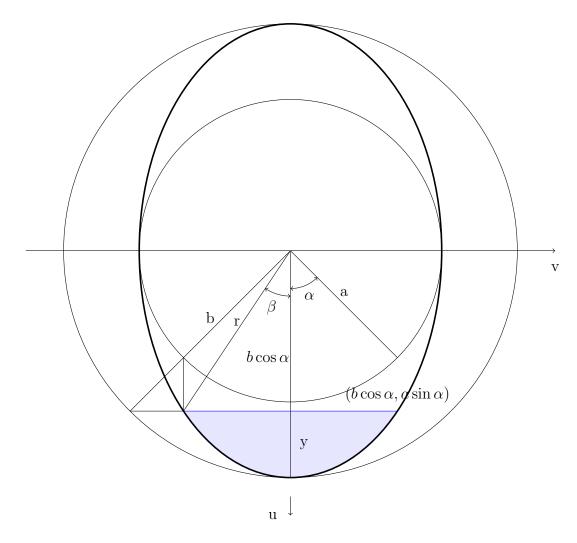


Figure 6: Elliptical Section (a < b)

$$E(\alpha, \eta) = \alpha - \frac{1}{2}\eta^{2} \int_{0}^{\alpha} \sin^{2}t dt - \frac{1}{2 \cdot 4}\eta^{4} \int_{0}^{\alpha} \sin^{4}t dt - \frac{1 \cdot 3}{2 \cdot 4 \cdot 6}\eta^{6} \int_{0}^{\alpha} \sin^{6}t dt + \dots$$
 (78)

$$\int \sin^{n}t dt = -\frac{1}{n} \sin^{n-1}\cos t + \frac{n}{n-1} \int \sin^{n-2}t dt$$

$$\int_{0}^{\alpha} \sin^{2}t dt = -\frac{1}{2} \sin \alpha \cos \alpha + \frac{1}{2}\alpha$$

$$\int_{0}^{\alpha} \sin^{4}t dt = -\frac{1}{4} \sin^{3}\alpha \cos \alpha + \frac{3}{4} \int_{0}^{\alpha} \sin^{2}t dt$$

$$\int_{0}^{\alpha} \sin^{6}t dt = -\frac{1}{6} \sin^{5}\alpha \cos \alpha + \frac{5}{6} \int_{0}^{\alpha} \sin^{4}t dt$$

$$\int_{0}^{\alpha} \sin^{2n}t dt = -\frac{1}{2n} \sin^{2n-1}\alpha \cos \alpha + \frac{2n-1}{2n} \int_{0}^{\alpha} \sin^{2n-2}t dt$$

7.2.2 a < b

For a < b, the wet perimeter is (Figure 6)

$$P = 2\int_0^\alpha \sqrt{a^2 \cos^2 t + b^2 \sin^2 t} dt = 2b \int_0^\alpha \sqrt{1 - (1 - a^2/b^2) \cos^2 t} dt = 2bF(\alpha, \zeta)$$
 with $\zeta^2 = 1 - a^2/b^2$. (80)

$$F(\alpha, \eta) = \alpha - \frac{1}{2} \zeta^{2} \int_{0}^{\alpha} \cos^{2}t dt - \frac{1}{2^{2} \cdot 2!} \zeta^{4} \int_{0}^{\alpha} \cos^{4}t dt - \frac{1 \cdot 3}{2^{3} \cdot 3!} \zeta^{6} \int_{0}^{\alpha} \cos^{6}t dt - \cdots$$
(81)

$$\int \cos^{n}t dt = \frac{1}{n} \sin t \cos^{n-1}t + \frac{n}{n-1} \int \cos^{n-2}t dt$$

$$\int_{0}^{\alpha} \cos^{2}t dt = \frac{1}{2} \sin \alpha \cos \alpha + \frac{1}{2}\alpha$$

$$\int_{0}^{\alpha} \cos^{4}t dt = \frac{1}{4} \sin \alpha \cos^{3}\alpha + \frac{3}{4} \int_{0}^{\alpha} \cos^{2}t dt$$

$$\int_{0}^{\alpha} \cos^{6}t dt = \frac{1}{6} \sin \alpha \cos^{5}\alpha + \frac{5}{6} \int_{0}^{\alpha} \cos^{4}t dt$$

$$\int_{0}^{\alpha} \cos^{2n}t dt = \frac{1}{2n} \sin \alpha \cos^{2n-1}\alpha + \frac{2n-1}{2n} \int_{0}^{\alpha} \cos^{2n-2}t dt$$

For both cases,

$$P' = 2\sqrt{a^2 \cos^2 \alpha + b^2 \sin^2 \alpha} \tag{83}$$

$$P'' = -\frac{(a^2 - b^2)\sin 2\alpha}{\sqrt{a^2 \sin^2 \alpha + b^2 \cos^2 \alpha}}$$

$$\tag{84}$$

7.3 Flow Area

The flow area is

$$A = 2 \int_{b\cos\alpha}^{b} v du = -2ab \int_{\alpha}^{0} \sin^{2}t dt = ab \int_{0}^{\alpha} (1 - \cos 2t) dt = ab(\alpha - \frac{1}{2}\sin 2\alpha)$$
 (85)

$$A' = ab(1 - \cos 2\alpha) \tag{86}$$

$$A'' = 2ab\sin 2\alpha \tag{87}$$

$$T = 2a\sin\alpha \tag{88}$$

$$y = b(1 - \cos \alpha) \tag{89}$$

$$\frac{\partial y}{\partial \alpha} = b \sin \alpha \tag{90}$$

$$\cos \alpha = 1 - y/b \tag{91}$$

7.4 Calculate Maximum Discharge Using Manning's Equation

To reach maximum capacity, the α is solved using Newton-Raphson method

$$\alpha_{i+1} = \alpha_i - \frac{f(\alpha_i)}{f'(\alpha_i)},\tag{92}$$

with Eq. 60,

$$f(\alpha) = 5A'P - 2AP' = 0, (93)$$

and

$$f'(\alpha) = 3A'P' + 5A''P - 2AP''. \tag{94}$$

7.5 Calculate Normal Depth

For Q less than Q_{max} ,

$$\alpha_{i+1} = \alpha_i - \frac{f_d(\alpha_i)}{f_d'(\alpha_i)} \tag{95}$$

where

$$f_d(\alpha_i) = \frac{K_u}{n} A^{5/3} P^{-2/3} S^{1/2} - Q \tag{96}$$

$$f_d'(\alpha_i) = \frac{K_u}{n} \left(\frac{5}{3} R^{2/3} A' - \frac{2}{3} R^{5/3} P' \right) S^{1/2}$$
(97)

7.6 Calculate Critical Depth

$$\frac{\partial A}{\partial y}(\alpha) = \frac{\frac{\partial A}{\partial \alpha}}{\frac{\partial y}{\partial \alpha}} = \frac{ab(1 - \cos 2\alpha)}{b\sin \alpha} = 2a\sin \alpha \tag{98}$$

$$\frac{\partial}{\partial \alpha} \left(\frac{\partial A}{\partial y} \right) = 2a \cos \alpha \tag{99}$$

$$f_c(\alpha) = gA^3 - Q^2 \frac{\partial A}{\partial y} = gA^3 - 2aQ^2 \sin \alpha$$
 (100)

$$f_c'(\alpha) = 3gA^2 \frac{\partial A}{\partial \alpha} - Q^2 \frac{\partial}{\partial \alpha} \left(\frac{\partial A}{\partial y} \right)$$
 (101)

8 Arc

8.1 Geometry

$$r_{b} = AB = BO = BE$$

$$r_{t} = AC = CG$$

$$r_{c} = DE = DF = DG$$

$$c = BC = BO - CO = BO - (OG - CG) = r_{b} + r_{t} - rise$$

$$(102)$$

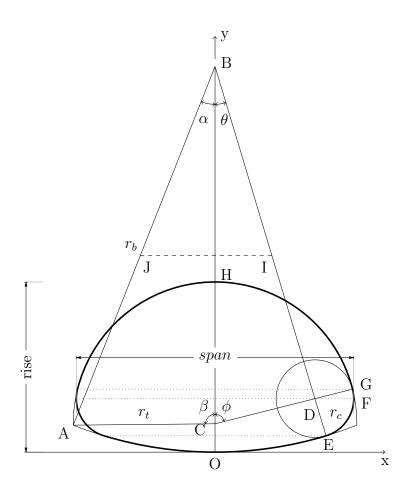


Figure 7: Arc Section

$$\cos \alpha = \frac{r_b^2 + c^2 - r_t^2}{2r_b c}$$

$$\cos \beta = \frac{r_t^2 + c^2 - r_b^2}{2r_t c}$$

$$\cos \theta = \frac{(r_b - r_c)^2 + c^2 - (r_t - r_c)^2}{2(r_b - r_c)c}$$

$$\cos \phi = \frac{(r_t - r_c)^2 + c^2 - (r_b - r_c)^2}{2(r_t - r_c)c}$$
(103)

$$x_A = r_b \sin \alpha$$

$$y_A = r_b(1 - \cos \alpha)$$

$$y_B = r_b$$

$$y_C = rise - r_t = OG - CG$$

$$x_D = (r_b - r_c) \sin \theta = (r_t - r_c) \sin \phi$$

$$y_D = r_b - (r_b - r_c) \cos \theta = y_F$$

$$x_E = r_b \sin \theta$$

$$y_E = r_b(1 - \cos \theta)$$

$$x_F = x_D + r_c$$

$$x_G = r_t \sin \phi$$

$$y_G = y_C + r_t \cos \phi = rise - r_t(1 - \cos \phi)$$

$$y_H = rise$$

$$A_E = r_b^2(\theta - \sin \theta \cos \theta)$$

$$P_E = 2r_b\theta$$

$$T_E = 2r_b \sin \theta$$

$$A_F = A_E + r_c^2(\pi/2 - \theta) + (x_E + x_D)(y_D - y_E)$$

$$P_F = P_E + 2r_c(\pi/2 - \theta)$$

$$T_F = 2(x_D + r_c)$$

$$A_G = A_F + r_c^2(\pi/2 - \phi) + (x_D + x_G)(y_G - y_D)$$

$$P_G = P_F + 2r_c(\pi/2 - \phi)$$

$$T_G = 2r_t \sin \phi$$

$$A_T = A_G + r_t^2(\phi - \sin \phi \cos \phi)$$

$$P_T = P_G + 2r_t\phi$$

$$(104)$$

For $0 \le y \le y_E$, $0 \le t \le \theta$

$$x = r_b \sin t$$

$$y = r_b (1 - \cos t)$$

$$y' = r_b \sin t = \frac{\partial y}{\partial t}$$

$$A = r_b^2 (t - \sin t \cos t)$$

$$A' = r_b^2 (1 - \cos 2t) = \frac{\partial A}{\partial t}$$

$$\frac{\partial A}{\partial y}(t) = 2r_b \sin t$$

$$\frac{\partial}{\partial t} \left(\frac{\partial A}{\partial y}\right) = 2r_b \cos t$$

$$P = 2r_b t$$

$$T = 2r_b \sin t$$
(106)

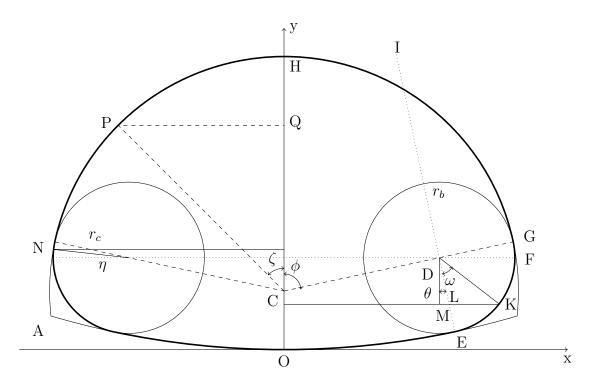


Figure 8: Arc Section

For
$$y_E \leq y \leq y_F$$
, $0 \leq w \leq \pi/2 - \theta$

$$x = x_D + r_c \sin(\omega + \theta)$$

$$x_L = x_D + r_c \cos(\omega + \theta) \tan \theta$$

$$x'_L = -r_c \sin(\omega + \theta) \tan \theta$$

$$x''_L = -r_c \cos(\omega + \theta) \tan \theta$$

$$x''_L = -r_c \cos(\omega + \theta) \tan \theta$$

$$y = y_D - r_c \cos(\omega + \theta)$$

$$y'' = r_c \cos(\omega + \theta)$$

$$A = A_E + r_c^2 \omega - r_c^2 \sin(\omega + \theta) \cos(\omega + \theta) + r_c^2 \cos^2(\omega + \theta) \tan \theta + (x_E + x_L)(y - y_E)$$

$$A' = r_c^2 - r_c^2 \cos[2(\omega + \theta)] - r_c^2 \sin[2(\omega + \theta)] \tan \theta + (x_E + x_L)y' + x'_L(y - y_E)$$

$$A'' = 2r_c^2 \sin[2(\omega + \theta)] - 2r_c^2 \cos[2(\omega + \theta)] \tan \theta + (x_E + x_L)y'' + x'_L(y - y_E) + x'_Ly'$$

$$\frac{\partial A}{\partial y}(t) = \frac{A'}{y'}$$

$$\frac{\partial A}{\partial y}(t) = \frac{A''}{y'}$$

$$P = P_E + 2r_c \omega$$

$$T = 2x_D + 2r_c \sin(\omega + \theta)$$
(107)

For
$$y_F \leq y \leq y_G$$
, $0 \leq \eta \leq \pi/2 - \phi$

$$x = x_D + r_c \cos \eta$$

$$y = y_D + r_c \sin \eta$$

$$A = A_F + r_c^2 \eta + (2x_D + r_c \cos \eta) r_c \sin \eta$$

$$P = P_F + 2r_c \eta$$

$$T = 2x_D + 2r_c \cos \eta$$

$$y' = r_c \cos \eta$$

$$y'' = -r_c \sin \eta$$

$$A' = r_c^2 + 2x_D r_c \cos \eta + r_c^2 \cos(2\eta)$$

$$A'' = -2x_D r_c \sin \eta - 2r_c^2 \sin(2\eta)$$

$$\frac{\partial A}{\partial y}(t) = \frac{A'}{y'}$$

$$\frac{\partial}{\partial t} \left(\frac{\partial A}{\partial y}\right) = \frac{A''y' - A'y''}{y'^2}$$

For $y_G \leq y \leq y_H$, $0 \leq \zeta \leq \phi$

$$y = y_C + r_t \cos \zeta$$

$$A = A_T - r_t^2(\zeta - \sin \zeta \cos \zeta)$$

$$P = P_T - 2r_t \zeta = P_G + 2r_t(\phi - \zeta)$$

$$T = 2r_t \sin \zeta$$

$$y' = -r_t \sin \zeta$$

$$y'' = -r_t \cos \zeta$$

$$A' = -r_t^2 [1 - \cos(2\zeta)]$$

$$A'' = -2r_t^2 \sin(2\zeta)$$

$$P'(\zeta) = -2r_t$$

$$P''(\zeta) = 0$$

$$\frac{\partial A}{\partial y}(t) = \frac{A'}{y'} = 2r_t \sin \zeta$$

$$\frac{\partial}{\partial t} \left(\frac{\partial A}{\partial y}\right) = 2r_t \cos \zeta$$
(109)

8.2 Calculate Maximum Discharge Using Manning's Equation

Maximum discharge occurs close to the top of arch, the ζ is solved using Newton-Raphson method

$$\zeta_{i+1} = \zeta_i - \frac{f(\zeta_i)}{f'(\zeta_i)},\tag{110}$$

with Eq. 60,

$$f(\zeta) = 5A'P - 2AP',\tag{111}$$

and

$$f'(\zeta) = 3A'P' + 5A''P - 2AP''. \tag{112}$$

8.3 Calculate Normal Depth

For Q less than Q_{max} ,

$$\alpha_{i+1} = \alpha_i - \frac{f_d(\alpha_i)}{f_d'(\alpha_i)} \tag{113}$$

where

$$f_d(\alpha_i) = \frac{K_u}{n} A^{5/3} P^{-2/3} S^{1/2} - Q \tag{114}$$

$$f_d'(\alpha_i) = \frac{K_u}{n} \left(\frac{5}{3} R^{2/3} A' - \frac{2}{3} R^{5/3} P' \right) S^{1/2}$$
 (115)

8.4 Critical Flow

Critical flow occurs when the specific energy of the cross-section

$$E = \frac{v^2}{2g} + y = \frac{Q^2}{2gA^2} + y \tag{116}$$

reaches a minimum. Namely,

$$\frac{\partial E}{\partial y} = -\frac{Q^2}{qA^3} \frac{\partial A}{\partial y} + 1 = 0 \tag{117}$$

Newton-Raphson method is used to calculate critical depth as

$$y_{c,i+1} = y_{c,i} - \frac{f(y_{c,i})}{f'(y_{c,i})}$$
(118)

where

$$f_c(y) = gA^3 - Q^2 \frac{\partial A}{\partial y} \tag{119}$$

$$f'_c(y) = 3gA^2 \frac{\partial A}{\partial y} - Q^2 \frac{\partial^2 A}{\partial y^2}$$
 (120)

for A and y are parameterized as a function of t with $A=A(t), A'=\frac{\partial A(t)}{\partial t}, y=y(t),$ $y'=\frac{\partial y(t)}{\partial t},$

$$t_{c,i+1} = t_{c,i} - \frac{f(t_{c,i})}{f'(t_{c,i})}$$
(121)

$$f_c(t) = gA^3(t) - Q^2 \frac{\partial A}{\partial y}(t)$$
(122)

$$f'_{c}(t) = 3gA^{2} \frac{\partial A}{\partial t} - Q^{2} \frac{\partial}{\partial t} \left(\frac{\partial A}{\partial y} \right)$$
 (123)

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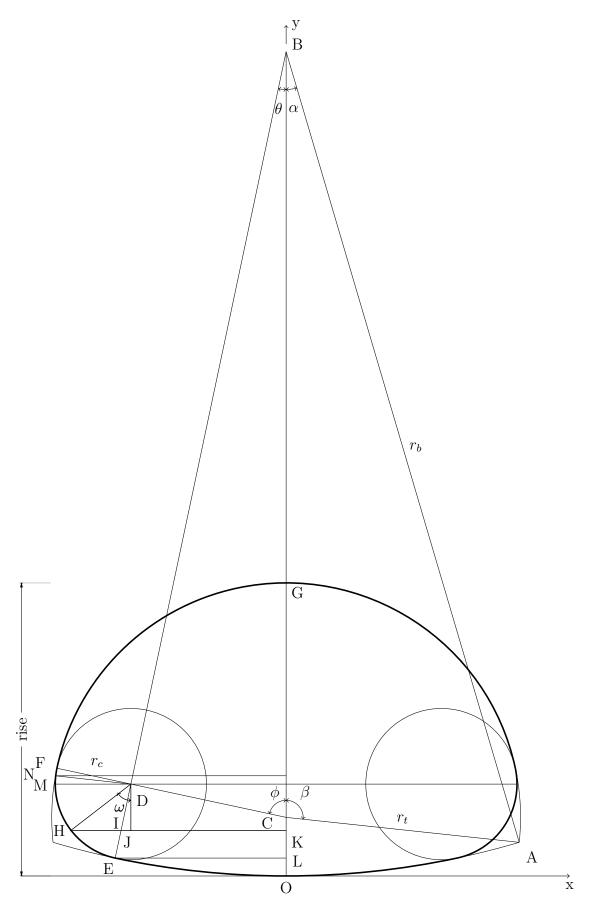


Figure 9: Arc Section 19