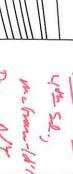
Water Q.



1 = R;/R

COLLISION EFFICIENCY 0.4

0.6

0.2

I=0.5-

I=0.25

■I=0.05

10/2

10-

00

194/10

(1990)

Figure 6.21 Collision efficiencies in brownian coagulation. Variations for Hamaker's constant A, particle radii  $R_i/R_j$ , and ionic

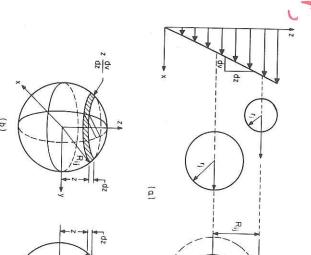
#### Fluid shear

strength I.

In the second transport process, collisions between particles occur be-

thickness dz (or area dA) streamlines and consider the rate of fluid flow dq into the disc of as the velocity gradient is uniform. Assume that the particles follow in Fig. 6.22. The velocity of the fluid at a height z is given by z(dv/dz), the flow of liquid into a sphere of influence of radius  $R_{ij} = R_i + R_j$  as diffusion, consider a j particle at the center of a coordinate system and cause of bulk motion of the fluid in laminar shear. As for brownian

$$dq = z \left(\frac{dv}{dz}\right) dA = z \left(\frac{dv}{dz}\right) 2(R_{ij}^2 - z^2)^{1/2} dz$$
 (6.31)

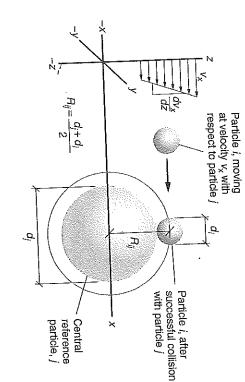


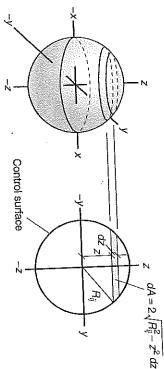
(a) velocity gradient and collision sphere;(b) flow-(c) areal projection in direction of flow. Figure 6.22 Schematic for Smoluchowski's theory of (b)

and  $d_j$  and mean velocity gradient dv/dzEquation (6.33) is also expressed in terms  $=\frac{4}{3}n_in_j\left(R_i+R_j\right)$ 

 $(N_{\perp}) = \frac{1}{2} n n (d + 1)$ 

#### 9 Coagulation and Flocculation





Definition sketch for analysis of the flocculation process. Figure 9-18

defined, as shown on Fig. 9-18. The velocity gradient on Fig. 9-18 is When fluid flow is laminar and steady, the velocity gradients are well proportional to the shear stress on the fluid elements because it is a can be determined from geometric considerations, as illustrated below. Newtonian fluid. Given a uniform velocity gradient, the rate of flocculation

Rate of flocculation of heterodisperse particles

perse) particles subjected to uniform mixing may be derived using the relationships shown on Fig. 9-18. The flow rate of fluid into an area element The rate of macroscale flocculation in a system of unequal size (heterodisdA of the control surface is given by the following expression (Swift and Friedlander, 1964; Smoluchowski, 1917):

$$dq = \text{(velocity) (differential area)} = \left(z \frac{dv_x}{dz}\right) \left(2\sqrt{R_{ij}^2 - z^2} dz\right) \tag{9-30}$$

#### 9 Coagulation and Flocculation

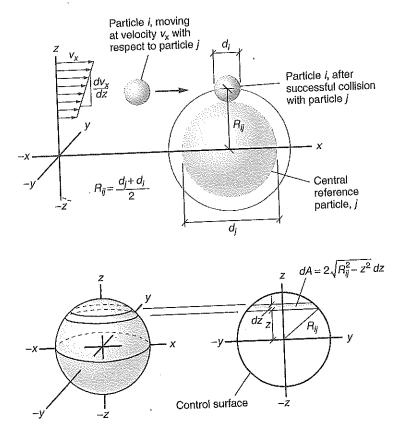


Figure 9-18 Definition sketch for analysis of the flocculation process.

Velocity gradients

When fluid flow is laminar and steady, the velocity gradients are well defined, as shown on Fig. 9-18. The velocity gradient on Fig. 9-18 is proportional to the shear stress on the fluid elements because it is a Newtonian fluid. Given a uniform velocity gradient, the rate of flocculation can be determined from geometric considerations, as illustrated below.

Rate of flocculation of heterodisperse particles

The rate of macroscale flocculation in a system of unequal size (heterodisperse) particles subjected to uniform mixing may be derived using the relationships shown on Fig. 9-18. The flow rate of fluid into an area element dA of the control surface is given by the following expression (Swift and Friedlander, 1964; Smoluchowski, 1917):

$$dq = \text{(velocity)} \text{ (differential area)} = \left(z\frac{dv_x}{dz}\right)\left(2\sqrt{R_{ij}^2 - z^2} dz\right)$$
 (9-30)

Substituting Eq. 9-38 into Eq. 9-36 results in the following equation for the rate of flocculation of a monodisperse system:

$$r_M = \frac{4\Omega \overline{G}\alpha n_i}{\pi} \tag{9-39}$$

Thus, the macroscale flocculation rate for a monodispersed suspension is a first-order rate expression with respect to  $n_i$ , and the rate constant is directly proportional to the velocity gradient and the floc volume fraction.

Rate correction for turbulent conditions

In practice, fluid flow in most flocculation units is turbulent. Under turbulent conditions, the velocity gradient is not well defined and will vary locally in the flocculation unit. As discussed in Sec. 6-10, Camp and Stein (1943) developed an expression relating  $\overline{G}$  to mixing power (see Eq. 6-140) by equating the velocity gradients to the power dissipated per unit volume (P/V) for uniform shear flow:

$$\overline{G} = \sqrt{\frac{P}{\mu V}} \tag{9-40}$$

where  $\overline{G} = \text{RMS}$  velocity gradient, s<sup>-1</sup>

 $P = \text{power of mixing input to flocculation basin, W (note 1 W = 1 \text{ kg·m}^2/\text{s}^3)}$ 

 $V = \text{volume of flocculation basin, m}^3$ 

 $\mu = \text{dynamic viscosity of water, kg/m·s}$ 

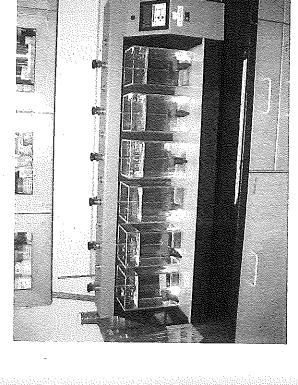
In turbulent flow, the rate of aggregation for particles smaller than the Kolmogorov eddy size (see Eq. 6-136) is approximately the same as it is for laminar flow because flow within eddies is laminar. Using this argument, the flocculation rate for turbulent shear should be similar to the rate-for laminar flow. While this may be true in theory, it has been found that the rate is much higher due to interactions between eddies (Logan, 1999). Consequently, the rate becomes proportional to  $\overline{G}$  (Harris et al., 1966), and an empirical rate constant must be employed, shown below for the monodisperse system:

$$r_{M,T} = K_A \overline{G} \Omega n_i \tag{9-41}$$

where  $r_{M,T}$  = rate of flocculation for turbulent flow  $K_A$  = empirical aggregation constant

The aggregation constant depends on system chemistry, the heterogeneity of the suspension, and variations in the scale and intensity of turbulence, which are not incorporated in the velocity gradient. Because of the different flow patterns and distributions of velocity gradients promoted by various mixing devices,  $K_A$  must be determined experimentally.

Ac'= concentration him



o form nucleating sites for the formation of

are often added to waters that contain low it), and fine sand have all been used as coagrre used in conjunction with inorganic coagu-, or both. Because the density of these particles c particles, the settling velocities of flocculated

formation of vortex flow in which the particles rotate in the same position Jar test apparatus. Note use of square containers to limit the Figure 9-12

relative to each other.

ling rate. It should be noted that improper n cause mudballs to form in gravity filters that ar tests have to be conducted. The important nated in jars and full-scale implementation are s formed. Flocculant aids are added after the re particles are already destabilized. The time depends on water temperature and the type cussed, the main advantage of using flocculant harged organic polymers that were discussed used as flocculant aids as opposed to primary

nd the SiO<sub>2</sub> flocculant aid. A typical dosage is soluble at high pH. Usually, the concentrated neutralized (usually with sulfuric acid) prior nmediately to the water. In some instances, ce alum is the primary coagulatant because the used to activate the silica, and this will produce ortant inorganic flocculant aid that is used in can be effective in cold water. It is usually stored backwashing.

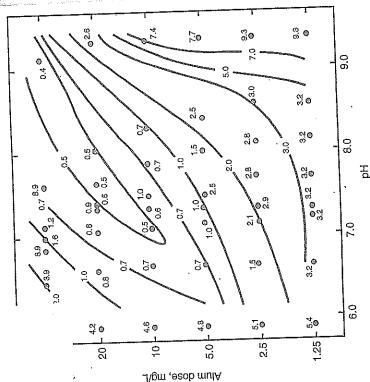
ed over the years (see Black et al., 1957; Tekippe permits rapid evaluation of a range of coagulant ar test apparatus is shown on Fig. 9-12. As shown s consists of six batch reactors, each equipped shaped jars are used to avoid vortex flow, which ocess, the selection of coagulants and dosage is testing procedure for determining coagulant ar test" procedure. Developed originally by ically using bench-scale and pilot-scale studies. eting reactions and mechanisms that are operrs are used

mix period to simulate flocculation. Flocs are allowed to settle and samples jar testing procedures are available in Kawamura (2000), ASTM (2008), and with simple batch addition of the coagulant or coagulants followed by a slow-(2) NOM removal as measured by dissolved organic carbon (DOC) or a dissolved coagulant concentrations of Fe or Al coagulants; and (4) sludge volume that is produced. If direct filtration is to be used, the filterability AWWA (2011). Generally, the test consists of a rapid-mix phase (blending) are taken from the supernatant. These parameters should be measured as part of the jar test routine: (1) turbidity or suspended solids removal; should be evaluated using a filterability test. The filterability is evaluated by filtering the mixed suspension through a 5- or 8-µm laboratory filter to surrogate measure of dissolved NOM, such as  $\overline{\mathrm{UV}}$  at 254 nm; (3) residual simulate a granular medium filter.

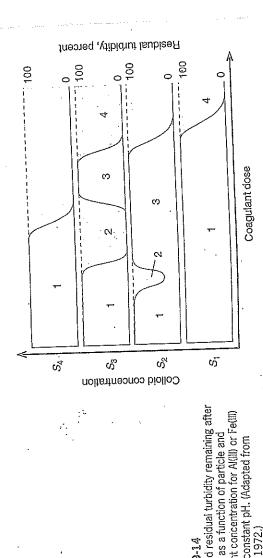
and pH for turbidity removal for given water are summarized on Fig. 9-13. As shown on Fig. 9-13, the optimum alum dose and pH would be approximately The results of a series of jar tests to determine the optimum alum dose 8 mg/L and 7, respectively, because the turbidity is minimized under these conditions. However, it must be emphasized that the raw-water particle concentration and NOM vary with water quality, and thus the optimum coagulant dosage also changes as the water quality changes.

# ANALYSIS OF COAGULATION PROCESS USING JAR TEST PROCEDURE

A conceptual diagram in which the residual turbidity from jar tests conducted for waters with different particle concentrations is illustrated on



losage. (Adapted from 978.) Points shown on the pogram as function of pH hs represent constant the value denoted on the sent turbidity values and



9-4 Coagulation Practice

is higher, then no cationic metal ion species would be formed for charge neutralization. Furthermore, the impact of NOM on the coagulant dose has not been considered.

#### Water with low concentration of particles

For the lowest particle concentration, St, particles are removed by sweep flocculation (precipitation) because the concentration is too low for them to flocculate and settle. These particles are also destabilized by the hydrolysis products of the coagulant. The particle concentration S<sub>1</sub> may be a good candidate for direct filtration or dissolved air flotation for liquid-solid separation.

## Water with moderate concentration of particles

At a slightly higher particle concentration, S2, some flocculation and coagulant is added, the particles stabilize with a positive charge and the turbidity increases, as shown in zone 3. However, as the coagulant dosage is increased even further, sweep flocculation is again observed but at a lower coagulant concentration than for a particle concentration S1 because the settling occurs in zone 2, in, which adsorption and charge neutralization occur. However, the concentration is too low for effective flocculation and a high degree of turbidity removal is not possible. Further, if more particle concentration is higher, which allows for more effective flocculation and settling at a lower coagulant dosage.

### Water with high concentration of particles

Nearly complete removal by charge neutralization occurs in zone 2 and the particles stabilize at higher coagulant dosages. Sweep flocculation occurs in zone 4, and it occurs at lower coagulant dosages than for particle concentration S<sub>2</sub> or S<sub>1</sub>. Sweep flocculation occurs because a substantial portion of the floc volume comes from the particles, and this allows for At a higher particle concentration, S3, all four zones can clearly be found. effective flocculation at lower coagulant dosages.

# Water with very high concentration of particles

to neutralize the charges on the particles coincides with the onset of Q precipitation. Another noteworthy effect is that zone 2 starts at successively neutralization regions merge because the coagulant concentration required For the highest particle concentration, St, the sweep floc and charge higher coagulant dosages as the particle concentration increases. As result, the charge neutralization region is said to exhibit stoichiometry.

#### Analysis of results

it concentration for Al(III) or Fe(III) constant pH. (Adapted from

as a function of particle and