

One application of the MGIS project to a variational problem modelling a damaged elasto-plastic material

Fifth MFront User Meeting

Goustan BACQUAERT

École des Ponts ParisTech/Sorbonne Université

EDF Lab Paris Saclay

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Outline

- 1 Framework and objectives
- 2 MFront integration of the local model
- 3 MGIS usefulness for developing a regularized model
- 4 Perspectives

1 Framework and objectives

- Experimental observations on granular materials
- Work basing support

2 MFront integration of the local model

- From the local formulation to the MFront vocabulary
- A need to regularize the local model

3 MGIS usefulness for developing a regularized model

- A regularisation by the damage gradient
- The Algorithmic strategy
- Numerical applications

4 Perspectives

At the macroscopic scale

A key-vocabulary to describe the experimental observations:

- Hardening and softening behaviours.
 - Contractancy and dilatancy behaviours.
 - Critical state.
- + Typical evolutions with cyclic loads, hydro-mechanical coupling, etc.

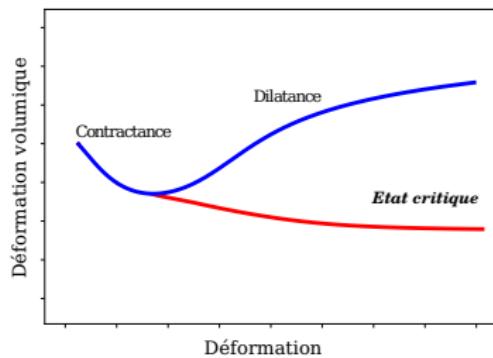
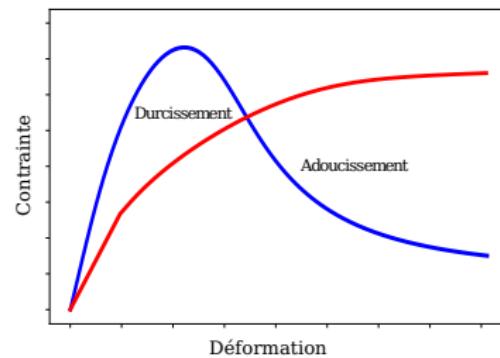


FIG.: Two possible stress and volumetric strain responses for a dense or loose sand

At the microscopic scale

Use of digital image correlations to visualize the mechanical transformations at the grain level ($\sim 10^{-4} - 10^{-2}$ mm):

- Heterogeneous strain field.
- Shear bands growth.
- High levels of porosity.

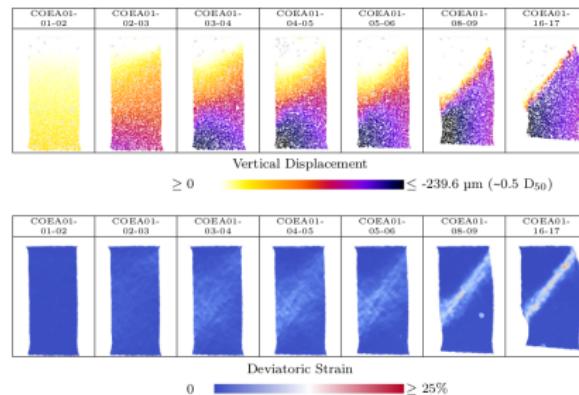


FIG.: Visualization of displacement and strain localisations from a simple compression test¹

1. Desrues, J. and Andò, E., Strain localisation in granular media, *Comptes Rendus Physique*, 2015

Framework of the model development

Context: 6 months of Master internship, at EDF Lab², with SU and ENPC³ supports.

Initial feed: a generalised standard material (GSM) model coupling plasticity with damage⁴.

Summarized ideas

Sound elasticity with **damaged** kinematic hardening:

$$\psi(\boldsymbol{\varepsilon}, \boldsymbol{\varepsilon}^p, \alpha) = \psi_e(\boldsymbol{\varepsilon} - \boldsymbol{\varepsilon}^p) + \psi_h(\boldsymbol{\varepsilon}^p, \alpha)$$

$$\psi_e(\boldsymbol{\varepsilon} - \boldsymbol{\varepsilon}^p) = \frac{1}{2} (\boldsymbol{\varepsilon} - \boldsymbol{\varepsilon}^p) : \mathbb{C} : (\boldsymbol{\varepsilon} - \boldsymbol{\varepsilon}^p) \quad ; \quad \psi_h(\boldsymbol{\varepsilon}^p, \alpha) = \frac{1}{2} \boldsymbol{\varepsilon}^p : \mathbb{H}(\alpha) : \boldsymbol{\varepsilon}^p$$

Linear superposition of Drucker-Prager and Griffith dissipations:

$$\varphi(\dot{\boldsymbol{\varepsilon}}^p, \dot{\alpha}) = \frac{\sigma_0}{k} \dot{\varepsilon}_v^p + D_1 \dot{\alpha}$$

2. Alves Fernandes, V., Raude, S. and Voldoire, F., Département ERMES

3. Kondo, D. and Maurini, C., Laboratoire D'Alembert, Bleyer, J., Laboratoire Navier

4. Marigo, J.-J. and Kazymyrenko, K., A micromechanical inspired model for the coupled to damage elasto-plastic behavior of geomaterials under compression, Mechanics & Industry, 2019

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Pseudo-code

Behaviour	Thermodynamic force	Criterium
Plasticity	$\mathbf{X} = \boldsymbol{\sigma} - \mathbb{H}(\alpha) : \boldsymbol{\varepsilon}^P$	$f_p(\mathbf{X}) = X_{eq} + kX_m - \sigma_0 \leq 0$
Damage	$Y = -\frac{1}{2}\boldsymbol{\varepsilon}^P : \mathbb{H}'(\alpha) : \boldsymbol{\varepsilon}^P$	$f_d(Y) = Y - D_1 \leq 0$

MFront outline

- ① Elastic prediction ($\Delta\boldsymbol{\varepsilon}^P = 0, \Delta\alpha = 0$):

$$f_p(\boldsymbol{\varepsilon} + \Delta\boldsymbol{\varepsilon}, \boldsymbol{\varepsilon}^P, \alpha) \leq 0$$

- ② If $f_p > 0$ plastic correction ($\Delta\boldsymbol{\varepsilon}^P \neq 0, \Delta\alpha = 0$):

$$f_p(\boldsymbol{\varepsilon} + \Delta\boldsymbol{\varepsilon}, \boldsymbol{\varepsilon}^P + \Delta\boldsymbol{\varepsilon}^P, \alpha) = 0 ; f_d(\boldsymbol{\varepsilon}^P + \Delta\boldsymbol{\varepsilon}^P, \alpha) \leq 0$$

- ③ If $f_d > 0$ damage correction ($\Delta\boldsymbol{\varepsilon}^P \neq 0, \Delta\alpha > 0$):

$$f_p(\boldsymbol{\varepsilon} + \Delta\boldsymbol{\varepsilon}, \boldsymbol{\varepsilon}^P + \Delta\boldsymbol{\varepsilon}^P, \alpha + \Delta\alpha) = 0 ; f_d(\boldsymbol{\varepsilon}^P + \Delta\boldsymbol{\varepsilon}^P, \alpha + \Delta\alpha) = 0$$

Numerical responses vs available data

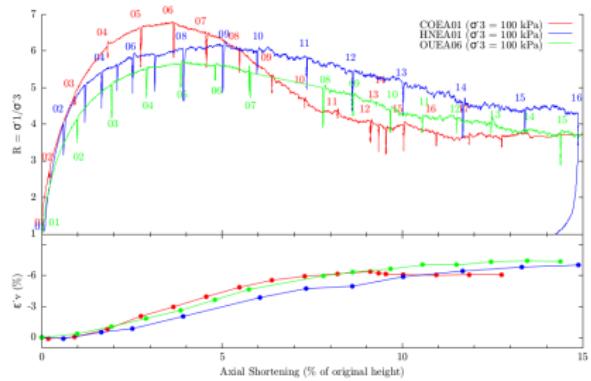


FIG.: Simple compressive test response on dense sands¹

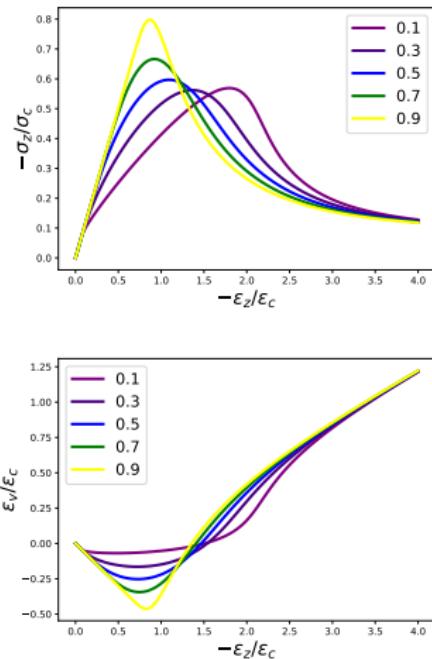


FIG.: Some parametric studies (mtest)

Numerical pathologies from the softening behaviour

Damage responsible for a softening behaviour, incompatible with a well-posed model:

- Field localisation on one element in finite element simulations.
- Mesh dependency (band size and orientation, level of dissipation).
- Mistrust of the numerical results.

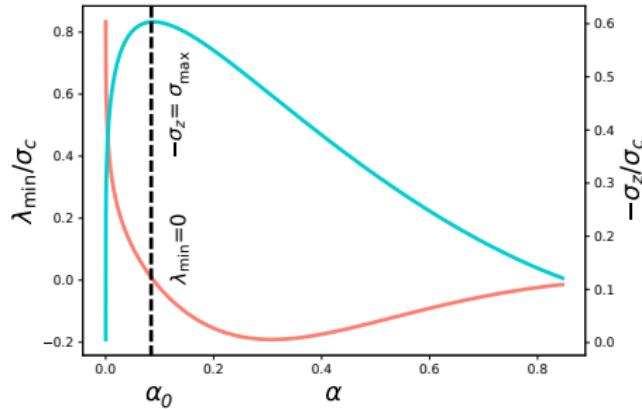
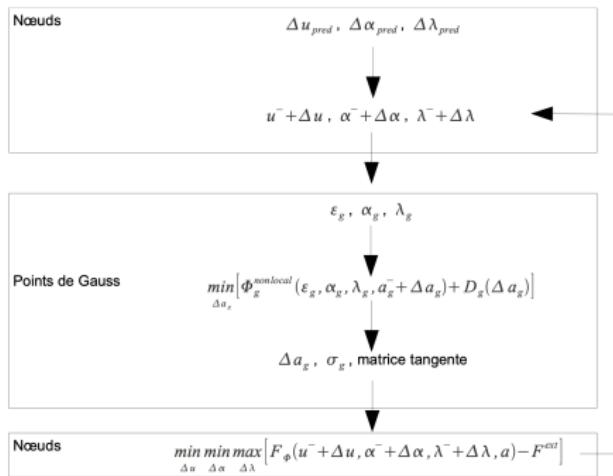


FIG.: Hill unicity criterium at the integration point (λ_{\min} minimal eigenvalue of C_{tg})

Regularisation at EDF

Available technique to regularize purely damage (without plasticity) model in Code_Aster by constructing a non-local one by adding the internal variable gradient. Concisely⁵:

- The internal variable a is defined at the integration points.
- It must be duplicate by α defined at the nodes to have access to its gradient.
- Lagrange multipliers λ to constrain $\alpha = a$ are thus introduced.



FEniCS: internal variable directly defined at nodes, no need of duplication. Strategy privileged in the internship to study one proposed regularized model.

5. Kazymyrenko, K., Modélisation non locale à gradients de variables internes GRAD.VARI, 2011

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New variational formulation

Global potential defined on the structure Ω , with the introduction of the damage gradient correlated to an intrinsic length l .

$$\Psi(\varepsilon, \varepsilon^p, \alpha) = \int_{\Omega} \psi(\varepsilon(\underline{x}), \varepsilon^p(\underline{x}), \alpha(\underline{x})) d\Omega$$

$$\Phi(\dot{\varepsilon}^p, \dot{\alpha}) = \int_{\Omega} \varphi(\dot{\varepsilon}^p(\underline{x}), \dot{\alpha}(\underline{x})) d\Omega + D_1 l^2 \int_{\Omega} \nabla \alpha(\underline{x}) \cdot \nabla \dot{\alpha}(\underline{x}) d\Omega$$

$$\mathcal{E} = \Psi(\varepsilon, \varepsilon^p, \alpha) + \int_0^t \Phi(\dot{\varepsilon}^p, \dot{\alpha}) dt - \mathcal{W}_{\text{ext,d}}$$

Regularisation inspired by phase-field models applied to brittle fracture problem. Similar applications to another damage/plasticity coupling can be found⁶. Minimising \mathcal{E} according to the three involved fields (in some appropriate spaces) enable to compute a mechanical consistent solution (respecting equilibrium equation, plastic and damage criteria).

⁶. Alessi, R. Marigo J.-J. and Vidoli, S., Gradient damage models coupled with plasticity: Variational formulation and main properties, *Mechanics of Materials*, 2015

An efficient interaction between MFront and FEniCS

The mechanical problem is divided into two sub-problems, handled by **MFront** and **FEniCS** environments, connected thanks to the MGIS project.

- ① Resolution of the local equations with damage α fixed under **MFront**, on the fields of displacement \underline{u} and plastic strain ε^P . The integrator handles only a **linear kinematic hardening behaviour**.
- ② Damage minimization under **FEniCS**. Optimization under constraints ($\dot{\alpha} > 0$) of a **convex functional** with a TA0 solver:

$$\mathcal{E}_{\text{obj}}(\alpha) = \int_{\Omega} \left(\frac{1}{2} \varepsilon^P(\underline{x}) : \mathbb{H}(\alpha(\underline{x})) : \varepsilon^P(\underline{x}) + D_1 \left(\alpha(\underline{x}) + \frac{l^2}{2} \|\nabla \alpha(\underline{x})\|^2 \right) \right) d\Omega$$

Remarks:

- Well-posed **MFront/FEniCS** sub-problems to compute a mesh-independent solution verifying the mechanical equations.
- Which sub-problem is the most time-consuming? From numerical experiments, the α -minimization one.

Pseudo-code

Resolution at the load step i

- ★ Initialisation with the previous solution at $i - 1$:

$$\underline{u}^{(i,0)} = \underline{u}^{(i-1)} ; \quad \varepsilon^{\boldsymbol{p},(i,0)} = \varepsilon^{\boldsymbol{p},(i-1)} ; \quad \alpha^{(i,0)} = \alpha^{(i-1)}$$

- ➊ MFront sub-problem on $\underline{u}^{(i,k+1)}$ and $\varepsilon^{\boldsymbol{p},(i,k+1)}$:

$$f_{\boldsymbol{p}} \left(\varepsilon \left(\underline{u}^{(i,k+1)} \right), \varepsilon^{\boldsymbol{p},(i,k+1)}, \alpha^{(i,k)} \right) \leq 0$$

- ➋ FEniCS sub-problem on $\alpha^{(i,k+1)}$:

$$\alpha^{(i,k+1)} = \underset{\alpha^{(i-1)} \leq \alpha \leq 1}{\arg \min} \mathcal{E}_{\text{obj}}^{(i,k+1)}(\alpha)$$

- ★ Iteration on k until convergence (on $\|\alpha^{(i,k+1)} - \alpha^{(i,k)}\|_\infty$ for instance):

$$\underline{u}^{(i)} = \underline{u}^{(i,k+1)} ; \quad \varepsilon^{\boldsymbol{p},(i)} = \varepsilon^{\boldsymbol{p},(i,k+1)} ; \quad \alpha^{(i)} = \alpha^{(i,k+1)}$$

Translation in python

Shortly in a notebook (Remark: $d = \alpha$):

```
# Material definition (properties, behaviour, hypothesis)
material = mf.MFrontNonlinearMaterial("../materials/src/libBehaviour.dylib",
                                         "MK_law",
                                         hypothesis="axisymmetric",
                                         material_properties=mat_prop)

# Definition of the 1 sub-problem (at the damage fixed)
problem_1 = mf.MFrontNonlinearProblem(u, material, quadrature_degree=2, bcs=bcu)
problem_1.register_external_state_variable("Damage", d)

# Definition of the 2 sub-problem (relative to the damage)
problem_2 = PETScTAOSolver()

# At each load step, the alternated minimisation is called
def alternate_minimization(u, d, tol=1e-5, Nitermax=500):

    for niter in range(Nitermax):

        problem_1.solve(u.vector())
        problem_2.solve(DamageProblem(), d.vector(), dlb.vector(), dub.vector())

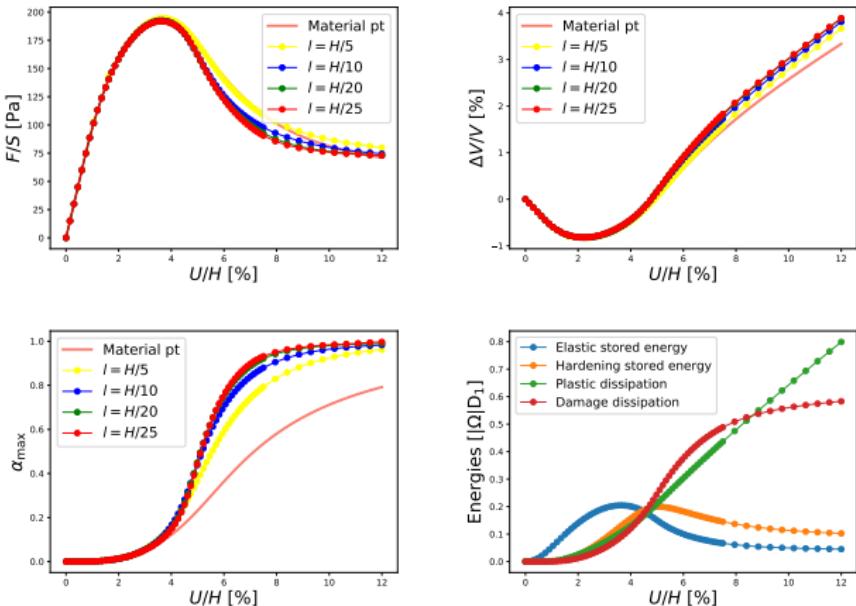
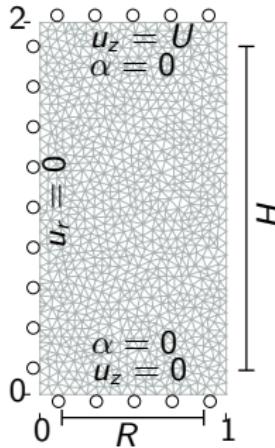
        Res = max(d.vector().get_local()-dold.vector().get_local())
        dold.assign(d)

        if nRes < tol:
            break

    dlb.assign(d)
```

Macroscopic responses on axisymmetric tests

material = mf.MFrontNonlinearMaterial(...,hypothesis='axisymmetric')



- Displacement: \mathcal{P}_2 .
- Damage: \mathcal{P}_1 .

FIG.: Macroscopic responses according to I

Plastic and damage fields

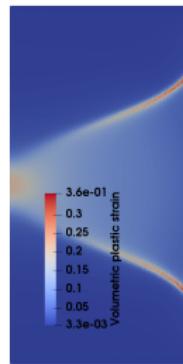
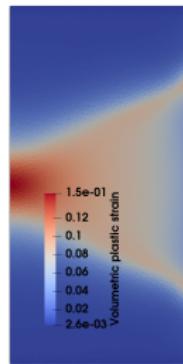
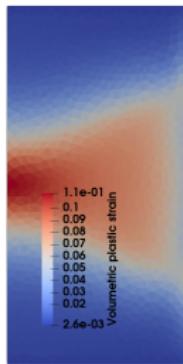
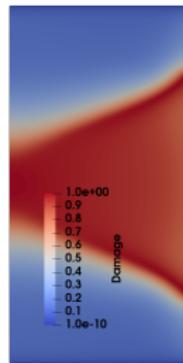
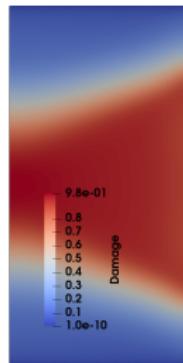
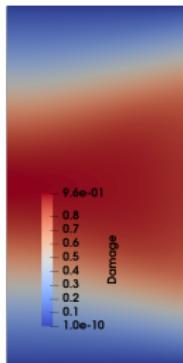
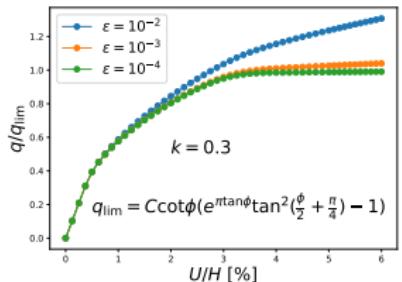
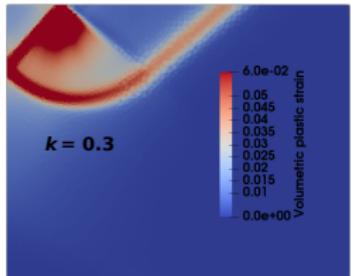
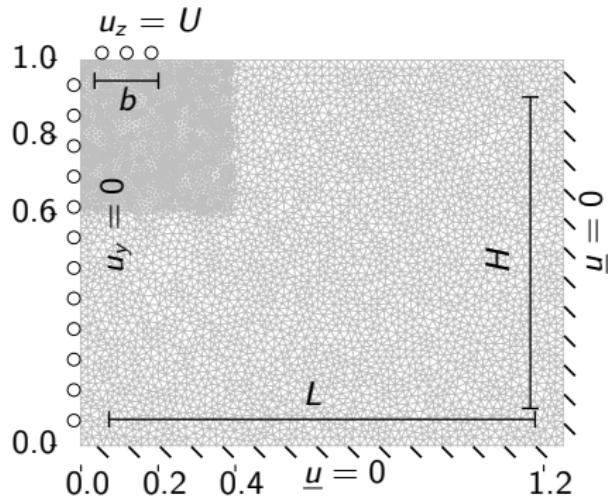


FIG.: Final damage and plastic fields ($U/H = 12\%$) for $I = H/5, H/10$ et $H/25$

Toward more interesting simulations

`material = mf.MFrontNonlinearMaterial(...,hypothesis='plane_strain')`



$$\text{Limit analysis: } q_{\lim} = C \cot \phi (e^{\pi \tan \phi} \tan^2 (\frac{\phi}{2} + \frac{\pi}{4}) - 1)$$

$$\text{Residual hardening: } \psi_h(\boldsymbol{\varepsilon}^p) = \frac{1}{2} E_0 \|\boldsymbol{\varepsilon}^p\|^2$$

Numerical observations

$\mathbb{H}_\epsilon \leftrightarrow \mathbb{H}(\alpha)$. What difference can be expected?

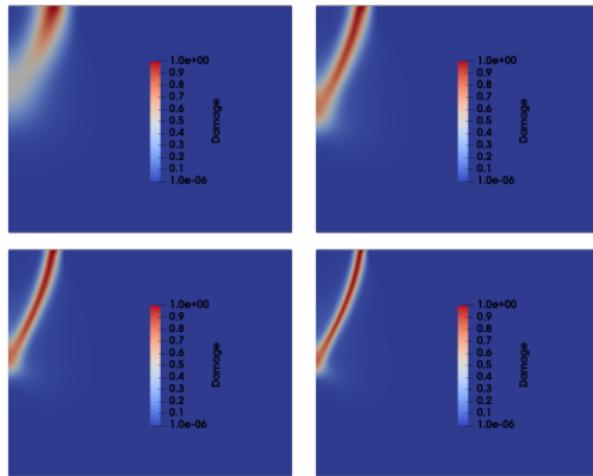
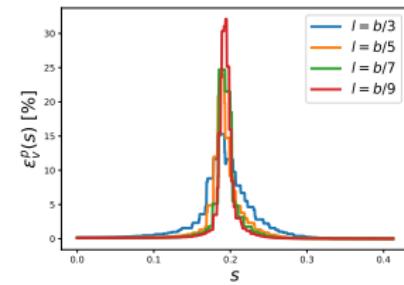
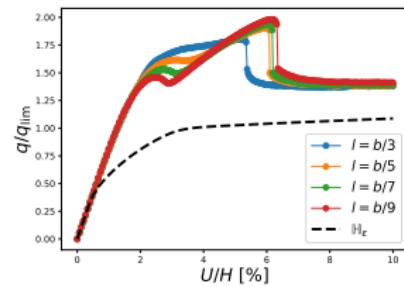


FIG.: Damage field at $U/H = 4\%$ according to l



Numerical observations

Results raising interesting comments:

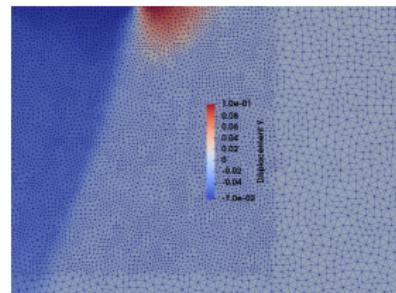
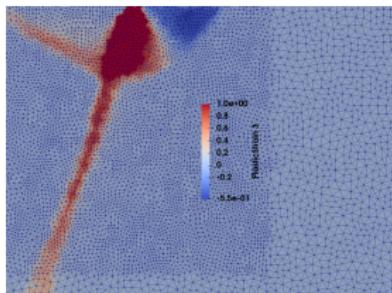


FIG.: ε_{yz}^p and u_z fields at the maximal load ($U/H = 10\%$)

- The length / localizes the plastic strains before a breakdown appears, then a Prantdl mechanism is approached.
- The Young modulus E_0 influences the asymptotic charge. Different from limit analysis.

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Coming work

Enrichment of the local model to take into account cyclic and undrained behaviours (with fluid interaction). Not an easy task since:

- ① Proposing a cyclic model respecting GSM principles relevant to soils is not an easy task.
- ② Adding fluid interaction entails issues of stability which should be carefully considered.

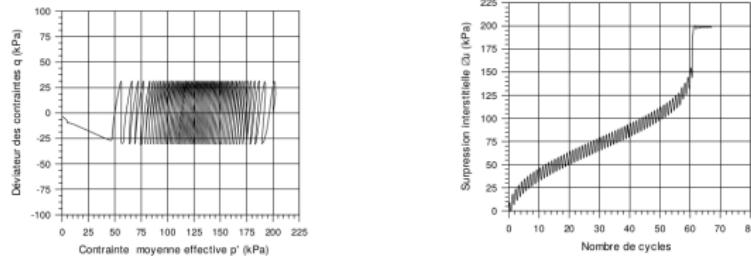


FIG.: Fluid pressure and stress responses from an undrained simple compression of a loose sand⁷

Transposition of the finally adopted model to Code_Aster. Ultimate goal aimed at the end of the Cifre thesis collaborating EDF, ENPC and SU (2020-2023).

7. Canou, J., Instabilités de liquéfaction et phénomène de mobilité cyclique dans les sables, *Rev. Fr. Geotech.*, 2002