STRUCTURAL GENERAL NOTES:

BUILDING CODES:

GOVERNING BUILDING CODE – FLORIDA BUILDING CODE – 2004 W/ 2007 REVISIONS
ACI 318–02
ASCE 7–02

DESIGN LOADS:

LIVE LOADS

ROOF	30 PSF
ASSEMBLY	100 PSF
STAIRS/BALCONY/WALKWAY	100 PSF
OFFICE/LABORATORY	50 PSF
STORAGE	75 PSF
CLASSROOM	40 PSF
MECHANICAL RM.	100 PSF
COMPUTER RM.	50 PSF
TYP. SUPERIMPOSED DL	15 PSF
PARTITION DL	20 PSF

WIND LOADS
ASCE 7–02
V=146 MPH, I=1.15, EXPOSURE C

GENERAL:

THE GENERAL CONTRACTOR SHALL CHECK, REVIEW AND VERIFY ALL PLANS, DIMENSIONS AND SITE CONDITIONS WITH THE ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION. THE ENGINEER OF RECORD SHALL BE NOTIFIED IN WRITING OF ANY DISCREPANCY OR OMISSIONS NOTED ON THE DRAWINGS OR, IN THE SPECIFICATIONS. NO DIMENSIONS SHALL BE SCALED FROM THESE DRAWINGS. THE ENGINEER WILL NOT BE RESPONSIBLE FOR ERRORS CAUSED BY USING SCALED DIMENSION.

THE STRUCTURAL DRAWINGS SHALL BE WORKED TOGETHER WITH ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND CIVIL DRAWINGS TO LOCATE DEPRESSED SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS REGLETS, BOLT SETTINGS, SLEEVES, ETC. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE WORK.

THE GENERAL CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR APPROVAL BEFORE FABRICATION OR ERECTION OF ANY STRUCTURAL SYSTEM.

THE CONTRACTOR SHALL ADEQUATELY PROTECT HIS WORK, ADJACENT PROPERTY AND THE PUBLIC, AND BE RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS ACTIONS.

THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL EXCAVATION PROCEDURES AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT. WHEN PERFORMING WORK BELOW GRADE, CARE SHALL BE TAKEN TO AVOID DAMAGING ANY EXISTING UTILITIES. ALL UNKNOWN UTILITIES, DISCOVERED DURING CONSTRUCTION SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER. ANY DAMAGED TO EXISTING UTILITIES AND/OR STRUCTURES SHALL BE REPORTED TO ALL AFFECTED PARTIES, INCLUDING THE ENGINEER.

THE FRAME SHALL BE BUILT TRUE AND PLUMB. TEMPORARY BRACING SHALL BE INTRODUCED WHEREVER NECESSARY TO TAKE CARE OF ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING EQUIPMENT AND OPERATION OF SAME. SUCH BRACING SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND SHALL BE LEFT IN PLACE AS LONG AS REQUIRED FOR SAFETY.

APPROVAL OF ANY WORK BY THE THRESHOLD INSPECTOR DOES NOT RELIEVE THE GENERAL CONTRACTOR FROM COMPLIANCE WITH THE STRUCTURAL DRAWINGS.

CONSTRUCTION TOLERANCE SHALL CONFORM TO ACI STANDARDS SET FORTH IN THE APPLICABLE SECTIONS OF THE MANUAL OF STANDARD PRACTICE. MINIMUM TOLERANCE SHALL BE AS FOLLOWS:

A. VARIATIONS FROM THE PLUMB

1. IN THE LINE SURFACES OF COLUMNS, PIERS, WALLS AND IN AREAS AS FOLLOWS:

IN ANY 10 FT. OF LENGTH.....1/4 INCH
MAXIMUM FOR ENTIRE LENGTH.....1 INCH

2. FOR EXPOSED CORNER COLUMNS, CONTROL JOINT GROOVES AND OTHER LINES.

IN ANY 20 FT. OF LENGTH.....1/4 INCH
MAXIMUM FOR ENTIRE LENGTH.....1/2 INCH

B. VARIATION FROM THE LEVEL OR FROM THE GRADES INDICATED ON THE DRAWINGS

1. IN SLAB SOFFITS, CEILINGS, BEAMS SOFFITS AND IN AREAS AS FOLLOWS:

IN ANY 10 FT OF LENGTH.....1/4 INCH
IN ANY BAY OR IN ANY 20 FT. OF LENGTH.....3/8 INCH
MAXIMUM FOR ENTIRE LENGTH.....3/4 INCH

2. IN EXPOSED LINTELS, SILLS, PARAPETS, HORIZONTAL GROOVES AND OTHER LINES:

IN ANY BAY OR ANY 20 FT. OF LENGTH.....1/4 INCH
MAXIMUM FOR ENTIRE LENGTH.....1/2 INCH

ELEVATIONS REFERENCE GROUND FLR. (0'–0") U.N.O.

CONCRETE:

SHALL BE A MIX DESIGNED IN ACCORDANCE WITH ACI 318–02 TO ACHIEVE A MIN. 28–DAY COMPRESSIVE STRENGTH (Fc') AS FOLLOWS (UNO):
FOUNDATIONS = 3000 PSI (FOOTINGS, GRADE BEAMS, SLAB ON GRADE, RETAINING WALLS) (2)
ALL OTHER CONC. = 4000 PSI

ALL CONCRETE AND CONCRETE COMPONENTS SHALL BE EXTRACTED, PROCESSED AND MANUFACTURED IN THE STATE OF FLORIDA, IN ACCORDANCE WITH LEED REQUIREMENTS.

SLUMP SHALL BE 5" +/– 1" EXCEPT FOR HIGH STRENGTH CONCRETE CONTAINING A SUPERPLASTICIZER.

NO WATER SHALL BE ADDED TO THE CONCRETE AT THE JOB SITE. TRANSPORTING, PLACING, DEPOSITING AND CURING OF CONCRETE SHALL COMPLY WITH ACI 318–02. CONCRETE SHALL BE COMPACTED BY MECHANICAL VIBRATION.

MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF ANY CONCRETE WORK. FLY ASH SHALL BE INCLUDED IN THE MIX DESIGNS AND SHALL CONFORM TO ASTM C 618.

THE OWNER SHALL CONTRACT AN INDEPENDENT TESTING LABORATORY APPROVED BY THE ENGINEER TO PERFORM CONCRETE STRENGTH TEST IN ACCORDANCE WITH ASTM STANDARDS AND ACI 318–02. A MINIMUM OF 4 TEST CYLINDER SHALL BE TAKEN DAILY FROM EVERY 50 CUBIC YARD OR FRACTION OF THERE FOR EACH TYPE OF CONCRETE PRIOR TO PLACEMENT. THE TESTING LAB SHALL PROVIDE THE ENGINEER WITH COPIES OF ALL TEST RESULT.

NO PIPES OR CONDUITS EXCEEDING 1/3 THE SLAB THICKNESS IN OUTSIDE DIAMETER SHALL BE EMBEDDED IN THE STRUCTURAL CONCRETE FLOOR OR ROOF WITHOUT WRITTEN APPROVAL FROM THE ENGINEER. WHERE PIPES OR CONDUITS ARE PERMITTED, THEY SHALL BE PLACED NO CLOSER THAN THREE DIAMETERS O.C. AND SHALL BE LOCATED SO AS NOT TO IMPAIR THE STRENGTH OF THE STRUCTURE.

CONSTRUCTION JOINTS USING APPROVED BULKHEADS MAY BE MADE AT THE CENTER OF BEAM OR SLAB SPANS WHERE STOPPAGE OF CONCRETE WORK IS NECESSARY. ANY OTHER CONSTRUCTION JOINT REQUESTED BY THE CONTRACTOR SHALL BE SUBMITTED TO ENGINEER'S FOR REVIEW.

REMOVE ALL DEBRIS FROM FORMS BEFORE POURING CONCRETE.

EPOXY SYSTEM SHALL BE HILTI HIT HY–150 (OR APPROVED EQUAL) UNO. (2)
LW CONC. SHALL HAVE MAX. UNIT WT.= 110 PCF (3)

REINFORCING STEEL:

REINFORCING STEEL SHALL BE DEFORMED BARS, FREE FROM LOOSE RUST AND SCALE, AND CONFORMING TO ASTM A615 GRADE 60.

ALL REINFORCING STEEL SHALL BE ACCURATELY LOCATED AND FIRMLY HELD IN PLACE BEFORE AND DURING THE PLACEMENT OF CONCRETE. WIRE ACCESSORIES SHALL HAVE UPTURNED LEGS AND SHALL BE PLASTIC DIPPED AFTER FABRICATION. CHAIRS USED IN BALCONIES OR OTHER AREAS EXPOSED TO THE WEATHER SHALL BE PLASTIC.

SUPPORT BARS SHALL BE # 4 BARS OR GREATER, AND NOT SPACED MORE THAN 4'–0" O.C. SUPPORT BARS AND ENDS OF MAIN REINFORCING SHALL NOT EXTENDED MORE THAN 1'–6" PAST OUTERMOST CHAIR OR SUPPORT BAR.
A MINIMUM OF 3 SUPPORT BARS AND 3 INDIVIDUAL HIGH CHAIRS FOR EACH SUPPORT BAR SHALL BE PROVIDED FOR TOP REINFORCING.

ALL PLACEMENT OF REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE MANUAL OF STANDARD PRACTICE OF THE CONCRETE REINFORCING STEEL INSTITUTE.

PLASTIC TIPPED COLUMN SPACERS SHALL BE PROVIDED FOR VERTICAL COLUMN REINFORCING STEEL, SUCH THAT A 2" MINIMUM CLEARANCE IS MAINTAINED.

CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

FOOTINGS ..... 3" (1 1/2" FOR TOP BARS)
INTERIOR SLABS & WALLS .....3/4"
EXTERIOR SLABS AND WALLS
EXPOSED TO WEATHER..... 1 1/2" (2" FOR #6 BARS OR LARGER)
BEAMS..... 1 1/2" (TO STIRRUPS)
COLUMNS..... 1 1/2" (TO TIES)

SHOP DRAWINGS OF ALL REINFORCING STEEL SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE FABRICATION.

THE CONTRACTOR SHALL PROVIDE 5 TONS OF STEEL REINFORCEMENT FOR THE ENGINEER TO USE AT HIS DISCRETION DURING CONSTRUCTION. THE CONTRACTOR SHALL GIVE CREDIT TO THE OWNER FOR ANY UNUSED PORTION OF THE ALLOWANCE AT THE END OF THE PROJECT.

PROVIDE # 5 TOP & BOTTOM CONT. AT SLAB EDGES.
PROVIDE # 5 X 4'–0" DIAGONAL AT SLAB RE–ENTRANT CORNERS TOP & BOTTOM.

STEEL REINFORCING SHALL BE SUPPLIED BY LEED–CERTIFIED MILL ONLY.

SEE SPLICE SCHEDULE ON S2.01

FOUNDATIONS:

FOUNDATIONS HAVE BEEN DESIGNED FOLLOWING THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT PROVIDED BY NUTTING ENGINEERS, DATED MAR., 2009. SHALLOW FOUNDATIONS HAVE BEEN DESIGNED FOR A NET ALLOWABLE BEARING PRESSURE OF 4500 PSF.

ALL FOUNDATION CONCRETE SHALL BE CAST IN THE DRY. DEWATERING OPERATION SHALL BE DONE IN SUCH A WAY THAT GROUND WATER LEVELS OUTSIDE THE SITE WILL BE MAINTAINED IN ORDER TO AVOID SETTLEMENT AND DAMAGE TO NEARBY BUILDINGS AND STRUCTURES.

GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR SHORING, SHEETING AND BRACING OF EXCAVATIONS.

FOUNDATIONS SHOWN MUST BE CONSTRUCTED IN ACCORDANCE W/THE RECOMMENDATIONS OF THE GEOTECH. REPORT.

(2) FOUNDATIONS SHALL BE EARTH – FORMED.

WELDED WIRE FABRIC:

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND BE SUPPORTED ON SLAB BOLSTERS SPACED AT 3'–0" O.C. PROVIDE FLAT SHEETS ONLY.

CONCRETE SLABS ON FILL:

SHALL BE PLACED ON VAPOR BARRIER SUPPORTED BY 6" MIN. CLEAN, NON–ORGANIC, GRANULAR SOIL, COMPACTED TO 98% MODIFIED PROCTOR DENSITY AS PER ASTM D1557. THE SUBGRADE SHALL BE PREPARED IN ACCORDANCE W/ THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER. SEE ARCHITECTURAL DRAWINGS FOR VAPOR BARRIER DETAILS.

FORMING, SHORING & RESHORING:

THE CONTRACTOR SHALL FURNISH FORMING, SHORING AND RESHORING DRAWINGS PREPARED BY A SPECIALTY ENGINEER. PLANS SHALL INCLUDE ALL SLAB, BEAM, AND COLUMN FORMS WITH BRACES AS REQUIRED.

SHORING AND RESHORING PLAN, TO INCLUDE LEVELS OF SHORING REQUIRED, TYPE AND SPACING OF SHORES AND BACK SHORES, CONCRETE STRENGTH REQUIRED FOR STRIPPING, AND DETAILED PROCEDURES FOR THE ENTIRE OPERATION.

DESIGN FORMS AND SHORES FOR HORIZONTAL CONCRETE MEMBERS FOR NOT LESS THAN THE DEAD LOAD PLUS 50 PSF CONSTRUCTION LOAD, AND FOR THE CUMULATIVE LOADS OF SUPPORTED FLOORS. DESIGN WOOD SHORES WITH A SAFETY FACTOR OF 3 AND METAL SHORES WITH A SAFETY FACTOR OF 2.

PRIOR TO EACH POUR THE SHORING AND RESHORING SHALL BE INSPECTED BY THE SPECIALTY ENGINEER OR HIS AUTHORIZED REPRESENTATIVE TO INSURE COMPLIANCE WITH HIS DESIGN.

FOR SPECIAL CONDITIONS SUCH AS TRANSFER BEAMS, TRANSFER COLUMNS OR WALLS, OR ANY OTHER STRUCTURAL ELEMENTS TRANSFERRING LOAD, THE SPECIALTY ENGINEER PREPARING THE FORM WORK SHOP DRAWINGS SHALL CONTACT THE ENGINEER OF RECORD FOR ADDITIONAL INFORMATION REGARDING LOADS TO BE TRANSFERRED AT THOSE ELEMENTS.

REINFORCED MASONRY:

CONSTRUCT REINFORCED MASONRY IN ACCORDANCE WITH ACI 530 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" AND ACI 530.1 "SPECIFICATION FOR MASONRY STRUCTURES". PLACE MASONRY UNITS IN RUNNING BOND.

USE CONCRETE MASONRY UNITS CONFORMING TO ASTM C90. MASONRY COMPRESSIVE STRENGTH (F'm) TO BE 1500 PSI. USE ONLY MASONRY UNITS THAT ARE A MINIMUM OF 50% SOLID.

USE TYPE M OR S MORTAR IN ACCORDANCE WITH ASTM C270. USE 3/8" MORTAR JOINTS FOR ALL MASONRY UNITS. MORTAR ALL HEAD JOINTS, THE FACE SHELLS OF ALL BED JOINTS, AND THE BED JOINTS OF WEBS ADJACENT TO CELLS THAT ARE TO BE GROUTED. VERTICAL CELLS TO BE GROUTED SHALL BE ALIGNED AND UNOBSTRUCTED. ANY MORTAR PROTRUDING INTO CELL CAVITIES THAT ARE TO BE GROUTED SHOULD BE REMOVED. ALLOW A MINIMUM OF 24 HOURS FOR MORTAR TO CURE BEFORE PLACING GROUT.

USE ALL GROUT CONFORMING TO ASTM C–476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI IN 28 DAYS, TESTED IN ACCORDANCE WITH ASTM C–39, SLUMP OF 9" TO 11". TEST SAMPLES FOR COMPRESSIVE STRENGTH. TEST EVERY 30 YARDS MINIMUM, OR EACH DAYS GROUTING.

USE STANDARD NO. 9 GAGE LADDER TYPE MASONRY REINFORCING IN EVERY OTHER COURSE (U.O.N.) USE PREFABRICATED CORNERS AND TEES AT WALL INTERSECTIONS. OVERLAP DISCONTINUOUS ENDS A MINIMUM OF 12". HORIZONTAL REINFORCING SHALL CONFORM TO ASTM A–82.

PROVIDE 14 GAGE DOVE TAIL ANCHORS (5 1/2" LONG) AND INSERTS IN EVERY OTHER COURSE AT MASONRY– COLUMN INTERSECTIONS.

PROVIDE CLEANOUT OPENINGS FOR EACH GROUTED CELL. WHERE CELLS ARE TO BE GROUTED, USE BAR SPACERS IN EVERY 4TH COURSE.

IN HIGH–LIFT GROUTING USE A MAXIMUM LIFT OF 4'–0", WITH NOT LESS THAN 30 MINUTES, OR MORE THAN ONE HOUR BETWEEN LIFTS. MECHANICALLY VIBRATE EACH LIFT AND RECONSOLIDATE THE PREVIOUS LIFT AFTER PLACING THE NEXT LIFT. THE MAXIMUM TOTAL POUR SHALL BE 12'–0".

REINFORCED MASONRY SHALL BE INSPECTED BY A SPECIAL INSPECTOR AS PER THE FLORIDA BUILDING CODE.

ALL WALLS ARE NON–LOAD–BRG. U.N.O. & SHALL BE ERECTED AFTER REMOVAL OF SHORING FROM STRUCTURE.

REINFORCING IN MASONRY WALLS SHALL BE CONTINUOUS W/48 BAR DIA. SPLICES & TERMINATE W/STD. HOOK.
PROVIDE 3" ø BLOCK–OUT IN SLAB OR THREADED INSERT IN CONCRETE BEAM SOFFIT.

PROVIDE VERTICAL REINF. IN GROUTED CELLS AS SHOWN ON PLAN. PROVIDE 1 REINF. CELL EACH SIDE OF OPENINGS LESS THAN 4'–0" WIDE . PROVIDE 2 REINF. CELLS EACH SIDE OF OPENINGS 4'–0" TO 8'–0" WIDE . PROVIDE A CONCRETE TIE COLUMN EACH SIDE OF OPENINGS OVER 8'–0" WIDE, U.N.O. PROVIDE REINF. CELL AT CORNERS & ENDS OF WALLS.

SEE DETAIL B/S4.00 FOR BRACING BELOW CONC. JSTS.

STRUCTURAL STEEL:

FABRICATE AND ERECT STRUCTURAL STEEL IN CONFORMANCE WITH THE AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", WITH COMMENTARY. FABRICATE ALL BEAMS W/ MILL CAMBER UP.

STRUCTURAL STEEL SHALL CONFORM TO:

ROLLED SHAPES	ASTM A572 GRADE 50
PLATES AND BARS	ASTM A36
STEEL TUBING	ASTM A500 GRADE B
STEEL PIPE	ASTM A53 GRADE B
ANCHOR BOLTS	ASTM A307
FRAMING BOLTS	ASTM A325

PROVIDE HARDENED WASHERS CONFORMING TO ASTM F 436 FOR ALL BOLTS.

CUT, DRILL, OR PUNCH HOLES PERPENDICULAR TO METAL SURFACES. DO NOT FLAME CUT HOLES OR ENLARGE HOLES BY BURNING.

HOT DIP GALVANIZE, AFTER FABRICATION, ALL STRUCTURAL STEEL EXPOSED TO THE WEATHER.

PROVIDE TEMPORARY BRACING AS NECESSARY TO INSURE A STABLE STRUCTURE DURING CONSTRUCTION.

SHOP DRAWINGS OF ALL STRUCTURAL STEEL SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE FABRICATION.

PRE–FAB. STEEL TRUSSES SHALL BE VULCRAFT (OR APPROVED EQUAL). HANDLING & ERECTION SHALL CONFORM TO SJI STANDARDS.

ALL STEEL SHALL BE SUPPLIED BY LEED–CERTIFIED MILL.

METAL DECK SHALL BE VULCRAFT (OR APPROVED EQUAL) & SHALL BE INSTALLED IN SHTS. W/ 3 EQ. SPANS MIN.

WELDING:

ALL WELDING SHALL BE DONE BY AWS–CERTIFIED WELDERS. WELDING SHALL BE MADE WITH E70XX ELECTRODES AND CONFORM TO THE CURRENT RECOMMENDATIONS OF THE AISC AND THE AMERICAN WELDING SOCIETY.

ALL FIELD WELDING SHALL BE TESTED BY A TESTING LAB AFTER COMPLETION. SUBMIT THE RESULTS TO THE ENGINEER OF RECORD. THE TESTING LAB SHALL BE HIRED BY THE OWNER.

SHOP DRAWINGS:

REVIEW OF SHOP DRAWINGS BY THE ENGINEER IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AS PRESENTED BY THE CONTRACT DOCUMENTS. NO DETAILED CHECK OF QUANTITIES OR DIMENSIONS WILL BE MADE. REVIEW OF SHOP DRAWINGS IS NOT CONDUCTED FOR DETERMINING THE ACCURACY AND COMPLETENESS OF DETAILS OR FOR SUBSTANTIATING FABRICATION, INSTALLATION INSTRUCTIONS, OR PERFORMANCE, ALL OF WHICH SHALL REMAIN THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

DO NOT REPRODUCE THE STRUCTURAL CONTRACT DOCUMENTS FOR USE AS SHOP DRAWINGS.

ALL CHANGES AND ADDITIONS MADE ON RE–SUBMITTALS MUST BE CLEARLY FLAGGED AND NOTED. ENGINEER REVIEW WILL BE LIMITED TO THOSE ITEMS FLAGGED AND NOTED.

SUBMIT TO THE ENGINEER A MAXIMUM OF FIVE ORIGINALS ONLY AFTER THEY HAVE BEEN APPROVED BY THE CONTRACTOR. A MAXIMUM OF FOUR MARKED COPIES WILL BE RETURNED TO THE ARCHITECT BY THE ENGINEER.

SHOP DRAWINGS REQUIRING A SPECIALTY ENGINEER:

A SPECIALTY ENGINEER IS A FLORIDA REGISTERED PROFESSIONAL ENGINEER, NOT THE ENGINEER OF RECORD, WHO SPECIALIZES IN AND WHO UNDERTAKES THE DESIGN OF STRUCTURAL COMPONENTS OR STRUCTURAL SYSTEMS INCLUDED IN A SPECIFIC SUBMITTAL PREPARED FOR THIS PROJECT.

THE SPECIALTY ENGINEER IS TO BE RETAINED BY THE SUPPLIER OF THE SPECIALTY ITEM OR MAY BE AN EMPLOYEE OF THAT SUPPLIER. ALL COSTS CONNECTED WITH THE USE OF A SPECIALTY ENGINEER SHALL BE INCLUDED IN THE BID PRICE OF THE SUPPLIER FOR THE ITEM IN QUESTION.

SHOP DRAWINGS REQUIRING SPECIALTY ENGINEERING ARE AS FOLLOWS: FORMING, SHORING AND RESHORING, PRE–CAST CONC. JOISTS, STL. TRUSSES, STL.–STL. CONNECTIONS, RAILINGS, PREFABRICATED STAIRS, EXTERIOR DOORS AND WINDOWS, CANOPIES AND CURTAIN WALL SYSTEMS.

SHOP DRAWINGS AND CALCULATIONS REQUIRE THE IMPRESSED SEAL, DATE, AND SIGNATURE OF THE SPECIALTY ENGINEER. COMPUTER PRINTOUTS ARE AN ACCEPTABLE SUBSTITUTE FOR MANUAL COMPUTATIONS PROVIDED THEY ARE ACCOMPANIED BY SUFFICIENT DESCRIPTIVE INFORMATION TO PERMIT THEIR PROPER EVALUATION. SUCH DESCRIPTIVE INFORMATION SHALL BEAR THE IMPRESSED SEAL AND SIGNATURE OF THE SPECIALTY ENGINEER AS AN INDICATION THAT HE HAS ACCEPTED RESPONSIBILITY FOR THE RESULTS.

THRESHOLD INSPECTION:

THIS IS A THRESHOLD BUILDING. THE OWNER SHALL HIRE A THRESHOLD INSPECTOR, CERTIFIED IN THE STATE OF FLORIDA, WITH AT LEAST 7 YEARS EXPERIENCE IN THE DESIGN AND INSPECTION OF SIMILAR STRUCTURES. THE THRESHOLD INSPECTOR SHALL SUBMIT HIS QUALIFICATIONS TO THE ENGINEER OF RECORD FOR APPROVAL.

THE THRESHOLD INSPECTOR SHALL CONDUCT INSPECTIONS AND PREPARE DAILY REPORTS ACCORDING TO THE THRESHOLD INSPECTION PLAN PREPARED BY PISTORINO & ALAM CONSULTING ENGINEERS, INC.

THE THRESHOLD INSPECTOR SHALL NOT SUPERVISE THE CONTRACTOR'S EMPLOYEES. THE INSPECTOR SHALL ONLY OBSERVE THE CONTRACTORS WORK AND EITHER APPROVE OR NOT APPROVE THE WORK. APPROVAL OF THE CONTRACTORS WORK BY THE THRESHOLD INSPECTOR DOES NOT DIMINISH THE GENERAL CONTRACTOR'S RESPONSIBILITY FOR MEETING THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.

(2)

MASONRY LINTEL SCHEDULE

WALL THICKNESS	EXPOSURE	MAX. OPENING WIDTH	TYPE	DEPTH	ADD REINF. (GROUTED)		
					BOT	TOP	MID
8"	INTERIOR	4'–0"	8U8	8"			
8"	EXTERIOR	6'–0"	8F8–OB/IT	8"		#5	

(3)
NOTES:
(1) LINTELS SHALL BE PRE–CAST CONCRETE BY "CAST–CRETE" (OR APPROVED EQUAL)
(2) LINTELS SHALL HAVE 8" MIN. BEARING EA. END.
(3) CONSULT EOR FOR CONDITIONS NOT SCHEDULED

CONNECTIONS:

STEEL TO STEEL CONNECTIONS SHALL BE DESIGNED BY A SPECIALTY ENGINEER. CONNECTIONS SHOWN ON THE STRUCTURAL DWGS. ARE MINIMUM REQUIREMENTS. SEE CONN. SCHEDULE ON S3.01 FOR SERVICE LOADS. IF NOT SCHEDULED, DESIGN CONN. FOR MAX. LOAD SHOWN IN AISC MANUAL OF STEEL CONSTRUCTION TABLES: "ALLOWABLE LOADS ON BEAMS" OR "ALLOWABLE CONCENTRIC LOADS ON COLUMNS." (2)

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REVISION 02 - AUGUST 14 2009

OFFICE REGISTRATION #: AA C000465

Date		Revision
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10-28-09	(3)	REVISION 03

Issue (#)	Issue Date / For
7.24.07	SCHEMATIC DESIGN APPROVAL
11.12.07	SCHEMATIC DESIGN APPROVAL
02.22.08	SCHEMATIC DESIGN APPROVAL
03.28.08	SCHEMATIC DESIGN APPROVAL
10.21.08	PROGRESS DESIGN DEVELOPMENT
11.26.08	100 % DESIGN DEVELOPMENT
02.25.09	FOUNDATION PERMIT
03.18.09	50% CONSTRUCTION DOCUMENTS
05.18.09	100% CONSTRUCTION DOCUMENTS

PROJECT NO.: 2441

SHEET INDEX

GENERAL NOTES

S1.00