First Order Time Response Second Order Time Response Response and Root Location Specification of The Step Response

Lecture Module - Time Response

ME3050 - Dynamics Modeling and Controls

Mechanical Engineering
Tennessee Technological University

Time Response

Lecture Module - Time Response

- Topic 1 First Order Time Response
- Topic 2 Second Order Time Response
- Topic 3 Response and Root Location
- Topic 4 Specification of The Step Response

Topic 1 - First Order Time Response

- Second Order Free Response
- Second Order Forced Response
- Response and System Stability
- Equation Components

Model and EOM

Consider the model of the moving mass we derived.



The EOM is:

$$m\dot{v} + cv = 0$$

Solution with Laplace Transforms Method

$$\mathcal{L}\{m\dot{v}+cv=0\} \implies m[sV(s)-v(0)]+cV(s)=0$$

$$(ms+c)V(s) = \frac{mv(0)}{(ms+c)} = \frac{V(0)}{s+\frac{c}{m}}$$

We can find the expected result from the table.

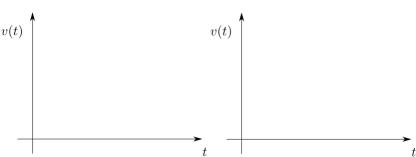
$$v(t) = v(0)e^{-\frac{c}{m}t} = v(0)e^{-\frac{t}{\tau}}$$
 with $\tau =$

econd Order Forced Response esponse and System Stability quation Components

Sketch Response Equation

Sketch the System Response in the time Domain.

$$v(t) = v(0)e^{-\frac{t}{\tau}}$$



Is this a stable system? What does that mean?

econd Order Free Response econd Order Forced Responso esponse and System Stability quation Components

Stability in Dynamic Systems

A dynamic system is stable if ...

First Order Time Response
Second Order Time Response
Response and Root Location
Specification of The Step Response

Second Order Free Response Second Order Forced Response Response and System Stability Equation Components

Second Order Forced Response

Step Input Function

Consider the model subject to a Step Input, f(t).



$$m\dot{v} + cv = f(t)$$

$$f(t) = \begin{cases} 0 & t < 0 \\ F & t \ge 0 \end{cases}$$

Solution with Laplace Transforms Method - Step 1

The method of Laplace Transforms is shown.

$$\mathcal{L}\{m\dot{v} + cv = F\} \implies m[sV(s) - v(0)] + cV(s) = \frac{F}{c}$$
$$(ms + c)V(s) = \frac{F}{s} + mv(0)$$

Solve for V(s).

$$V(s) = \frac{F}{s(ms+c)} + \frac{mv(0)}{ms+c}$$

Solution with Laplace Transforms Method - Step 2

Expand V(s) as a partial fraction.

$$V(s) = \frac{F}{s(ms+c)} + \frac{mv(0)}{ms+c} \implies \frac{F}{s(ms+c)} = \frac{a}{s} + \frac{b}{ms+c}$$

'Cover up' to find the coefficients.

$$a = \frac{F}{m \times 0 + c}$$
 and $b = \frac{F}{\frac{-c}{m}} = \frac{-Fm}{c}$

This leads to a form that can be inverted with the table.

$$V(s) = \frac{F}{c} \left\{ \frac{1}{s} - \frac{1}{s + \frac{c}{m}} \right\} + \frac{v(0)}{s + \frac{c}{m}}$$

Solution with Laplace Transforms Method - Step 3

Can you find these terms in the Table of Laplace Transforms?

$$V(s) = \frac{F}{c} \left\{ \frac{1}{s} - \frac{1}{s + \frac{c}{m}} \right\} + \frac{v(0)}{s + \frac{c}{m}}$$

The inverse Laplace transform of V(s) gives the time response.

$$v(t) = \frac{F}{C} \{ 1 - e^{-\frac{t}{\tau}} \} + v(0)e^{-\frac{t}{\tau}} = \{ v(0) - \frac{F}{c} \} e^{-\frac{t}{\tau}} + \frac{F}{c}$$

Second Order Free Response Second Order Forced Response Response and System Stability Equation Components

Response and System Stability

Second Order Free Response Second Order Forced Response Response and System Stability Equation Components

Response and System Stability

Sketch Response Equation

Sketch the System Response in the time Domain.

$$v(t) = \{v(0) - \frac{F}{c}\}e^{-\frac{t}{\tau}} + \frac{F}{c}$$

$$v(t)$$

Is this a stable system?

Components of the Response

In these forms we can see the different components of the response.

$$v(t) = \frac{F}{C} \{ 1 - e^{-\frac{t}{\tau}} \} + v(0)e^{-\frac{t}{\tau}} = \{ v(0) - \frac{F}{c} \} e^{-\frac{t}{\tau}} + \frac{F}{c}$$

- Forced Response
- Free Response
- Transient Response
- Steady-State Response

Second Order Free Response Second Order Forced Respons Response and System Stability Equation Components

References

 System Dynamics, Palm III, Third Edition - Section 8.1 -Response of First Order Systems - pg. 475

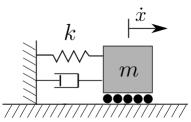
Topic 2 - Second Order Time Response

- Damped Natural Frequency and Damping Ratio
- The Overdamped Case
- The Critically Damped Case
- The Underdamped Case

he Critically Damped Case

Mass Spring Model

Consider the mass-spring system without damping.



The EOM is:

$$m\ddot{x} + kx = 0$$
 with

$$x(t = 0) = x_0$$
, and $v(t = 0) = v_0$

Solution with Laplace Transforms Method

Solve for x(t) with a method of your choice.

$$x(t) = \frac{v_0}{\omega_n} sin(\omega_n t) + x_0 cos(w_n t)$$
 with $\omega_n = \sqrt{\frac{k}{m}}$

he Critically Damped Case

Phase Shift

The solution is commonly written as a single oscillating term with a phase shift ϕ .

$$x(t) = \frac{v_0}{\omega_n} sin(\omega_n t) + x_0 cos(w_n t)$$
 with $\omega_n = \sqrt{\frac{k}{m}}$

Is equivalent to:

$$x(t) = Acos(\omega_n t - \phi) A = \sqrt{x_0^2 + \left[\frac{v_0}{\omega_n}\right]^2} \phi = tan^{-1}(\frac{v_0}{x_0\omega_n})$$

Sine could be used instead.

$$x(t) = Asin(\omega_n t + \phi)$$
 $A = \sqrt{x_0^2 + [rac{v_0}{\omega_n}]^2}$ $\phi = tan^{-1}(rac{x(0)\omega_n}{v_0})$

The Overdamped Case
The Critically Damped Case

Sketch of Free Response

Sketch the **free** response in the time domain.

$$x(t) = \frac{v_0}{\omega_n} sin(\omega_n t) + x_0 cos(w_n t)$$

$$= Acos(\omega_n t - \phi) \quad with \quad \phi = tan^{-1}(\frac{v_0}{x_0 \omega_n})$$

$$x(t)$$

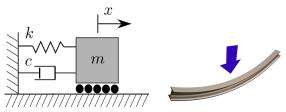
Is this a stable system? What does the phase shift ϕ represent?

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The Underdamped Case

Second Order System with Damping

Now, consider the mass-spring system with damping present.



The EOM is:

$$m\ddot{x}+c\dot{x}+kx=0$$
 with $x(t=0)=x_0$, and $v(t=0)=v_0$

he Critically Damped Case

Solution with Trial Solution Method

The trial solution method is used to derive the response equation in terms of the system variables and parameters.

$$m\ddot{x} + c\dot{x} + kx = 0 \implies (mr^2 + cr + k)Ae^{rt} = 0$$

You can see the *characteristic equation* becomes:

$$(mr^2+cr+k)=0$$

Solve for the roots. In system dynamics they are called $s_{1,2}$

$$r_{1,2} = s_{1,2} = \frac{-c \pm \sqrt{c^2 - 4mk}}{2m} = -\frac{c}{2m} \pm \sqrt{\left(\frac{c}{2m}\right)^2 - \frac{k}{m}}$$

The Roots of the System

The roots of the system determine the behavior.

$$s_{1,2} = \frac{-c \pm \sqrt{c^2 - 4mk}}{2m}$$

The discriminant $c^2 - 4mk$ drives the case in the trial solution method.

- IF \Longrightarrow Case 1: Distinct and Real
- ullet IF \longrightarrow Case 2: Repeated and Real
- IF _____ ⇒ Case 3: Complex Conjugate Pair

The Underdamped Case

Damping Cases and the Critical Damping Value

In a system with known mass and spring constant, the damping value determines the behavior. The damping value that causes the *discriminant* to equal zero (case 2) is the critical damping value.

$$c^2 - 4mk = 0 \implies c = \sqrt{4mk} = 2\sqrt{mk}$$

 $c_{critical} = 2\sqrt{mk}$

The Damping Ratio

The damping ratio ζ is the ratio of damping c to the critical damping value $c_{critical}$.

$$\zeta = \frac{c}{c_{critical}} = \frac{c}{2\sqrt{mk}}$$

Re-write the roots with this new quantity.

$$s_{1,2} = -\zeta \omega_n \pm \omega_n \sqrt{\zeta^2 - 1} = -\zeta \omega_n \pm j \omega_n \sqrt{1 - \zeta^2}$$

We define a new quantity, damped natural frequency.

$$\omega_d = \omega_n \sqrt{1 - \zeta^2}$$

Now re-write the roots again in terms of ζ and ω_d .

$$s_{1,2} = \zeta \omega_n \pm j \omega_d$$



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Damped Natural Frequency and Damping Ratio

The behavior of the system depends on the damping ratio.

Case 1	$c>2\sqrt{mk}$	Overdamped	$\mid \zeta > 1 \mid$
Case 2	$c=2\sqrt{mk}=c_{critical}$	Critically Damped	$\zeta=1$
Case 3	$c < 2\sqrt{mk}$	Underdamped	$\mid \zeta < 1 \mid$

The Overdamped Case

The roots are real and distinct and the system does not oscillate.

Overdamped
$$c>2\sqrt{mk} \implies \zeta>1$$

$$s_{1,2} = -\zeta \pm \omega_n \sqrt{\zeta^2 - 1}$$

$$x(t) = C_1 e^{(-\zeta \omega_n + \omega_n \sqrt{\zeta^2 - 1})t} + C_2 e^{(-\zeta \omega_n - \omega_n \sqrt{\zeta^2 - 1})t}$$

$$x(t) = e^{-\zeta \omega_n} \{ C_1 e^{\omega_n \sqrt{\zeta^2 - 1}t} + C_2 e^{\omega_n \sqrt{\zeta^2 - 1}t} \}$$

$$C_1 = \frac{-v_0 + (-\zeta + \sqrt{\zeta^2 - 1})\omega_n x_0}{2\omega_n \sqrt{\zeta^2 - 1}} \quad C_2 = \frac{-v_0 + (\zeta + \sqrt{\zeta^2 - 1})\omega_n x_0}{2\omega_n \sqrt{\zeta^2 - 1}}$$

The Critically Damped Case

The roots are real and repeated and the system does not oscillate.

Critically Damped
$$\,c=2\sqrt{mk}\,\implies\,\zeta=1\,$$

$$s_{1,2} = \frac{-c}{2m} = \zeta \omega_n = -\omega_n$$

$$x(t) = C_1 e^{-\omega_n t} + C_2 t e^{-\omega_n t}$$

$$\dot{x} = -\omega_n C_1 e^{-\omega_n t} + C_2 t (-\omega_n e^{-\omega_n t}) + e^{-\omega_n t} (C_2)$$

$$x(t=0) = x_0 \implies C_1 = x_0$$

$$v(t=0) = v_0 \implies v_0 = -\omega_n C_1 + 0 + (1)C_2 \implies C_2 = v_0 + \omega_n x_0$$

The Underdamped Case

Take the derivative and solve for the second unknown.

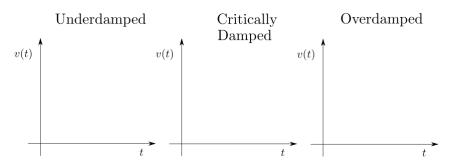
$$\begin{split} &x(t) = e^{-\zeta\omega_n t} \{Acos(\omega_d t) + Bsin(\omega_d t)\} \\ &\dot{x}(t) = e^{-\zeta\omega_n t} (-\omega_d Asin(\omega_d t)) + Acos(\omega_d t) (-\zeta\omega_n e^{-\zeta\omega_n t}) \\ &+ e^{-\zeta\omega_n t} (\omega_d Bcos(\omega_d t)) + Bsin(\omega_d t) e^{-\zeta\omega_n t} \\ &\dot{x}(t=0) = A(1) (-\zeta\omega_n (1)) + (1)\omega_d B(1) \implies B = \frac{v_0 + \zeta\omega_n x_0}{\omega_d} \end{split}$$

Finally we get to the reponse equation.

$$x(t) = e^{-\zeta \omega_n t} \{ x_0 cos(\omega_d t) + rac{v_0 + \zeta \omega_n x_0}{\omega_d} sin(\omega_d t) \}$$

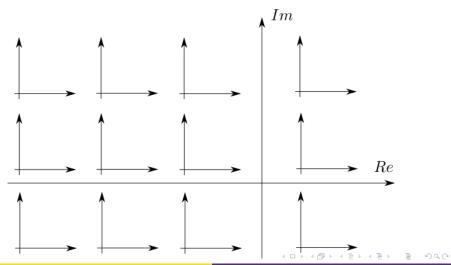
Response of the Three Different Cases

Each of the three cases behaves in a characteristic way.



Damped Natural Frequency and Damping Ratio The Overdamped Case The Critically Damped Case The Underdamped Case

Affects of Damping Ratio and Damped Natural Frequency

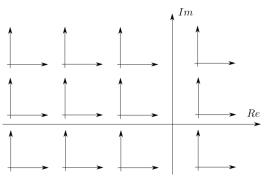


Topic 3 - Response and Root Location

- The Complex Plane
- Along a Vertical Line
- Along a Horizontal Line
- Along a Diagonal Line

Affect of Root Location on Response

The *location* of the root in the *complex plane* shows the affects of the roots on the system behviour.



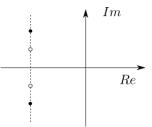
First Order Time Response Second Order Time Response Response and Root Location Specification of The Step Response

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The Complex Plane

Along a Vertical Line

As the root moves along a vertical line...

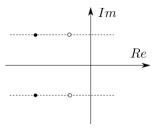


The Complex Plane Along a Vertical Line Along a Horizontal Line Along a Diagonal Line

Along a Vertical Line

Along a Horizontal Line

As the root moves along a horizontal line...



The Complex Plane Along a Vertical Line Along a Horizontal Line Along a Diagonal Line

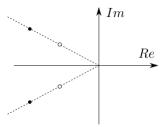
Along a Horizontal Line

The Complex Plane Along a Vertical Line Along a Horizontal Line Along a Diagonal Line

Along a Horizontal Line

Along a Diagonal Line

As the root moves along a diagonal line...



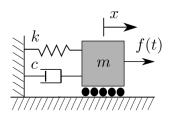
What about the angle of this line?

Topic 3 - Specification of The Step Response

- The Unit Step Response
- Rise, Peak, and Settling Time
- Maximum Overshoot and The Damping Ratio
- System Identificatio

The Mass Spring Damper

Now, consider the mass-spring system with damping present subject to **step** input. This models instantly turning on the input force f(t).



Heavyside's Step Function

$$f(t) = \begin{cases} 0 & t < 0 \\ F & t \ge 0 \end{cases}$$

The EOM is:

$$m\ddot{x} + c\dot{x} + kx = f(t)$$
 with $x(t = 0) = x_0$ and $v(t = 0) = x_0$

Unit Step Response

The unit step response is a special case of the forced response in which f(t) is the step function of unit magnitude (F=1).

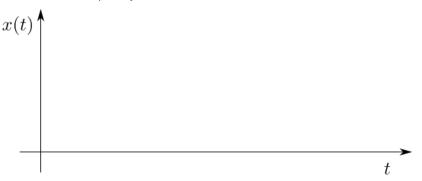
Overdamped
$$x(t) = \frac{1}{k} (\frac{r_2}{r_1 - r_2} e^{-r_1 t} - \frac{r_1}{r_1 - r_2} e^{-r_2 t} + 1$$

$$r_{1,2} = -s_{1,2}$$
 Critically Damped
$$x(t) = \frac{1}{K} [(-1 - \omega_n t) e^{-\omega_n t} + 1]$$

$$x(t) = \frac{1}{k} \left[\frac{1}{\sqrt{1 - \zeta^2}} e^{-\zeta \omega_n t} sin(\omega_d t + \phi + 1) \right]$$

$$\phi = tan^{-1} \left(\frac{\sqrt{1 - \zeta^2}}{\zeta} \right) + \pi$$

We are going to derive several quatities that describes the response of an underdamped system.



Rise Time

The **rise time** is the time at which the response first equals the steady state value.

$$x(t) = rac{1}{k} \left[rac{1}{\sqrt{1-\zeta^2}} e^{-\zeta \omega_n t} sin(\omega_d t + \phi) + 1
ight]$$

Set the transient term to zero and solve for t.

$$e^{-\zeta\omega_n t} sin(\omega_d t + \phi) = 0 \implies sin(\omega_d t + \phi) = 0 \implies \omega_d t + \phi = 2\pi$$
 $t_{rise} = t_r = \frac{2\pi - \phi}{\omega_d t}$

The **peak time** is the time at which the response equals the maximum value. Find the derivative of the response equation and set it equal to zero.

$$\dot{x}(t) = \left(\frac{1}{K} \frac{1}{\sqrt{1 - \zeta^2}}\right) \left[e^{-\zeta \omega_n t} (\omega_d \cos(\omega_d t + \phi)) + \sin(\omega_d t + \phi)(-\zeta \omega_n e^{-\zeta \omega_d t})\right]$$

$$\sin(\omega_d) t = 0 \implies \omega_d t = \pi \implies t_{peak} = t_p = \frac{\pi}{\omega_d}$$

The **settling time** is the time at which the response decays to a certain percentage of the steady state value.

It can be esitmated as:

$$t_{settling} = t_s = -rac{ln(tolerance)}{\zeta \omega_n}$$

$$2\% \implies tolerance = 0.02$$

$$5\% \implies tolerance = 0.05$$

The **maximum overshoot** is the response beyond the steady state value.

$$M_p = x(t_p) - x_{ss} \implies M_p = \frac{1}{k} e^{\frac{-\pi \zeta}{\sqrt{1-\zeta^2}}}$$

This is often expressed as a percentage.

$$M_{\%} = \frac{x(t_p) - x_{ss}}{x_{ss}} 100 = 100e^{\frac{-\pi\zeta}{\sqrt{1-\zeta^2}}}$$

Damping Ratio from Maximum Overshoot

The damping ratio can be determined from the maximum overshoot!

$$M_{\%} = 100e^{\frac{-\pi\zeta}{\sqrt{1-\zeta^2}}}$$

Solve for ζ .

$$\zeta = \frac{R}{\sqrt{\pi^2 + R^2}}$$
 with $R = \ln\left(\frac{100}{M\%}\right)$

Damping Ratio from Log Decrement

The logarithmic decrement is the natural log of the ratio of the amplitudes of any two successive peaks:

$$\delta = \frac{1}{n} \ln \frac{x(t)}{x(t+nT)}$$

x(t) is the overshoot (amplitude - final value) at time t and x(t+nT) is the overshoot of the peak n periods away.

The damping ratio is then found from the logarithmic decrement by:

$$\zeta = rac{1}{\sqrt{1 + \left(rac{2\pi}{\delta}
ight)^2}}$$

The Unit Step Response Rise, Peak, and Settling Time Maximum Overshoot and The Damping Ratio System Identificatio

Damping Ratio from Log Decrement

What is the significance of all of this?

Why do we care about all of these new parameters?

The Unit Step Response Rise, Peak, and Settling Time Maximum Overshoot and The Damping Ratio System Identificatio

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