Generalised truss solver

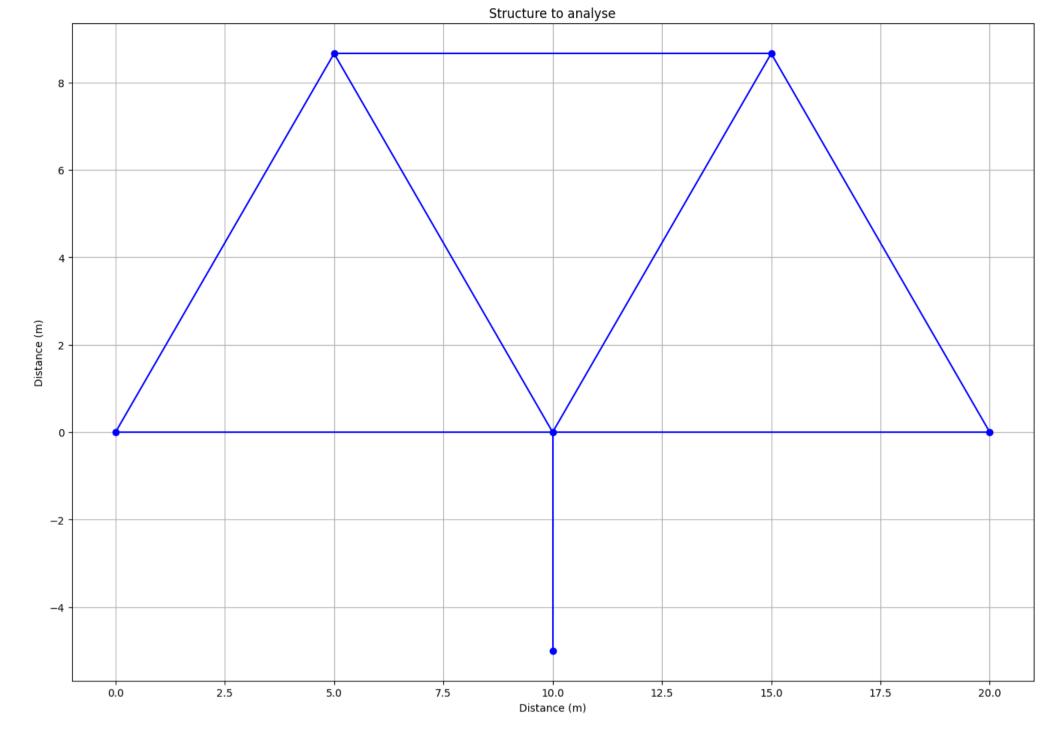
```
In [1]: # DEPENDENCIES
    import copy #Allows us to create copies of objects in memory
    import math #Math functionality
    import numpy as np #Numpy for working with arrays
    import matplotlib.pyplot as plt #Plotting functionality
```

Structure Data Entry

```
#Constants
      E = 200*10**9 \#(N/m^2)
      A = 0.005 \#(m^2)
      xFac = 50 #Scale factor for plotted displacements
      #Nodal coordinates [x, y] (in ascending node order)
      nodes = np.array([[0,0],
                    [5,8.66],
                    [15,8.66],
                    [20,0],
                    [10,0],
                    [10, -5]]
      #Members [node_i, node_j]
      members = np.array([[1,2],
                    [2,3],
                    [3,4],
                    [4,5],
                    [1,5],
                    [2,5],
                    [3,5],
                    [5,6]])
      restrainedDoF = [7,8,11,12] #The degrees of freedom restrained by supports
      forceVector = np.array([[0,-200000,0,0,0,0,0,0,0,0,0]]).T #Vector of externally applied forces
```

Plot structure to confirm before proceeding

```
In [3]: fig = plt.figure()
        axes = fig.add_axes([0.1,0.1,2,2])
        fig.gca().set_aspect('equal', adjustable='box')
        #Plot members
        for mbr in members:
            node_i = mbr[0] #Node number for node i of this member
            node_j = mbr[1] #Node number for node j of this member
            ix = nodes[node_i-1,0] #x-coord of node i of this member
            iy = nodes[node_i-1,1] #y-coord of node i of this member
            jx = nodes[node_j-1,0] #x-coord of node j of this member
            jy = nodes[node_j-1,1] #y-coord of node j of this member
            #Index of DoF for this member
            ia = 2*node_i-2 #horizontal DoF at node i of this member
            ib = 2*node_i-1 #vertical DoF at node i of this member
            ja = 2*node_j-2 #horizontal DoF at node j of this member
            jb = 2*node_j-1 #vertical DoF at node j of this member
            axes.plot([ix,jx],[iy,jy],'b') #Member
        #Plot nodes
        for node in nodes:
            axes.plot([node[0]],[node[1]],'bo')
        axes.set_xlabel('Distance (m)')
        axes.set_ylabel('Distance (m)')
        axes.set_title('Structure to analyse')
        axes.grid()
        plt.show()
```



Calculate member orientation and length

```
In [4]: #Define a function to calculate member orientation and length
        def memberOrientation(memberNo):
            memberIndex = memberNo-1 #Index identifying member in array of members
            node_i = members[memberIndex][0] #Node number for node i of this member
            node_j = members[memberIndex][1] #Node number for node j of this member
            xi = nodes[node_i-1][0] #x-coord for node i
            yi = nodes[node_i-1][1] #y-coord for node i
            xj = nodes[node_j-1][0] #x-coord for node j
            yj = nodes[node_j-1][1] #y-coord for node j
            #Angle of member with respect to horizontal axis
            dx = xj-xi #x-component of vector along member
            dy = yj-yi #y-component of vector along member
            mag = math.sqrt(dx**2 + dy**2) #Magnitude of vector (length of member)
            memberVector = np.array([dx,dy])#Member represented as a vector
            #Need to capture quadrant first then appropriate reference axis and offset angle
            if(dx>0 and dy==0):
                theta = 0
            elif(dx==0 and dy>0):
                theta = math.pi/2
            elif(dx<0 and dy==0):</pre>
                theta = math.pi
            elif(dx==0 and dy<0):
                theta = 3*math.pi/2
            elif(dx>0 and dy>0):
                # 0<theta<90
                refVector = np.array([1,0]) # Vector describing the positive x-axis
                theta = math.acos(refVector.dot(memberVector)/(mag))#Standard formula for the angle between two vectors
            elif(dx<0 and dy>0):
                # 90<theta<180
                refVector = np.array([0,1]) # Vector describing the positive y-axis
                theta = (math.pi/2) + math.acos(refVector.dot(memberVector)/(mag))#Standard formula for the angle between two vectors
            elif(dx<0 and dy<0):</pre>
                # 180<theta<270
                refVector = np.array([-1,0]) # Vector describing the negative x-axis
                theta = math.pi + math.acos(refVector.dot(memberVector)/(mag))#Standard formula for the angle between two vectors
            else:
                # 270<theta<360
                refVector = np.array([0,-1]) # Vector describing the negative y-axis
                theta = (3*math.pi/2) + math.acos(refVector.dot(memberVector)/(mag))#Standard formula for the angle between two vectors
            return [theta, mag]
```

```
In [5]: #Calculate orientation and length for each member and store
    orientations = np.array([])#Initialise an array to hold orientations
    lengths = np.array([]) #Initialise an array to hold lengths
    for n, mbr in enumerate(members):
        [angle, length] = memberOrientation(n+1)
        orientations = np.append(orientations,angle)
        lengths = np.append(lengths,length)
```

Define a function to calculate member global stiffness matrix

```
In [6]: #Define a function to calculate the global stiffness matrix of an axially loaded bar

def calculateKg(memberNo):
    """
    Calculate the global stiffness matrix for an axially loaded bar
    memberNo: The member number
    """
    theta = orientations[memberNo-1]
    mag = lengths[memberNo-1]

    c = math.cos(theta)
    s = math.sin(theta)

K11 = (E*A/mag)*np.array([[c**2,c*s],[c*s,s**2]]) #Top left quadrant of global stiffness matrix
    K12 = (E*A/mag)*np.array([[-c**2,-c*s],[-c*s,-s**2]]) #Top right quadrant of global stiffness matrix
    K21 = (E*A/mag)*np.array([[c**2,c*s],[-c*s,-s**2]]) #Bottom left quadrant of global stiffness matrix
    K22 = (E*A/mag)*np.array([[c**2,c*s],[c*s,s**2]]) #Bottom right quadrant of global stiffness matrix
    return [K11, K12, K21,K22]
```

Build the primary stiffness matrix, Kp

```
nDoF = np.amax(members)*2 #Total number of degrees of freedom in the problem
Kp = np.zeros([nDoF,nDoF]) #Initialise the primary stiffness matrix
for n, mbr in enumerate(members):
#note that enumerate adds a counter to an iterable (n)
    #Calculate the quadrants of the global stiffness matrix for the member
    [K11, K12, K21, K22] = calculateKg(n+1)
    node_i = mbr[0] #Node number for node i of this member
    node_j = mbr[1] #Node number for node j of this member
    #Primary stiffness matrix indices associated with each node
    #i.e. node 1 occupies indices 0 and 1 (accessed in Python with [0:2])
    ia = 2*node_i-2 #index 0
    ib = 2*node_i-1 #index 1
    ja = 2*node_j-2 #index 2
    jb = 2*node_j-1 #index 3
    Kp[ia:ib+1,ia:ib+1] = Kp[ia:ib+1,ia:ib+1] + K11
    Kp[ia:ib+1,ja:jb+1] = Kp[ia:ib+1,ja:jb+1] + K12
    Kp[ja:jb+1,ia:ib+1] = Kp[ja:jb+1,ia:ib+1] + K21
    Kp[ja:jb+1,ja:jb+1] = Kp[ja:jb+1,ja:jb+1] + K22
```

Extract structure stiffness matrix, Ks

```
In [8]: restrainedIndex = [x - 1 for x in restrainedDoF] #Index for each restrained DoF (list comprehension)

#Reduce to structure stiffness matrix by deleting rows and columns for restrained DoF

Ks = np.delete(Kp,restrainedIndex,0) #Delete rows

Ks = np.delete(Ks,restrainedIndex,1) #Delete columns

Ks = np.matrix(Ks) # Convert Ks from numpy.ndarray to numpy.matrix to use build in inverter function
```

Solve for displacements

```
In [9]: forceVectorRed = copy.copy(forceVector)# Make a copy of forceVector so the copy can be edited, leaving the original unchanged forceVectorRed = np.delete(forceVectorRed,restrainedIndex,0) #Delete rows corresponding to restrained DoF

U = Ks.I*forceVectorRed
```

Solve for reactions

```
In [10]: #Construct the global displacement vector
UG = np.zeros(nDoF) #Initialise an array to hold the global displacement vector
c=0 #Initialise a counter to track how many restraints have been imposed
for i in np.arange(nDoF):
    if i in restrainedIndex:
        #Impose zero displacement
        UG[i] = 0
    else:
        #Assign actual displacement
        UG[i] = U[c]
        c=c+1
```

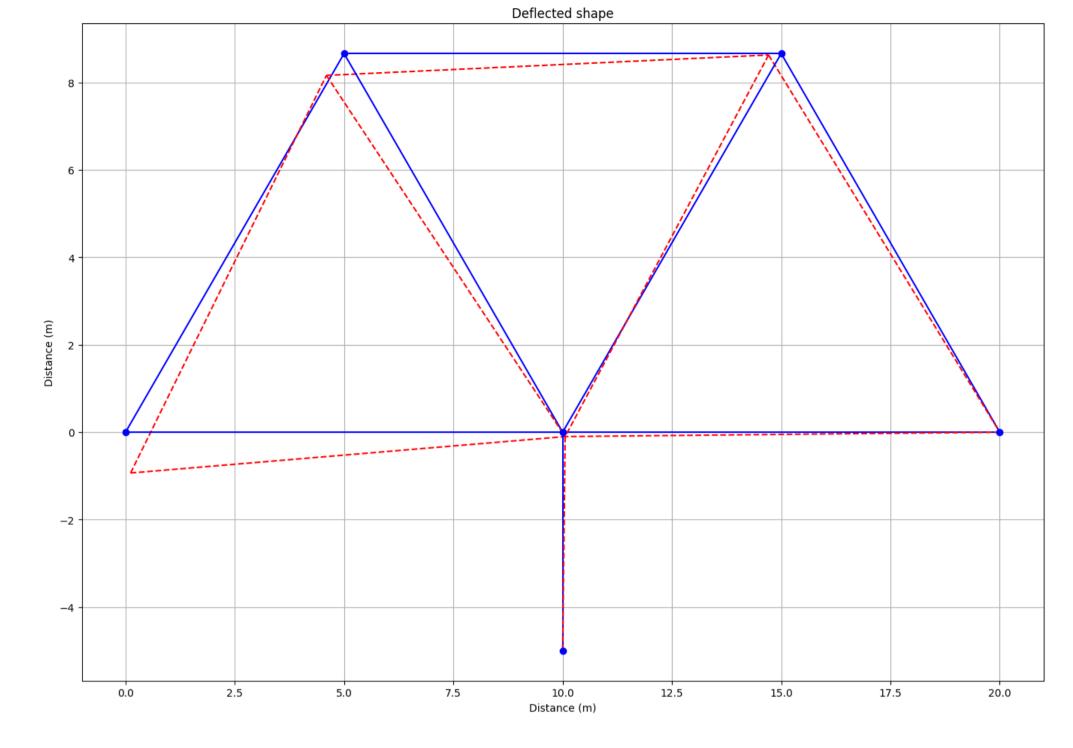
```
UG = np.array([UG]).T
FG = np.matmul(Kp,UG)
```

Solve for member forces

```
In [11]: mbrForces = np.array([]) #Initialise an array to hold member forces
         for n, mbr in enumerate(members):
             theta = orientations[n]
             mag = lengths[n]
             node_i = mbr[0] #Node number for node i of this member
             node_j = mbr[1] #Node number for node j of this member
             #Primary stiffness matrix indices associated with each node
             ia = 2*node_i-2
             ib = 2*node_i-1
             ja = 2*node_j-2
             jb = 2*node_j-1
             #Transformation matrix
             c = math.cos(theta)
             s = math.sin(theta)
             T = np.array([[c,s,0,0],[0,0,c,s]])
             disp = np.array([[UG[ia],UG[ib],UG[ja],UG[jb]]]).T #Global displacements
             disp_local = np.matmul(T,disp)[0] #Local displacements
             F_axial = (A*E/mag)*(disp_local[1]-disp_local[0]) #Axial Loads
             mbrForces = np.append(mbrForces,F_axial) #Store axial Loads
```

Plotting

```
In [12]: fig = plt.figure()
         axes = fig.add_axes([0.1,0.1,2,2])
         fig.gca().set_aspect('equal', adjustable='box')
         #Plot members
         for mbr in members:
             node_i = mbr[0] #Node number for node i of this member
             node_j = mbr[1] #Node number for node j of this member
             ix = nodes[node_i-1,0] #x-coord of node i of this member
             iy = nodes[node_i-1,1] #y-coord of node i of this member
             jx = nodes[node_j-1,0] #x-coord of node j of this member
             jy = nodes[node_j-1,1] #y-coord of node j of this member
             #Index of DoF for this member
             ia = 2*node_i-2 #horizontal DoF at node i of this member
             ib = 2*node_i-1 #vertical DoF at node i of this member
             ja = 2*node_j-2 #horizontal DoF at node j of this member
             jb = 2*node_j-1 #vertical DoF at node j of this member
             axes.plot([ix,jx],[iy,jy],'b') #Member
             axes.plot([ix + UG[ia,0]*xFac, jx + UG[ja,0]*xFac], [iy + UG[ib,0]*xFac, jy + UG[jb,0]*xFac],'--r') #Deformed member
         #Plot nodes
         for node in nodes:
             axes.plot([node[0]],[node[1]],'bo')
         axes.set_xlabel('Distance (m)')
         axes.set_ylabel('Distance (m)')
         axes.set_title('Deflected shape')
         axes.grid()
         plt.show()
```



Summary output

Node 6: Ux = 0.0 m, Uy = 0.0 m

```
In [13]: #Generate output statements
          print("REACTIONS")
          for i in np.arange(0,len(restrainedIndex)):
              index = restrainedIndex[i]
              print("Reaction at DoF {one}: {two} kN".format(one = index+1, two = round(FG[index].item()/1000,2)))
          print("")
          print("MEMBER FORCES")
          for n, mbr in enumerate(members):
              print("Force in member {one} (nodes {two} to {three}) is {four} kN".format(one = n+1, two=mbr[0], three=mbr[1], four=round(mbrForces[n]
          print("")
          print("NODAL DISPLACEMENTS")
          for n, node in enumerate(nodes):
              ix = 2*(n+1)-2 #horizontal DoF for this node
              iy = 2*(n+1)-1 #vertical DoF for this node
              ux = round(UG[ix,0],5) #Horizontal nodal displacement
              uy = round(UG[iy,0],5) #Vertical nodal displacement
              print("Node {one}: Ux = {two} m, Uy = {three} m".format(one=n+1, two=ux, three=uy))
        REACTIONS
        Reaction at DoF 7: 0.0 kN
        Reaction at DoF 8: -200.0 kN
        Reaction at DoF 11: 0.0 kN
        Reaction at DoF 12: 400.0 kN
        MEMBER FORCES
        Force in member 1 (nodes 1 to 2) is 230.94 kN
        Force in member 2 (nodes 2 to 3) is 230.95 kN
        Force in member 3 (nodes 3 to 4) is 230.94 kN
        Force in member 4 (nodes 4 to 5) is -115.47 kN
        Force in member 5 (nodes 1 to 5) is -115.47 kN
        Force in member 6 (nodes 2 to 5) is -230.94 kN
        Force in member 7 (nodes 3 to 5) is -230.94 kN
        Force in member 8 (nodes 5 to 6) is -400.0 kN
        NODAL DISPLACEMENTS
        Node 1: Ux = 0.00231 \text{ m}, Uy = -0.01867 \text{ m}
        Node 2: Ux = -0.00808 \text{ m}, Uy = -0.01 \text{ m}
        Node 3: Ux = -0.00577 \text{ m}, Uy = -0.00067 \text{ m}
        Node 4: Ux = 0.0 \text{ m}, Uy = 0.0 \text{ m}
        Node 5: Ux = 0.00115 \text{ m}, Uy = -0.002 \text{ m}
```