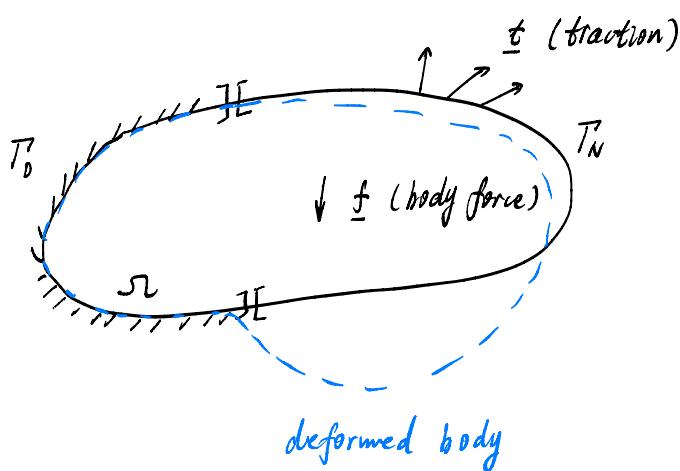


Lec 06 : Linear elasticity ; Spurious solutions

- Linear elasticity
- Incompressible elasticity (volume locking)
- Some other issues (hourgassing, shear locking)

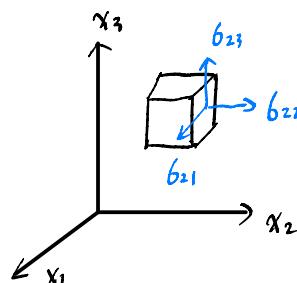
Linear elasticity

Strong form



$$\Omega \subset \mathbb{R}^{n_{sd}}$$

n_{sd} : spatial dimension (e.g., $n_{sd}=3$)



\underline{b} : Cauchy stress tensor

Problem statement: Find displacement $\underline{u}: \Omega \rightarrow \mathbb{R}^{n_{sd}}$ such that

$$\left\{ \begin{array}{l} \nabla \cdot \underline{b} + \underline{f} = 0 \text{ in } \Omega \text{ (equilibrium)} \\ \underline{u} = \underline{g} \text{ on } T_D \text{ (Dirichlet B.C.)} \\ \underline{n} \cdot \underline{b} = \underline{t} \text{ on } T_N \text{ (Neumann B.C.)} \end{array} \right. \quad \text{or} \quad \left\{ \begin{array}{l} b_{ij,i} + f_j = 0 \text{ in } \Omega \\ u_i = g_i \text{ on } T_D \\ n_i b_{ij} = t_j \text{ on } T_N \end{array} \right.$$

Constitutive law: $\underline{b} = \underline{\underline{C}} : \underline{\underline{\epsilon}}$ (or $b_{ij} = C_{ijkl} \epsilon_{kl}$)

Kinematic relation: $\underline{\underline{\epsilon}} = \frac{1}{2} (\underline{\underline{u}} \underline{\underline{\sigma}} + \underline{\underline{\sigma}} \underline{\underline{u}}) \quad (\text{or } \epsilon_{ij} = \frac{1}{2} (u_{i,j} + u_{j,i}) = u_{(i,j)})$

Let us assume the body is isotropic, then

$$C_{ijkl} = \mu (\delta_{ik} \delta_{jl} + \delta_{il} \delta_{jk}) + \lambda \delta_{ij} \delta_{kl} \Rightarrow b_{ij} = \lambda u_{kk} \delta_{ij} + 2\mu u_{(i,j)}$$

where λ and μ are the Lamé parameters, and μ is shear modulus often denoted by G . We can represent λ and μ using Young's modulus E and Poisson's ratio ν :

$$\lambda = \frac{\nu E}{(1+\nu)(1-2\nu)}, \quad \mu = G = \frac{E}{2(1+\nu)}$$

Weak form

Trial function space $S = \{\underline{u} \mid \underline{u} \in [H^1(\Omega)]^{N_{sd}}, \underline{u} = \underline{0} \text{ on } \Gamma_0\}$

This means $(u_1, u_2, u_3) \in H^1(\Omega) \times H^1(\Omega) \times H^1(\Omega)$ ($\underline{u}(x) = \sum_{i=1}^{N_{sd}} u_i(x) \underline{e}_i$)

Test function space $V = \{\underline{v} \mid \underline{v} \in [H^1(\Omega)]^{N_{sd}}, \underline{v} = \underline{0} \text{ on } \Gamma_0\}$

Start with $\int_{\Omega} (\underline{\sigma} \cdot \underline{v}) \cdot \underline{u} + \int_{\Omega} \underline{f} \cdot \underline{v} = 0 \quad \forall \underline{v} \in V$

$$\Rightarrow \int_{\Omega} \delta_{ij,i} v_j + \int_{\Omega} f_j v_j = 0 \Rightarrow \int_{\partial\Omega} u_i \delta_{ij} v_j - \int_{\Omega} \delta_{ij} v_{j,i} + \int_{\Omega} f_j v_j = 0$$

$$\Rightarrow \int_{\Omega} \delta_{ij} v_{j,i} = \int_{\Gamma_N} t_j v_j + \int_{\Omega} f_j v_j \quad \text{or} \quad a(\underline{v}, \underline{u}) = f(\underline{v}) \quad (\text{weak form})$$

Let us work on $\int_{\Omega} \delta_{ij} v_{j,i}$ a little further:

Note that $\underline{\sigma}$ is a symmetric tensor, i.e., $\sigma_{ij} = \sigma_{ji}$, then

$$\delta_{ij} v_{j,i} = \delta_{ij} v_{i,j} = C_{ijkl} u_{k,l} v_{i,j} = (\mu(\delta_{ik}\delta_{jl} + \delta_{il}\delta_{jk}) + \lambda \delta_{ij}\delta_{kl}) u_{k,l} v_{i,j}$$

$$= v_{i,j} \bar{C}_{ijkl} u_{k,l} + v_{i,i} \lambda u_{k,k}, \text{ where } \bar{C}_{ijkl} = \mu(\delta_{ik}\delta_{jl} + \delta_{il}\delta_{jk})$$

Therefore $a(\underline{v}, \underline{u})$ is a symmetric bilinear form.

Also, $b_{ij} v_{j,i} = v_{i,j} \bar{C}_{ijkl} u_{k,l} + v_{i,i} n u_{k,k}$ due to symmetry of \bar{C}_{ijkl} .

Galerkin approximation

Trial function space $S^h = \left\{ \underline{u}^h \in S \mid \underline{u}^h = \sum_{A=1}^{n_n} \underline{u}^A \phi^A(\underline{x}) \right\} \subset S$

Test function space $V^h = \left\{ \underline{v}^h \in V \mid \underline{v}^h = \sum_{A=1}^{n_n} \underline{v}^A \phi^A(\underline{x}) \right\} \subset V$

Where, n_n : number of total nodes ($\# \text{DoF} = n_n \times n_{sd}$)

ϕ^A : shape function (constructed just like before)

$\underline{u}^A = \sum_{i=1}^{n_{sd}} \underline{u}_i^A e_i$, \underline{u}_i^A : unknown DoF

Galerkin problem statement: Find $\underline{u}^h \in S^h$ s.t. $a(\underline{v}^h, \underline{u}^h) = \bar{F}(\underline{v}^h) \quad \forall \underline{v}^h \in V^h$

Matrix form

$$a\left(\sum_{A=1}^{n_n} \underline{v}^A \phi^A(\underline{x}), \sum_{A=1}^{n_n} \underline{u}^A \phi^A(\underline{x})\right) = \bar{F}\left(\sum_{A=1}^{n_n} \underline{v}^A \phi^A(\underline{x})\right) \quad \forall \underline{v}^h \in V^h$$

must be zero (matrix form)

\Leftrightarrow

$$\left\{ [\underline{v}^1]^T [\underline{v}^2]^T \cdots [\underline{v}^{n_n}]^T \right\} \underbrace{\begin{pmatrix} [\underline{K}^1] & [\underline{K}^{12}] & \cdots & [\underline{K}^{1n_n}] \\ [\underline{K}^{21}] & [\underline{K}^{22}] & \cdots & [\underline{K}^{2n_n}] \\ \vdots & \ddots & \vdots & \vdots \\ [\underline{K}^{n_n1}] & [\underline{K}^{n_n2}] & \cdots & [\underline{K}^{n_nn}] \end{pmatrix}}_{\substack{\uparrow \\ \text{arbitrary}}} - \begin{pmatrix} [\underline{F}^1] \\ [\underline{F}^2] \\ \vdots \\ [\underline{F}^{n_n}] \end{pmatrix} = 0$$

where $[\underline{v}^A]^T = [v_1^A, v_2^A], [v_1^B] = \begin{bmatrix} u_1^B \\ u_2^B \end{bmatrix}, [\underline{u}^B] = \begin{bmatrix} u_1^B \\ u_2^B \end{bmatrix}, [\underline{v}^A]^T [\underline{K}^{AB}] [\underline{u}^B] = a(\underline{v}^A \phi^A(\underline{x}), \underline{u}^B \phi^B(\underline{x}))$

$$\Rightarrow a(\underline{v}^A \phi^A(\underline{x}), \underline{u}^B \phi^B(\underline{x})) = \int_{\Omega} v_i^A \frac{\partial \phi^A}{\partial x_j} \bar{C}_{ijkl} \frac{\partial \phi^B}{\partial x_l} u_k^B + v_i^A \frac{\partial \phi^A}{\partial x_i} n \frac{\partial \phi^B}{\partial x_k} u_k^B$$

$$\Rightarrow K_{ik}^{AB} = \int_{\Omega} \frac{\partial \phi^A}{\partial x_j} \bar{C}_{ijkl} \frac{\partial \phi^B}{\partial x_l} + \frac{\partial \phi^A}{\partial x_i} n \frac{\partial \phi^B}{\partial x_k} \quad (\text{Can you find this integral?})$$

$$\text{Also, } [\underline{v}^A]^T [\underline{F}^A] = F(\underline{v}^A \phi^A(x)) = \int_{\Gamma^N} t_j v_j^A \phi^A + \int_{\Omega} f_j v_j^A \phi^A$$

$$\Rightarrow \bar{F}_i^A = \int_{\Gamma^N} t_i \phi^A + \int_{\Omega} f_i \phi^A$$

After imposing Dirichlet B.C., you may solve for $\begin{Bmatrix} [\underline{u}^1] \\ [\underline{u}^2] \\ \vdots \\ [\underline{u}^{n_u}] \end{Bmatrix}$.

Incompressible elasticity

Problem statement: Find displacement $\underline{u}(x)$ and pressure $p(x)$ such that

$$\left\{ \begin{array}{l} \nabla \cdot \underline{\underline{\epsilon}} + \underline{f} = \underline{0} \text{ in } \Omega \text{ (equilibrium)} \\ \nabla \cdot \underline{u} + p/\rho = 0 \text{ in } \Omega \text{ (constraint)} \\ \underline{u} = \underline{g} \text{ on } \Gamma_D \text{ (Dirichlet B.C.)} \\ \underline{n} \cdot \underline{\underline{\epsilon}} = \underline{t} \text{ on } \Gamma_N \text{ (Neumann B.C.)} \end{array} \right. \quad \text{or} \quad \left\{ \begin{array}{l} b_{ij,i} + f_j = 0 \text{ in } \Omega \\ u_{i,i} + p/\rho = 0 \text{ in } \Omega \\ u_i = g_i \text{ on } \Gamma_D \\ n_i b_{ij} = t_j \text{ on } \Gamma_N \end{array} \right.$$

$$\text{Note: } b_{ij} = \lambda \epsilon_{ijk} \delta_{ij} + 2\mu u_{(ij)} = -p \delta_{ij} + 2\mu u_{(ij)}$$

This problem statement works for

① Incompressible solid: $\lambda \rightarrow \infty$ ($\nabla \cdot \underline{u} = 0$)

② Compressible solid: λ is finite ($\nabla \cdot \underline{u} + p/\rho = 0$)

This (u, p) formulation will be the same as the (u) only formulation.

The weak form is to find $\underline{u} \in S$ and $p \in L^2(\Omega)$ such that

$$\int_{\Omega} v_{i,j} \bar{C}_{ijk} u_{k,\ell} - \int_{\Omega} v_{i,i} p - \int_{\Omega} g (u_{i,i} + p/\rho) = \int_{\Gamma_N} t_i v_i + \int_{\Omega} f_i v_i$$

for $\forall \underline{v} \in V$ and $g \in L^2(\Omega)$.

For Galerkin approximation, let $\underline{u}^h \in S^h$ and $\underline{p}^h \in V^h$ as usual.

$$P^h \in P^h = \left\{ p^h \in L^2(\Omega) \mid p^h = \sum_{\tilde{A}=1}^{\tilde{n}_h} p^{\tilde{A}} \psi^{\tilde{A}}(x) \right\}$$

$$q^h \in Q^h = \left\{ q^h \in L^2(\Omega) \mid q^h = \sum_{\tilde{A}=1}^{\tilde{n}_h} q^{\tilde{A}} \psi^{\tilde{A}}(x) \right\} \quad (\text{ } p^h \text{ and } q^h \text{ are the same})$$

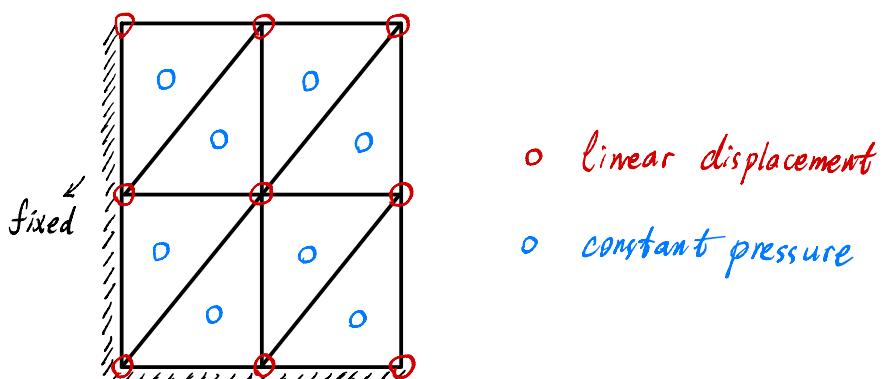
Show (in homework) that the matrix form is :

$$\begin{matrix} 2n_n \\ \tilde{n}_n \end{matrix} \left\{ \underbrace{\begin{bmatrix} K & G \\ G^T & M \end{bmatrix}}_{2n_n} \underbrace{\begin{bmatrix} u \\ p \end{bmatrix}}_{\tilde{n}_n} \right\} = \begin{bmatrix} \bar{F} \\ 0 \end{bmatrix} \right\}_{\tilde{n}_n} \quad \text{find } K_{ij}^{AB}, G_i^{AB}, M^{AB}, \bar{F}_i^A$$

Volume locking

We will illustrate a fundamental difficulty using a simple example about incompressible solid. The example will show that you cannot arbitrarily choose approximate function spaces for u and p (S^h and P^h).

Consider a mesh :



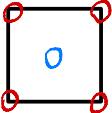
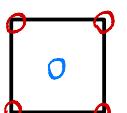
For incompressibility, we have $n \rightarrow \infty$ and $\int_{\Omega} q^h \nabla \cdot \underline{u}^h = \sum_{e=1}^{N_{el}} \int_{\Omega^e} q^h \nabla \cdot \underline{u}^h = 0$,

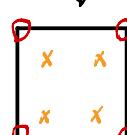
where q^h is an arbitrary constant on each triangle.

$\Rightarrow \int_{\Omega^e} \nabla \cdot \underline{u}^h = 0$ for any element Ω^e . This means the area of each triangle must NOT change.

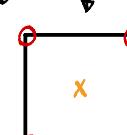
In this example, it further implies that all displacement nodes \bullet must be zero. We call this volume locking (or mesh locking).

Remarks:

- * Changing to a suitable pair of finite elements (e.g.,  bilinear displacements and constant pressure) can resolve the volume locking issue.
- * A systematic study is beyond the scope of this course. Refer to the Babuška-Brezzi (or LBB) condition.
- * In the nearly incompressible case (λ is finite and a very large number), the locking issue does NOT go away.
 - You may select a suitable scheme of finite elements (e.g., )
 - Alternatively, you may use the (u) only formulation and apply the "selective reduced integration" scheme:

$$K_{ik}^{AB} = \int_{\Omega} \frac{\partial \phi^A}{\partial x_j} \bar{C}_{ijkl} \frac{\partial \phi^B}{\partial x_l} + \int_{\Omega} \frac{\partial \phi^A}{\partial x_i} \lambda \frac{\partial \phi^B}{\partial x_k}$$


2×2 Gaussian quadrature



One-point Gaussian quadrature

- It has been shown that the two strategies above are equivalent. See details in Fig 4.4.3 in Hughes book.

Some other issues

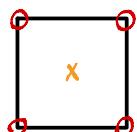
We will discuss some other difficulties of the Galerkin FEM and possible remedies.

N hourglassing

Reduced integration may lead to spurious zero-energy modes.

Consider a 2D bilinear elasticity element, using one-point Gaussian quadrature

for integration:



One-point Gaussian quadrature

Elastic energy can be defined as $\bar{E} = \frac{1}{2} a(\underline{u}, \underline{u})$

$$\text{Then, } \frac{1}{2} a(\underline{u}^h, \underline{u}^h) = \frac{1}{2} \int_{\Omega} (u_{i,j}^h \bar{C}_{ijkl} u_{k,l}^h + u_{i,i}^h \eta u_{k,k}^h) dx$$

$$\approx \frac{1}{2} \left(u_{i,j}^h \bar{C}_{ijkl} u_{k,l}^h + u_{i,i}^h \eta u_{k,k}^h \right) \Bigg|_{\underline{\xi}=0} \cdot j(\underline{\xi}=0) \cdot 4$$

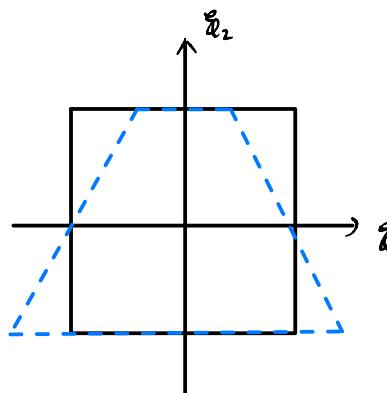
↑ weight

reminder:

$$\underline{u}^h = \sum_{A=1}^{n_n} \underline{u}^A \phi_A^A(\underline{x})$$

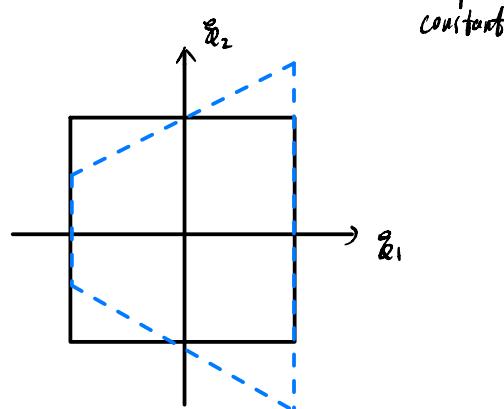
$$u_{i,j}^h = \sum_{A=1}^{n_n} u_i^A \frac{\partial \phi_A^A}{\partial x_j}$$

Hourglass mode I: $\underline{u}^h(\underline{\xi}) = C \underline{\xi}_1 \underline{\xi}_2 \underline{e}_1$



parametric domain
deformed shape

Hourglass mode II: $\underline{u}^h(\underline{\xi}) = C \underline{\xi}_1 \underline{\xi}_2 \underline{e}_2$



parametric domain
deformed shape

Note: $u_{i,j}^h = \frac{\partial u_i^h}{\partial x_j} = \frac{\partial u_i^h}{\partial \xi_k} \frac{\partial \xi_k}{\partial x_j} \Bigg|_{\underline{\xi}=0} = 0$ for $1 \leq i, j \leq n_{sd}$ for both mode I and II

Therefore $\bar{E}^h = \frac{1}{2} a(\underline{u}^h, \underline{u}^h) = 0$ for the two modes.

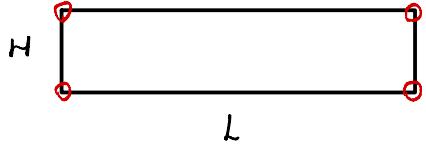
Why it is called hourgassing? Such deformation creates no energy!



The element is considered as too "soft". But we still want to use one-point integration due to efficiency, see Hughes book Section 4.8 for the Kostoff-Frazier element as a solution.

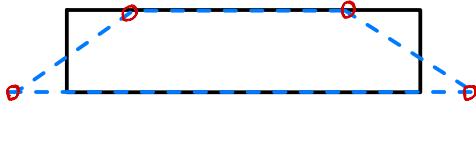
Shear locking

Sometimes the standard Galerkin elements can be too "stiff". One such case occurs for bending of a typical 2D bilinear elasticity element



element (physical domain)

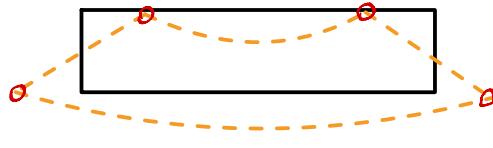
$\frac{L}{H} \gg 1$ makes situation worse



element response

$$\underline{u}^h(\underline{x}) = C \begin{pmatrix} x_1 & x_2 \end{pmatrix} \underline{\epsilon}_1$$

↑
constant



correct response

bending-like

Solution to this challenge: Enrich the element with more modes (see Hughes book Section 4.7)