

GENERAL STRUCTURAL NOTES

PART 1 - GENERAL REQUIREMENTS AND DESIGN CRITERIA

1.1 SPECIFICATIONS

- A. Unless otherwise noted, details, sections and notes contained in the structural contract documents shall be considered typical for all similar conditions even if not explicitly referenced.
- B. Deficient work and/or work not in conformance with the contract documents shall be repaired at the contractor's expense. The contractor shall compensate the client for services arising from deficient work, review of modifications/contractor substitution, or expediting of submittals.
- C. Cost of investigation and/or redesign incurred by the Engineer of Record due to contractor errors will be at the contractor's expense.
- D. The contractor shall submit a single dimensioned and coordinated drawing for each level showing the locations of all sleeves and openings required by all trades prior to initiating any work.
- E. Loads imposed on the base building structure and temporary conditions intended to accommodate construction means and methods are not explicitly considered in this design. The contractor shall advise the Engineer of Record regarding construction loads and temporary conditions imposed on the building structure and shall compensate the Engineer of Record for reviewing these conditions.
- F. Structure is designed to be stable in the final condition with all elements in place. Contractor and Contractor's Engineer shall verify strength and stability during construction, and provide temporary bracing as required.
- G. Vehicular barrier required as shown in drawings.

1.2 ELEVATIONS & DIMENSIONS

- A. All dimensions, elevations and conditions shall be verified in the field by the contractors and any discrepancies shall be brought to the attention of the engineer for clarification before proceeding with the affected part of the work. Dimensions and elevations noted in the contract documents as (+/-) and all field conditions shall be verified in the field (V.I.F.) by the contractors prior to the submissions of shop drawings. Upon receipt of shop drawings, the engineer has the right to assume that all field dimensions, elevations and conditions have been verified by the contractors and that the shop drawings accurately reflect such verifications unless stated otherwise on the shop drawings.

1.3 BUILDING CODE AND REFERENCED STANDARDS

- A. Connecticut State Building Code (CTSBC), 2018 with Current Revisions
- B. ASCE/SEI 7-10 Minimum Design Loads for Buildings and Other Structures

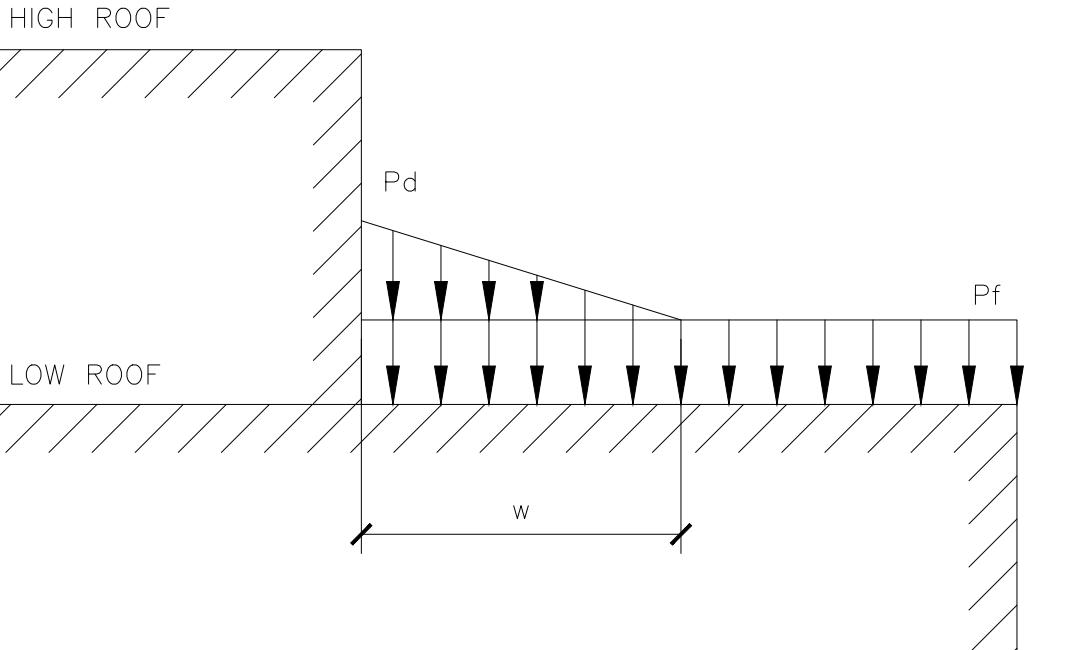
1.4 DESIGN LOADS

- A. Dead Loads All permanent stationary construction.

- B. Floor Live loads (See load maps)

C. Roof Snow Load Parameters

- Where appropriate, drifting snow loads have been considered in accordance with Section 1608 of the Building Code
- | | |
|------------------------------------|--------|
| 1. Ground Snow Load, Pg | 35 psf |
| 2. Flat Roof Snow Load, Pf | 21 psf |
| 3. Snow Exposure Factor, Ce | 0.9 |
| 4. Snow Load Importance Factor, IS | 1.0 |
| 5. Thermal Factor, Ct | 1.0 |
| 6. Drift Surcharge, Pd | 51 psf |
| 7. Drift Width, w | 7 ft |



D. Wind Load Parameters

- 1. Basic Wind Speed (3 sec gust) 120 mph
- 2. Risk Category II
- 3. Internal Pressure Coefficient ±0.18
- 4. East-West Direction, Wind Force Resisting System
 - a. Wind Exposure Category B
 - b. Design Base Shear, V 42 kips
- 5. North-South Direction, Wind Force Resisting System
 - a. Wind Exposure Category B
 - b. Design Base Shear, V 56 kips
- 6. Components & Cladding (See load maps)

E. Seismic Load Parameters

- 1. Seismic Importance Factor, IE 1.0
- 2. Spectral Response Acceleration, Ss 0.197 g
- 3. Spectral Response Acceleration, S1 0.065 g
- 4. Site Class C
- 5. Spectral Response Coefficient, SDS 0.157 g
- 6. Spectral Response Coefficient, SD1 0.074 g
- 7. Seismic Design Category B
- 8. Analysis Procedure
 - a. Equivalent Lateral Force
- 9. East-West Direction, Seismic Force Resisting System
 - a. Steel System Non Specifically Detailed for Seismic Resistance
 - b. Design Base Shear, V 43 kips
 - c. Seismic Response Coefficient, CS 0.048
 - d. Response Modification Factor, R 3.00
- 10. North-South Direction, Seismic Force Resisting System
 - a. Steel System Non Specifically Detailed for Seismic Resistance
 - b. Design Base Shear, V 43 kips
 - c. Seismic Response Coefficient, CS 0.048
 - d. Response Modification Factor, R 3.00

1.5 LATERAL LOAD RESISTING SYSTEM

- A. All lateral load resistance and stability of the building in the completed structure is provided by concentric steel braces in each orthogonal direction (see framing plans, S-100 series, for locations). See lateral system on S-200 series for elevations and member forces. The proposed CIP concrete slab and CIP concrete slab on metal deck will serve as horizontal diaphragms that distribute the wind and seismic forces horizontally to the braces, which carry the applied lateral loads to the building foundation.

PART 2 - FOUNDATIONS

- 2.1 REFERENCE GEOTECHNICAL REPORT: Foundation design is in accordance with the recommendations provided in Report of Geotechnical Engineering, 2 Old Tophet Road, Roxbury, CT by Down to Earth Consulting, LLC dated March 26th, 2021.

2.2 FOUNDATION DESIGN PARAMETERS

- A. Soil capacity: Footing and Slab on Grade shall bear on soil with a minimum bearing capacity of:

1. 3TSF

- B. Refer to foundation schedules for reference bearing capacity implemented in the design of the foundation element.

- 2.3 EXCAVATION: All foundation excavation to be inspected by the Geotechnical Engineer. The elevations shown on the drawings are anticipated and actual elevations are to be established in the field by the Geotechnical Engineer, but in no case shall the bottom of the footing, mat foundation, or pile cap be located less than [3.5] feet below the lowest adjacent surface exposed to freezing.

- 2.4 BACKFILL UNDER SLAB ON GRADE: Backfill where required below slabs with approved granular soil placed in 6 in. layers and compacted to 95% density at optimum moisture content as defined by ASTM D-1557, Method D.

- 2.5 BACKFILL AGAINST WALLS: Do not backfill against walls until wall concrete is at full design strength and all permanent horizontal construction is in place and at full design strength

- Backfill with approved material placed in 6 in. layers and compacted to 95% density at optimum moisture content as defined by ASTM D-1557, Method D.

- 2.6 FOUNDATION PLACEMENT & PROTECTION: Do not place foundation concrete in water or on frozen ground. Protect in-place foundations and slabs from frost penetration until the project is complete. Do not use salt or chloride compounds to de-ice the site.

2.7 UNDERPINNING

- A. Where underpinning of the adjacent existing building foundation is required, use the "pit method." Contractor to provide engineering and means and methods for underpinning scheme. Underpinning shop drawings and calculations shall be signed and sealed by a Professional Engineer Licensed in the State of Connecticut and shall be submitted to the Engineer of Record for review. Contractor's Engineer to provide special inspections required by the CTSBC. Contractor's Engineer to be hired directly by Owner for special inspections.

2.8 TEMPORARY SHORING

- A. Contractor to provide engineering and means and methods for temporary shoring and/or bracing. Temporary shoring shop drawings and calculations shall be signed and sealed by a Professional Engineer licensed in the State of Connecticut and shall be submitted to the Engineer of Record for review.

PART 3 - CONCRETE WORK

3.1 CONCRETE MIX PROPERTIES

A. Element (Normal Weight, UON)

	28 day strength	W/C	Air Content (Max.)
1. Footings/Mats	5,000 psi	0.40	6%+/- 1.5
2. Grade Beams	5,000 psi	0.40	6%+/- 1.5
3. Concrete Slab-on-Grade	5,000 psi	0.40	6%+/- 1.5
4. Concrete on Metal Deck	4,000 psi	0.45	less than 3%

B. Portland Cement:

- 8. ASTM C150, Type II

- 9. When concrete is in contact with soil, the type of exposure shall determine the cement type:

- 9.1. High Sulfate Exposure: Type V plus pozzolan

- 9.2. Moderate Sulfate Exposure: Type II

- 10. The contractor shall determine the level of exposure by testing or other suitable means.

C. Density:

- 1. Lightweight = 115 pcf

- 2. Normal Weight = 150 pcf

D. Fly Ash:

- 1. ASTM C618, Class F

- 2. Shall not exceed a maximum of 25 percent the total cementitious material by mass.

E. Chloride Ion Content:

- 1. Maximum water-soluble ion content shall not exceed a maximum of 0.15 percent by weight of cement determined by ASTM C1218 at age between 28 and 42 days.

3.2 BASE PLATE GROUT: 10,000 psi 28-day compressive strength.

3.3 STEEL REINFORCEMENT

- E. ASTM A615 Grade 60, deformed.

- F. ASTM A497 welded wire reinforcement (Use flat sheets only).
- G. Provide #6 chair bars, high chairs, ties, clips, slab bolsters and other accessories where not specified on the drawings in accordance with Manual of Standard Practice or Detailing Reinforcing Concrete Structures ACI 315 or CRSI-WRSI Manual of Standard Practice. Use plastic tips on all chairs placed on the sides of concrete formwork.

- H. Do not tack or spot-weld crossing bars.

- I. All steel elements exposed to weather should utilize epoxy coated or galvanized rebar.

3.4 REINFORCEMENT AT OPENINGS

- A. U.O.N. provide 2 - #6 at each side of openings in walls and slabs
- B. Extend 2'-6" beyond the opening or as detailed, except vertical bars at sides of openings in walls are to extend from floor to floor
- C. Bars may be moved aside at openings or sleeves, but do not cut or omit.

3.5 SPLICING OF REINFORCEMENT:

- B. As shown on drawings but not less than 65 bar diameters for walls.
- C. Provide a lap of 8 in or 1 1/2 spaces, whichever is larger, for W.W.R.
- D. Tie wires together at lap.

3.6 MINIMUM REINFORCEMENT

- A. Reinforce wall with at least 0.0015 times the area of concrete and 0.0025 times the area of concrete in the longitudinal and horizontal directions, respectively. Reinforcement may be equally divided between layers at each face in the same direction, not exceed 1/8 of the total reinforcement for each direction at the exterior face.
- B. In slabs, provide at least 0.0018 times the area of concrete in each direction.

3.7 REINFORCEMENT SHOP DRAWINGS

- A. Shop drawings shall be submitted to the Structural Engineer for review and approval, no concrete work shall commence without approved shop drawings.
- B. Location of all construction joints not shown in drawings shall be submitted to Structural Engineer for approval prior to detailing of reinforcing. All construction joints to be clearly shown in rebar shop drawings. Structural Engineer may require additional reinforcing at construction joints.
- C. Conduit locations not indicated in the Structural Drawings must be submitted for approval to the Structural Engineer prior to placement.

3.8 MINIMUM CONCRETE CLEAR COVER

- A. Concrete placed against earth: 3 in.
- B. Slabs-on-grade bottom: 2 in.
- C. Slabs-on-grade top: 2 in.
- D. Pavement slabs top: 2 in.
- E. Formed concrete exposed to earth, water, or weather: 2 in.
- F. Formed slabs, top and bottom: 1 in.
- G. Interior faces of walls: 1 in.
- H. Columns or piers (main reinforcement): 2 in.

3.9 POST-INSTALLED ANCHORS

- A. Expansion Anchors: Hilti HY-200.
- 1. Install per Hilti installation recommendations.
- 2. Provide standard depth of embedment as listed by Hilti, U.O.N.
- 3. Provide Stainless Steel anchors and hardware in all exterior applications.

3.10 STANDARD SPECIFICATIONS AND REFERENCE STANDARDS

- A. ACI 318-14 Building Code Requirements for Structural Concrete
- B. CRSI Manual of Standard Practice
- C. Follow the latest recommendations and specifications of the American Concrete Institute:
 - 1. ACI 302 Concrete Floor and Slab Construction
 - 2. ACI 304 Measuring, Mixing, Transporting and Placing Concrete
 - 3. ACI 305 Hot Weather Concreting
 - 4. ACI 306 Cold Weather Concreting
 - 5. ACI 315 Detailing Reinforcing Steel
 - 6. ACI 318 General Design of Items Not Otherwise Specified
 - 7. ACI 347 Formwork

3.11 HOUSEKEEPING PADS AND CURBS:

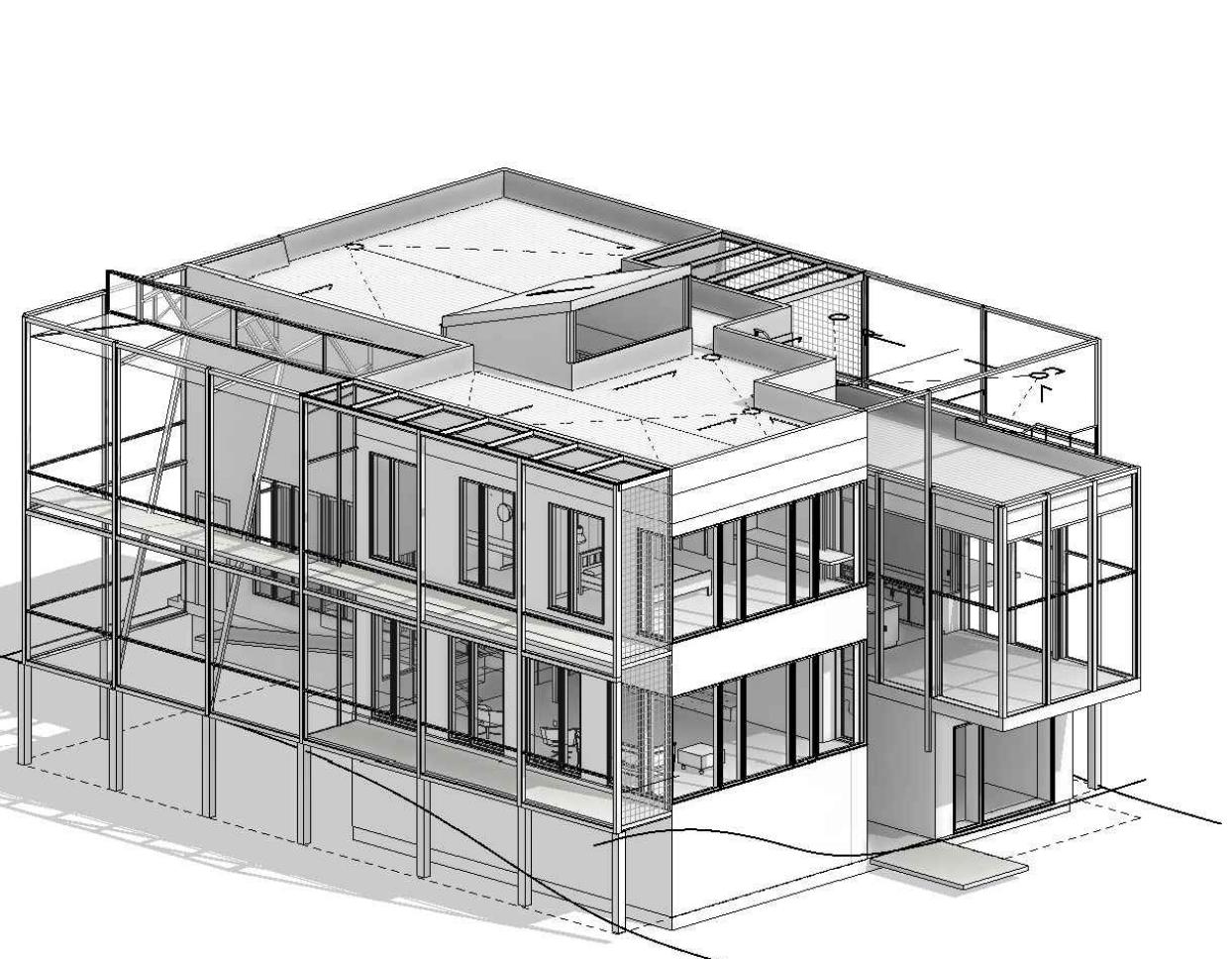
- A. Pads and curbs may be shown on plan in certain instances for reference only.
- See Architectural and Mechanical Drawings and Specifications and coordinate with equipment manufacturer's requirements and location.

- B. Use same concrete as base slab U.O.N.

- C. Maximum pad thickness is 6 inches.

4.1 ROD FOR BRACES

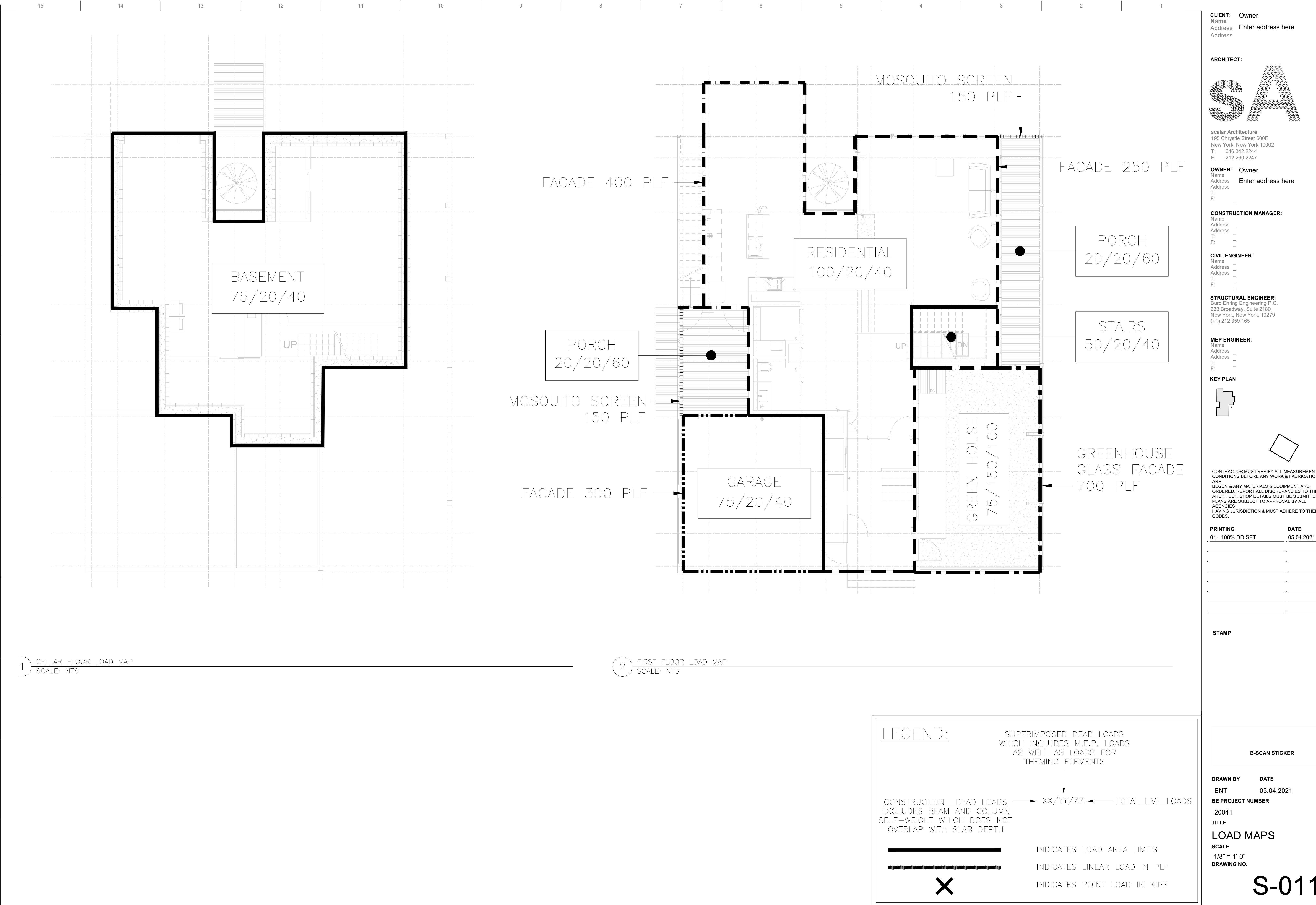
- A. ASTM A36

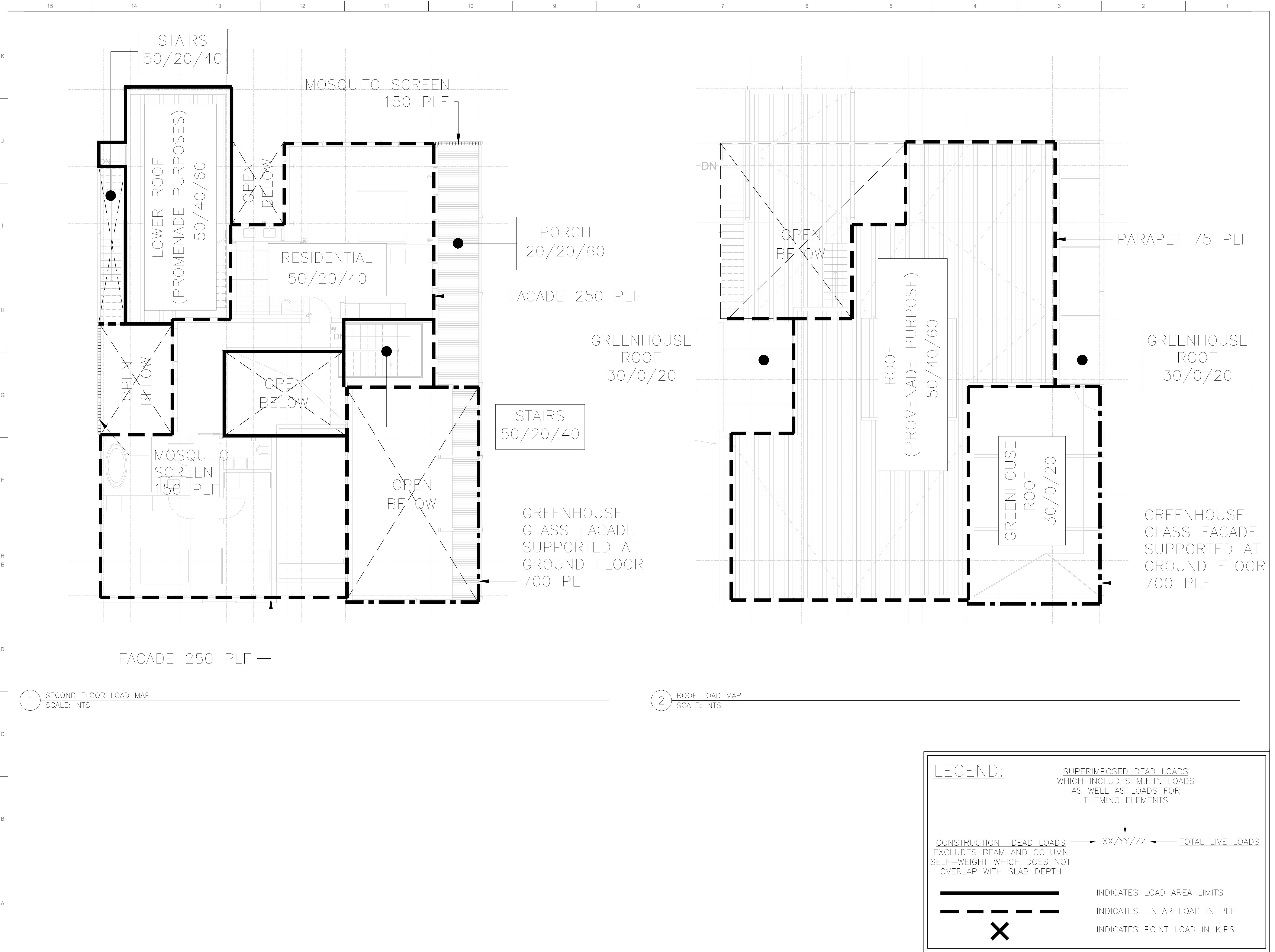


DRAWING LIST

DATE	ISSUE	SHEET	DRAWING No.	DRAWING TITLE
06/29/21	100% DD SET	1	S-001	GENERAL NOTES
09/04/21	100% DD SET	2	S-002	GENERAL NOTES CONT'D
12/08/20	PROGRESS SD SET	3	S-011	LOAD MAPS
	ISSUE	4	S-012	LOAD MAPS
		5	FO-101	FOUNDATION PLAN
		6	FO-111	FOUNDATION TYPICAL DETAILS
		7	FO-112	FOUNDATION TYPICAL DETAILS
		8	FO-113	FOUNDATION TYPICAL DETAILS
		9	FO-114	FOUNDATION TYPICAL DETAILS
		10	FO-115	FOUNDATION TYPICAL DETAILS
		11	FO-121	FOUNDATION TYPICAL SECTIONS
</				

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PART 4 - STRUCTURAL STEEL	PART 6 - SPECIAL INSPECTIONS													
4.1 STRUCTURAL SHAPES	6.1 Structural Special Inspections per CTSBC:													
A. Wide Flange Shapes	ASTM A992 (Fy = 50 ksi)	Special Inspection	CTSBC Reference											
B. Hollow Structural Sections	ASTM A500, Gr. C (Fy = 50 ksi)			Structural Steel - Details	1705.2.1									
C. Angles	ASTM A36, U.O.N. (Fy = 36 ksi)			Structural Steel - High Strength Bolting	1705.2.1									
D. Channels	ASTM A36, U.O.N. (Fy = 36 ksi)			Structural Steel - Welding	1705.2.1									
E. Plate:	ASTM A36, U.O.N. (Fy = 36 ksi)			Structural Cold-Formed Steel	1705.2.5									
4.2 STRUCTURAL PIPE				Concrete - Cast-In-Place	1705.3									
A. ASTM A53, Type E, Grade B or ASTM A501(Fy = 42 ksi)				Masonry	1705.4									
4.3 BOLTED CONNECTIONS				Concrete Design Mix	1705.3									
A. ASTM A325 and A490.				Concrete Sampling and Testing	1705.3									
4.4 ANCHOR RODS				Subgrade Inspection	1705.6									
A. ASTM F1554 Grade 55 bolts (U.O.N.) with Supplementary Requirement S1 (weldability).				Subsurface Conditions -	1705.6									
4.5 RODS FOR BRACES				Fill Placement & In Place Density										
A. ASTM A36				Post-Installed Anchors	1705.3									
4.6 WELDING ELECTRODES														
A. Conform to AWS Specifications for electrodes based on welding process and the type and grade of steel. E70XX electrodes (MIN.) for fillet welds.														
4.7 FABRICATION														
A. Shop fabricate to greatest extent possible by welding including beam stiffeners, column caps and bases, holes and connections.														
B. Submit complete shop drawings from field dimensions for the Architect's approval of all structural steel prior to fabrication.														
4.8 ERECTION														
A. Provide anchor rods, steel wedges, threaded screws or shims to support and plumb all columns.														
B. Grout solid under base plates immediately after columns are plumb.														
C. Provide bearing plates and wall anchors or anchor rods for all beams resting on concrete and all other necessary connecting hardware.														
D. Set anchor rods using template.														
E. Do not field cut or field modify any structural steel without prior written approval by architect for each specific case.														
4.9 PAINT														
A. Shop prime all steel not encased in concrete or not fireproofed.														
B. See Architectural Drawings and Specifications for finish coat requirements.														
4.10 FRAMING														
A. Beams are equally spaced and installed with natural camber up, U.O.N.														
B. Cantilever beams are same size as back span, U.O.N.														
C. Connections shown are schematic only and are to be designed by the fabricator unless explicitly indicated otherwise.														
4.11 STANDARD SPECIFICATIONS														
A. AISC 2005 Specification for Structural Steel Buildings														
B. AISC Code of Standard Practice for Steel Buildings and Bridges														
1. AWS D1.1 Structural Welding Code - Steel														
PART 5 - STEEL DECK AND SHEAR STUDS														
5.1 STEEL DECK														
A. Provide steel deck made from galvanized steel with minimum yield strength of 35 ksi.														
B. See Drawings and Specifications for gauge and profile.														
C. Provide sheet metal pour stops with thickness based on SDI criteria (SDI Publication #29); 14 gauge min. thickness.														
D. All Steel Deck and supporting members are sized and spaced assuming at least a two span condition for the metal deck. The steel deck supplier, installer and general contractor shall coordinate installation and shoring requirements for single span deck.														
5.2 SHEAR STUDS														
A. Provide headed type studs which conform to ASTM A108 Grade 1015 or 1020 cold finished carbon steel.														
B. Provide $\frac{3}{4}$ in. diameter by 5 in. long studs, U.O.N.														
C. See the drawings for number and locations of studs.														
D. Space studs uniformly along length of beam, U.O.N.														
E. Provide a minimum of 1 in. from the edge of any stud and the face of concrete, a metal deck rib or similar discontinuity.														
5.3 STANDARD SPECIFICATIONS														
A. AISC Specifications per PART.														
B. AISI Specification for the Design of Cold-Formed Steel Structural Members														
C. SDI Code of Recommended Practice and Specifications for Composite Steel Floor Deck														
D. AWS Structural Welding Code - Steel and Structural Steel Welding Code - Sheet Steel														
PART 6 - SPECIAL INSPECTIONS														
6.1 Structural Special Inspections per CTSBC:														
Special Inspection	CTSBC Reference													
Structural Steel - Details	1705.2.1													
Structural Steel - High Strength Bolting	1705.2.1													
Structural Steel - Welding	1705.2.1													
Structural Cold-Formed Steel	1705.2.5													
Concrete - Cast-In-Place	1705.3													
Masonry	1705.4													
Concrete Design Mix	1705.3													
Concrete Sampling and Testing	1705.3													
Subgrade Inspection	1705.6													
Subsurface Conditions -														
Fill Placement & In Place Density														
Post-Installed Anchors	1705.3													
PART 7 - CONSTRUCTION NOTES														
7.1 General Notes														
7.2 Foundation Notes														
7.3 Column Notes														
7.4 Beam Notes														
7.5 Deck Notes														
7.6 Shear Stud Notes														
7.7 Column Bracing Notes														
7.8 Column Splices Notes														
7.9 Column Anchors Notes														
7.10 Column Ties Notes														
7.11 Column Shoring Notes														
7.12 Column Splices Notes														
7.13 Column Anchors Notes														
7.14 Column Ties Notes														
7.15 Column Shoring Notes														
7.16 Column Splices Notes														
7.17 Column Anchors Notes														
7.18 Column Ties Notes														
7.19 Column Shoring Notes														
7.20 Column Splices Notes														
7.21 Column Anchors Notes														
7.22 Column Ties Notes														
7.23 Column Shoring Notes														





The logo for CHITECT consists of two main elements. On the right is a large, hollow letter 'A' formed by a grid of smaller 'A' characters, creating a recursive fractal effect. To the left of this 'A' is a large, solid letter 'S' also composed of a grid of smaller 'S' characters, mirroring the structure of the 'A'. The entire logo is rendered in a high-contrast black and white style.

Bar Architecture
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New York, New York 10002
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STRUCTURAL ENGINEER: BÜRO EHRING

Ehring Engineering P.C.
Broadway, Suite 2180
New York, New York, 10279
212 359 165



TRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE UN & ANY MATERIALS & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT. SHOP DETAILS MUST BE SUBMITTED. INS ARE SUBJECT TO APPROVAL BY ALL AGENCIES IN JURISDICTION & MUST ADHERE TO THEIR DES.

INTING DATE
100% DD SET 05-04-2024

100% DD SET 05.04.2021

AMB

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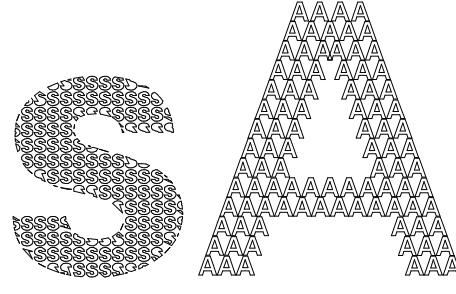
<u>LEGEND:</u>	SUPERIMPOSED DEAD LOADS WHICH INCLUDES M.E.P. LOADS AS WELL AS LOADS FOR THEMING ELEMENTS	B-SCAN STICKER
<u>CONSTRUCTION DEAD LOADS</u> EXCLUDES BEAM AND COLUMN SELF-WEIGHT WHICH DOES NOT OVERLAP WITH SLAB DEPTH	XX/YY/ZZ	DRAWN BY DATE ENT 05.04.2021
	INDICATES LOAD AREA LIMITS	BE PROJECT NUMBER 20041
	INDICATES LINEAR LOAD IN PLF	TITLE LOAD MAPS
	INDICATES POINT LOAD IN KIPS	SCALE 1/8" = 1'-0"
		DRAWING NO. S-012

S-012

15 | 14 | 13 | 12 | 11 | 10 | 9 | 8 | 7 | 6 | 5 | 4 | 3 | 2 | 1

CLIENT: Owner
Name Enter address here
Address

ARCHITECT:



scalar Architecture
195 Chrystie Street 600E
New York, New York 10002
T: 646.342.2244
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OWNER: Owner
Name Enter address here
Address
T:
F:

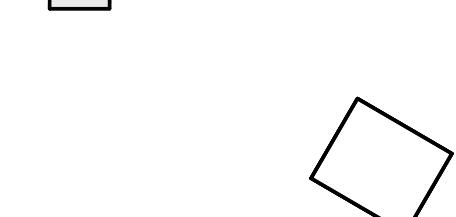
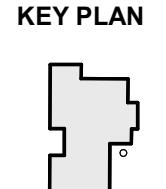
CONSTRUCTION MANAGER:
Name Address
Address
T:
F:

CIVIL ENGINEER:
Name Address
Address
T:
F:

STRUCTURAL ENGINEER:
Buro Happold Engineering P.C.
233 Broadway, Suite 2180
New York, New York, 10279
(+1) 212 359 165

MEP ENGINEER:
Name Address
Address
T:
F:

KEY PLAN



CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE BEGUN & ANY MATERIALS & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT. SHOP DETAILS MUST BE SUBMITTED. PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

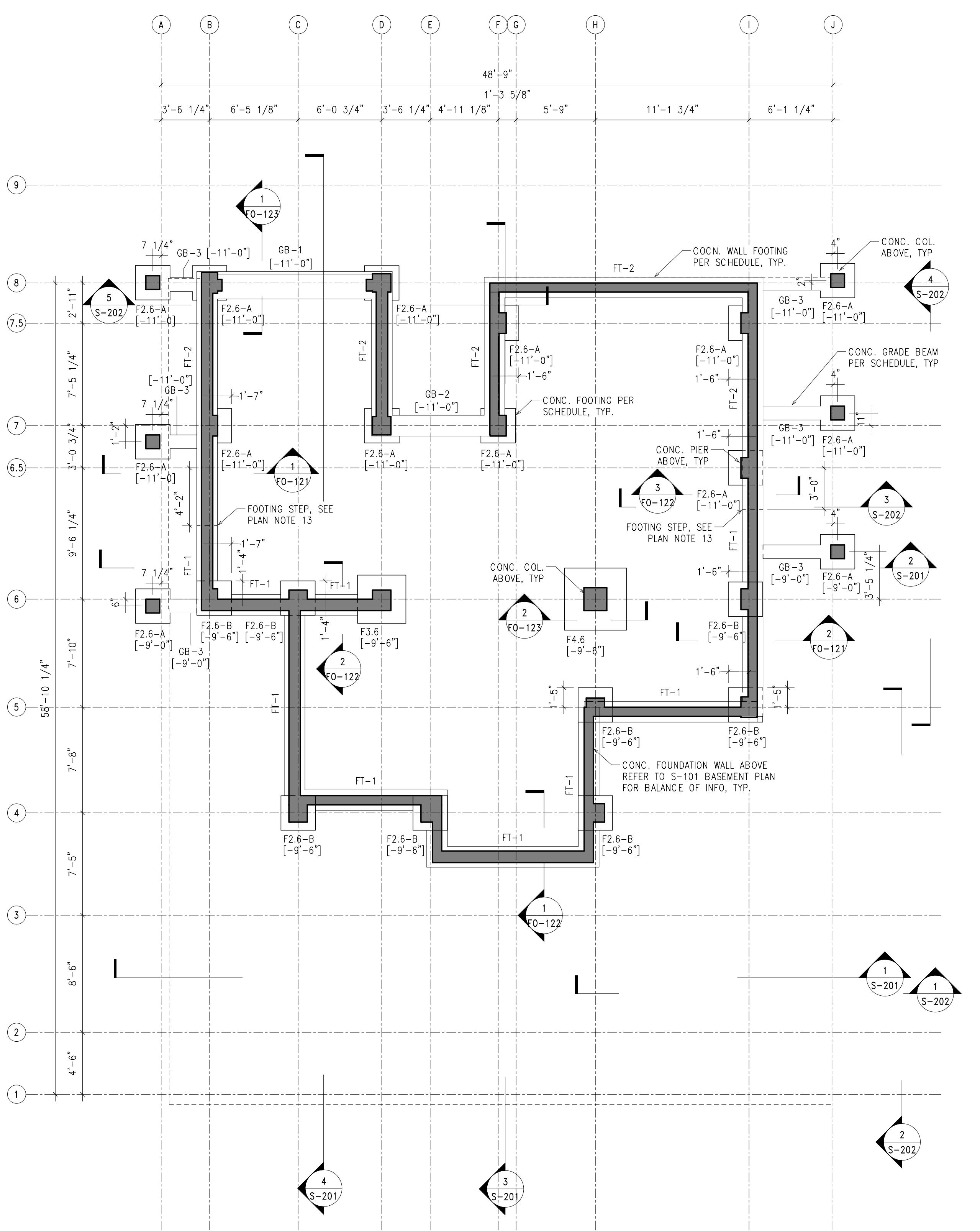
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01 - 100% DD SET 05.04.2021

STAMP

B-SCAN STICKER

FOUNDATION PLAN

3/16" = 1'-0"



TOP OF SLAB ELEVATION U.O.N.:

AS NOTED

SLAB THICKNESS:

AS NOTED

SLAB CONCRETE STRENGTH:

f'c = 4,000 PSI

SLAB BASIC BARS (E.W., U.O.N.):

5"-THICK S.O.G.: 6.0X6.0 W5.5XW5.5 WWR

PLAN NOTES:

- 1 ALL ELEVATIONS [xx'-xx"] CALLED OUT ON PLAN REFER TO PROJECT ELEVATION.
- 2 TOP OF BASEMENT SLAB AND MAT SLAB EL SHOWN ON PLAN.
- 3 SEE PLAN FOR REINFORCEMENT IN ADDITION TO TYPICAL REINFORCEMENT. PLACE ADDITIONAL REINFORCEMENT BETWEEN LAYERS OF TYPICAL REINFORCEMENT U.O.N.
- 4 (T) DENOTES TOP SLAB REINFORCEMENT. (B) DENOTES BOTTOM SLAB REINFORCEMENT. [xx'-xx"] IN FOUNDATION TAG INDICATES ELEVATION OF BOTTOM OF FOOTING.
- 5 FOR COLUMN SCHEDULE, SEE DRAWING S-300 SERIES DWGS.
- 6 ALL CONDUIT AND PIPELINE EMBEDDED IN MAT SHAL RUN BELOW TOP REINFORCEMENT. FOR ADDITIONAL REINFORCING AT FLOOR DRAINS OR CONDUIT BOXES THAT INTERRUPT REINFORCING, SEE TYPICAL FOUNDATION DETAILS.
- 7 PROVIDE EPOXY COATED TOP BARS @ PARKING SLAB & MAT FND.
- 8 PROVIDE "DCI CORROSION INHIBITOR" ADMIXTURE FOR ALL PARKING SLABS
- 9 PROVIDE LIQUID DENSIFIER/SEALER
- 10 USE FLY ASH IN CONCRETE MIX DESIGN @ PARKING SLABS @ MAT FND.
- 11 TOPPING SLAB PITCH MAY NOT EXCEED 2% IN ANY DIRECTION AT ACCESSIBLE PARKING SPACES AND ACCESS ISLES.
- 12 BOTTOM OF FOOTING SUBJECT TO CHANGE DEPENDING ON SITE CONDITIONS. REFER TO GEOTECHNICAL REPORT AND FOUNDATION DETAILS.

DRAWN BY DATE

ENT 05.04.2021

BE PROJECT NUMBER

20041

TITLE

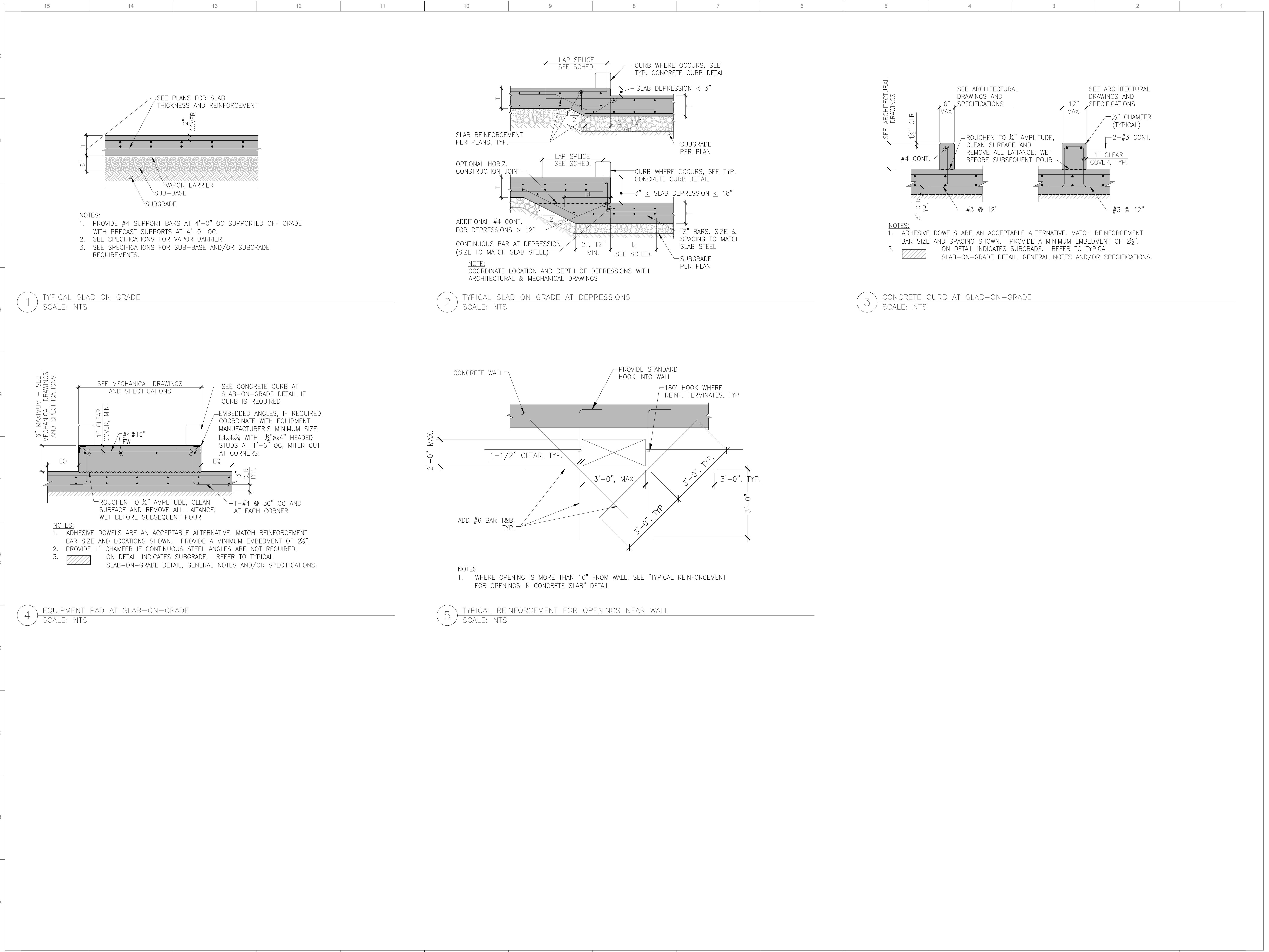
FOUNDATION PLAN

SCALE

As indicated

DRAWING NO.

FO-101



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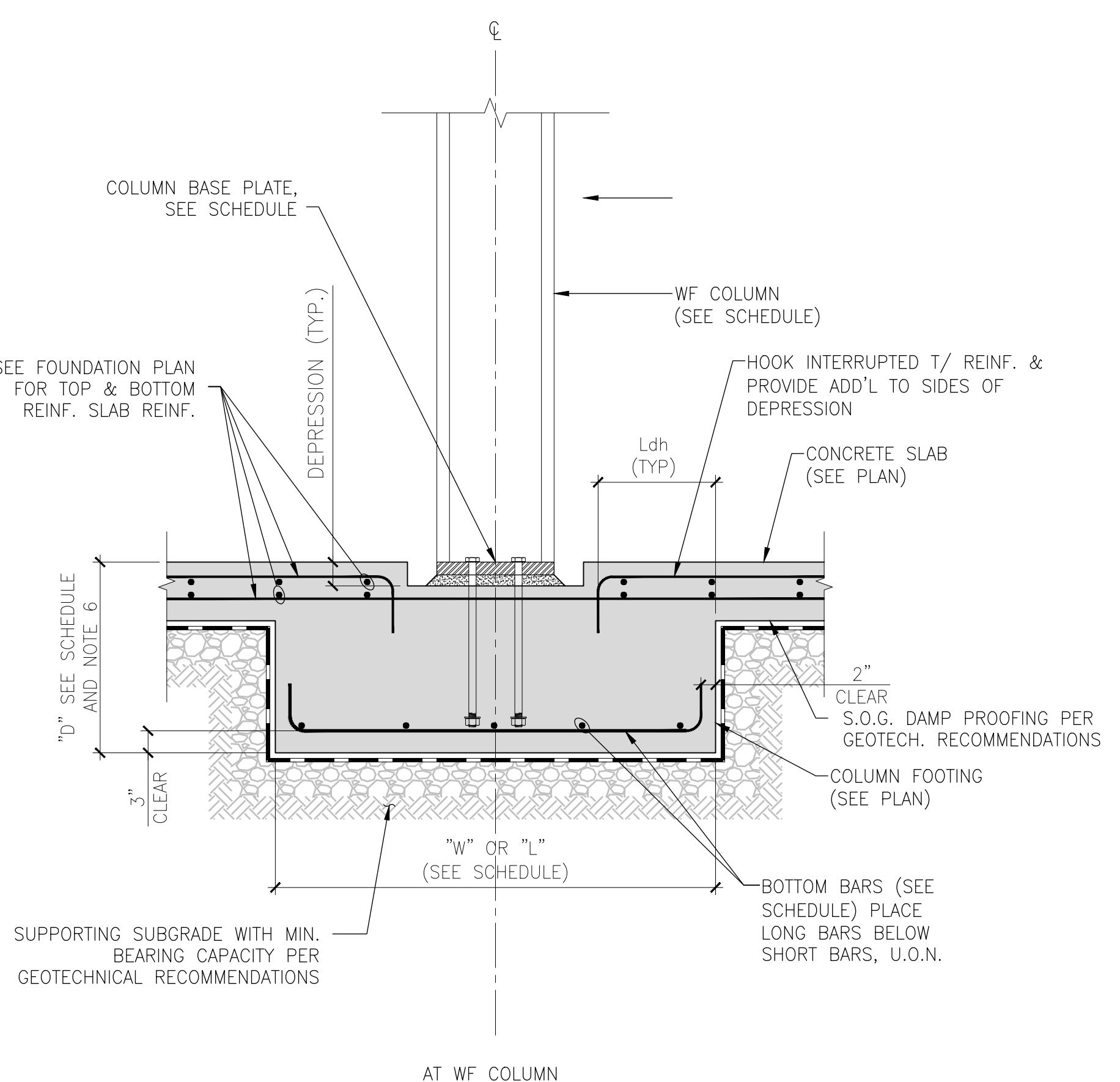
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B

A

FOOTING SCHEDULE								
MARK	SIZE			BOTTOM REINFORCEMENT		TOP REINFORCEMENT		REMARKS
	SHORT	LONG	THICKNESS	LONG BARS	SHORT BARS	LONG BARS	SHORT BARS	
F2.6A	2'-6"	2'-6"	1'-6"	4-#5	4-#5	-	-	-
F2.6B	2'-6"	2'-6"	2'-0"	4-#5	4-#5	-	-	-
F3.6	3'-6"	3'-6"	2'-0"	5-#6	5-#6	-	-	-
F4.6	4'-6"	4'-6"	2'-0"	6-#6	6-#6	-	-	-

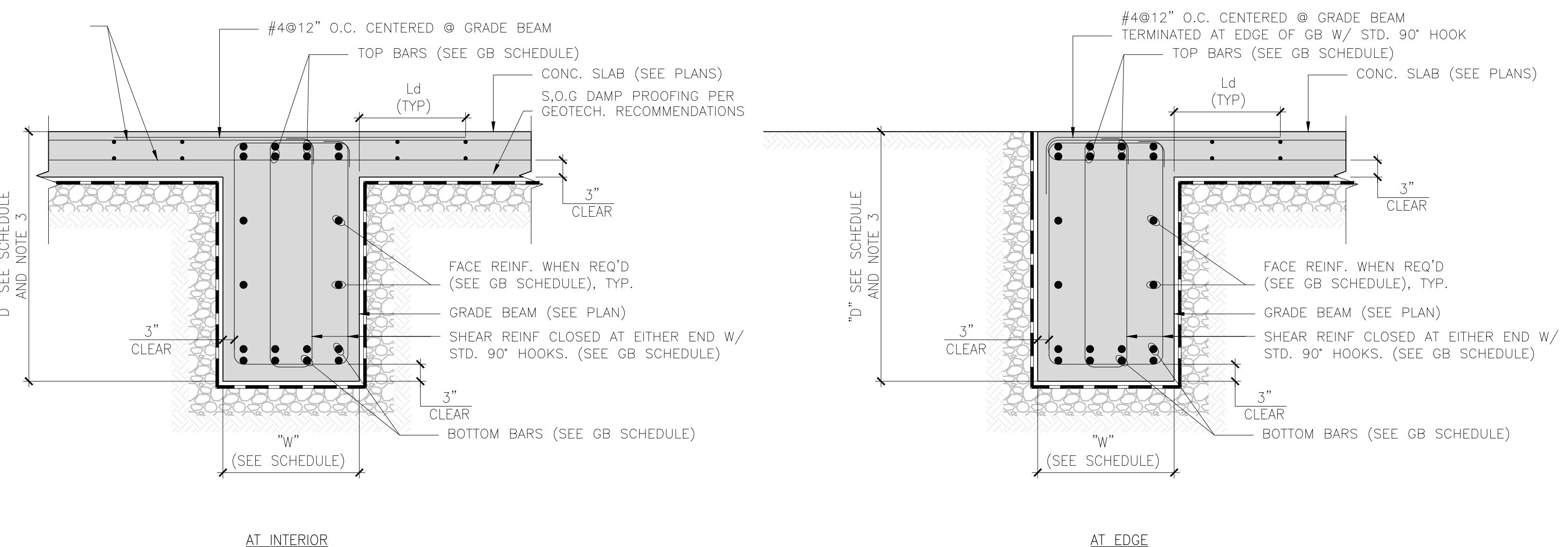
- NOTES:
- MINIMUM DIMENSIONS SHOWN.
 - DIMENSIONS ON PLAN SUPERCEDE ALL TYPICAL DIMENSIONS, TYP.
 - SEE GENERAL NOTES FOR PILE DIAMETERS.
 - ALL FOOTING REINFORCEMENT SHALL BE BENT WITH A STD HOOK AT EA. END.
 - FOOTINGS ARE CENTERED ON THE COLUMNS ABOVE U.O.N. (SEE PLAN).
 - A MINIMUM OF 42" BELOW FINAL GRADE REQUIRED FOR FROST PROTECTION, TYP. THIS MINIMUM EMBEDMENT IS NOT APPLICABLE TO FOOTING BEARING ENTIRELY ON CLEAN, INTACT BEDROCK, AS LONG AS MINIMUM FOOTING DEPTHS ARE AT LEAST 12" BELOW GROUND SURFACE. IN THIS CONDITION, DEPTH "D" CAN BE REDUCED, BUT IT SHALL BE NOT LESS THAN 2'-0" MINIMUM.
 - INTERIOR FOOTINGS IN HEATED AREA DO NOT NEED TO MEET 42" MIN. FROST PROTECTION.
 - REFER TO GEOTECHNICAL RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT PREPARED BY "DOWN TO EARTH CONSULTING, LLC" ON MARCH 26, 2021.



1 COLUMN FOOTING SCHEDULE & DETAIL
SCALE: NTS

GRADE BEAM SCHEDULE						
MARK	WIDTH "W"	DEPTH "D"	TOP REINFORCING	BOTTOM REINFORCING	SHEAR REINFORCING	REMARKS
GB-1	2'-0"	3'-6"	4-#6	4-#6	2L-#4@12" O.C.	3-#5 FACE REINF. EA. SIDE
GB-2	1'-6"	3'-6"	3-#6	3-#6	2L-#4@12" O.C.	3-#5 FACE REINF. EA. SIDE
GB-3	1'-0"	1'-6"	2-#5	2-#5	2L-#4@6" O.C.	
GB-4	2'-0"	2'-0"	3-#6	3-#6	2L-#4@9" O.C.	
GB-5	1'-10"	3'-6"	4-#6	4-#6	2L-#4@12" O.C.	3-#5 FACE REINF. EA. SIDE
GB-6	2'-0"	3'-6"	5-#6	5-#6	3L-#4@12" O.C.	3-#5 FACE REINF. EA. SIDE

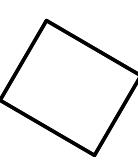
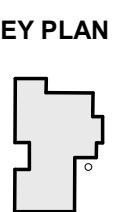
- NOTES:
- MINIMUM DIMENSIONS SHOWN.
 - DIMENSIONS ON PLAN SUPERCEDE ALL TYPICAL DIMENSIONS, TYP.
 - A MINIMUM OF 42" BELOW FINAL GRADE REQUIRED FOR FROST PROTECTION, TYP. THIS MINIMUM EMBEDMENT IS NOT APPLICABLE TO GRADE BEAMS BEARING ENTIRELY ON CLEAN, INTACT BEDROCK, AS LONG AS MINIMUM FOOTING DEPTHS ARE AT LEAST 12" BELOW GROUND SURFACE. IN THIS CONDITION, DEPTH "D" CAN BE REDUCED, BUT SHALL BE NOT LESS THAN 2'-0" MINIMUM.
 - GRADE BEAM TO BE POURED MONOLITHICALLY.
 - IN CASE OF CHANGE IN DEPTH "D", THE TRANSITION SHOULD FOLLOW THE RECOMMENDATION IN THE GEOTECH. REPORT AND IT SHALL BE LOCATED AT LEAST 5'-0" AWAY FROM COL. FOOTING.
 - REFER TO GEOTECH. RECOMMENDATIONS PROVIDED IN THE GEOTECH. REPORT PREPARED BY "DOWN TO EARTH CONSULTING, LLC" ON MARCH 26, 2021.



2 GRADE BEAM DETAIL & SCHEDULE
SCALE: NTS

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CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE BEGUN & ANY MATERIALS & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT OR STRUCTURAL ENGINEER. DRAWN PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

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ENT 05.04.2021

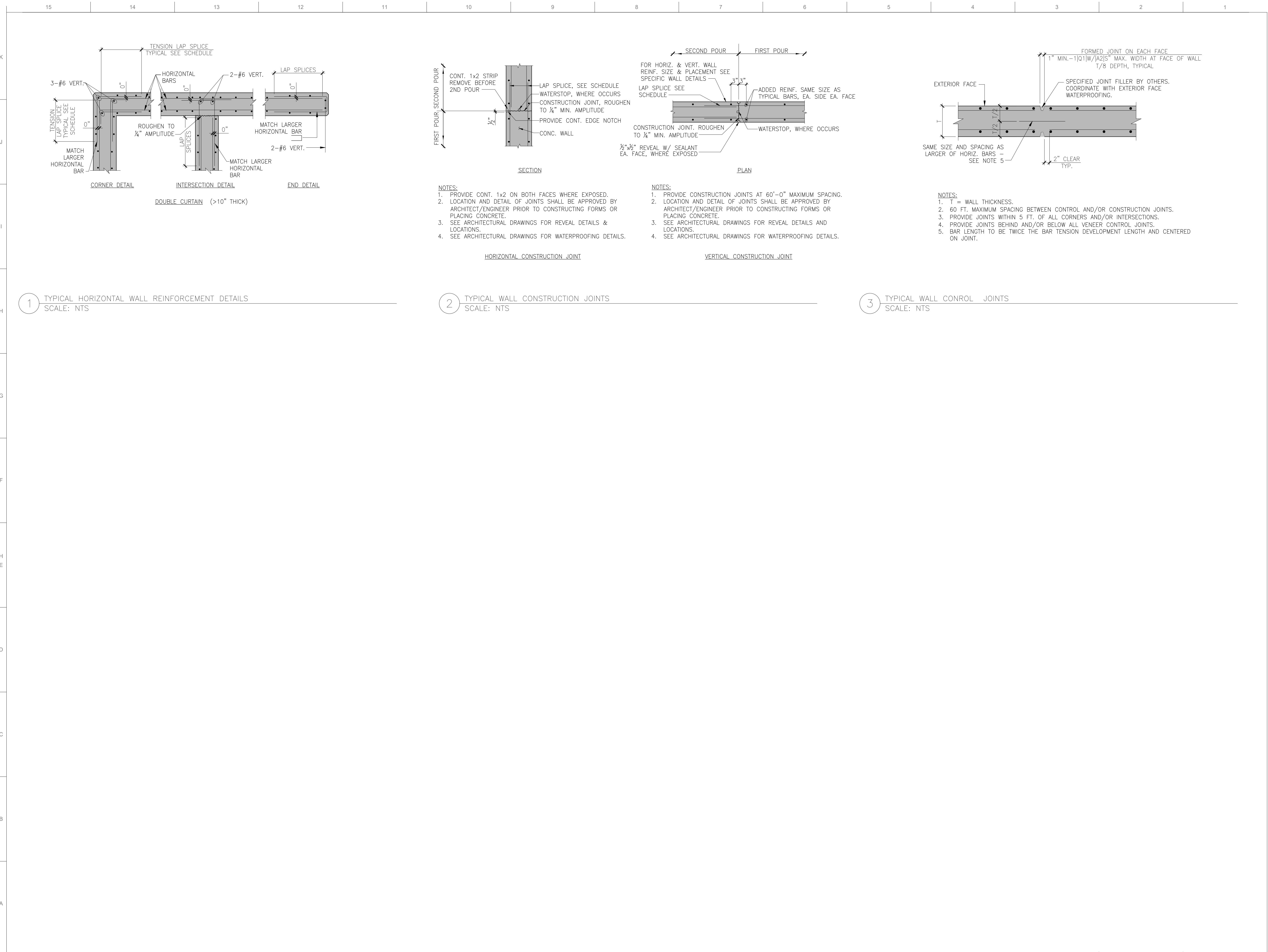
BE PROJECT NUMBER

20041

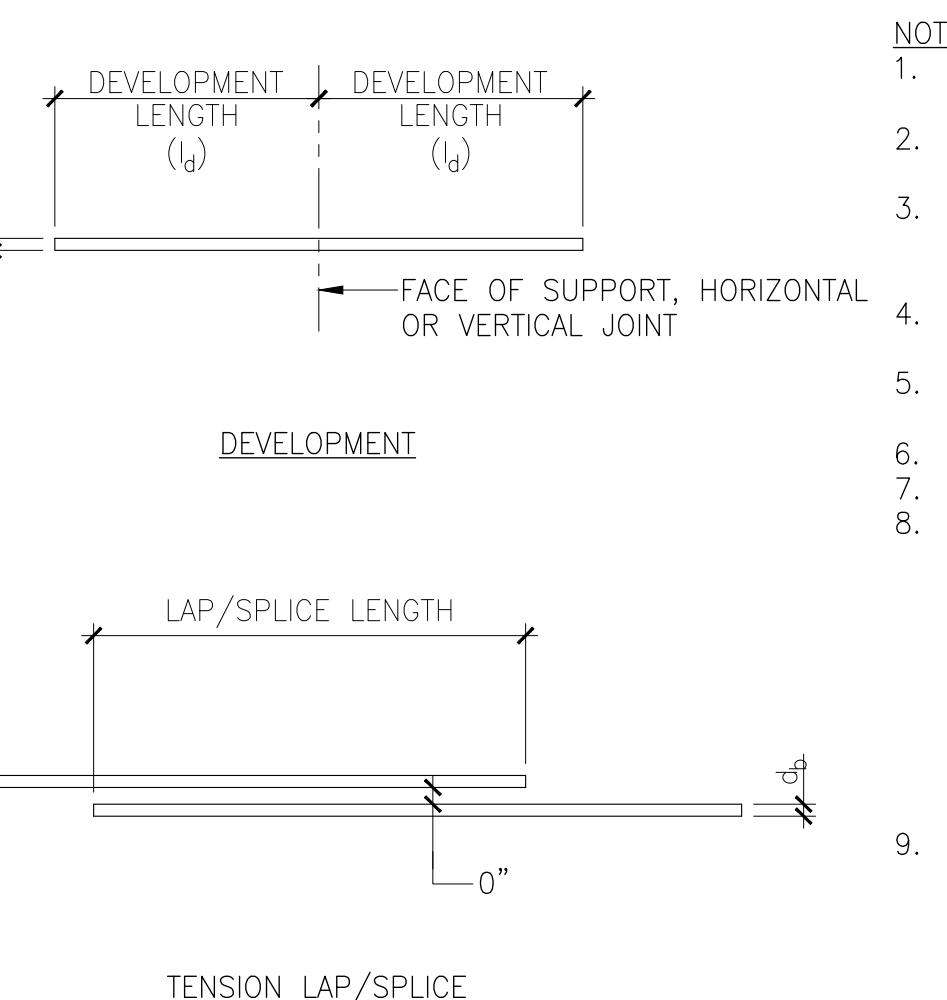
TITLE

FOUNDATION TYPICAL DETAILS
DRAWING NO.

FO-112



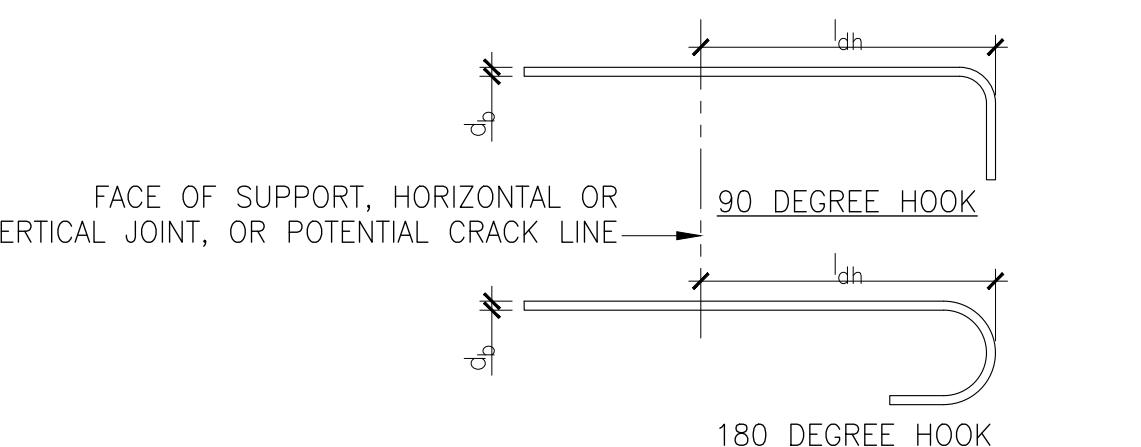
BAR SIZE	TENSION DEVELOPMENT LENGTH INCHES (l_d)										TENSION LAP/SPICE LENGTH INCHES (l_{sl})									
	TOP BARS					OTHER BARS					TOP BARS					OTHER BARS				
	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI	3000 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI
#3	22	19	17	16	15	17	15	13	131	12	29	25	23	21	19	23	20	17	17	16
#4	29	25	23	21	19	22	19	17	16	15	38	33	30	27	25	29	25	23	21	19
#5	36	31	28	26	24	28	24	22	20	19	47	41	37	26	31	37	32	29	27	25
#6	43	37	34	31	29	33	29	26	24	22	56	49	45	34	37	43	38	34	31	28
#7	63	54	49	45	42	48	42	38	34	32	82	71	64	58	54	63	55	50	45	42
#8	72	62	56	51	48	55	48	43	39	37	94	81	73	67	62	72	63	56	51	48
#9	81	70	63	58	54	62	54	48	44	41	106	91	82	75	70	81	71	63	58	54
#10	91	79	71	65	60	70	61	54	50	46	119	103	93	85	78	91	80	71	64	60
#11	101	87	78	72	67	78	67	60	56	52	132	114	102	94	87	102	88	78	73	67



- NOTES:
- REFER TO HOOKED REINFORCEMENT TENSION DEVELOPMENT LENGTH SCHEDULE IN CONCRETE WHEN THE STRAIGHT DEVELOPMENT LENGTH IN TENSION CANNOT BE ACCOMMODATED IN THE CONCRETE SECTION.
 - TABULATED DEVELOPMENT LENGTHS ARE BASED ON REINFORCING STEEL YIELD STRENGTH $F_y = 60$ KSI AND NORMAL WEIGHT CONCRETE. LAP SPLICE LENGTHS ARE CLASS B, UNLESS OTHERWISE NOTED IN DRAWINGS.
 - TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE CAST IN THE MEMBER BELOW, THE BARS TO BE DEVELOPED OR SPLICED. THE TOP BAR FACTOR SHALL BE APPLIED TO HORIZONTAL BARS IN WALLS.
 - WHEN BARS OF DIFFERENT SIZE ARE LAP SPliced IN TENSION, SPLICE LENGTH SHALL BE THE LARGER OF l_d OF THE LARGER BAR AND TENSION LAP SPLICE LENGTH OF THE SMALLER BAR.
 - ALL TABULATED VALUES ARE MINIMUM LENGTHS, IN CASE OF CONFLICT WITH PLANS, SECTIONS, OR DETAILS, USE THE LONGER LENGTH.
 - d_b = BAR DIAMETER.
 - l_d = DEVELOPMENT LENGTH.
 - ADJUST TABULATED LENGTHS BY THE FOLLOWING MULTIPLICATION FACTORS WHERE APPPLICABLE. NOTE THAT FACTORS ARE CUMULATIVE: (E.G. 1.30x1.50 = 1.95)
 - A. LIGHT WEIGHT CONCRETE: 1.33
 - B. 3 OR LESS BUNDLED BARS: 1.20
 - C. 4 OR MORE BUNDLED BARS: 1.33
 - D. CLEAR SPACING LESS THAN $2d_b$: 1.50
 - E. CLASS A LAP SPLICE: 0.77
 - F. EPOXY COATED BARS: 1.50
 - WELDED AND/OR MECHANICAL SPLICES MAY BE USED IF APPROVED BY THE STRUCTURAL ENGINEER OF RECORD PROVIDED THAT THE SPLICE IS CAPABLE OF DEVELOPING AT LEAST 125% OF THE YIELD STRENGTH OF THE LARGER BAR IN TENSION. WHERE WELDED AND/OR MECHANICAL SPLICES ARE TO BE USED, THE GENERAL CONTRACTOR SHALL SUBMIT FULL DATA ON THE PROPOSED MATERIAL, PROCEDURES, AND INSTALLATION INSTRUCTIONS TO THE ENGINEER FOR REVIEW AS A SHOP DRAWING SUBMISSION.
 - USE MECHANICAL COUPLERS FOR #14 AND LARGER BARS.
 - FOR LAP SPLICES IN CONCRETE MASONRY, SEE MASONRY REINFORCEMENT DETAILS.

1 STRAIGHT REINFORCEMENT DEVELOPMENT AND SPLICE LENGTH SCHEDULE FOR CONCRETE
SCALE: NTS

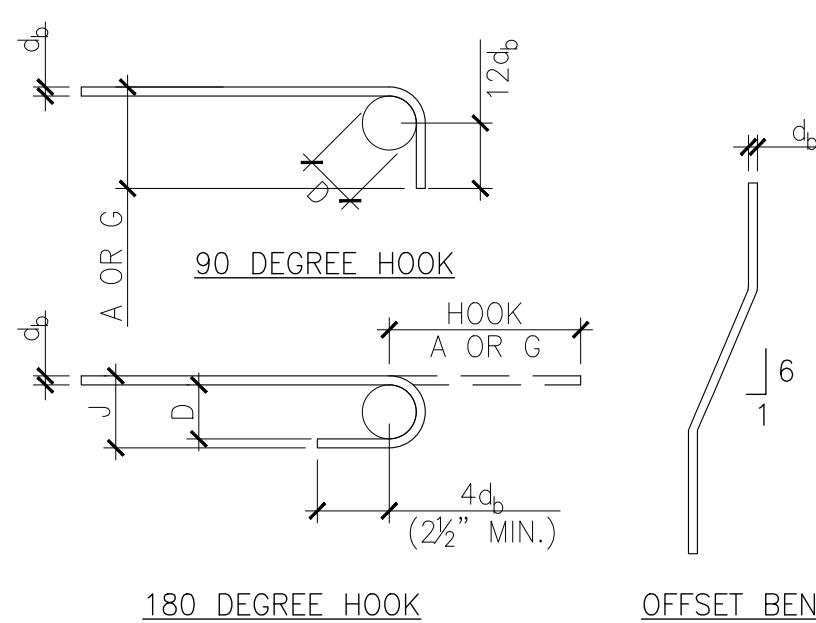
BAR SIZE	TENSION DEVELOPMENT LENGTH FOR HOOKED BARS (l_{dh}) INCHES				
	3,000 PSI	4,000 PSI	5,000 PSI	6,000 PSI	7,000 PSI
#3	9	8	7	6	6
#4	11	10	9	8	8
#5	14	12	11	10	9
#6	17	15	13	12	11
#7	20	17	15	14	13
#8	22	19	17	16	15
#9	25	22	20	18	17
#10	28	24	22	20	18
#11	31	27	24	22	20



- NOTES:
- SEE TYPICAL REINFORCEMENT BEND DETAIL FOR ADDITIONAL INFORMATION.
 - TABULATED DEVELOPMENT LENGTHS ARE BASED ON REINFORCING STEEL YIELD STRENGTH ($F_y = 60$ KSI) AND NORMAL WEIGHT CONCRETE.
 - ALL TABULATED VALUES ARE MINIMUM LENGTHS. IN CASE OF CONFLICT WITH THE PLANS, SECTIONS, OR DETAILS, USE THE LONGER LENGTH.
 - d_b = BAR DIAMETER
 - l_d = TENSION DEVELOPMENT LENGTH (HOOK BARS)
 - ADJUST TABULATED LENGTHS BY THE FOLLOWING MULTIPLICATION FACTORS WHERE APPPLICABLE. NOTE THAT THE FACTORS ARE CUMULATIVE: (E.G. 1.33 x 1.20 = 1.60)
 - A. REINFORCING BAR STRENGTH OTHER THAN 60 KSI: $(F_y/60,000)$
 - B. LIGHT WEIGHT CONCRETE: 1.33
 - C. EPOXY COATED BARS: 1.20

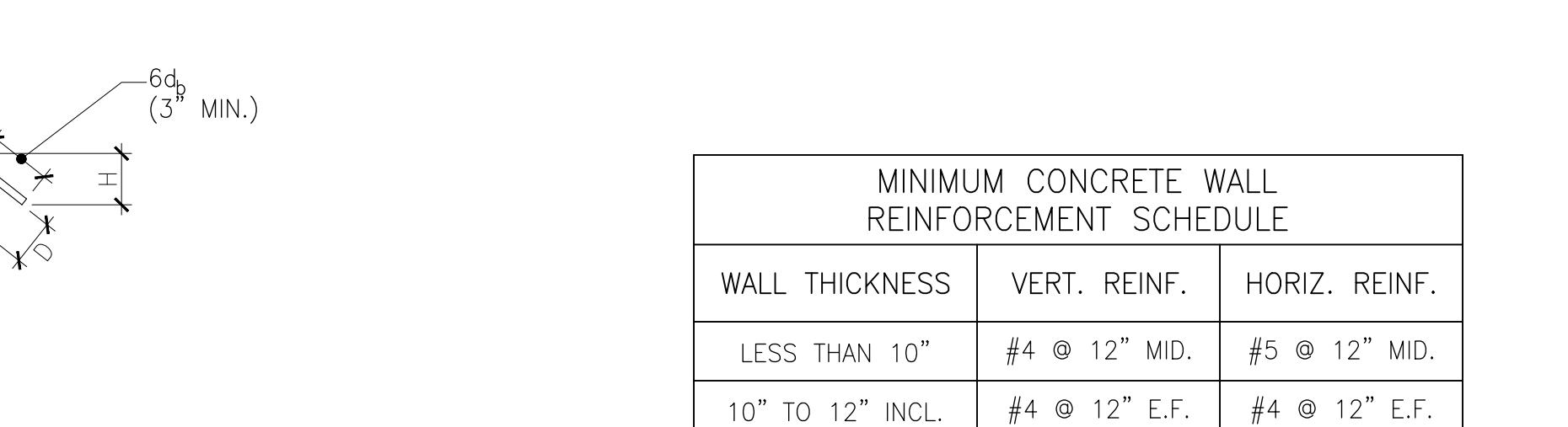
2 HOOKED REINFORCEMENT TENSION DEVELOPMENT LENGTH SCHEDULE IN CONCRETE
SCALE: NTS

BAR SIZE	BEND DIAMETER (D) INCHES	180° HOOKS		90° HOOKS	
		A OR G INCHES	J INCHES	A OR G INCHES	J INCHES
#3	2 1/4	5	3	6	
#4	3	6	4	8	
#5	3 3/4	7	5	10	
#6	4 1/2	8	6	12	
#7	5 1/4	10	7	14	
#8	6	11	8	16	
#9	9 1/2	15	11 3/4	19	
#10	10 1/4	17	13 1/4	22	
#11	12	19	14 3/4	24	



- NOTES:
- ALL BENDS SHALL BE MADE COLD AND SHALL BE MADE PRIOR TO PARTIAL EMBEDMENT IN CONCRETE.
 - d_b = BAR DIAMETER.
 - D = BEND DIAMETER, MEASURED ON THE INSIDE OF BAR.

BAR SIZE	BEND DIAMETER (D) INCHES	90° HOOK		135° HOOKS	
		HOOK A OR G INCHES	H (APPROX.) INCHES	HOOK A OR G INCHES	H (APPROX.) INCHES
#3	1 1/2	4	4 1/4	3	
#4	2	4 1/2	4 1/2	3	
#5	2 1/2	6	5 1/2	3 3/4	



MINIMUM CONCRETE WALL REINFORCEMENT SCHEDULE		
WALL THICKNESS	VERT. REINF.	HORIZ. REINF.
LESS THAN 10"	#4 @ 12" MID.	#5 @ 12" MID.
10" TO 12" INCL.	#4 @ 12" E.F.	#4 @ 12" E.F.
12" TO 20" INCL.	#5 @ 12" E.F.	#5 @ 12" E.F.

PROVIDE THE ABOVE MINIMUM REINFORCEMENT IN ALL CONCRETE WALLS WHERE REINFORCEMENT IS NOT INDICATED ON THE DRAWINGS

4 TYPICAL TIE AND STIRRUP HOOKS IN CONCRETE AND MASONRY
SCALE: NTS

5 MINIMUM WALL REINFORCEMENT SCHEDULE
SCALE: NTS

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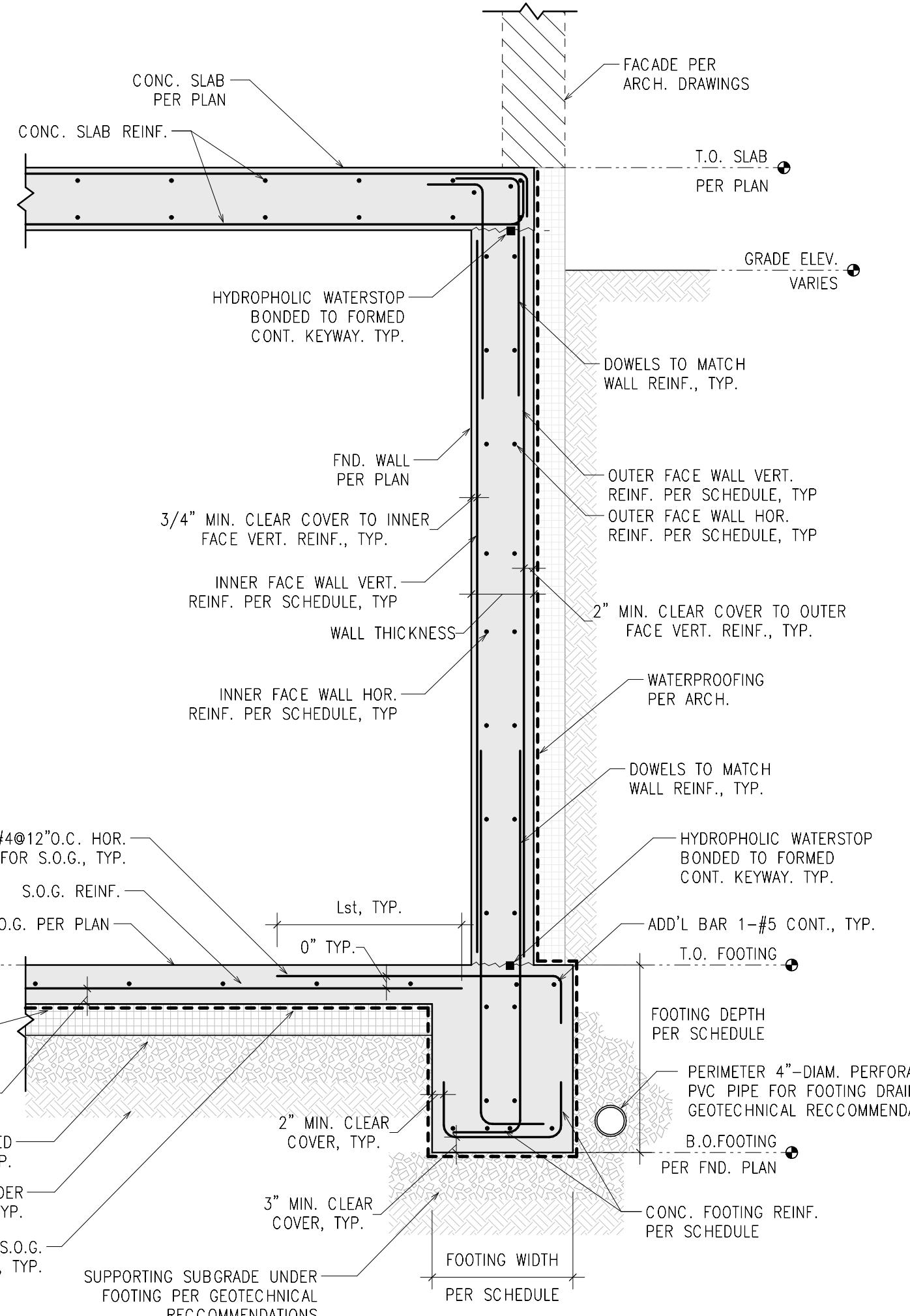
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PRINTING DATE
01 - 100% DD SET 05.04.2021
BE PROJECT NUMBER 20041
TITLE FOUNDATION TYPICAL DETAILS
DRAWING NO. FO-114

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FOUNDATION WALL REINFORCEMENT SCHEDULE						
WALL TAG	THICKNESS	VERTICAL REINFORCEMENT		HORIZONTAL REINFORCEMENT		MARKS
		EXTERIOR FACE	INTERIOR FACE	EXTERIOR FACE	INTERIOR FACE	
FW-1	10"	#4@12" O.C.	#6@12" O.C.	#4@12" O.C.	#4@12" O.C.	-
FW-2	10"	#4@12" O.C.	#4@12" O.C.	#4@12" O.C.	#4@12" O.C.	INTERIOR WALL WITH NO SOIL PRESSURE
FW-3	8"	#4@12" O.C.	#5@12" O.C.	#4@12" O.C.	#4@12" O.C.	-
FW-4	10"	#4@12" O.C.	#6@12" O.C.	#4@12" O.C.	#4@12" O.C.	-
FW-5	8"	#4@12" O.C.	#4@12" O.C.	#4@12" O.C.	#4@12" O.C.	EXTERIOR WALL WITH NO SOIL PRESSURE
FW-6	10"	#4@12" O.C.	#4@12" O.C.	#4@12" O.C.	#4@12" O.C.	EXTERIOR WALL WITH NO SOIL PRESSURE

WALL FOOTING REINFORCEMENT SCHEDULE							
WALL FOOTING TAG	SIZE		BOTTOM REINFORCEMENT		TOP REINFORCEMENT		MARKS
	WIDTH	DEPTH	LONG BARS	SHORT BARS	LONG BARS	SHORT BARS	
FT-1	18"	24"	3-#5	#6@10" O.C.	-	-	SEE NOTES FOR FOOTING DEPTH
FT-2	18"	42"	3-#6	#6@6" O.C.	-	-	SEE NOTES FOR FOOTING DEPTH

NOTES:

- MINIMUM DIMENSIONS SHOWN.
- DIMENSIONS ON PLANS SUPERCEDE ALL TYPICAL DIMENSIONS, TYP.
- REFER TO GENERAL NOTES FOR MINIMUM CLEAR COVER FOR CONCRETE U.O.N.
- ALL FOOTING REINFORCEMENT SHALL BE BENT WITH A STANDARD HOOK AT EACH END.
- WALL FOOTING ARE CENTERED ON THE WALL ABOVE U.O.N. (SEE PLAN).
- MINIMUM OF 42" BELOW FINAL GRADE REQUIRED FOR FROST PROTECTION, TYP. THIS MINIMUM EMBEDMENT IS NOT APPLICABLE TO FOOTING BEARING ENTIRELY ON CLEAN, INTACT BEDROCK, AS LONG AS MINIMUM FOOTING DEPTHS ARE AT LEAST 12" BELOW GROUND SURFACE. IN THIS CONDITION, DEPTH "D" CAN BE REDUCED, BUT IT SHALL BE NOT LESS THAN 2'-0" MINIMUM.
- INTERIOR FOOTINGS IN HEATED AREAS DO NOT NEED TO MEET 42" MIN. FROST PROTECTION, AND BOTTOM OF FOOTING SHALL BE LOCATED 2'-0" MINIMUM BELOW TOP OF SLAB ON GRADE.
- REFER TO GEOTECHNICAL RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT PREPARED BY "DOWN TO EARTH CONSULTING, LLC" ON MARCH 26, 2021.

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CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE BEGUN & ANY MATERIALS & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT OR STRUCTURAL ENGINEER. DRAWN PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

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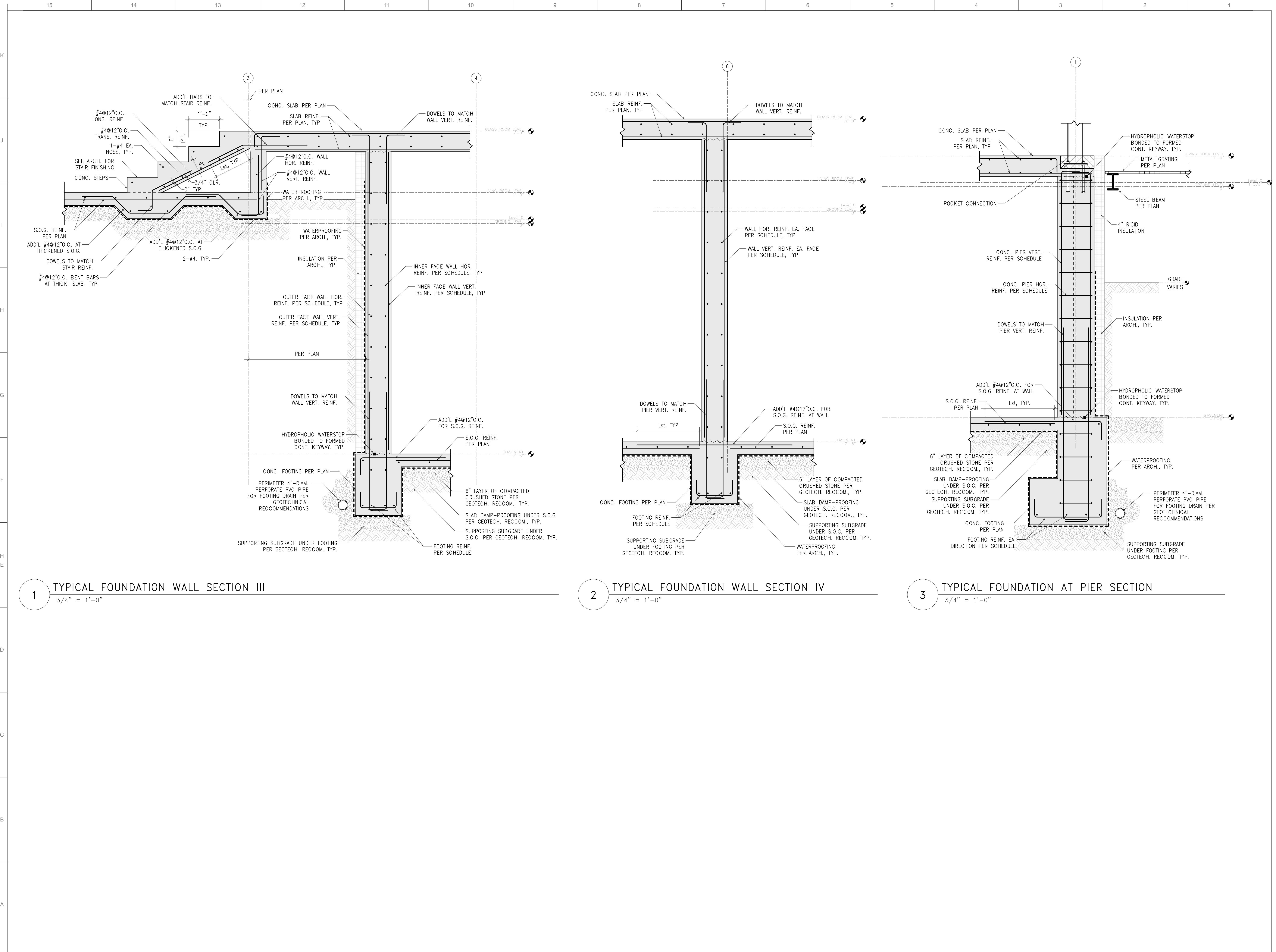
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ENT 06.23.2021

BE PROJECT NUMBER
20041

TITLE
FO-115

DRAWING NO.

FOUNDATION TYPICAL DETAILS
FO-115



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KEY PLAN

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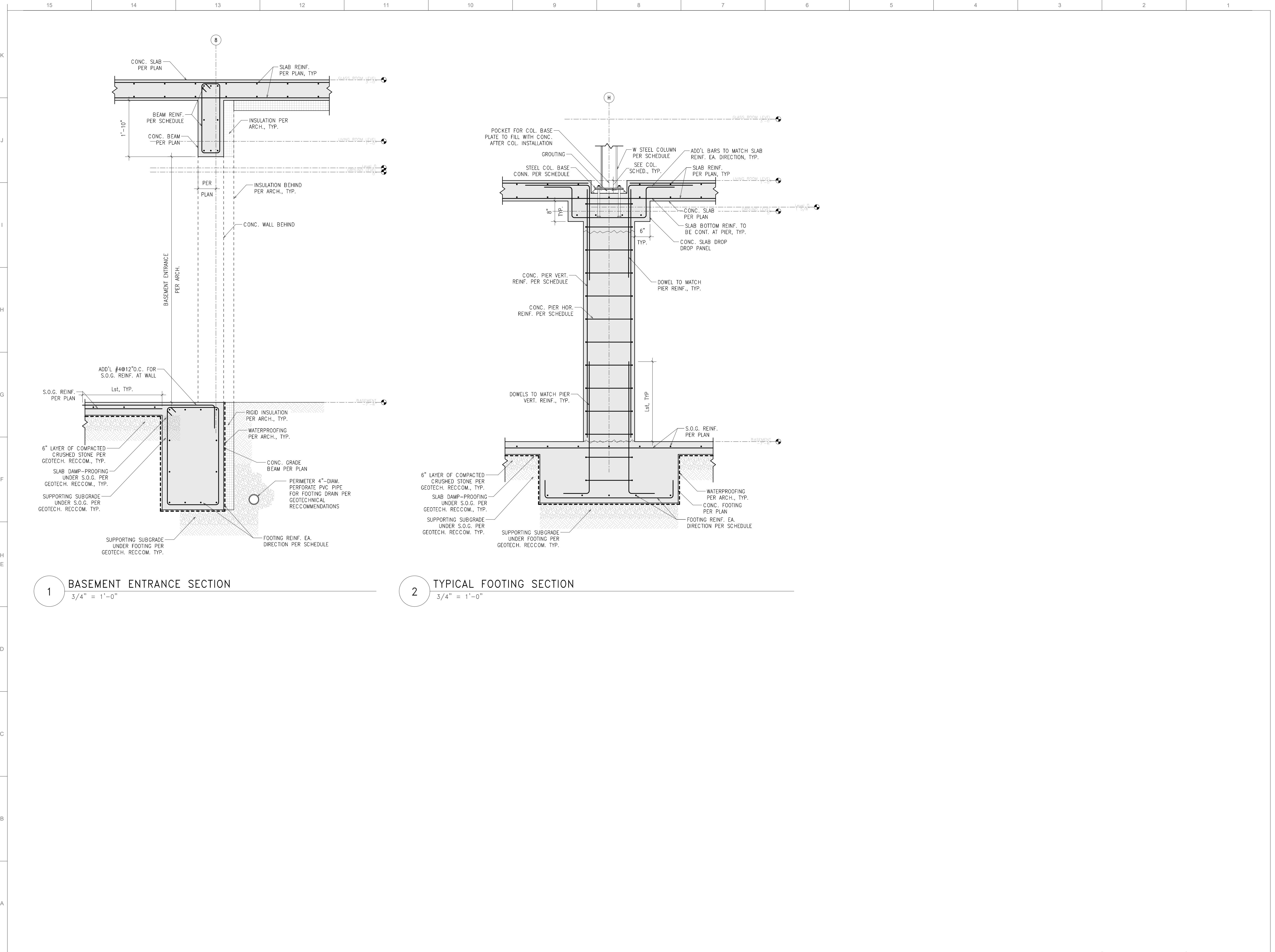
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TITLE

**FOUNDATION TYPICAL
SECTIONS**

DRAWING NO.

FO-122



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KEY PLAN

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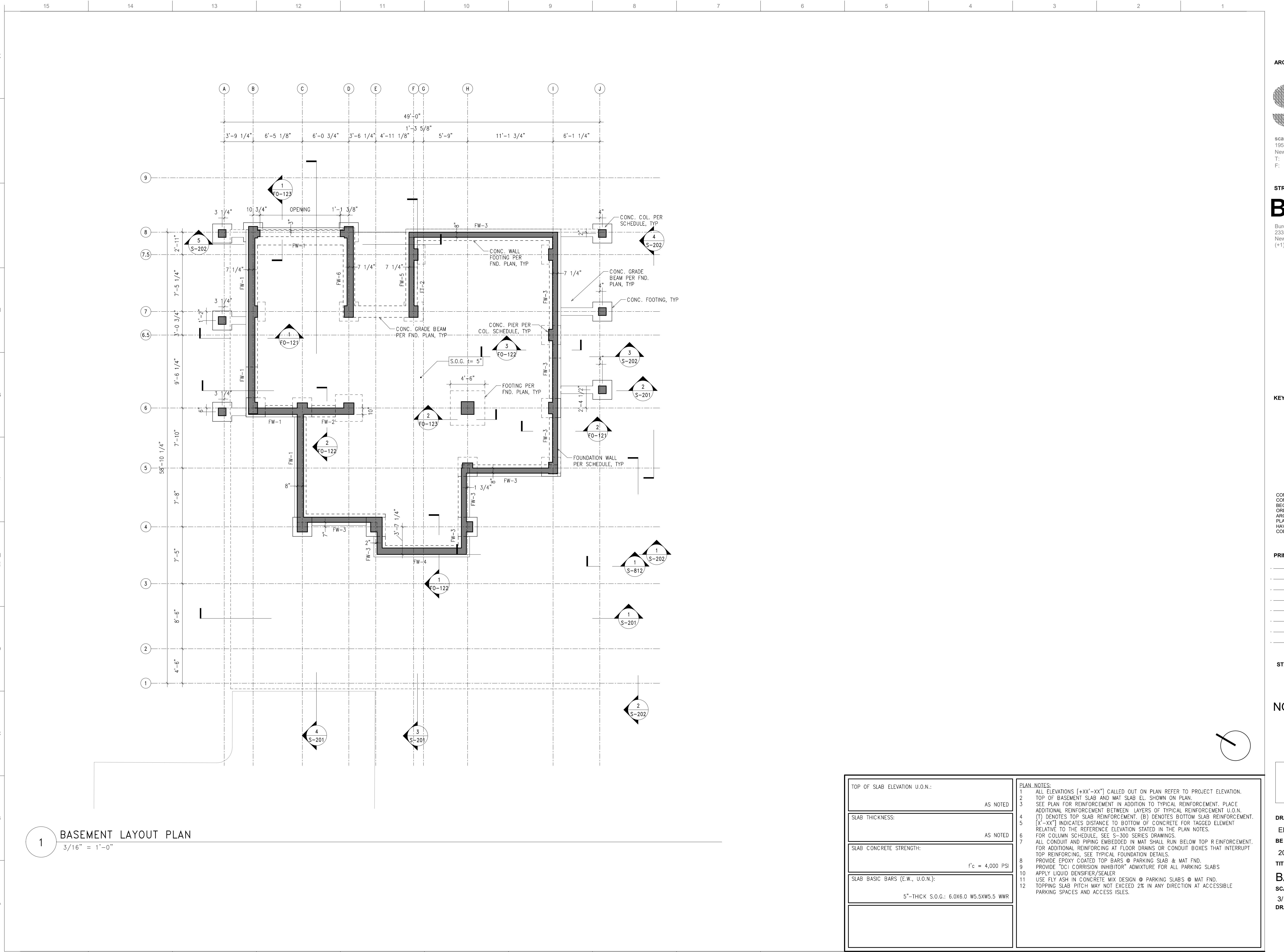
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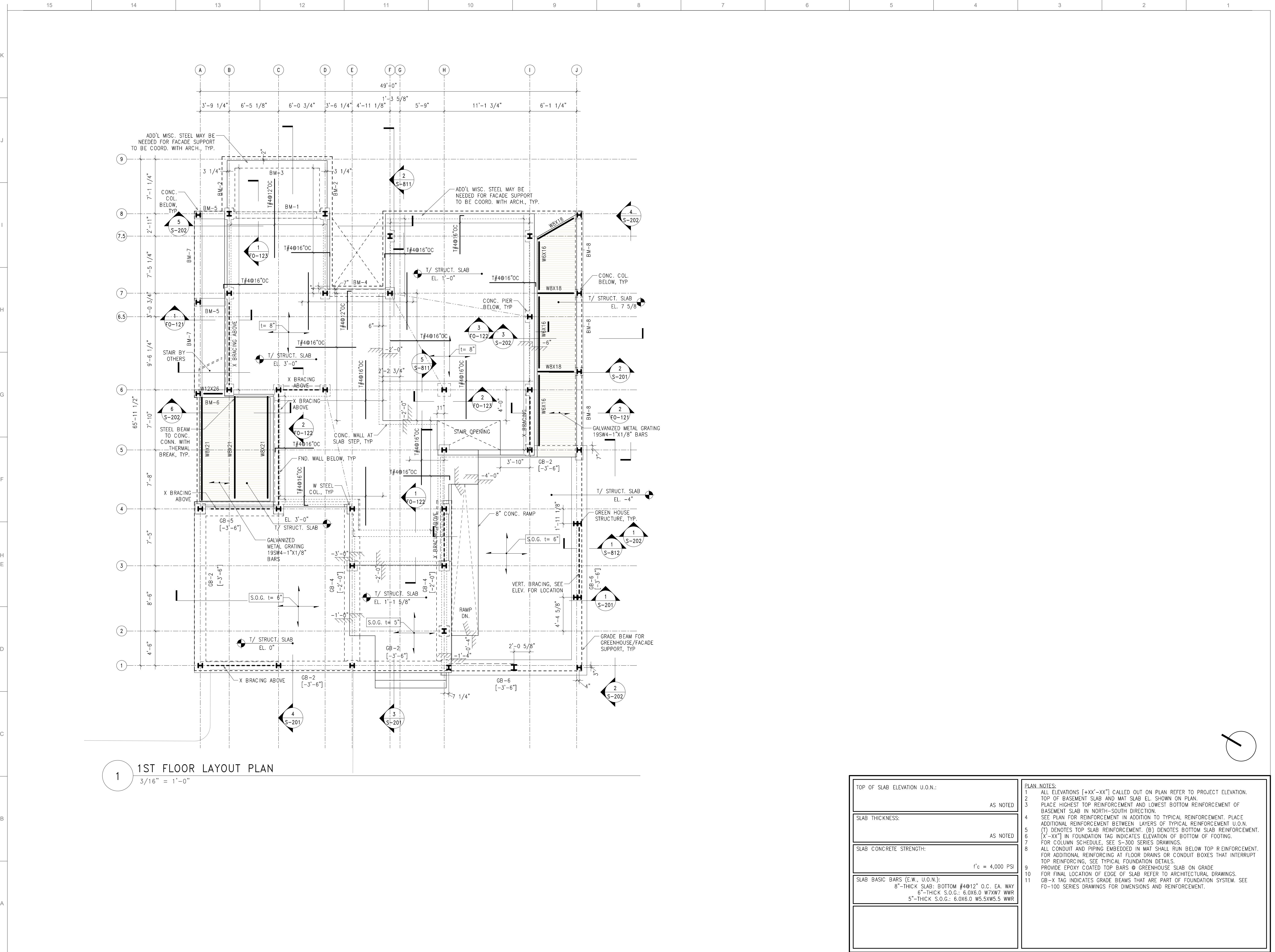
TITLE

**FOUNDATION TYPICAL
SECTIONS**

DRAWING NO.

FO-123





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 BE PROJECT NUMBER
 20041
 TITLE

GROUND FL FRAMING PLAN
 $5'-0" - 1'-0"$
 DRAWING NO.

S-102

TOP OF SLAB ELEVATION U.O.N.: AS NOTED
SLAB THICKNESS: AS NOTED
SLAB CONCRETE STRENGTH: $f'_c = 4,000 \text{ PSI}$
SLAB BASIC BARS (E.W., U.O.N.): 8"-THICK SLAB- BOTTOM #4@12" O.C. EA. WAY 6"-THICK S.O.G.- 6.0X6.0 W7XW7 WWR 5"-THICK S.O.G.- 6.0X6.0 W5.5XW5.5 WWR

PLAN NOTES:

- 1 ALL ELEVATIONS [XX'-XX"] CALLED OUT ON PLAN REFER TO PROJECT ELEVATION.
- 2 TOP OF BASEMENT SLAB AND MAT SLAB EL. SHOWN ON PLAN.
- 3 PLACE HIGHEST TOP REINFORCEMENT AND LOWEST BOTTOM REINFORCEMENT OF BASEMENT SLAB IN NORTH-SOUTH DIRECTION.
- 4 SEE PLAN FOR REINFORCEMENT IN ADDITION TO TYPICAL REINFORCEMENT. PLACE ADDITIONAL REINFORCEMENT BETWEEN LAYERS OF TYPICAL REINFORCEMENT U.O.N.
- 5 (T) DENOTES TOP SLAB REINFORCEMENT, (B) DENOTES BOTTOM SLAB REINFORCEMENT.
- 6 [XX'-XX"] IN FOUNDATION TAG INDICATES ELEVATION OF BOTTOM OF FOOTING.
- 7 FOR COLUMN SCHEDULE, SEE S-300 SERIES DRAWINGS.
- 8 ALL CONDUIT AND PIPELINE EMBEDDED IN MAT SHALL RUN BELOW TOP R REINFORCEMENT. FOR ADDITIONAL REINFORCING AT FLOOR DRAINS OR CONDUIT BOXES THAT INTERRUPT TOP REINFORCING, SEE TYPICAL FOUNDATION DETAILS.
- 9 PROVIDE EPOXY COATED TOP BARS @ GREENHOUSE SLAB ON GRADE.
- 10 FOR FINAL LOCATION OF EDGE OF SLAB REFER TO ARCHITECTURAL DRAWINGS.
- 11 FOR CONDUIT AND PIPELINE EMBEDDED IN MAT SHALL RUN BELOW TOP R REINFORCEMENT. FOR ADDITIONAL REINFORCING AT FLOOR DRAINS OR CONDUIT BOXES THAT INTERRUPT TOP REINFORCING, SEE TYPICAL FOUNDATION DETAILS.

15 | 14 | 13 | 12 | 11 | 10 | 9 | 8 | 7 | 6 | 5 | 4 | 3 | 2 | 1

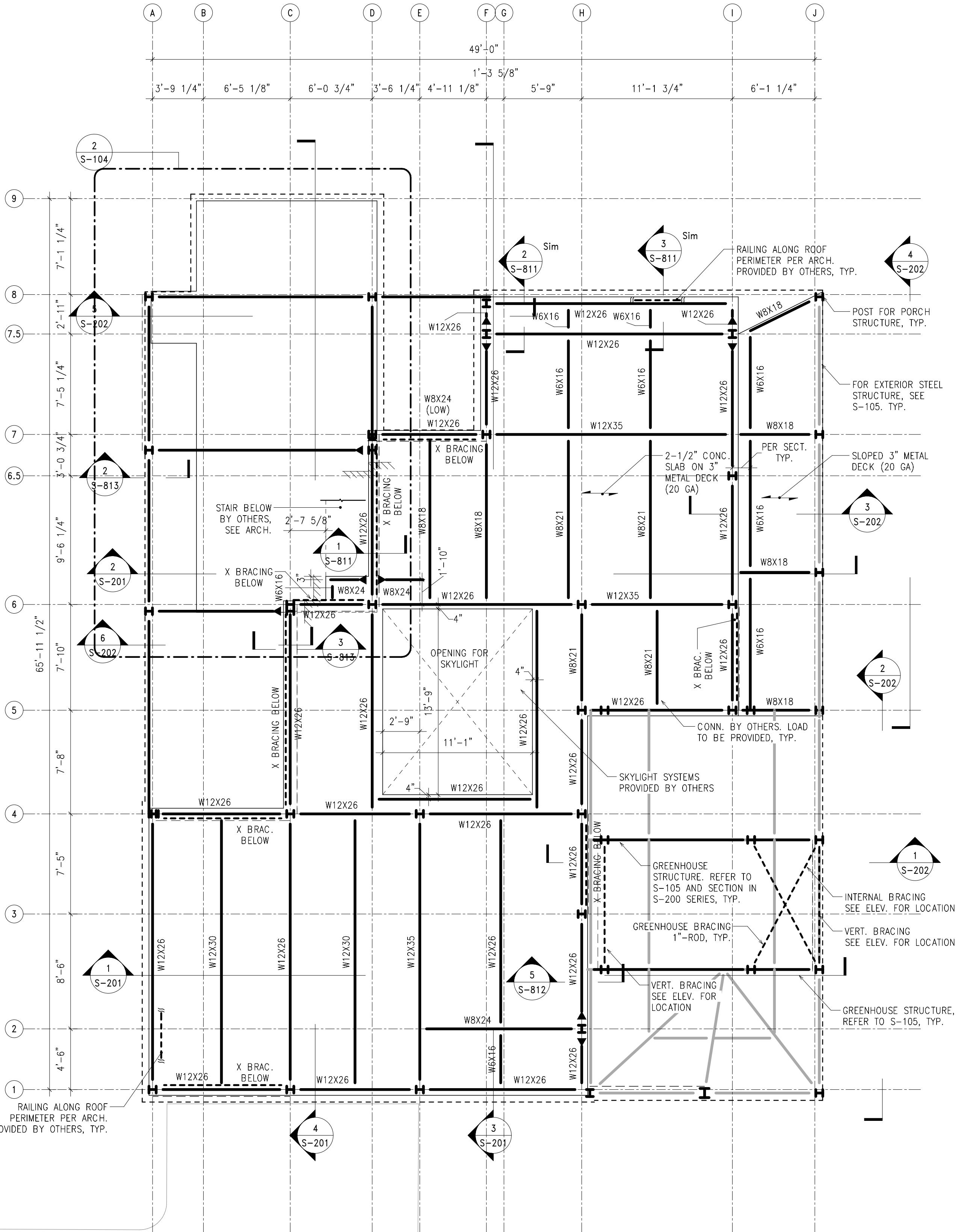
ARCHITECT:

The logo for scalar Architecture consists of two main graphic elements. On the left, a stylized lowercase letter 's' is formed by a repeating pattern of the letters 'SS'. On the right, a large, solid black triangle is composed of a repeating pattern of the letters 'AA'.

STRUCTURAL ENGINEER:

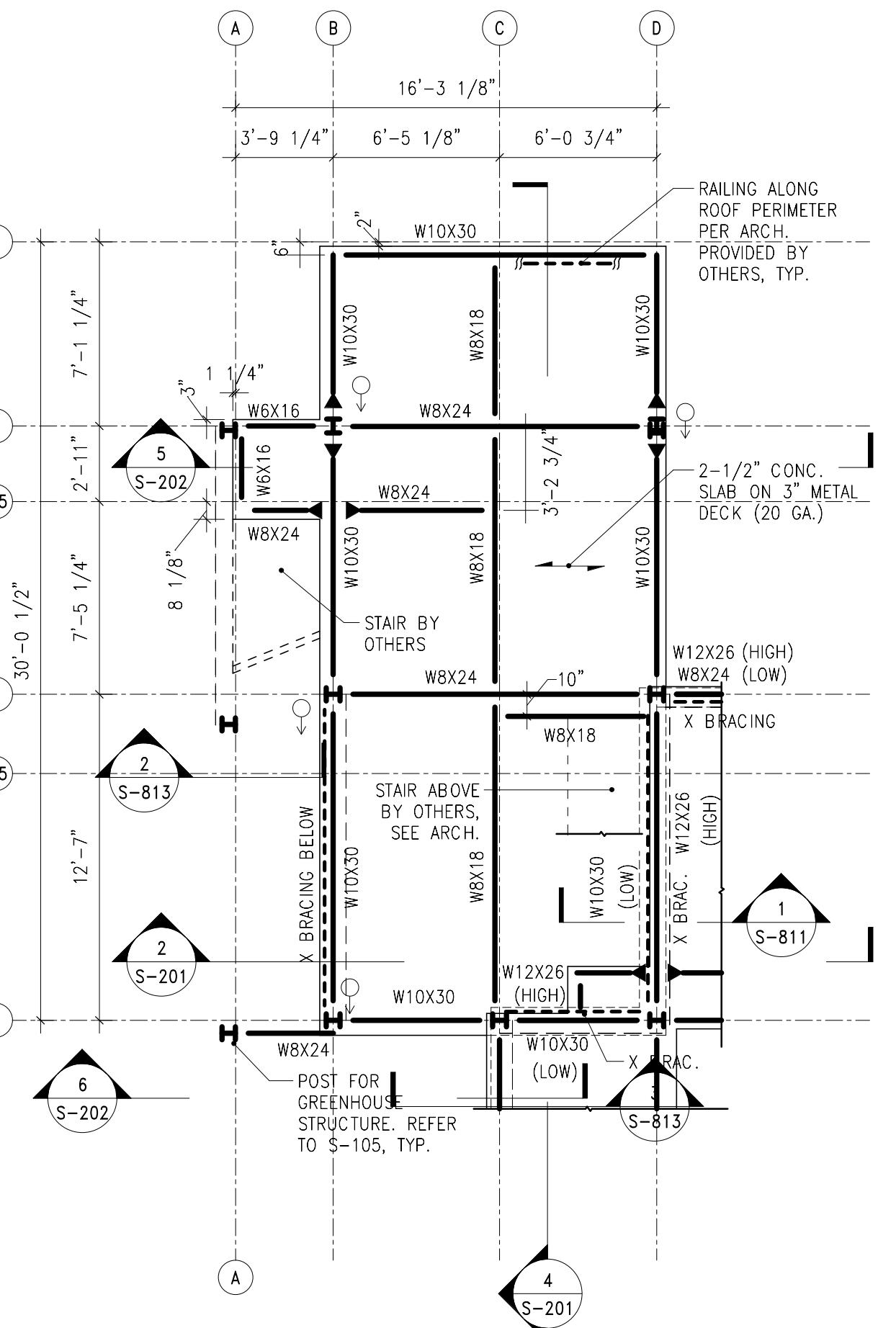
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1 ROOF FRAMING PLAN

PRIVACY



2 LOWER ROOF PARTIAL FRAMING PLAN

CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE UNDERTAKEN. REPORT ALL DISCREPANCIES TO THE ARCHITECT. SHOP DETAILS MUST BE SUBMITTED. PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

ST

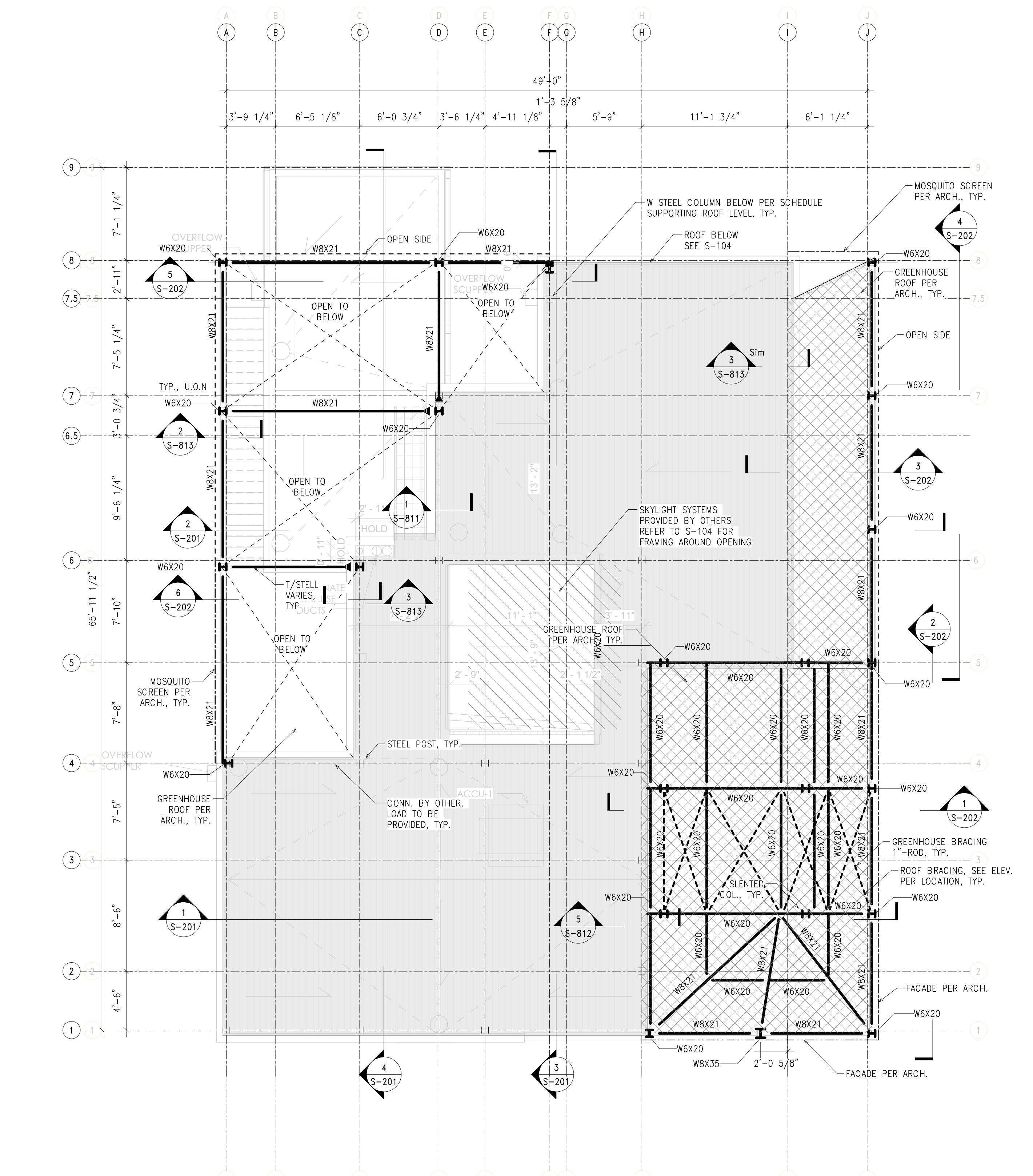
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TOP OF SLAB ELEVATION U.O.N.:	PLAN NOTES:	
AS NOTED	1 ALL ELEVATIONS [+XX'-XX"] CALLED OUT ON PLAN REFER TO PROJECT ELEVATION.	
SLAB THICKNESS:	2 TOP OF BASEMENT SLAB AND MAT SLAB EL. SHOWN ON PLAN.	
AS NOTED	3 SEE PLAN FOR REINFORCEMENT IN ADDITION TO TYPICAL REINFORCEMENT. PLACE ADDITIONAL REINFORCEMENT BETWEEN LAYERS OF TYPICAL REINFORCEMENT U.O.N.	
SLAB CONCRETE STRENGTH:	4 (T) DENOTES TOP SLAB REINFORCEMENT. (B) DENOTES BOTTOM SLAB REINFORCEMENT. 5 [X'-XX"] INDICATES DISTANCE TO BOTTOM OF CONCRETE FOR TAGGED ELEMENT RELATIVE TO THE REFERENCE ELEVATION STATED IN THE PLAN NOTES.	
f'c = 4,000 PSI	6 FOR COLUMN SCHEDULE, SEE S-300 SERIES DRAWINGS.	
SLAB BASIC BARS (E.W., U.O.N.):	7 ALL CONDUIT AND PIPING EMBEDDED IN MAT SHALL RUN BELOW TOP REINFORCEMENT. FOR ADDITIONAL REINFORCING AT FLOOR DRAINS OR CONDUIT BOXES THAT INTERRUPT TOP REINFORCING, SEE TYPICAL FOUNDATION DETAILS.	
2-1/2" SLAB ON METAL DECK: 4.0X4.0 W4.0XW4.0 WWR	8 FOR FINAL LOCATION OF EDGE OF SLAB REFER TO ARCHITECTURAL DRAWINGS.	
	B-SCAN STICKER	
	DRAWN BY	DATE
	ENT	06.23.2021
	BE PROJECT NUMBER	
	20041	
	TITLE	
	ROOF FRAMING PLAN	
	SCALE	
	3/16" = 1'-0"	
	DRAWING NO.	
	S-104	

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1 GREENHOUSE ROOF FRAMING PLAN
 $3/16'' = 1'-0''$

TOP OF SLAB ELEVATION U.O.N.:	N/A
SLAB THICKNESS:	N/A
SLAB CONCRETE STRENGTH:	N/A
SLAB BASIC BARS (E.W., U.O.N.):	N/A

PLAN NOTES:
1 ALL ELEVATIONS [xx'-xx"] CALLED OUT ON PLAN REFER TO PROJECT ELEVATION.
2 FOR FINAL LOCATION OF FAÇADE AND GREENHOUSE STRUCTURE REFER TO ARCH. DWGS.
HSS SIZES TO BE CONSIDERED PRELIMINARY AND SUBJECT TO CHANGE BASED ON
FINAL CONNECTION DESIGN PROVIDED BY OTHERS.

DRAWN BY DATE
ENT 06.23.2021

BE PROJECT NUMBER

20041

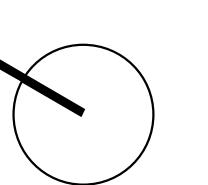
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GREENHOUSE FRAMING
PLAN

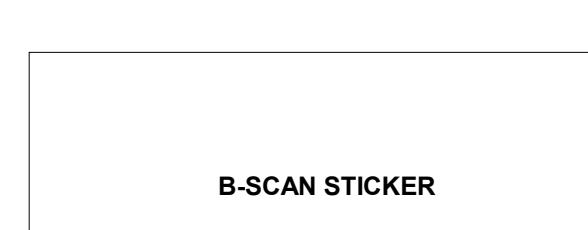
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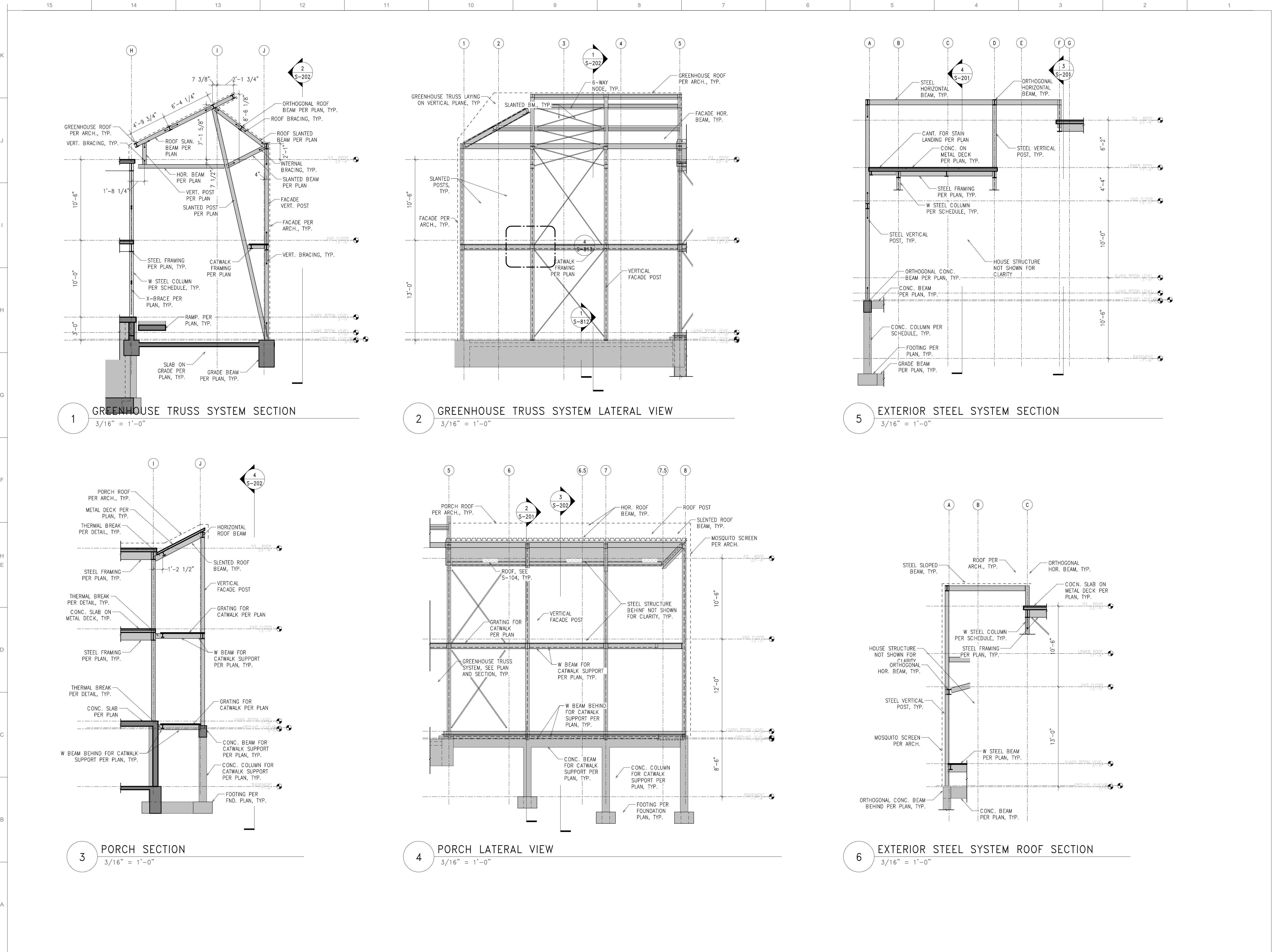
S-105

B-SCAN STICKER



✓

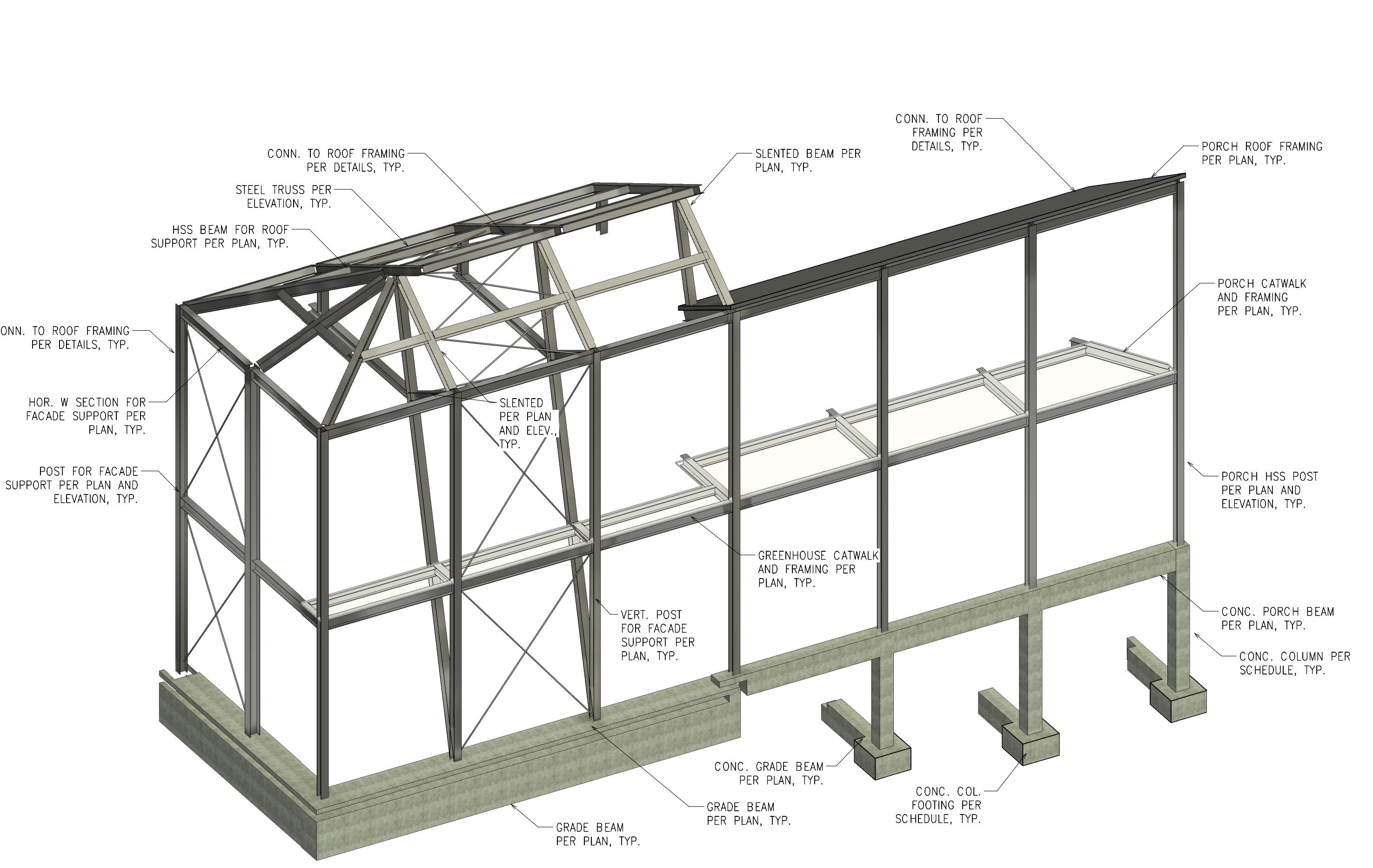




K

100

GREENHOUSE AXONOMETRY



ARCHITE

The logo for scalar Architecture consists of two main graphic elements. On the left, a large, stylized lowercase letter 's' is formed by a continuous pattern of small, repeating 'SS' icons. On the right, a large, solid black triangle is filled with a repeating pattern of small, tilted 'AA' icons.

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KEY PLAN

CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE BEGUN & ANY MATERIALS & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT. SHOP DETAILS MUST BE SUBMITTED. PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

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B-SCAN STICKER

DRAWN BY DATE

FNT 06 23 2021

BE PROJECT NUMBER

20041

2004

THE
OPEN

GREENHOUSE

AXO

DRAWING NO.

DRAWING

22

1

S-211

15 | 14 | 13 | 12 | 11 | 10 | 9 | 8 | 7 | 6 | 5 | 4 | 3 | 2 | 1

K		J		I		H		G		F		E		D		C		B		A					
ROOF																									
23'-6"	DEAD LOAD =	LIVE LOAD =																							
LOWER ROOF																									
16'-10"	DEAD LOAD =	LIVE LOAD =																							
2ND FLOOR																									
13'-0"	DEAD LOAD =	LIVE LOAD =																							
1ST FLOOR (HALL & KITCHEN)																									
3'-0"	DEAD LOAD =	LIVE LOAD =																							
1ST FLOOR (ENTRANCE & LIVING ROOM)																									
1'-0"	DEAD LOAD =	LIVE LOAD =																							
GROUND FLOOR																									
0'-0"	DEAD LOAD =	LIVE LOAD =																							
BASEMENT																									
-7"-6"	BP-4	BP-4	BP-2	BP-2	BP-2	BP-4	BP-4	BP-4	BP-2	BP-4	BP-2	BP-3	BP-3	BP-4	BP-2	BP-3	BP-3	BP-2	BP-1	BP-3	BP-1	BP-1	BP-1	BP-1	
B/PLATE ELEV. (SEE NOTE 1)	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"	-3 1/2"		
COLUMN LOCATION	1/A	1/C	1/E	2/H	3/E	3/H	4/A	4/C	4/E	4/H	5/H	5/I	6/B	6/C	6/D	6/H	6/I	6.5/I	7/B	7/D	7/F	7.5/F	7.5/I	8/B	8/J
	8	4	13	7	14	8	13	7	14	8	13	7	14	8	13	7	14	8	13	7	14	8	13	7	14

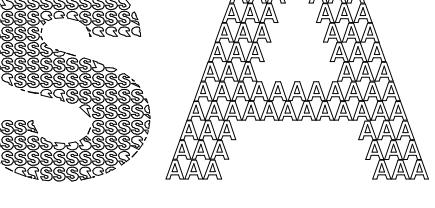
1 COLUMN SCHEDULE

1" = 1'-0"

COLUMN SCHEDULE

NOTES:	CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE BEGUN & ANY MATERIAL & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT. SHOP DETAILS MUST BE SUBMITTED. PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.																								
1. BOTTOM OF STEEL BASE PLATE IS PROVIDED FROM TOP OF CONCRETE PIER OR GRADE BEAM ELEVATION, U.O.N.																									
2. DEAD AND LIVE AXIAL LOADS ARE NOT PROVIDED FOR CONCRETE PIERS ENCASED INTO FOUNDATION WALLS																									
3. REFER TO TYP. COLUMN DETAILS FOR BALANCE OF INFO.																									
4. LOADS INDICATED IN SCHEDULE ARE IN KIPS.																									

CLIENT: Owner
Name _____
Address _____
Enter address here
Address _____

ARCHITECT:

scalar Architecture
195 Chrystie Street 600E
New York, New York 10002
T: 646.342.2244
F: 212.260.2247

OWNER: Owner
Name _____
Address _____
Address _____
T: _____
F: _____

CONSTRUCTION MANAGER:
Name _____
Address _____
Address _____
T: _____
F: _____

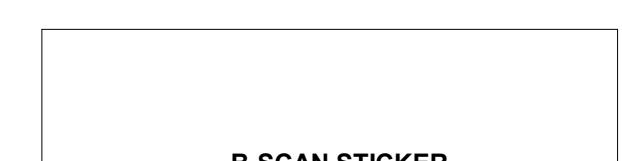
CIVIL ENGINEER:
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Address _____
Address _____
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F: _____
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MEP ENGINEER:
Name _____
Address _____
Address _____
T: _____
F: _____
KEY PLAN

CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE BEGUN & ANY MATERIAL & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT. SHOP DETAILS MUST BE SUBMITTED. PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

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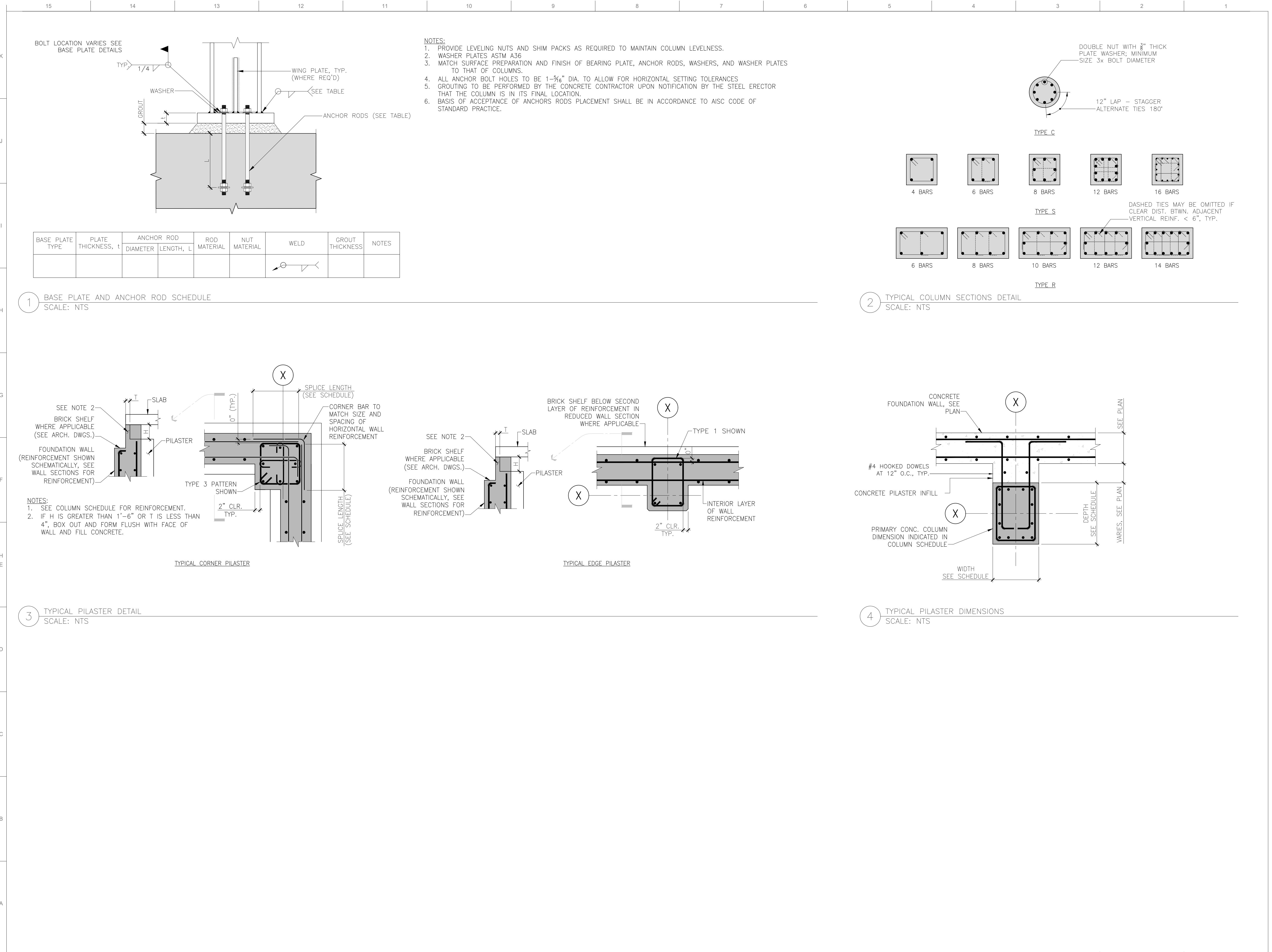
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sa PROJECT NUMBER
20041
TITLE

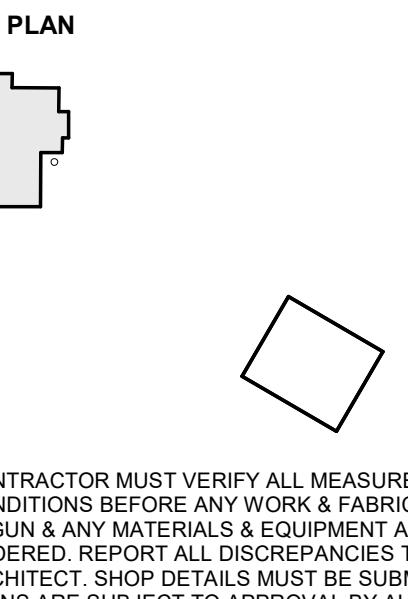
COLUMN SCHEDULE
SCALE
1" = 1'-0"
DRAWING NO.

S-301



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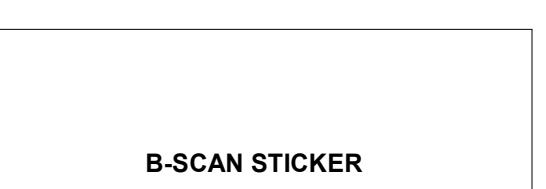
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DATE 05.04.2021

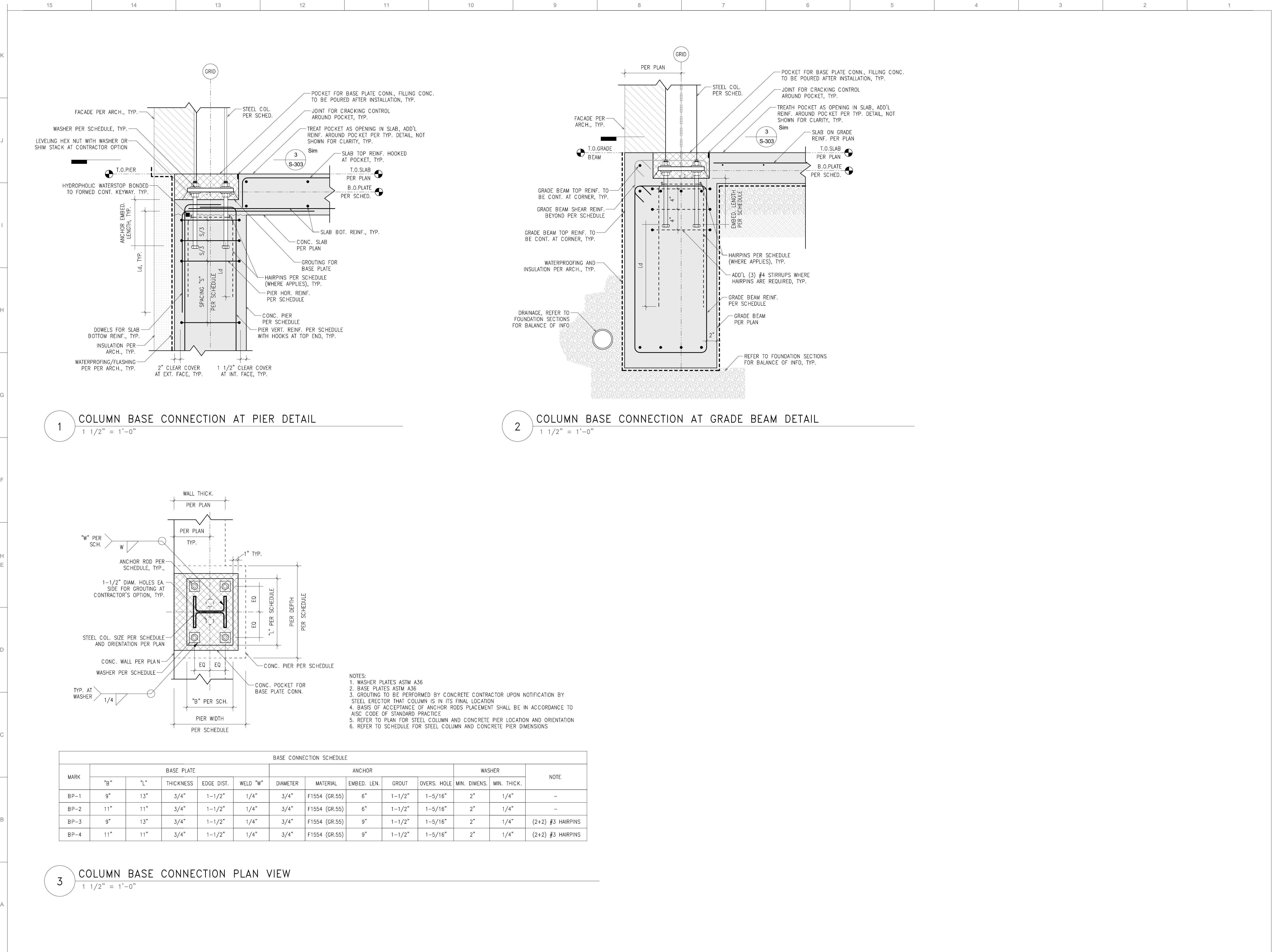
BE PROJECT NUMBER 20041

TITLE

**TYPICAL CONCRETE
COLUMN DETAILS**

DRAWING NO.

S-302



ARCHITECT:
SA
scalar Architecture
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KEY PLAN

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PRINTING **DATE**

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DRAWN BY **DATE**
ENT 06.23.2021

BE PROJECT NUMBER

20041

TITLE

**BASE PLATE TYPICAL
DETAILS**
DRAWING NO.
S-303

15 | 14 | 13 | 12 | 11 | 10 | 9 | 8 | 7 | 6 | 5 | 4 | 3 | 2 | 1

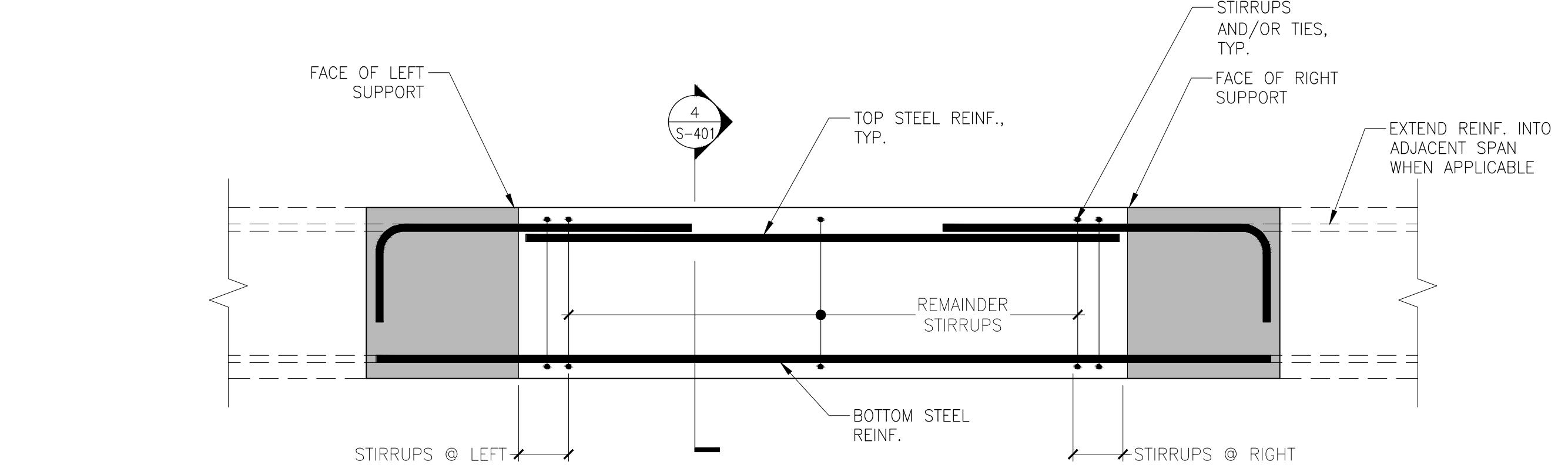
K

CONCRETE BEAM SCHEDULE													
DESIGN ID	SIZE		TOP REINFORCEMENT		BOTTOM REINFORCEMENT		SIDE REINFORCEMENT		SHEAR REINFORCEMENT		REMAINDER	REMARKS	
	B (IN)	D (IN)	# BARS AND BAR NO. LEFT END	# BARS AND BAR NO. RIGHT END	# BARS	BAR NO.	# BARS	BAR NO.	LEGS	BAR NO.	SPACING		
BM-1	10	30	2 # 5	2 # 5	2	#5	-	-	2	#4	12"		
BM-2	12	18	3 # 5	2 # 5	2	#5	-	-	2	#3	6"		
BM-3	12	18	2 # 5	2 # 5	2	#5	-	-	2	#3	6"		
BM-4	12	42	3 # 5	3 # 5	3	#5	-	-	2	#4	12"		
BM-5	12	12	2 # 5	2 # 5	2	#5	-	-	2	#3	5"		
BM-6	12	18	2 # 5	2 # 5	2	#5	-	-	2	#3	6"		
BM-7	12	18	2 # 5	2 # 5	2	#5	-	-	2	#3	6"		
BM-8	12	18	2 # 5	2 # 5	2	#5	-	-	2	#3	6"		

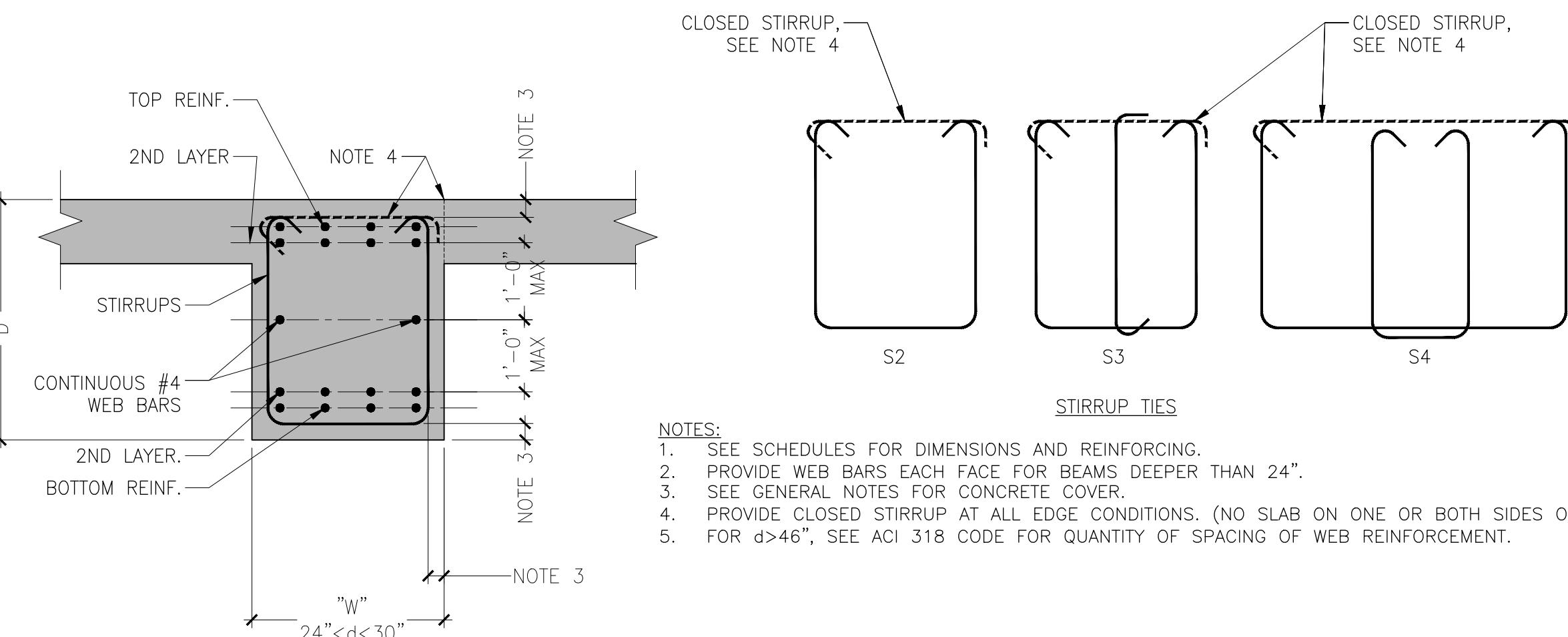
- NOTES:
1. B = MINIMUM BEAM WIDTH
 2. D = MINIMUM BEAM DEPTH
 3. R = RIGHT
 4. L = LEFT
 5. STIRRUP ENDS ARE DEFINED BY ORIENTATION OF LABEL IN PLAN AS FOLLOWS: (LEFT END) CB-# (RIGHT END)
 6. REFER TO TYPICAL CONCRETE BEAM DETAIL FOR ADDITIONAL INFORMATION.

1 CONCRETE BEAM SCHEDULE

SCALE: NTS



(2) TYPICAL CONCRETE BEAM LAYOUT
SCALE: NTS

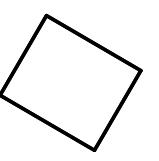
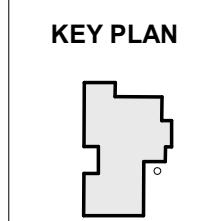


- NOTES:
1. SEE SCHEDULES FOR DIMENSIONS AND REINFORCING.
 2. PROVIDE WEB BARS EACH FACE FOR BEAMS DEEPER THAN 24".
 3. SEE GENERAL NOTES FOR CONCRETE COVER.
 4. PROVIDE CLOSED STIRRUP AT ALL EDGE CONDITIONS. (NO SLAB ON ONE OR BOTH SIDES OF BEAM).
 5. FOR d>46", SEE ACI 318 CODE FOR QUANTITY OF SPACING OF WEB REINFORCEMENT.

(3) TYPICAL BEAM REINFORCEMENT
SCALE: NTS

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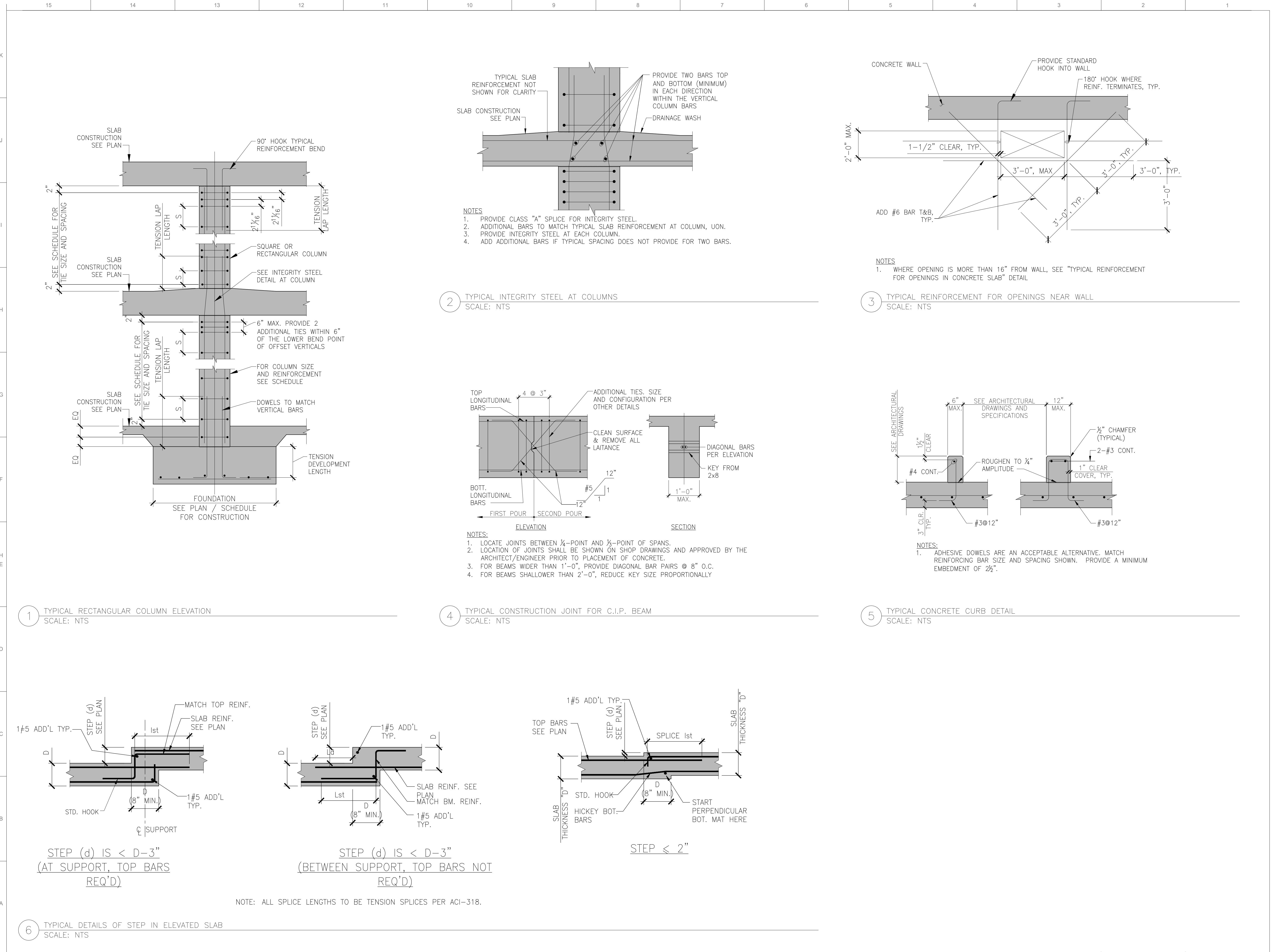
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BE PROJECT NUMBER
20041

TITLE
TYPICAL CONCRETE BEAM

DETAILS
DRAWING NO.

S-401



S-601

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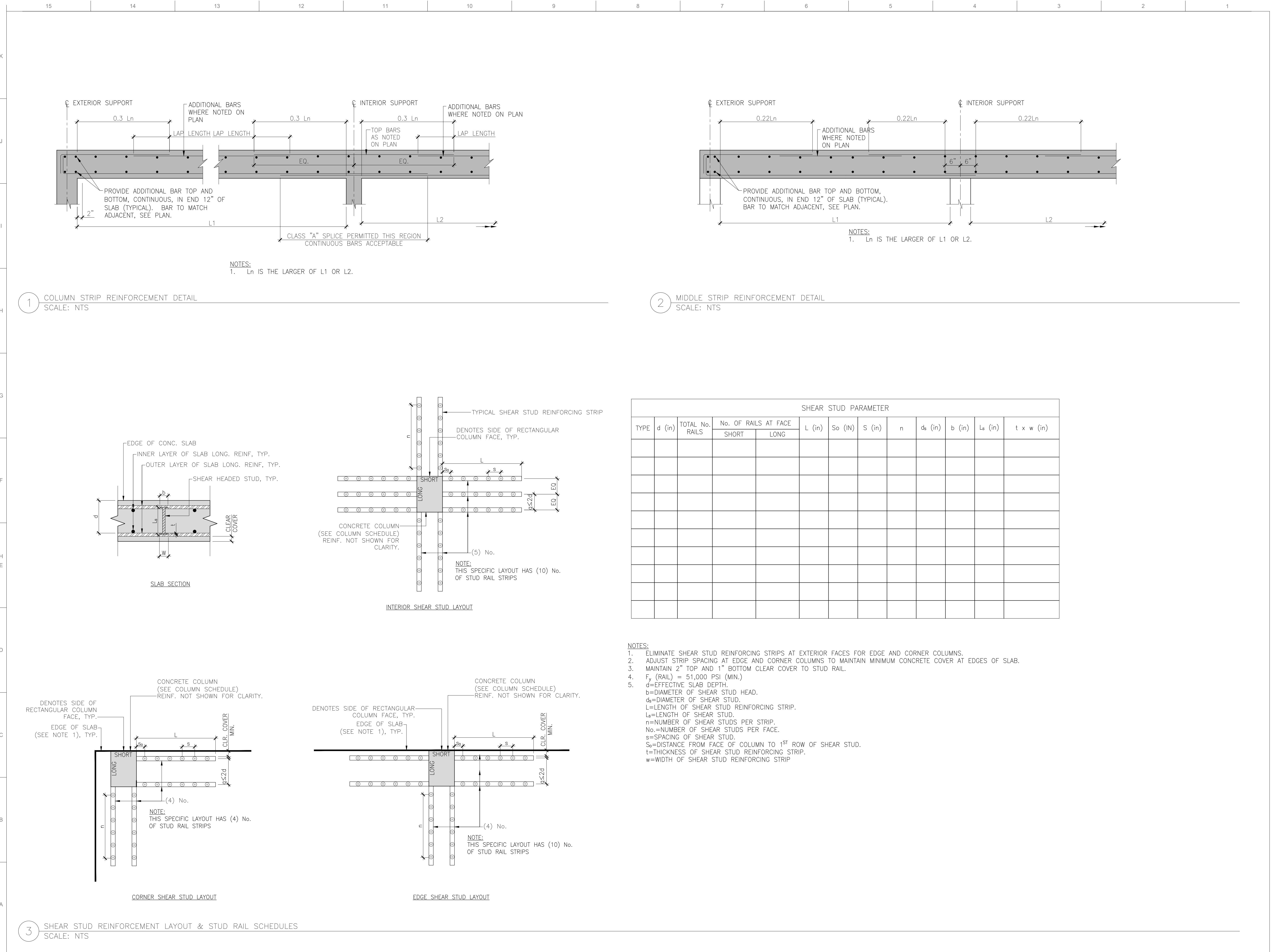
BE PROJECT NUMBER

20041

TITLE

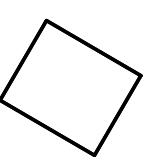
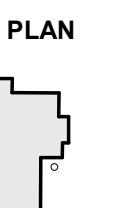
TYPICAL CONCRETE DETAILS

DRAWING NO.



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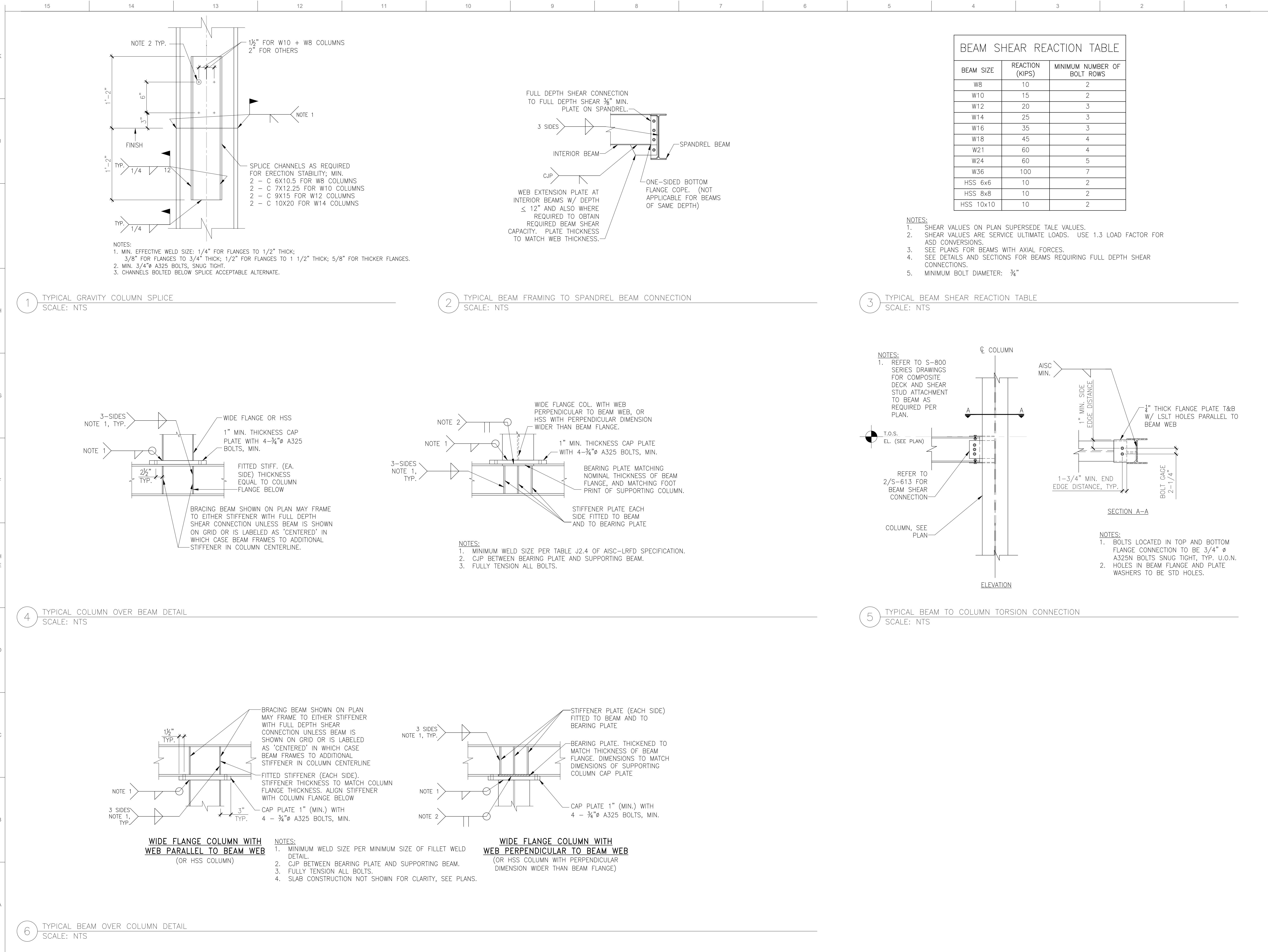
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BE PROJECT NUMBER
20041
TITLE

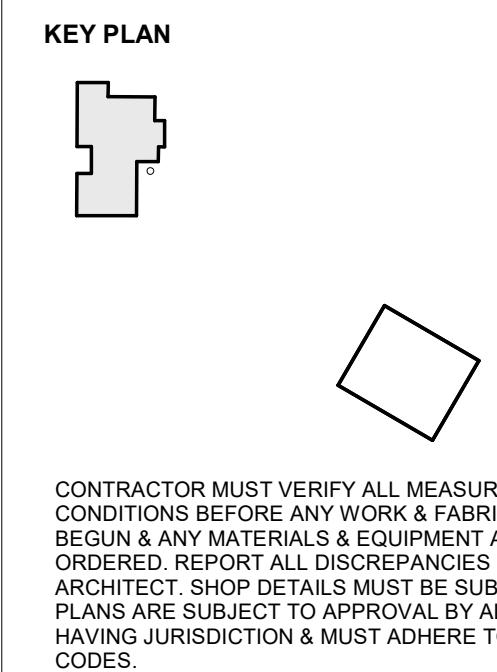
**TYPICAL CONCRETE
DETAILS**
DRAWING NO.

S-602



ARCHITECT:
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CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE BEGUN & ANY MATERIALS & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT & STRUCTURAL ENGINEER AS SOON AS POSSIBLE. PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

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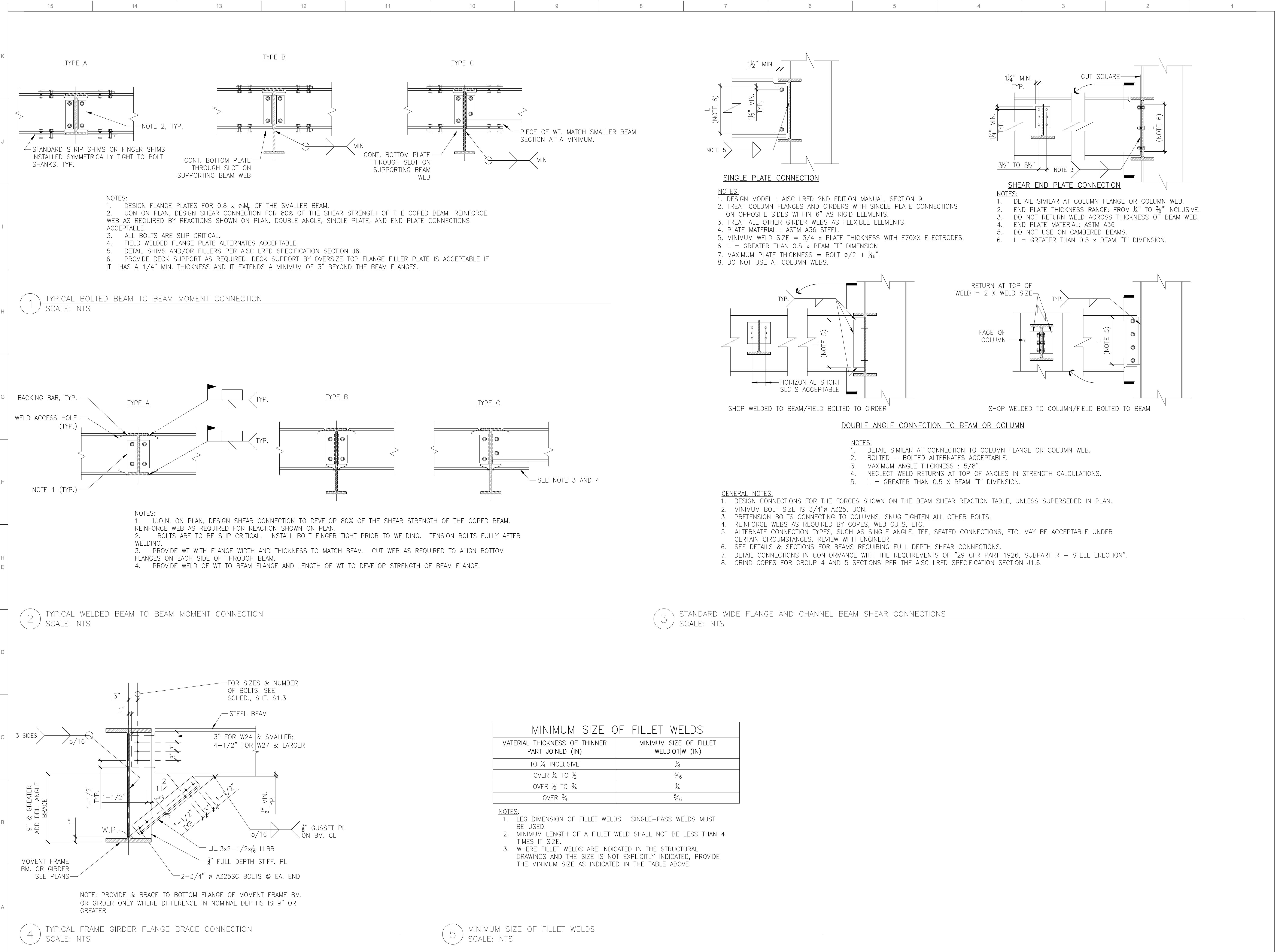


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ENT 05.04.2021
BE PROJECT NUMBER
20041
TITLE

STEEL TYPICAL DETAILS
SCALE

DRAWING NO.

S-801



STRUCTURAL ENGINEER:

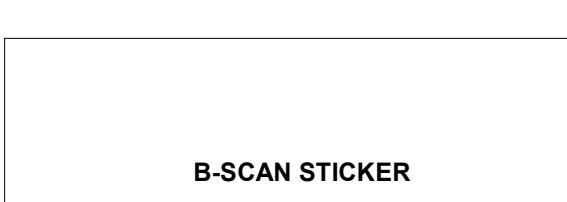
BÜRO EHRING

CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE UN & ANY MATERIALS & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT. SHOP DETAILS MUST BE SUBMITTED. PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

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DRAWN BY **DATE**

ENT 05.04

20041

2004 |

STEEL TYPICAL DETAIL

STER

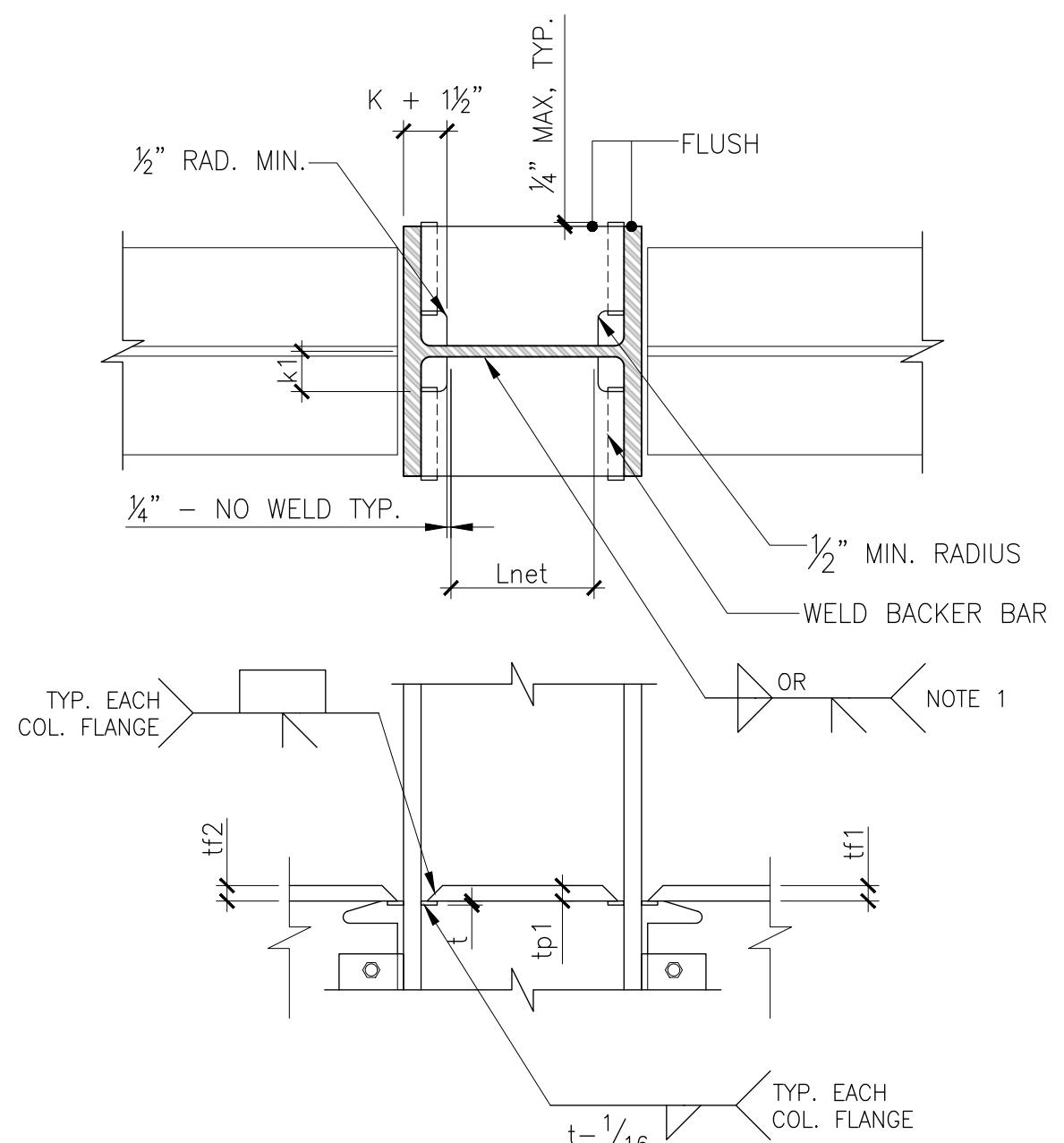
SCALE

DRAWING NO.

S-802

15 | 14 | 13 | 12 | 11 | 10 | 9 | 8 | 7 | 6 | 5 | 4 | 3 | 2 | 1

K



CONTINUITY PLATE NOTES:
1. REQUIRED TOTAL STRENGTH = $0.6(tp_1)(L_{net})F_y$.
2. tp_1 = GREATER OF tf_1 OR tf_2 .
3. t = THICKNESS OF BACKING BAR.

1 TYPICAL SEISMIC MOMENT FRAME CONNECTION - 1
SCALE: NTS

2 TYPICAL SEISMIC MOMENT FRAME CONNECTION - 2
SCALE: NTS

3 TYPICAL SEISMIC MOMENT FRAME CONNECTION - 3
SCALE: NTS

NOTES:
1. DETAIL IS SIMILAR FOR COLUMN ON WEAK AXIS.
2. CONNECTION FORCES ARE THE GREATER OF THOSE SHOWN ON PLAN AND ON NOTES 3 & 4.
3. CANTILEVER CONNECTION FORCES:
M = 65% OF FLEXURAL STRENGTH OF THE CANTILEVER BEAM DEFINED AS FIYIS TIMES THE PLASTIC MODULES OF THE SECTION
V = 50% OF THE SHEAR STRENGTH OF THE CANTILEVER BEAM DEFINED AS FIYIS X D X TIWIS
4. BACKSPAN CONNECTION FORCES:
M = SAME AS CANTILEVER SPAN
V = PER "BEAM SHEAR REACTION TABLE AND CONNECTION NOTES"
5. MATCH CONTINUITY PLATE WIDTH AND THICKNESS WITH CONTACT AREA OF CONNECTION FLANGE PLATES. SLOPE BOTTOM PLATES FOR UP TO 4" DIFFERENCE IN BEAM NOMINAL DEPTHS, USE TWO BOTTOM CONTINUITY PLATES FOR GREATER DIFFERENCE IN DEPTH.
6. CONNECTION SHOWN AS A SINGLE BEVEL EDGE PREPARATION, DOUBLE BEVEL EDGE PREPARATION AT FABRICATOR'S DISCRETION.
7. PJP OR FILLET WELD AS REQUIRED TO TRANSFER FLANGE PLATE FORCES TO COLUMN WEB.
8. FULL CONTACT FILLER PLATES AS REQUIRED.
9. SHEAR CONNECTION ON FULL DEPTH SINGLE PLATE AT CANTILEVER END AND ON CONNECTION DESIGNED PER THE STANDARD WIDE FLANGE BEAM SHEAR CONNECTIONS TYPICAL DETAIL AT THE BACKSPAN END.
10. BOLTED MOMENT CONNECTION AT CANTILEVER AND BACKSPAN EXCEPT WHERE BACKSPAN IS MOMENT CONNECTED AT BOTH ENDS AS PART OF A MOMENT FRAME.
11. ALL BOLTS DESIGNED AS PRETENSIONED, $\frac{7}{8}$ " Ø A325 MINIMUM BOLTS.

15 | 14 | 13 | 12 | 11 | 10 | 9 | 8 | 7 | 6 | 5 | 4 | 3 | 2 | 1

4 STIFFENER PLATES MOMENT FRAME CONNECTION
SCALE: NTS

5 TYPICAL CANTILEVER WIDE FLANGE BEAM CONNECTION
SCALE: NTS

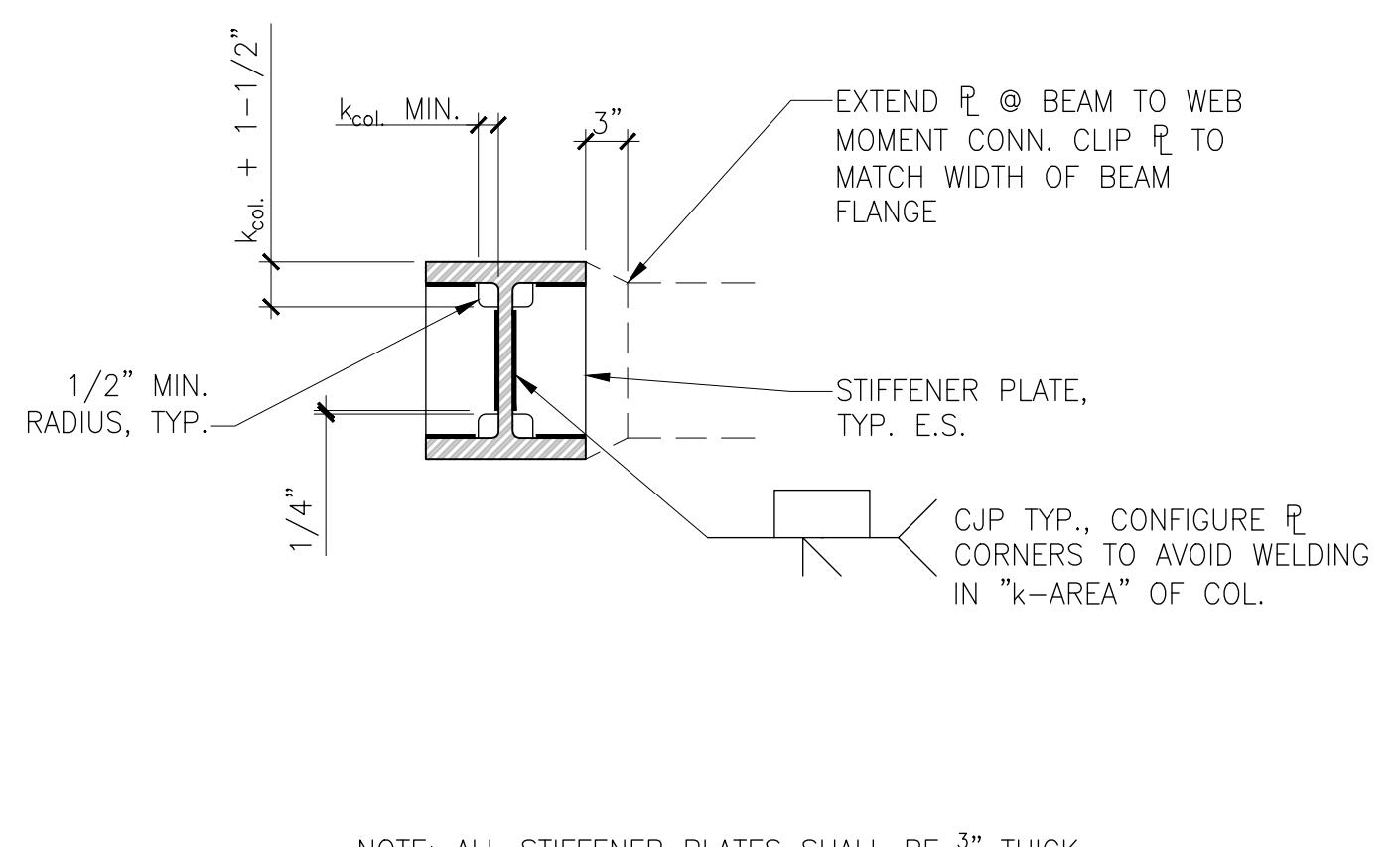
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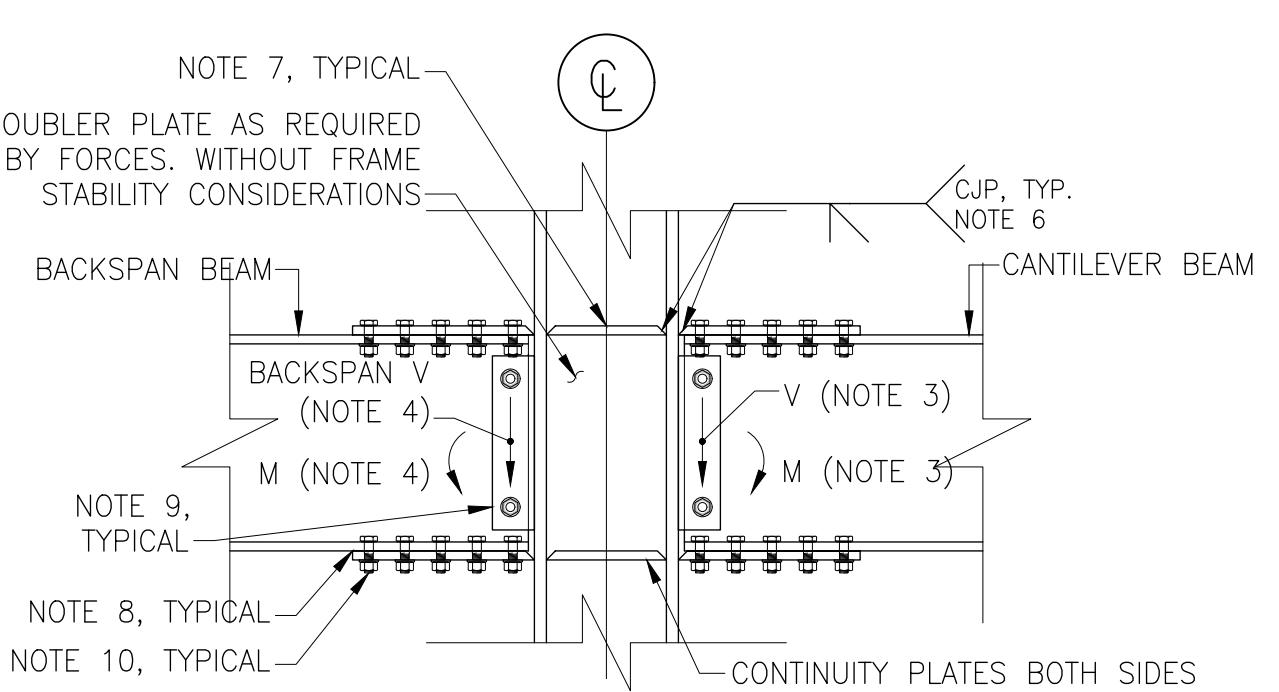
STRUCTURAL ENGINEER:

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KEY PLAN



NOTE: ALL STIFFENER PLATES SHALL BE $\frac{3}{4}$ " THICK



CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE BEGUN & ANY MATERIALS & EQUIPMENT ARE ORDERED. REPORT ALL INCIDENCES TO THE ARCHITECT. SUPPORT DETAILS MUST BE SUBMITTED. PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

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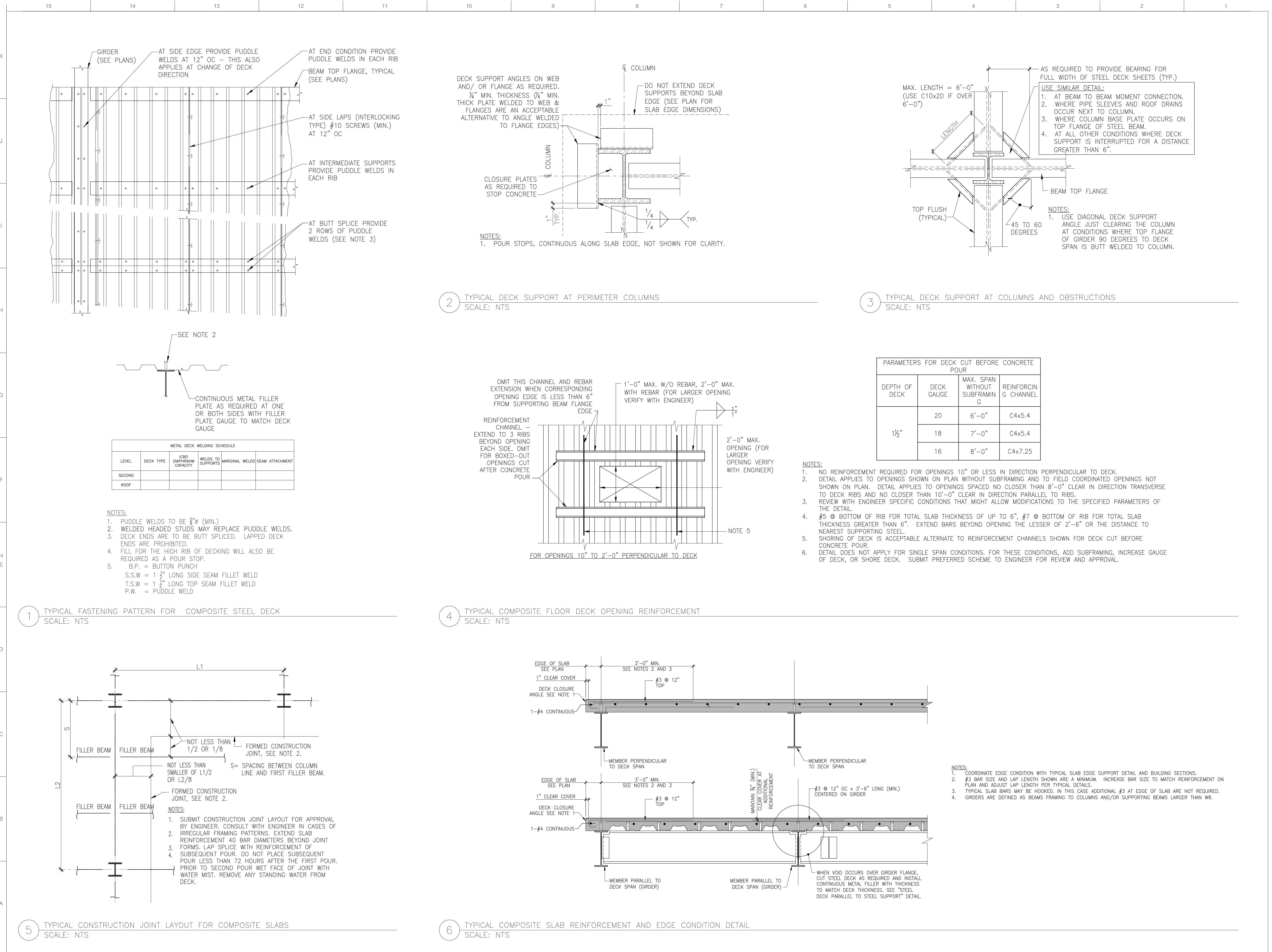
STEEL TYPICAL DETAILS

SCALE

AS SHOWN

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B-SCAN STICKER

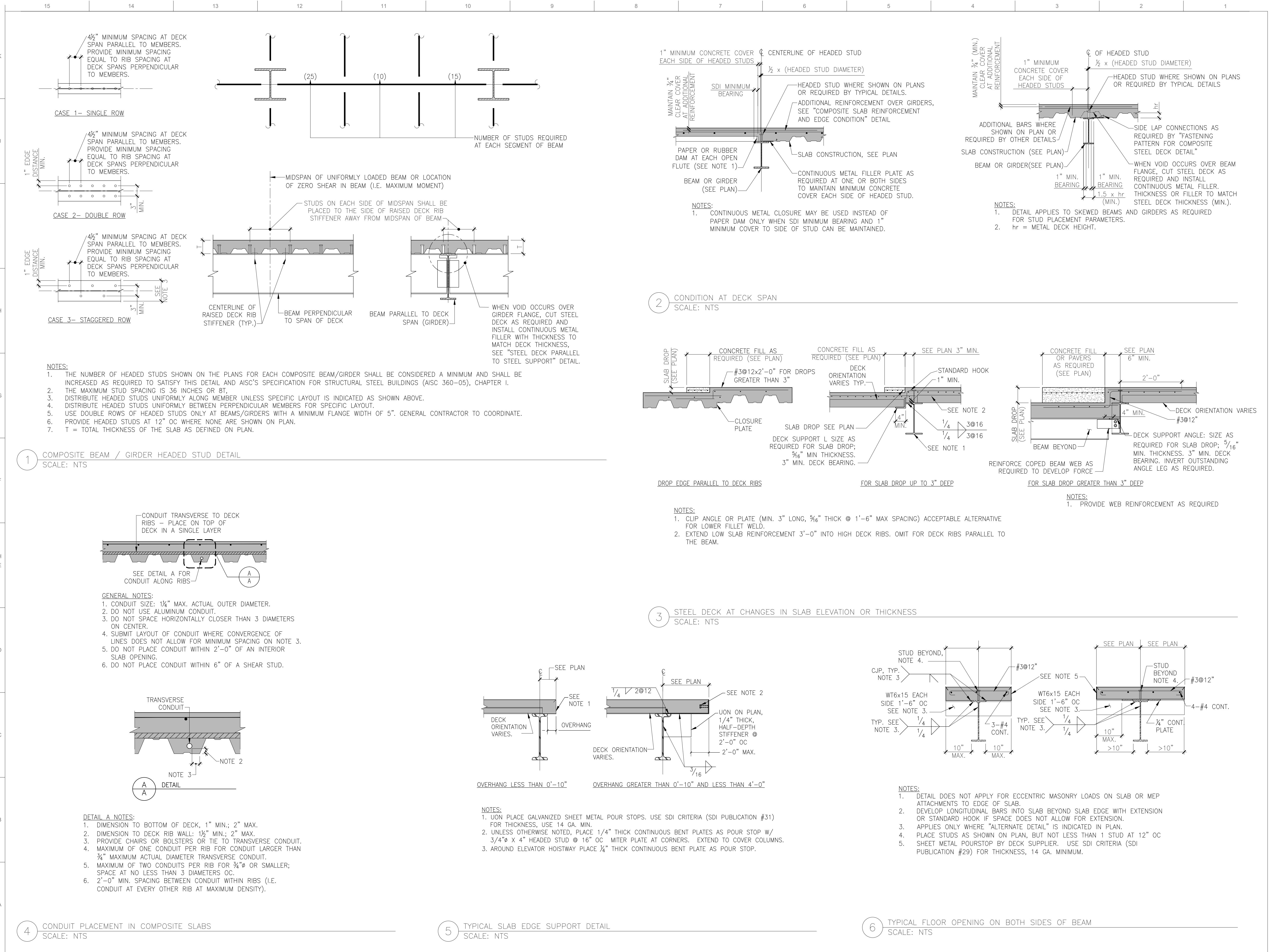
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20041

TITLE
STEEL TYPICAL DETAILS
SCALE

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S-804



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CONTRACTOR MUST VERIFY ALL MEASUREMENTS & CONDITIONS BEFORE ANY WORK & FABRICATION ARE BEGUN & ANY MATERIALS & EQUIPMENT ARE ORDERED. REPORT ALL DISCREPANCIES TO THE ARCHITECT & ENGINEER AS SOON AS POSSIBLE. PLANS ARE SUBJECT TO APPROVAL BY ALL AGENCIES HAVING JURISDICTION & MUST ADHERE TO THEIR CODES.

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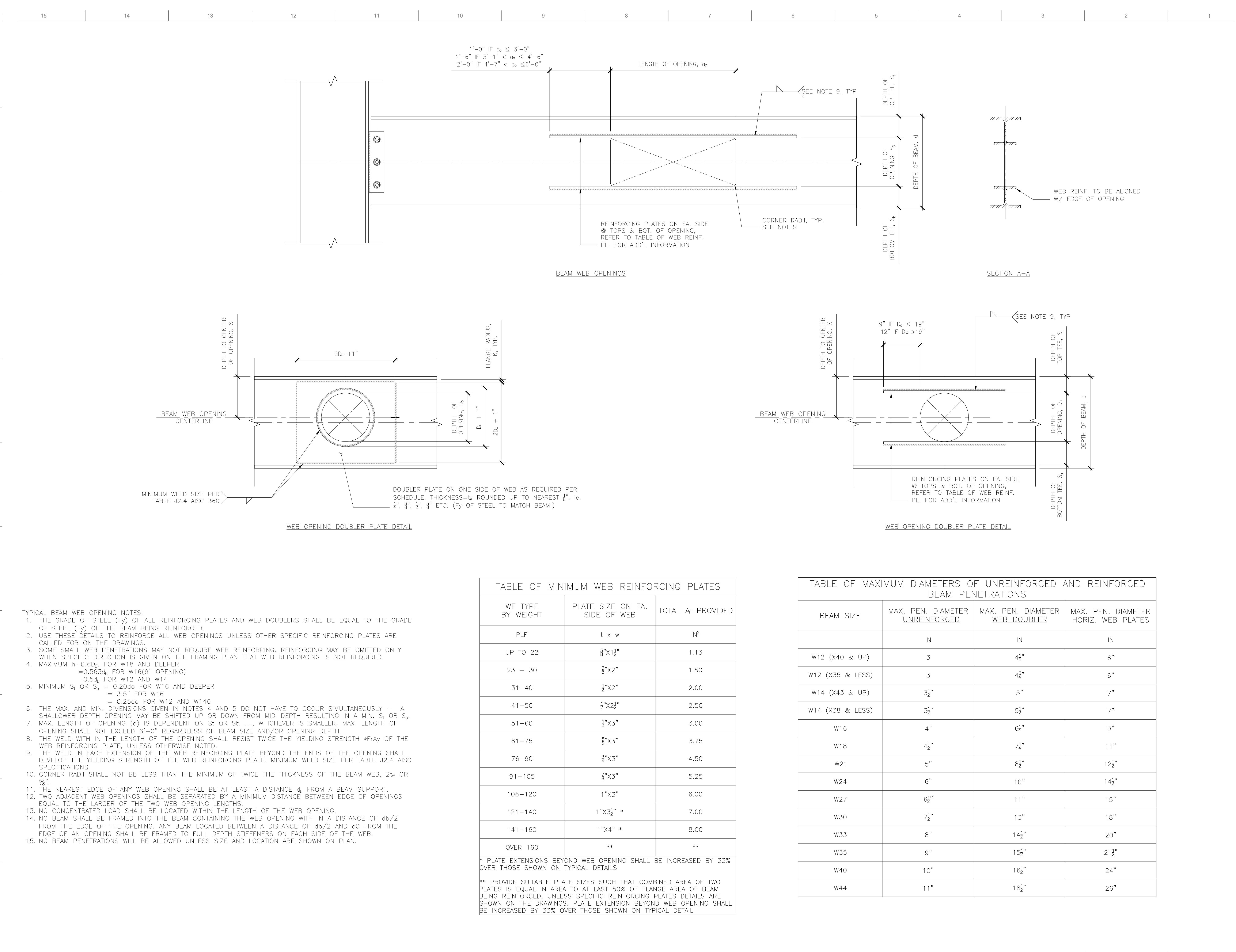
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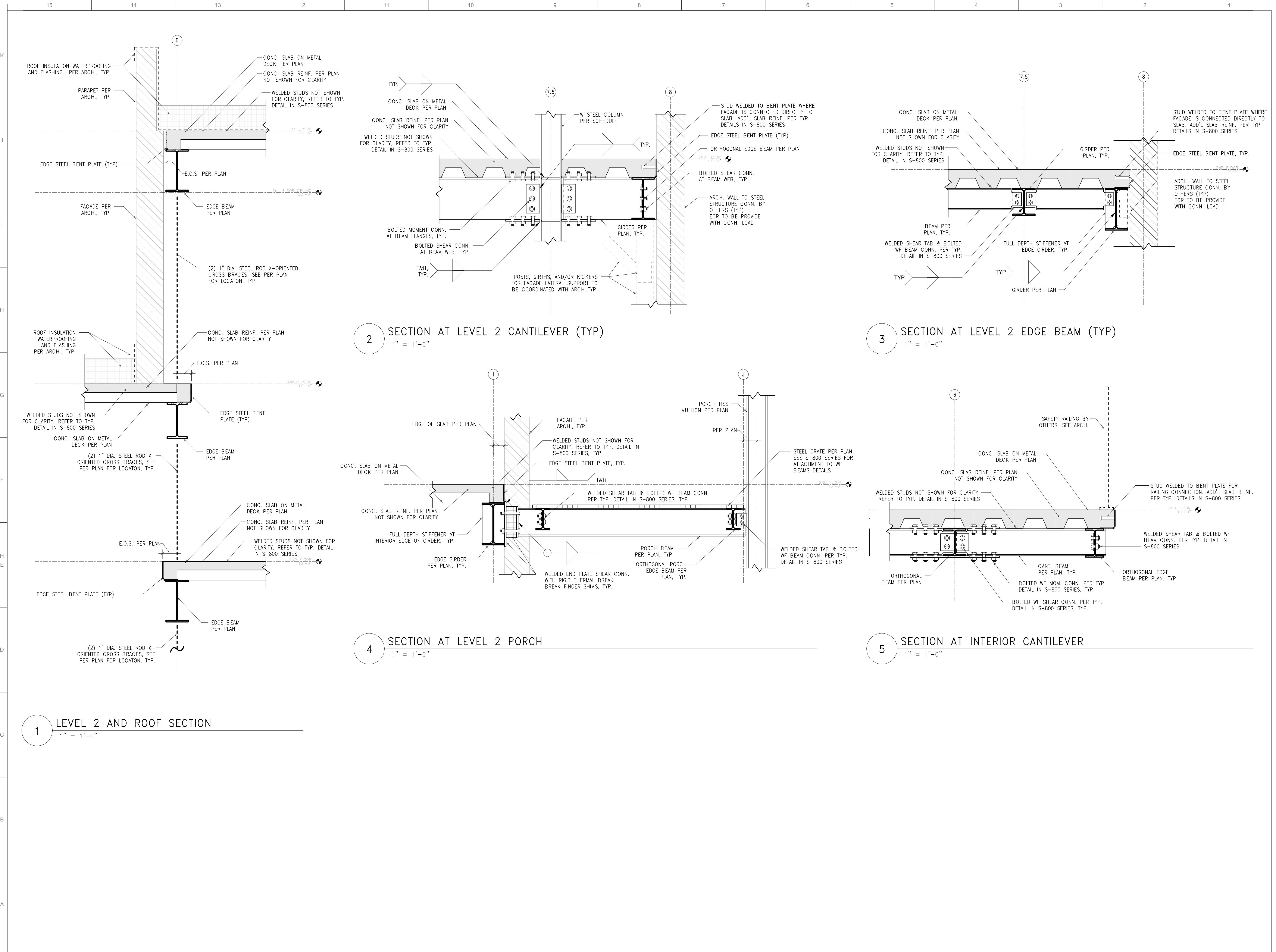
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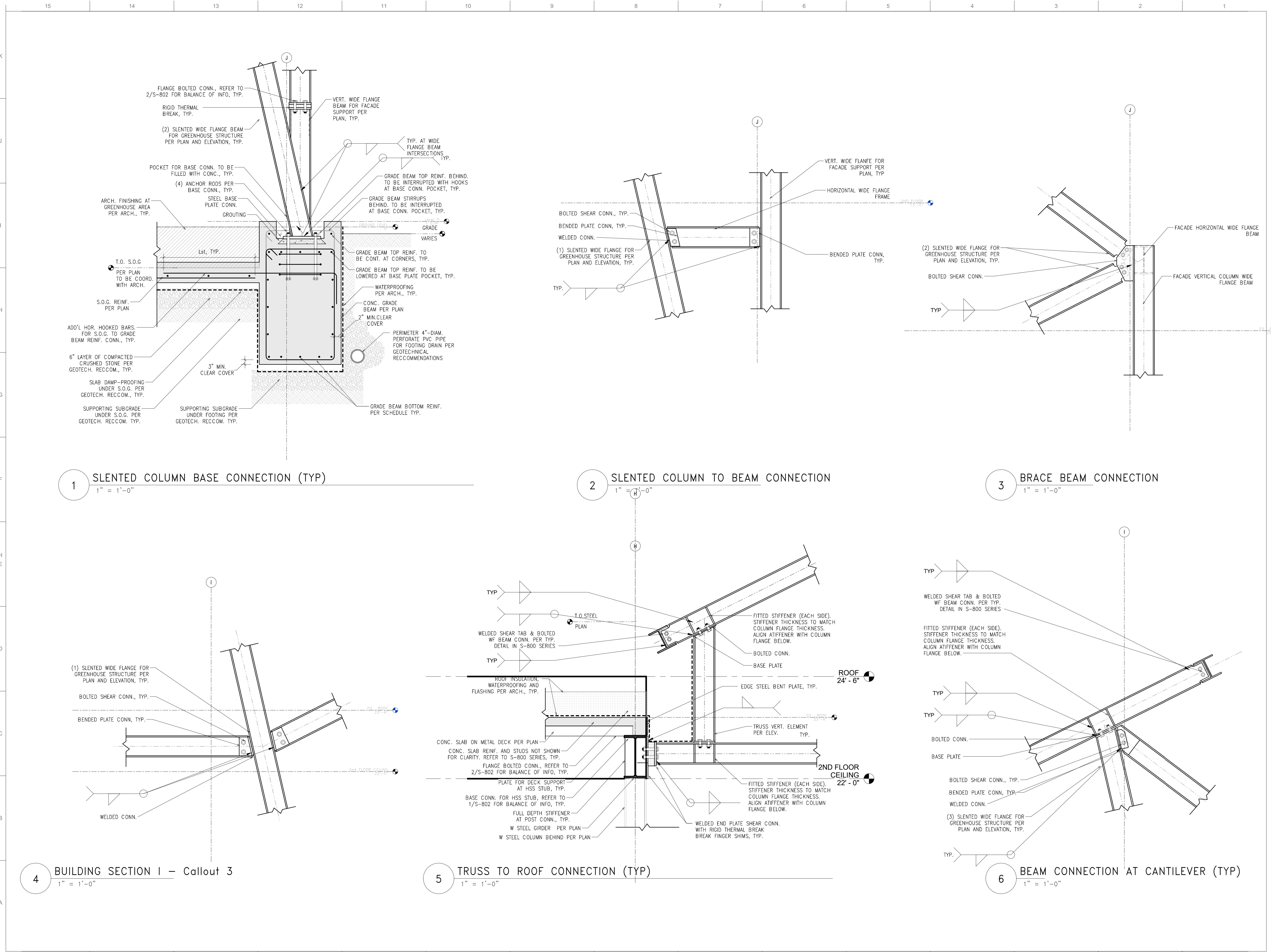
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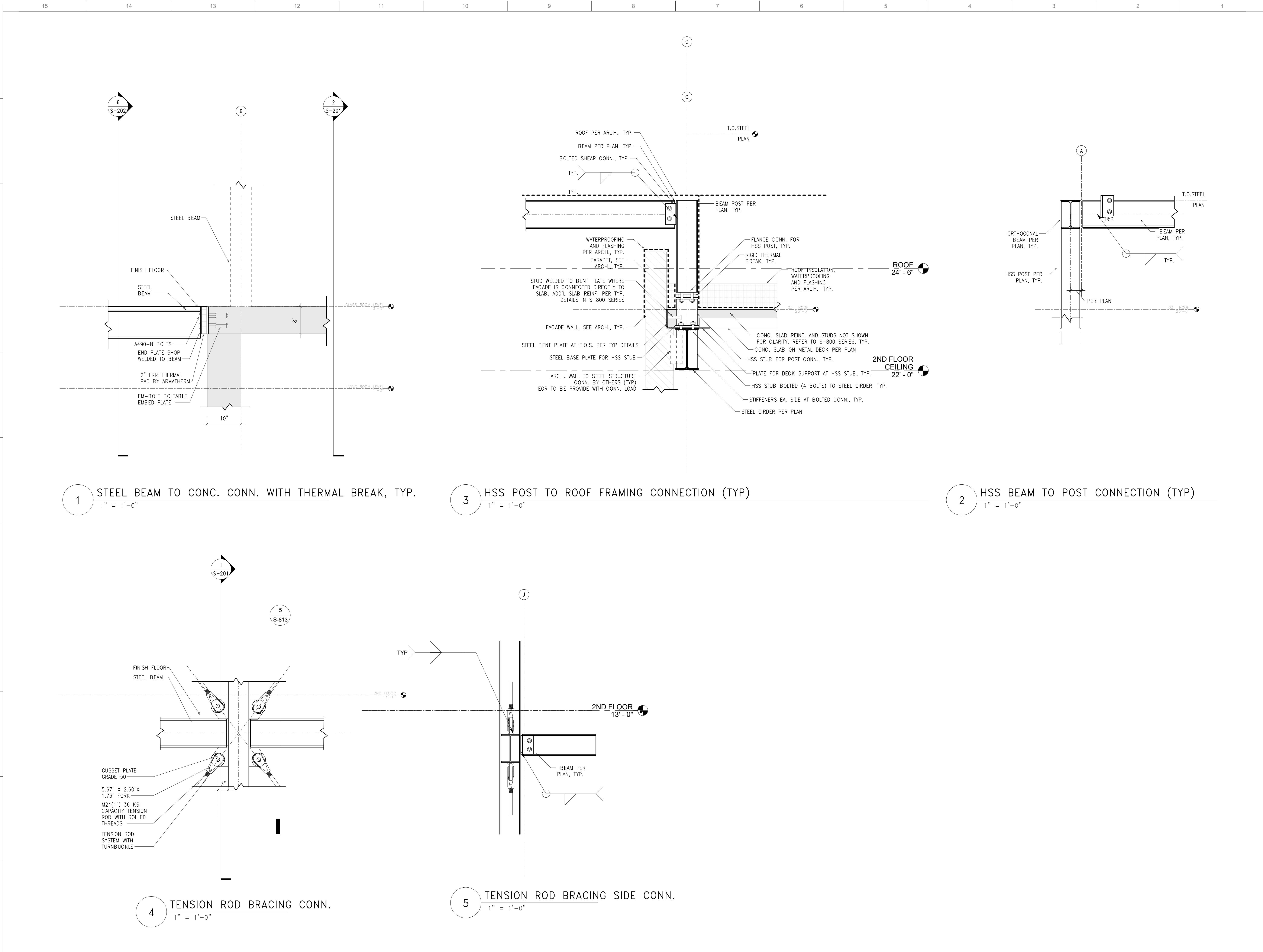
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