

*The CHAIN\_2D Code for Simulating Two-Dimensional  
Movement of Water, Heat, and Multiple Solutes  
in Variably-Saturated Porous Media*

Version 1.1

Research Report No. 136

October, 1994

U. S. SALINITY LABORATORY  
AGRICULTURAL RESEARCH SERVICE  
U. S. DEPARTMENT OF AGRICULTURE  
RIVERSIDE, CALIFORNIA

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J. Šimunek and M. Th. van Genuchten

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## DISCLAIMER

This report documents the CHAIN\_2D computer program for simulating two-dimensional variably-saturated water flow, heat movement, and the transport of solutes involved in sequential first-order decay reactions. CHAIN\_2D is a public domain code, and as such may be used and copied freely. The code has been verified against a large number of test cases. However, no warranty is given that the program is completely error-free. If you do encounter problems with the code, find errors, or have suggestions for improvement, please contact one of the authors at

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## ABSTRACT

Šimůnek, J. and M. Th. van Genuchten. 1994. The CHAIN\_2D Code for Simulating Two-Dimensional Movement of Water, Heat, and Multiple Solutes in Variably-Saturated Porous Media, Version 1.1. Research Report No. 136, U. S. Salinity Laboratory, USDA, ARS, Riverside, California.

This report documents the CHAIN\_2D computer program for simulating two-dimensional variably saturated water flow, heat transport, and the movement of solutes involved in sequential first-order decay reactions. The program numerically solves the Richards' equation for saturated-unsaturated water flow and the convection-dispersion equation for heat and solute transport. The flow equation incorporates a sink term to account for water uptake by plant roots. The water flow part of the model can deal with prescribed head, gradient, and flux boundaries, as well as boundaries controlled by atmospheric conditions. Free drainage boundary condition and a simplified representation of nodal drains using results of electric analog experiments is also included. The heat transport equation considers transport due to conduction and convection with flowing water. The solute transport equations consider convective-dispersive transport in the liquid phase, as well as diffusion in the gaseous phase. The transport equations also include provisions for nonlinear nonequilibrium reactions between the solid and liquid phases, linear equilibrium reactions between the liquid and gaseous phases, zero-order production, and two first-order degradation reactions: one which is independent of other solutes, and one which provides the coupling between solutes involved in the sequential first-order decay reactions. The program may be used to analyze water and solute movement in unsaturated, partially saturated, or fully saturated porous media. CHAIN\_2D can handle flow regions delineated by irregular boundaries. The flow region itself may be composed of nonuniform soils having an arbitrary degree of local anisotropy. Flow and transport can occur in the vertical plane, the horizontal plane, or in a three-dimensional region exhibiting radial symmetry about the vertical axis.

The governing flow and transport equations are solved numerically using Galerkin-

type linear finite element schemes. Depending upon the size of the problem, the matrix equations resulting from discretization of the governing equations are solved using either Gaussian elimination for banded matrices or the conjugate gradient method for symmetric matrices, or the ORTHOMIN method for asymmetric matrices. The program is written in ANSI standard FORTRAN 77. Computer memory is a function of the problem definition. This report serves as both a user manual and reference document. Detailed instructions are given for data input preparation. Example input and selected output files are also provided.

## TABLE OF CONTENTS

DISCLAIMER .....	iii
ABSTRACT .....	v
TABLE OF CONTENTS .....	vii
LIST OF FIGURES .....	xi
LIST OF TABLES .....	xv
LIST OF VARIABLES .....	xix
1. INTRODUCTION .....	1
2. VARIABLY SATURATED WATER FLOW .....	5
2.1. <i>Governing Flow Equation</i> .....	5
2.2. <i>Root Water Uptake</i> .....	5
2.3. <i>The Unsaturated Soil Hydraulic Properties</i> .....	8
2.4. <i>Scaling of the Soil Hydraulic Functions</i> .....	11
2.4.1. <i>Spatial variability of the Soil Hydraulic Functions</i> .....	11
2.4.2. <i>Temperature Dependence of the Soil Hydraulic Functions</i> .....	12
2.5. <i>Initial and Boundary Conditions</i> .....	13
3. EQUILIBRIUM TRANSPORT OF SOLUTES INVOLVED IN SEQUENTIAL FIRST-ORDER DECAY REACTIONS .....	17
3.1. <i>Governing Solute Transport Equations</i> .....	17
3.2. <i>Initial and Boundary Conditions</i> .....	23
3.3. <i>Effective Dispersion Coefficient</i> .....	25
3.4. <i>Temperature Dependence of Transport and Reaction Coefficients</i> .....	26
4. HEAT TRANSPORT .....	27
4.1. <i>Governing Heat Transport Equations</i> .....	27
4.2. <i>Apparent Thermal Conductivity Coefficient</i> .....	27
4.3. <i>Initial and Boundary Conditions</i> .....	28
5. NUMERICAL SOLUTION OF THE WATER FLOW EQUATION .....	31
5.1. <i>Space Discretization</i> .....	31
5.2. <i>Time Discretization</i> .....	35
5.3. <i>Numerical Solution Strategy</i> .....	35
5.3.1. <i>Iterative Process</i> .....	35
5.3.2. <i>Treatment of the Water Capacity Term</i> .....	36
5.3.3. <i>Time Control</i> .....	37

5.3.4. <i>Treatment of Pressure Head Boundary Conditions</i> . . . . .	38
5.3.5. <i>Flux and Gradient Boundary Conditions</i> . . . . .	38
5.3.6. <i>Atmospheric Boundary Conditions and Seepage Faces</i> . . . . .	39
5.3.7. <i>Tile Drains as Boundary Conditions</i> . . . . .	39
5.3.8. <i>Water Balance Computations</i> . . . . .	41
5.3.9. <i>Computation of Nodal Fluxes</i> . . . . .	42
5.3.10. <i>Water Uptake by Plant Roots</i> . . . . .	43
5.3.11. <i>Evaluation of the Soil Hydraulic Properties</i> . . . . .	43
5.3.12. <i>Implementation of Hydraulic Conductivity Anisotropy</i> . . . . .	44
5.3.13. <i>Steady-State Analysis</i> . . . . .	44
<b>6. NUMERICAL SOLUTION OF THE SOLUTE TRANSPORT EQUATION . . . . .</b>	<b>47</b>
6.1. <i>Space Discretization</i> . . . . .	47
6.2. <i>Time Discretization</i> . . . . .	49
6.3. <i>Numerical Solution Strategy</i> . . . . .	50
6.3.1. <i>Solution Process</i> . . . . .	50
6.3.2. <i>Upstream Weighted Formulation</i> . . . . .	51
6.3.3. <i>Implementation of First-Type Boundary Conditions</i> . . . . .	52
6.3.4. <i>Implementation of Third-Type Boundary Conditions</i> . . . . .	53
6.3.5. <i>Mass Balance Calculations</i> . . . . .	55
6.3.6. <i>Oscillatory Behavior</i> . . . . .	57
<b>7. PROBLEM DEFINITION . . . . .</b>	<b>59</b>
7.1. <i>Construction of Finite Element Mesh</i> . . . . .	59
7.2. <i>Coding of Soil Types and Subregions</i> . . . . .	60
7.3. <i>Coding of Boundary Conditions</i> . . . . .	61
7.4. <i>Program Memory Requirements</i> . . . . .	66
7.5. <i>Matrix Equation Solvers</i> . . . . .	68
<b>8. EXAMPLE PROBLEMS . . . . .</b>	<b>71</b>
8.1. <i>Example 1 - Column Infiltration Test</i> . . . . .	72
8.2. <i>Example 2 - Water Flow in a Field Soil Profile Under Grass</i> . . . . .	76
8.3. <i>Example 3 - Two-Dimensional Unidirectional Solute Transport</i> . . . . .	82
8.4. <i>Example 4 - One-Dimensional Solute Transport with Nitrification Chain</i> . . . . .	86
8.5. <i>Example 5 - One-Dimensional Solute Transport with Nonlinear Cation Adsorption</i> . . . . .	90
8.6. <i>Example 6 - One-Dimensional Solute Transport with Nonequilibrium Adsorption</i> . . . . .	94
8.7. <i>Example 7 - Water and Solute Infiltration Test</i> . . . . .	96
<b>9. INPUT DATA . . . . .</b>	<b>107</b>
9.1. <i>Description of Data Input Blocks</i> . . . . .	107
9.2. <i>Example Input Files</i> . . . . .	128

10. OUTPUT DATA .....	145
10.1. <i>Description of Data Output Files</i> .....	145
10.2. <i>Example Output Files</i> .....	155
11. PROGRAM ORGANIZATION .....	167
11.1. <i>Description of Program Units</i> .....	167
11.2. <i>List of Significant CHAIN_2D Program Variables</i> .....	173
12. REFERENCES .....	187
APPENDIX - MESH GENERATOR .....	A-1



## LIST OF FIGURES

<u>Figure</u>	<u>Page</u>
<b>Fig. 2.1.</b> Schematic of the plant water stress response function, $\alpha_r(h)$ , as used by <i>Feddes et al. [1978]</i> .....	6
<b>Fig. 2.2.</b> Schematic of the potential water uptake distribution function, $b(x,z)$ , in the soil root zone .....	7
<b>Fig. 2.3.</b> Schematics of the soil water retention (a) and hydraulic conductivity (b) functions as given by equations (2.11) and (2.12), respectively .....	10
<b>Fig. 8.1.</b> Flow system and finite element mesh for example 1 .....	72
<b>Fig. 8.2.</b> Retention and relative hydraulic conductivity functions for example 1. The open circles are UNSAT2 input data [ <i>Davis and Neuman, 1983</i> ] .....	74
<b>Fig. 8.3.</b> Instantaneous, $q_0$ , and cumulative, $I_0$ , infiltration rates simulated with the CHAIN_2D (solid lines) and UNSAT2 (triangles) codes for example 1 .....	75
<b>Fig. 8.4.</b> Flow system and finite element mesh for example 2 .....	76
<b>Fig. 8.5.</b> Unsaturated hydraulic properties of the first and second soil layers for example 2 .....	78
<b>Fig. 8.6.</b> Precipitation and potential transpiration rates for example 2 .....	79
<b>Fig. 8.7.</b> Cumulative values for the actual transpiration and bottom discharge rates for example 2 as simulated by CHAIN_2D (solid line) and SWATRE (triangles) .....	80
<b>Fig. 8.8.</b> Pressure head at the soil surface and mean pressure head of the root zone for example 2 as simulated by CHAIN_2D (solid lines) and SWATRE (solid circles) .....	81
<b>Fig. 8.9.</b> Location of the groundwater table versus time for example 2 as simulated by CHAIN_2D (solid line) and SWATRE (open circles) computer programs .....	82

Fig. 8.10.	Advancement of the concentration front ( $c=0.1$ ) for example 3a as calculated with CHAIN_2D (dotted lines) and the analytical solution (solid lines) . . . . .	84
Fig. 8.11.	Concentration profile at the end of the simulation ( $t=365$ days) for example 3a as calculated with CHAIN_2D (dotted lines) and the analytical solution (solid lines) . . . . .	85
Fig. 8.12.	Advancement of the concentration front ( $c=0.1$ ) for example 3b as calculated by CHAIN_2D (dotted lines) and the analytical solution (solid lines) . . . . .	85
Fig. 8.13.	Concentration profile at the end of the simulation ( $t=365$ days) for example 3b as calculated with CHAIN_2D (dotted line) and the analytical solution (solid lines) . . . . .	86
Fig. 8.14.	Analytically and numerically calculated concentration profiles for $\text{NH}_4^+$ , $\text{NO}_2^-$ , and $\text{NO}_3^-$ after 200 hours for example 4. . . . .	88
Fig. 8.15.	Analytically and numerically calculated concentration profiles for $\text{NH}_4^+$ (top), $\text{NO}_2^-$ (middle), and $\text{NO}_3^-$ (bottom) after 50, 100, and 200 hours for example 4. . . . .	89
Fig. 8.16.	Mg breakthrough curves for Abist loam calculated with the MONOD, HYDRUS, and CHAIN_2D codes (data points from Selim <i>et al.</i> [1987]) (example 5). . . . .	91
Fig. 8.17.	Ca breakthrough curves for Abist loam calculated with the MONOD and CHAIN_2D codes (data points from Selim <i>et al.</i> [1987]) (example 5). . . . .	93
Fig. 8.18.	Observed and calculated effluent curves for Boron movement through Glendale clay loam (data points from <i>van Genuchten</i> [1981]) (example 6). . . . .	95
Fig. 8.19.	Flow system and finite element mesh for example 7 . . . . .	100
Fig. 8.20.	Initial (top) and steady state (bottom) pressure head profiles for example 7 . . . . .	101
Fig. 8.21.	Temperature profiles after 1 (top) and 10 days (bottom) for example 7 . . . . .	102

<b>Fig. 8.22.</b>	Concentration profiles for the first solute after 2.5, 5, 7.5, and 10 days for example 7. ....	103
<b>Fig. 8.23.</b>	Concentration profiles for the second solute after 2.5, 5, 7.5, and 10 days for example 7. ....	104
<b>Fig. 8.24.</b>	Concentration profiles for the third solute after 2.5, 5, 7.5, and 10 days for example 7. ....	105



## LIST OF TABLES

<u>Table</u>	<u>Page</u>
Table 7.1. Initial settings of $Kode(n)$ , $Q(n)$ , and $h(n)$ for constant boundary conditions .....	61
Table 7.2. Initial settings of $Kode(n)$ , $Q(n)$ , and $h(n)$ for variable boundary conditions .....	62
Table 7.3. Definition of the variables $Kode(n)$ , $Q(n)$ , and $h(n)$ when an atmospheric boundary condition is applied .....	63
Table 7.4. Definition of the variables $Kode(n)$ , $Q(n)$ , and $h(n)$ when variable head or flux boundary conditions are applied .....	63
Table 7.5. Initial setting of $Kode(n)$ , $Q(n)$ , and $h(n)$ for seepage faces .....	65
Table 7.6. Initial setting of $Kode(n)$ , $Q(n)$ , and $h(n)$ for drains .....	65
Table 7.7. List of array dimensions in CHAIN_2D .....	67
Table 8.1. Input parameters for example 3 .....	84
Table 8.2. Input parameters for example 4 .....	88
Table 8.3. Input parameters for example 5 .....	90
Table 8.4. Input parameters for example 6 .....	94
Table 8.5. Hydraulic input parameters for example 7 .....	98
Table 8.6. Heat transport input parameters for example 7 .....	98
Table 8.7. Solute transport input parameters for example 7 .....	99
Table 9.1. Block A - Basic information .....	109
Table 9.2. Block B - Material information .....	111
Table 9.3. Block C - Time information .....	112
Table 9.4. Block D - Root water uptake information .....	113
Table 9.5. Block E - Seepage face information .....	114
Table 9.6. Block F - Drainage information .....	115
Table 9.7. Block G - Solute transport information .....	116
Table 9.8. Block H - Heat transport information .....	120
Table 9.9. Block I - Nodal information .....	122
Table 9.10. Block J - Element information .....	124

Table 9.11.	Block K - Boundary geometry information . . . . .	125
Table 9.12.	Block L - Atmospheric information . . . . .	126
Table 9.13.	Input data for example 1 (input file 'SELECTOR.IN') . . . . .	128
Table 9.14.	Input data for example 1 (input file 'GRID.IN') . . . . .	129
Table 9.15.	Input data for example 2 (input file 'SELECTOR.IN') . . . . .	130
Table 9.16.	Input data for example 2 (input file 'ATMOSPH.IN') . . . . .	131
Table 9.17.	Input data for example 2 (input file 'GRID.IN') . . . . .	132
Table 9.18.	Input data for example 3b (input file 'SELECTOR.IN') . . . . .	133
Table 9.19.	Input data for example 3 (input file 'GRID.IN') . . . . .	134
Table 9.20.	Input data for example 4 (input file 'SELECTOR.IN') . . . . .	135
Table 9.21.	Input data for example 4 (input file 'GRID.IN') . . . . .	136
Table 9.22.	Input data for example 5 (input file 'SELECTOR.IN') . . . . .	137
Table 9.23.	Input data for example 5 (input file 'GRID.IN') . . . . .	138
Table 9.24.	Input data for example 6 (input file 'SELECTOR.IN') . . . . .	139
Table 9.25.	Input data for example 6 (input file 'GRID.IN') . . . . .	140
Table 9.26.	Input data for example 7 (input file 'SELECTOR.IN') . . . . .	141
Table 9.27.	Input data for example 7 (input file 'GRID.IN') . . . . .	143
Table 10.1.	H_MEAN.OUT - mean pressure heads . . . . .	148
Table 10.2.	V_MEAN.OUT - mean and total water fluxes . . . . .	149
Table 10.3.	CUM_Q.OUT - total cumulative water fluxes . . . . .	150
Table 10.4.	RUN_INF.OUT - time and iteration information . . . . .	151
Table 10.5.	SOLUTE.OUT - actual and cumulative concentration fluxes . . . . .	152
Table 10.6.	BALANCE.OUT - mass balance variables . . . . .	153
Table 10.7.	A_LEVEL.OUT - mean pressure heads and total cumulative fluxes . . . . .	154
Table 10.8.	Output data for example 1 (part of output file 'H.OUT') . . . . .	155
Table 10.9.	Output data for example 1 (output file 'CUM_Q.OUT') . . . . .	156
Table 10.10.	Output data for example 2 (output file 'RUN_INF.OUT') . . . . .	157
Table 10.11.	Output data for example 2 (part of output file 'A_LEVEL.OUT') . . . . .	158
Table 10.12.	Output data for example 3b (part of output file 'CONC.OUT') . . . . .	159

Table 10.13.	Output data for example 4 (output files 'SOLUTE1.OUT', 'SOLUTE2.OUT', and 'SOLUTE3.OUT') .....	160
Table 10.14.	Output data for example 5 (output file 'CHECK.OUT') .....	161
Table 10.15.	Output data for example 5 (part of output file 'CONC.OUT') .....	162
Table 10.16.	Output data for example 6 (part of output file 'OBSNOD.OUT') .....	163
Table 10.17.	Output data for example 7 (output files 'SOLUTE1.OUT', 'SOLUTE2.OUT', and 'SOLUTE3.OUT') .....	164
Table 10.18.	Output data for example 7 (part of output file 'BALANCE.OUT') .....	165
Table 11.1.	Input subroutines/files .....	168
Table 11.2.	Output subroutines/files .....	170
Table 11.3.	List of significant integer variables .....	173
Table 11.4.	List of significant real variables .....	175
Table 11.5.	List of significant logical variables .....	180
Table 11.6.	List of significant arrays .....	182
Table A.1.	Input file 'GENER2.IN' for finite element mesh generator .....	A-2
Table A.2.	Input file 'GENER2.IN' for the first example .....	A-5
Table A.3.	Input file 'GENER2.IN' for the second example .....	A-6
Table A.4.	Input file 'GENER2.IN' for the third example .....	A-7
Table A.5.	Input file 'GENER2.IN' for the forth example .....	A-8
Table A.6.	Input file 'GENER2.IN' for the fifth example .....	A-9
Table A.7.	Input file 'GENER2.IN' for the sixth example .....	A-10
Table A.8.	Input file 'GENER2.IN' for the seventh example .....	A-11



## LIST OF VARIABLES

$a$	ion activity in soil solution [-]
$a_r$	dimensionless water stress response function [-]
$a_v$	air content [ $L^3L^{-3}$ ]
$\bar{a}$	ion activity on the exchange surfaces [-]
$A$	amplitude of temperature sine wave [K]
$A_e$	area of a triangular element [ $L^2$ ]
$A_{gh}$	parameter in equation (7.1) [ $LT^{-1}$ ]
$[A]$	coefficient matrix in the global matrix equation for water flow, [ $LT^{-1}$ ] or [ $L^2T^{-1}$ ] $^\dagger$
$b$	normalized root water uptake distribution, [ $L^{-2}$ ] or [ $L^{-3}$ ] $^\dagger$
$b'$	arbitrary root water uptake distribution, [ $L^{-2}$ ] or [ $L^{-3}$ ] $^\dagger$
$b_i, c_i$	geometrical shape factors [L]
$b_1, b_2, b_3$	empirical parameters to calculate thermal conductivity $\lambda_0$ [ $MLT^{-3}K^{-1}$ ] (e.g. $Wm^{-1}K^{-1}$ )
$B_{gh}$	parameter in equation (7.1) [ $L^{-1}$ ]
{ $B$ }	vector in the global matrix equation for water flow, [ $L^2T^{-1}$ ] or [ $L^3T^{-1}$ ] $^\dagger$
$c$	solution concentration [ $ML^{-3}$ ]
$c'$	finite element approximation of $c$ [ $ML^{-3}$ ]
$c_i$	initial solution concentration [ $ML^{-3}$ ]
$c_n$	value of the concentration at node $n$ [ $ML^{-3}$ ]
$c_r$	concentration of the sink term [ $ML^{-3}$ ]
$c_0$	prescribed concentration boundary condition [ $ML^{-3}$ ]
$C$	volumetric heat capacity of porous medium [ $ML^{-1}T^{-2}K^{-1}$ ] (e.g. $Jm^{-3}K^{-1}$ )
$C_d$	factor used to adjust the hydraulic conductivity of elements in the vicinity of drains [-]
$C_g$	volumetric heat capacity of gas phase [ $ML^{-1}T^{-2}K^{-1}$ ] (e.g. $Jm^{-3}K^{-1}$ )
$C_n$	volumetric heat capacity of solid phase [ $ML^{-1}T^{-2}K^{-1}$ ] (e.g. $Jm^{-3}K^{-1}$ )
$C_o$	volumetric heat capacity of organic matter [ $ML^{-1}T^{-2}K^{-1}$ ] (e.g. $Jm^{-3}K^{-1}$ )
$C_T$	total solution concentration [ $ML^{-3}$ ] ( $mmol_c l^{-1}$ )
$C_w$	volumetric heat capacity of liquid phase [ $ML^{-1}T^{-2}K^{-1}$ ] (e.g. $Jm^{-3}K^{-1}$ )

$Cr_i^e$	local Courant number [-]
$d$	thickness of stagnant boundary layer [L]
$d_e$	effective drain diameter [L]
$D$	side length of the square in the finite element mesh surrounding a drain (elements have adjusted hydraulic conductivities) [L]
$D_g$	ionic or molecular diffusion coefficient in the gas phase [ $L^2T^{-1}$ ]
$D_{ij}$	effective dispersion coefficient tensor in the soil matrix [ $L^2T^{-1}$ ]
$D_{ij}^g$	diffusion coefficient tensor for the gas phase [ $L^2T^{-1}$ ]
$D_{ij}^w$	dispersion coefficient tensor for the liquid phase [ $L^2T^{-1}$ ]
$D_L$	longitudinal dispersivity [L]
$D_T$	transverse dispersivity [L]
$D_w$	ionic or molecular diffusion coefficient in free water [ $L^2T^{-1}$ ]
$\{D\}$	vector in the global matrix equation for water flow, [ $L^2T^{-1}$ ] or [ $L^3T^{-1}$ ] <sup>†</sup>
$e_n$	subelements which contain node $n$ [-]
$E$	maximum (potential) rate of infiltration or evaporation under the prevailing atmospheric conditions [ $LT^{-1}$ ]
$E_a$	activation energy of a reaction or process [ $ML^2T^{-2}M^{-1}$ ] ( $m^2s^{-2}mol^{-1}$ )
$f$	fraction of exchange sites assumed to be at equilibrium with the solution concentration [-]
$\{f\}$	vector in the global matrix equation for solute transport, [ $MT^{-1}L^{-1}$ ] or [ $MT^{-1}$ ] <sup>†</sup>
$[F]$	coefficient matrix in the global matrix equation for water flow, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup>
$g$	gas concentration [ $ML^{-3}$ ]
$g_{atm}$	gas concentration above the stagnant boundary layer [ $ML^{-3}$ ]
$\{g\}$	vector in the global matrix equation for solute transport, [ $MT^{-1}L^{-1}$ ] or [ $MT^{-1}$ ] <sup>†</sup>
$[G]$	coefficient matrix in the global matrix equation for solute transport, [ $L^2T^{-1}$ ] or [ $L^3T^{-1}$ ] <sup>†</sup>
$h$	pressure head [L]
$h^*$	scaled pressure head [L]
$h'$	finite element approximation of $h$ [L]
$h_A$	minimum pressure head allowed at the soil surface [L]
$h_n$	nodal values of the pressure head [L]

$h_{ref}$	pressure head at reference temperature $T_{ref}$ [L]
$h_s$	air-entry value in the soil water retention function [L]
$h_s$	maximum pressure head allowed at the soil surface [L]
$h_T$	pressure head at soil temperature $T$ [L]
$h_0$	initial condition for the pressure head [L]
$k$	$k$ th chain number [-]
$k_g$	empirical constant relating the solution and gas concentrations [-]
$k_s$	empirical constant relating the solution and adsorbed concentrations [ $L^3 M^{-1}$ ]
$K$	unsaturated hydraulic conductivity [ $LT^{-1}$ ]
$K^*$	scaled unsaturated hydraulic conductivity [ $LT^{-1}$ ]
$K^A$	dimensionless anisotropy tensor for the unsaturated hydraulic conductivity $K$ [-]
$K_{drain}$	adjusted hydraulic conductivity in the elements surrounding a drain [ $LT^{-1}$ ]
$K_{ex}$	dimensionless thermodynamic equilibrium constant [-]
$K_H$	Henry's Law constant [ $MT^2 M^{-1} L^{-2}$ ]
$K_{ij}^A$	components of the dimensionless anisotropy tensor $K^A$ [-]
$K_k$	measured value of the unsaturated hydraulic conductivity at $\theta_k$ [ $LT^{-1}$ ]
$K_r$	relative hydraulic conductivity [-]
$K_{ref}$	hydraulic conductivity at reference temperature $T_{ref}$ [ $LT^{-1}$ ]
$K_s$	saturated hydraulic conductivity [ $LT^{-1}$ ]
$K_T$	hydraulic conductivity at soil temperature $T$ [ $LT^{-1}$ ]
$K_v$	Vanselow selectivity coefficient [-]
$K_{12}$	selectivity coefficient [-]
$L$	length of the side of an element [L]
$L_i$	local coordinate [-]
$L_n$	length of a boundary segment [L]
$L_t$	width of soil surface associated with transpiration, [L] or [ $L^2$ ] <sup>†</sup>
$L_x$	width of the root zone [L]
$L_z$	depth of the root zone [L]
$m$	parameter in the soil water retention function [-]

$M^0$	cumulative amount of solute removed from the flow region by zero-order reactions, $[ML^{-1}]$ or $[M]^\dagger$
$M^1$	cumulative amount of solute removed from the flow region by first-order reactions, $[ML^{-1}]$ or $[M]^\dagger$
$M_r$	cumulative amount of solute removed from the flow region by root water uptake, $[ML^{-1}]$ or $[M]^\dagger$
$M_t$	amount of solute in the flow region at time $t$ , $[ML^{-1}]$ or $[M]^\dagger$
$M_e^t$	amount of solute in element $e$ at time $t$ , $[ML^{-1}]$ or $[M]^\dagger$
$M_0$	amount of solute in the flow region at the beginning of the simulation, $[ML^{-1}]$ or $[M]^\dagger$
$M_0^e$	amount of solute in element $e$ at the beginning of the simulation, $[ML^{-1}]$ or $[M]^\dagger$
$n$	exponent in the soil water retention function [-]
$n_i$	components of the outward unit vector normal to boundary $\Gamma_N$ or $\Gamma_G$ [-]
$n_s$	number of solutes involved in the chain reaction [-]
$N$	total number of nodes [-]
$N_e$	number of subelements $e_n$ which contain node $n$ [-]
$O$	actual rate of inflow/outflow to/from a subregion, $[L^2T^{-1}]$ or $[L^3T^{-1}]^\dagger$
$Pe_i^e$	local Peclet number [-]
$q_i$	components of the Darcian fluid flux density $[LT^{-1}]$
$Q_n^A$	convective solute flux at node $n$ , $[MT^{-1}L^{-1}]$ or $[MT^{-1}]^\dagger$
$Q_n^D$	dispersive solute flux at node $n$ , $[MT^{-1}L^{-1}]$ or $[MT^{-1}]^\dagger$
$Q_n^T$	total solute flux at node $n$ , $[MT^{-1}L^{-1}]$ or $[MT^{-1}]^\dagger$
$\{Q\}$	vector in the global matrix equation for water flow, $[L^2T^{-1}]$ or $[L^3T^{-1}]^\dagger$
$[Q]$	coefficient matrix in the global matrix equation for solute transport, $[L^2]$ or $[L^3]^\dagger$
$R$	retardation factor [-]
$R_u$	universal gas constant $[ML^2T^{-2}K^{-1}M^{-1}]$ ( $= 8.314 \text{ kg m}^2\text{s}^{-2}\text{K}^{-1}\text{mol}^{-1}$ )
$s$	adsorbed solute concentration [-]
$S$	sink term $[T^{-1}]$
$S_e$	degree of saturation [-]
$S_{ek}$	degree of saturation at $\theta_k$ [-]
$S_p$	spatial distribution of the potential transpiration rate $[T^{-1}]$

$S_r$	cation exchange capacity [ $\text{MM}^{-1}$ ] ( $\text{mmol}_{\text{c}}\text{kg}^{-1}$ )
$[S]$	coefficient matrix in the global matrix equation for solute transport, [ $\text{L}^2\text{T}^{-1}$ ] or [ $\text{L}^3\text{T}^{-1}$ ] <sup>†</sup>
$t$	time [T]
$t^*$	local time within the time period $t_p$ [T]
$t_p$	period of time covering one complete cycle of the temperature sine wave [T]
$T$	temperature [K]
$T_a$	actual transpiration rate per unit surface length [ $\text{LT}^{-1}$ ]
$\bar{T}$	average temperature at soil surface during period $t_p$ [K]
$T^A$	absolute temperature [K]
$T_i$	initial temperature [K]
$T_p$	potential transpiration rate [ $\text{LT}^{-1}$ ]
$T_r^A$	reference absolute temperature [K] ( $293.15\text{K}=20^\circ\text{C}$ )
$T_0$	prescribed temperature boundary condition [K]
$v$	average pore-water velocity [ $\text{LT}^{-1}$ ]
$V$	volume of water in each subregion, [ $\text{L}^2$ ] or [ $\text{L}^3$ ] <sup>†</sup>
$V_{new}$	volume of water in each subregion at the new time level, [ $\text{L}^2$ ] or [ $\text{L}^3$ ] <sup>†</sup>
$V_{old}$	volume of water in each subregion at the previous time level, [ $\text{L}^2$ ] or [ $\text{L}^3$ ] <sup>†</sup>
$V_t$	volume of water in the flow domain at time $t$ , [ $\text{L}^2$ ] or [ $\text{L}^3$ ] <sup>†</sup>
$V_t^e$	volume of water in element $e$ at time $t$ , [ $\text{L}^2$ ] or [ $\text{L}^3$ ] <sup>†</sup>
$V_0$	volume of water in the flow domain at time zero, [ $\text{L}^2$ ] or [ $\text{L}^3$ ] <sup>†</sup>
$V_0^e$	volume of water in element $e$ at time zero, [ $\text{L}^2$ ] or [ $\text{L}^3$ ] <sup>†</sup>
$W$	total amount of energy in the flow region, [ $\text{MLT}^{-2}$ ] or [ $\text{ML}^2\text{T}^{-2}$ ] <sup>†</sup>
$x_i$	spatial coordinates ( $i=1,2$ ) [L]
$Z_0$	characteristic impedance of a transmission line analog to drain
$Z_0'$	characteristic impedance of free space ( $\approx 376.7$ ohms)
$\alpha$	coefficient in the soil water retention function [ $\text{L}^{-1}$ ]
$\alpha^w$	weighing factor [-]
$\alpha_h$	scaling factor for the pressure head [-]
$\alpha_h^*$	temperature scaling factor for the pressure head [-]

$\alpha_K$	scaling factor for the hydraulic conductivity [-]
$\alpha_K^*$	temperature scaling factor for the hydraulic conductivity [-]
$\alpha_\theta$	scaling factor for the water content [-]
$\beta$	empirical constant in adsorption isotherm [-]
$\gamma_s$	zero-order rate constant for solutes in the gas phase [ $ML^{-3}T^{-1}$ ]
$\gamma_i$	activity coefficient in soil solution [ $L^3M^{-1}$ ] ( $1\text{ mol}^{-1}$ )
$\gamma_s$	zero-order rate constant for solutes adsorbed onto the solid phase [ $T^{-1}$ ]
$\gamma_w$	zero-order rate constants for solutes in the liquid phase [ $ML^{-3}T^{-1}$ ]
$\Gamma_e$	boundary segments connected to node $n$
$\Gamma_D$	part of flow domain boundary where Dirichlet type conditions are specified
$\Gamma_G$	part of flow domain boundary where gradient type conditions are specified
$\Gamma_N$	part of flow domain boundary where Neumann type conditions are specified
$\Gamma_C$	part of flow domain boundary where Cauchy type conditions are specified
$\delta_{ij}$	Kronecker delta [-]
$\Delta t$	time increment [T]
$\Delta t_{max}$	maximum permitted time increment [T]
$\Delta t_{min}$	minimum permitted time increment [T]
$\epsilon$	temporal weighing factor [-]
$\epsilon_a^c$	absolute error in the solute mass balance, [ $ML^{-1}$ ] or [ $M$ ] <sup>†</sup>
$\epsilon_a^w$	absolute error in the water mass balance, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup>
$\epsilon_r^c$	relative error in the solute mass balance [%]
$\epsilon_r^w$	relative error in the water mass balance [%]
$\epsilon_0$	permittivity of free space (used in electric analog representation of drains)
$\eta$	empirical constant in adsorption isotherm [ $L^3M^{-3}$ ]
$\theta$	volumetric water content [ $L^3L^{-3}$ ]
$\theta^*$	scaled volumetric water content [ $L^3L^{-3}$ ]
$\theta_a$	parameter in the soil water retention function [ $L^3L^{-3}$ ]
$\theta_k$	volumetric water content corresponding to $K_k$ [ $L^3L^{-3}$ ]
$\theta_m$	parameter in the soil water retention function [ $L^3L^{-3}$ ]

$\theta_n$	volumetric solid phase fraction [ $L^3L^{-3}$ ]
$\theta_o$	volumetric organic matter fraction [ $L^3L^{-3}$ ]
$\theta_r$	residual soil water content [ $L^3L^{-3}$ ]
$\theta_s$	saturated soil water content [ $L^3L^{-3}$ ]
$\kappa$	parameter which depends on the type of flow being analyzed, [-] or [ $L$ ] <sup>†</sup>
$\lambda$	first-order rate constant [ $T^{-1}$ ]
$\lambda_i$	apparent thermal conductivity tensor of the soil [ $MLT^{-3}K^{-1}$ ] (e.g. $Wm^{-1}K^{-1}$ )
$\lambda_L$	longitudinal thermal dispersivity [L]
$\lambda_T$	transverse thermal dispersivity [L]
$\lambda_0$	thermal conductivity of porous medium in the absence of flow [ $MLT^{-3}K^{-1}$ ] (e.g. $Wm^{-1}K^{-1}$ )
$\mu_g$	first-order rate constant for solutes in the gas phase [ $T^{-1}$ ]
$\mu_{ref}$	dynamic viscosity at reference temperature $T_{ref}$ [ $MT^{-1}L^{-1}$ ]
$\mu_s$	first-order rate constant for solutes adsorbed onto the solid phase [ $T^{-1}$ ]
$\mu_T$	dynamic viscosity at temperature $T$ [ $MT^{-1}L^{-1}$ ]
$\mu_w$	first-order rate constant for solutes in the liquid phase [ $T^{-1}$ ]
$\mu'_g$	first-order rate constant for chain solutes in the gas phase [ $T^{-1}$ ]
$\mu'_s$	first-order rate constant for chain solutes adsorbed onto the solid phase [ $T^{-1}$ ]
$\mu'_w$	first-order rate constant for chain solutes in the liquid phase [ $T^{-1}$ ]
$\xi_i$	activity coefficient on the exchange surfaces [ $MM^{-1}$ ] ( $kg\ mol^{-1}$ )
$\rho$	bulk density of porous medium [ $ML^{-3}$ ]
$\rho_d$	dimensionless ratio between the side of the square in the finite element mesh surrounding the drain, $D$ , and the effective diameter of a drain, $d_e$ [-]
$\rho_{ref}$	density of soil water at reference temperature $T_{ref}$ [ $ML^{-3}$ ]
$\rho_T$	density of soil water at temperature $T$ [ $ML^{-3}$ ]
$\sigma_{ref}$	surface tension at reference temperature $T_{ref}$ [ $MT^{-2}$ ]
$\sigma_T$	surface tension at temperature $T$ [ $MT^{-2}$ ]
$\sigma_1$	prescribed flux boundary condition at boundary $\Gamma_N$ [ $LT^{-1}$ ]
$\sigma_2$	prescribed gradient boundary condition at boundary $\Gamma_G$ [-]
$\tau_a$	tortuosity factor in the gas phase [-]

$\tau_w$	tortuosity factor in the liquid phase [-]
$\phi_n$	linear basis functions [-]
$\phi_n''$	upstream weighted basis functions [-]
$\psi$	prescribed pressure head boundary condition at boundary $\Gamma_D$ [L]
$\omega$	first-order adsorption rate constant [ $T^{-1}$ ]
$\omega_a$	angle between principal direction of $K_1^A$ and the $x$ -axis of the global coordinate system [-]
$\omega_s$	performance index used as a criterion to minimize or eliminate numerical oscillations [-]
$\Omega$	flow region
$\Omega_e$	domain occupied by element $e$
$\Omega_R$	region occupied by the root zone

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<sup>†</sup> for plane and axisymmetric flow, respectively

## 1. INTRODUCTION

The importance of the unsaturated zone as an integral part of the hydrological cycle has long been recognized. The zone plays an inextricable role in many aspects of hydrology, including infiltration, soil moisture storage, evaporation, plant water uptake, groundwater recharge, runoff and erosion. Initial studies of the unsaturated (vadose) zone focused primarily on water supply studies, inspired in part by attempts to optimally manage the root zone of agricultural soils for maximum crop production. Interest in the unsaturated zone has dramatically increased in recent years because of growing concern that the quality of the subsurface environment is being adversely affected by agricultural, industrial and municipal activities. Federal, state and local action and planning agencies, as well as the public at large, are now scrutinizing the intentional or accidental release of surface-applied and soil-incorporated chemicals into the environment. Fertilizers and pesticides applied to agricultural lands inevitably move below the soil root zone and may contaminate underlying groundwater reservoirs. Chemicals migrating from municipal and industrial disposal sites also represent environmental hazards. The same is true for radionuclides emanating from energy waste disposal facilities.

The past several decades has seen considerable progress in the conceptual understanding and mathematical description of water flow and solute transport processes in the unsaturated zone. A variety of analytical and numerical models are now available to predict water and/or solute transfer processes between the soil surface and the groundwater table. The most popular models remain the Richards' equation for variably saturated flow, and the Fickian-based convection-dispersion equation for solute transport. Deterministic solutions of these classical equations have been used, and likely will continue to be used in the near future, for predicting water and solute movement in the vadose zone, and for analyzing specific laboratory or field experiments involving unsaturated water flow and/or solute transport. These models are also helpful tools for extrapolating information from a limited number of field experiments to different soil, crop and climatic conditions, as well as to different tillage and water management schemes.

Once released into the subsurface environment, industrial and agricultural chemicals

are generally subjected to a large number of simultaneous physical, chemical, and biological processes, including sorption-desorption, volatilization, photolysis, and biodegradation, as well as their kinetics. The extent of degradation, sorption and volatilization largely determines the persistence of a pollutant in the subsurface [Chiou, 1989]. For example, the fate of organic chemicals in soils is known to be strongly affected by the kinetics of biological degradation. *Alexander and Scow* [1989] gave a review of some of the equations used to represent the kinetics of biodegradation. These equations include zero-order, half-order, first-order, three-half-order, mixed-order, logistic, logarithmic, Michaelis-Menton, and Monod type (with or without growth) expressions. While most of these expressions have a theoretical bases, they are commonly used only in an empirical fashion by fitting the equations to observed data. Zero- and first-order kinetic equations remain the most popular for describing biodegradation of organic compounds, mostly because of their simplicity and the ease in which they can be incorporated in solute transport models. Conditions for the application of these two equations are described by *Alexander and Scow* [1989].

One special group of degradation reactions involves decay chains in which solutes are subject to sequential (or consecutive) decay reactions. Problems of solute transport involving sequential first-order decay reactions frequently occur in soil and groundwater systems. Examples are the migration of various radionuclides [*Lester et al.*, 1975; *Rogers*, 1978; *Gureghian*, 1981; *Gureghian and Jansen*, 1983], the simultaneous movement of interacting nitrogen species [*Cho*, 1971; *Misra et al.*, 1974; *Wagenet et al.*, 1976; *Tillotson et al.*, 1980], organic phosphate transport [*Castro and Rolston*, 1977], and the transport of certain pesticides and their metabolites [*Bromilow and Leistra*, 1980; *Wagenet and Hutson*, 1987].

While in the past most pesticides were regarded as involatile, volatilization is now increasingly recognized as an important process affecting the fate of pesticides in field soils [*Glotfelty and Schomburg*, 1989; *Spencer*, 1991]. Another process affecting pesticide fate and transport is the relative reactivity of solutes in the sorbed and solution phases. Several processes such as gaseous and liquid molecular diffusion, and convective-dispersive transport, act only on solutes that are not adsorbed. Degradation of organic compounds

likely occurs mainly, or even exclusively, in the liquid phase [Pignatello, 1989]. On the other side, radioactive decay takes place equally in the solution and adsorbed phases, while other reactions may occur only in the sorbed phase.

Several analytical solutions have been published for simplified transport systems involving consecutive decay reactions [Cho, 1971; Wagenet *et al.*, 1976; Harada *et al.*, 1980; Higashi and Pigford, 1980; van Genuchten, 1985]. Unfortunately, analytical solutions for more complex situations, such as for transient water flow or the nonequilibrium solute transport with nonlinear reactions, are not available and/or cannot be derived, in which case numerical models must be employed. To be useful, such numerical models must allow for different reaction rates to take place in the solid, liquid, and gaseous phases, as well as for a correct distribution of the solutes among the different phases.

The purpose of this report is to document the CHAIN\_2D computer program for simulating two-dimensional variably-saturated water flow, heat movement, and the transport of solutes involved in sequential first-order decay reactions. The program numerically solves the Richards' equation for saturated-unsaturated water flow and the convection-dispersion equation for heat and solute transport. The water flow equation incorporates a sink term to account for water uptake by plant roots. The heat transport equation considers movement by conduction as well as convection with flowing water. The governing convection-dispersion solute transport equations are written in a very general form by including provisions for nonlinear nonequilibrium reactions between the solid and liquid phases, and linear equilibrium reaction between liquid and gaseous phases. Hence, both adsorbed and volatile solutes such as pesticides can be considered. The solute transport equations also incorporate the effects of zero-order production, first-order degradation independent of other solutes, and first-order decay/production that provides the required coupling between the solutes involved in the sequential first-order reactions. The transport models also account for convection and dispersion in the liquid phase, as well as for diffusion in the gas phase, thus permitting one to simulate solute transport simultaneously in both the liquid and gaseous phases. CHAIN\_2D at present considers up to six solutes which can be either coupled in a unidirectional chain or may move independent of each other.

The CHAIN\_2D code may be used to analyze water and solute movement in unsaturated, partially saturated, or fully saturated porous media. The program can handle flow domains delineated by irregular boundaries. The flow region itself may be composed of nonuniform soils having an arbitrary degree of local anisotropy. Flow and transport can occur in the vertical plane, the horizontal plane, or in a three-dimensional region exhibiting radial symmetry about a vertical axis. The water flow part of the model considers prescribed head and flux boundaries, as well as boundaries controlled by atmospheric conditions or free drainage. A simplified representation of nodal drains using results of electric analog experiments is also included. The first and third-type boundary conditions can be prescribe in the solute and heat transport part of the model.

The governing flow and transport equations are solved numerically using Galerkin-type linear finite element schemes. Depending upon the size of the problem, the matrix equations resulting from discretization of the governing equations are solved using either Gaussian elimination for banded matrices or the conjugate gradient method for symmetric matrices, and the ORTHOMIN method for asymmetric matrices [Mendoza *et al.*, 1991]. The program is an extension of the SWMS\_2D code for simulating water flow and solute transport in two-dimensional variably saturated media [Šimunek *et al.*, 1992], which in turn was based in part on the variably saturated flow code SWMII of Vogel [1987] and early numerical work by Neuman and colleagues [Neuman, 1972, 1973, Neuman *et al.*, 1974; Neuman, 1975; Davis and Neuman, 1983]. Experience obtained with a model simulating two-dimensional heat transport and two-phase transport of carbon dioxide [Šimunek and Suarez, 1993b] was used in developing CHAIN\_2D. The code is written in ANSI standard FORTRAN 77, and hence can be compiled, linked and run on any standard micro-, mini-, or mainframe system, as well as on personal computers. The source code was developed and tested on a PC 486 using the Microsoft's Fortran PowerStation compiler.

This report serves as both a user manual and reference document. Detailed instructions are given for data input preparation. Example input and selected output files are also provided. Two 3½ or 5¼ inch floppy diskettes containing the input and output files of seven examples discussed in this report, as well as the source code, are available upon request from the authors.

## 2. VARIABLY SATURATED WATER FLOW

### 2.1. Governing Flow Equation

Consider two-dimensional isothermal Darcian flow of water in a variably saturated rigid porous medium and assume that the air phase plays an insignificant role in the liquid flow process. The governing flow equation for these conditions is given by the following modified form of the Richards' equation:

$$\frac{\partial \theta}{\partial t} = \frac{\partial}{\partial x_i} [K(K_{ij}^A \frac{\partial h}{\partial x_j} + K_{iz}^A)] - S \quad (2.1)$$

where  $\theta$  is the volumetric water content [ $L^3L^{-3}$ ],  $h$  is the pressure head [ $L$ ],  $S$  is a sink term [ $T^{-1}$ ],  $x_i$  ( $i=1,2$ ) are the spatial coordinates [ $L$ ],  $t$  is time [ $T$ ],  $K_{ij}^A$  are components of a dimensionless anisotropy tensor  $K^A$ , and  $K$  is the unsaturated hydraulic conductivity function [ $LT^{-1}$ ] given by

$$K(h,x,z) = K_s(x,z) K_r(h,x,z) \quad (2.2)$$

where  $K_r$  is the relative hydraulic conductivity and  $K_s$  the saturated hydraulic conductivity [ $LT^{-1}$ ]. The anisotropy tensor  $K_{ij}^A$  in (2.1) is used to account for an anisotropic medium. The diagonal entries of  $K_{ij}^A$  equal one and the off-diagonal entries zero for an isotropic medium. If (2.1) is applied to planar flow in a vertical cross-section,  $x_1=x$  is the horizontal coordinate and  $x_2=z$  is the vertical coordinate, the latter taken to be positive upward. Einstein's summation convention is used in (2.1) and throughout this report. Hence, when an index appears twice in an algebraic term, this particular term must be summed over all possible values of the index.

### 2.2. Root Water Uptake

The sink term,  $S$ , in (2.1) represents the volume of water removed per unit time from a unit volume of soil due to plant water uptake. *Feddes et al. [1978]* defined  $S$  as

$$S(h) = a_r(h) S_p \quad (2.3)$$

where the water stress response function  $a_r(h)$  is a prescribed dimensionless function (Fig. 2.1) of the soil water pressure head ( $0 \leq a_r \leq 1$ ), and  $S_p$  is the potential water uptake rate [ $T^{-1}$ ]. Figure 2.1. gives a schematic plot of the stress response function as used by *Feddes et al.* [1978]. Notice that water uptake is assumed to be zero close to saturation (i.e., wetter than some arbitrary "anaerobiosis point",  $h_1$ ). For  $h < h_4$  (the wilting point pressure head), water uptake is also assumed to be zero. Water uptake is considered optimal between pressure heads  $h_2$  and  $h_3$ , whereas for pressure head between  $h_3$  and  $h_4$  (or  $h_1$  and  $h_2$ ), water uptake decreases (or increases) linearly with  $h$ .  $S_p$  is equal to the water uptake rate during periods of no water stress when  $a_r(h)=1$ .

When the potential water uptake rate is equally distributed over a two-dimensional rectangular root domain,  $S_p$  becomes

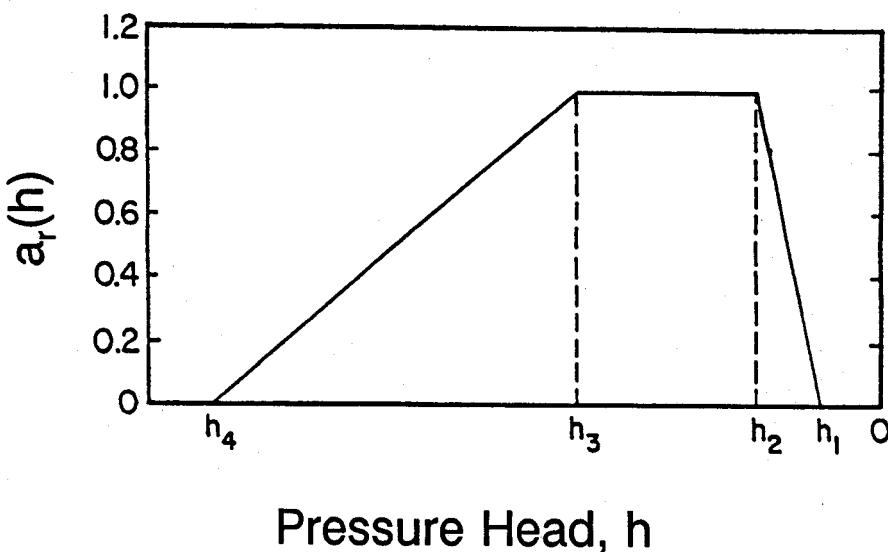


Fig. 2.1. Schematic of the plant water stress response function,  $a_r(h)$ , as used by *Feddes et al.* [1978].

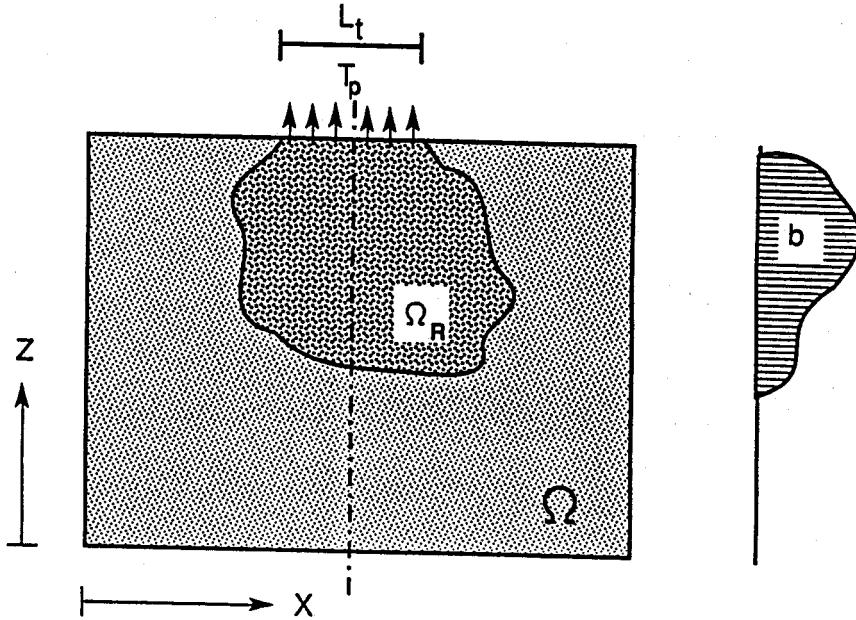


Fig. 2.2. Schematic of the potential water uptake distribution function,  $b(x,z)$ , in the soil root zone.

$$S_p = \frac{1}{L_x L_z} L_t T_p \quad (2.4)$$

where  $T_p$  is the potential transpiration rate [ $LT^{-1}$ ],  $L_z$  is the depth [L] of the root zone,  $L_x$  is the width [L] of the root zone, and  $L_t$  is the width [L] of the soil surface associated with the transpiration process. Notice that  $S_p$  reduces to  $T_p/L_z$  when  $L_t=L_x$ .

Equation (2.4) may be generalized by introducing a non-uniform distribution of the potential water uptake rate over a root zone of arbitrary shape [Vogel, 1987]:

$$S_p = b(x,z) L_t T_p \quad (2.5)$$

where  $b(x,z)$  is the normalized water uptake distribution [ $L^{-2}$ ]. This function describes the spatial variation of the potential extraction term,  $S_p$ , over the root zone (Fig. 2.2), and is obtained from  $b'(x,z)$  as follows

$$b(x,z) = \frac{b'(x,z)}{\int_{\Omega_R} b'(x,z) d\Omega} \quad (2.6)$$

where  $\Omega_R$  is the region occupied by the root zone, and  $b'(x,z)$  is an arbitrarily prescribed distribution function. Normalizing the uptake distribution ensures that  $b(x,z)$  integrates to unity over the flow domain, i.e.,

$$\int_{\Omega_R} b(x,z) d\Omega = 1 \quad (2.7)$$

From (2.5) and (2.7) it follows that  $S_p$  is related to  $T_p$  by the expression

$$\frac{1}{L_t} \int_{\Omega_R} S_p d\Omega = T_p \quad (2.8)$$

The actual water uptake distribution is obtained by substituting (2.5) into (2.3):

$$S(h,x,z) = a_r(h,x,z) b(x,z) L_t T_p \quad (2.9)$$

whereas the actual transpiration rate,  $T_a$ , is obtained by integrating (2.9) as follows

$$T_a = \frac{1}{L_t} \int_{\Omega_R} S d\Omega = T_p \int_{\Omega_R} a_r(h,x,z) b(x,z) d\Omega \quad (2.10)$$

### 2.3. The Unsaturated Soil Hydraulic Properties

The unsaturated soil hydraulic properties in the CHAIN\_2D code are described by a set of closed-form equations resembling those of *van Genuchten* [1980] who used the statistical pore-size distribution model of *Mualem* [1976] to obtain a predictive equation for the unsaturated hydraulic conductivity function. The original van Genuchten equations were modified to add extra flexibility in the description of the hydraulic properties near saturation [*Šír et al.*, 1985; *Vogel and Císlarová*, 1988]. The soil water retention,  $\theta(h)$ , and hydraulic conductivity,  $K(h)$ , functions in CHAIN\_2D are given by

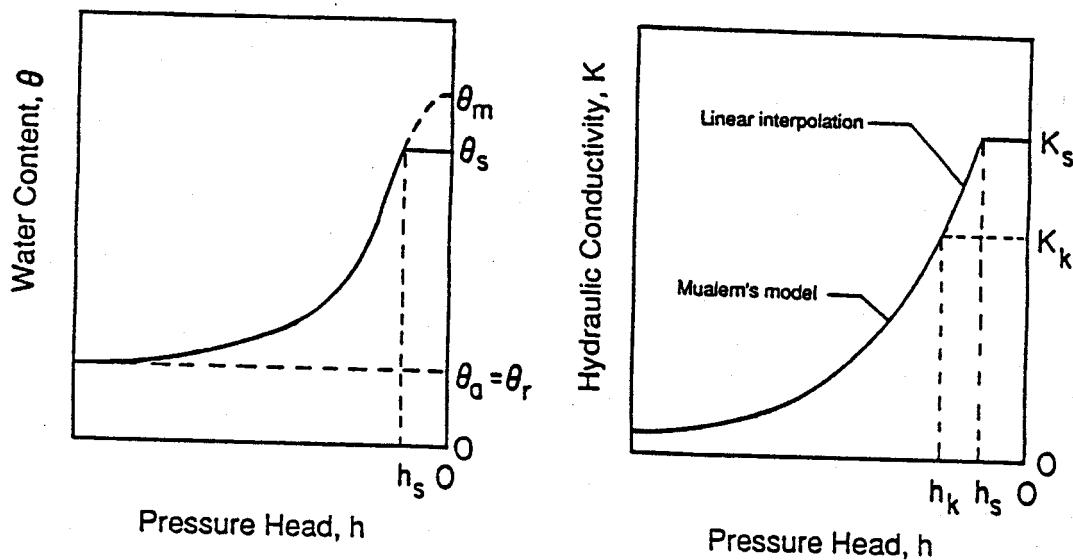


Fig. 2.3. Schematics of the soil water retention (a) and hydraulic conductivity (b) functions as given by equations (2.11) and (2.12), respectively.

function were replaced by the fictitious (extrapolated) parameters  $\theta_a \leq \theta_r$  and  $\theta_m \geq \theta_s$ , as shown in Fig. 2.3. The approach maintains the physical meaning of  $\theta_r$  and  $\theta_s$  as measurable quantities. Equation (2.13) assumes that the predicted hydraulic conductivity function is matched to a measured value of the hydraulic conductivity,  $K_k = K(\theta_k)$ , at some water content,  $\theta_k$ , less than or equal to the saturated water content, i.e.,  $\theta_k \leq \theta_s$ , and  $K_k \leq K_s$ , [Vogel and Císlarová, 1988; Luckner et al., 1989].

Inspection of (2.11) through (2.17) shows that the hydraulic characteristics contain 9 unknown parameters:  $\theta_r$ ,  $\theta_s$ ,  $\theta_a$ ,  $\theta_m$ ,  $\alpha$ ,  $n$ ,  $K_s$ ,  $K_k$ , and  $\theta_k$ . When  $\theta_a = \theta_r$ ,  $\theta_m = \theta_k = \theta_s$ , and  $K_k = K_s$ , the soil hydraulic functions reduce to the original expressions of van Genuchten [1980]:

$$\theta(h) = \begin{cases} \theta_r + \frac{\theta_s - \theta_r}{[1 + |\alpha h|^n]^m} & h < 0 \\ \theta_s & h \geq 0 \end{cases} \quad (2.18)$$

$$\theta(h) = \begin{cases} \theta_a + \frac{\theta_m - \theta_a}{(1 + |\alpha h|^n)^m} & h < h_s \\ \theta_s & h \geq h_s \end{cases} \quad (2.11)$$

and

$$K(h) = \begin{cases} K_s K_r(h) & h \leq h_k \\ K_k + \frac{(h - h_k)(K_s - K_k)}{h_s - h_k} & h_k < h < h_s \\ K_s & h \geq h_s \end{cases} \quad (2.12)$$

respectively, where

$$K_r = \frac{K_k}{K_s} \left[ \frac{S_e}{S_{ek}} \right]^{1/2} \left[ \frac{F(\theta_r) - F(\theta)}{F(\theta_r) - F(\theta_k)} \right]^2 \quad (2.13)$$

$$F(\theta) = \left[ 1 - \left( \frac{\theta - \theta_a}{\theta_m - \theta_a} \right)^{1/m} \right]^m \quad (2.14)$$

$$m = 1 - 1/n, \quad n > 1 \quad (2.15)$$

$$S_e = \frac{\theta - \theta_r}{\theta_s - \theta_r} \quad (2.16)$$

$$S_{ek} = \frac{\theta_k - \theta_r}{\theta_s - \theta_r} \quad (2.17)$$

in which  $\theta_r$  and  $\theta_s$  denote the residual and saturated water contents, respectively, and  $K_s$  is the saturated hydraulic conductivity. To increase the flexibility of the analytical expressions, and to allow for a non-zero air-entry value,  $h_s$ , the parameters  $\theta_r$  and  $\theta_s$  in the retention

$$K(h) = \begin{cases} K_s K_r(h) & h < 0 \\ K_s & h \geq 0 \end{cases} \quad (2.19)$$

where

$$K_r = S_e^{1/2} [1 - (1 - S_e^{1/m})^m]^2 \quad (2.20)$$

## 2.4. Scaling of the Soil Hydraulic Functions

### 2.4.1. Spatial variability of the Soil Hydraulic Functions

**CHAIN\_2D** implements a scaling procedure designed to simplify the description of the spatial variability of the unsaturated soil hydraulic properties in the flow domain. The code assumes that the hydraulic variability in a given area can be approximated by means of a set of linear scaling transformations which relate the individual soil hydraulic characteristics  $\theta(h)$  and  $K(h)$  to reference characteristics  $\theta^*(h^*)$  and  $K^*(h^*)$ . The technique is based on the similar media concept introduced by *Miller and Miller* [1956] for porous media which differ only in the scale of their internal geometry. The concept was extended by *Simmons et al.* [1979] to materials which differ in morphological properties, but which exhibit 'scale-similar' soil hydraulic functions. Three independent scaling factors are embodied in **CHAIN\_2D**. These three scaling parameters may be used to define a linear model of the actual spatial variability in the soil hydraulic properties as follows [*Vogel et al.*, 1991]:

$$\begin{aligned} K(h) &= \alpha_K K^*(h^*) \\ \theta(h) &= \theta_r + \alpha_\theta [\theta^*(h^*) - \theta_r^*] \\ h &= \alpha_h h^* \end{aligned} \quad (2.21)$$

in which, for the most general case,  $\alpha_\theta$ ,  $\alpha_h$  and  $\alpha_K$  are mutually independent scaling factors

for the water content, the pressure head and the hydraulic conductivity, respectively. Less general scaling methods arise by invoking certain relationships between  $\alpha_\theta$ ,  $\alpha_h$  and/or  $\alpha_K$ . For example, the original Miller-Miller scaling procedure is obtained by assuming  $\alpha_\theta=1$  (with  $\theta_r^* = \theta_r$ ), and  $\alpha_K=\alpha_h^{-2}$ . A detailed discussion of the scaling relationships given by (2.21), and their application to the hydraulic description of heterogeneous soil profiles, is given by *Vogel et al.* [1991].

#### 2.4.2. Temperature Dependence of the Soil Hydraulic Functions

A similar scaling technique as described above is used to express the temperature dependence of the soil hydraulic functions. Based on capillary theory that assumes that the influence of temperature on the soil water pressure head can be quantitatively predicted from the influence of temperature on surface tension, *Philip and de Vries* [1957] derived following equation

$$\frac{dh}{dT} = \frac{h}{\sigma} \frac{d\sigma}{dT} \quad (2.22)$$

where  $T$  is temperature [K] and  $\sigma$  is the surface tension at the air-water interface [ $MT^{-2}$ ]. From (2.22) it follows that

$$h_T = \frac{\sigma_T}{\sigma_{ref}} h_{ref} = \alpha_h^* h_{ref} \quad (2.23)$$

where  $h_T$  and  $h_{ref}$  ( $\sigma_T$  and  $\sigma_{ref}$ ) are pressure heads (surface tensions) at temperature  $T$  and reference temperature  $T_{ref}$ , respectively; and  $\alpha_h^*$  is the temperature scaling factor for the pressure head.

Following *Constantz* [1982], the temperature dependence of the hydraulic conductivity can be expressed as

$$K_T(\theta) = \frac{\mu_{ref}}{\mu_T} \frac{\rho_T}{\rho_{ref}} K_{ref}(\theta) = \alpha_K^* K_{ref}(\theta) \quad (2.24)$$

where  $K_{ref}$  and  $K_T$  denote hydraulic conductivities at the reference temperature  $T_{ref}$  and soil temperature  $T$ , respectively;  $\mu_{ref}$  and  $\mu_T$  ( $\rho_{ref}$  and  $\rho_T$ ) similarly represent the dynamic viscosity [ $ML^{-1}T^{-1}$ ] (density of soil water [ $ML^{-3}$ ]) at temperatures  $T_{ref}$  and  $T$ , respectively; and  $\alpha_K^*$  is the temperature scaling factor for the hydraulic conductivity.

## 2.5. Initial and Boundary Conditions

The solution of Eq. (2.1) requires knowledge of the initial distribution of the pressure head within the flow domain,  $\Omega$ :

$$h(x,z,t) = h_0(x,z) \quad \text{for } t = 0 \quad (2.25)$$

where  $h_0$  is a prescribed function of  $x$  and  $z$ .

CHAIN\_2D implements three types of conditions to describe system-independent interactions along the boundaries of the flow region. These conditions are specified pressure head (Dirichlet type) boundary conditions of the form

$$h(x,z,t) = \psi(x,z,t) \quad \text{for } (x,z) \in \Gamma_D \quad (2.26)$$

specified flux (Neumann type) boundary conditions given by

$$-[K(K_{ij}^A \frac{\partial h}{\partial x_j} + K_{iz}^A)]n_i = \sigma_1(x,z,t) \quad \text{for } (x,z) \in \Gamma_N \quad (2.27)$$

and specified gradient boundary conditions

$$(K_{ij}^A \frac{\partial h}{\partial x_j} + K_{iz}^A)n_i = \sigma_2(x,z,t) \quad \text{for } (x,z) \in \Gamma_G \quad (2.28)$$

where  $\Gamma_D$ ,  $\Gamma_N$ , and  $\Gamma_G$  indicate Dirichlet, Neumann, and gradient type boundary segments, respectively;  $\psi$  [L],  $\sigma_1$  [ $LT^{-1}$ ], and  $\sigma_2$  [-] are prescribed functions of  $x$ ,  $z$  and  $t$ ; and  $n_i$  are the

components of the outward unit vector normal to boundary  $\Gamma_N$  or  $\Gamma_G$ . As pointed out by *McCord* [1991], the use of the term "Neumann type boundary condition" for the flux boundary is not very appropriate since this term should hold for a gradient type condition (see also Section 3.2 for solute transport). However, since the use of the Neumann condition is standard in the hydrologic literature [*Neuman*, 1972; *Neuman et al.*, 1974], we shall also use this term to indicate flux boundaries throughout this report. CHAIN\_2D implements the gradient boundary condition only in terms of a unit vertical hydraulic gradient simulating free drainage from a relatively deep soil profile. This situation is often observed in field studies of water flow and drainage in the vadose zone [*Sisson*, 1987; *McCord*, 1991]. *McCord* [1991] states that the most pertinent application of (2.28) is its use as a bottom outflow boundary condition for situations where the water table lies far below the domain of interest.

In addition to the system-independent boundary conditions given by (2.26), (2.27) and (2.28), CHAIN\_2D considers two different types of system-dependent boundary conditions which cannot be defined a priori. One of these involves soil-air interfaces which are exposed to atmospheric conditions. The potential fluid flux across these interfaces is controlled exclusively by external conditions. However, the actual flux depends also on the prevailing (transient) soil moisture conditions. Soil surface boundary conditions may change from prescribed flux to prescribed head type conditions (and vice-versa). In the absence of surface ponding, the numerical solution of (2.1) is obtained by limiting the absolute value of the flux by the following two conditions [*Neuman et al.*, 1974]:

$$|K(K_{ij}^A \frac{\partial h}{\partial x_j} + K_{iz}^A)n_i| \leq E \quad (2.29)$$

and

$$h_A \leq h \leq h_s \quad (2.30)$$

where  $E$  is the maximum potential rate of infiltration or evaporation under the current atmospheric conditions,  $h$  is the pressure head at the soil surface, and  $h_A$  and  $h_s$  are, respectively, minimum and maximum pressure heads allowed under the prevailing soil

conditions. The value for  $h_A$  is determined from the equilibrium conditions between soil water and atmospheric water vapor, whereas  $h_S$  is usually set equal to zero. CHAIN\_2D assumes that any excess water on the soil surface is immediately removed. When one of the end points of (2.30) is reached, a prescribed head boundary condition will be used to calculate the actual surface flux. Methods of calculating  $E$  and  $h_A$  on the basis of atmospheric data have been discussed by *Feddes et al.* [1974].

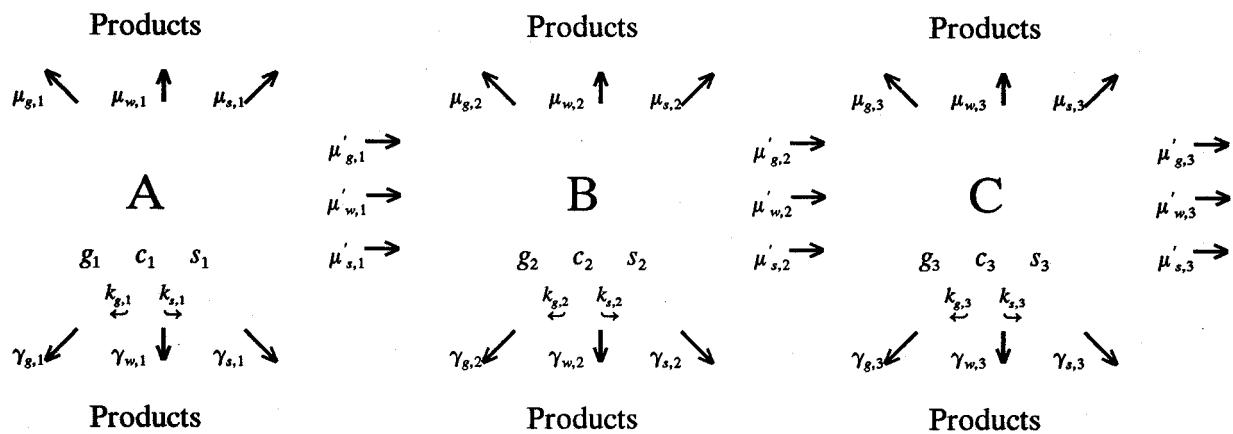
A second type of system-dependent boundary conditions considered in CHAIN\_2D is a seepage face through which water leaves the saturated part of the flow domain. In this case, the length of the seepage face is not known a priori. CHAIN\_2D assumes that the pressure head is always uniformly equal to zero along a seepage face. Additionally, the code assumes that water leaving the saturated zone across a seepage face is immediately removed by overland flow or some other removal process.



### 3. NONEQUILIBRIUM TRANSPORT OF SOLUTES INVOLVED IN SEQUENTIAL FIRST-ORDER DECAY REACTIONS

#### 3.1. Governing Solute Transport Equations

We assume that solutes can exist in all three phases (liquid, solid, and gaseous) and that the decay and production processes can be different in each phase. Interactions between the solid and liquid phases may be described by nonlinear nonequilibrium equations, while interactions between the liquid and gaseous phases are assumed to be linear and instantaneous. We further assume that the solutes are transported by convection and dispersion in the liquid phase, as well as by diffusion in the gas phase. A general structure of the system of first-order decay reactions for three solutes (A, B and C) is as follows:



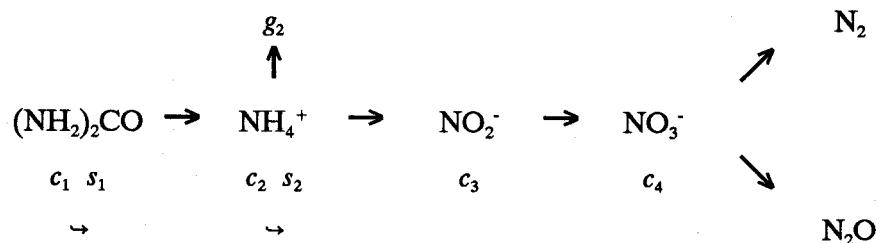
where  $c$ ,  $s$ , and  $g$  represent concentrations in the liquid, solid, and gaseous phases, respectively; the subscripts  $s$ ,  $w$ , and  $g$  refer to solid, liquid and gaseous phases, respectively; straight arrows represent the different zero-order ( $\gamma$ ) and first-order ( $\mu$ ,  $\mu'$ ) rate reactions, and circular arrows ( $k_g$ ,  $k_s$ ) indicate equilibrium distribution coefficients between phases.

Typical examples of sequential first-order decay chains are:

1. Radionuclides [*van Genuchten*, 1985]

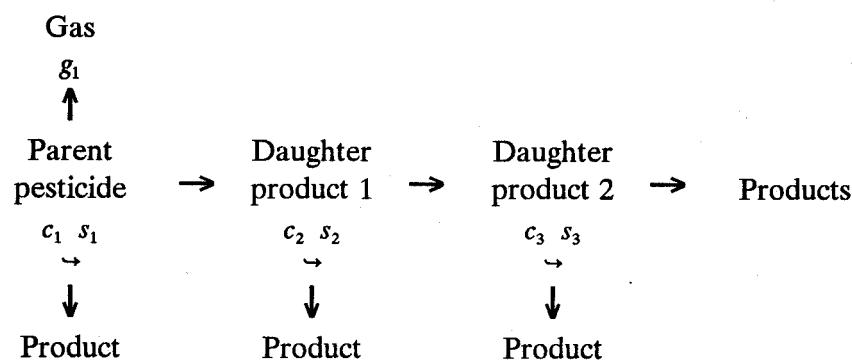


2. Nitrogen [*Tillotson et al.*, 1980]

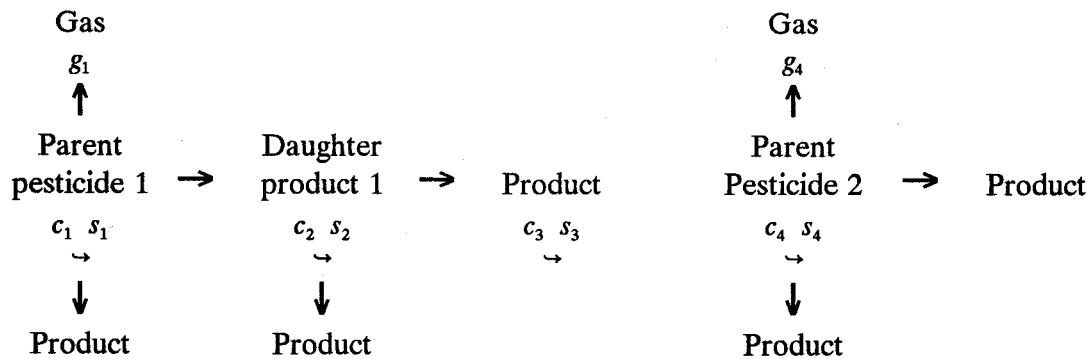


3. Pesticides [*Wagenet and Hutson*, 1987]:

a) Uninterrupted chain - one reaction path:



b) Interrupted chain - two independent reaction paths:



CHAIN\_2D at present considers up to six solutes which can be either coupled in a unidirectional chain or may move independently of each other.

The partial differential equations governing two-dimensional nonequilibrium chemical transport of solutes involved in a sequential first-order decay chain during transient water flow in a variably saturated rigid porous medium are taken as

$$\begin{aligned} \frac{\partial \theta c_1}{\partial t} + \frac{\partial \rho s_1}{\partial t} + \frac{\partial a_v g_1}{\partial t} = & \frac{\partial}{\partial x_i} (\theta D_{ij,1}^w \frac{\partial c_1}{\partial x_j}) + \frac{\partial}{\partial x_i} (a_v D_{ij,1}^g \frac{\partial g_1}{\partial x_j}) - \frac{\partial q_i c_1}{\partial x_i} - S c_{r,1} - \\ & - (\mu_{w,1} + \mu'_{w,1}) \theta c_1 - (\mu_{s,1} + \mu'_{s,1}) \rho s_1 - (\mu_{g,1} + \mu'_{g,1}) a_v g_1 + \gamma_{w,1} \theta + \gamma_{s,1} \rho + \gamma_{g,1} a_v \end{aligned} \quad (3.1)$$

$$\begin{aligned} \frac{\partial \theta c_k}{\partial t} + \frac{\partial \rho s_k}{\partial t} + \frac{\partial a_v g_k}{\partial t} = & \frac{\partial}{\partial x_i} (\theta D_{ij,k}^w \frac{\partial c_k}{\partial x_j}) + \frac{\partial}{\partial x_i} (a_v D_{ij,k}^g \frac{\partial g_k}{\partial x_j}) - \frac{\partial q_i c_k}{\partial x_i} - \\ & - (\mu_{w,k} + \mu'_{w,k}) \theta c_k - (\mu_{s,k} + \mu'_{s,k}) \rho s_k - (\mu_{g,k} + \mu'_{g,k}) a_v g_k + \mu'_{w,k-1} \theta c_{k-1} - \\ & + \mu'_{s,k-1} \rho s_{k-1} + \mu'_{g,k-1} a_v g_{k-1} + \gamma_{w,k} \theta + \gamma_{s,k} \rho + \gamma_{g,k} a_v - S c_{r,k} \quad k \in (2, n_s) \end{aligned} \quad (3.2)$$

where  $c$ ,  $s$ , and  $g$  are solute concentrations in the liquid [ $\text{ML}^{-3}$ ], solid [ $\text{MM}^{-1}$ ], and gaseous [ $\text{ML}^{-3}$ ], phases, respectively;  $q_i$  is the  $i$ -th component of the volumetric flux density [ $\text{LT}^{-1}$ ],  $\mu_w$ ,  $\mu_s$ , and  $\mu_g$  are first-order rate constants for solutes in the liquid, solid, and gas phases [ $\text{T}^{-1}$ ], respectively;  $\mu'_w$ ,  $\mu'_s$ , and  $\mu'_g$  are similar first-order rate constants providing connections between individual chain species,  $\gamma_w$ ,  $\gamma_s$ , and  $\gamma_g$  are zero-order rate constants for the liquid [ $\text{ML}^{-3}\text{T}^{-1}$ ], solid [ $\text{T}^{-1}$ ], and gas [ $\text{ML}^{-3}\text{T}^{-1}$ ] phases, respectively;  $\rho$  is the soil bulk density [ $\text{M L}^{-3}$ ],  $a_v$  is the air content [ $\text{L}^3\text{L}^{-3}$ ],  $S$  is the sink term in the water flow equation (2.1),  $c_r$  is the concentration of the sink term [ $\text{ML}^{-3}$ ],  $D_{ij}^w$  is the dispersion coefficient tensor [ $\text{L}^2\text{T}^{-1}$ ] for the liquid phase, and  $D_{ij}^g$  is the diffusion coefficient tensor [ $\text{L}^2\text{T}^{-1}$ ] for the gas phase. As before, the subscripts  $w$ ,  $s$ , and  $g$  correspond with the liquid, solid and gas phases, respectively; while the subscript  $k$  represents the  $k$ th chain number, and  $n_s$  is the number of solutes involved in the chain reaction. The indicial notation used in this report assumes summations over indices  $i$  and  $j$  ( $i,j = 1,2$ ), but not over index  $k$ . The nine zero- and first-order rate constants in (3.1) and (3.2) may be used to represent a variety of reactions or transformations

including biodegradation, volatilization, and precipitation. Note the differences in the signs of the first-order rate constants in comparison with the SWMS\_2D code.

**CHAIN\_2D** assumes nonequilibrium interaction between the solution ( $c$ ) and adsorbed ( $s$ ) concentrations, and equilibrium interaction between the solution ( $c$ ) and gas ( $g$ ) concentrations of the solute in the soil system. The adsorption isotherm relating  $s_k$  and  $c_k$  is described by a generalized nonlinear equation of the form

$$s_k = \frac{k_{s,k} c_k^{\beta_k}}{1 + \eta_k c_k^{\beta_k}} \quad k \in (1, n_s) \quad (3.3)$$

$$\frac{\partial s_k}{\partial t} = \frac{k_{s,k} \beta_k c_k^{\beta_k - 1}}{(1 + \eta_k c_k^{\beta_k})^2} \frac{\partial c_k}{\partial t} + \frac{c_k^{\beta_k}}{1 + \eta_k c_k^{\beta_k}} \frac{\partial k_{s,k}}{\partial t} - \frac{k_{s,k} c_k^{2\beta_k}}{(1 + \eta_k c_k^{\beta_k})^2} \frac{\partial \eta_k}{\partial t} + \frac{k_{s,k} c_k^{\beta_k} \ln c_k}{(1 + \eta_k c_k^{\beta_k})^2} \frac{\partial \beta_k}{\partial t}$$

where  $k_{s,k}$  [ $\text{L}^3\text{M}^{-1}$ ],  $\beta_k$  [-] and  $\eta_k$  [ $\text{L}^3\text{M}^{-1}$ ] are empirical coefficients. The Freundlich, Langmuir, and linear adsorption equations are special cases of equation (3.3). When  $\beta_k=1$ , equation (3.3) becomes the Langmuir equation, when  $\eta_k=0$ , equation (3.3) becomes the Freundlich equation, and when both  $\beta_k=1$  and  $\eta_k=0$ , equation (3.3) leads to a linear adsorption isotherm. Solute transport without adsorption is described with  $k_{s,k}=0$ . While the coefficients  $k_{s,k}$ ,  $\beta_k$ , and  $\eta_k$  in equation (3.3) are assumed to be independent of concentration, they are permitted to change as a function of time through their dependency on temperature. This feature will be discussed later.

The concept of two-site sorption [Selim *et al.*, 1977; van Genuchten and Wagener, 1989] is implemented in **CHAIN\_2D** to permit consideration of nonequilibrium adsorption-desorption reactions. The two-site sorption concept assumes that the sorption sites can be divided into two fractions:

$$s_k = s_k^e + s_k^k \quad k \in (1, n_s) \quad (3.4)$$

Sorption,  $s_k^e$  [ $\text{MM}^{-1}$ ], on one fraction of the sites (the type-1 sites) is assumed to be instantaneous, while sorption,  $s_k^k$  [ $\text{MM}^{-1}$ ], on the remaining (type-2) sites is considered to be time-dependent. At equilibrium we have for the type-1 (equilibrium) and type-2 (kinetic)

sites, respectively

$$s_k^e = fs_k \quad k \in (1, n_s) \quad (3.5)$$

$$s_k^k = (1 - f)s_k \quad k \in (1, n_s) \quad (3.6)$$

where  $f$  is the fraction of exchange sites assumed to be in equilibrium with the solution phase [-]. Because type-1 sorption sites are always at equilibrium, differentiation of (3.5) gives immediately the sorption rate for the type-1 equilibrium sites:

$$\frac{\partial s_k^e}{\partial t} = f \frac{\partial s_k}{\partial t} \quad k \in (1, n_s) \quad (3.7)$$

Sorption on the type-2 nonequilibrium sites is assumed to be a first-order kinetic rate process. Following *Toride et al.* [1993], the mass balance equation for the type-2 sites in the presence of production and degradation is given by

$$\frac{\partial s_k^k}{\partial t} = \omega_k \left[ (1 - f) \frac{k_{s,k} c_k^{\beta_k}}{1 + \eta_k c_k^{\beta_k}} - s_k^k \right] - (\mu_{s,k} + \mu'_{s,k}) s_k^k + (1 - f) \gamma_{s,k} \quad k \in (1, n_s) \quad (3.8)$$

where  $\omega_k$  is the first-order rate constant for the  $k$ th solute [ $T^{-1}$ ].

The concentrations  $g_k$  and  $c_k$  are related by a linear expression of the form

$$g_k = k_{g,k} c_k \quad k \in (1, n_s) \quad (3.9)$$

where  $k_{g,k}$  is an empirical constant [-] equal to  $(K_H R_u T^4)^{-1}$  [Stumm and Morgan, 1981] in which  $K_H$  is Henry's Law constant [ $MT^2M^{-1}L^{-2}$ ],  $R_u$  is the universal gas constant [ $ML^2T^{-2}K^{-1}M^{-1}$ ] and  $T^4$  is absolute temperature [K].

Substituting (3.3) through (3.9) into (3.1) and (3.2), and using the continuity equation describing the isothermal Darcian flow of water in a variably saturated porous medium, i.e.,

$$\frac{\partial \theta}{\partial t} = -\frac{\partial q_i}{\partial x_i} - S \quad (3.10)$$

leads to the following equation

$$-\theta R_k \frac{\partial c_k}{\partial t} - q_i \frac{\partial c_k}{\partial x_i} + \frac{\partial}{\partial x_i} (\theta D_{ij,k} \frac{\partial c_k}{\partial x_j}) + F_k c_k + G_k = 0 \quad k \in (1, n_s) \quad (3.11)$$

in which  $D_{ij}$  [L<sup>2</sup>T<sup>-1</sup>] is an effective dispersion coefficient tensor given by

$$\theta D_{ij,k} = \theta D_{ij,k}^w + a_v D_{ij,k}^g k_{g,k} \quad k \in (1, n_s) \quad (3.12)$$

The coefficients  $F_k$  and  $G_k$  in (3.11) are defined as

$$\begin{aligned} F_k(c_k) = & -(\mu_{w,k} + \mu'_{w,k})\theta - (\mu_{s,k} + \mu'_{s,k})\rho f \frac{k_{s,k} c_k^{\beta_k-1}}{1 + \eta_k c_k^{\beta_k}} - (\mu_{g,k} + \mu'_{g,k})a_v k_{g,k} + \\ & + S + k_{g,k} \frac{\partial \theta}{\partial t} - a_v \frac{\partial k_{g,k}}{\partial t} - \omega_k \rho \frac{(1-f)k_{s,k} c_k^{\beta_k-1}}{1 + \eta_k c_k^{\beta_k}} - g_k(c_k) \quad k \in (1, n_s) \end{aligned} \quad (3.13)$$

$$\begin{aligned} G_1(c_1) = & \gamma_{w,1}\theta + \gamma_{s,1}f\rho + \gamma_{g,1}a_v - Sc_{r,1} + \omega_1\rho s_1^k \\ G_k(c_k) = & (\mu'_{w,k-1}\theta + \mu'_{s,k-1}f\rho \frac{k_{s,k-1} c_{k-1}^{\beta_{k-1}-1}}{1 + \eta_{k-1} c_{k-1}^{\beta_{k-1}}} + \mu_{g,k-1}a_v k_{g,k-1})c_{k-1} + \\ & + \mu'_{s,k-1}\rho s_{k-1}^k + \gamma_{w,k}\theta + \gamma_{s,k}f\rho + \gamma_{g,k}a_v - Sc_{r,k} + \omega_k\rho s_k^k \quad k \in (2, n_s) \end{aligned} \quad (3.14)$$

where the variable  $g_k$  accounts for possible changes in the adsorption parameters caused by temperature changes in the system as follows (see also section 3.4):

$$g_k(c_k) = \rho f \left[ \frac{c_k^{\beta_k-1}}{1 + \eta_k c_k^{\beta_k}} \frac{\partial k_{s,k}}{\partial t} - \frac{k_{s,k} c_k^{2\beta_k-1}}{(1 + \eta_k c_k^{\beta_k})^2} \frac{\partial \eta_k}{\partial t} + \frac{k_{s,k} \ln c_k c_k^{\beta_k-1}}{(1 + \eta_k c_k^{\beta_k})^2} \frac{\partial \beta_k}{\partial t} \right] \quad k \in (1, n_s) \quad (3.15)$$

The retardation factor  $R_k$  [-] in (3.11) is given by

$$R_k(c_k) = 1 + \frac{\rho}{\theta} \frac{fk_{s,k}\beta_k c_k^{\beta_k-1}}{(1 + \eta_k c_k^{\beta_k})^2} + \frac{a_v k_{g,k}}{\theta} \quad k \in (1, n_s) \quad (3.16)$$

Equation (3.11) has the same form as equation (3.4) of Šimunek *et al.* [1992], except that the coefficients  $R_k$ ,  $F_k$ , and  $G_k$  are now nonlinear in  $c_k$  because of the consideration of the nonlinear sorption. Because of this similarity, the development of the numerical solution of (3.11) is largely the same as that followed by Šimunek *et al.* [1992] for the SWMS\_2D code. In order to solve equation (3.11), it is necessary to know the water content  $\theta$  and the volumetric flux  $q_i$ . Both variables are obtained from solutions of the Richards' equation. Assuming that  $\mu_w'$ ,  $\mu_s'$ ,  $\mu_g'$ ,  $\eta_k$ , and  $k_g$  are zero, and  $f$  and  $\beta_k$  are equal to one, the entire system of equations (3.1) through (3.16) simplifies into a system of mutually independent solutes as described in SWMS\_2D [Šimunek *et al.*, 1992].

### 3.2. Initial and Boundary Conditions

The solution of (3.11) requires knowledge of the initial concentration within the flow region,  $\Omega$ , i.e.,

$$\begin{aligned} c(x, z, 0) &= c_i(x, z) \\ s^k(x, z, 0) &= s_i^k(x, z) \end{aligned} \quad (3.17)$$

where  $c_i$  [ $\text{ML}^{-3}$ ] and  $s_i^k$  [-] are prescribed functions of  $x$  and  $z$ . The initial condition for  $s^k$  must be specified only when nonequilibrium adsorption is considered. The subscript  $k$  is dropped in (3.17) and throughout the remainder of this report, thus assuming that the transport-related equations in the theoretical development and numerical solution apply to each of the solutes in the decay chain.

Two types of boundary conditions (Dirichlet and Cauchy type conditions) can be specified along the boundary of  $\Omega$ . First-type (or Dirichlet type) boundary conditions prescribe the concentration along a boundary segment  $\Gamma_D$ :

$$c(x,z,t) = c_0(x,z,t) \quad \text{for } (x,z) \in \Gamma_D \quad (3.18)$$

whereas third-type (Cauchy type) boundary conditions may be used to prescribe the concentration flux along a boundary segment  $\Gamma_C$  as follows:

$$-\theta D_{ij} \frac{\partial c}{\partial x_j} n_i + q_i n_i c = q_i n_i c_0 \quad \text{for } (x,z) \in \Gamma_C \quad (3.19)$$

in which  $q_i n_i$  represents the outward fluid flux,  $n_i$  is the outward unit normal vector and  $c_0$  is the concentration of the incoming fluid [ML<sup>-3</sup>]. In some cases, for example when  $\Gamma_C$  is an impermeable boundary ( $q_i n_i = 0$ ) or when water flow is directed out of the region, (3.19) reduces to a second-type (Neumann type) boundary condition of the form:

$$\theta D_{ij} \frac{\partial c}{\partial x_j} n_i = 0 \quad \text{for } (x,z) \in \Gamma_N \quad (3.20)$$

A different type of boundary condition is needed for volatile solutes when they are present in both the liquid and gas phases. This situation requires a third-type boundary condition which has on the right-hand side an additional term to account for gas diffusion through a stagnant boundary layer of thickness  $d$  [L] on the soil surface. The additional solute flux is proportional to the difference in gas concentrations above and below this boundary layer [Jury et al., 1983]. This modified boundary condition has the form

$$-\theta D_{ij} \frac{\partial c}{\partial x_j} n_i + q_i n_i c = q_i n_i c_0 + \frac{D_g}{d} (k_g c - g_{atm}) \quad \text{for } (x,z) \in \Gamma_C \quad (3.21)$$

where  $D_g$  is the molecular diffusion coefficient in the gas phase [L<sup>2</sup>T<sup>-1</sup>] and  $g_{atm}$  is the gas concentration above the stagnant boundary layer [ML<sup>-3</sup>] (Jury et al. [1983] assumed  $g_{atm}$  to be zero). Similarly as for (3.19), (3.21) reduces to a second-type (Neumann type) boundary condition when water flow is zero or directed out of the region:

$$-\theta D_{ij} \frac{\partial c}{\partial x_j} n_i = \frac{D_g}{d} (k_g c - g_{atm}) \quad \text{for } (x,z) \in \Gamma_N \quad (3.22)$$

Equations (3.21) and (3.22) can only be used when the additional gas diffusion flux is

positive. *Jury et al.* [1983] discussed how to estimate the thickness of the boundary layer,  $d$ , and recommended  $d=0.5$  cm as a good average value for a bare surface.

### 3.3. Effective Dispersion Coefficient

The components of the dispersion tensor in the liquid phase,  $D_{ij}^w$ , are given by [Bear, 1972]

$$\theta D_{ij}^w = D_T |q| \delta_{ij} + (D_L - D_T) \frac{q_j q_i}{|q|} + \theta D_w \tau_w \delta_{ij} \quad (3.23)$$

where  $D_w$  is the molecular diffusion coefficient in free water [ $L^2 T^{-1}$ ],  $\tau_w$  is a tortuosity factor in the liquid phase [-],  $|q|$  is the absolute value of the Darcian fluid flux density [ $LT^{-1}$ ],  $\delta_{ij}$  is the Kronecker delta function ( $\delta_{ij}=1$  if  $i=j$ , and  $\delta_{ij}=0$  if  $i\neq j$ ), and  $D_L$  and  $D_T$  are the longitudinal and transverse dispersivities, respectively [L]. After adding the diffusion contribution from the gas phase, the individual components of the effective dispersion tensor in the soil matrix for two-dimensional transport are as follows:

$$\begin{aligned} \theta D_{xx} &= D_L \frac{q_x^2}{|q|} + D_T \frac{q_z^2}{|q|} + \theta D_w \tau_w + a_v D_g k_g \tau_g \\ \theta D_{zz} &= D_L \frac{q_z^2}{|q|} + D_T \frac{q_x^2}{|q|} + \theta D_w \tau_w + a_v D_g k_g \tau_g \\ \theta D_{xz} &= (D_L - D_T) \frac{q_x q_z}{|q|} \end{aligned} \quad (3.24)$$

where  $D_g$  is the molecular diffusion coefficient in the gas phase [ $L^2 T^{-1}$ ] and  $\tau_g$  is a tortuosity factor in the gas phase [-].

The tortuosity factors for both phases are evaluated in CHAIN\_2D as a function of the water and gas contents using the relationship of *Millington and Quirk* [1961]:

$$\tau_w = \frac{\theta^{7/3}}{\theta_s^2} \quad (3.25)$$

$$\tau_g = \frac{a_v^{7/3}}{\theta_s^2}$$

### 3.4. Temperature Dependence of Transport and Reaction Coefficients

Several of the diffusion ( $D_w$ ,  $D_g$ ), zero-order production ( $\gamma_w$ ,  $\gamma_s$ ,  $\gamma_g$ ), first-order degradation ( $\mu_w'$ ,  $\mu_s'$ ,  $\mu_g'$ ,  $\mu_w$ ,  $\mu_s$ , and  $\mu_g$ ), and adsorption ( $k_s$ ,  $k_g$ ,  $\beta$ ,  $\eta$ ,  $\omega$ ) coefficients may be strongly dependent upon temperature. CHAIN\_2D assumes that this dependency can be expressed by the Arrhenius equation [Stumm and Morgan, 1981]. After some modification, this equation can be expressed in the general form [Šimunek and Suarez, 1993a]

$$a_T = a_r \exp \left[ \frac{E_a (T^A - T_r^A)}{R_u T^A T_r^A} \right] \quad (3.26)$$

where  $a_r$  and  $a_T$  are the values of the coefficient being considered at a reference absolute temperature  $T_r^A$  and absolute temperature  $T^A$ , respectively;  $R_u$  is the universal gas constant, and  $E_a$  [ $\text{ML}^2\text{T}^{-2}\text{M}^{-1}$ ] is the activation energy of the particular reaction or process being modeled.

## 4. HEAT TRANSPORT

### 4.1. Governing Heat Transport Equations

Neglecting the effects of water vapor diffusion, two-dimensional heat transport can be described as [Sophocleous, 1979]:

$$C(\theta) \frac{\partial T}{\partial t} = \frac{\partial}{\partial x_i} [\lambda_{ij}(\theta) \frac{\partial T}{\partial x_j}] - C_w q_i \frac{\partial T}{\partial x_i} \quad (4.1)$$

where  $\lambda_{ij}(\theta)$  is the apparent thermal conductivity of the soil [ $MLT^{-3}K^{-1}$ ] (e.g.  $Wm^{-1}K^{-1}$ ) and  $C(\theta)$  and  $C_w$  are the volumetric heat capacities [ $ML^{-1}T^{-2}K^{-1}$ ] (e.g.  $Jm^{-3}K^{-1}$ ) of the porous medium and the liquid phase, respectively. Volumetric heat capacity is defined as the product of the bulk density and gravimetric heat capacity. The first term on the right-hand side of (4.1) represents heat flow due to conduction and the second term accounts for heat being transported by flowing water. We do not consider the transfer of latent heat by vapor movement. According to *de Vries* [1963] the volumetric heat capacity can be expressed as

$$C(\theta) = C_n \theta_n + C_o \theta_o + C_g \theta_g + C_w \theta_w \approx (1.92 \theta_n + 2.51 \theta_o + 4.18 \theta_g) 10^6 \quad [J m^{-3} K^{-1}] \quad (4.2)$$

where  $\theta$  refers to a volumetric fraction [ $L^3L^{-3}$ ], and subscripts  $n$ ,  $o$ ,  $g$ ,  $w$  represent solid phase, organic matter, gas phase and liquid phase, respectively.

### 4.2. Apparent Thermal Conductivity Coefficient

The apparent thermal conductivity,  $\lambda_{ij}(\theta)$ , combines the thermal conductivity  $\lambda_0(\theta)$  of the porous medium (solid plus water) in the absence of flow, and the macrodispersivity which is assumed to be a linear function of the velocity [*de Marsily*, 1986]. In analogy with the dispersion coefficient for solute transport, the apparent thermal conductivity  $\lambda_{ij}(\theta)$  is given by [*Šimunek and Suarez*, 1993b]

$$\lambda_{ij}(\theta) = \lambda_T C_w |q| \delta_{ij} + (\lambda_L - \lambda_T) C_w \frac{q_i q_j}{|q|} + \lambda_0(\theta) \delta_{ij} \quad (4.3)$$

where  $|q|$  is the absolute value of the Darcian fluid flux density [ $LT^{-1}$ ],  $\delta_{ij}$  is the Kronecker delta function as before, and  $\lambda_L$  and  $\lambda_T$  are the longitudinal and transverse thermal dispersivities [L], respectively. The individual components of the thermal conductivity tensor for two-dimensional transport are as follows:

$$\begin{aligned}\lambda_{xx} &= \lambda_L C_w \frac{q_x^2}{|q|} + \lambda_T C_w \frac{q_z^2}{|q|} + \lambda_0 \\ \lambda_{zz} &= \lambda_L C_w \frac{q_z^2}{|q|} + \lambda_T C_w \frac{q_x^2}{|q|} + \lambda_0 \\ \lambda_{xz} &= (\lambda_L - \lambda_T) C_w \frac{q_x q_z}{|q|}\end{aligned}\quad (4.4)$$

The volumetric heat capacity of the liquid phase is included here in the definition of the thermal conductivity in order to have the dimensions of the thermal dispersivities in the length units [*de Marsily*, 1986]. The thermal conductivity,  $\lambda_0(\theta)$ , accounts for the tortuosity of the porous medium, and is described with the simple equation [*Chung and Horton*, 1987]

$$\lambda_0(\theta) = b_1 + b_2 \theta_w + b_3 \theta_w^{0.5} \quad (4.5)$$

where  $b_1$ ,  $b_2$  and  $b_3$  are empirical parameters [ $MLT^{-3}K^{-1}$ ] (e.g.  $Wm^{-1}K^{-1}$ ).

#### 4.3. Initial and Boundary Conditions

Equation (4.1) will be solved subject to the general initial condition

$$T(x, z, 0) = T_i(x, z) \quad (4.6)$$

where  $T_i$  is a prescribed function of  $x$  and  $z$ .

Two types of boundary conditions (Dirichlet and Cauchy type conditions) can again be specified along the boundary of  $\Omega$ . First-type (or Dirichlet type) boundary conditions

prescribe the temperature along a boundary segment  $\Gamma_D$ :

$$T(x,z,t) = T_0(x,z,t) \quad \text{for } (x,z) \in \Gamma_D \quad (4.7)$$

whereas third-type (Cauchy type) boundary conditions prescribe the heat flux along a boundary segment  $\Gamma_C$  as follows

$$-\lambda_{ij} \frac{\partial T}{\partial x_j} n_i + T C_w q_i n_i = T_0 C_w q_i n_i \quad \text{for } (x,z) \in \Gamma_C \quad (4.8)$$

in which  $q_i n_i$  represents the outward fluid flux,  $n_i$  is the outward unit normal vector and  $T_0$  is the temperature of the incoming fluid. When  $\Gamma_C$  is an impermeable boundary ( $q_i n_i = 0$ ) or when water flow is directed out of the region, (4.8) reduces to a second-type (Neumann type) boundary condition of the form:

$$\lambda_{ij} \frac{\partial T}{\partial x_j} n_i = 0 \quad \text{for } (x,z) \in \Gamma_N \quad (4.9)$$

The atmospheric boundary condition for soil temperature is assumed to be given by a sine function as follows [Kirkham and Powers, 1972]:

$$T_0 = \bar{T} + A \sin\left(\frac{2\pi t^*}{t_p} - \frac{7\pi}{12}\right) \quad (4.10)$$

where  $t_p$  is the period of time [T] necessary to complete one cycle of the sine wave (taken to be 1 day),  $\bar{T}$  is the average temperature at the soil surface [K] during period  $t_p$ ,  $A$  is the amplitude of the sine wave [K], and  $t^*$  is the local time [T] within the period  $t_p$ . The second term within the argument of the sine function is included to allow the highest temperature to occur at 1 p.m.



## 5. NUMERICAL SOLUTION OF THE WATER FLOW EQUATION

The Galerkin finite element method with linear basis functions is used to obtain a solution of the flow equation (2.1) subject to the imposed initial and boundary conditions. Since the Galerkin method is relatively standard and has been covered in detail elsewhere [Neuman, 1975; Zienkiewicz, 1977; Pinder and Gray, 1977], only the most pertinent steps in the solution process are given here. The development given here is the same as in the SWMS\_2D manual [*Šimunek et al.*, 1992].

### 5.1. Space Discretization

The flow region is divided into a network of triangular elements. The corners of these elements are taken to be the nodal points. The dependent variable, the pressure head function  $h(x,z,t)$ , is approximated by a function  $h'(x,z,t)$  as follows

$$h'(x,z,t) = \sum_{n=1}^N \phi_n(x,z) h_n(t) \quad (5.1)$$

where  $\phi_n$  are piecewise linear basis functions satisfying the condition  $\phi_n(x_m, z_m) = \delta_{nm}$ ,  $h_n$  are unknown coefficients representing the solution of (2.1) at the nodal points, and  $N$  is the total number of nodal points.

The Galerkin method postulates that the differential operator associated with the Richards' equation (2.1) is orthogonal to each of the  $N$  basis functions, i.e.,

$$\int_{\Omega} \left\{ \frac{\partial \theta}{\partial t} - \frac{\partial}{\partial x_i} \left[ K(K_{ij}^A \frac{\partial h}{\partial x_j} + K_{iz}^A) \right] + S \right\} \phi_n d\Omega = 0 \quad (5.2)$$

Applying Green's first identity to (5.2), and replacing  $h$  by  $h'$ , leads to

$$\sum_e \int_{\Omega_e} \left( \frac{\partial \theta}{\partial t} \phi_n + K K_{ij}^A \frac{\partial h'}{\partial x_j} \frac{\partial \phi_n}{\partial x_i} \right) d\Omega =$$

$$\sum_e \int_{\Gamma_e} K \left( K_{ij}^A \frac{\partial h'}{\partial x_j} + K_{iz}^A \right) n_i \phi_n d\Gamma + \sum_e \int_{\Omega_e} \left( -K K_{iz}^A \frac{\partial \phi_n}{\partial x_i} - S \phi_n \right) d\Omega$$
(5.3)

where  $\Omega_e$  represents the domain occupied by element  $e$ , and  $\Gamma_e$  is a boundary segment of element  $e$ . Natural flux-type (Neumann) and gradient type boundary conditions can be immediately incorporated into the numerical scheme by specifying the line integral in equation (5.3).

After imposing additional simplifying assumptions to be discussed later, and performing integration over the elements, the procedure leads to a system of time-dependent ordinary differential equations with nonlinear coefficients. In matrix form, these equations are given by

$$[F] \frac{d\{\theta\}}{dt} + [A]\{h\} = \{Q\} - \{B\} - \{D\}$$
(5.4)

where

$$A_{nm} = \sum_e K_l K_{ij}^A \int_{\Omega_e} \phi_l \frac{\partial \phi_n}{\partial x_i} \frac{\partial \phi_m}{\partial x_j} d\Omega$$

$$= \sum_e \frac{\kappa}{4A_e} \bar{K} [K_{xx}^A b_m b_n + K_{xz}^A (c_m b_n + b_m c_n) + K_{zz}^A c_n c_m]$$
(5.5)

$$B_n = \sum_e K_l K_{iz}^A \int_{\Omega_e} \phi_l \frac{\partial \phi_n}{\partial x_i} d\Omega = \sum_e \frac{\kappa}{2} \bar{K} (K_{xz}^A b_n + K_{zz}^A c_n)$$
(5.6)

$$F_{nm} = \delta_{nm} \sum_e \int_{\Omega_e} \phi_n d\Omega = \delta_{nm} \sum_e \frac{\kappa}{3} A_e \quad (5.7)$$

$$Q_n = - \sum_e \sigma_{1l} \int_{\Gamma_e} \phi_l \phi_n d\Gamma = - \sum_e \sigma_{1n} \lambda_n \quad (5.8)$$

$$D_n = \sum_e S_l \int_{\Omega_e} \phi_l \phi_n d\Omega = \sum_e \frac{\kappa}{12} A_e (3 \bar{S} + S_n) \quad (5.9)$$

where the overlined variables represent average values over an element  $e$ , the subscripts  $i$  and  $j$  are space direction indices ( $i,j = 1,2$ ), and

$$l = 1, 2, \dots, N \quad m = 1, 2, \dots, N \quad n = 1, 2, \dots, N$$

$$\begin{aligned} b_i &= z_j - z_k & c_i &= x_k - x_j \\ b_j &= z_k - z_i & c_j &= x_i - x_k \\ b_k &= z_i - z_j & c_k &= x_j - x_i \end{aligned} \quad (5.10)$$

$$A_e = \frac{c_k b_j - c_j b_k}{2} \quad \bar{K} = \frac{K_i + K_j + K_k}{3} \quad \bar{S} = \frac{S_i + S_j + S_k}{3}$$

Equation (5.8) is valid for a flux-type boundary condition. For a gradient-type boundary condition the variable  $\sigma_1$  in (5.8) must be replaced by the product of the hydraulic conductivity  $K$  and the prescribed gradient  $\sigma_2 (= 1)$ . Equations (5.5) through (5.9) hold for flow in a two-dimensional Cartesian  $(x,z)$  domain, as well as for flow in an axisymmetric  $(x,z)$  system in which  $x$  is used as the radial coordinate. For plane flow we have

$$\kappa = 1 \quad \lambda_n = \frac{L_n}{2} \quad (5.11)$$

while for axisymmetric flow

$$\kappa = 2\pi \frac{x_i + x_j + x_k}{3} \quad \lambda_n = L_n \pi \frac{x'_n + 2x_n}{3} \quad (5.12)$$

The subscripts  $i$ ,  $j$  and  $k$  in equations (5.10) and (5.12) represent the three corners of a triangular element  $e$ .  $A_e$  is the area of element  $e$ ,  $\bar{K}$  and  $\bar{S}$  are the average hydraulic conductivity and root water extraction values over element  $e$ ,  $L_n$  is the length of the boundary segment connected to node  $n$ , and  $x'_n$  is the  $x$ -coordinate of a boundary node adjacent to node  $n$ . The symbol  $\sigma_n$  in equation (5.8) stands for the flux [ $LT^{-1}$ ] across the boundary in the vicinity of boundary node  $n$  (positive when directed outward of the system). The boundary flux is assumed to be uniform over each boundary segment. The entries of the vector  $Q_n$  are zero at all internal nodes which do not act as sources or sinks for water.

The numerical procedure leading to (5.4) incorporates two important assumptions in addition to those related to the Galerkin finite element approach. One assumption concerns the time derivatives of the nodal values of the water content in (5.4). These time derivatives were weighted according to

$$\frac{d\theta_n}{dt} = \frac{\sum_e \int_{\Omega_e} \frac{\partial \theta}{\partial t} \phi_n d\Omega}{\sum_e \int_{\Omega_e} \phi_n d\Omega} \quad (5.13)$$

This assumption implements mass-lumping which has been shown to improve the rate of convergence of the iterative solution process [e.g., Neuman, 1973].

A second assumption in the numerical scheme is related to the anisotropy tensor  $K^A$  which is taken to be constant over each element. By contrast, the water content  $\theta$ , the hydraulic conductivity  $K$ , the soil water capacity  $C$ , and the root water extraction rate  $S$ , at a given point in time are assumed to vary linearly over each element,  $e$ . For example, the water content is expanded over each element as follows:

$$\theta(x, z) = \sum_{n=1}^3 \theta(x_n, z_n) \phi_n(x, z) \quad \text{for } (x, z) \in \Omega_e \quad (5.14)$$

where  $n$  stands for the corners of element  $e$ . The advantage of linear interpolation is that no numerical integration is needed to evaluate the coefficients in (5.4).

## 5.2. Time Discretization

Integration of (5.4) in time is achieved by discretizing the time domain into a sequence of finite intervals and replacing the time derivatives by finite differences. An implicit (backward) finite difference scheme is used for both saturated and unsaturated conditions:

$$[F] \frac{\{\theta\}_{j+1} - \{\theta\}_j}{\Delta t_j} + [A]_{j+1} \{h\}_{j+1} = \{Q\}_j - \{B\}_{j+1} - \{D\}_j \quad (5.15)$$

where  $j+1$  denotes the current time level at which the solution is being considered,  $j$  refers to the previous time level, and  $\Delta t_j = t_{j+1} - t_j$ . Equation (5.15) represents the final set of algebraic equations to be solved. Since the coefficients  $\theta$ ,  $A$ ,  $B$ ,  $D$ , and  $Q$  ( $Q$  for only gradient-type boundary conditions) are functions of  $h$ , the set of equations is generally highly nonlinear. Note that the vectors  $D$  and  $Q$  are evaluated at the old time level.

## 5.3. Numerical Solution Strategy

### 5.3.1. Iterative Process

Because of the nonlinear nature of (5.15), an iterative process must be used to obtain solutions of the global matrix equation at each new time step. For each iteration a system of linearized algebraic equations is first derived from (5.15) which, after incorporation of the boundary conditions, is solved using either Gaussian elimination or the conjugate gradient method (see Section 7.5). The Gaussian elimination process takes advantage of

the banded and symmetric features of the coefficient matrices in (5.15). After inversion, the coefficients in (5.15) are re-evaluated using the first solution, and the new equations are again solved. The iterative process continues until a satisfactory degree of convergence is obtained, i.e., until at all nodes in the saturated (or unsaturated) region the absolute change in pressure head (or water content) between two successive iterations becomes less than some small value determined by the imposed absolute pressure head (or water content) tolerance [Šimunek and Suarez, 1993b]. The first estimate (at zero iteration) of the unknown pressure heads at each time step is obtained by extrapolation from the pressure head values at the previous two time levels.

### 5.3.2. Treatment of the Water Capacity Term

The iteration process is extremely sensitive to the method used for evaluating the water content term ( $\Delta\theta/\Delta t$ ) in equation (5.15). The present version of CHAIN\_2D code uses the "mass-conservative" method proposed by Celia *et al.* [1990]. Their method has been shown to provide excellent results in terms of minimizing the mass balance error. The mass-conservative method proceeds by separating the water content term into two parts:

$$[F] \frac{\{\theta\}_{j+1} - \{\theta\}_j}{\Delta t_j} = [F] \frac{\{\theta\}_{j+1}^{k+1} - \{\theta\}_{j+1}^k}{\Delta t_j} + [F] \frac{\{\theta\}_{j+1}^k - \{\theta\}_j}{\Delta t_j} \quad (5.16)$$

where  $k+1$  and  $k$  denote the current and previous iteration levels, respectively; and  $j+1$  and  $j$  the current and previous time levels, respectively. Notice that the second term on the right hand side of (5.16) is known prior to the current iteration. The first term on the right hand side can be expressed in terms of the pressure head, so that (5.16) becomes

$$[F] \frac{\{\theta\}_{j+1} - \{\theta\}_j}{\Delta t_j} = [F][C]_{j+1} \frac{\{h\}_{j+1}^{k+1} - \{h\}_{j+1}^k}{\Delta t_j} + [F] \frac{\{\theta\}_{j+1}^k - \{\theta\}_j}{\Delta t_j} \quad (5.17)$$

where  $C_{nm} = \delta_{nm} C_n$ , in which  $C_n$  represents the nodal value of the soil water capacity. The first term on the right hand side of (5.17) should vanish at the end of the iteration process if the numerical solution converges. This particular feature guarantees relatively small mass

balance errors in the solution.

### 5.3.3. Time Control

Three different time discretizations are introduced in CHAIN\_2D: (1) time discretizations associated with the numerical solution, (2) time discretizations associated with the implementation of boundary conditions, and (3) time discretizations which provide printed output of the simulation results (e.g., nodal values of dependent variables, water and solute mass balance components, and other information about the flow regime).

Discretizations 2 and 3 are mutually independent; they generally involve variable time steps as described in the input data file. Discretization 1 starts with a prescribed initial time increment,  $\Delta t$ . This time increment is automatically adjusted at each time level according to the following rules [Mls, 1982; Vogel, 1987]:

- a. Discretization 1 must coincide with time values resulting from discretizations 2 and 3.
- b. Time increments cannot become less than a preselected minimum time step,  $\Delta t_{min}$ , nor exceed a maximum time step,  $\Delta t_{max}$  (i.e.,  $\Delta t_{min} \leq \Delta t \leq \Delta t_{max}$ ).
- c. If, during a particular time step, the number of iterations necessary to reach convergence is  $\leq 3$ , the time increment for the next time step is increased by multiplying  $\Delta t$  by a predetermined constant  $> 1$  (usually between 1.1 and 1.5). If the number of iterations is  $\geq 7$ ,  $\Delta t$  for the next time level is multiplied by a constant  $< 1$  (usually between 0.3 and 0.9).
- d. If, during a particular time step, the number of iterations at any time level becomes greater than a prescribed maximum (usually between 10 and 50), the iterative process for that time level is terminated. The time step is subsequently reset to  $\Delta t/3$ , and the iterative process restarted.

The selection of optimal time steps,  $\Delta t$ , is also influenced by the solution scheme for solute transport (see Section 6.3.6).

### 5.3.4. Treatment of Pressure Head Boundary Conditions

Finite element equations corresponding to Dirichlet nodes where the pressure head is prescribed can, at least in principle, be eliminated from the global matrix equation. An alternative and numerically simpler approach is to replace the Dirichlet finite element equations by dummy expressions of the form [Neuman, 1974]

$$\delta_{nm} h_m = \psi_n \quad (5.18)$$

where  $\delta_{nm}$  is the Kronecker delta and  $\psi_n$  is the prescribed value of the pressure head at node  $n$ . The values of  $h_m$  in all other equations are set equal to  $\psi_n$  and the appropriate entries containing  $\psi_n$  in the left hand side matrix are incorporated into the known vector on the right-hand side of the global matrix equation. When done properly, this rearrangement will preserve symmetry in the matrix equation. This procedure is applied only when Gaussian elimination is used to solve the matrix equations. When the conjugate gradient solver is used, then the finite element equation representing the Dirichlet node is modified as follows. The right hand side of this equation is set equal to the prescribed pressure head multiplied by a large number ( $10^{30}$ ), and entry on the left hand side representing the Dirichlet node is set equal to this large number. After solving for all pressure heads, the value of the flux  $Q_n$  can be calculated explicitly and accurately from the original finite element equation associated with node  $n$  [e.g., Lynch, 1984].

### 5.3.5. Flux and Gradient Boundary Conditions

The values of the fluxes  $Q_n$  at nodal points along prescribed flux and gradient boundaries are computed according to equation (5.8). Internal nodes which act as Neumann type sources or sinks have values of  $Q_n$  equal to the imposed fluid injection or extraction rate.

### 5.3.6. Atmospheric Boundary Conditions and Seepage Faces

Atmospheric boundaries are simulated by applying either prescribed head or prescribed flux boundary conditions depending upon whether equation (2.29) or (2.30) is satisfied [Neuman, 1974]. If (2.30) is not satisfied, node  $n$  becomes a prescribed head boundary. If, at any point in time during the computations, the calculated flux exceeds the specified potential flux in (2.29), the node will be assigned a flux equal to the potential value and treated again as a prescribed flux boundary.

All nodes expected to be part of a seepage face during code execution must be identified a priori. During each iteration, the saturated part of a potential seepage face is treated as a prescribed pressure head boundary with  $h=0$ , while the unsaturated part is treated as a prescribed flux boundary with  $Q=0$ . The lengths of the two surface segments are continually adjusted [Neuman, 1974] during the iterative process until the calculated values of  $Q$  (equation (5.8)) along the saturated part, and the calculated values of  $h$  along the unsaturated part, are all negative, thus indicating that water is leaving the flow region through the saturated part of the surface boundary only.

### 4.3.7. Tile Drains as Boundary Conditions

The representation of tile drains as boundary conditions is based on studies by Vimoke *et al.* [1963] and Fipps *et al.* [1986]. The approach uses results of electric analog experiments conducted by Vimoke and Taylor [1962] who reasoned that drains can be represented by nodal points in a regular finite element mesh, provided adjustments are made in the hydraulic conductivity,  $K$ , of neighboring elements. The adjustments should correspond to changes in the electric resistance of conducting paper as follows

$$K_{\text{drain}} = K C_d \quad (5.19)$$

where  $K_{\text{drain}}$  is the adjusted conductivity [ $\text{LT}^{-1}$ ], and  $C_d$  is the correction factor [-].  $C_d$  is determined from the ratio of the effective radius,  $d_e$  [L], of the drain to the side length,  $D$

[L], of the square formed by finite elements surrounding the drain node [Vimoke *et al.*, 1962]:

$$C_d = \frac{Z'_0}{Z_0} \approx \frac{\sqrt{\mu_0 / \epsilon_0}}{138 \log_{10} \rho_d + 6.48 - 2.34A - 0.48B - 0.12C} \quad (5.20)$$

where  $Z'_0$  is the characteristic impedance of free space ( $\approx 376.7$  ohms),  $\mu_0$  is the permeability of free space,  $\epsilon_0$  is the permittivity of free space, and  $Z_0$  is the characteristic impedance of a transmission line analog of the drain. The coefficients in (5.20) are given by

$$\begin{aligned} \rho_d &= \frac{D}{d_e} & A &= \frac{1 + 0.405 \rho_d^{-4}}{1 - 0.405 \rho_d^{-4}} \\ B &= \frac{1 + 0.163 \rho_d^{-8}}{1 - 0.163 \rho_d^{-8}} & C &= \frac{1 + 0.067 \rho_d^{-12}}{1 - 0.067 \rho_d^{-12}} \end{aligned} \quad (5.21)$$

where  $d_e$  is the effective drain diameter to be calculated from the number and size of small openings in the drain tube [Mohammad and Skaggs, 1984], and  $D$  is the size of the square in the finite element mesh surrounding the drain having adjusted hydraulic conductivities. The approach above assumes that the node representing a drain must be surrounded by finite elements (either triangular or quadrilateral) which form a square whose hydraulic conductivities are adjusted according to (5.19). This method of implementing drains by means of a boundary condition gives an efficient, yet relatively accurate, prediction of the hydraulic head in the immediate vicinity of the drain, as well as of the drain flow rate [Fipps *et al.*, 1986]. More recent studies have shown that the correction factor,  $C_d$ , could be further reduced by a factor of 2 [Rogers and Fouss, 1989] or 4 [Tseng, 1994, personal communication]. These two studies compared numerical simulations of the flow of ponded water into a tile drain system with an analytical solution given by Kirkham [1949]. Pressure head contours calculated numerically with the original correction factor  $C_d$  (5.20), as well as with the additionally reduced correction factor  $C_d/4$ , were compared with the analytical results in Šimůnek *et al.* [1994].

### 5.3.8. Water Balance Computations

The CHAIN\_2D code performs water balance computations at prescribed times for several preselected subregions of the flow domain. The water balance information for each subregion consists of the actual volume of water,  $V$ , in that subregion, and the rate,  $O$ , of inflow or outflow to or from the subregion.  $V$  and  $O$  are given by

$$V = \sum_e \kappa A_e \frac{\theta_i + \theta_j + \theta_k}{3} \quad (5.22)$$

and

$$O = \frac{V_{new} - V_{old}}{\Delta t} \quad (5.23)$$

respectively, where  $\theta_i$ ,  $\theta_j$ , and  $\theta_k$  are water contents evaluated at the corner nodes of element  $e$ , and where  $V_{new}$  and  $V_{old}$  are volumes of water in the subregion computed at the current and previous time levels, respectively. The summation in (5.22) is taken over all elements within the subregion.

The absolute error in the mass balance is calculated as

$$\epsilon_a^w = V_t - V_0 + L_t \int_0^t T_a dt - \int_0^t \sum_{n_r} Q_n dt \quad (5.24)$$

where  $V_t$  and  $V_0$  are the volumes of water in the flow domain at time  $t$  and zero, respectively, as calculated with (5.22). The third term on the right-hand side represents the cumulative root water uptake amount, while the fourth term gives the cumulative flux through nodes,  $n_r$ , located along the boundary of the flow domain or at internal source and sink nodes.

The accuracy of the numerical solution is evaluated in terms of the relative error,  $\epsilon_r^w$  [%], in the water mass balance as follows:

$$\epsilon_r^w = \frac{|\epsilon_a^w|}{\max \left[ \sum_e |V_t^e - V_0^e|, L_t \int_0^t T_a dt + \int_0^t \sum_n |Q_n| dt \right]} \cdot 100 \quad (5.25)$$

where  $V_t^e$  and  $V_0^e$  are the volumes of water in element  $e$  at times  $t$  and zero, respectively. Note that CHAIN\_2D does not relate the absolute error to the volume of water in the flow domain, but instead to the maximum value of two quantities. The first quantity represents the sum of the absolute changes in water content over all elements, whereas the second quantity is the sum of the absolute values of all fluxes in and out of the flow domain. This criterion is much more strict than the usual criterion involving the total volume of water in the flow domain. This is because cumulative boundary fluxes are often much smaller than the volume in the domain, especially at the beginning of the simulation.

### 5.3.9. Computation of Nodal Fluxes

Components of the Darcian flux are computed at each time level during the simulation only when the water flow and solute transport equations are solved simultaneously. When the flow equation is being solved alone, the flux components are calculated only at selected print times. The  $x$ - and  $z$ -components of the nodal fluxes are computed for each node  $n$  according to:

$$q_x = -\frac{K_n}{N_e} \sum_{e_n} \left[ \frac{\gamma_i^x h_i + \gamma_j^x h_j + \gamma_k^x h_k}{2A_e} + K_{xz}^A \right]$$

$$q_z = -\frac{K_n}{N_e} \sum_{e_n} \left[ \frac{\gamma_i^z h_i + \gamma_j^z h_j + \gamma_k^z h_k}{2A_e} + K_{zz}^A \right] \quad (5.26)$$

$$\gamma_n^x = K_{xx}^A b_n + K_{xz}^A c_n$$

$$\gamma_n^z = K_{xz}^A b_n + K_{zz}^A c_n$$

where  $N_e$  is the number of sub-elements  $e_n$  adjacent to node  $n$ . Einstein's summation

convention is not used in (5.26).

### 5.3.10. Water Uptake by Plant Roots

CHAIN\_2D considers the root zone to consist of all nodes,  $n$ , for which the potential root water uptake distribution,  $b$  (see Section 2.2), is greater than zero. The root water extraction rate is assumed to vary linearly over each element; this leads to approximation (5.9) for the root water extraction term  $D_n$  in the global matrix equation. The values of the actual root extraction rate  $S_n$  in (5.9) are evaluated with (2.9). In order to speed up the calculations, the extraction rates  $S_n$  are calculated at the old time level and are not updated during the iterative solution process at a given time step. CHAIN\_2D calculates the total rate of transpiration per unit soil surface length using the equation

$$T_a = \frac{1}{L_t} \sum_e \kappa A_e \bar{S} \quad (5.27)$$

in which the summation takes place over all elements within the root zone.

### 5.3.11. Evaluation of the Soil Hydraulic Properties

At the beginning of a numerical simulation, CHAIN\_2D generates for each soil type in the flow domain a table of water contents, hydraulic conductivities, and specific water capacities from the specified set of hydraulic parameters. The values of  $\theta_i$ ,  $K_i$  and  $C_i$  in the table are evaluated at prescribed pressure heads  $h_i$  within a specified interval  $(h_a, h_b)$ . The entries in the table are generated such that

$$\frac{h_{i+1}}{h_i} = \text{constant} \quad (5.28)$$

which means that the spacing between two consecutive pressure head values increases in a logarithmic fashion. Values for the hydraulic properties,  $\theta(h)$ ,  $K(h)$  and  $C(h)$ , are computed

during the iterative solution process using linear interpolation between the entries in the table. If an argument  $h$  falls outside the prescribed interval ( $h_a, h_b$ ), the hydraulic characteristics are evaluated directly from the hydraulic functions, i.e., without interpolation. The above interpolation technique was found to be much faster computationally than direct evaluation of the hydraulic functions over the entire range of pressure heads, except when very simple hydraulic models were used.

### 5.3.12. *Implementation of Hydraulic Conductivity Anisotropy*

Since the hydraulic conductivity anisotropy tensor,  $\mathbf{K}^A$ , is assumed to be symmetric, it is possible to define at any point in the flow domain a local coordinate system for which the tensor  $\mathbf{K}^A$  is diagonal (i.e., having zeroes everywhere except on the diagonal). The diagonal entries  $K_1^A$  and  $K_2^A$  of  $\mathbf{K}^A$  are referred to as the principal components of  $\mathbf{K}^A$ .

The CHAIN\_2D code permits one to vary the orientation of the local principal directions from element to element. For this purpose, the local coordinate axes are subjected to a rotation such that they coincide with the principal directions of the tensor  $\mathbf{K}^A$ . The principal components  $K_1^A$  and  $K_2^A$ , together with the angle  $\omega_a$  between the principal direction of  $K_1^A$  and the  $x$ -axis of the global coordinate system, are specified for each element. Each locally determined tensor  $\mathbf{K}^A$  is transformed to the global ( $x,z$ ) coordinate system at the beginning of the simulation using the following rules:

$$\begin{aligned} K_{xx}^A &= K_1^A \cos^2 \omega_a + K_2^A \sin^2 \omega_a \\ K_{zz}^A &= K_1^A \sin^2 \omega_a + K_2^A \cos^2 \omega_a \\ K_{xz}^A &= (K_2^A - K_1^A) \sin \omega_a \cos \omega_a \end{aligned} \quad (5.29)$$

### 5.3.13. *Steady-State Analysis*

All transient flow problems are solved by time marching until a prescribed time is

reached. The steady-state problem can be solved in the same way, i.e., by time marching until two successive solutions differ less than some prescribed pressure head tolerance. **CHAIN\_2D** implements a faster way of obtaining the steady-state solution without having to go through a large number of time steps. The steady-state solution for a set of imposed boundary conditions is obtained directly during one set of iterations at the first time step by equating the time derivative term in the Richards' equation (2.1) to zero.



## 6. NUMERICAL SOLUTION OF THE SOLUTE TRANSPORT EQUATION

The Galerkin finite element method is also used to solve the solute and heat transport equations (equations (3.11) and (4.1), respectively) subject to appropriate initial and boundary conditions. Since the heat transport equation (4.1) has the same mathematical form as the (linearized) solute transport equation (3.11), the numerical solution will be given here only for solute transport. The solution procedure largely parallels the approach used in Section 5 for the flow equation.

### 6.1. Space Discretization

The dependent variable, the concentration function  $c(x,z,t)$ , is approximated by a finite series  $c'(x,z,t)$  of the form

$$c'(x,z,t) = \sum_{n=1}^N \phi_n(x,z) c_n(t) \quad (6.1)$$

where  $\phi_n$  are the selected linear basis functions,  $c_n$  are the unknown time dependent coefficients which represent solutions of (3.11) at the finite element nodal points and, as before,  $N$  is the total number of nodal points. Application of the standard Galerkin method leads to the following set of  $N$  equations

$$\int_{\Omega} [-\theta R \frac{\partial c}{\partial t} - q_i \frac{\partial c}{\partial x_i} + \frac{\partial}{\partial x_i} (\theta D_{ij} \frac{\partial c}{\partial x_j}) + Fc + G] \phi_n d\Omega = 0 \quad (6.2)$$

Application of Green's theorem to the second derivatives in (6.2) and substitution of  $c$  by  $c'$  results in the following system of time-dependent differential equations

$$\begin{aligned} & \sum_e \int_{\Omega_e} [(-\theta R \frac{\partial c'}{\partial t} - q_i \frac{\partial c'}{\partial x_i} + Fc' + G) \phi_n - \theta D_{ij} \frac{\partial c'}{\partial x_j} \frac{\partial \phi_n}{\partial x_i}] d\Omega \\ & + \sum_e \int_{\Gamma_e} \theta D_{ij} \frac{\partial c'}{\partial x_j} n_i \phi_n d\Gamma = 0 \end{aligned} \quad (6.3)$$

or in matrix form:

$$[Q] \frac{d\{c\}}{dt} + [S]\{c\} + \{f\} = -\{Q^D\} \quad (6.4)$$

where

$$Q_{nm} = \delta_{nm} \sum_e (-\theta R)_l \int_{\Omega_e} \phi_l \phi_n d\Omega = - \sum_e \frac{\kappa A_e}{12} (3\bar{\theta R} + \theta_n R_n) \delta_{nm} \quad (6.5)$$

$$\begin{aligned} S_{nm} &= \sum_e [(-q_i)_l \int_{\Omega_e} \phi_l \phi_n \frac{\partial \phi_m}{\partial x_i} d\Omega - (\theta D_{ij})_l \int_{\Omega_e} \phi_l \frac{\partial \phi_n}{\partial x_i} \frac{\partial \phi_m}{\partial x_j} d\Omega + F_l \int_{\Omega_e} \phi_l \phi_n \phi_m d\Omega] = \\ &= \sum_e \left\{ -\frac{\kappa b_m}{24} (3\bar{q}_x + q_{xn}) - \frac{\kappa c_m}{24} (3\bar{q}_z + q_{zn}) + \frac{\kappa A_e}{60} (3\bar{F} + F_n + F_m) (1 + \delta_{nm}) \right. \\ &\quad \left. - \frac{\kappa}{4A_e} [b_m b_n \theta \bar{D}_{xx} + (b_m c_n + c_m b_n) \theta \bar{D}_{xz} + c_m c_n \theta \bar{D}_{zz}] \right\} \end{aligned} \quad (6.6)$$

$$f_n = \sum_e G_l \int_{\Omega_e} \phi_l \phi_n d\Omega = \sum_e \frac{\kappa A_e}{12} (3\bar{G} + G_n) \quad (6.7)$$

in which the overlined variables represent average values over a given element  $e$ . The notation in the above equations is similar as in (5.10). The boundary integral in (6.3) represents the dispersive flux,  $Q_n^D$ , across the boundary and will be discussed later in Section 6.3.4.

The derivation of equations (6.5) through (6.7) invoked several important assumptions in addition to those involved in the Galerkin finite element approach [*Huyakorn and Pinder, 1983; van Genuchten, 1978*]. First, the different coefficients under the integral signs ( $\theta R$ ,  $q_i$ ,  $\theta D_{ij}$ ,  $F$ ,  $G$ ) were expanded linearly over each element, similarly as for the dependent variable, i.e., in terms of their nodal values and associated basis functions. Second, mass lumping was invoked by redefining the nodal values of the time derivative in (6.4) as weighted averages over the entire flow region:

$$\frac{dc_n}{dt} = \frac{\sum_e \int_{\Omega_e} \theta R \frac{\partial c'}{\partial t} \phi_n d\Omega}{\sum_e \int_{\Omega_e} \theta R \phi_n d\Omega} \quad (6.8)$$

## 6.2. Time Discretization

The Galerkin method is used only for approximating the spatial derivatives while the time derivatives are discretized by means of finite differences. A first-order approximation of the time derivatives leads to the following set of algebraic equations:

$$[Q]_{j+\epsilon} \frac{\{c\}_{j+1} - \{c\}_j}{\Delta t} + \epsilon [S]_{j+1} \{c\}_{j+1} + (1 - \epsilon) [S]_j \{c\}_j + \epsilon \{f\}_{j+1} + (1 - \epsilon) \{f\}_j = 0 \quad (6.9)$$

where  $j$  and  $j+1$  denote the previous and current time levels, respectively;  $\Delta t$  is the time increment, and  $\epsilon$  is a time weighing factor. The incorporation of the dispersion flux,  $Q_n^D$ , into matrix  $[Q]$  and vector  $\{f\}$  is discussed in Section 6.3.4. The coefficient matrix  $[Q]_{j+\epsilon}$  is evaluated using weighted averages of the current and previous nodal values of  $\theta$  and  $R$ . Equation (6.9) can be rewritten in the form:

$$[G]\{c\}_{j+1} = \{g\} \quad (6.10)$$

where

$$\begin{aligned} [G] &= \frac{1}{\Delta t} [Q]_{j+\epsilon} + \epsilon [S]_{j+1} \\ \{g\} &= \frac{1}{\Delta t} [Q]_{j+\epsilon} \{c\}_j - (1 - \epsilon) [S]_j \{c\}_j - \epsilon \{f\}_{j+1} - (1 - \epsilon) \{f\}_j \end{aligned} \quad (6.11)$$

Higher-order approximations for the time derivative in the transport equation were derived by *van Genuchten* [1976, 1978]. The higher-order effects may be incorporated into

the transport equation by introducing time-dependent dispersion corrections as follows

$$D_{ij}^- = D_{ij} - \frac{q_i q_j \Delta t}{6\theta^2 R} \quad (6.12)$$

$$D_{ij}^+ = D_{ij} + \frac{q_i q_j \Delta t}{6\theta^2 R}$$

where the superscripts + and - indicate evaluation at the old and new time levels, respectively.

### 6.3. Numerical Solution Strategy

#### 6.3.1. Solution Process

The solution process at each time step proceeds as follows. First, an iterative procedure is used to obtain the solution of the Richards' equation (2.1) (see Section 5.3.1). After achieving convergence, the solution of the transport equation (6.10) is implemented. This is done by first determining the nodal values of the fluid flux from nodal values of the pressure head by applying Darcy's law. Nodal values of the water content and the fluid flux at the previous time level are already known from the solution at the previous time step. Values for the water content and the fluid flux are subsequently used as input to the transport equations (first for heat transport and then for solute transport), leading to the system of linear algebraic equations given by (6.10). The structure of the final set of equations depends upon the value of the temporal weighing factor,  $\epsilon$ . The explicit ( $\epsilon=0$ ) and fully implicit ( $\epsilon=1$ ) schemes require that the global matrix  $[G]$  and the vector  $\{g\}$  be evaluated at only one time level (the previous or current time level). All other schemes require evaluation at both time levels. Also, all schemes except for the explicit formulation ( $\epsilon=0$ ) lead to an asymmetric banded matrix  $[G]$ . The associated set of algebraic equations is solved using either a standard asymmetric matrix equation solver [e.g., Neuman, 1972], or the ORTHOMIN method [Mendoza *et al.*, 1991], depending upon the size of final matrix. By contrast, the explicit scheme leads to a diagonal matrix  $[G]$  which is much easier to solve

(but generally requires smaller time steps).

Since the heat transport equation is linear, there is no need for an iterative solution process for heat flow. The same is true for the transport of solutes undergoing only linear sorption reactions. On the other hand, iteration is needed when a nonlinear reaction between the solid and liquid phase is considered. The iteration procedure for solute transport is very similar to that for water flow. The coefficients in (6.10) are re-evaluated using each iteration, and the new equations are again solved using results of the previous iteration. The iterative process continues until a satisfactory degree of convergence is obtained, i.e., until at all nodes the absolute change in concentration between two successive iterations becomes less than some small value determined by the imposed relative and absolute concentration tolerances.

### 6.3.2. Upstream Weighted Formulation

Upstream weighing is provided as an option in the CHAIN\_2D to minimize some of the problems with numerical oscillations when relatively steep concentration fronts are being simulated. For this purpose the second (flux) term of equation (6.3) is not weighted by regular linear basis functions  $\phi_n$ , but instead using the nonlinear functions  $\phi_n^w$  [Yeh and Tripathi, 1990]

$$\begin{aligned}\phi_1^w &= L_1 - 3\alpha_3^w L_2 L_1 + 3\alpha_2^w L_3 L_1 \\ \phi_2^w &= L_2 - 3\alpha_1^w L_3 L_2 + 3\alpha_3^w L_1 L_2 \\ \phi_3^w &= L_3 - 3\alpha_2^w L_1 L_3 + 3\alpha_1^w L_2 L_3\end{aligned}\tag{6.13}$$

where  $\alpha_i^w$  is a weighing factor associated with the length of the element size opposite to node  $i$ , and  $L_i$  are the local coordinates. The weighing factors are evaluated using the equation of Christie *et al.* [1976]:

$$\alpha_i^w = \coth\left(\frac{uL}{2D}\right) - \frac{2D}{uL} \quad (6.14)$$

where  $u$ ,  $D$  and  $L$  are the flow velocity, dispersion coefficient and length associated with side  $i$ . The weighing functions  $\phi^w$  ensure that relatively more weight is placed on the flow velocities of nodes located at the upstream side of an element. Evaluating the integrals in (6.3) shows that the following additional terms must be added to the entries of the global matrix  $S_{nm}$  in equation (6.6):

$$S_{ij}^{e'} = S_{ij}^e - \frac{b_j}{40} [2q_{x1}(\alpha_2^w - \alpha_3^w) + q_{x2}(\alpha_2^w - 2\alpha_3^w) + q_{x3}(2\alpha_2^w - \alpha_3^w)] \\ - \frac{c_j}{40} [2q_{z1}(\alpha_2^w - \alpha_3^w) + q_{z2}(\alpha_2^w - 2\alpha_3^w) + q_{z3}(2\alpha_2^w - \alpha_3^w)] \quad (6.15)$$

$$S_{2j}^{e'} = S_{2j}^e - \frac{b_j}{40} [q_{x1}(2\alpha_3^w - \alpha_1^w) + 2q_{x2}(\alpha_3^w - \alpha_1^w) + q_{x3}(\alpha_3^w - 2\alpha_1^w)] \\ - \frac{c_j}{40} [q_{z1}(2\alpha_3^w - \alpha_1^w) + 2q_{z2}(\alpha_3^w - \alpha_1^w) + q_{z3}(\alpha_3^w - 2\alpha_1^w)] \quad (6.16)$$

and

$$S_{3j}^{e'} = S_{3j}^e - \frac{b_j}{40} [q_{x1}(\alpha_1^w - 2\alpha_2^w) + q_{x2}(2\alpha_1^w - \alpha_2^w) + 2q_{x3}(\alpha_1^w - \alpha_2^w)] \\ - \frac{c_j}{40} [q_{z1}(\alpha_1^w - 2\alpha_2^w) + q_{z2}(2\alpha_1^w - \alpha_2^w) + 2q_{z3}(\alpha_1^w - \alpha_2^w)] \quad (6.17)$$

The weighing factors are applied only to those element sides that are inclined within 10 degrees from the flow direction.

### 6.3.3. Implementation of First-Type Boundary Conditions

Individual equations in the global matrix equation which correspond to nodes at which the concentration is prescribed are replaced by new equations:

$$\delta_{nm} c_m = c_{n0} \quad (6.18)$$

where  $c_{n0}$  is the prescribed value of the concentration at node  $n$ . This is done only when Gaussian elimination is used to solve the matrix equation. A similar procedure as for water flow (described in Section 5.3.4) is applied when the ORTHOMIN method is used. Because of asymmetry of the global matrix  $[G]$ , no additional manipulations are needed in the resulting system of equations as was the case for the water flow solution.

The total material flux,  $Q_n^T$ , through a boundary at node  $n$  consists of the dispersive flux,  $Q_n^D$ , and the convective flux,  $Q_n^A$ :

$$Q_n^T = Q_n^D + Q_n^A \quad (6.19)$$

The dispersive boundary nodal flux is not known explicitly but must be calculated from equation (6.4). Hence, the dispersion flux,  $Q_n^D$ , for node  $n$  can be calculated as

$$Q_n^D = -[\epsilon S_{nm}^{j+1} + (1-\epsilon) S_{nm}^j] c_m^j - \epsilon f_n^{j+1} - (1-\epsilon) f_n^j - Q_{nn}^{j+\epsilon} \frac{c_n^{j+1} - c_n^j}{\Delta t} \quad (6.20)$$

The convective flux is evaluated as

$$Q_n^A = Q_n c_n \quad (6.21)$$

where the fluid flux  $Q_n$  is known from the solution of the water flow equation.

### 6.3.4. Implementation of Third-Type Boundary Conditions

Equation (3.19) is rewritten as follows

$$\theta D_y \frac{\partial c'}{\partial x_j} n_i = q_i n_i (c - c_0) \quad (6.22)$$

When substituted into the last term of (6.3), the boundary integral becomes

$$\sum_e \int_{\Gamma_e} \theta D_{ij} \frac{\partial c'}{\partial x_j} n_i \phi_n d\Gamma = Q_n c_n - Q_n c_{n0} \quad (6.23)$$

The first term on the right-hand side of (6.23) represents the convective flux. This term is incorporated into the coefficient matrix  $[S]$  of (6.4). The last term of (6.23) represents the total material flux, which is added to the known vector  $\{f\}$ .

At nodes where free outflow of water and its dissolved solutes takes place, the exit concentration  $c_0$  is equal to the local (nodal) concentration  $c_n$ . In this case the dispersive flux becomes zero and the total material flux through the boundary is evaluated as

$$Q_n^T = Q_n c_n \quad (6.24)$$

The Cauchy boundary condition for volatile solutes is treated in a similar way. Equation (3.21) is rewritten as follows

$$\theta D_{ij} \frac{\partial c}{\partial x_j} n_i = q_i n_i (c - c_0) - \frac{D_g}{d} (k_g c - g_{atm}) \quad (6.25)$$

Again, when substituted into the last term of (6.3), the boundary integral becomes

$$\sum_e \int_{\Gamma_e} \theta D_{ij} \frac{\partial c'}{\partial x_j} n_i \phi_n d\Gamma = Q_n c_n - Q_n c_{n0} - \frac{D_g \Delta L}{d} (k_g c_n - g_{atm}) \quad (6.26)$$

where  $\Delta L$  is the length of the boundary associated with node  $n$ . The last term of (6.26) representing the gas diffusion flux through the stagnant boundary layer at the soil surface is directly added to the vector  $\{f\}$  in equation (6.9), whereas the term containing  $k_g$  and unknown concentration  $c_n$  is incorporated into the coefficient matrix  $[S]$ . The other terms on the right-hand side of (6.26) are treated in the same way as described above for equation (6.23).

### 6.3.5. Mass Balance Calculations

The total amount of mass in the entire flow domain, or in a preselected subregion, is given by

$$M = \sum_e \int_{\Omega_e} (\theta c + a_v g + \rho s) d\Omega = \sum_e \int_{\Omega_e} [(\theta + a_v k_g + \rho f \frac{k_s c^{\beta-1}}{1 + \eta c^\beta}) c + \rho s^k] d\Omega \quad (6.27)$$

The summation is taken over all elements within the specified region.

The cumulative amounts  $M^0$  and  $M^1$  of solute removed from the flow region by zero- and first-order reactions, respectively, are calculated as follows

$$\begin{aligned} M_1^0 &= - \int_0^t \sum_e \int_{\Omega_e} (\gamma_{w,1} \theta + \gamma_{s,1} \rho + \gamma_{g,1} a_v) d\Omega dt \\ M_k^0 &= - \int_0^t \sum_e \int_{\Omega_e} [(\mu'_{w,k-1} \theta + \mu'_{s,k-1} \rho f \frac{k_{s,k-1} c_{k-1}^{\beta_{k-1}-1}}{1 + \eta_{k-1} c_{k-1}^{\beta_{k-1}}} + \mu'_{g,k-1} a_v k_{g,k-1}) c_{k-1} + \\ &\quad + \mu'_{s,k-1} \rho s_{k-1}^k + \gamma_{w,k} \theta + \gamma_{s,k} \rho + \gamma_{g,k} a_v] d\Omega dt \end{aligned} \quad (6.28)$$

$$\begin{aligned} M^1 &= \int_0^t \sum_e \int_{\Omega_e} \{[(\mu_w + \mu'_w) \theta + (\mu_s + \mu'_s) \rho f \frac{k_s c^{\beta-1}}{1 + \eta c^\beta} + (\mu_g + \mu'_g) a_v k_g] c + \\ &\quad + (\mu_s + \mu'_s) \rho s^k\} d\Omega dt \end{aligned} \quad (6.29)$$

whereas the cumulative amount  $M_r$  of solute taken up by plant roots is given by

$$M_r = \int_0^t \sum_{e_R} \int_{\Omega_e} S c_r d\Omega dt \quad (6.30)$$

where  $e_R$  represents the elements making up the root zone.

Finally, when all boundary material fluxes, decay reactions, and root uptake mass fluxes have been computed, the following mass balance should hold for the flow domain as

a whole:

$$M_t - M_0 = + \int_0^t \sum_{n_r} Q_n^T dt - M^0 - M^1 - M_r \quad (6.31)$$

where  $M_t$  and  $M_0$  are the amounts of solute in the flow region at times  $t$  and zero, respectively, as calculated with (6.27), and  $n_r$  represents nodes located along the boundary of the flow domain or at internal sinks and/or sources. The difference between the left- and right-hand sides of (6.31) represents the absolute error,  $\epsilon_a^c$ , in the solute mass balance. Similarly as for water flow, the accuracy of the numerical solution for solute transport is evaluated by using the relative error,  $\epsilon_r^c$  [%], in the solute mass balance as follows

$$\epsilon_r^c = \frac{100 |\epsilon_a^c|}{\max \left[ \sum_e |M_t^e - M_0^e|, |M^0| + |M^1| + |M_r| + \int_0^t \sum_{n_r} |Q_n^T| dt \right]} \quad (6.32)$$

where  $M_0^e$  and  $M_t^e$  are the amounts of solute in element  $e$  at times 0 and  $t$ , respectively. Note again that CHAIN\_2D does not relate the absolute error to the total amount of mass in the flow region. Instead, the program uses as a reference the maximum value of (1) the absolute change in element concentrations as summed over all elements, and (2) the sum of the absolute values of all cumulative solute fluxes across the flow boundaries including those resulting from sources and sinks in the flow domain.

The total amount of heat energy in the entire flow domain, or in a preselected subregion, is given by

$$W = \sum_e \int_{\Omega_e} (C_n \theta_n + C_o \theta_o + C_w \theta + C_g a_v) T^A d\Omega \quad (6.33)$$

where  $T^A$  is the absolute temperature [K]. The summation is taken over all elements within the specified region.

### 6.3.6. Oscillatory Behavior

Numerical solutions of the transport equation often exhibit oscillatory behavior and/or excessive numerical dispersion near relatively sharp concentration fronts. These problems can be especially serious for convection-dominated transport characterized by small dispersivities. One way to partially circumvent numerical oscillations is to use upstream weighing as discussed in Section 6.3.2. Undesired oscillations can often be prevented also by selecting an appropriate combination of space and time discretizations. Two dimensionless numbers may be used to characterize the space and time discretizations. One of these is the grid Peclet number,  $Pe_i^e$ , which defines the predominant type of the solute transport (notably the ratio of the convective and dispersive transport terms) in relation to coarseness of the finite element grid:

$$Pe_i^e = \frac{q_i \Delta x_i}{\theta D_{ii}} \quad (6.34)$$

where  $\Delta x_i$  is the characteristic length of a finite element. The Peclet number increases when the convective part of the transport equation dominates the dispersive part, i.e., when a relatively steep concentration front is present. To achieve acceptable numerical results, the spatial discretization must be kept relatively fine to maintain a low Peclet number. Numerical oscillation can be virtually eliminated when the local Peclet numbers do not exceed about 5. However, acceptably small oscillations may be obtained with local Peclet numbers as high as 10 [Huyakorn and Pinder, 1983]. Undesired oscillation for higher Peclet numbers can be effectively eliminated by using upstream weighing (see Section 6.3.2).

A second dimensionless number which characterizes the relative extent of numerical oscillations is the Courant number,  $Cr_i^e$ . The Courant number is associated with the time discretization as follows

$$Cr_i^e = \frac{q_i \Delta t}{\theta R \Delta x_i} \quad (6.35)$$

Three stabilizing options are used in CHAIN\_2D to avoid oscillations in the

numerical solution of the solute transport equation [*Šiminek and Suarez, 1993b*]. One option is upstream weighing (see Section 6.3.2), which effectively eliminates undesired oscillations at relatively high Peclet numbers. A second option for minimizing or eliminating numerical oscillations uses the criterion developed by *Perrochet and Berod* [1993]

$$Pe \cdot Cr \leq \omega_s \quad (= 2) \quad (6.36)$$

where  $\omega_s$  is the performance index [-]. This criterion indicates that convection-dominated transport problems having large  $Pe$  numbers can be safely simulated provided  $Cr$  is reduced according to (6.36) [*Perrochet and Berod, 1993*]. When small oscillations in the solution can be tolerated,  $\omega_s$  can be increased to about 5 or 10.

A third stabilization option implemented in CHAIN\_2D also utilizes criterion (6.36). However, instead of decreasing  $Cr$  to satisfy equation (6.36), this option introduces artificial dispersion to decrease the Peclet number. The amount of additional longitudinal dispersion,  $\bar{D}_L$  [L], is given by [*Perrochet and Berod, 1993*]

$$\bar{D}_L = \frac{|q| \Delta t}{R \theta \omega_s} - D_L - \frac{\theta D_w \tau}{|q|} \quad (6.37)$$

The maximum permitted time step is calculated for all three options, as well as with the additional requirement that the Courant number must remain less than or equal to 1. The time step calculated in this way is subsequently used as one of the time discretization rules (rule No. B) discussed in section 5.3.3.

## 7. PROBLEM DEFINITION

### 7.1. *Construction of Finite Element Mesh*

The finite element mesh is constructed by dividing the flow region into quadrilateral and/or triangular elements whose shapes are defined by the coordinates of the nodes that form the element corners. The program automatically subdivides the quadrilaterals into triangles which are then treated as subelements.

Transverse lines [Neuman, 1974] formed by element boundaries must transect the mesh along the general direction of its shortest dimension. These transverse lines should be continuous and non-intersecting, but need not be straight. The nodes are numbered sequentially from 1 to  $NumNP$  (total number of nodes) by proceeding along each transverse line in the same direction. Elements are numbered in a similar manner. The maximum number of nodes on any transverse line,  $IJ$ , is used to determine the effective size of the finite element matrix (i.e., its band width). To minimize memory and time requirements,  $IJ$  should be kept as small as possible. The above rules for defining the finite element mesh apply only when Gaussian elimination is used to solve the matrix equations. Iterative methods (such as the conjugate gradient and ORTHOMIN methods) are not so restrictive since only non-zero entries in the coefficient matrix are stored in memory, and since the computational efficiency is less dependent upon the bandwidth of the matrix as compared to direct equation solvers.

The finite element dimensions must be adjusted to a particular problem. They should be made relatively small in directions where large hydraulic gradients are expected. Region with sharp gradients are usually located in the vicinity of the internal sources or sinks, or close to the soil surface where highly variable meteorological factors can cause fast changes in pressure head. Hence, we recommend to normally use relatively small elements at and near the soil surface. The size of elements can gradually increase with depth to reflect the generally much slower changes in pressure heads at deeper depths. The element dimensions should also depend upon the soil hydraulic properties. For example, coarse-

textured soils having relatively high  $n$ -values and small  $\alpha$ -values (see Eqs. (2.11) and (2.18)) generally require a finer discretization than fine-textured soils. We also recommend using elements having approximately equal sizes to decrease numerical errors. For axisymmetric three-dimensional flow systems, the vertical axis must coincide with, or be to the left of, the left boundary of the mesh. No special restrictions are necessary to facilitate the soil root zone.

## 7.2. Coding of Soil Types and Subregions

*Soil Types* - An integer code beginning with 1 and ending with  $NMat$  (the total number of soil materials) is assigned to each soil type in the flow region. The appropriate material code is subsequently assigned to each nodal point  $n$  of the finite element mesh.

Interior material interfaces do not coincide with element boundaries. When different material numbers are assigned to the corner nodes of a certain element, material properties of this element will be averaged automatically by the finite element algorithm. This procedure will somewhat smooth soil interfaces.

A set of soil hydraulic parameters and solute transport characteristics must be specified for each soil material. Also, the user must define for each element the principal components of the conductivity anisotropy tensor, as well as the angle between the local and global coordinate systems.

As explained in Section 2.3, one additional way of changing the unsaturated soil hydraulic properties in the flow domain is to introduce scaling factors associated with the water content, the pressure head and the hydraulic conductivity. The scaling factors are assigned to each nodal point  $n$  in the flow region.

*Subregions* - Water and solute mass balances are computed separately for each specified subregion. The subregions may or may not coincide with the material regions. Subregions are characterized by an integer code which runs from 1 to  $NLay$  (the total number of subregions). A subregion code is assigned to each element in the flow domain.

### 7.3. Coding of Boundary Conditions

Flow boundary conditions were programmed in a fairly similar way as done in the UNSAT1 and UNSAT2 models of *Neuman* [1972] and *Neuman et al.* [1974], and in the SWMS\_2D code [*Šiminek et al.*, 1992]. Unit vertical hydraulic gradient boundary conditions simulating free drainage are implemented in CHAIN\_2D, in addition to the boundary conditions used in SWMS\_2D. A boundary code,  $Kode(n)$ , must be assigned to each node,  $n$ . If node  $n$  is to have a prescribed pressure head during a time step (Dirichlet boundary condition),  $Kode(n)$  must be set positive during that time step. If the volumetric flux of water entering or leaving the system at node  $n$  is prescribed during a time step (Neumann boundary condition),  $Kode(n)$  must be negative or zero.

*Constant Boundary Conditions* - The values of constant boundary conditions for a particular node,  $n$ , are given by the initial values of the pressure head,  $h(n)$ , in case of Dirichlet boundary conditions, or by the initial values of the recharge/discharge flux,  $Q(n)$ , in case of Neumann boundary conditions. Table 7.1 summarizes the use of the variables  $Kode(n)$ ,  $Q(n)$  and  $h(n)$  for various types of nodes.

Table 7.1. Initial settings of  $Kode(n)$ ,  $Q(n)$ , and  $h(n)$  for constant boundary conditions.

Node Type	$Kode(n)$	$Q(n)$	$h(n)$
Internal; not sink/source	0	0.0	Initial Value
Internal; sink/source (Dirichlet condition)	1	0.0	Prescribed
Internal; sink/source (Neumann condition)	-1	Prescribed	Initial Value
Impermeable Boundary	0	0.0	Initial Value
Specified Head Boundary	1 <sup>†</sup>	0.0	Prescribed
Specified Flux Boundary	-1 <sup>‡</sup>	Prescribed	Initial Value

<sup>†</sup> 5 and/or 6 may also be used

<sup>‡</sup> -5 and/or -6 may also be used

*Variable Boundary Conditions* - Three types of variable boundary conditions can be imposed:

1. Atmospheric boundary conditions for which  $Kode(n) = \pm 4$ ,
2. Variable pressure head boundary conditions for which  $Kode(n) = +3$ , and
3. Variable flux boundary conditions for which  $Kode(n) = -3$ .

These conditions can be specified along any part of the boundary. It is not possible to specify more than one time-dependent boundary condition for each type. Initial settings of the variables  $Kode(n)$ ,  $Q(n)$  and  $h(n)$  for the time-dependent boundary conditions are given in Table 7.2.

Table 7.2. Initial settings of  $Kode(n)$ ,  $Q(n)$ , and  $h(n)$  for variable boundary conditions.

Node Type	$Kode(n)$	$Q(n)$	$h(n)$
Atmospheric Boundary	-4	0.0	Initial Value
Variable Head Boundary	+3	0.0	Initial Value
Variable Flux Boundary	-3	0.0	Initial Value

Atmospheric boundary conditions are implemented when  $Kode(n) = \pm 4$ , in which case time-dependent input data for the precipitation,  $Prec$ , and evaporation,  $rSoil$ , rates must be specified in the input file ATMOSPH.IN. The potential fluid flux across the soil surface is determined by  $rAtm = rSoil - Prec$ . The actual surface flux is calculated internally by the program. Two limiting values of surface pressure head must also be provided:  $hCritS$  which specifies the maximum allowed pressure head at the soil surface (usually 0.0), and  $hCritA$  which specifies the minimum allowed surface pressure head (defined from equilibrium conditions between soil water and atmospheric vapor). The program automatically switches the value of  $Kode(n)$  from -4 to +4 if one of these two limiting points is reached. Table 7.3 summarizes the use of the variables  $rAtm$ ,  $hCritS$  and  $hCritA$  during program execution.  $Width(n)$  in the table denotes the length of the boundary segment associated with node  $n$ .

Table 7.3. Definition of the variables  $Kode(n)$ ,  $Q(n)$  and  $h(n)$  when an atmospheric boundary condition is applied.

$Kode(n)$	$Q(n)$	$h(n)$	Event
-4	$-Width(n)*rAtm$	Unknown	$rAtm = rSoil-Prec$
+4	Unknown	$hCritA$	Evaporation capacity is exceeded
+4	Unknown	$hCritS$	Infiltration capacity is exceeded

Variable head and flux boundary conditions along a certain part of the boundary are implemented when  $Kode(n) = +3$  and -3, respectively. In that case, the input file ATMOSPH.IN must contain the prescribed time-dependent values of the pressure head,  $ht$ , or the flux,  $rt$ , imposed along the boundary. The values of  $ht$  or  $rt$  are assigned to particular nodes at specified times according to rules given in Table 7.4.

Table 7.4. Definition of the variables  $Kode(n)$ ,  $Q(n)$  and  $h(n)$  when variable head or flux boundary conditions are applied.

Node Type	$Kode(n)$	$Q(n)$	$h(n)$
Variable Head Boundary	+3	Unknown	$ht$
Variable Flux Boundary	-3	$-Width(n)*rt$	Unknown

*Water Uptake by Plant Roots* - The program calculates the rate at which plants extract water from the soil root zone by evaluating the term  $D$  (equation (5.9)) in the finite element formulation. The code requires that  $Kode(n)$  be set equal to 0 or negative for all nodes in the root zone. Values of the potential transpiration rate,  $rRoot$ , must be specified at preselected times in the input file ATMOSPH.IN. Actual transpiration rates are calculated internally by the program as discussed in Section 2.2. The root uptake parameters are taken

from an input file SELECTOR.IN. Values of the function  $Beta(n)$ , which describes the potential water uptake distribution over the root zone (equation (2.5)), must be specified for each node in the flow domain (see the description of input Block I in Table 9.9 of Section 9). All parts of the flow region where  $Beta(n) > 0$  are treated as the soil root zone.

*Deep Drainage from the Soil Profile* - Vertical drainage,  $q(h)$ , across the lower boundary of the soil profile is sometimes approximated by a flux which depends on the position of groundwater level (e.g., Hopmans and Stricker, 1989). If available, such a relationship can be implemented in the form of a variable flux boundary condition for which  $Kode(n) = -3$ . This boundary condition is implemented in CHAIN\_2D by setting the logical variable  $qGWL$  in the input file ATMOSPH.IN equal to ".true.". The discharge rate  $Q(n)$  assigned to node  $n$  is determined by the program as  $Q(n) = -Width(n)*q(h)$  where  $h$  is the local value of the pressure head, and  $q(h)$  is given by

$$q(h) = -A_{qh} \exp(B_{qh} |h - GWL0L|) \quad (7.1)$$

where  $A_{qh}$  and  $B_{qh}$  are empirical parameters which must be specified in the input file ATMOSPH.IN, together with  $GWL0L$  which represents the reference position of the groundwater level (usually set equal to the z-coordinate of the soil surface).

*Free Drainage* - Unit vertical hydraulic gradient boundary conditions can be implemented in the form of a variable flux boundary condition for which  $Kode(n) = -3$ . This boundary condition is implemented in CHAIN\_2D by setting the logical variable *FreeD* in the input file SELECTOR.IN equal to ".true.". The program determines the discharge rate  $Q(n)$  assigned to node  $n$  as  $Q(n) = -Width(n)*K(h)$ , where  $h$  is the local value of the pressure head, and  $K(h)$  is the hydraulic conductivity corresponding to this pressure head.

*Seepage Faces* - The initial settings of the variables  $Kode(n)$ ,  $Q(n)$  and  $h(n)$  for nodes along a seepage face are summarized in Table 7.5. All potential seepage faces must be identified before starting the numerical simulation. This is done by providing a list of nodes

along each potential seepage face (see input Block E as defined in Table 9.5 of Section 9).

Table 7.5. Initial setting of  $Kode(n)$ ,  $Q(n)$ , and  $h(n)$  for seepage faces.

Node Type	$Kode(n)$	$Q(n)$	$h(n)$
Seepage Face (initially saturated)	+2	0.0	0.0
Seepage Face (initially unsaturated)	-2	0.0	Initial Value

*Drains* - Table 7.6 summarizes the initial settings of the variables  $Kode(n)$ ,  $Q(n)$  and  $h(n)$  for nodes representing drains. All drains must be identified before starting the numerical simulation. This is done by providing a list of nodes representing drains, together with a list of elements around each drain whose hydraulic conductivities are to be adjusted according to discussion in Section 5.3.7 (see also input Block F as defined in Table 9.6 of Section 9).

Table 7.6. Initial setting of  $Kode(n)$ ,  $Q(n)$ , and  $h(n)$  for drains.

Node Type	$Kode(n)$	$Q(n)$	$h(n)$
Drain (initially saturated)	+5	0.0	0.0
Drain (initially unsaturated)	-5	0.0	Initial Value

*Solute and Heat Transport Boundary Conditions* - The original version 1.1. of SWMS\_2D [Šimůnek *et al.*, 1992] assumed a strict relationship between the boundary conditions for water flow and solute transport. A first-type boundary condition for water flow forced the boundary condition for solute transport also to be of the first-type.

Similarly, a second-type boundary condition for water flow induced a second- or third-type boundary condition for solute transport depending upon direction of the water flux. These strict relationships between the boundary conditions for water flow and solute transport have been abandoned in CHAIN\_2D. Selection of the type of boundary condition for the solute transport is now much more independent of the boundary condition implemented for water flow. The type of boundary condition to be invoked for solute or heat transport is specified by the input variable *KodCB* or *KodTB*, respectively. A positive sign of this variable means that a first-type boundary condition will be used. When *KodCB* (*KodTB*) is negative, CHAIN\_2D selects a third-type boundary condition when the calculated water flux is directed into the region, or a second-type boundary condition when the water flux is zero or directed out of the region. One exception to these rules occurs for atmospheric boundary conditions when  $Kode(n) = \pm 4$  and  $Q(n) < 0$ . CHAIN\_2D assumes that solutes can leave the flow region across atmospheric boundaries only by gas diffusion. The solute flux in this situation becomes zero, i.e.,  $c_0 = 0$  in equation (6.22). Cauchy and Neumann boundary conditions are automatically applied to internal sinks/sources depending upon the direction of water flow. The dependence or independence of the solute and heat boundary conditions on time or the system is still defined through the variable *Kode(n)* as discussed above.

Although CHAIN\_2D can implement first-type boundary conditions, we recommend users to invoke third-type conditions where possible. This is because third-type conditions, in general, are physically more realistic and preserve solute mass in the simulated system (e.g., *van Genuchten and Parker* [1984]; *Leij et al.* [1991]).

#### 7.4. Program Memory Requirements

One single parameter statement is used at the beginning of the code to define the problem dimensions. All major arrays in the program are adjusted automatically according to these dimensions. This feature makes it possible to change the dimensions of the problem to be simulated without having to recompile all program subroutines. Different problems can be investigated by changing the dimensions in the parameter statement at the

beginning of the main program, and subsequently linking all previously compiled subroutines with the main program when creating an executable file. Table 7.7 lists the array dimensions which must be defined in the parameter statement.

Table 7.7. List of array dimensions in CHAIN\_2D.

Dimension	Description
<i>NumNPD</i>	Maximum number of nodes in finite element mesh
<i>NumElD</i>	Maximum number of elements in finite element mesh
<i>MBandD</i>	Maximum dimension of the bandwidth of matrix $A$ when Gaussian elimination is used. Maximum number of nodes adjacent to a particular node, including itself, when iterative matrix solvers are used.
<i>NumBPD</i>	Maximum number of boundary nodes for which $Kode(n) \neq 0$
<i>NSeepD</i>	Maximum number of seepage faces
<i>NumSPD</i>	Maximum number of nodes along a seepage face
<i>NDrD</i>	Maximum number of drains
<i>NElDrD</i>	Maximum number of elements surrounding a drain
<i>NMatD</i>	Maximum number of materials
<i>NTabD</i>	Maximum number of items in the table of hydraulic properties generated by the program for each soil material
<i>NumKD</i>	Maximum number of available code number values (equals 6 in present version)
<i>NSD</i>	Maximum number of solutes (equals 6 in present version)
<i>NObsD</i>	Maximum number of observation nodes for which the pressure head, the water content, temperature and concentration are printed at each time level
<i>MNorth</i>	Maximum number of orthogonalizations performed when iterative solvers are used

## 7.5. Matrix Equation Solvers

Discretization of the governing partial differential equations for water flow (2.1), solute transport (3.11) and heat movement (4.1) leads to the system of linear equations

$$[\mathcal{A}] \{x\} = \{b\} \quad (7.2)$$

in which matrix  $[\mathcal{A}]$  is symmetric for water flow and asymmetric for solute and heat transport.

The original version of SWMS\_2D [*Šimunek et al.*, 1992] uses Gaussian elimination to solve both systems of linear algebraic equations. The invoked solvers took advantage of the banded nature of the coefficient matrices and, in the case of water flow, of the symmetric properties of the matrix. Such direct solution methods have several disadvantages as compared to iterative methods. Direct methods require a fixed number of operations (depending upon the size of the matrix) which increases approximately by the square of the number of nodes [*Mendoza et al.*, 1991]. Iterative methods, on the other hand, require a variable number of repeated steps which increases at a much smaller rate (about 1.5) with the size of a problem [*Mendoza et al.*, 1991]. A similar reduction also holds for the memory requirement since iterative methods do not require one to store non-zero matrix elements. Memory requirements, therefore, increase at a much smaller rate with the size of the problem when iterative solvers are used [*Mendoza et al.*, 1991]. Round-off errors also represent less of a problem for iterative methods as compared to direct methods. This is because round-off errors in iterative methods are self-correcting [*Letniowski*, 1989]. Finally, for time-dependent problems, a reasonable approximation of the solution (i.e., the solution at the previous time step) exists for iterative methods, but not for direct methods [*Letniowski*, 1989]. In general, direct methods are more appropriate for relatively small problems, while iterative methods are more suitable for larger problems.

Many iterative methods have been used in the past for handling large sparse matrix equations. These methods include Jacobi, Gauss-Seidel, alternating direction implicit (ADI), block successive over-relaxation (BSSOR), successive line over-relaxation (SLOR), and

strongly implicit procedures (SIP), among others [Letniowski, 1989]. More powerful preconditioned accelerated iterative methods, such as the preconditioned conjugate gradient method (PCG) [Behie and Vinsome, 1982], were introduced more recently. Sudicky and Huyakorn [1991] gave three advantages of the PCG procedure as compared to other iterative methods: PCG can be readily modified for finite element methods with irregular grids, the method does not require iterative parameters, and PCG usually outperforms its iterative counterparts for situations involving relatively stiff matrix conditions.

**CHAIN\_2D** implements both direct and iterative methods for solving the system of linear algebraic equations given by (7.2). Depending upon the size of matrix  $[A]$ , we use either direct Gaussian elimination or the preconditioned conjugate gradient method [Mendoza et al., 1991] for water flow and the ORTHOMIN (preconditioned conjugate gradient squared) procedure [Mendoza et al., 1991] for solute transport. Gaussian elimination is used if either the bandwidth of matrix  $[A]$  is smaller than 20, or the total number of nodes is smaller than 500. The iterative methods used in **CHAIN\_2D** were adopted from the ORTHOFEM software package of Mendoza et al. [1991].

The preconditioned conjugate gradient and ORTHOMIN methods consist of two essential parts: initial preconditioning, and the iterative solution with either conjugate gradient or ORTHOMIN acceleration [Mendoza et al., 1991]. Incomplete lower-upper (ILU) preconditioning is used in ORTHOFEM when matrix  $[A]$  is factorized into lower and upper triangular matrices by partial Gaussian elimination. The preconditioned matrix is subsequently repeatedly inverted using updated solution estimates to provide a new approximation of the solution. The orthogonalization-minimization acceleration technique is used to update the solution estimate. This technique insures that the search direction for each new solution is orthogonal to the previous approximate solution, and that either the norm of the residuals (for conjugate gradient acceleration [Meijerink and van der Vorst, 1981]) or the sum of the squares of the residuals (for ORTHOMIN [Behie and Vinsome, 1982]) is minimized. More details about the two methods is given in the user's guide of ORTHOFEM [Mendoza et al., 1991] or in Letniowski [1989]. Letniowski [1989] also gives a comprehensive review of accelerated iterative methods, as well as of the preconditional

**techniques.**

## 8. EXAMPLE PROBLEMS

Seven example problems are presented in this section. The first three examples are identical to those presented with the first version of SWMS\_2D [*Šiminek et al.*, 1992]. The other four examples in this section were included mainly for mathematical verification purposes, and for demonstrating new features of CHAIN\_2D, i.e., non-equilibrium and nonlinear adsorption, sequential first-order decay reactions, solute diffusion in the gas phase, and heat transport.

Examples 1 and 2 provide comparisons of the water flow part of CHAIN\_2D code with results from both the UNSAT2 code of *Neuman* [1974] and the SWATRE code of *Belmans et al.* [1983]. Example 3 serves to verify the accuracy of the solute transport part of CHAIN\_2D by comparing numerical results against those obtained with a two-dimensional analytical solution during steady-state groundwater flow. The results obtained with CHAIN\_2D code for these three examples are identical to the results obtained with SWMS\_2D. Example 4 serves to verify the accuracy of CHAIN\_2D by comparing numerical results for a problem with three solutes involved in a sequential first-order decay reaction against results obtained with an analytical solution during one-dimensional steady-state water flow [*van Genuchten*, 1985]. Example 5 considers one-dimensional transport of a solute undergoing nonlinear cation adsorption. Numerical results are compared with experimental data and previous numerical solutions obtained with the MONOC code of *Selim et al.* [1987] and the HYDRUS code of *Kool and van Genuchten* [1991]. Example 6 serves to verify the accuracy of CHAIN\_2D in describing non-equilibrium adsorption by comparing numerical results against experimental data and previous numerical predictions during one-dimensional steady-state water flow [*van Genuchten*, 1981]. Example 7 demonstrates numerical results for a field infiltration experiment involving a two-layered axisymmetric three-dimensional flow domain. The infiltrating water was assumed to have a higher temperature than the soil, and to contain an organic compound (parent pesticide) which in the soil profile degraded into two sequential daughter products. The selected input and output files of the examples are listed at the end of Sections 9 and 10, respectively.

### 8.1. Example 1 - Column Infiltration Test

This example simulates a one-dimensional laboratory infiltration experiment discussed by *Skaggs et al.* [1970]. The example was used later by *Davis and Neuman* [1983] as a test problem for the UNSAT2 code. Hence, the example provides a means of comparing results obtained with the CHAIN\_2D and UNSAT2 codes.

Figure 8.1 gives a graphical representation of the soil column and the finite element mesh used for the numerical simulations. The soil water retention and relative hydraulic conductivity functions of the sandy soil are presented in Figure 8.2. The sand was assumed to be at an initial pressure head of -150 cm. The soil hydraulic properties were assumed to

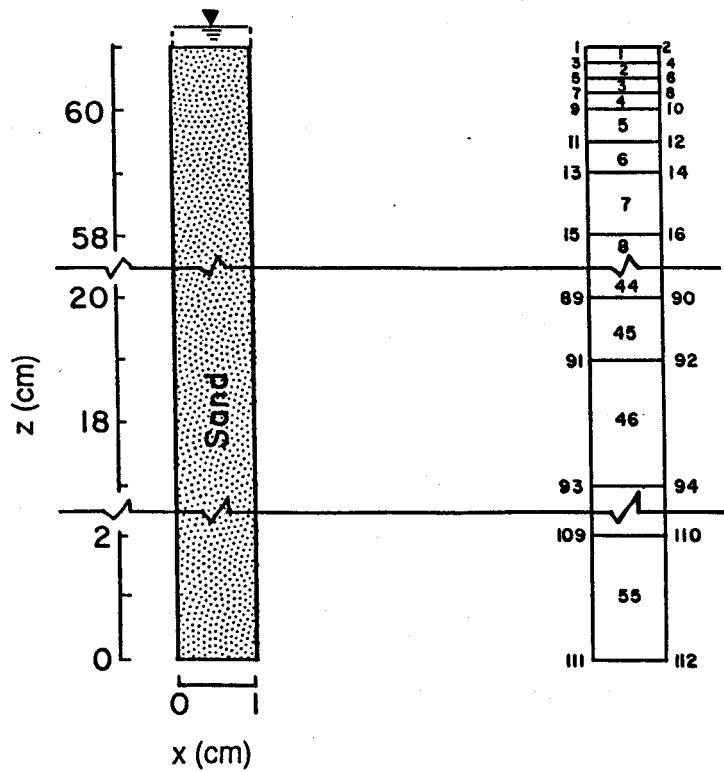


Fig. 8.1. Flow system and finite element mesh for example 1.

be homogenous and isotropic with a saturated hydraulic conductivity of 0.0433 cm/min. The column was subjected to ponded infiltration (a Dirichlet boundary condition) at the soil surface, resulting in one-dimensional vertical water flow. The open bottom boundary of the soil column was simulated by implementing a no-flow boundary condition during unsaturated flow ( $h < 0$ ), and a seepage face with  $h = 0$  when the bottom boundary becomes saturated (this last condition was not reached during the simulation). The impervious sides of the column were simulated by imposing no-flow boundary conditions.

The simulation was carried out for 90 min, which corresponds to the total time duration of the experiment. Figure 8.3 shows the calculated instantaneous ( $q_0$ ) and cumulative ( $I_0$ ) infiltration rates simulated with CHAIN\_2D. The calculated results agree closely with those obtained by *Davis and Neuman* [1983] using their UNSAT2 code.

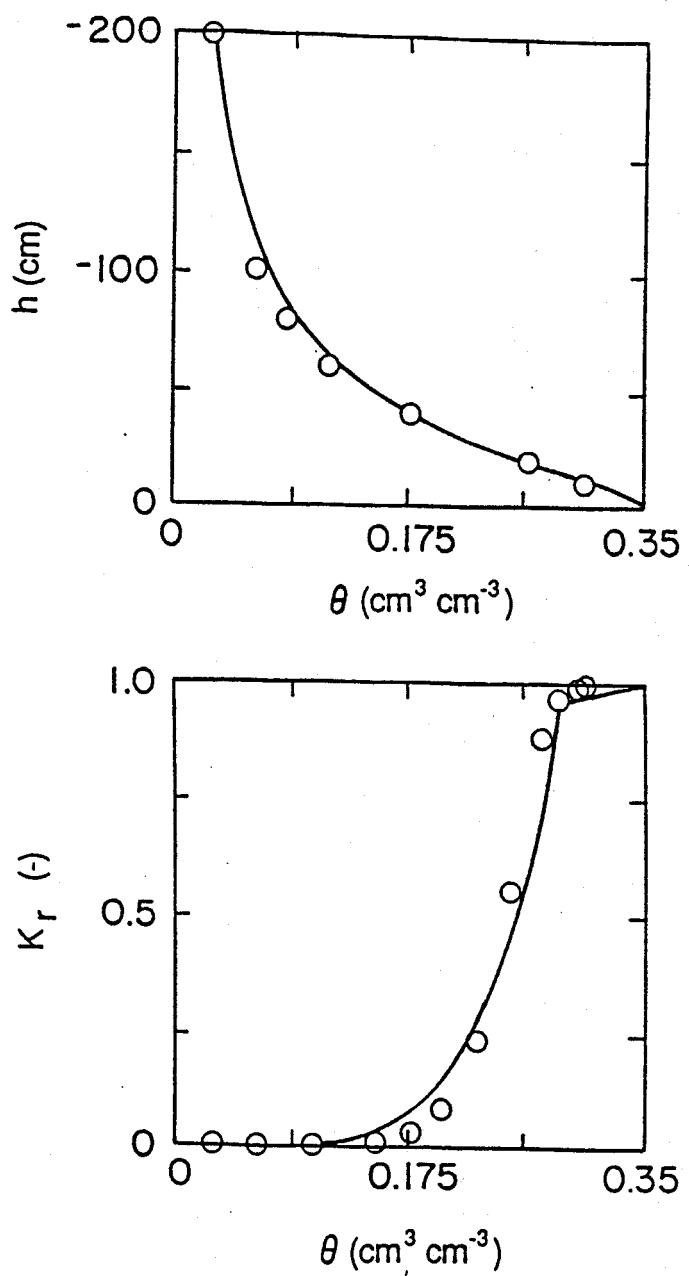


Fig. 8.2. Retention and relative hydraulic conductivity functions for example 1. The open circles are UNSAT2 input data [Davis and Neuman, 1983].

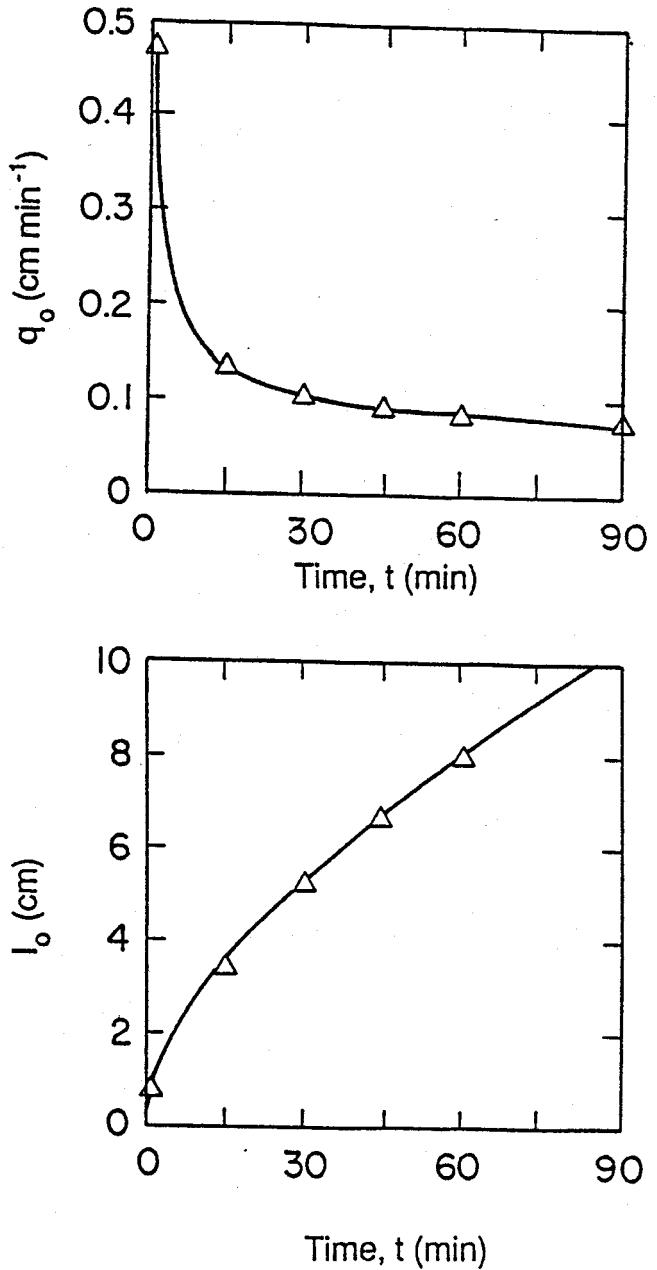


Fig. 8.3. Instantaneous,  $q_0$ , and cumulative,  $I_0$ , infiltration rates simulated with the CHAIN\_2D (solid lines) and UNSAT2 (triangles) codes for example 1.

## 8.2. Example 2 - Water Flow in a Field Soil Profile Under Grass

This example considers one-dimensional water flow in a field profile of the Hupselse Beek watershed in the Netherlands. Atmospheric data and observed ground water levels provided the required boundary conditions for the numerical model. Calculations were performed for the period of April 1 to September 30 of the relatively dry year 1982. Simulation results obtained with CHAIN\_2D will be compared with those generated with the SWATRE computer program [Feddes *et al.*, 1978, Belmans *et al.*, 1983].

The soil profile (Fig. 8.4) consisted of two layers: a 40-cm thick A-horizon, and a B/C-horizon which extended to a depth of about 300 cm. The depth of the root zone was 30 cm. The mean scaled hydraulic functions of the two soil layers in the Hupselse Beek area [Císlarová, 1987; Hopmans and Stricker, 1989] are presented in Figure 8.5.

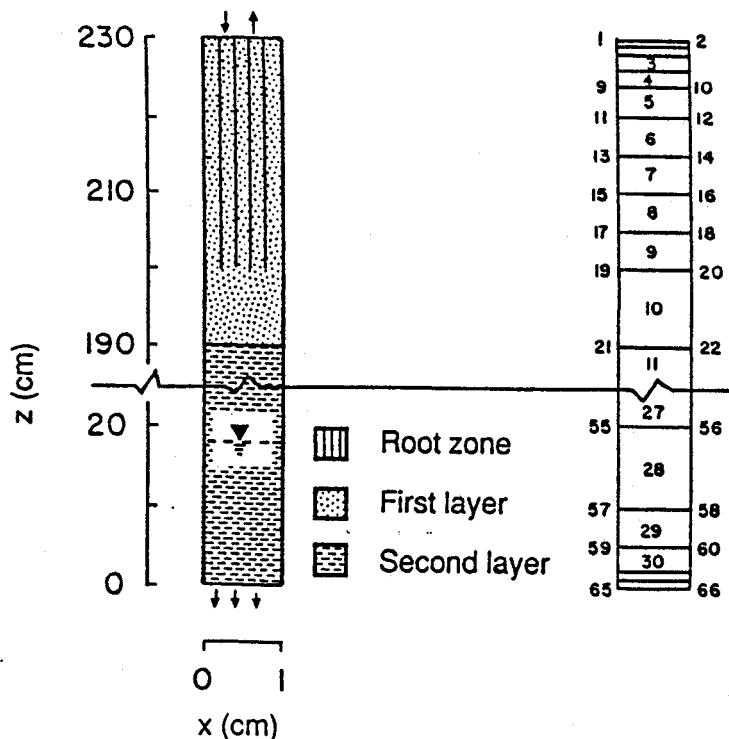


Fig. 8.4. Flow system and finite element mesh for example 2.

The soil surface boundary conditions involved actual precipitation and potential transpiration rates for a grass cover. The surface fluxes were incorporated by using average daily rates distributed uniformly over each day. The bottom boundary condition consisted of a prescribed drainage flux - groundwater level relationship,  $q(h)$ , as given by equation (7.1). The groundwater level was initially set at 55 cm below the soil surface. The initial moisture profile was taken to be in equilibrium with the initial ground water level.

Figure 8.6 presents input values of the precipitation and potential transpiration rates. Calculated cumulative transpiration and cumulative drainage amounts as obtained with the CHAIN\_2D and SWATRE codes are shown in Figure 8.7. The pressure head at the soil surface and the arithmetic mean pressure head of the root zone during the simulated season are presented in Figure 8.8. Finally, Figure 8.9 shows variations in the calculated groundwater level with time.

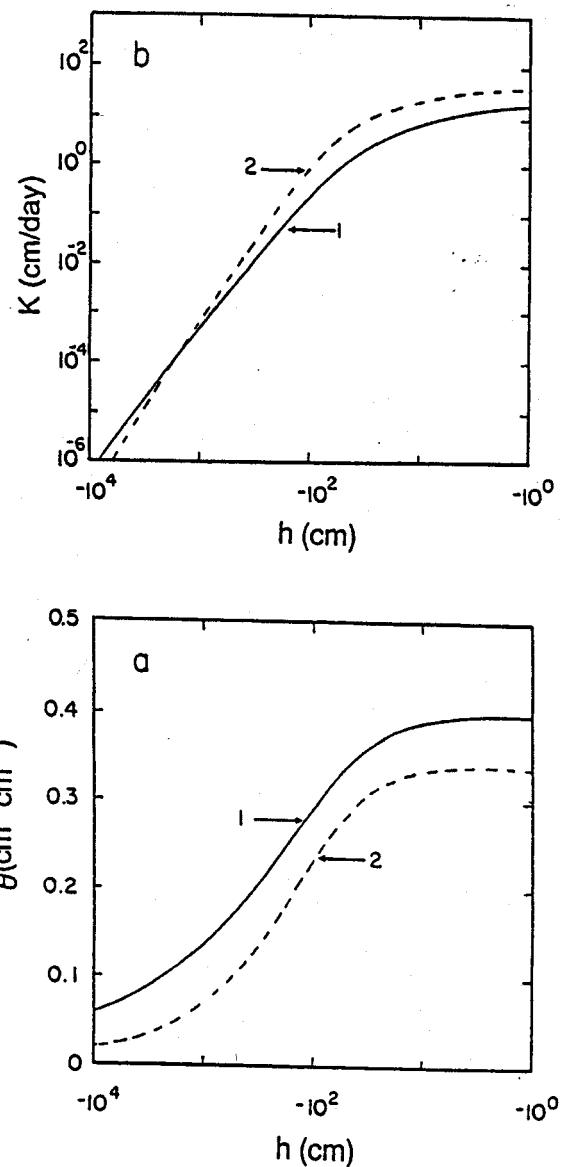


Fig. 8.5. Unsaturated hydraulic properties of the first and second soil layers for example 2.

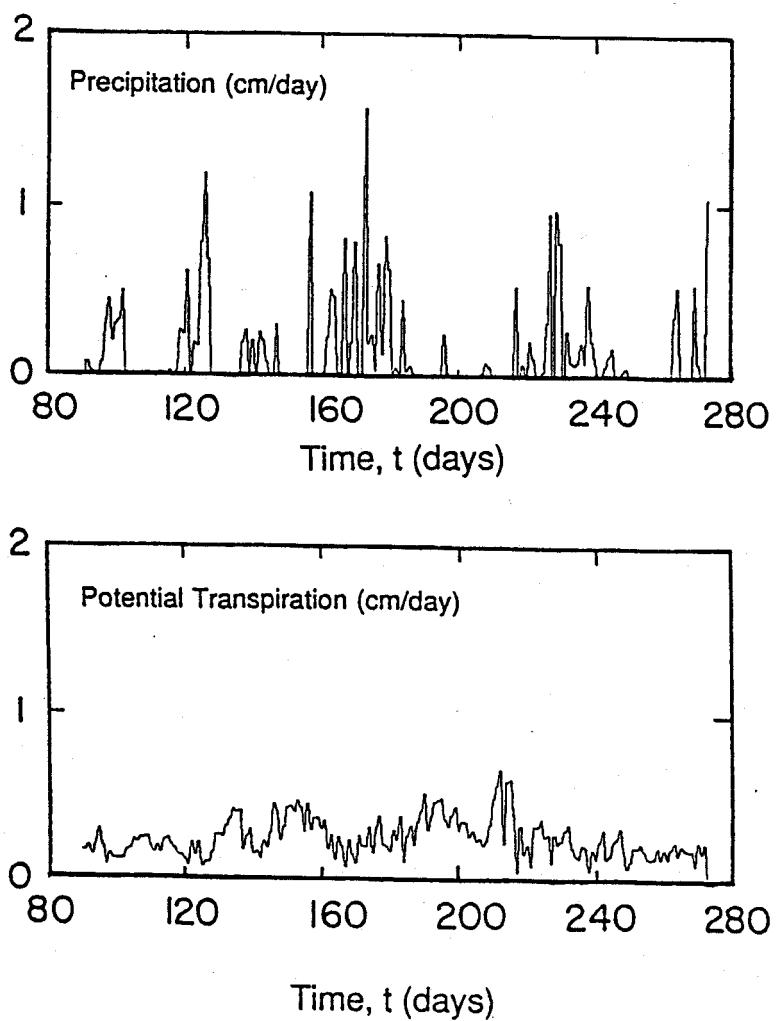


Fig. 8.6. Precipitation and potential transpiration rates for example 2.

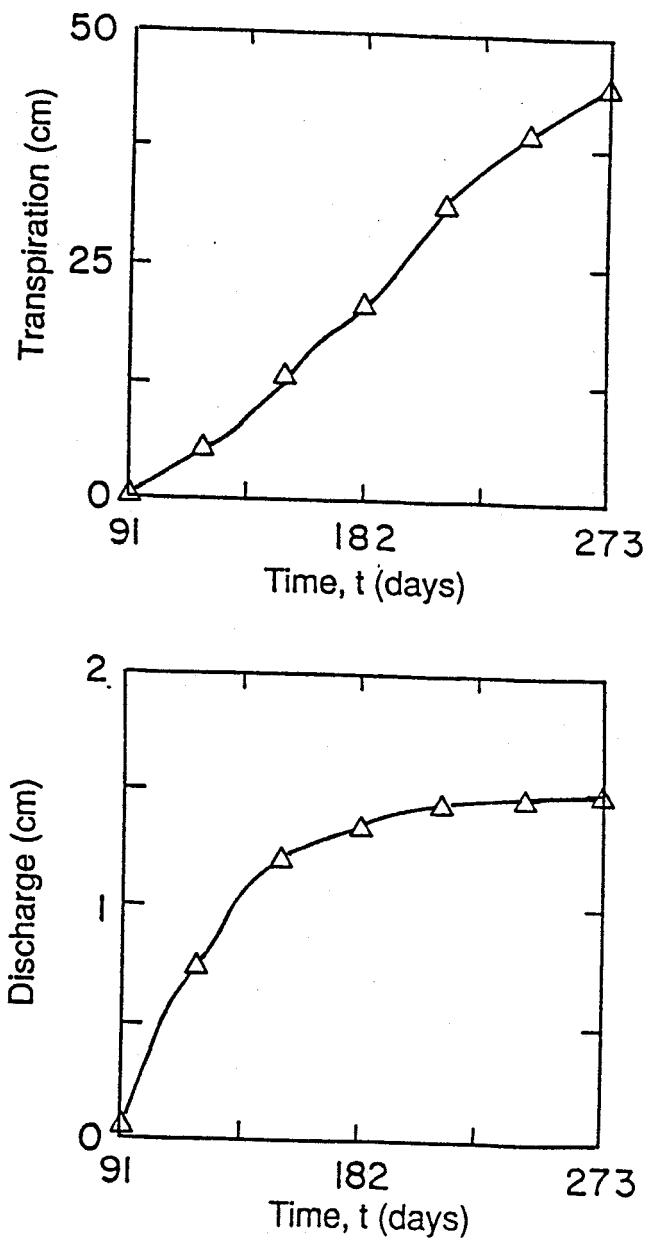


Fig. 8.7. Cumulative values for the actual transpiration and bottom discharge rates for example 2 as simulated with CHAIN\_2D (solid line) and SWATRE (triangles).

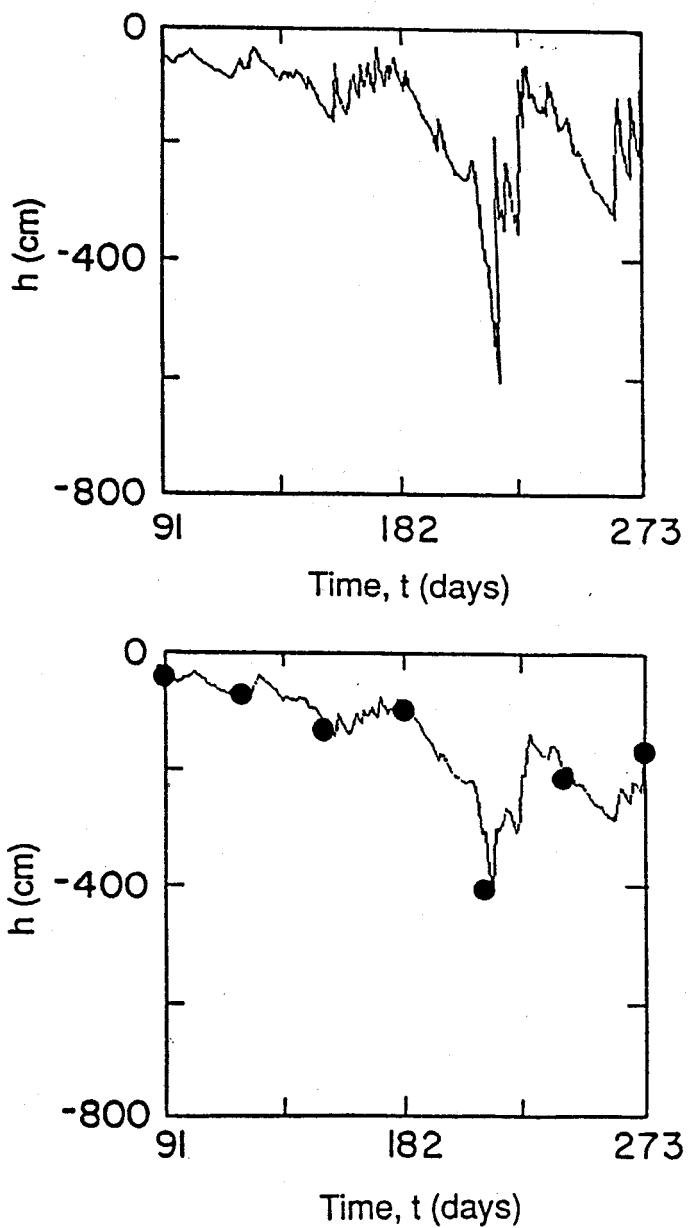


Fig. 8.8. Pressure head at the soil surface and mean pressure head of the root zone for example 2 as simulated with CHAIN\_2D (solid lines) and SWATRE (solid circles).

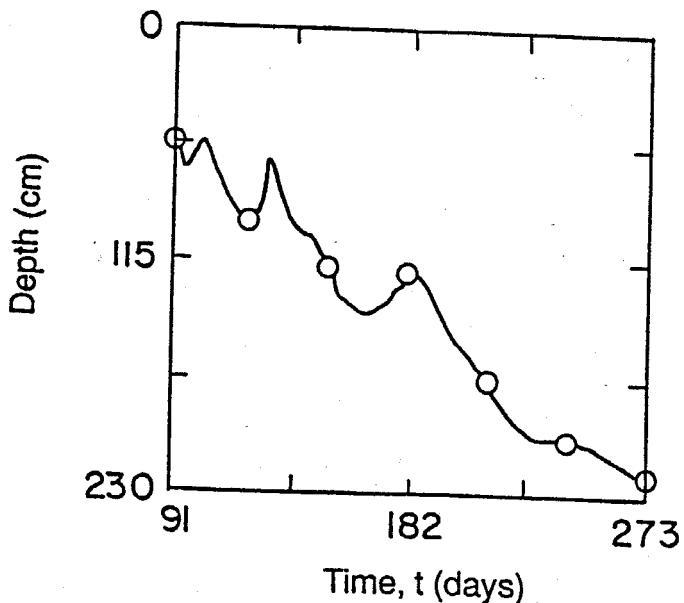


Fig. 8.9. Location of the groundwater table versus time for example 2 as simulated with the CHAIN\_2D (solid line) and SWATRE (open circles) computer programs.

### 8.3. Example 3 - Two-Dimensional Solute Transport

This example was used to verify the mathematical accuracy of the solute transport part of CHAIN\_2D. Cleary and Ungs [1978] published several analytical solutions for two-dimensional dispersion problems. One of these solutions holds for solute transport in a homogeneous, isotropic porous medium during steady-state unidirectional groundwater flow. The solute transport equation (3.11) for this situation reduces to

$$D_T \frac{\partial^2 c}{\partial x^2} + D_L \frac{\partial^2 c}{\partial z^2} - v \frac{\partial c}{\partial z} - \lambda R c = R \frac{\partial c}{\partial t} \quad (8.1)$$

where  $\lambda$  is a first-order degradation constant,  $D_L$  and  $D_T$  are the longitudinal and transverse dispersion coefficients, respectively;  $v$  is the average pore water velocity ( $q_z/\theta$ ) in the flow direction, and  $z$  and  $x$  are the spatial coordinates parallel and perpendicular to the direction

of flow. The initially solute-free medium is subjected to a solute source,  $c_0$ , of unit concentration. The source covers a length  $2a$  along the inlet boundary at  $z=0$ , and is located symmetrically about the coordinate  $x=0$ . The transport region of interest is the half-plane ( $z \geq 0$ ;  $-\infty \leq x \leq \infty$ ). The boundary conditions may be written as:

$$\begin{aligned} c(x, 0, t) &= c_0 & -a \leq x \leq a \\ c(x, 0, t) &= 0 & \text{other values of } x \\ \lim_{z \rightarrow \infty} \frac{\partial c}{\partial z} &= 0 & (8.2) \\ \lim_{x \rightarrow \pm \infty} \frac{\partial c}{\partial x} &= 0 \end{aligned}$$

The analytical solution of the above transport problem is [Javandel *et al.*, 1984, Leij and Bradford, 1994]

$$c(x, z, t) = \frac{c_0 z}{4(\pi D_L)^{1/2}} \exp\left(\frac{\nu z}{2D_L}\right) \int_0^{t/R} \exp\left[-\left(\lambda R + \frac{\nu^2}{4D_L}\right)\tau - \frac{z^2}{4D_L\tau}\right] \tau^{-3/2} \cdot [\operatorname{erf}\left(\frac{a-x}{2(D_T\tau)^{1/2}}\right) + \operatorname{erf}\left(\frac{a+x}{2(D_T\tau)^{1/2}}\right)] d\tau \quad (8.3)$$

The input transport parameters for two simulations are listed in Table 8.1. The width of the source was assumed to be 100 m. Because of symmetry, calculations were carried out only for the quarter plane where  $x \geq 0$  and  $z \geq 0$ .

Figure 8.10 shows the calculated concentration front (taken at a concentration of 0.1) at selected times for the first set of transport parameters in Table 8.1. Note the close agreement between the analytical and numerical results. Excellent agreement is also obtained for the calculated concentration distributions after 365 days at the end of the simulation (Fig. 8.11). Figures 8.12 and 8.13 show similar results for the second set of transport parameters listed in Table 8.1.

Table 8.1. Input parameters for example 3.

Parameter	Example 3a	Example 3b
$v$ [m/day]	0.1	1.0
$D_T$ [m <sup>2</sup> /day]	1.0	0.5
$D_L$ [m <sup>2</sup> /day]	1.0	1.0
$\lambda$ [day <sup>-1</sup> ]	0.0	0.01
$R$ [-]	1.0	3.0
$c_0$ [-]	1.0	1.0

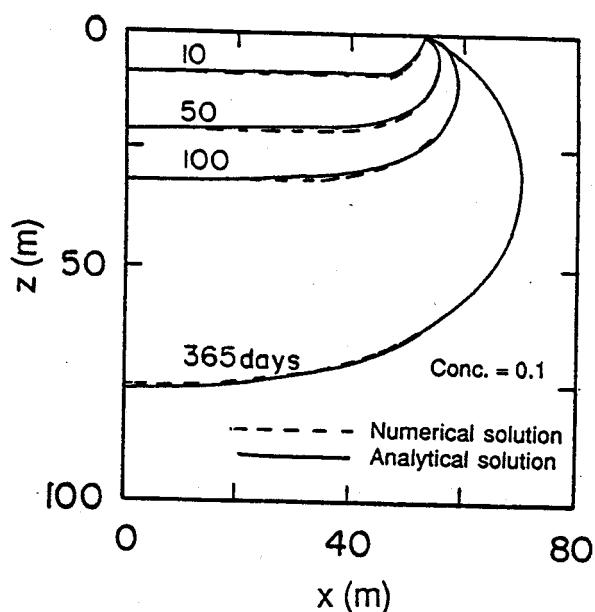


Fig. 8.10. Advancement of the concentration front ( $c=0.1$ ) for example 3a as calculated with CHAIN\_2D (dotted lines) and the analytical solution (solid lines).

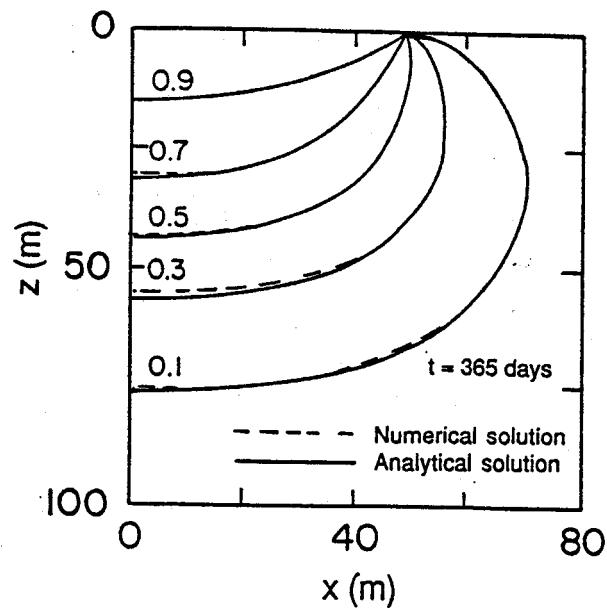


Fig. 8.11. Concentration profile at the end of the simulation ( $t=365$  days) for example 3a calculated with CHAIN\_2D (dotted lines) and the analytical solution (solid lines).

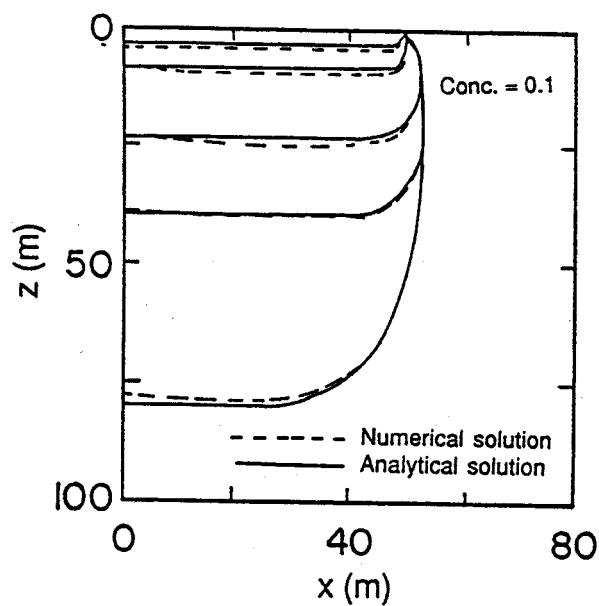


Fig. 8.12. Advancement of the concentration front ( $c=0.1$ ) for example 3b as calculated with CHAIN\_2D (dotted lines) and the analytical solution (solid lines).

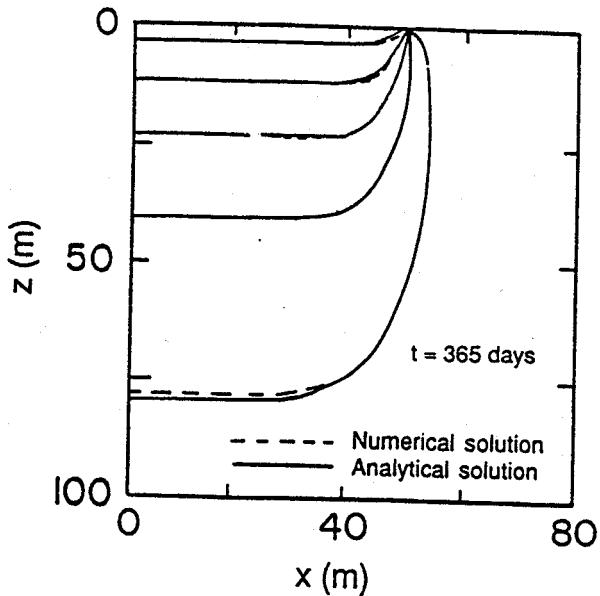


Fig. 8.13. Concentration profile at the end of the simulation ( $t=365$  days) for example 3b as calculated with CHAIN\_2D (dotted lines) and the analytical solution (solid lines).

#### 8.4. Example 4 - One-Dimensional Solute Transport with Nitrification Chain

This example was used to verify in part the mathematical accuracy of the solute transport part of CHAIN\_2D. Numerical results will be compared with results generated with an analytical solution published by *van Genuchten* [1985] for one-dimensional convective-dispersive transport of solutes involved in sequential first-order decay reactions. The analytical solution holds for solute transport in a homogeneous, isotropic porous medium during steady-state unidirectional groundwater flow. Solute transport equations (3.1) and (3.2) for this situation reduce to

$$R_1 \frac{\partial c_1}{\partial t} = D \frac{\partial^2 c_1}{\partial x^2} - v \frac{\partial c_1}{\partial x} - \mu_1 R_1 c_1 \quad (8.4)$$

$$R_i \frac{\partial c_i}{\partial t} = D \frac{\partial^2 c_i}{\partial x^2} - v \frac{\partial c_i}{\partial x} + \mu_{i-1} R_{i-1} c_{i-1} - \mu_i R_i c_i \quad i = 2, 3 \quad (8.5)$$

where  $\mu$  is a first-order degradation constant,  $D$  is the dispersion coefficient,  $v$  is the average pore water velocity ( $q_x/\theta$ ) in the flow direction,  $x$  is the spatial coordinate in the direction of flow, and where it is assumed that 3 solutes participate in the decay chain. The specific example used here applies to the three-species nitrification chain



and is the same as described by *van Genuchten* [1985], and earlier by *Cho* [1971]. The boundary conditions may be written as:

$$\begin{aligned} \left[ -D \frac{\partial c_1}{\partial x} + v c_1 \right] &= v c_{0,1}(0, t) \\ \left[ -D \frac{\partial c_i}{\partial x} + v c_i \right] &= 0 \quad i = 2, 3 \\ \lim_{x \rightarrow \infty} \frac{\partial c_i}{\partial x} &= 0 \quad i = 1, 2, 3 \end{aligned} \quad (8.7)$$

The experiment involves the application of a  $\text{NH}_4^+$  solution to an initially solute-free medium ( $c_i = 0$ ). The input transport parameters for the simulation are listed in Table 8.2.

Figure 8.14 shows concentration profiles for all three solutes at time 200 hours, calculated both numerically with CHAIN\_2D and analytically with the CHAIN code of *van Genuchten* [1985]. Figure 8.15 shows the concentration profiles at three different times (50, 100, and 200 hours) for  $\text{NH}_4^+$ ,  $\text{NO}_2^-$ , and  $\text{NO}_3^-$ , respectively. The numerical results in each case duplicated the analytical results.

Table 8.2. Input parameters for example 4.

Parameter	Value
$v$ [cm/hour]	1.0
$D$ [cm <sup>2</sup> /hour]	0.18
$\mu_1$ [hour <sup>-1</sup> ]	0.005
$\mu_2$ [hour <sup>-1</sup> ]	0.1
$\mu_3$ [hour <sup>-1</sup> ]	0.0
$R_1$ [-]	2.0
$R_2$ [-]	1.0
$R_3$ [-]	1.0
$c_i$ [-]	0.0
$c_{0,1}$ [-]	1.0

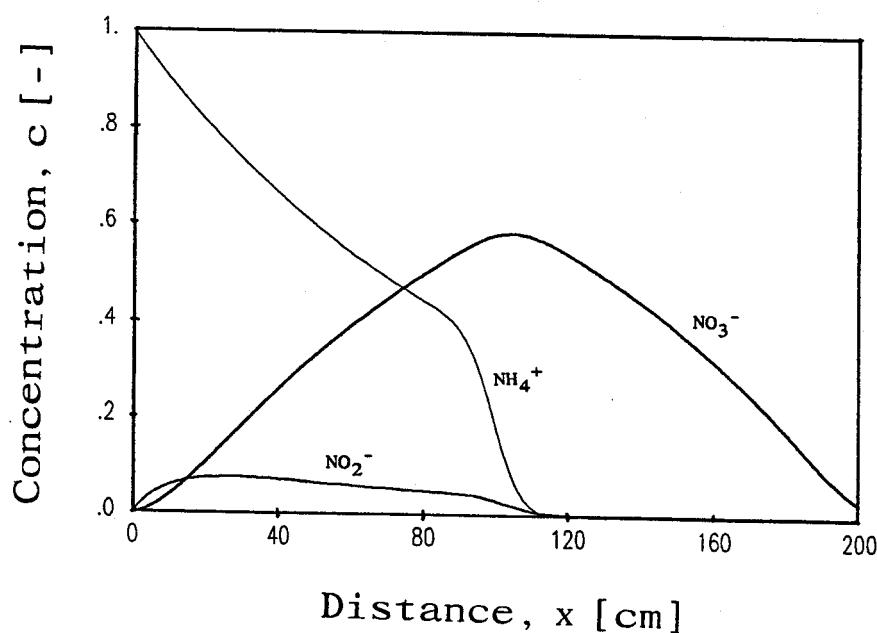


Fig. 8.14. Analytically and numerically calculated concentration profiles for  $\text{NH}_4^+$ ,  $\text{NO}_2^-$ , and  $\text{NO}_3^-$  after 200 hours for example 4.

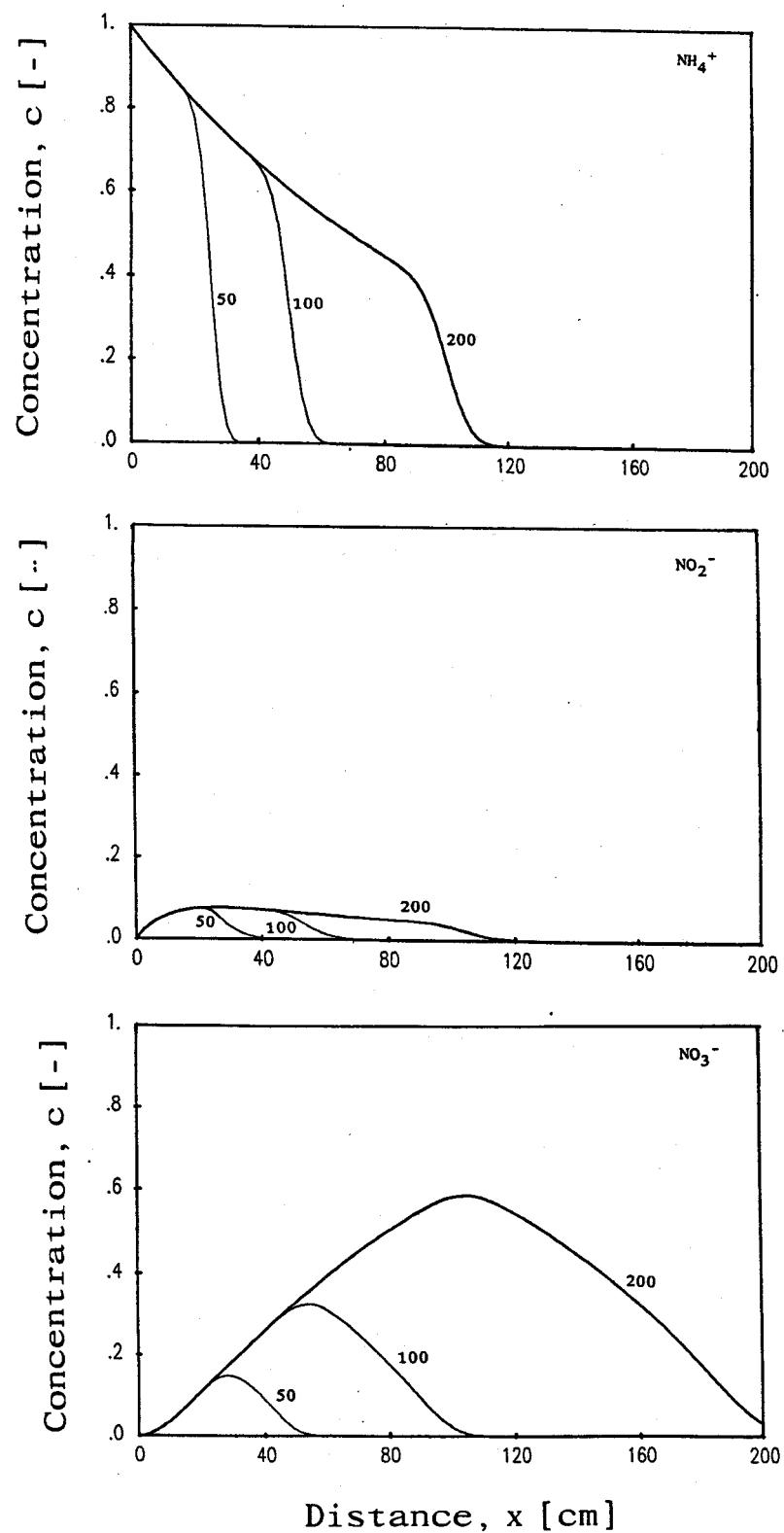


Fig. 8.15. Analytically and numerically calculated concentration profiles for  $\text{NH}_4^+$  (top),  $\text{NO}_2^-$  (middle),  $\text{NO}_3^-$  (bottom) after 50, 100, and 200 hours for example 4.

## 8.5. Example 5 - One-Dimensional Solute Transport with Nonlinear Cation Adsorption

The experiment discussed in this example was conducted by *Selim et al.* [1987], and used later for validation of the HYDRUS code [*Kool and van Genuchten*, 1991]. The soil in this experiment was Abist loam. A 10.75-cm long soil column was first saturated with a 10 mmol<sub>c</sub>L<sup>-1</sup> CaCl<sub>2</sub> solution. The experiment consisted of applying a 14.26 pore volume pulse ( $t=358.05$  hours) of 10 mmol<sub>c</sub>L<sup>-1</sup> MgCl<sub>2</sub> solution, followed by the original CaCl<sub>2</sub> solution. The adsorption isotherm was determined with the help of batch experiments [*Selim et al.*, 1987], and fitted with the Freundlich equation (3.3) [*Kool and van Genuchten*, 1991]. The Freundlich isotherm parameters, as well as other transport parameters for this problem, are listed in Table 8.3. First- and second-type boundary conditions were applied at the top and bottom of the soil column, respectively.

The observed Mg breakthrough curve is shown in Figure 8.16, together with simulated breakthrough curves obtained with CHAIN\_2D, the MONOC code of *Selim et al.* [1987] and the HYDRUS code of *Kool and van Genuchten* [1991]. The results indicate a reasonable prediction of the measured breakthrough curve using CHAIN\_2D, and close correspondence between the simulated results obtained with the CHAIN\_2D and MONOC models. The CHAIN\_2D results became identical to those generated with HYDRUS when a third-type boundary condition was invoked at the top of the soil column.

Table 8.3. Input parameters for example 5.

Parameter	Value
$q$ [cm/hour]	0.271
$D$ [cm <sup>2</sup> /hour]	1.167
$\rho$ [g/cm <sup>3</sup> ]	0.884
$\theta$ [-]	0.633
$c_0$ [mmol <sub>c</sub> /L]	10.0
$k_s$ [cm <sup>3</sup> /g]	1.687
$\beta$ [-]	1.615

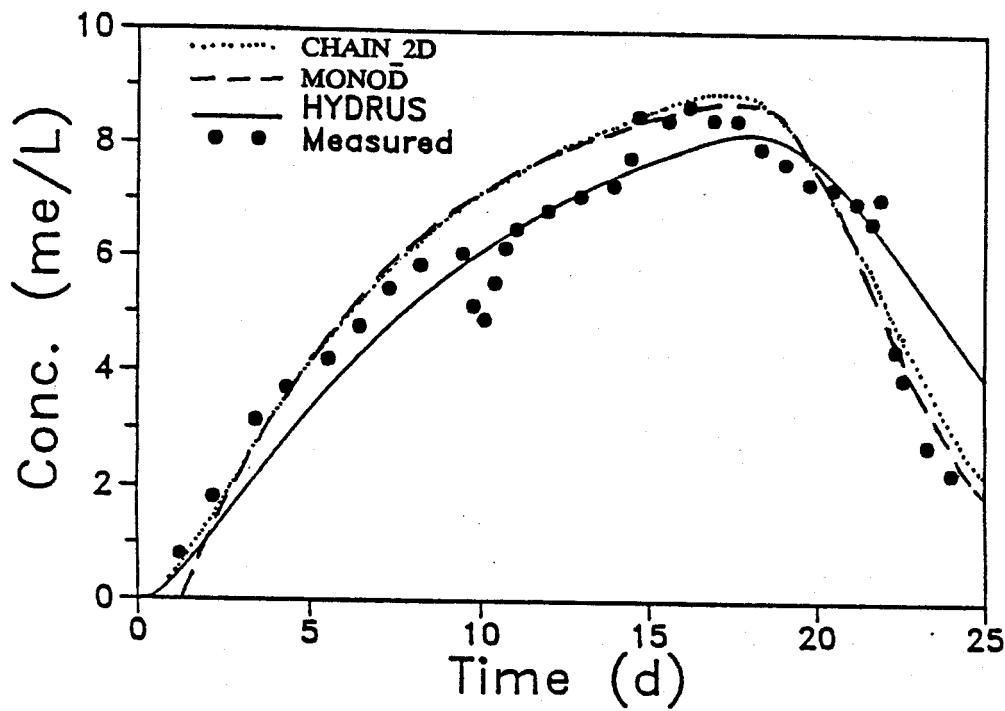


Fig. 8.16. Mg breakthrough curves for Abist loam calculated with the MONOD, HYDRUS, and CHAIN\_2D codes (data points from Selim *et al.* [1978]).

The Langmuir adsorption isotherm can also be used to model the exchange of homovalent ions. Parameters in the Langmuir adsorption isotherm for homovalent ion exchange may be derived as follows. Ion exchange for two ions with valences  $n$  and  $m$  can be expressed in a generalized form as [Sposito, 1981]

$$K_\alpha = \left( \frac{\bar{a}_1}{a_1} \right)^m \left( \frac{a_2}{\bar{a}_2} \right)^n \quad (8.8)$$

where  $K_\alpha$  is the dimensionless thermodynamic equilibrium constant, and  $\alpha$  and  $\bar{\alpha}$  denote the ion activities in the soil solution and on the exchange surfaces [-], respectively.

$$\begin{aligned} a_i &= \gamma_i c_i & i &= 1, 2 \\ \bar{a}_i &= \xi_i s_i & i &= 1, 2 \end{aligned} \tag{8.9}$$

where  $c_i$  [ $\text{ML}^{-3}$ ] (mmol/l) and  $s_i$  [ $\text{MM}^{-1}$ ] (mmol/kg) are solution and exchangeable concentrations, respectively, and  $\gamma_i$  and  $\xi_i$  are activity coefficients in the soil solution [ $\text{L}^3\text{M}^{-1}$ ] (l/mmol) and on the exchange surfaces [ $\text{MM}^{-1}$ ] (kg/mmol), respectively. Substituting (8.9) into (8.8) gives

$$K_{12} = K_v \frac{\gamma_1^m}{\gamma_2^n} = K_{ex} \frac{\xi_2^n}{\xi_1^m} \frac{\gamma_1^m}{\gamma_2^n} = \frac{s_1^m}{s_2^n} \frac{c_2^n}{c_1^m} \tag{8.10}$$

where  $K_v$  denotes the Vanselow selectivity coefficient [-], while  $K_{12}$  will be simply referred to as the selectivity coefficient [-]. Assuming that both the total solution concentration,  $C_T$  [ $\text{ML}^{-3}$ ] (mmol<sub>c</sub>/l), and the cation exchange capacity,  $S_T$  [ $\text{MM}^{-1}$ ] (mmol<sub>c</sub>/kg), are time invariant, i.e.,

$$\begin{aligned} nc_1 + mc_2 &= C_T \\ ns_1 + ms_2 &= S_T \end{aligned} \tag{8.11}$$

the Langmuir parameters  $k_s$  and  $\eta$  in (3.3) for the incoming solute become

$$\begin{aligned} k_s &= \frac{K_{12} S_T}{C_T} \\ \eta &= \frac{\vartheta(K_{12} - 1)}{C_T} \end{aligned} \tag{8.12}$$

whereas for the solute initially in the soil column:

$$k_s = \frac{S_r}{K_{12} C_r} \quad (8.13)$$

$$\eta = \frac{\vartheta(1 - K_{12})}{K_{12} C_r}$$

The parameter  $\vartheta$  in (8.12) and (8.13) equals 1 for monovalent ions, and 2 for divalent ions.

The selectivity coefficient  $K_{12}$  for example 5 was measured by *Selim et al.* [1987] ( $K_{12}=0.51$ ). From the total solution concentration ( $C_r=10 \text{ mmol}_e/\text{l}$ ) and the known cation exchange capacity ( $S_r=62 \text{ mmol}_e/\text{kg}$ ), it follows that the parameters in the Langmuir adsorption isotherm for the incoming solute (Mg) are  $k_s=3.126$  and  $\eta=-0.098$ , while those for the solute initially in the soil profile (Ca) the parameters are  $k_s=12.157$  and  $\eta=0.192$ . The observed Ca breakthrough curve is shown in Figure 8.17, together with the simulated breakthrough curves obtained with the CHAIN\_2D and MONOC codes [*Selim et al.*, 1987]. Note the closed agreement between the numerical results and the experimental data.

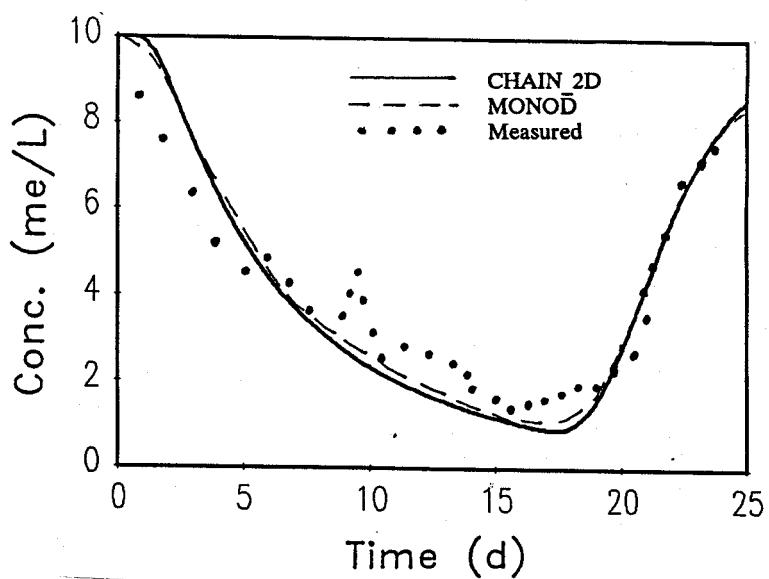


Fig. 8.17. Ca breakthrough curves for Abist loam calculated with the MONOD and CHAIN\_2D codes (data points from *Selim et al.* [1978]) (example 5).

## 8.6. Example 6 - One-Dimensional Solute Transport with Nonequilibrium Adsorption

This example considers the movement of a boron ( $H_3BO_4$ ) pulse through Glendale clay loam [van Genuchten, 1981]. The numerical simulation uses solute transport parameters that were fitted to the breakthrough curve with the CFITIM parameter estimation model [van Genuchten, 1981] assuming a two-site chemical nonequilibrium sorption model analogous to the formulation discussed in Section 3, but for steady-state water flow. Input parameters for example 6 are listed in Table 8.4. Figure 8.18 compares CHAIN\_2D numerical results with the experimental data, and with a numerical simulation assuming physical non-equilibrium and nonlinear adsorption [van Genuchten, 1981].

Table 8.4. Input parameters for example 6.

Parameter	Value
$q$ [cm/day]	17.12
$D$ [ $cm^2/day$ ]	49.0
$\theta$ [-]	0.445
$\rho$ [g/cm <sub>3</sub> ]	1.222
$c_0$ [mmol <sub>c</sub> /L]	20.0
$k_s$ [cm <sup>3</sup> /g]	1.14
$\beta$ [-]	1.0
$\eta$ [-]	0.0
$f$ [-]	0.47
$\omega$ [1/day]	0.320
$t_p$ [day]	5.06

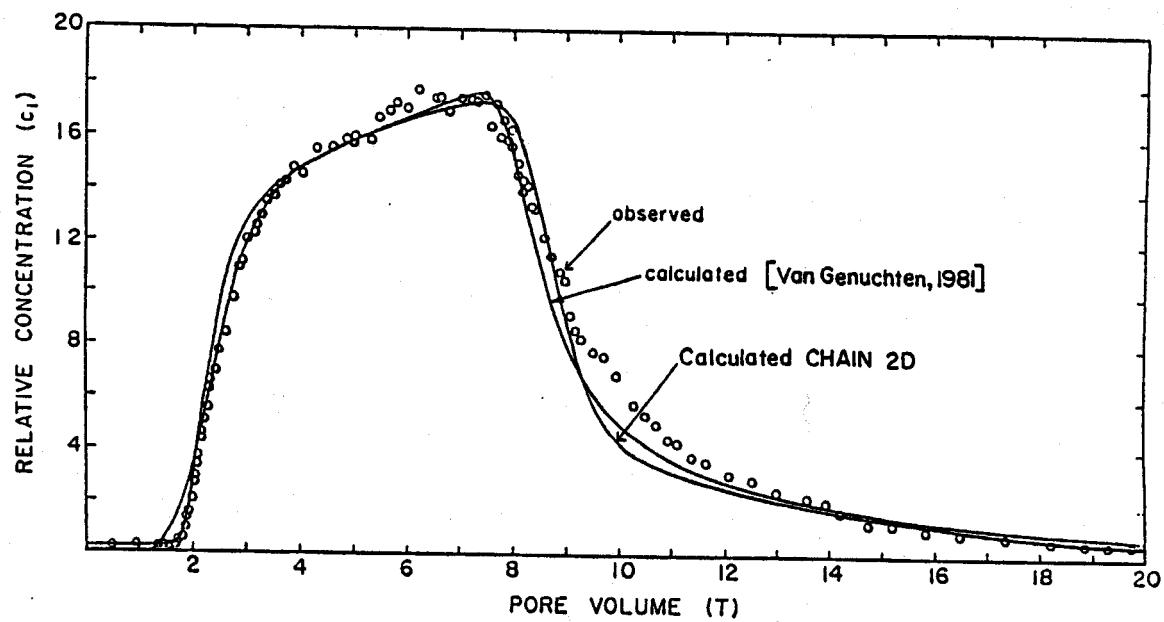
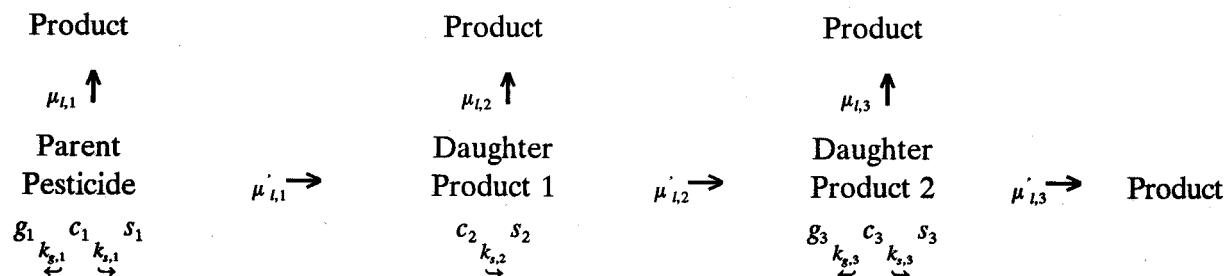


Fig. 8.18. Observed and calculated effluent curves for Boron movement through Glendale clay (data points from *van Genuchten* [1981]) (example 6).

### 8.7. Example 7 - Water and Solute Infiltration Test

This example corresponds to example 4 in the SWMS\_2D manual [Šimunek *et al.*, 1992]. The example concerns the movement of water and a dissolved solute from a single-ring infiltrometer into the soil profile consisting of two layers: a 40-cm thick A-horizon, and an underlying B/C-horizon. The hydraulic functions of the two soil layers are the same as those used in Example 2. The axisymmetric flow system and associated finite element mesh for the ponded infiltration experiment are shown in Figure 8.19. The soil profile had an initial temperature of 20°C, whereas the infiltrated water had a temperature of 30°C and contained an organic (parent) compound (the pesticide aldicarb) which is known to degrade by oxidation into two sequential daughter products (sulfone and sulfoxide) [Ou *et al.*, 1988]. Each of the three solutes also undergoes hydrolytical first-order decay that leads to products which are not simulated or monitored during their subsequent transport (i.e., oxime, sulfone oxime, and sulfoxide oxime) [Ou *et al.*, 1988]. All major solutes adsorb onto the solid phase, and volatilization is considered for the first and the third solutes. The reaction pathway is schematically given by:



The example is used here to illustrate variably-saturated water flow, heat movement, and solute transport in a layered and radially symmetric three-dimensional soil profile.

Calculations were carried out over a period of 10 days. The pressure head profile obtained in example problem 2 at the beginning of June 1982 was taken as the initial condition for the water flow equation, similarly as in example 4 of Šimunek *et al.* [1992]. The soil profile was assumed to be initially free of any solutes. All sides of the flow region were considered to be impervious, except for a small portion around the origin at the

surface (the ponded surface inside the ring infiltrometer) where constant pressure head and concentration flux boundary conditions were imposed, as well as the lower right corner where the groundwater level was kept constant. The concentration of the infiltrating water was free of any solute except for the first five days of the simulation during which time the infiltrating water contained the parent solute of unit concentration. Boundary condition (3.22) was used for the non-ponded part of the soil surface, thereby assuming the existence of a stagnant boundary layer of thickness  $d$  at the soil surface through which volatile solutes moved to the atmosphere by gas diffusion only. Water and solute extraction by plant roots was not considered.

Tables 8.5, 8.6, and 8.7 list the unsaturated soil hydraulic, heat and solute transport parameters, respectively. The solute and heat transport parameters were assumed to be the same for the two soil layers. The heat transport parameters  $b_1$ ,  $b_2$ ,  $b_3$ ,  $C_n$ ,  $C_o$ , and  $C_w$  in Table 8.6 were taken from *Chung and Horton [1987]*. The solute parameters  $k_s$ ,  $k_g$ ,  $\mu_w$ ,  $\mu'_w$ , and  $D_g$  in Table 8.7 were taken from *Wagenet and Hutson [1987]*.

Figure 8.20 presents the initial and steady-state pressure head profiles. The steady-state profile for water flow was reached after approximately 2 or 3 days. Temperature profiles at times 1 and 10 days are shown in Figure 8.21. The heat front moved relatively slowly into the soil profile in comparison with the solute front (shown later) because of the high volumetric heat capacity of the relatively wet soil. Figures 8.22, 8.23, and 8.24 show concentration profiles for all three solutes at times 2.5, 5, 7.5, and 10 days. After its application in the infiltrating water during the first five days of the simulation (Fig. 8.22a, b), the first (parent) solute is transported further by water flow and gaseous diffusion, as well as is being degraded by two first-order decay reactions, such that the soil profile is practically free of this solute after 10 days (Fig. 8.22c, d). The second solute exists exclusively because of first-order degradation of the first solute. Hence, the second solute initially corresponds mainly with the first solute, but subsequently moves faster through the soil profile because of less sorption. Note that the highest concentrations of the second solute were reached after complete application of the first solute (Fig. 8.23c, d). Similar features as for the second solute also apply to the third solute. In particular, notice that the soil profile is almost free of this solute after 2.5 days (Fig. 8.24a), and that the highest

concentrations of the third solute (while being much smaller than those for the first two solutes) were reached at the end of the simulation (Fig. 8.24d).

Table 8.5. Hydraulic input parameters for example 7.

Parameter	1st layer	2nd layer
$\theta_s = \theta_m = \theta_k$	0.399	0.339
$\theta_r = \theta_a$	0.000	0.000
$K_s = K_k$ [m/day]	0.298	0.454
$\alpha$ [1/m]	1.74	1.39
$n$ [-]	1.38	1.60

Table 8.6. Heat transport input parameters for example 7.

Parameter	Value
$\theta_n$ [-]	0.600 <sup>†</sup> (0.660 <sup>‡</sup> )
$\theta_o$ [-]	0.001
$\lambda_L$ [m]	0.005
$\lambda_T$ [m]	0.001
$b_1$ [Wm <sup>-1</sup> K <sup>-1</sup> ]	0.243
$b_2$ [Wm <sup>-1</sup> K <sup>-1</sup> ]	0.393
$b_3$ [Wm <sup>-1</sup> K <sup>-1</sup> ]	1.534
$C_n$ [Jm <sup>-3</sup> K <sup>-1</sup> ]	1.92*10 <sup>6</sup>
$C_o$ [Jm <sup>-3</sup> K <sup>-1</sup> ]	2.51*10 <sup>6</sup>
$C_w$ [Jm <sup>-3</sup> K <sup>-1</sup> ]	4.18*10 <sup>6</sup>
$T_i$ [°C]	20
$T_o$ [°C]	30

<sup>†</sup> for the first layer

<sup>‡</sup> for the second layer

Table 8.7. Solute transport input parameters  
for example 7.

Parameter	Value
$\rho$ [kg/m <sup>3</sup> ]	1300
$D_w$ [m <sup>2</sup> /day]	0.00374
$D_s$ [m <sup>2</sup> /day]	0.432
$D_L$ [m]	0.005
$D_T$ [m]	0.001
$k_{s,1}$ [m <sup>3</sup> /kg]	0.0001
$k_{s,2}$ [m <sup>3</sup> /kg]	0.00005
$k_{s,3}$ [m <sup>3</sup> /kg]	0.0002
$k_{g,1}$ [-]	1.33*10 <sup>-7</sup>
$k_{g,2}$ [-]	0.0
$k_{g,3}$ [-]	1.33*10 <sup>-3</sup>
$\mu_{w,1}$ [1/day]	0.2
$\mu_{w,2}$ [1/day]	0.01
$\mu_{w,3}$ [1/day]	0.005
$\mu'_{w,1}$ [1/day]	0.36
$\mu'_{w,2}$ [1/day]	0.024
$\mu'_{w,3}$ [1/day]	0.0024
$c_{0,1}$ [-]	1.0
$c_{0,2}$ [-]	0.0
$c_{0,3}$ [-]	0.0
$d$ [m]	0.005

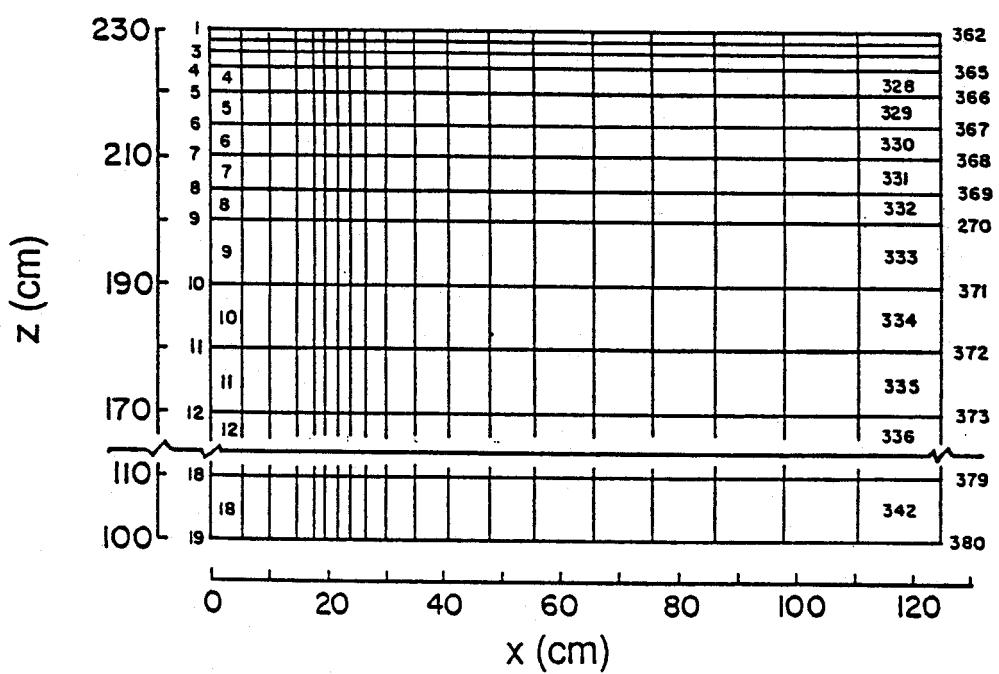
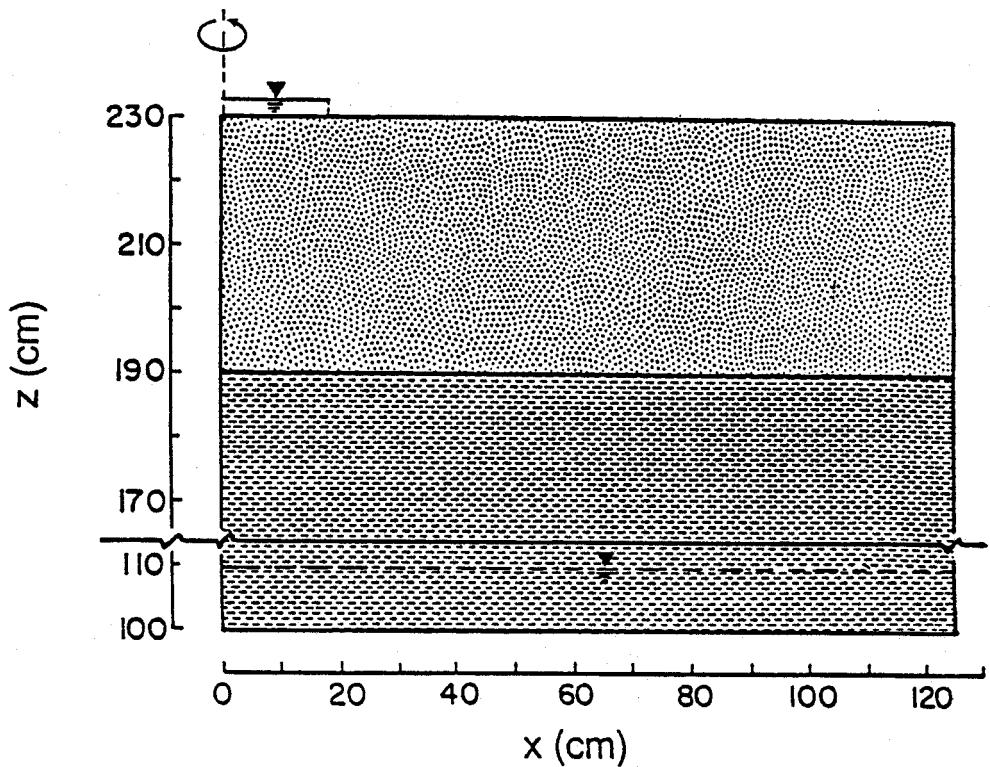


Fig. 8.19. Flow system and finite element mesh for example 7.

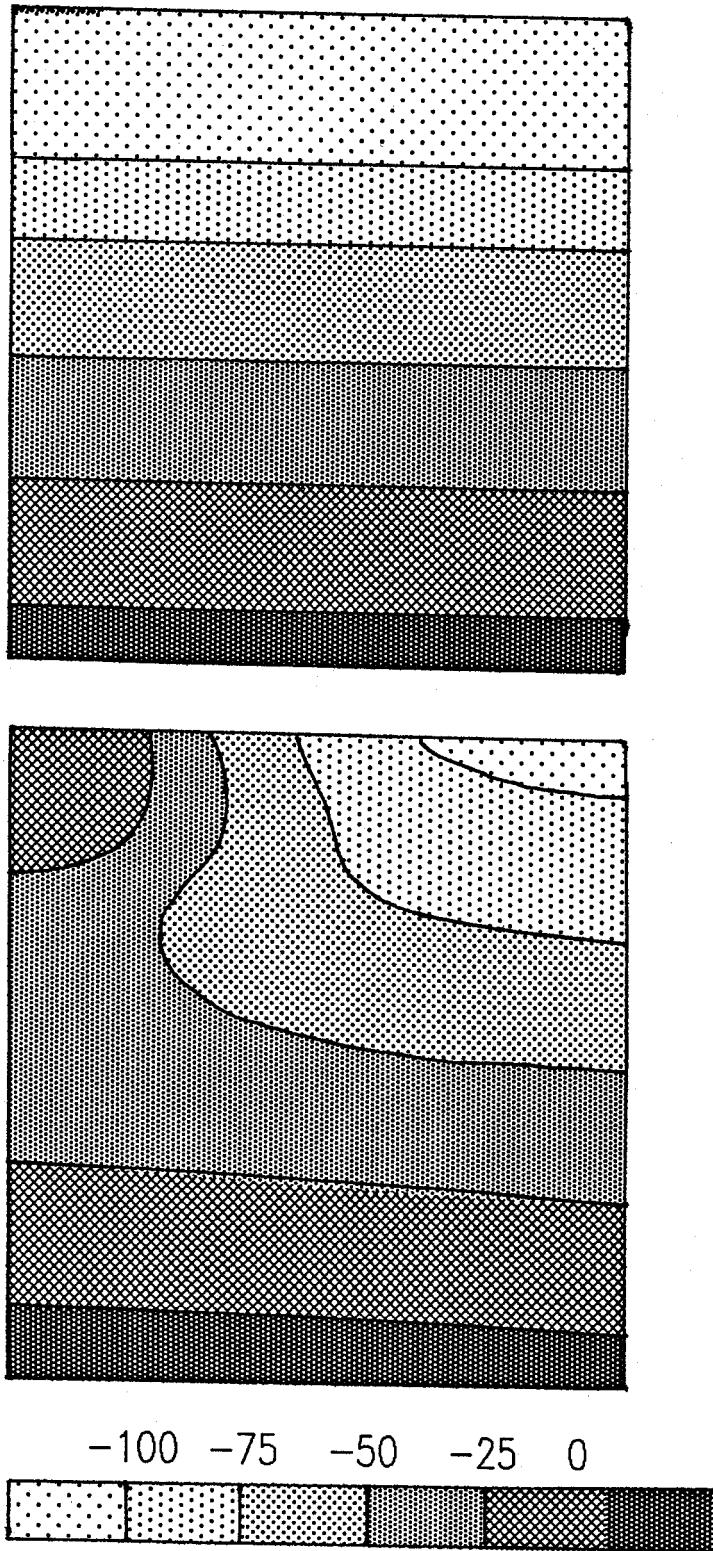
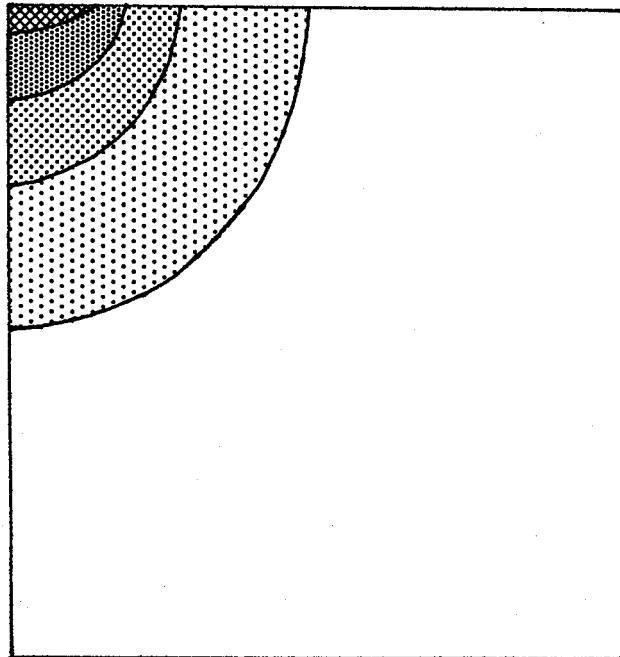
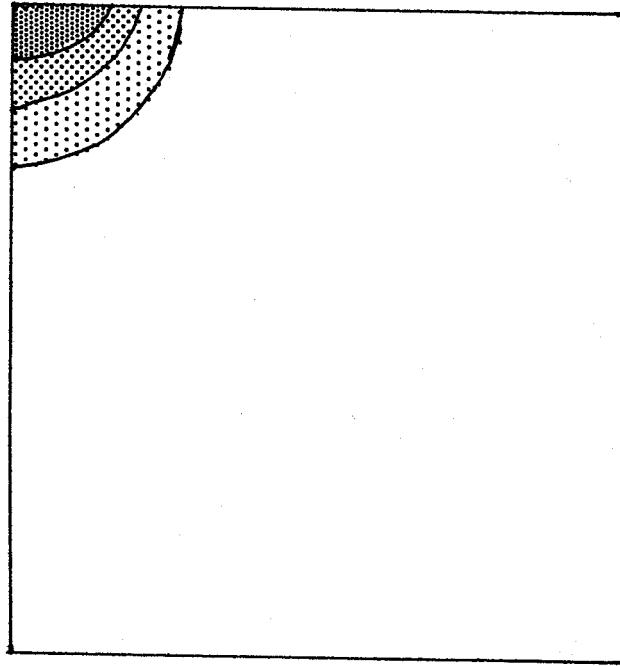


Fig. 8.20. Initial (top) and steady state (bottom) pressure head profiles for example 7.



22    24    26    28



Fig. 8.21. Temperature profiles after 1 (top) and 10 days (bottom) for example 7.

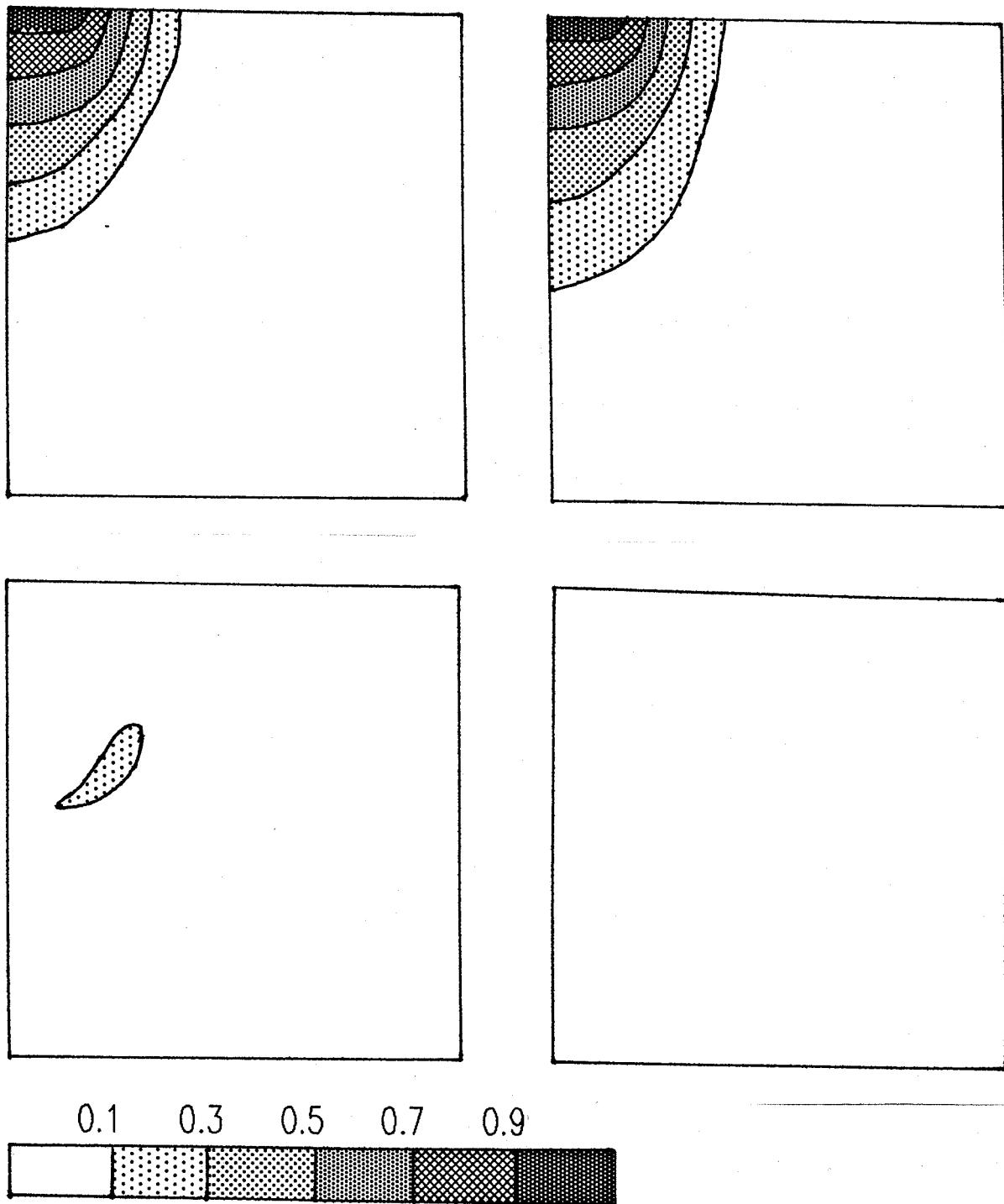
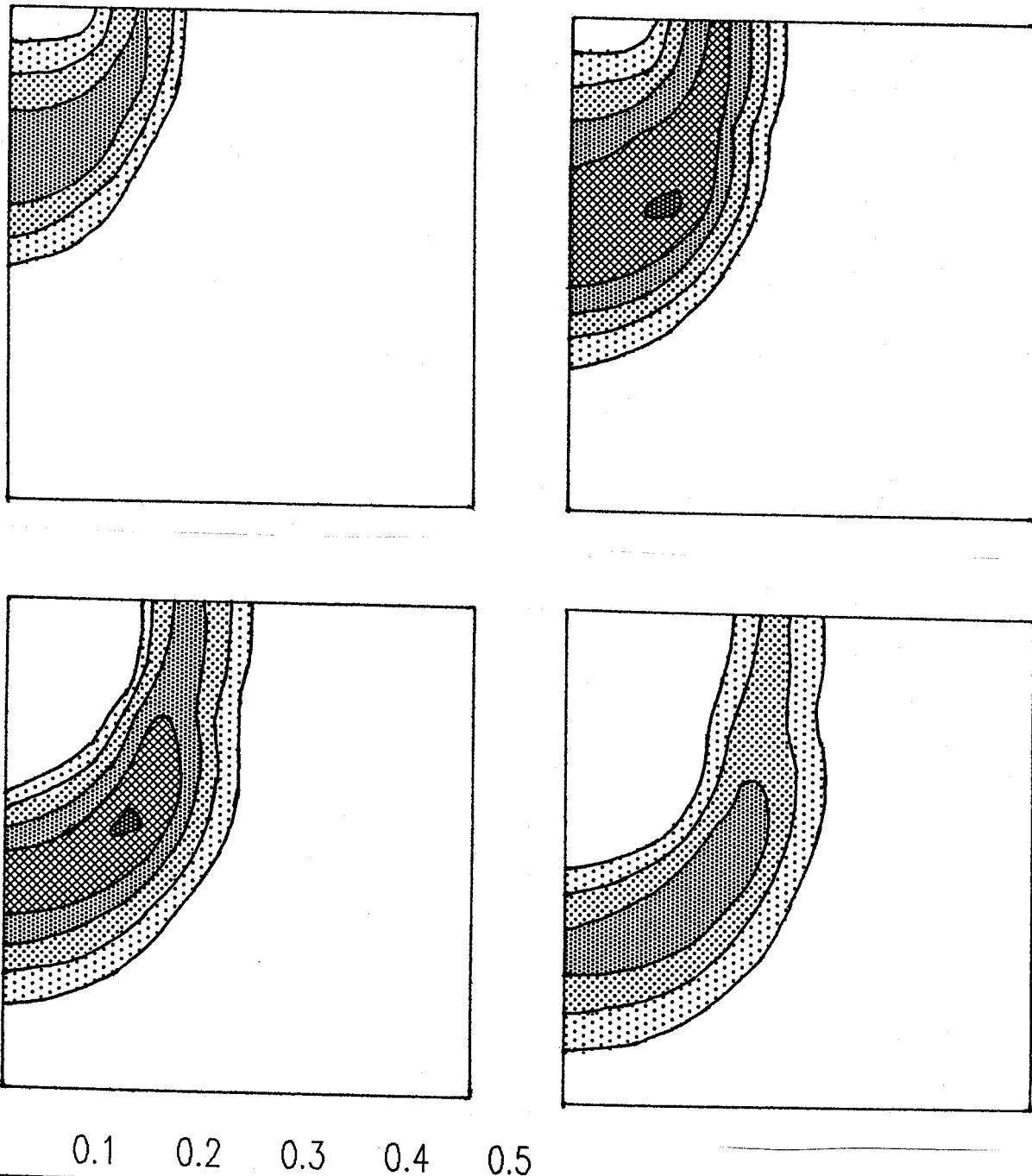


Fig. 8.22. Concentration profiles for the first solute after 2.5, 5, 7.5, and 10 days for example 7.



**Fig. 8.23.** Concentration profiles for the second solute after 2.5, 5, 7.5, and 10 days for example 7.

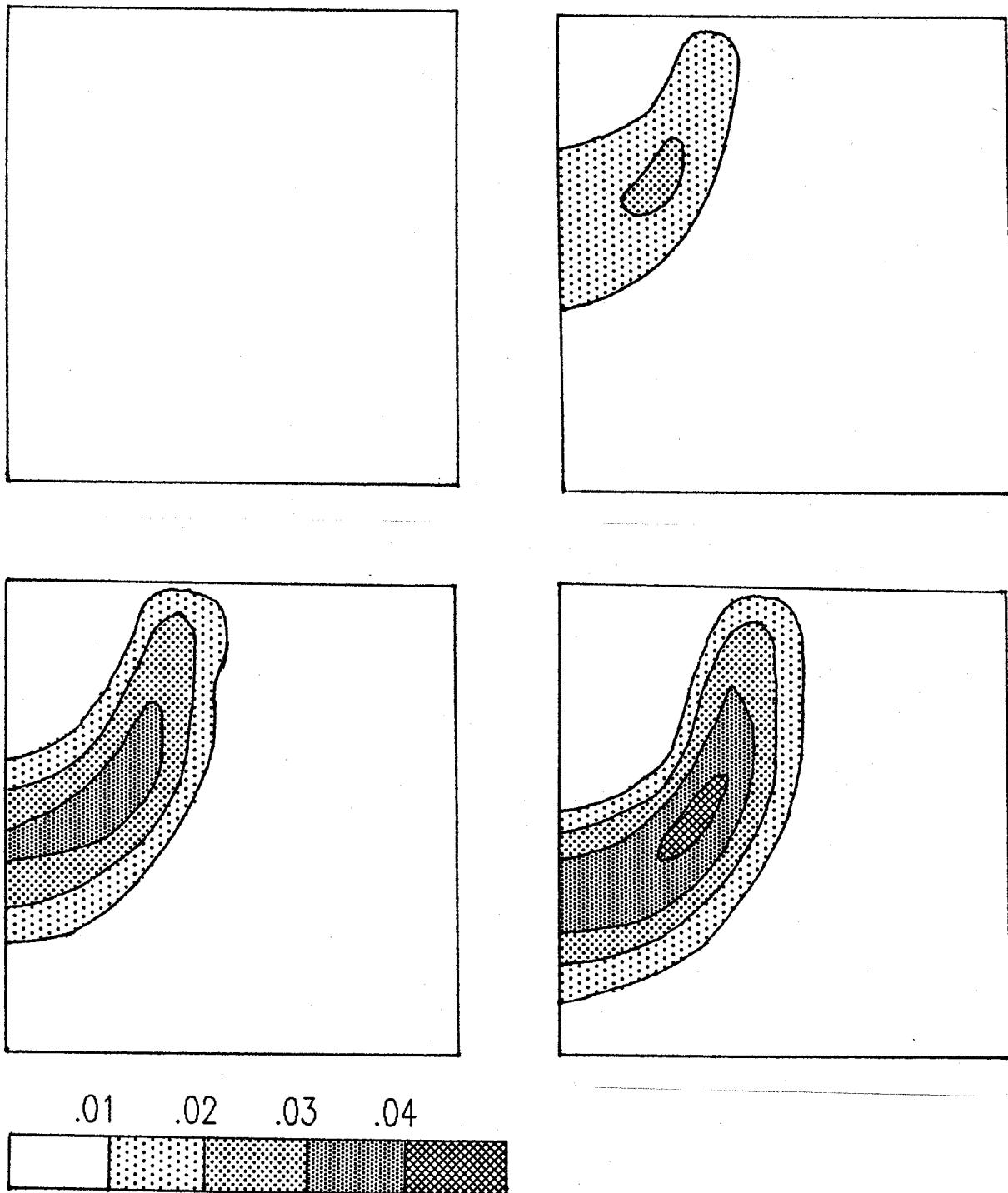


Fig. 8.24. Concentration profiles for the third solute after 2.5, 5, 7.5, and 10 days for example 7.



## 9. INPUT DATA

The input data for CHAIN\_2D are given in three separate input files. These input files consist of one or more input blocks identified by the letters from A through L. The input files and blocks must be arranged as follows:

### SELECTOR.IN

- A. Basic Information
- B. Material Information
- C. Time Information
- D. Root Water Uptake Information
- E. Seepage Face Information
- F. Drainage Information
- G. Solute Transport Information
- H. Heat Transport Information

### GRID.IN

- I. Nodal Information
- J. Element Information
- K. Boundary Geometry Information

### ATMOSPH.IN

- L. Atmospheric Information

The various input blocks are described in detail in Section 9.1, while Section 9.2 lists the actual input files for examples 1 through 7 discussed in Section 8. The output files for these examples are discussed in Section 10.

### 9.1. *Description of Data Input Blocks*

Tables 9.1 through 9.12 describe the data required for each input block. All data are read in using list-directed formatting (free format). Comment lines are provided at the beginning of, and within, each input block to facilitate, among other things, proper identification of the function of the block and the input variables. The comment lines are ignored during program execution; hence, they may be left blank but should not be omitted. All input files must be placed in the directory CHAIN\_2D.IN. The program assumes that

all input data are specified in a consistent set of units for mass M, length L, and time T. The values of temperature should be specified in degrees Celsius.

Most of the information in Tables 9.1 through 9.12 should be self-explanatory. Table 9.9 (Block I) is used to define, among other things, the nodal coordinates and initial conditions for the pressure head, temperature and solute concentrations. One short-cut may be used when generating the nodal coordinates. The short-cut is possible when two nodes (e.g.,  $N_1$  and  $N_2$ ), not adjacent to each other, are located along a transverse line such that  $N_2$  is greater than  $N_1 + 1$ . The program will automatically generate nodes between  $N_1$  and  $N_2$ , provided all of the following conditions are met simultaneously: (1) all nodes along the transverse line between nodes  $N_1$  and  $N_2$  are spaced at equal intervals, (2) values of the input variables  $hNew(n)$ ,  $Beta(n)$ ,  $Axz(n)$ ,  $Bxz(n)$ ,  $Dxz(n)$ ,  $Temp(n)$ , and  $Conc(1,n)$  through  $Conc(NS,n)$  vary linearly between nodes  $N_1$  and  $N_2$ , and (3) values of  $Kode(n)$ ,  $Q(n)$  and  $MatNum(n)$  are the same for all  $n = N_1, N_1 + 1, \dots, N_2 - 1$  (see Table 9.9).

A similar short-cut is possible when generating the elements in Block J (Table 9.10). Consider two elements,  $E_1$  and  $E_2$ , between two transverse lines such that  $E_2$  is greater than  $E_1$ . The program requires input data only for element  $E_1$  (i.e., data for elements  $E_1 + 1$  through  $E_2$  may be omitted), provided the following two conditions are met simultaneously: (1) all elements between  $E_1$  and  $E_2$  are quadrilaterals, including  $E_1$  and  $E_2$ , and (2) all elements,  $E_1, \dots, E_2$ , are assigned the same values of  $Angle(e)$ ,  $ConA1(e)$ ,  $ConA2(e)$ , and  $LayNum(e)$  as defined in Table 9.10.

Table 9.1. Block A - Basic information.

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Char	<i>Hed</i>	Heading.
4	-	-	Comment line.
5	Char	<i>LUnit</i>	Length unit (e.g., 'cm').
5	Char	<i>TUnit</i>	Time unit (e.g., 'min').
5	Char	<i>MUnit</i>	Mass unit for concentration (e.g., 'g', 'mol', '-').
6	-	-	Comment line.
7	Integer	<i>Kat</i>	Type of flow system to be analyzed: 0 for a horizontal (areal) system 1 for axisymmetric flow 2 for vertical flow in a cross-section
8	-	-	Comment line.
9	Integer	<i>MaxIt</i>	Maximum number of iterations allowed during any time step (usually 20).
9	Real	<i>TolTh</i>	Absolute water content tolerance for nodes in the unsaturated part of the flow region [-] (its recommended value is 0.0001). <i>TolTh</i> represents the maximum desired absolute change in the value of the water content, $\theta$ , between two successive iterations during a particular time step.
9	Real	<i>TolH</i>	Absolute pressure head tolerance for nodes in the saturated part of the flow region [L] (its recommended value is 0.1 cm). <i>TolH</i> represents the maximum desired absolute change in the value of the pressure head, $h$ , between two successive iterations during a particular time step.
10	-	-	Comment line.
11	Logical	<i>lWat</i>	Set this logical variable equal to .true. when transient water flow is considered. Set this logical variable equal to .false. when steady-state water flow is to be calculated.
11	Logical	<i>lChem</i>	Set this logical variable equal to .true. if solute transport is to be considered.
11	Logical	<i>CheckF</i>	Set this logical variable equal to .true. if the grid input data are to be printed for checking.
11	Logical	<i>ShortF</i>	.true. if information is to be printed only at preselected times, but not at each time step (T-level information, see Section 10.1), .false. if information is to be printed at each time step.
11	Logical	<i>FluxF</i>	.true. if detailed information about the element fluxes and discharge/recharge rates is to be printed.

Table 9.1. (continued)

Record	Type	Variable	Description
11	Logical	<i>AtmInf</i>	.true. if atmospheric boundary conditions are supplied via the input file ATMOSPH.IN, .false. if the file ATMOSPH.IN is not provided (i.e., in case of time independent boundary conditions).
11	Logical	<i>SeepF</i>	.true. if one or more seepage faces is to be considered.
11	Logical	<i>DrainF</i>	Set this logical variable equal to .true. if a drain is to be simulated by means of a boundary condition. Otherwise set equal to .false.. Section 4.3.7 explains how tile drains can be represented as boundary conditions in a regular finite element mesh.
11	Logical	<i>FreeD</i>	Set this variable equal to .true. if a unit vertical hydraulic gradient boundary condition (free drainage) is used at the bottom boundary. Otherwise set equal to .false. .
11	Logical	<i>lTemp</i>	Set this logical variable equal to .true. if heat transport is to be considered.
11	Logical	<i>IWDep</i>	.true. if hydraulic properties are to be considered as temperature dependent. .false. otherwise (see Section 2.4.2).
11	Logical	<i>lEquil</i>	.true. if equilibrium or no adsorption is considered in the solute transport equation. .false. if nonequilibrium adsorption is considered for at least one solute species.

Table 9.2. Block B - Material information.

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Integer	<i>NMat</i>	Number of soil materials. Materials are identified by the material number, <i>MatNum</i> , specified in Block I.
3	Integer	<i>NLay</i>	Number of subregions for which separate water balances are being computed. Subregions are identified by the subregion number, <i>LayNum</i> , specified in Block J.
3	Real	<i>ha</i>	Absolute value of the upper limit [L] of the pressure head interval below which a table of hydraulic properties will be generated internally for each material ( $h_a$ must be greater than 0.0; e.g. 0.001 cm) (see Section 5.3.11).
3	Real	<i>hb</i>	Absolute value of the lower limit [L] of the pressure head interval for which a table of hydraulic properties will be generated internally for each material (e.g. 1000 m). One may assign to $h_b$ the highest (absolute) expected pressure head to be expected during a simulation. If the absolute value of the pressure head during program execution lies outside of the interval $[h_a, h_b]$ , then appropriate values for the hydraulic properties are computed directly from the hydraulic functions (i.e., without interpolation in the table).
3	Integer	<i>NPar</i>	Number of parameters specified for each material (i.e., 9 in case of the modified van Genuchten model). If the original van Genuchten model is to be used, then set $\theta_a = \theta_r$ , $\theta_m = \theta_k = \theta_s$ , and $K_k = K_s$ (see Section 2.3 for the description of unsaturated soil hydraulic properties).
4	-	-	Comment line.
5	Real	<i>Par(1,M)</i>	Parameter $\theta_r$ for material $M$ [-].
5	Real	<i>Par(2,M)</i>	Parameter $\theta_s$ for material $M$ [-].
5	Real	<i>Par(3,M)</i>	Parameter $\theta_a$ for material $M$ [-].
5	Real	<i>Par(4,M)</i>	Parameter $\theta_m$ for material $M$ [-].
5	Real	<i>Par(5,M)</i>	Parameter $\alpha$ for material $M$ [ $L^{-1}$ ].
5	Real	<i>Par(6,M)</i>	Parameter $n$ for material $M$ [-].
5	Real	<i>Par(7,M)</i>	Parameter $K_s$ for material $M$ [ $LT^{-1}$ ].
5	Real	<i>Par(8,M)</i>	Parameter $K_k$ for material $M$ [ $LT^{-1}$ ].
5	Real	<i>Par(9,M)</i>	Parameter $\theta_k$ for material $M$ [-].
Record 5 information is provided for each material $M$ (from 1 to <i>NMat</i> ). If <i>IWDep</i> =.true. (Block A) then the parameters of the soil hydraulic properties $Par(i,M)$ must be specified at reference temperature $T_{ref}=20^\circ C$ .			

Table 9.3. Block C - Time information.

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Real	<i>dt</i>	Initial time increment, $\Delta t$ [T]. Initial time step should be estimated in dependence on the problem solved. For problems with high pressure gradients (e.g. infiltration into an initially dry soil), $\Delta t$ should be relatively small.
3	Real	<i>dtMin</i>	Minimum permitted time increment, $\Delta t_{min}$ [T].
3	Real	<i>dtMax</i>	Maximum permitted time increment, $\Delta t_{max}$ [T].
3	Real	<i>dMul</i>	If the number of required iterations at a particular time step is less than or equal to 3, then $\Delta t$ for the next time step is multiplied by a dimensionless number $dMul \geq 1.0$ (its value is recommended not to exceed 1.3).
3	Real	<i>dMul2</i>	If the number of required iterations at a particular time step is greater than or equal to 7, then $\Delta t$ for the next time step is multiplied by $dMul2 \leq 1.0$ (e.g. 0.33).
3	Integer	<i>MPL</i>	Number of specified print-times at which detailed information about the pressure head, water content, flux, temperature, concentrations, and the water and solute balances will be printed.
4	-	-	Comment line.
5	Real	<i>TPrint(1)</i>	First specified print-time [T].
5	Real	<i>TPrint(2)</i>	Second specified print-time [T].
.	.	.	.
5	Real	<i>TPrint(MPL)</i>	Last specified print-time [T].

Table 9.4. Block D - Root water uptake information.<sup>†</sup>

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Real	<i>P0</i>	Value of the pressure head, $h_1$ (Fig. 2.1), below which roots start to extract water from the soil.
3	Real	<i>P2H</i>	Value of the limiting pressure head, $h_3$ , below which the roots cannot extract water at the maximum rate (assuming a potential transpiration rate of $r2H$ ).
3	Real	<i>P2L</i>	As above, but for a potential transpiration rate of $r2L$ .
3	Real	<i>P3</i>	Value of the pressure head, $h_4$ , below which root water uptake ceases (usually equal to the wilting point).
3	Real	<i>r2H</i>	Potential transpiration rate [ $LT^{-1}$ ] (currently set at 0.5 cm/day).
3	Real	<i>r2L</i>	Potential transpiration rate [ $LT^{-1}$ ] (currently set at 0.1 cm/day).
			The above input parameters permit one to make the variable $h_3$ a function of the potential transpiration rate, $T_p$ ( $h_3$ presumably decreases at higher transpiration rates). CHAIN_2D currently implements the same linear interpolation scheme as used in several versions of the SWATRE code (e.g., Wesseling and Brandyk [1985]) and in the code SWMS_2D [Šimůnek <i>et al.</i> , 1992]. The scheme is based on the following interpolation:
			$h_3 = P2H + \frac{P2L - P2H}{r2H - r2L} (r2H - T_p) \quad \text{for } r2L < T_p < r2H$
			$h_3 = P2L \quad \text{for } T_a \leq r2L$
			$h_3 = P2H \quad \text{for } T_a \geq r2H$
4	-	-	Comment line.
5	Real	<i>POptm(1)</i>	Value of the pressure head, $h_2$ , below which roots start to extract water at the maximum possible rate (material number 1).
5	Real	<i>POptm(2)</i>	As above (material number 2).
5	Real	<i>POptm(NMat)</i>	As above (for material number <i>NMat</i> ).

<sup>†</sup> Block D is not read in if the logical variable *SinkF* (Block L) is set equal to .false..

Table 9.5. Block E - Seepage face information.<sup>†</sup>

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Integer	<i>NSeep</i>	Number of seepage faces expected to develop.
4	-	-	Comment line.
5	Integer	<i>NSP(1)</i>	Number of nodes on the first seepage face.
5	Integer	<i>NSP(2)</i>	Number of nodes on the second seepage face.
.	.	.	.
5	Integer	<i>NSP(NSeep)</i>	Number of nodes on the last seepage face.
6	-	-	Comment line.
7	Integer	<i>NP(1,1)</i>	Sequential global number of the first node on the first seepage face.
7	Integer	<i>NP(1,2)</i>	Sequential global number of the second node on the first seepage face.
.	.	.	.
7	Integer	<i>NP(1,NSP(1))</i>	Sequential global number of the last node on the first seepage face.
			Record 7 information is provided for each seepage face.

<sup>†</sup> Block E is not read in if the logical variable *SeepF* (Block A) is set equal to .false..

Table 9.6. Block F - Drainage information.<sup>†</sup>

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Integer	<i>NDr</i>	Number of drains. See Section 5.3.7 for a discussion on how tile drains can be represented as boundary conditions in a regular finite element mesh.
3	Real	<i>DrCorr</i>	Additional reduction in the correction factor $C_d$ (See Section 5.3.7).
4	-	-	Comment line.
5	Integer	<i>ND(1)</i>	Global number of the first drain.
5	Integer	<i>ND(2)</i>	Global number of the second drain.
.	.	.	.
5	Integer	<i>ND(NDr)</i>	Global number of the last drain.
6	-	-	Comment line.
7	Integer	<i>NEID(1)</i>	Number of elements surrounding the first drain.
7	Integer	<i>NEID(2)</i>	Number of elements surrounding the second drain.
.	.	.	.
7	Integer	<i>NEID(NDr)</i>	Number of elements surrounding the last drain.
8	-	-	Comment line.
9	Real	<i>EfDim(1,1)</i>	Effective diameter of the first drain, $d_e$ [L] (see Section 5.3.7).
9	Real	<i>EfDim(2,1)</i>	Dimension of the square in finite element mesh representing the first drain, $D$ [L] (see Section 5.3.7).
			Record 9 information is provided for each drain.
10	-	-	Comment line.
11	Integer	<i>KEIDr(1,1)</i>	Global number of the first element surrounding the first drain.
11	Integer	<i>KEIDr(1,2)</i>	Global number of the second element surrounding the first drain.
.	.	.	.
11	Integer	<i>KEIDr(1,NEID(1))</i>	Global number of the last element surrounding the first drain.
			Record 11 information is provided for each drain.

<sup>†</sup> Block F is not read in if the logical variable *DrainF* (Block A) is set equal to .false. .

Table 9.7. Block G - Solute transport information.<sup>†</sup>

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Real	<i>Epsi</i>	Temporal weighing coefficient. =0.0 for an explicit scheme. =0.5 for a Crank-Nicholson implicit scheme. =1.0 for a fully implicit scheme.
3	Logical	<i>lUpW</i>	.true. if upstream weighing formulation is to be used. .false. if the original Galerkin formulation is to be used.
3	Logical	<i>lArtD</i>	.true. if artificial dispersion is to be added in order to fulfill the stability criterion <i>PeCr</i> (see Section 6.3.6). .false. otherwise.
3	Logical	<i>lTDep</i>	.true. if at least one transport or reaction coefficient ( <i>ChPar</i> ) is temperature dependent. .false. otherwise. If <i>lTDep</i> = .true., then all values of <i>ChPar(i,M)</i> should be specified at a reference temperature $T_r = 20^\circ\text{C}$ .
3	Real	<i>cTolA</i>	Absolute concentration tolerance [ $\text{ML}^{-3}$ ], the value is dependent on the units used (set equal to zero if nonlinear adsorption is not considered).
3	Real	<i>cTolR</i>	Relative concentration tolerance [-] (set equal to zero if nonlinear adsorption is not considered).
3	Integer	<i>MaxItC</i>	Maximum number of iterations allowed during any time step for solute transport - usually 20 (set equal to zero if nonlinear adsorption is not considered).
3	Real	<i>PeCr</i>	Stability criteria (see Section 6.3.6). Set equal to zero when <i>lUpW</i> is equal to .true..
4	-	-	Comment line.
5	Real	<i>ChPar(1,M)</i>	Bulk density of material <i>M</i> , $\rho$ [ $\text{ML}^{-3}$ ].
5	Real	<i>ChPar(2,M)</i>	Longitudinal dispersivity for material type <i>M</i> , $D_L$ [L].
5	Real	<i>ChPar(3,M)</i>	Transverse dispersivity for material type <i>M</i> , $D_T$ [L].
5	Real	<i>ChPar(4,M)</i>	Dimensionless fraction of the adsorption sites classified as type-1, i.e., sites with instantaneous sorption. Set equal to 1 if equilibrium transport is to be considered.
			Record 5 information is provided for each material <i>M</i> (from 1 to <i>NMat</i> ).
6	-	-	Comment line.
7	Real	<i>ChPar(5,M)</i>	Ionic or molecular diffusion coefficient in free water, $D_w$ [ $\text{L}^2\text{T}^{-1}$ ].
7	Real	<i>ChPar(6,M)</i>	Ionic or molecular diffusion coefficient in gas phase, $D_g$ [ $\text{L}^2\text{T}^{-1}$ ].
8	-	-	Comment line.

Table 9.7. (continued)

Record	Type	Variable	Description
9	Real	<i>ChPar(7,M)</i>	Adsorption isotherm coefficient, $k_s$ , for material type $M$ [ $L^3M^{-1}$ ]. Set equal to zero if no adsorption is to be considered.
9	Real	<i>ChPar(8,M)</i>	Adsorption isotherm coefficient, $\eta$ , for material type $M$ [ $L^3M^{-1}$ ]. Set equal to zero if Langmuir adsorption isotherm is not to be considered.
9	Real	<i>ChPar(9,M)</i>	Adsorption isotherm coefficient, $\beta$ , for material type $M$ [-]. Set equal to one if Freundlich adsorption isotherm is not to be considered.
9	Real	<i>ChPar(10,M)</i>	Equilibrium distribution constant between liquid and gas phases, $k_g$ , material type $M$ [-].
9	Real	<i>ChPar(11,M)</i>	First-order rate constant for dissolved phase, $\mu_w$ , material type $M$ [ $T^{-1}$ ].
9	Real	<i>ChPar(12,M)</i>	First-order rate constant for solid phase, $\mu_s$ , material type $M$ [ $T^{-1}$ ].
9	Real	<i>ChPar(13,M)</i>	First-order rate constant for gas phase, $\mu_g$ , material type $M$ [ $T^{-1}$ ].
9	Real	<i>ChPar(14,M)</i>	Rate constant, $\mu_w'$ , representing a first-order decay for the first solute and zero-order production for the second solute in the dissolved phase, material type $M$ [ $T^{-1}$ ].
9	Real	<i>ChPar(15,M)</i>	Same as above for the solid phase, $\mu_s'$ , material type $M$ [ $T^{-1}$ ].
9	Real	<i>ChPar(16,M)</i>	Same as above for the gas phase, $\mu_g'$ , material type $M$ [ $T^{-1}$ ].
9	Real	<i>ChPar(17,M)</i>	Zero-order rate constant for dissolved phase, $\gamma_w$ , material type $M$ [ $ML^{-3}T^{-1}$ ].
9	Real	<i>ChPar(18,M)</i>	Zero-order rate constant for solid phase, $\gamma_s$ , material type $M$ [ $T^{-1}$ ].
9	Real	<i>ChPar(19,M)</i>	Zero-order rate constant for gas phase, $\gamma_g$ , material type $M$ [ $ML^{-3}T^{-1}$ ].
9	Real	<i>ChPar(20,M)</i>	First-order mass transfer coefficient for nonequilibrium adsorption, $\omega$ , material type $M$ [ $T^{-1}$ ].
			Record 9 information is provided for each material $M$ (from 1 to $NMat$ ).
			Record 6 through 9 informations are provided for each solute (from 1 to $NS$ ).
10,11	-	-	Comment lines.
12	Real	<i>TDep(5)</i>	Activation energy for parameter <i>ChPar(5,M)</i> [ $ML^2T^{-2}M^{-1}$ ] (See Section 3.4). This parameter should be specified in $J\ mol^{-1}$ . Set equal to 0 if <i>ChPar(5,M)</i> is temperature independent.
12	Real	<i>TDep(6)</i>	Same for parameter <i>ChPar(6,M)</i> [ $ML^2T^{-2}M^{-1}$ ].
13	-	-	Comment line.
14	Real	<i>TDep(7)</i>	Same for parameter <i>ChPar(7,M)</i> [ $ML^2T^{-2}M^{-1}$ ].
.	.	.	.
14	Real	<i>TDep(20)</i>	Same for parameter <i>ChPar(20,M)</i> [ $ML^2T^{-2}M^{-1}$ ].
			Records 11 through 14 information is provided only when the logical variable <i>ITDep</i> of record 3 is set equal to .true..
15	-	-	Comment line.

Table 9.7. (continued)

Record	Type	Variable	Description
16	Integer	<i>KodCB(1)</i>	Code specifying the type of boundary condition for solute transport applied to a particular node. Positive and negative signs indicate that first-, or second- or third- (depending upon the calculated water flux $Q$ ), type boundary conditions are implemented, respectively. In case of time-independent boundary conditions ( $Kode(1) = \pm 1, \pm 2, \pm 5$ , or $\pm 6$ ; See Block I), <i>KodCB(1)</i> also refers to the field <i>cBound</i> for the value of the solute transport BC. $cBound(i,abs(KodCB(1)))$ is the value of the boundary condition for node $KXB(1)$ (the first of a set of sequentially numbered boundary nodes for which $Kode(N)$ is not equal to zero). Permissible values are $\pm 1, \pm 2, \dots, \pm 5, \pm 6, -7$ . When <i>KodCB(1)</i> equals -7, a special type of boundary condition for volatile solutes described by equations (3.21) and (3.22) is applied.
16	Integer	<i>KodCB(2)</i>	Same as above for the second boundary node.
.	.	.	.
16	Integer	<i>KodCB(NumBP)</i>	Same as above for the last boundary node.
17	-	-	Comment line.
18	Real	<i>cBound(1,1)</i>	Value of the concentration for the first time-independent BC [ $ML^{-3}$ ]. Set equal to zero if no $KodCB(n) = \pm 1$ is specified.
18	Real	<i>cBound(1,2)</i>	Value of the concentration for the second time-independent BC [ $ML^{-3}$ ]. Set equal to zero if no $KodCB(n) = \pm 2$ is specified.
.	.	.	.
18	Real	<i>cBound(1,5)</i>	Value of the concentration for the fifth time-independent BC [ $ML^{-3}$ ]. If water uptake is specified, then <i>cBound(1,5)</i> is automatically used for the concentration of water removed from the flow region by root water uptake [ $ML^{-3}$ ]. Set equal to zero if no $KodCB(n) = \pm 5$ and no root solute uptake is specified.
18	Real	<i>cBound(1,6)</i>	Value of the concentration for the sixth time-independent BC [ $ML^{-3}$ ]. If internal sources are specified, then <i>cBound(1,6)</i> is automatically used for the concentration of water injected into the flow region through internal sources [ $ML^{-3}$ ]. Set equal to zero if no $KodCB(n) = \pm 6$ and no internal sources are specified.
18	Real	<i>cBound(1,7)</i>	Concentration of the incoming fluid in equation (3.21) [ $ML^{-3}$ ]. Set equal to zero if no $KodCB(n) = -7$ is specified.
18	Real	<i>cBound(1,8)</i>	Concentration above the stagnant boundary layer, $g_{am}$ (see equation (3.21)) [ $ML^{-3}$ ]. Set equal to zero if no $KodCB(n) = -7$ is specified.
18	Real	<i>cBound(1,9)</i>	Thickness of the stagnant boundary layer, $d$ [L] (see equation (3.21)). Set equal to zero if no $KodCB(n) = -7$ is specified.

Record 18 information is provided for each solute (from 1 to  $NS$ ).

Table 9.7. (continued)

Record	Type	Variable	Description
19	-	-	Comment line.
20	Real	<i>tPulse</i>	Time duration of the concentration pulse [T].

<sup>†</sup>Block G is not needed when the logical variable *lChem* in Block A is set equal to .false. .

Table 9.8. Block H - Heat transport information.<sup>†</sup>

Record	Type	Symbol	Description
1,2	-	-	Comment lines.
3	Real	<i>TPar</i> (1, <i>M</i> )	Volumetric solid phase fraction of material <i>M</i> , $\theta_n$ [-].
3	Real	<i>TPar</i> (2, <i>M</i> )	Volumetric organic matter fraction of material <i>M</i> , $\theta_o$ [-].
3	Real	<i>TPar</i> (3, <i>M</i> )	Longitudinal thermal dispersivity of material <i>M</i> , $\lambda_L$ [L].
3	Real	<i>TPar</i> (4, <i>M</i> )	Transverse thermal dispersivity of material <i>M</i> , $\lambda_T$ [L].
3	Real	<i>TPar</i> (5, <i>M</i> )	Coefficient $b_1$ in the thermal conductivity function [MLT <sup>3</sup> K <sup>-1</sup> ] (e.g. Wm <sup>-1</sup> K <sup>-1</sup> ) (see equation (4.5)).
3	Real	<i>TPar</i> (6, <i>M</i> )	Coefficient $b_2$ in the thermal conductivity function [MLT <sup>3</sup> K <sup>-1</sup> ] (e.g. Wm <sup>-1</sup> K <sup>-1</sup> ) (see equation (4.5)).
3	Real	<i>TPar</i> (7, <i>M</i> )	Coefficient $b_3$ in the thermal conductivity function [MLT <sup>3</sup> K <sup>-1</sup> ] (e.g. Wm <sup>-1</sup> K <sup>-1</sup> ) (see equation (4.5)).
3	Real	<i>TPar</i> (8, <i>M</i> )	Volumetric heat capacity of solid phase of material <i>M</i> , $C_n$ [ML <sup>-1</sup> T <sup>-2</sup> K <sup>-1</sup> ] (e.g. Jm <sup>-3</sup> K <sup>-1</sup> ).
3	Real	<i>TPar</i> (9, <i>M</i> )	Volumetric heat capacity of organic matter of material <i>M</i> , $C_o$ [ML <sup>-1</sup> T <sup>-2</sup> K <sup>-1</sup> ] (e.g. Jm <sup>-3</sup> K <sup>-1</sup> ).
3	Real	<i>TPar</i> (10, <i>M</i> )	Volumetric heat capacity of liquid phase of material <i>M</i> , $C_w$ [ML <sup>-1</sup> T <sup>-2</sup> K <sup>-1</sup> ] (e.g. Jm <sup>-3</sup> K <sup>-1</sup> ).
			Record 3 is required for each soil material <i>M</i> (from 1 to <i>NMat</i> ).
4	-	-	Comment line.
5	Integer	<i>KodTB</i> (1)	Code specifying the type of boundary condition for heat transport applied to a particular node. Positive and negative signs mean that first-, or second- or third- (depending upon the calculated water flux <i>Q</i> ) type boundary conditions will be implemented, respectively. In case of time-independent boundary conditions ( <i>Kode</i> (1)=±1,±2,±5, or ±6; See Block I), <i>KodTB</i> (1) refers to the vector <i>TBound</i> for the value of the heat transport BC. <i>TBound</i> (abs( <i>KodTB</i> (1))) is the value of the boundary condition for node <i>KXB</i> (1) (the first of a set of sequentially numbered boundary nodes for which <i>Kode</i> ( <i>N</i> ) is not equal to zero). Permissible values are ±1,...,±6.
5	Integer	<i>KodTB</i> (2)	Same as above for the second boundary node.
.	.	.	.
5	Integer	<i>KodTB</i> ( <i>NumBP</i> )	Same as above for the last boundary node.
6	-	-	Comment line.
7	Real	<i>TBound</i> (1)	Value of the first time-independent thermal boundary condition [°C]. Set equal to zero if no <i>KodTB</i> ( <i>n</i> )=±1 is specified.
7	Integer	<i>TBound</i> (2)	Value of the second time-independent thermal boundary condition [°C]. Set equal to zero if no <i>KodTB</i> ( <i>n</i> )=±2 is specified.
.	.	.	.
.	.	.	.

Table 9.8. (continued)

Record	Type	Variable	Description
7	Integer	<i>TBound(6)</i>	Value of the sixth time-independent thermal boundary condition [ $^{\circ}\text{C}$ ]. Set equal to zero if no $\text{KodTB}(n) = \pm 6$ is specified. If internal sources are specified, then <i>TBound(6)</i> is automatically used for the temperature of water injected into the flow region through internal source.
8	-	-	Comment line.
9	Real	<i>Ampl</i>	Temperature amplitude at the soil surface [K] prescribed for nodes where $\text{Kode}(n) = \pm 4$ . Set equal to zero when no $\text{Kode}(n) = \pm 4$ is specified.
9	Real	<i>tPeriod</i>	Time interval for the completion of one temperature cycle (usually 1 day) [T].

<sup>†</sup> Block H need not be supplied if logical variables *iTemp* (Block A) is set equal to .false.

Table 9.9. Block I - Nodal information.

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Integer	<i>NumNP</i>	Number of nodal points.
3	Integer	<i>NumEl</i>	Number of elements (quadrilaterals and/or triangles).
3	Integer	<i>IJ</i>	Maximum number of nodes on any transverse line. Set equal to zero if <i>IJ</i> >10.
3	Integer	<i>NumBP</i>	Number of boundary nodes for which <i>Kode(N)</i> is not equal to 0.
3	Integer	<i>NS</i>	Number of solutes in a chain reaction.
3	Integer	<i>NObs</i>	Number of observation nodes for which values of the pressure head, the water content, temperature (for <i>lTemp</i> =.true.), and the solution and sorbed concentrations (for <i>lChem</i> =.true.) are printed at each time level.
4	-	-	Comment line.
5	Integer	<i>n</i>	Nodal number.
5	Integer	<i>Kode(n)</i>	Code specifying the type of boundary condition applied to a particular node. Permissible values are $0, \pm 1, \pm 2, \pm 3, \pm 4, \dots, \pm \text{NumKD}$ (see Section 7.3).
5	Real	<i>x(n)</i>	<i>x</i> -coordinate of node <i>n</i> [L] (always a horizontal coordinate).
5	Real	<i>z(n)</i>	<i>z</i> -coordinate of node <i>n</i> [L]. <i>z</i> is the vertical coordinate for problems involving vertical planar or axisymmetric flow. For axisymmetric flow, <i>z</i> coincides with the vertical axis of symmetry.
5	Real	<i>hNew(n)</i>	Initial value of the pressure head at node <i>n</i> [L]. If <i>lWat</i> =.false. in Block A, then <i>hNew(n)</i> represents the initial guess of the pressure head for steady state conditions.
5	Real	<i>Q(n)</i>	Prescribed recharge/discharge rate at node <i>n</i> ; [ $L^2T^{-1}$ ] for planar flow, [ $L^3T^{-1}$ ] for axisymmetric flow. <i>Q(n)</i> is negative when directed out of the system. When no value for <i>Q(n)</i> is needed, set <i>Q(n)</i> equal to zero.
5	Integer	<i>MatNum(n)</i>	Index for material whose hydraulic and transport properties are assigned to node <i>n</i> .
5	Real	<i>Beta(n)</i>	Value of the water uptake distribution, <i>b(x,z)</i> [ $L^{-2}$ ], in the soil root zone at node <i>n</i> . Set <i>Beta(n)</i> equal to zero if node <i>n</i> lies outside the root zone.
5	Real	<i>Axz(n)</i>	Nodal value of the dimensionless scaling factor $\alpha_h$ [-] associated with the pressure head.
5	Real	<i>Bxz(n)</i>	Nodal value of the dimensionless scaling factor $\alpha_K$ [-] associated with the saturated hydraulic conductivity.
5	Real	<i>Dxz(n)</i>	Nodal value of the dimensionless scaling factor $\alpha_\theta$ [-] associated with the water content.

Table 9.9. (continued)

Record	Type	Variable	Description
5	Real	<i>Temp(n)</i>	Initial value of temperature at node $n$ [ $^{\circ}$ C] (if <i>lTemp</i> = .false., then set equal to 0 or any initial value, which is to be used with temperature dependent water flow and solute transport).
5	Real	<i>Conc(1,n)</i>	Initial value of the concentration of the first solute at node $n$ [ $ML^{-3}$ ] (do not have to be specified if <i>lChem</i> = .false.).
5	Real	<i>Conc(2,n)</i>	Initial value of concentration of the second solute at node $n$ [ $ML^{-3}$ ] (must not be specified if <i>lChem</i> = .true. and <i>NS</i> < 2).
.	.	.	.
5	Real	<i>Conc(i,n)</i>	Initial value of the concentration of the last solute at node $n$ [ $ML^{-3}$ ] (must not be specified if <i>lChem</i> = .true. and <i>NS</i> < <i>i</i> )..
5	Real	<i>Sorb(1,n)</i>	Initial value of the adsorbed concentration on type-2 sites of the first solute at node $n$ [ $ML^{-3}$ ]. This variable does not have to be specified if <i>lChem</i> = .false.. or <i>lEquil</i> = .true. .
5	Real	<i>Sorb(2,n)</i>	Initial value of the adsorbed concentration on type-2 sites of the second solute at node $n$ [ $ML^{-3}$ ]. This variable does not have to be specified if <i>lChem</i> = .false. or <i>lEquil</i> = .true. or <i>NS</i> < 2.
.	.	.	.
5	Real	<i>Sorb(NS,n)</i>	Initial value of the adsorbed concentration on type-2 sites of the <i>NS</i> th solute at node $n$ [ $ML^{-3}$ ]. This variable does not have to be specified if <i>lChem</i> = .false. or <i>lEquil</i> = .true. .
In general, record 5 information is required for each node $n$ , starting with $n=1$ and continuing sequentially until $n=NumNP$ . Record 5 information for certain nodes may be skipped if several conditions are satisfied (see beginning of this section).			

Table 9.10. Block J - Element information.

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Integer	$e$	Element number.
3	Integer	$KX(e,1)$	Global nodal number of corner node $i$ .
3	Integer	$KX(e,2)$	Global nodal number of corner node $j$ .
3	Integer	$KX(e,3)$	Global nodal number of corner node $k$ .
3	Integer	$KX(e,4)$	Global nodal number of corner node $l$ . Indices $i$ , $j$ , $k$ , and $l$ , refer to the corner nodes of an element $e$ taken in a counter-clockwise direction. For triangular elements $KX(e,4)$ must be equal to $KX(e,3)$ .
3	Real	$Angle(e)$	Angle in degrees between $K_1^A$ and the $x$ -coordinate axis assigned to each element $e$ .
3	Real	$ConA1(e)$	First principal component, $K_1^A$ , of the dimensionless tensor $\mathbf{K}^A$ which describes the local anisotropy of the hydraulic conductivity assigned to element $e$ .
3	Real	$ConA2(e)$	Second principal component, $K_2^A$ .
3	Integer	$LayNum(e)$	Subregion number assigned to element $e$ .
In general, record 3 information is required for each element $e$ , starting with $e=1$ and continuing sequentially until $e=NumEl$ . Record 3 information for certain elements may be skipped if several conditions are satisfied (see beginning of this section).			

Table 9.11. Block K - Boundary geometry information.

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Integer	<i>KXB(1)</i>	Global node number of the first of a set of sequentially numbered boundary nodes for which <i>Kode(n)</i> is not equal to zero.
3	Integer	<i>KXB(2)</i>	As above for the second boundary node.
.	.	.	.
3	Integer	<i>KXB(NumBP)</i>	As above for the last boundary node.
4	-	-	Comment line.
5	Real	<i>Width(1)</i>	Width of the boundary [L] associated with boundary node <i>KXB(1)</i> . <i>Width(n)</i> includes half the boundary length of each element connected to node <i>KXB(n)</i> along the boundary. The type of boundary condition assigned to <i>KXB(n)</i> is determined by the value of <i>Kode(n)</i> . In case of axisymmetric flow, <i>Width(n)</i> represents the area of the boundary strip [ $L^2$ ] associated with node <i>KXB(n)</i> , and along a horizontal boundary should be calculated as
			$Width(j) = \frac{\pi}{3} [(x_{j-1} + 2x_j)(x_j - x_{j-1}) + (x_{j+1} + 2x_j)(x_{j+1} - x_j)]$
			If a unit vertical hydraulic gradient or a deep drainage boundary condition is specified at node <i>n</i> , then <i>Width(n)</i> represents only the horizontal component of the boundary.
5	Real	<i>Width(2)</i>	As above for node <i>KXB(2)</i> .
.	.	.	.
5	Real	<i>Width(NumBP)</i>	As above for node <i>KXB(NumBP)</i> .
6	-	-	Comment line.
7	Real	<i>rLen</i>	Width of soil surface associated with transpiration [L]; represents surface area [ $L^2$ ] in case of axisymmetrical flow. Set <i>rLen</i> equal to zero for problems without transpiration.
8	-	-	Comment line.
9	Integer	<i>Node(1)</i>	Global node number of the first observation node for which values of the pressure head, the water content, temperature (for <i>lTemp</i> = .true.), and the solution and sorbed concentrations (for <i>lChem</i> = .true.) are printed at each time level. It does not have to be specified if <i>NObs</i> = 0.
9	Integer	<i>Node(2)</i>	Same as above for the second observation node. It does not have to be specified if <i>NObs</i> < 2.
.	.	.	.
9	Integer	<i>Node(NObs)</i>	Same as above for the last observation node.

Table 9.12. Block L - Atmospheric information.<sup>†</sup>

Record	Type	Variable	Description
1,2,3,4	-	-	Comment lines.
5	Logical	<i>SinkF</i>	Set this variable equal to .true. if water extraction from the root zone is imposed.
5	Logical	<i>qGWL</i>	Set this variable equal to .true. if the discharge-groundwater level relationship $q(GWL)$ given by equation (7.1) is used as the bottom boundary condition; $GWL = h - GWL0L$ , where $h$ is the pressure head at the boundary.
6	-	-	Comment line.
7	Real	<i>GWLOL</i>	Reference position of groundwater table (usually the z-coordinate of the soil surface).
7	Real	<i>Aqh</i>	Value of the parameter $A_{qh}$ [ $LT^{-1}$ ] in the $q(GWL)$ -relationship (equation (7.1)); set to zero if <i>qGWL</i> = .false.
7	Real	<i>Bqh</i>	Value of the parameter $B_{qh}$ [ $L^{-1}$ ] in the $q(GWL)$ -relationship (equation (7.1)); set to zero if <i>qGWL</i> = .false.
8	-	-	Comment line.
9	Real	<i>tInit</i>	Starting time [T] of the simulation.
9	Integer	<i>MaxAl</i>	Number of atmospheric data records.
10	-	-	Comment line.
11	Real	<i>hCritS</i>	Maximum allowed pressure head at the soil surface [L].
12	-	-	Comment line.
13	Real	<i>tAtm(i)</i>	Time for which the <i>i</i> -th data record is provided [T].
13	Real	<i>Prec(i)</i>	Precipitation [ $LT^{-1}$ ] (in absolute value).
13	Real	<i>rSoil(i)</i>	Potential evaporation rate [ $LT^{-1}$ ] (in absolute value).
13	Real	<i>rRoot(i)</i>	Potential transpiration rate [ $LT^{-1}$ ] (in absolute value).
13	Real	<i>hCritA(i)</i>	Absolute value of the minimum allowed pressure head at the soil surface [L].
13	Real	<i>rGWL(i)</i>	Drainage flux [ $LT^{-1}$ ] across the bottom boundary, or other time-dependent prescribed flux boundary condition (positive when water leaves the flow region), for nodes where <i>Kode(n)</i> = -3; set to zero when no <i>Kode(n)</i> = -3 boundary condition is specified.
13	Real	<i>GWL(i)</i>	Groundwater level [L] (usually negative), or other time-dependent prescribed head boundary condition, for nodes where <i>Kode(n)</i> = +3; set equal to zero when no <i>Kode(n)</i> = +3 is specified. The prescribed value of the pressure head is $h = GWL + GWL0L$ .

Table 9.12. (continued)

Record	Type	Variable	Description
13	Real	<i>Th3(i)</i>	Time-dependent temperature [K] prescribed for nodes where <i>Kode(n)</i> = $\pm 3$ (must not be specified if <i>lTemp</i> = .false. and <i>lChem</i> = .true.; set equal to zero when no <i>Kode(n)</i> = $\pm 3$ or when the flux <i>rGWL(i)</i> is directed out of the flow domain).
13	Real	<i>Th4(i)</i>	Time-dependent temperature [K] prescribed for nodes where <i>Kode(n)</i> = $\pm 4$ (must not be specified if <i>lTemp</i> = .false. and <i>lChem</i> = .true.; set equal to zero when no <i>Kode(n)</i> = $\pm 4$ ).
13	Real	<i>cPrec(1,i)</i>	Solute concentration of rainfall water [ $ML^{-3}$ ] (does not need to be specified if <i>lChem</i> = .false.).
13	Real	<i>crt(1,i)</i>	Time-dependent concentration of the drainage flux [ $ML^{-3}$ ], or some other time-dependent prescribed flux, for nodes where <i>Kode(n)</i> = $\pm 3$ and <i>KodCB(n) &lt; 0</i> (does not need to be specified if <i>lChem</i> = .false.; set equal to zero when no <i>Kode(n)</i> = $\pm 3$ and <i>KodCB(n) &lt; 0</i> , or when the flux <i>rGWL(i)</i> is directed out of the flow domain).
13	Real	<i>cht(1,i)</i>	Time-dependent concentration [ $ML^{-3}$ ] for the first-type boundary condition prescribed for nodes for which <i>Kode(n)</i> = $\pm 3$ and <i>KodCB(n) &gt; 0</i> (does not need to be specified if <i>lChem</i> = .false.; set equal to zero when no <i>Kode(n)</i> = $\pm 3$ and <i>KodCB(n) &gt; 0</i> ).
13	Real	<i>cPrec(2,i)</i>	Same as <i>cPrec(1,i)</i> for the second solute (does not need to be specified if <i>lChem</i> = .false. or <i>NS &lt; 2</i> ).
13	Real	<i>crt(2,i)</i>	Same as <i>crt(1,i)</i> for the second solute (does not need to be specified if <i>lChem</i> = .false. or <i>NS &lt; 2</i> ).
13	Real	<i>cht(2,i)</i>	Same as <i>cht(1,i)</i> for the second solute (does not need to be specified if <i>lChem</i> = .false. or <i>NS &lt; 2</i> ). The last three entries are entered for each solute from 1 to <i>NS</i> . The total number of atmospheric data records is <i>MaxAl</i> ( <i>i</i> =1,2,.., <i>MaxAl</i> ).

<sup>†</sup> Block L is not read in if the logical variable *AtmInf* (Block A) is set equal to .false. .

## 9.2. Example Input Files

Table 9.13. Input data for example 1 (input file 'SELECTOR.IN').

```
*** BLOCK A: BASIC INFORMATION ****
Heading
'Example 1 - Column Test'
LUnit TUnit MUnit          (units are obligatory for all input data)
'cm' 'sec' ''
Kat (0:horizontal plane, 1:axisymmetric vertical flow, 2:vertical plane)
2
MaxIt TolTh TolH (max. number of iteration and precision tolerance)
20 .0001 .1
lWat lChem Check Short Flux AtmIn SeepF Drain FreeD lTemp lWDep lEquil
t f f t t f t f f f f f f
*** BLOCK B: MATERIAL INFORMATION ****
NMat NLay hTab1 hTabN NPar
1 1 .001 200. 9
thr ths tha thm Alfa n Ks Kk thk
.02 .350 .02 .350 .0410 1.964 .000722 .000695 .2875
*** BLOCK C: TIME INFORMATION ****
dt dtMin dtMax DMul DMul2 MPL
1. .01 60. 1.1 .33 6
TPrint(1),TPrint(2),...,TPrint(MPL)           (print-time array)
60 900 1800 2700 3600 5400
*** BLOCK E: SEEPAGE INFORMATION (only if SeepF =.true.) ****
NSeep                               (number of seepage faces)
1
NSP(1),NSP(2),.....,NSP(NSeep)           (number of nodes in each s.f.)
2
NP(i,1),NP(i,2),.....,NP(i,NSP(i))     (nodal number array of i-th s.f.)
111 112
*** END OF INPUT FILE 'SELECTOR.IN' ****
```

Table 9.14. Input data for example 1 (input file 'GRID.IN').

```
*** BLOCK I: NODAL INFORMATION ****
  NumNP   NumEl    IJ    NumBP    NS    NObs
      112       55      2       4      0      0
  n  Code     x     z      h      Q      M      B    Axz    Bxz    Dxz    Temp
  1   1     0.00   61.00   0.75  0.00E+00  1  0.00   1.00   1.00   1.00   0.00
  2   1     1.00   61.00   0.75  0.00E+00  1  0.00   1.00   1.00   1.00   0.00
  3   0     0.00   60.75 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
  4   0     1.00   60.75 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
  5   0     0.00   60.50 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
  6   0     1.00   60.50 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
  7   0     0.00   60.25 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
  8   0     1.00   60.25 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
  9   0     0.00   60.00 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 10  0     1.00   60.00 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 11  0     0.00   59.50 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 12  0     1.00   59.50 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 13  0     0.00   59.00 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 14  0     1.00   59.00 -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
  .
  .
  .
 103  0     0.00   8.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 104  0     1.00   8.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 105  0     0.00   6.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 106  0     1.00   6.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 107  0     0.00   4.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 108  0     1.00   4.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 109  0     0.00   2.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 110  0     1.00   2.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 111 -2     0.00   0.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
 112 -2     1.00   0.00  -150.00 0.00E+00  1  0.00   1.00   1.00   1.00   0.00
*** BLOCK J: ELEMENT INFORMATION ****
  e   i   j   k   l Angle Aniz1 Aniz2 LayNum
  1   1   3   4   2   0.00  1.00  1.00   1
  2   3   5   6   4   0.00  1.00  1.00   1
  3   5   7   8   6   0.00  1.00  1.00   1
  4   7   9   10  8   0.00  1.00  1.00   1
  5   9   11  12  10  0.00  1.00  1.00   1
  6   11  13  14  12  0.00  1.00  1.00   1
  7   13  15  16  14  0.00  1.00  1.00   1
  8   15  17  18  16  0.00  1.00  1.00   1
  9   17  19  20  18  0.00  1.00  1.00   1
 10  19  21  22  20  0.00  1.00  1.00   1
  .
  .
  .
 48   95   97   98   96  0.00  1.00  1.00   1
 49   97   99  100   98  0.00  1.00  1.00   1
 50   99  101  102  100  0.00  1.00  1.00   1
 51  101  103  104  102  0.00  1.00  1.00   1
 52  103  105  106  104  0.00  1.00  1.00   1
 53  105  107  108  106  0.00  1.00  1.00   1
 54  107  109  110  108  0.00  1.00  1.00   1
 55  109  111  112  110  0.00  1.00  1.00   1
*** BLOCK K: BOUNDARY GEOMETRY INFORMATION ****
Node number array:
  1     2     111    112
Width array:
  0.50    0.50    0.50    0.50
Length:
  0.00
*** END OF INPUT FILE 'GRID.IN' ****
```

Table 9.15. Input data for example 2 (input file 'SELECTOR.IN').

```
*** BLOCK A: BASIC INFORMATION ****
Heading
'Example 2 - Grass Field Problem (Hupselse Beek 1982)'
LUnit TUnit MUnit (indicated units are obligatory for all input data)
'cm' 'day' '-'
Kat (0:horizontal plane, 1:axisymmetric vertical flow, 2:vertical plane)
2
MaxIt TolTh TolH (max. number of iteration and precision tolerances)
20 .0001 .1
lWat lChem Check Short Flux Atmin SeepF Drain FreeD lTemp lWDep lEquil
t f f t t f f f f f f t
*** BLOCK B: MATERIAL INFORMATION ****
NMat NLay hTab1 hTabN NPar
2 2 .001 1000. 9
thr ths tha thm Alfa n Ks Kk thk
.0001 .399 .0001 .399 .0174 1.3757 29.75 29.75 .399
.0001 .339 .0001 .339 .0139 1.6024 45.34 45.34 .339
*** BLOCK C: TIME INFORMATION ****
dt dtMin dtMax DMul DMul2 MPL
.02 1e-10 0.50 1.3 .3 6
TPrint(1),TPrint(2),...,TPrint(MPL) (print-time array)
120 151 181 212 243 273
*** BLOCK D: SINK INFORMATION ****
P0 P2H P2L P3 r2H r2L
-10. -200. -800. -8000. 0.5 0.1
POptm(1),POptm(2),...,POptm(NMat)
-25. -25.
*** END OF INPUT FILE 'SELECTOR.IN' ****
```

Table 9.16. Input data for example 2 (input file 'ATMOSPH.IN').

\*\*\* BLOCK L: ATMOSPHERIC INFORMATION  
 \*\*\* Hupselse Beek 1982  
 \*\*\*\*  
 SinkF qGWL F  
 t t  
 GWLOL Aqh Bqh  
 230 -.1687 -.02674 (if qGWL F=f then Aqh=Bqh=0)  
 tInit MaxAL (MaxAL = number of atmospheric data-records)  
 90. 183  
 hCrits 1.e30 (max. allowed pressure head at the soil surface)  
 rAtm Prec rSoil rRoot hCritA rt ht  
 91 0 0 0.16 1000000 0 0  
 92 0.07 0 0.18 1000000 0 0  
 93 0.02 0 0.13 1000000 0 0  
 94 0 0 0.20 1000000 0 0  
 95 0 0 0.28 1000000 0 0  
 96 0.07 0 0.18 1000000 0 0  
 97 0.29 0 0.08 1000000 0 0  
 98 0.44 0 0.14 1000000 0 0  
 99 0.20 0 0.11 1000000 0 0  
 100 0.29 0 0.11 1000000 0 0  
 101 0.32 0 0.11 1000000 0 0  
 102 0.49 0 0.11 1000000 0 0  
 103 0.01 0 0.16 1000000 0 0  
 104 0 0 0.17 1000000 0 0  
 105 0 0 0.22 1000000 0 0  
 106 0 0 0.21 1000000 0 0  
 107 0 0 0.23 1000000 0 0  
 108 0 0 0.23 1000000 0 0  
 109 0 0 0.24 1000000 0 0  
 110 0 0 0.18 1000000 0 0  
 111 0 0 0.15 1000000 0 0  
 112 0 0 0.19 1000000 0 0  
 113 0.01 0 0.15 1000000 0 0  
 114 0.01 0 0.22 1000000 0 0  
 115 0 0 0.23 1000000 0 0  
 116 0.02 0 0.20 1000000 0 0  
 117 0 0 0.17 1000000 0 0  
 118 0.02 0 0.14 1000000 0 0  
 119 0.26 0 0.13 1000000 0 0  
 120 0.24 0 0.11 1000000 0 0  
 . . . . . .  
 256 0 0 0.13 1000000 0 0  
 257 0 0 0.14 1000000 0 0  
 258 0 0 0.20 1000000 0 0  
 259 0 0 0.14 1000000 0 0  
 260 0 0 0.19 1000000 0 0  
 261 0 0 0.14 1000000 0 0  
 262 0 0 0.20 1000000 0 0  
 263 0.35 0 0.23 1000000 0 0  
 264 0.52 0 0.16 1000000 0 0  
 265 0 0 0.21 1000000 0 0  
 266 0 0 0.19 1000000 0 0  
 267 0 0 0.18 1000000 0 0  
 268 0 0 0.18 1000000 0 0  
 269 0.53 0 0.09 1000000 0 0  
 270 0.07 0 0.23 1000000 0 0  
 271 0 0 0.17 1000000 0 0  
 272 0 0 0.22 1000000 0 0  
 273 1.04 0 0 1000000 0 0

Table 9.17. Input data for example 2 (input file 'GRID.IN').

Table 9.18. Input data for example 3b (input file 'SELECTOR.IN').

```
*** BLOCK A: BASIC INFORMATION ****
Heading
'Example 3b - Comparison with the 2-D Analytical Solution'
LUnit TUnit MUnit          (indicated units are obligatory for all input data)
'm' 'days' ''
Kat      (0:horizontal plane, 1:axisymmetric vertical flow, 2:vertical plane)
2
MaxIt TolTh TolH          (maximum number of iteration and precision tolerances)
20 .0001 .1
lWat lChem CheckF ShortF FluxF AtmInF SeepF DrainF FreeD lTemp lWDep lEquil
f t f t f t f f f f t
*** BLOCK B: MATERIAL INFORMATION ****
NMat NLay hTab1 hTabN NPar
1 1 .001 200. 9
thr ths tha thm Alfa n Ks Kk thk
.02 .300 .02 .30 .0410 1.964 .3 .3 .30
*** BLOCK C: TIME INFORMATION ****
dt dtMin dtMax DMul DMul2 MPL
1 .0001 100. 1.3 .33 3
TPrint(1),TPrint(2),...,TPrint(MPL)          (print-time array)
50 100 365
*** BLOCK E: SEEPAGE INFORMATION (only if SeepF =.true.) ****
NSeep          (number of seepage faces)
1
NSP(1),NSP(2),.....,NSP(NSeep)          (number of nodes in each seepage faces)
15
NP(i,1),NP(i,2),.....,NP(i,NSP(i))          (nodal number array of i-th seepage faces)
301 302 303 304 305 306 307 308 309 310 311
312 313 314 315
*** BLOCK G: SOLUTE TRANSPORT INFORMATION ****
Epsi lUpW LArtD LTDep cTolA cTolR MaxItC PeCr
0.5 f f f 0.0 0.0 1 2.
Bulk.d. DispL DispT Frac (1...NMat)
1500 1.0 0.5 1.0
Dif.w. Dif.g. ----- (1.solute)
0.0 0.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.0004 0.0 1.0 0.0 0.01 0.01 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
KodCB(1),KodCB(2),.....,KodCB(NumBP)
1 1 1 1 1 1 1
2 2 2 2 2 2 2 2
2 2 2 2 2 2 2 2
2 2 2 2 2 2 2 2
cTop cBot
1. 0. 0. 0. 0. 0. 0. 0.
tPulse
366
*** END OF INPUT FILE 'SELECTOR.IN' ****
```

Table 9.19. Input data for example 3 (input file 'GRID.IN').

```
*** BLOCK I: NODAL INFORMATION ****
NumNP NumEl IJ NumBP NS NObs
  315   280   15    30   1   0
n  Code   x     z     h     Q     M     B   Axz   Bxz   Dxz   Temp   Conc
  1   1     0.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  1.00E+00
  2   1    10.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  1.00E+00
  3   1    20.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  1.00E+00
  4   1    30.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  1.00E+00
  5   1    40.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  1.00E+00
  6   1    45.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  1.00E+00
  7   1    49.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  1.00E+00
  8   1    51.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
  9   1    55.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 10  1    60.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 11  1    67.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 12  1    75.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 13  1    85.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 14  1   100.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 15  1   120.00  0.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 16  0     0.00 -5.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 17  0    10.00 -5.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 18  0    20.00 -5.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 19  0    30.00 -5.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 20  0    40.00 -5.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 21  0    45.00 -5.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 22  0    49.00 -5.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
  .      .      .      .      .      .      .      .      .      .      .
  .      .      .      .      .      .      .      .      .      .      .
 310  2    60.00 -200.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 311  2    67.00 -200.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 312  2    75.00 -200.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 313  2    85.00 -200.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 314  2   100.00 -200.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
 315  2   120.00 -200.00  0.00  0.00E+00  1   0.00  1.00  1.00  0.00  0.00E+00
*** BLOCK J: ELEMENT INFORMATION ****
e i j k l Angle Aniz1 Aniz2 LayNum
  1 1 16 17 2 0.00 1.00 1.00 1
  2 2 17 18 3 0.00 1.00 1.00 1
  3 3 18 19 4 0.00 1.00 1.00 1
  4 4 19 20 5 0.00 1.00 1.00 1
  5 5 20 21 6 0.00 1.00 1.00 1
  6 6 21 22 7 0.00 1.00 1.00 1
  7 7 22 23 8 0.00 1.00 1.00 1
  .      .      .      .      .
 277 296 311 312 297 0.00 1.00 1.00 1
 278 297 312 313 298 0.00 1.00 1.00 1
 279 298 313 314 299 0.00 1.00 1.00 1
 280 299 314 315 300 0.00 1.00 1.00 1
*** BLOCK K: BOUNDARY GEOMETRY INFORMATION ****
Node number array:
  1      2      3      4      5      6      7
  8      9     10     11     12     13     14
 15     301    302    303    304    305    306
 307    308    309    310    311    312    313
 314    315
Width array:
  5.00   10.00   10.00   10.00   7.50   4.50   3.00
  3.00   4.50    6.00    7.50    9.00   12.50  17.50
 10.00   5.00   10.00   10.00   10.00   7.50   4.50
  3.00   3.00   4.50    6.00    7.50   9.00   12.50
 17.50   10.00
Length:
  0.00
*** END OF INPUT FILE 'GRID.IN' ****
```

Table 9.19. Input data for example 4 (input file 'SELECTOR.IN').

```
*** BLOCK A: BASIC INFORMATION ****
Heading
'Example 4 - Solute Transport with Nitrification Chain'
LUnit TUnit MUnit          (indicated units are obligatory for all input data)
'm' 'days' '-'
Kat          (0:horizontal plane, 1:axisymmetric vertical flow, 2:vertical plane)
2
MaxIt TolTh TolH          (maximum number of iterations and precision tolerance)
20 .0001 .01
lWat lChem CheckF ShortF FluxF AtmInF SeepF DrainF FreeD lTemp lWDep lEquil
f t f t f t f f f f t
*** BLOCK B: MATERIAL INFORMATION ****
NMat Nlay hTab1 hTabN NPar
1 1 .001 200. 9
thr ths tha thm Alfa n Ks Kk thk
.0 1.0 .0 1.0 .05 2.0 1.0 1.0 1.0
*** BLOCK C: TIME INFORMATION ****
dt dtMin dtMax DMul DMul2 MPL
1 .0001 100. 1.3 .33 3
TPrint(1),TPrint(2),...,TPrint(MPL)          (print-time array)
50 100 200
*** BLOCK E: SEEPAGE INFORMATION (only if SeepF =.true.) ****
NSeep          (number of seepage faces)
1
NSP(1),NSP(2),.....,NSP(NSeep)          (number of nodes in each s.f.)
2
NP(i,1),NP(i,2),.....,NP(i,NSP(i))          (nodal number array of i-th s.f.)
401 402
*** BLOCK G: SOLUTE TRANSPORT INFORMATION ****
Epsi lUpW lArtD lTDep cToLA cToLR MaxItC PeCr
0.5 f f f 0.0 0.0 1 10
Bulk.d. DispL DispT Frac (1...NMat)
1000 0.0 0.0 1.0
Dif.w. Dif.g. ----- (1.solute)
0.18 0.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.001 0.0 1.0 0.0 0.0 0.0 0.005 0.005 0.0 0.0 0.0 0.0 0.0 0.0
Dif.w. Dif.g. ----- (2.solute)
0.18 0.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.0 0.0 1.0 0.0 0.0 0.0 0.1 0.0 0.0 0.0 0.0 0.0 0.0 0.0
Dif.w. Dif.g. ----- (3.solute)
0.18 0.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.0 0.0 1.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
KodCB(1),KodCB(2),.....,KodCB(NumBP)
-1 -1 -2 -2
cTop cBot
1. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.
0. 0. 0. 0. 0. 0. 0. 0. 0.
tPulse
366
*** END OF INPUT FILE 'SELECTOR.IN' ****
```

Table 9.21. Input data for example 4 (input file 'GRID.IN').

```

*** BLOCK I: NODAL INFORMATION ****
NumNP NumEl IJ NumBP NS NObs
 402    200   2     4    3    0

n Code x z h Q M B Axz Bxz Dxz Temp Conc1 Conc2 Conc3
1  1  0.00 0.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
2  1  1.00 0.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
3  0  0.00 -1.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
4  0  1.00 -1.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
5  0  0.00 -2.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
6  0  1.00 -2.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
7  0  0.00 -3.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
8  0  1.00 -3.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
9  0  0.00 -4.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
10 0  1.00 -4.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
11 0  0.00 -5.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
12 0  1.00 -5.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
13 0  0.00 -6.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
14 0  1.00 -6.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00

. . .
. . .
. . .

395 0  0.00 -197.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
396 0  1.00 -197.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
397 0  0.00 -198.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
398 0  1.00 -198.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
399 0  0.00 -199.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
400 0  1.00 -199.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
401 2  0.00 -200.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
402 2  1.00 -200.00 0.00 0.00E+00 1  0.00 1.00 1.00 1.00 0.00 0.000E+00 0.000E+00 0.000E+00
*** BLOCK J: ELEMENT INFORMATION ****
e i j k l Angle Aniz1 Aniz2 LayNum
1 1 3 4 2 0.00 1.00 1.00 1
2 3 5 6 4 0.00 1.00 1.00 1
3 5 7 8 6 0.00 1.00 1.00 1
4 7 9 10 8 0.00 1.00 1.00 1
5 9 11 12 10 0.00 1.00 1.00 1
6 11 13 14 12 0.00 1.00 1.00 1
7 13 15 16 14 0.00 1.00 1.00 1
8 15 17 18 16 0.00 1.00 1.00 1
9 17 19 20 18 0.00 1.00 1.00 1
10 19 21 22 20 0.00 1.00 1.00 1

. .
. .
. .

190 379 381 382 380 0.00 1.00 1.00 1
191 381 383 384 382 0.00 1.00 1.00 1
192 383 385 386 384 0.00 1.00 1.00 1
193 385 387 388 386 0.00 1.00 1.00 1
194 387 389 390 388 0.00 1.00 1.00 1
195 389 391 392 390 0.00 1.00 1.00 1
196 391 393 394 392 0.00 1.00 1.00 1
197 393 395 396 394 0.00 1.00 1.00 1
198 395 397 398 396 0.00 1.00 1.00 1
199 397 399 400 398 0.00 1.00 1.00 1
200 399 401 402 400 0.00 1.00 1.00 1
*** BLOCK K: BOUNDARY GEOMETRY INFORMATION ****
Node number array:
      1    2    401   402
Width array:
      0.50   0.50   0.50   0.50
Length:
      0.00
*** END OF INPUT FILE 'GRID.IN' ***

```

Table 9.22. Input data for example 5 (input file 'SELECTOR.IN').

```
*** BLOCK A: BASIC INFORMATION ****
Heading
'Example 5 - Solute Transport with Nonlinear Cation Adsorption - Freundlich isoterm'
LUnit TUnit MUnit          (indicated units are obligatory for all input data)
'cm' 'day' '-'
Kat          (0:horizontal plane, 1:axisymmetric vertical flow, 2:vertical plane)
2
MaxIt TolTh TolH          (maximum number of iterations and precision tolerances)
20 .0001 .01
lWat lChem CheckF ShortF FluxF AtminF SeepF DrainF FreeD lTemp lWDep lEquil
f t f t f t f f f f t
*** BLOCK B: MATERIAL INFORMATION ****
NMat NLay hTab1 hTabN NPar
1 1 .001 200. 9
thr ths tha thm Alfa n Ks Kk thk
.0 0.633 .0 0.633 .01 2.0 6.495 6.495 0.633
*** BLOCK C: TIME INFORMATION ****
dt dtMin dtMax DMul DMul2 MPL
.002 .002 1 1.3 .33 5
TPrint(1),TPrint(2),...,TPrint(MPL)          (print-time array)
5 10 15 20 25
*** BLOCK E: SEEPAGE INFORMATION (only if SeepF =.true.) ****
NSeep          (number of seepage faces)
1
NSP(1),NSP(2),.....,NSP(NSeep)          (number of nodes in each s.f.)
2
NP(i,1),NP(i,2),.....,NP(i,NSP(i))          (nodal number array of i-th s.f.)
87 88
*** BLOCK G: SOLUTE TRANSPORT INFORMATION ****
Epsi lUpW lArtD lTDep cTolA cTolR MaxItC PeCr
0.5 f f f .0001 .0001 20 2.
Bulk.d. Displ DispT Frac (1...NMat)
.884 2.727 0.0 1.0
Dif.w. Dif.g. ----- (1.solute)
0.0 0.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
1.687 0.0 1.6151 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
KodCB(1),KodCB(2),.....,KodCB(NumBP)
1 1 -2 -2
cTop cBot
10. 0. 0. 0. 0. 0. 0. 0.
tPulse
14.919
*** END OF INPUT FILE 'SELECTOR.IN' ****
```

Table 9.23. Input data for example 5 (input file 'GRID.IN').

Table 9.24. Input data for example 6 (input file 'SELECTOR.IN').

```
*** BLOCK A: BASIC INFORMATION ****
Heading
'Example 6 - Solute Transport with Kinetic Linear Cation Adsorption'
LUnit TUnit MUnit          (indicated units are obligatory for all input data)
'cm' 'day' '--'
Kat      (0:horizontal plane, 1:axisymmetric vertical flow, 2:vertical plane)
2
MaxIt TolTh TolH          (maximum number of iterations and precision tolerances)
20 .0001 .01
lWat lChem CheckF ShortF FluxF AtmInF SeepF DrainF FreeD lTemp lWDep lEquil
f t f t f t f f f f f f
*** BLOCK B: MATERIAL INFORMATION ****
NMat NLay hTab1 hTabN NPar
1 1 .001 200. 9
thr ths tha thm Alfa n Ks Kk thk
.0 0.445 .0 0.445 .01 2.0 17.12 17.12 0.445
*** BLOCK C: TIME INFORMATION ****
dt dtMin dtMax DMul DMul2 MPL
.01 .002 1 1.3 .33 7
TPrint(1),TPrint(2),...,TPrint(MPL)          (print-time array)
0.5 1 2.5 5 7 10 20
*** BLOCK E: SEEPAGE INFORMATION (only if SeepF =.true.) ****
NSeep          (number of seepage faces)
1
NSP(1),NSP(2),.....,NSP(NSeep)          (number of nodes in each s.f.)
2
NP(i,1),NP(i,2),.....,NP(i,NSP(i))          (nodal number array of i-th s.f.)
31 32
*** BLOCK G: SOLUTE TRANSPORT INFORMATION ****
Epsi lUpW lArtD lTDep cToLA cToLR MaxItC PeCr
0.5 f f f .0001 .0001 10 2.
Bulk.d. DispL DispT Fract (1...NMat)
1.222 0.0 0.0 0.47
Dif.w. Dif.g. ----- (1.solute)
49.0 0.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
1.14 0.0 1.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.320
KodCB(1),KodCB(2),.....,KodCB(NumBP)
-1 -1 -2 -2
cTop cBot
20. 0. 0. 0. 0. 0. 0. 0. 0.
tPulse
5.060
*** END OF INPUT FILE 'SELECTOR.IN' ****
```

Table 9.25. Input data for example 6 (input file 'GRID.IN').

```
*** BLOCK I: NODAL INFORMATION ****
NumNP NumEl IJ NumBP NS NObs
 32      15    2      4    1     1
n Code x   z   h   Q   M   B Axz Bxz Dxz Temp Conc1 Sorb1
 1  1  0.00 .00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
 2  1  1.00 .00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
 3  0  0.00 -2.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
 4  0  1.00 -2.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
 5  0  0.00 -4.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
 6  0  1.00 -4.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
 7  0  0.00 -6.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
 8  0  1.00 -6.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
 9  0  0.00 -8.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
10 0  1.00 -8.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
11 0  0.00 -10.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
12 0  1.00 -10.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
13 0  0.00 -12.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
14 0  1.00 -12.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
15 0  0.00 -14.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
16 0  1.00 -14.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
17 0  0.00 -16.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
18 0  1.00 -16.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
19 0  0.00 -18.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
20 0  1.00 -18.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
21 0  0.00 -20.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
22 0  1.00 -20.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
23 0  0.00 -22.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
24 0  1.00 -22.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
25 0  0.00 -24.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
26 0  1.00 -24.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
27 0  0.00 -26.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
28 0  1.00 -26.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
29 0  0.00 -28.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
30 0  1.00 -28.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
31 2  0.00 -30.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
32 2  1.00 -30.00 .00 .00E+00 1 .00 1.00 1.00 20.00 .000E+00 .000E+00
*** BLOCK J: ELEMENT INFORMATION ****
e i j k l Angle Aniz1 Aniz2 LayNum
 1 1 3 4 2 .00 1.00 1.00 1
 2 3 5 6 4 .00 1.00 1.00 1
 3 5 7 8 6 .00 1.00 1.00 1
 4 7 9 10 8 .00 1.00 1.00 1
 5 9 11 12 10 .00 1.00 1.00 1
 6 11 13 14 12 .00 1.00 1.00 1
 7 13 15 16 14 .00 1.00 1.00 1
 8 15 17 18 16 .00 1.00 1.00 1
 9 17 19 20 18 .00 1.00 1.00 1
10 19 21 22 20 .00 1.00 1.00 1
11 21 23 24 22 .00 1.00 1.00 1
12 23 25 26 24 .00 1.00 1.00 1
13 25 27 28 26 .00 1.00 1.00 1
14 27 29 30 28 .00 1.00 1.00 1
15 29 31 32 30 .00 1.00 1.00 1
*** BLOCK K: BOUNDARY GEOMETRY INFORMATION ****
Node number array:
 1     2     31    32
Width array:
 .50    .50    .50    .50
Length:
 0.00
Observation Nodes (2,...,NObs):
31
*** END OF INPUT FILE GRID.IN ****
```

Table 9.26. Input data for example 7 (input file 'SELECTOR.IN').

```
*** BLOCK A: BASIC INFORMATION ****
Heading
'Example 7 - Water and Solute Infiltration Test '
LUnit TUnit MUnit          (indicated units are obligatory for all input data)
'cm' 'min' '-'
Kat           (0:horizontal plane, 1:axisymmetric vertical flow, 2:vertical plane)
1
MaxIt TolTh TolH      (maximum number of iterations and precision tolerances)
20 .0001 .1
lWat lChem CheckF ShortF FluxF AtmInF SeepF DrainF FreeD lTemp lWDep lEquil
t t f t f f f f t f t
*** BLOCK B: MATERIAL INFORMATION ****
NMat NLay hTab1 hTabN NPar
2 2 .001 200. 9
thr ths tha thm Alfa n Ks Kk thk
.0001 .399 .0001 .399 .0174 1.3757 .0207 .0207 .399
.0001 .399 .0001 .399 .0139 1.6024 .0315 .0315 .339
*** BLOCK C: TIME INFORMATION ****
dt dtMin dtMax DMul DMul2 MPL
.1 .001 1000. 1.33 .33 6
TPrint(1),TPrint(2),...,TPrint(MPL)          (print-time array)
720 1440 3600 7200 10800 14400
*** BLOCK G: SOLUTE TRANSPORT INFORMATION ****
Epsi lUpW lArtD lTDep cTolA cTolR MaxItC PeCr
0.5 t f t 0.0 0.0 1 2.
Bulk.d. DispL DispT Frac (1...NMat)
1.30 0.5 0.1 1.0
1.30 0.5 0.1 1.0
Dif.w. Dif.g. ----- (1.solute)
0.026 3.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.1 0.0 1.0 1.33e-7 1.39e-4 0.0 0.0 2.5e-4 0.0 0.0 0.0 0.0 0.0 0.0
0.1 0.0 1.0 1.33e-7 1.39e-4 0.0 0.0 2.5e-4 0.0 0.0 0.0 0.0 0.0 0.0
Dif.w. Dif.g. ----- (2.solute)
0.026 3.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.05 0.0 1.0 0.0 6.94e-6 0.0 0.0 1.67e-5 0.0 0.0 0.0 0.0 0.0 0.0
0.05 0.0 1.0 0.0 6.94e-6 0.0 0.0 1.67e-5 0.0 0.0 0.0 0.0 0.0 0.0
Dif.w. Dif.g. ----- (3.solute)
0.026 3.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.2 0.0 1.0 1.33e-3 3.47e-6 0.0 0.0 1.67e-6 0.0 0.0 0.0 0.0 0.0 0.0
0.2 0.0 1.0 1.33e-3 3.47e-6 0.0 0.0 1.67e-6 0.0 0.0 0.0 0.0 0.0 0.0
Temperature dependence
Dif.w. Dif.g. ----- (1.solute)
0.0 0.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.0 0.0 0.0 0.0 51213. 0.0 0.0 81171 0.0 0.0 0.0 0.0 0.0 0.0
Dif.w. Dif.g. ----- (2.solute)
0.0 0.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.0 0.0 0.0 0.0 51213. 0.0 0.0 81171 0.0 0.0 0.0 0.0 0.0 0.0
Dif.w. Dif.g. ----- (3.solute)
0.0 0.0
KS Nu Beta Henry SnkL1 SnkS1 SnkG1 SnkL1' SnkS1' SnkG1' SnkL0 SnkS0 SnkG0 Alfa
0.0 0.0 0.0 0.0 51213. 0.0 0.0 81171 0.0 0.0 0.0 0.0 0.0 0.0
KodCB(1),KodCB(2),...,KodCB(NumBP)
-1 -1 -1 -1 -7 -7 -7 -7 -7
-7 -7 -7 -7 -7 -7 -7 -7 -7
2 2
cTop cBot
1. 0. 0. 0. 0. 0. 0. 0. 0.5
0. 0. 0. 0. 0. 0. 0. 0. 0.5
0. 0. 0. 0. 0. 0. 0. 0. 0.5
tPulse
7200
```

Table 9.26. (continued)

```
*** BLOCK H: HEAT TRANSPORT INFORMATION ****
Qn   Qo   Th. Disper.   B1      B2      B3      Cn      Co      Cw
0.600  0.001  0.5    0.1  5.25e+6  8.45e+6  3.313e+7  6.912e+7  9.036e+7  1.505e+8
0.660  0.001  0.5    0.1  5.25e+6  8.45e+6  3.313e+7  6.912e+7  9.036e+7  1.505e+8
KodTB(1),KodTB(2),.....,KodTB(NumBP)
-1     -1     -1     -1     -1     -2     -2     -2     -2     -2
-2     -2     -2     -2     -2     -2     -2     -2     -2     -2
-3     -3
TBound(1),TBound(2),.....,TBound(6)
30.      0.      0.      0.      0.      0.
Amplitude tPeriod
0.      3600.
*** END OF INPUT FILE 'SELECTOR.IN' ****
```

Table 9.27. Input data for example 7 (input file 'GRID.IN').

\*\*\* BLOCK I: NODAL INFORMATION \*\*\*\*

	NumNP	NumEl	IJ	NumBP	NS	NObs								
	380	342	19	22	3	0								
n	Code	X	Z	h	Q	M	B	Axz	Bxz	Dxz	Temp	Conc1	Conc2	Conc3
1	1	0.00	230.00	0.00	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
2	0	0.00	228.00	-145.50	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
3	0	0.00	226.00	-143.40	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
4	0	0.00	224.00	-141.00	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
5	0	0.00	220.00	-135.60	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
6	0	0.00	215.00	-127.70	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
7	0	0.00	210.00	-119.00	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
8	0	0.00	205.00	-109.90	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
9	0	0.00	200.00	-100.50	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
10	0	0.00	190.00	-82.80	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
11	0	0.00	180.00	-71.00	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
12	0	0.00	170.00	-60.30	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
13	0	0.00	160.00	-49.80	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
14	0	0.00	150.00	-39.60	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
15	0	0.00	140.00	-29.50	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
16	0	0.00	130.00	-19.40	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
17	0	0.00	120.00	-9.40	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
18	0	0.00	110.00	0.60	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
19	0	0.00	100.00	10.20	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
20	1	5.00	230.00	0.00	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
21	0	5.00	228.00	-145.50	0.000E+00	1	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
373	0	125.00	170.00	-60.30	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
374	0	125.00	160.00	-49.80	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
375	0	125.00	150.00	-39.60	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
376	0	125.00	140.00	-29.50	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
377	0	125.00	130.00	-19.40	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
378	0	125.00	120.00	-9.40	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
379	5	125.00	110.00	0.60	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
380	5	125.00	100.00	10.20	0.000E+00	2	0.0	1.0	1.0	1.0	20.0	0.000E+00	0.000E+00	0.000E+00
e	i	j	k	l	Angle	Aniz1	Aniz2	LayNum						
1	2	21	20	1	0.00	1.00	1.00	1						
2	3	22	21	2	0.00	1.00	1.00	1						
3	4	23	22	3	0.00	1.00	1.00	1						
4	5	24	23	4	0.00	1.00	1.00	1						
5	6	25	24	5	0.00	1.00	1.00	1						
6	7	26	25	6	0.00	1.00	1.00	1						
7	8	27	26	7	0.00	1.00	1.00	1						
.	.	.	.	.	.	.	.	.						
336	355	374	373	354	0.00	1.00	1.00	2						
337	356	375	374	355	0.00	1.00	1.00	2						
338	357	376	375	356	0.00	1.00	1.00	2						
339	358	377	376	357	0.00	1.00	1.00	2						
340	359	378	377	358	0.00	1.00	1.00	2						
341	360	379	378	359	0.00	1.00	1.00	2						
342	361	380	379	360	0.00	1.00	1.00	2						
1	20	39	58	77	96	115								
134	153	172	191	210	229	248								
267	286	305	324	343	362	379								
380														
26.18	157.08	273.32	300.55	261.80	238.76	263.89								
366.52	579.10	857.66	1221.04	1688.09	2277.66	3008.61								
3899.77	4970.01	6238.17	7723.10	9443.65	5292.55	7853.98								
3926.99														
0.00														



## 10. OUTPUT DATA

The program output consists of  $17 + (n_s - 1)$  output files organized into 3 groups:

**T-level information**

H\_MEAN.OUT  
V\_MEAN.OUT  
CUM\_Q.OUT  
RUN\_INF.OUT  
SOLUTE.OUT  
OBSNOD.OUT

**P-level information**

H.OUT  
TH.OUT  
CONC.OUT  
TEMP.OUT  
Q.OUT  
VX.OUT  
VZ.OUT  
BOUNDARY.OUT  
BALANCE.OUT

**A-level information**

A\_LEVEL.OUT

In addition, some of the input data are printed to file CHECK.OUT. A separate output file SOLUTE.OUT is created for each solute. All output files are directed to subdirectory CHAIN\_2D.OUT, which must be created by the user prior to program execution. The various output files are described in detail in Section 10.1. Section 10.2 lists selected output files for examples 1 through 7 (see Section 8). The input files for these examples were discussed in Section 9.2.

### 10.1. *Description of Data Output Files*

The file CHECK.OUT contains a complete description of the finite element mesh, the boundary code of each node, and the hydraulic and transport properties of each soil material. Finite element mesh data are printed only when the logical variable *CheckF* in

input Block A (Table 9.1) is set equal to .true..

*T-level information* - This group of output files contains information which is printed at the end of each time step. Printing can be suppressed by setting the logical variable *ShortF* in input Block A equal to .true.; the information is then printed only at selected print times. Output files printed at the T-level are described in Tables 10.1 through 10.5. Output file OBSNOD.OUT brings the information about the time change of the pressure head, water content, temperature, and solution and sorbed concentrations, in specified observation nodes.

*P-level information* - P-level information is printed only at prescribed print times. The following output files are printed at the P-level:

H.OUT	Nodal values of the pressure head
TH.OUT	Nodal values of the water content
CONC.OUT	Nodal values of the solution and sorbed concentrations
TEMP.OUT	Nodal values of the temperature
Q.OUT	Discharge/recharge rates assigned to boundary or internal sink/source nodes
VX.OUT	Nodal values of the x-components of the Darcian flux vector
VZ.OUT	Nodal values of the z-components of the Darcian flux vector
BOUNDARY.OUT	This file contains information about each boundary node, $n$ , for which $Kode(n) \neq 0$ , including the discharge/recharge rate, $Q(n)$ , the boundary flux, $q(n)$ , the pressure head $h(n)$ , the water content $\theta(n)$ , the temperature $Temp(n)$ , and the concentration $Conc(n_s, n)$ .
BALANCE.OUT	This file gives the total amount of water, heat and solute inside each specified subregion, the inflow/outflow rates to/from that subregion, together with the mean pressure head ( $hMean$ ), mean temperature ( $TMean$ ) and the mean concentration ( $cMean$ ) over each subregion (see Table 10.6). Absolute and relative errors in the water and solute mass balances are also printed to this file.

The output files H.OUT, TH.OUT, CONC.OUT, TEMP.OUT, Q.OUT, VZ.OUT and VX.OUT provide printed tables of the specific variables. To better identify the output, each printed line starts with the nodal number and spatial coordinates of the first node on that line for which information is printed. Users can easily reprogram the original

subroutines to restructure the output for their specific needs.

*A-level information* - A-level information is printed each time a time-dependent boundary condition is specified. The information is directed to output file **A\_LEVEL.OUT** (Table 10.7).

Table 10.1. H\_MEAN.OUT - mean pressure heads.

---

<i>hAtm</i>	Mean valuee of the pressure head calculated over a set of nodes for which <i>Kode(n)</i> = ±4 (i.e., along part of a boundary controlled by atmospheric conditions) [L].
<i>hRoot</i>	Mean value of the pressure head over a region for which <i>Beta(n)</i> >0 (i.e., within the root zone) [L].
<i>hKode3</i>	Mean value of the pressure head calculated over a set of nodes for which <i>Kode(n)</i> = ±3 (i.e., along part of a boundary where the groundwater level, the bottom flux, or other time-dependent pressure head and/or flux is imposed) [L].
<i>hKode1</i>	Mean value of the pressure head calculated over a set of nodes for which <i>Kode(n)</i> = ±1 (i.e., along part of a boundary where time-independent pressure heads and/or fluxes are imposed) [L].
<i>hSeep</i>	Mean value of the pressure head calculated over a set of nodes for which <i>Kode(n)</i> = ±2 (i.e., along seepage faces) [L].
<i>hKode5</i>	Mean value of the pressure head calculated over a set of nodes for which <i>Kode(n)</i> = ±5 [L].
⋮	⋮
<i>hKodeN</i>	Mean value of the pressure head calculated over a set of nodes for which <i>Kode(n)</i> = ± <i>NumKD</i> [L].

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**Table 10.2. V\_MEAN.OUT - mean and total water fluxes.<sup>‡</sup>**

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<i>rAtm</i>	Potential surface flux per unit atmospheric boundary ( <i>Kode(n)</i> = ±4) [LT <sup>-1</sup> ].
<i>rRoot</i>	Potential transpiration rate, <i>T<sub>p</sub></i> [LT <sup>-1</sup> ].
<i>vAtm</i>	Mean value of actual surface flux per unit atmospheric boundary ( <i>Kode(n)</i> = ±4) [LT <sup>-1</sup> ].
<i>vRoot</i>	Actual transpiration rate, <i>T<sub>a</sub></i> [LT <sup>-1</sup> ].
<i>vKode3</i>	Total value of the bottom or other flux across part of a boundary where the groundwater level, the bottom flux, or other time-dependent pressure head and/or flux is imposed ( <i>Kode(n)</i> = ±3), [L <sup>2</sup> T <sup>-1</sup> ] or [L <sup>3</sup> T <sup>-1</sup> ] <sup>†</sup> .
<i>vKode1</i>	Total value of the boundary flux across part of a boundary where time-independent pressure heads and/or fluxes are imposed, including internal sinks/sources ( <i>Kode(n)</i> = ±1), [L <sup>2</sup> T <sup>-1</sup> ] or [L <sup>3</sup> T <sup>-1</sup> ].
<i>vSeep</i>	Total value of the boundary flux across a potential seepage face ( <i>Kode(n)</i> = ±2), [L <sup>2</sup> T <sup>-1</sup> ] or [L <sup>3</sup> T <sup>-1</sup> ].
<i>vKode5</i>	Total value of the flux across a boundary containing nodes for which <i>Kode(n)</i> = ±5, [L <sup>2</sup> T <sup>-1</sup> ] or [L <sup>3</sup> T <sup>-1</sup> ].
⋮	⋮
<i>vKodeN</i>	Total value of the flux across a boundary containing nodes for which <i>Kode(n)</i> = ± <i>NumKD</i> , [L <sup>2</sup> T <sup>-1</sup> ] or [L <sup>3</sup> T <sup>-1</sup> ].

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<sup>†</sup> For plane and axisymmetric flow, respectively

<sup>‡</sup> Boundary fluxes are positive when water is removed from the system.

Table 10.3. CUM\_Q.OUT - total cumulative water fluxes.<sup>‡</sup>

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<i>CumQAP</i>	Cumulative total potential surface flux across the atmospheric boundary ( <i>Kode(n)</i> = ±4), [L <sup>2</sup> ] or [L <sup>3</sup> ] <sup>†</sup> .
<i>CumQRP</i>	Cumulative total potential transpiration rate, [L <sup>2</sup> ] or [L <sup>3</sup> ] <sup>†</sup> .
<i>CumQA</i>	Cumulative total actual surface flux across the atmospheric boundary ( <i>Kode(n)</i> = ±4), [L <sup>2</sup> ] or [L <sup>3</sup> ] <sup>†</sup> .
<i>CumQR</i>	Cumulative total actual transpiration rate, [L <sup>2</sup> ] or [L <sup>3</sup> ] <sup>†</sup> .
<i>CumQ3</i>	Cumulative total value of the bottom or other boundary flux across part of a boundary where the groundwater level, the bottom flux, or other time-dependent pressure head and/or flux is imposed ( <i>Kode(n)</i> = ±3), [L <sup>2</sup> ] or [L <sup>3</sup> ] <sup>†</sup> .
<i>CumQ1</i>	Cumulative total value of the flux across part of a boundary along which time-independent pressure heads and/or fluxes are imposed, including internal sinks/sources ( <i>Kode(n)</i> = ±1), [L <sup>2</sup> ] or [L <sup>3</sup> ] <sup>†</sup> .
<i>CumQS</i>	Cumulative total value of the flux across a potential seepage faces ( <i>Kode(n)</i> = ±2), [L <sup>2</sup> ] or [L <sup>3</sup> ] <sup>†</sup> .
<i>CumQ5</i>	Cumulative total value of the flux across a boundary containing nodes for which <i>Kode(n)</i> = ±5, [L <sup>2</sup> ] or [L <sup>3</sup> ] <sup>†</sup> .
⋮	⋮
<i>CumQN</i>	Cumulative total value of the flux across a boundary containing nodes for which <i>Kode(n)</i> = ± <i>NumKD</i> , [L <sup>2</sup> ] or [L <sup>3</sup> ] <sup>†</sup> .

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<sup>†</sup> For plane and axisymmetric flow, respectively

<sup>‡</sup> Boundary fluxes are positive when water is removed from the system.

Table 10.4. RUN\_INF.OUT - time and iteration information.

<i>TLevel</i>	Time-level (current time-step number) [-].
<i>Time</i>	Time, $t$ , at current time-level [T].
<i>dt</i>	Time step, $\Delta t$ [T].
<i>IterW</i>	Number of iterations for water flow [-].
<i>IterC</i>	Number of iterations for solute transport [-].
<i>ItCum</i>	Cumulative number of iterations [-].
<i>Peclet</i>	Maximum local Peclet number [-].
<i>Courant</i>	Maximum local Courant number [-].

Table 10.5. SOLUTE.OUT - actual and cumulative concentration fluxes.<sup>‡</sup>

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<i>CumCh0</i>	Cumulative amount of solute removed from the flow region by zero-order reactions (positive when removed from the system), $[ML^{-1}]$ or $[M]^t$ .
<i>CumCh1</i>	Cumulative amount of solute removed from the flow region by first-order reactions, $[ML^{-1}]$ or $[M]^t$ .
<i>CumChR</i>	Cumulative amount of solute removed from the flow region by root water uptake $S$ , $[ML^{-1}]$ or $[M]^t$ .
<i>ChemS1</i>	Cumulative solute flux across part of a boundary along which time-independent pressure heads and/or fluxes are imposed, including internal sink/sources ( $Kode(n) = \pm 1$ ), $[ML^{-1}]$ or $[M]^t$ .
<i>ChemS2</i>	Cumulative solute flux across a potential seepage faces ( $Kode(n) = \pm 2$ ), $[ML^{-1}]$ or $[M]^t$ .
<i>ChemS3</i>	Cumulative solute flux across part of a boundary along which the groundwater level, the bottom flux, or other time-dependent pressure head and/or flux is imposed ( $Kode(n) = \pm 3$ ), $[ML^{-1}]$ or $[M]^t$ .
<i>ChemS4</i>	Cumulative total solute flux across the atmospheric boundary ( $Kode(n) = \pm 4$ ), $[ML^{-1}]$ or $[M]^t$ .
<i>ChemS5</i>	Cumulative total solute flux across an internal or external boundary containing nodes for which $Kode(n) = \pm 5$ , $[ML^{-1}]$ or $[M]^t$ .
⋮	⋮
<i>ChemSN</i>	Cumulative total solute flux across an internal or external boundary containing nodes for which $Kode(n) = \pm NumKD$ , $[ML^{-1}]$ or $[M]^t$ .
<i>qc1</i>	Total solute flux across part of a boundary along which time-independent pressure heads and/or fluxes are imposed ( $Kode(n) = \pm 1$ ), $[ML^{-1}T^{-1}]$ or $[MT^{-1}]^t$ .
<i>qc2</i>	Total solute flux across a potential seepage face ( $Kode(n) = \pm 2$ ), $[ML^{-1}T^{-1}]$ or $[MT^{-1}]^t$ .
<i>qc3</i>	Total solute flux calculated across a boundary containing nodes for which $Kode(n) = \pm 3$ (i.e., along part of a boundary where the groundwater level, the bottom flux, or other time-dependent pressure head and/or flux is specified), $[ML^{-1}T^{-1}]$ or $[MT^{-1}]^t$ .
<i>qc4</i>	Total solute flux across the atmospheric boundary ( $Kode(n) = \pm 4$ ), $[ML^{-1}T^{-1}]$ or $[MT^{-1}]^t$ .
<i>qc5</i>	Total solute flux across an internal or external boundary containing nodes for which $Kode(n) = \pm 5$ , $[ML^{-1}T^{-1}]$ or $[MT^{-1}]^t$ .
⋮	⋮
<i>qcN</i>	Total solute flux across an internal or external boundary containing nodes for which $Kode(n) = \pm NumKD$ , $[ML^{-1}T^{-1}]$ or $[MT^{-1}]^t$ .

---

<sup>†</sup> For plane and axisymmetric flow, respectively

<sup>‡</sup> Same output file is created for each solute from 1 to NS. Values of the solute flux and the cumulative solute flux are positive when solute is removed from the system.

Table 10.6. BALANCE.OUT - mass balance variables.

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<i>Area</i>	Area of the entire flow domain or a specified subregion, $[L^2]$ or $[L^3]^t$ .
<i>Volume</i>	Volume of water in the entire flow domain or in a specified subregion, $[L^3]$ or $[L^3]^t$ .
<i>InFlow</i>	Inflow/Outflow to/from the entire flow domain or to/from a specified subregion, $[L^2T^{-1}]$ or $[L^3T^{-1}]^t$ .
<i>hMean</i>	Mean pressure head in the entire flow domain or in a specified subregion [L].
<i>TVol</i>	Amount of heat in the entire flow domain or in a specified subregion, $[MLT^{-2}]$ or $[ML^2T^{-2}]^t$ .
<i>TMean</i>	Mean temperature in the entire flow domain or in a specified subregion [K].
<i>ConcVol</i>	Amount of solute in the entire flow domain or in a specified subregion. This variable is given for all solutes from 1 to $NS$ , $[ML^{-1}]$ or $[M]^t$ .
<i>cMean</i>	Mean concentration in the entire flow domain or in a specified subregion. This variable is given for all solutes from 1 to $NS$ $[ML^{-3}]$ .
<i>WatBalT</i>	Absolute error in the water mass balance of the entire flow domain, $[L^2]$ or $[L^3]^t$ .
<i>WatBalR</i>	Relative error in the water mass balance of the entire flow domain [%].
<i>CncBalT</i>	Absolute error in the solute mass balance of the entire flow domain. This variable is given for all solutes from 1 to $NS$ , $[ML^{-1}]$ or $[M]^t$ .
<i>CncBalR</i>	Relative error in the solute mass balance of the entire flow domain. This variable is given for all solutes from 1 to $NS$ [%].

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<sup>t</sup> For plane and axisymmetric flow, respectively

Table 10.7. A\_LEVEL.OUT - mean pressure heads and total cumulative fluxes.\*

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<i>CumQAP</i>	Cumulative total potential flux across the atmospheric boundary ( $Kode(n) = \pm 4$ ), $[L^2]$ or $[L^3]^{\dagger}$ .
<i>CumQRP</i>	Cumulative total potential transpiration rate, $[L^2]$ or $[L^3]^{\dagger}$ .
<i>CumQA</i>	Cumulative total actual flux across the atmospheric boundary ( $Kode(n) = \pm 4$ ), $[L^2]$ or $[L^3]^{\dagger}$ .
<i>CumQR</i>	Cumulative total actual transpiration rate, $[L^2]$ or $[L^3]^{\dagger}$ .
<i>CumQ3</i>	Cumulative total bottom or other flux across a boundary along which the groundwater level, the bottom flux, or other time-dependent pressure head and/or flux is imposed ( $Kode(n) = \pm 3$ ), $[L^2]$ or $[L^3]^{\dagger}$ .
<i>hAtm</i>	Mean value of the pressure head calculated over a set of nodes for which $Kode(n) = \pm 4$ [L].
<i>hRoot</i>	Mean value of the pressure head over a region for which $Beta(n) > 0$ (i.e., the root zone) [L].
<i>hKode3</i>	Mean value of the pressure head over a set of nodes for which $Kode(n) = \pm 3$ [L].

---

\* For plane and axisymmetric flow, respectively

† Boundary fluxes are positive when water is removed from the system.

## 10.2. Example Output Files

Table 10.8. Output data for example 1 (part of output file 'H.OUT').

Time	***	5400.0000	***	
n	x(n)	z(n)	h(n)	h(n+1) ...
1	0.0	61.0	0.8	0.8
3	0.0	60.8	0.6	0.6
5	0.0	60.5	0.4	0.4
7	0.0	60.3	0.3	0.3
9	0.0	60.0	0.1	0.1
11	0.0	59.5	-0.2	-0.2
13	0.0	59.0	-0.5	-0.5
15	0.0	58.0	-1.2	-1.2
17	0.0	57.0	-1.8	-1.8
19	0.0	56.0	-2.5	-2.5
21	0.0	55.0	-3.1	-3.1
23	0.0	54.0	-3.7	-3.7
25	0.0	53.0	-4.4	-4.4
27	0.0	52.0	-5.0	-5.0
29	0.0	51.0	-5.7	-5.7
31	0.0	50.0	-6.3	-6.3
33	0.0	49.0	-7.0	-7.0
35	0.0	48.0	-7.6	-7.6
37	0.0	47.0	-8.3	-8.3
39	0.0	46.0	-8.9	-8.9
41	0.0	45.0	-9.5	-9.5
43	0.0	44.0	-10.1	-10.1
45	0.0	43.0	-10.8	-10.8
47	0.0	42.0	-11.4	-11.4
49	0.0	41.0	-12.0	-12.0
51	0.0	40.0	-12.6	-12.6
53	0.0	39.0	-13.2	-13.2
55	0.0	38.0	-13.7	-13.7
57	0.0	37.0	-14.3	-14.3
59	0.0	36.0	-14.9	-14.9
61	0.0	35.0	-15.4	-15.4
63	0.0	34.0	-16.0	-16.0
65	0.0	33.0	-16.5	-16.5
67	0.0	32.0	-17.0	-17.0
69	0.0	31.0	-17.6	-17.6
71	0.0	30.0	-18.1	-18.1
73	0.0	29.0	-18.8	-18.8
75	0.0	28.0	-19.5	-19.5
77	0.0	27.0	-20.3	-20.3
79	0.0	26.0	-21.3	-21.3
81	0.0	25.0	-22.5	-22.5
83	0.0	24.0	-23.9	-23.9
85	0.0	23.0	-25.6	-25.6
87	0.0	22.0	-27.8	-27.9
89	0.0	21.0	-30.8	-30.8
91	0.0	20.0	-34.8	-34.9
93	0.0	18.0	-48.7	-48.6
95	0.0	16.0	-81.9	-80.2
97	0.0	14.0	-137.1	-134.4
99	0.0	12.0	-149.5	-149.4
101	0.0	10.0	-150.0	-150.0
103	0.0	8.0	-150.0	-150.0
105	0.0	6.0	-149.9	-149.9
107	0.0	4.0	-149.7	-149.7
109	0.0	2.0	-149.0	-149.0
111	0.0	0.0	-147.4	-147.5

Table 10.9. Output data for example 1 (output file 'CUM\_Q.OUT').

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**Example 1 - Column Test**

Program CHAIN\_2D

Time independent boundary conditions

Vertical plane flow,  $V = L^*L$

Units:  $L = \text{cm}$ ,  $T = \text{sec}$ ,  $M = -$

All cumulative fluxes (CumQ) are positive out of the region

Time [T]	CumQAP [V]	CumQRP [V]	CumQA [V]	CumQR [V]	CumQ3 [V]	CumQ1 [V]	CumQS [V]	CumQ5 [V]	CumQ6.. [V]
60.0000	0.000E+00	0.000E+00	0.000E+00	0.000E+00	0.000E+00	-0.796E+00	0.000E+00	0.000E+00	0.000E+00
900.0000	0.000E+00	0.000E+00	0.000E+00	0.000E+00	0.000E+00	-0.340E+01	0.000E+00	0.000E+00	0.000E+00
1800.0000	0.000E+00	0.000E+00	0.000E+00	0.000E+00	0.000E+00	-0.505E+01	0.000E+00	0.000E+00	0.000E+00
2700.0000	0.000E+00	0.000E+00	0.000E+00	0.000E+00	0.000E+00	-0.643E+01	0.000E+00	0.000E+00	0.000E+00
3600.0000	0.000E+00	0.000E+00	0.000E+00	0.000E+00	0.000E+00	-0.767E+01	0.000E+00	0.000E+00	0.000E+00
5400.0000	0.000E+00	0.000E+00	0.000E+00	0.000E+00	0.000E+00	-0.991E+01	0.000E+00	0.000E+00	0.000E+00

---

Table 10.10. Output data for example 2 (output file 'RUN\_INF.OUT').

TLevel	Time	dt	Iter	ItCum
70	.120E+03	.500E+00	2	166
132	.151E+03	.500E+00	3	335
192	.181E+03	.500E+00	3	522
254	.212E+03	.500E+00	3	673
328	.243E+03	.500E+00	2	912
388	.273E+03	.500E+00	6	1067

Table 10.11. Output data for example 2 (part of output file 'A\_LEVEL.OUT').

Program CHAIN\_2D

Time dependent boundary conditions

Vertical plane flow,  $V = L^*L$

Units:  $L = \text{cm}$ ,  $T = \text{day}$ ,  $M = -$

All cumulative fluxes (CumQ) are positive out of the region

Time [T]	CumQAP [V]	CumQRP [V]	CumQA [V]	CumQR [V]	CumQ3 [V]	hAtm [L]	hRoot [L]	hKode3 [L]	A-level
91.0000	.000E+00	.160E+00	.000E+00	.160E+00	.373E-01	-58.2	-37.1	171.9	1
92.0000	-.700E-01	.340E+00	-.700E-01	.340E+00	.722E-01	-59.5	-39.0	169.4	2
93.0000	-.900E-01	.470E+00	-.900E-01	.470E+00	.105E+00	-62.3	-40.9	167.4	3
94.0000	-.900E-01	.670E+00	-.900E-01	.670E+00	.136E+00	-66.3	-43.8	164.3	4
95.0000	-.900E-01	.950E+00	-.900E-01	.950E+00	.164E+00	-71.2	-47.9	160.3	5
96.0000	-.160E+00	.113E+01	-.160E+00	.113E+01	.190E+00	-71.2	-50.8	158.0	6
97.0000	-.450E+00	.121E+01	-.450E+00	.121E+01	.215E+00	-63.6	-49.7	159.0	7
98.0000	-.890E+00	.135E+01	-.890E+00	.135E+01	.240E+00	-57.8	-46.1	162.1	8
99.0000	-.109E+01	.146E+01	-.109E+01	.146E+01	.268E+00	-60.9	-44.1	164.0	9
100.0000	-.138E+01	.157E+01	-.138E+01	.157E+01	.298E+00	-57.8	-42.6	166.0	10
101.0000	-.170E+01	.168E+01	-.170E+01	.168E+01	.329E+00	-55.1	-40.2	168.6	11
102.0000	-.219E+01	.179E+01	-.219E+01	.179E+01	.362E+00	-48.8	-36.2	173.7	12
103.0000	-.220E+01	.195E+01	-.220E+01	.195E+01	.400E+00	-57.2	-35.3	172.4	13
104.0000	-.220E+01	.212E+01	-.220E+01	.212E+01	.435E+00	-60.8	-38.6	169.4	14
105.0000	-.220E+01	.234E+01	-.220E+01	.234E+01	.468E+00	-64.8	-42.1	165.8	15
106.0000	-.220E+01	.255E+01	-.220E+01	.255E+01	.497E+00	-68.4	-45.8	162.5	16
107.0000	-.220E+01	.278E+01	-.220E+01	.278E+01	.524E+00	-72.3	-49.4	159.1	17
108.0000	-.220E+01	.301E+01	-.220E+01	.301E+01	.549E+00	-76.0	-53.0	155.9	18
109.0000	-.220E+01	.325E+01	-.220E+01	.325E+01	.572E+00	-79.7	-56.5	152.7	19
110.0000	-.220E+01	.343E+01	-.220E+01	.343E+01	.593E+00	-82.1	-59.5	150.1	20
111.0000	-.220E+01	.358E+01	-.220E+01	.358E+01	.613E+00	-84.1	-61.7	148.0	21
112.0000	-.220E+01	.377E+01	-.220E+01	.377E+01	.631E+00	-87.0	-64.2	145.7	22
113.0000	-.221E+01	.392E+01	-.221E+01	.392E+01	.649E+00	-88.4	-66.3	143.7	23
114.0000	-.222E+01	.414E+01	-.222E+01	.414E+01	.665E+00	-91.7	-68.8	141.5	24
115.0000	-.222E+01	.437E+01	-.222E+01	.437E+01	.681E+00	-95.6	-71.8	139.0	25
116.0000	-.224E+01	.457E+01	-.224E+01	.457E+01	.695E+00	-97.0	-74.4	136.8	26
117.0000	-.224E+01	.474E+01	-.224E+01	.474E+01	.709E+00	-99.6	-76.4	134.9	27
118.0000	-.226E+01	.488E+01	-.226E+01	.488E+01	.722E+00	-99.7	-77.9	133.2	28
119.0000	-.252E+01	.501E+01	-.252E+01	.501E+01	.735E+00	-86.9	-76.1	132.6	29
120.0000	-.276E+01	.512E+01	-.276E+01	.512E+01	.747E+00	-84.2	-73.1	133.2	30
121.0000	-.337E+01	.520E+01	-.337E+01	.520E+01	.760E+00	-68.2	-67.1	135.9	31
122.0000	-.337E+01	.541E+01	-.337E+01	.541E+01	.774E+00	-89.2	-67.4	137.2	32
123.0000	-.356E+01	.555E+01	-.356E+01	.555E+01	.788E+00	-84.3	-70.1	137.4	33
124.0000	-.373E+01	.576E+01	-.373E+01	.576E+01	.802E+00	-85.9	-70.3	137.1	34
125.0000	-.451E+01	.583E+01	-.451E+01	.583E+01	.816E+00	-62.0	-63.5	140.5	35
126.0000	-.569E+01	.593E+01	-.569E+01	.593E+01	.833E+00	-48.2	-51.0	151.4	36
127.0000	-.637E+01	.603E+01	-.637E+01	.603E+01	.855E+00	-51.3	-43.6	162.7	37
128.0000	-.637E+01	.619E+01	-.637E+01	.619E+01	.884E+00	-65.3	-43.4	163.8	38
129.0000	-.637E+01	.645E+01	-.637E+01	.645E+01	.912E+00	-70.7	-47.5	160.5	39
130.0000	-.637E+01	.671E+01	-.637E+01	.671E+01	.938E+00	-75.0	-51.7	156.9	40
.	.	.	.	.	.	.	.	.	.
259.0000	-.229E+02	.420E+02	-.229E+02	.419E+02	.149E+01	-336.3	-272.9	18.8	169
260.0000	-.229E+02	.422E+02	-.229E+02	.421E+02	.149E+01	-346.2	-277.6	18.1	170
261.0000	-.229E+02	.423E+02	-.229E+02	.423E+02	.149E+01	-350.5	-282.0	17.4	171
262.0000	-.229E+02	.425E+02	-.229E+02	.425E+02	.149E+01	-361.6	-287.1	16.7	172
263.0000	-.233E+02	.427E+02	-.233E+02	.427E+02	.149E+01	-216.3	-278.6	16.0	173
264.0000	-.238E+02	.429E+02	-.238E+02	.429E+02	.149E+01	-156.9	-246.6	15.3	174
265.0000	-.238E+02	.431E+02	-.238E+02	.431E+02	.149E+01	-228.3	-229.7	14.6	175
266.0000	-.238E+02	.433E+02	-.238E+02	.433E+02	.149E+01	-257.4	-237.0	13.9	176
267.0000	-.238E+02	.435E+02	-.238E+02	.434E+02	.149E+01	-276.8	-245.1	13.2	177
268.0000	-.238E+02	.437E+02	-.238E+02	.436E+02	.149E+01	-293.1	-252.9	12.6	178
269.0000	-.243E+02	.438E+02	-.243E+02	.437E+02	.149E+01	-157.4	-236.4	12.0	179
270.0000	-.244E+02	.440E+02	-.244E+02	.439E+02	.149E+01	-206.6	-219.2	11.4	180
271.0000	-.244E+02	.442E+02	-.244E+02	.441E+02	.149E+01	-240.3	-225.3	10.8	181
272.0000	-.244E+02	.444E+02	-.244E+02	.443E+02	.149E+01	-264.1	-234.5	10.3	182
273.0000	-.254E+02	.444E+02	-.254E+02	.443E+02	.149E+01	-103.1	-203.9	9.8	183

TABLE 10.12. Output data for example 3b (part of output file 'CONC.OUT').

Time *** 365.0000 *** Species 1																	
n	x(n)	z(n)	Conc(n)	Conc(n+1)	...												
1	.0	.0	1.000	1.000	1.000	1.000	1.000	1.000	.000	.000	.000	.000	.000	.000	.000	.000	.000
16	.0	-5.0	.864	.864	.864	.864	.867	.847	.650	.226	-.009	.001	.000	.000	.000	.000	.000
31	.0	-10.0	.747	.747	.747	.748	.750	.704	.491	.265	.020	-.002	.000	.000	.000	.000	.000
46	.0	-15.0	.646	.646	.645	.647	.646	.585	.398	.253	.044	-.002	.000	.000	.000	.000	.000
61	.0	-20.0	.559	.558	.558	.560	.554	.488	.333	.228	.059	.000	.000	.000	.000	.000	.000
76	.0	-25.0	.483	.482	.482	.485	.475	.409	.282	.202	.066	.004	-.001	.000	.000	.000	.000
91	.0	-30.0	.417	.417	.417	.419	.406	.345	.240	.177	.067	.007	-.001	.000	.000	.000	.000
106	.0	-35.0	.361	.360	.360	.363	.348	.291	.205	.155	.066	.010	-.001	.000	.000	.000	.000
121	.0	-40.0	.312	.311	.312	.314	.297	.247	.176	.135	.062	.012	-.001	.000	.000	.000	.000
136	.0	-45.0	.270	.269	.269	.271	.254	.210	.151	.118	.058	.013	.000	.000	.000	.000	.000
151	.0	-50.0	.233	.232	.233	.234	.217	.178	.130	.102	.053	.014	.000	.000	.000	.000	.000
166	.0	-55.0	.204	.201	.202	.203	.186	.153	.112	.089	.048	.014	.000	.000	.000	.000	.000
181	.0	-60.0	.170	.172	.173	.173	.157	.128	.095	.076	.043	.014	.001	.000	.000	.000	.000
196	.0	-70.0	.132	.130	.131	.130	.116	.095	.071	.058	.034	.013	.001	.000	.000	.000	.000
211	.0	-80.0	.095	.097	.096	.095	.084	.069	.052	.043	.027	.011	.002	.000	.000	.000	.000
226	.0	-90.0	.085	.078	.078	.074	.064	.053	.041	.034	.022	.010	.002	.000	.000	.000	.000
241	.0	-100.0	.043	.048	.046	.044	.040	.032	.025	.021	.014	.007	.002	.000	.000	.000	.000
256	.0	-125.0	.010	.012	.013	.012	.012	.010	.007	.006	.004	.002	.001	.000	.000	.000	.000
271	.0	-150.0	.002	.002	.003	.003	.003	.002	.002	.001	.001	.001	.000	.000	.000	.000	.000
286	.0	-175.0	.000	.000	.000	.001	.001	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000
301	.0	-200.0	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000

† This output file has different formatting than the original output file 'CONC.OUT'.

TABLE 10.13. Output data for example 4 (output files 'SOLUTE1.OUT, SOLUTE2.OUT, AND SOLUTE3.OUT').

All solute fluxes (SMean) and cumulative solute fluxes (ChemS) are positive out of the region						
Time [T]	CumCh0 [VM/L3]	CumCh1 [VM/L3]	CumChr [VM/L3]	ChemS(i), i=1, NumKD [VM/L3]	SMean(j), j=1, NumKD [VM/T/L3]	
50.00	0.000E+00	0.565E+01	0.000E+00	-0.500E+02	0.000E+00	0.000E+00
100.00	0.000E+00	0.211E+02	0.000E+00	-0.100E+03	0.000E+00	0.000E+00
200.00	0.000E+00	0.733E+02	0.000E+00	-0.200E+03	0.402E-29	0.000E+00

All solute fluxes (SMean) and cumulative solute fluxes (ChemS) are positive out of the region						
Time [T]	CumCh0 [VM/L3]	CumCh1 [VM/L3]	CumChr [VM/L3]	ChemS(i), i=1, NumKD [VM/L3]	SMean(j), j=1, NumKD [VM/T/L3]	
50.00	-0.576E+01	0.386E+01	0.000E+00	0.000E+00	0.000E+00	0.000E+00
100.00	-0.213E+02	0.175E+02	0.000E+00	0.000E+00	0.399E-35	0.000E+00
200.00	-0.736E+02	0.671E+02	0.000E+00	0.000E+00	0.296E-08	0.000E+00

All solute fluxes (SMean) and cumulative solute fluxes (ChemS) are positive out of the region						
Time [T]	CumCh0 [VM/L3]	CumCh1 [VM/L3]	CumChr [VM/L3]	ChemS(i), i=1, NumKD [VM/L3]	SMean(j), j=1, NumKD [VM/T/L3]	
50.00	-0.395E+01	0.000E+00	0.000E+00	0.000E+00	0.000E+00	0.000E+00
100.00	-0.177E+02	0.000E+00	0.000E+00	0.505E-29	0.000E+00	0.000E+00
200.00	-0.674E+02	0.000E+00	0.000E+00	0.166E+00	0.000E+00	0.000E+00

TABLE 10.14. Output data for example 5 (output file 'CHECK.OUT').

Example 5 - One-Dimensional Solute Transport with Nonlinear Cation Adsorption

Program CHAIN\_2D

Time independent boundary conditions

Vertical plane flow,  $V = L^*L$

Units: L = cm , T = day , M = -

Max. number of iterations 20

Absolute water content tolerance [-] .00010

Absolute pressure head tolerance [L] .01000

MatNum	thR	thS	tha	thm	alpha	n	Ks	Kk	thk
1	.000	.633	.000	.633	.100E-01	.200E+01	.649E+01	.649E+01	.633E+00

MatNum	Qe	Q	h	C	K
1	1.000	.633	.000	.00E+00	.649E+01
1	.990	.627	-14.249	.88E-03	.477E+01
1	.900	.570	-48.432	.22E-02	.196E+01
1	.850	.538	-61.974	.24E-02	.134E+01
1	.750	.475	-88.192	.24E-02	.645E+00
1	.650	.411	-116.913	.20E-02	.302E+00
1	.500	.317	-173.205	.14E-02	.824E-01
1	.350	.222	-267.643	.73E-03	.154E-01
1	.200	.127	-489.898	.25E-03	.119E-02
1	.100	.063	-994.987	.63E-04	.516E-04

Solute transport information

=====

Galerkin finite-element method

Number of species in the chain : 1

Mat.	Bulk.D.	DispL	DispT	Fraction
1	.8840	2.7270	.0000	1.0000

Dif.w.	Dif.g.	-----	( 1.solute)											
.000E+00	.000E+00													
Mat.	KS	Nu	Beta	Henry	SinkL1	SinkS1	SinkG1	SinkL1'	SinkS1'	SinkG1'	SinkL0	SinkS0	SinkG0	Alfa
1	.17E+01	.0	.16E+01	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0	.0

Freundlich nonlinear adsorption isotherm for material 1

Conc1	Conc2	Conc3	Conc4	cSink	cWell	Conc7	gAtm	d
.100E+02	.000E+00	.0000						

tPulse = 14.919

TABLE 10.15. Output data for example 5 (part of output file 'CONC.OUT').

Time ***	25.0000 *** Species	1			
n	x(n)	z(n)	Conc(n)	Conc(n+1)	...
1	0.0	0.0	0.000E+00	0.000E+00	
3	0.0	-0.3	0.154E-01	0.144E-01	
5	0.0	-0.5	0.322E-01	0.302E-01	
7	0.0	-0.8	0.506E-01	0.475E-01	
9	0.0	-1.0	0.706E-01	0.663E-01	
11	0.0	-1.3	0.925E-01	0.869E-01	
13	0.0	-1.5	0.116E+00	0.109E+00	
15	0.0	-1.8	0.142E+00	0.134E+00	
17	0.0	-2.0	0.170E+00	0.160E+00	
19	0.0	-2.3	0.200E+00	0.189E+00	
21	0.0	-2.5	0.233E+00	0.220E+00	
23	0.0	-2.8	0.269E+00	0.253E+00	
25	0.0	-3.0	0.307E+00	0.289E+00	
27	0.0	-3.3	0.348E+00	0.328E+00	
29	0.0	-3.5	0.391E+00	0.370E+00	
31	0.0	-3.8	0.438E+00	0.414E+00	
33	0.0	-4.0	0.488E+00	0.462E+00	
35	0.0	-4.3	0.541E+00	0.513E+00	
37	0.0	-4.5	0.598E+00	0.567E+00	
39	0.0	-4.8	0.658E+00	0.624E+00	
41	0.0	-5.0	0.721E+00	0.684E+00	
43	0.0	-5.3	0.787E+00	0.747E+00	
45	0.0	-5.5	0.856E+00	0.814E+00	
47	0.0	-5.8	0.929E+00	0.884E+00	
49	0.0	-6.0	0.100E+01	0.956E+00	
51	0.0	-6.3	0.108E+01	0.103E+01	
53	0.0	-6.5	0.116E+01	0.111E+01	
55	0.0	-6.8	0.124E+01	0.119E+01	
57	0.0	-7.0	0.133E+01	0.127E+01	
59	0.0	-7.3	0.141E+01	0.135E+01	
61	0.0	-7.5	0.150E+01	0.144E+01	
63	0.0	-7.8	0.158E+01	0.152E+01	
65	0.0	-8.0	0.167E+01	0.160E+01	
67	0.0	-8.3	0.175E+01	0.168E+01	
69	0.0	-8.5	0.183E+01	0.176E+01	
71	0.0	-8.8	0.191E+01	0.184E+01	
73	0.0	-9.0	0.198E+01	0.191E+01	
75	0.0	-9.3	0.205E+01	0.198E+01	
77	0.0	-9.5	0.211E+01	0.204E+01	
79	0.0	-9.8	0.216E+01	0.209E+01	
81	0.0	-10.0	0.220E+01	0.213E+01	
83	0.0	-10.3	0.223E+01	0.217E+01	
85	0.0	-10.5	0.225E+01	0.219E+01	
87	0.0	-10.8	0.225E+01	0.220E+01	

TABLE 10.16. Output data for example 6 (part of output file 'OBSNOD.OUT').

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Node( 31)					
time	hNew	theta	Temp	Conc	Sorb
.000	.000	.4450	20.000	.000E+00	.000E+00
.010	.000	.4450	20.000	.755E-21	.726E-24
.023	.000	.4450	20.000	.313E-18	.391E-21
.040	.000	.4450	20.000	.784E-16	.129E-18
.062	.000	.4450	20.000	.981E-14	.210E-16
.091	.000	.4450	20.000	.109E-11	.311E-14
.128	.000	.4450	20.000	.677E-10	.248E-12
.175	.000	.4450	20.000	.271E-08	.126E-10
.240	.000	.4450	20.000	.142E-06	.908E-09
.327	.000	.4450	20.000	.513E-05	.443E-07
.413	.000	.4450	20.000	.537E-04	.529E-06
.500	.000	.4450	20.000	.343E-03	.380E-05
.625	.000	.4450	20.000	.317E-02	.453E-04
.750	.000	.4450	20.000	.170E-01	.282E-03
.875	.000	.4450	20.000	.648E-01	.124E-02
.937	.000	.4450	20.000	.116E+00	.230E-02
1.000	.000	.4450	20.000	.195E+00	.411E-02
1.125	.000	.4450	20.000	.471E+00	.118E-01
1.250	.000	.4450	20.000	.979E+00	.285E-01
1.375	.000	.4450	20.000	.178E+01	.600E-01
1.500	.000	.4450	20.000	.287E+01	.113E+00
1.625	.000	.4450	20.000	.422E+01	.192E+00
1.750	.000	.4450	20.000	.573E+01	.303E+00
1.874	.000	.4450	20.000	.727E+01	.445E+00
1.999	.000	.4450	20.000	.875E+01	.617E+00
2.124	.000	.4450	20.000	.101E+02	.815E+00
2.249	.000	.4450	20.000	.112E+02	.104E+01
.	.	.	.	.	.
.	.	.	.	.	.
16.496	.000	.4450	20.000	.425E+00	.968E+00
16.621	.000	.4450	20.000	.411E+00	.940E+00
16.746	.000	.4450	20.000	.398E+00	.913E+00
16.871	.000	.4450	20.000	.385E+00	.887E+00
16.996	.000	.4450	20.000	.373E+00	.861E+00
17.121	.000	.4450	20.000	.361E+00	.836E+00
17.246	.000	.4450	20.000	.349E+00	.811E+00
17.371	.000	.4450	20.000	.338E+00	.788E+00
17.496	.000	.4450	20.000	.327E+00	.765E+00
17.621	.000	.4450	20.000	.317E+00	.742E+00
17.746	.000	.4450	20.000	.306E+00	.721E+00
17.870	.000	.4450	20.000	.297E+00	.700E+00
17.995	.000	.4450	20.000	.287E+00	.679E+00
18.120	.000	.4450	20.000	.278E+00	.659E+00
18.245	.000	.4450	20.000	.269E+00	.640E+00
18.370	.000	.4450	20.000	.260E+00	.621E+00
18.495	.000	.4450	20.000	.252E+00	.603E+00
18.620	.000	.4450	20.000	.244E+00	.585E+00
18.745	.000	.4450	20.000	.236E+00	.568E+00
18.870	.000	.4450	20.000	.228E+00	.551E+00
18.995	.000	.4450	20.000	.221E+00	.535E+00
19.120	.000	.4450	20.000	.214E+00	.519E+00
19.245	.000	.4450	20.000	.207E+00	.504E+00
19.370	.000	.4450	20.000	.200E+00	.489E+00
19.494	.000	.4450	20.000	.194E+00	.474E+00
19.619	.000	.4450	20.000	.187E+00	.460E+00
19.744	.000	.4450	20.000	.181E+00	.446E+00
19.869	.000	.4450	20.000	.175E+00	.433E+00
19.935	.000	.4450	20.000	.172E+00	.426E+00
20.000	.000	.4450	20.000	.169E+00	.420E+00

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end

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TABLE 10.17. Output data for example 7 (output files 'SOLUTE1.OUT', 'SOLUTE2.OUT', AND 'SOLUTE3.OUT').

All solute fluxes (SMean) and cumulative solute fluxes (ChemS) are positive out of the region					
Time [T]	CumCh0 [VM/L3]	CumCh1 [VM/L3]	CumChR [VM/L3]	ChemS(i), i=1, NumKD [VM/L3]	SMean(j), j=1, NumKD [VM/T/L3]
720.00	.000E+00	.325E+04	.000E+00	-.223E+05	.000E+00
1440.00	.000E+00	.109E+05	.000E+00	-.423E+05	.000E+00
3600.00	.000E+00	.487E+05	.000E+00	-.102E+06	.000E+00
7200.00	.000E+00	.135E+06	.000E+00	-.201E+06	.000E+00
10800.00	.000E+00	.180E+06	.000E+00	-.201E+06	.000E+00
14400.00	.000E+00	.192E+06	.000E+00	-.201E+06	.000E+00

All solute fluxes (SMean) and cumulative solute fluxes (ChemS) are positive out of the region					
Time [T]	CumCh0 [VM/L3]	CumCh1 [VM/L3]	CumChR [VM/L3]	ChemS(i), i=1, NumKD [VM/L3]	SMean(j), j=1, NumKD [VM/T/L3]
720.00	-.224E+04	.160E+02	-.000E+00	.000E+00	.000E+00
1440.00	-.744E+04	.105E+03	-.000E+00	.000E+00	.000E+00
3600.00	-.332E+05	.115E+04	-.000E+00	.000E+00	.000E+00
7200.00	-.915E+05	.635E+04	-.000E+00	.000E+00	.000E+00
10800.00	-.122E+06	.149E+05	-.000E+00	.000E+00	.000E+00
14400.00	-.130E+06	.237E+05	-.000E+00	.000E+00	.000E+00

All solute fluxes (SMean) and cumulative solute fluxes (ChemS) are positive out of the region					
Time [T]	CumCh0 [VM/L3]	CumCh1 [VM/L3]	CumChR [VM/L3]	ChemS(i), i=1, NumKD [VM/L3]	SMean(j), j=1, NumKD [VM/T/L3]
720.00	-.121E+02	.867E-02	.000E+00	.000E+00	.389E+00
1440.00	-.782E+02	.110E+00	.000E+00	.000E+00	.309E+01
3600.00	-.844E+03	.286E+01	.000E+00	.000E+00	.412E+02
7200.00	-.464E+04	.310E+02	.000E+00	.000E+00	.250E+03
10800.00	-.108E+05	.115E+03	.000E+00	.000E+00	.618E+03
14400.00	-.172E+05	.263E+03	.000E+00	.000E+00	.101E+04

TABLE 10.18. Output data for example 7 (part of output file 'BALANCE.OUT').

Example 7 - Water and Solute Infiltration Test				
Time [T]	Total	Sub-region number ...		
.0000		1	2	
Area [V]	.638E+07	.147E+07	.491E+07	
Volume [V]	.188E+07	.405E+06	.147E+07	
InFlow [V/T]	.000E+00	.000E+00	.000E+00	
hMean [L]	-.604E+02	-126.3	-40.6	
HeatVol [VM/L/T2]	.166E+18	.358E+17	.130E+18	
tMean [K]	20.000	20.000	20.000	
ConcVol [VM/L3] 1	.000E+00	.000E+00	.000E+00	
cMean [M/L3] 1	.000E+00	.000E+00	.000E+00	
ConcVol [VM/L3] 2	.000E+00	.000E+00	.000E+00	
cMean [M/L3] 2	.000E+00	.000E+00	.000E+00	
ConcVol [VM/L3] 3	.000E+00	.000E+00	.000E+00	
cMean [M/L3] 3	.000E+00	.000E+00	.000E+00	
.	.	.	.	
.	.	.	.	
14400.0000		1	2	
Area [V]	.638E+07	.147E+07	.491E+07	
Volume [V]	.196E+07	.455E+06	.151E+07	
InFlow [V/T]	-.367E+00	-.195E+00	-.172E+00	
hMean [L]	-.450E+02	-81.3	-34.1	
HeatVol [VM/L/T2]	.170E+18	.382E+17	.132E+18	
tMean [K]	20.943	21.572	20.754	
ConcVol [VM/L3] 1	.541E+04	.115E+04	.425E+04	
cMean [M/L3] 1	.196E-02	.173E-02	.203E-02	
ConcVol [VM/L3] 2	.106E+06	.240E+05	.824E+05	
cMean [M/L3] 2	.452E-01	.425E-01	.460E-01	
ConcVol [VM/L3] 3	.159E+05	.337E+04	.125E+05	
cMean [M/L3] 3	.441E-02	.393E-02	.456E-02	
WatBalT [V]	-.394E+04			
WatBalR [%]	.529			
CncBalT [VM/L3] 1	-.348E+04			
CncBalR [%] 1	.886			
CncBalT [VM/L3] 2	.542E+03			
CncBalR [%] 2	.353			
CncBalT [VM/L3] 3	-.231E+02			
CncBalR [%] 3	.125			



## 11. PROGRAM ORGANIZATION

### 11.1. *Description of Program Units*

The program consists of a main program and 66 subprograms. The subprograms are organized by means of 9 source files which are stored and compiled separately and then linked together with the main program to form an executable program. Below are a list and brief descriptions of the source files and the associated subprograms.

<b>CHAIN_2D.FOR</b>	(Main program unit)
<b>INPUT2.FOR</b>	BasInf, MatIn, GenMat, TmIn, SeepIn, DrainIn, NodInf, ElemIn, GeomIn, AtmIn, SinkIn, ChemIn, TempIn
<b>WATFLOW2.FOR</b>	WatFlow, Reset, Dirich, Solve, Shift, SetMat, Veloc
<b>TIME2.FOR</b>	TmCont, SetAtm, Fgh
<b>MATERIA2.FOR</b>	FK, FC, FQ, FH
<b>SINK2.FOR</b>	SetSnk, FAlfa
<b>OUTPUT2.FOR</b>	TLInf, ALInf, hOut, thOut, QOut, FlxOut, SubReg, BouOut, cOut, SolInf, TOut, ObsNod
<b>SOLUTE2.FOR</b>	Solute, Coef, SolMat, c_Bound, Disper, SolveT, WeFact, PeCour
<b>TEMPER2.FOR</b>	Temper, T_Bound, DispT
<b>ORTHOFEM.FOR</b>	IADMak, Insert, Find, ILU, DU, ORTHOMIN, LUSolv, MatM2, SDot, SDotK, SNRM, SAXPYK, SCopy, SCopyK

#### *Main program unit CHAIN\_2D.FOR*

This is the main program unit of CHAIN\_2D. This unit controls execution of the program and determines which optional subroutines are necessary for a particular application.

*Source file INPUT.FOR*

Subroutines included in this source file are designed to read data from different input blocks. The following table summarizes from which input file and input block (described in Section 9) a particular subroutine reads.

Table 11.1. Input subroutines/files.

Subroutine	Input Block	Input File
BasInf	A. Basic Information	
MatIn	B. Material Information	
TmIn	C. Time Information	
SinkIn	D. Sink Information	SELECTOR.IN
SeepIn	E. Seepage Information	
DrainIn	F. Drainage Information	
ChemIn	G. Solute Transport Information	
TempIn	H. Heat Transport Information	
NodInf	I. Nodal Information	
ElemIn	J. Element Information	GRID.IN
GeomIn	K. Boundary Geometry Information	
AtmIn	L. Atmospheric Information	ATMOSPH.IN

Subroutine **GenMat** generates for each soil type in the flow domain a table of water contents, hydraulic conductivities, and specific water capacities from set of hydraulic parameters.

*Source file WATFLOW2.FOR*

Subroutine **WatFlow** is the main subroutine for simulating water flow; this subroutine controls the entire iterative procedure of solving the Richards equation.

Subroutine **Reset** constructs the global matrix equation for water flow, including the right-hand side vector.

Subroutine **Dirich** modifies the global matrix equation by incorporating prescribed pressure head nodes.

Subroutine **Solve** solves the banded symmetric matrix equation for water flow by Gaussian elimination.

Subroutine **Shift** changes atmospheric or seepage face boundary conditions from Dirichlet type to Neumann type conditions, or vice versa, as needed. Also updates boundary conditions for the variable boundary fluxes (free and deep drainage).

Subroutine **Veloc** calculates nodal water fluxes.

Subroutine **SetMat** determines the nodal values of the hydraulic properties  $K(h)$ ,  $C(h)$  and  $\theta(h)$  by interpolation between intermediate values in the hydraulic property tables.

*Source file TIME2.FOR*

Subroutine **TmCont** adjusts the current value of the time increment  $\Delta t$ .

Subroutine **SetAtm** updates time variable boundary conditions.

Function **Fqh** describes the groundwater level - discharge relationship,  $q(h)$ , defined by equation (7.1). This function is called only from subroutine **SetAtm**.

*Source file MATERIA2.FOR*

This file includes the functions **FK**, **FC**, **FQ** and **FH** which define the unsaturated hydraulic properties  $K(h)$ ,  $C(h)$ ,  $\theta(h)$ , and  $h(\theta)$ , for each soil material.

*Source file SINK2.FOR*

This file includes subroutine **SetSnk** and function **FAlfa**. These subroutines calculate the actual root water extraction rate as a function of water stress in the soil root zone.

*Source file OUTPUT2.FOR*

The subroutines included in this file are designed to print data to different output files. Table 11.2 summarizes which output files are generated by a particular subroutine.

Table 11.2. Output subroutines/files.

Subroutine	Output File
TLInf	H_MEAN.OUT V_MEAN.OUT CUM_Q.OUT RUN_INF.OUT
SolInf	SOLUTE.OUT
hOut	H.OUT
thOut	TH.OUT
cOut	CONC.OUT
TOut	TEMP.OUT
QOut	Q.OUT
FlxOut	VX.OUT VZ.OUT
BouOut	BOUNDARY.OUT
SubReg	BALANCE.OUT
ALInf	A_LEVEL.OUT
ObsNod	OBSNOD.OUT

*Source file SOLUTE2.FOR*

Subroutine **Solute** is the main subroutine for simulating solute transport; this subroutine controls the entire iterative procedure of solving the nonlinear non-equilibrium convection-dispersion equation.

Subroutine **Coeff** calculates solute transport parameters.

Subroutine **SolMat** constructs the global matrix equation for transport, including the right-hand side vector.

Subroutine **c\_Bound** determines the values of the solute transport boundary codes,  $cKod(n)$ , and incorporates prescribed boundary conditions in the global matrix equation for solute transport.

Subroutine **Disper** calculates nodal values of the dispersion coefficients for solute transport.

Subroutine **SolveT** solves the final asymmetric banded matrix equation for solute transport using Gaussian elimination.

Subroutine **WeFact** computes the optimum weighing factors for all sides of all elements.

Subroutine **PeCour** computes the maximum local Peclet and Courant numbers and the maximum permissible time step.

#### *Source file TEMPER2.FOR*

Subroutine **Temper** is the main subroutine for simulating heat transport; this subroutine constructs the global matrix equation for heat transport, including the right-hand side vector.

Subroutine **T\_Bound** determines the values of the heat transport boundary, and incorporates prescribed boundary conditions in the global matrix equation for heat transport.

Subroutine **DispT** calculates nodal values of the apparent dispersion coefficients for heat transport.

#### *Source file ORTHOFEM.FOR*

The subroutines included in this file solve large sparse systems of linear algebraic equations by the preconditioned conjugate gradient method for symmetric matrices, and by the ORTHOMIN method for asymmetric matrices. The subroutines were adopted from *Mendoza et al. [1991]*. See *Mendoza et al. [1991]* for a detailed description of both methods.

Subroutine **IADMake** generates the adjacency matrix which determines nodal connections from the finite element incidence matrix.

**Subroutine Insert** adds node  $j$  to the adjacency list for node  $i$ .

**Subroutine Find** retrieves from the adjacency matrix the appropriate position of two global point in the coefficient matrix.

**Subroutine ILU** performs incomplete lower-upper decomposition of matrix  $[A]$ .

**Function DU** searches the  $i$ th row of the upper diagonal matrix for an adjacency of node  $j$ .

**Subroutine ORTHOMIN** governs the ORTHOMIN (conjugate gradient) acceleration.

**Subroutine LUSolv** performs lower diagonal matrix inversion by forward substitution, and upper diagonal matrix inversion by backward substitution.

**Subroutine MatM2** multiplies a matrix by a vector.

**Function SDot** calculates the dot product of two vectors.

**Function SDotK** calculates the dot product of a column in matrix by a vector.

**Function SNRM** computes the maximum norm of a vector.

**Subroutine SAXPYK** multiplies a column in a matrix by a scalar, and adds the resulting value to another vector.

**Subroutine SCopy** copies a vector into another vector.

**Subroutine SCopyK** copies a column in a matrix into a vector.

## 11.2. List of Significant CHAIN\_2D Program Variables.

Variables which appear in subroutines of the ORTHOFEM package are not given in the following tables. Consult the user's guide of ORTHOFEM [Mendoza *et al.*, 1991] for their definition.

Table 11.3. List of significant integer variables.

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<i>ALevel</i>	Time level at which a time-dependent boundary condition is specified.
<i>cKod</i>	Code which specifies the type of boundary condition used for solute transport.
<i>II</i>	Maximum number of nodes on any transverse line (Table 9.9).
<i>ItCum</i>	Cumulative number of iterations (Table 10.4).
<i>IterC</i>	Number of iterations necessary for the solution of the solute transport equation (Table 10.4).
<i>IterW</i>	Number of iterations necessary for the solution of the water flow equation (Table 10.4).
<i>Kat</i>	Type of flow system to be analyzed (Table 9.1).
<i>MaxAl</i>	Number of atmospheric data records (Table 9.12).
<i>MaxIt</i>	Maximum number of iterations allowed during any time step for the solution of water flow equation (Table 9.1).
<i>MaxItC</i>	Maximum number of iterations allowed during any time step for the solution of solute transport equation (Table 9.7).
<i>MBand</i>	Bandwidth (half-bandwidth) of the symmetric (asymmetric) matrix $A$ when Gaussian elimination is used. Maximum number of nodes adjacent to another node when iterative solvers are used.
<i>MBandD</i>	Maximum permitted bandwidth of matrix $A$ when Gaussian elimination is used. Maximum permitted number of nodes adjacent to another node when iterative solvers are used (Table 7.7).
<i>MPL</i>	Number of specified print-times at which detailed information about the pressure head, the water content, flux, temperature, solute concentration, and the soil water, solute and energy balances are printed (Table 9.3).
<i>NCom</i>	Number of corner nodes of a particular element.
<i>NDr</i>	Number of drains.
<i>NDrD</i>	Maximum permitted number of drains.
<i>NLay</i>	Number of subregions for which separate water balances are being computed (Table 9.2).
<i>NLevel</i>	Number of time levels at which the matrix $A$ and vector $B$ are assembled for solute transport.
<i>NMat</i>	Number of soil materials (Table 9.2).
<i>NMatD</i>	Maximum number of soil materials (Table 7.7).

Table 11.3. (continued)

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<i>NObs</i>	Number of observation nodes for which the pressure head, the water content, temperature and concentration are printed at each time level
<i>NObsD</i>	Maximum number of observation nodes for which the pressure head, the water content, temperature and concentration are printed at each time level
<i>NPar</i>	Number of unsaturated soil hydraulic parameters specified for each material (Table 9.2).
<i>NS</i>	Number of solutes (Table 9.9).
<i>NSD</i>	Maximum number of solutes (Table 7.7).
<i>NSeep</i>	Number of seepage faces expected to develop (Table 9.5).
<i>NSeepD</i>	Maximum number of seepage faces (Table 7.7).
<i>NTab</i>	Number of entries in the internally generated tables of the hydraulic properties (see Section 5.3.11).
<i>NTabD</i>	Maximum number of entries in the internally generated tables of the hydraulic properties (Table 7.7).
<i>NumBP</i>	Number of boundary nodes for which $Kode(n) \neq 0$ (Table 9.9).
<i>NumBPD</i>	Maximum number of boundary nodes for which $Kode(n) \neq 0$ (Table 7.7).
<i>NumEl</i>	Number of elements (quadrilaterals and/or triangles) (Table 9.9).
<i>NumElD</i>	Maximum number of elements in finite element mesh (Table 7.7).
<i>NumKD</i>	Maximum number of available code number values (Table 7.7).
<i>NumNP</i>	Number of nodal points (Table 9.9).
<i>NumNPD</i>	Maximum number of nodes in finite element mesh (Table 7.7).
<i>NumSEL</i>	Number of subelements (triangles).
<i>NumSPD</i>	Maximum number of nodes along a seepage face (Table 7.7).
<i>NUS</i>	Number of corner nodes of a particular element ( $=NCorn$ ).
<i>PLevel</i>	Print time-level (current print-time number).
<i>TLevel</i>	Time-level (current time-step number) (Table 10.4).

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Table 11.4. List of significant real variables.

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<i>AE</i>	Area of a triangular element, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup> .
<i>Alf</i>	1- <i>Epsi</i> , where <i>Epsi</i> is a temporal weighing coefficient [-].
<i>Alfa</i>	Parameter in the soil water retention function [ $L^{-1}$ ] (see Section 2.3).
<i>Angle</i>	Angle in degrees between the principal direction of $K_1^A$ and the <i>x</i> -axis of the global coordinate system assigned to each element (Table 9.10).
<i>Aqh</i>	Parameter $A_{qh}$ in equation (7.1) [ $LT^{-1}$ ] (Table 9.12).
<i>AreaR</i>	Area of the domain occupied by the root zone, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup> .
<i>AT</i>	Temperature scaling factor $\alpha_h^*$ associated with the pressure head [-].
<i>ATot</i>	Area of the entire flow domain, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup> ( <i>Area</i> in Table 10.6).
<i>Bqh</i>	Parameter $B_{qh}$ in equation (7.1) [ $L^{-1}$ ] (Table 9.12).
<i>BT</i>	Temperature scaling factor $\alpha_K^*$ associated with the hydraulic conductivity [-].
<i>cBalR</i>	Relative error in the solute mass balance of the entire flow domain [%] (see equation (6.32)) ( <i>CncBalR</i> in Table 10.6).
<i>cBalT</i>	Absolute error in the solute mass balance of the entire flow domain, [ $ML^{-1}$ ] or [ $M$ ] <sup>†</sup> (see equation (6.32)) ( <i>CncBalT</i> in Table 10.6).
<i>cBnd</i>	Value of the boundary condition for solute transport [ $ML^{-1}$ ].
<i>cE</i>	Average concentration of an element [ $ML^{-3}$ ].
<i>Change</i>	Inflow/Outflow to/from the flow domain, [ $L^2T^{-1}$ ] or [ $L^3T^{-1}$ ] <sup>†</sup> ( <i>InFlow</i> in Table 10.6).
<i>cMid</i>	Arithmetic mean of the concentration at the old and new time level [ $ML^{-3}$ ].
<i>cNewE</i>	Amount of solute in a particular element at the new time-level, [ $ML^{-1}$ ] or [ $M$ ] <sup>†</sup> .
<i>ConA1</i>	First principal component, $K_1^A$ , of the dimensionless anisotropy tensor $K^A$ [-] assigned to each element (Table 9.10).
<i>ConA2</i>	Second principal component, $K_2^A$ [-] (Table 9.10).
<i>Courant</i>	Maximum local Courant number [-] (Table 10.4).
<i>cSink</i>	Concentration of the sink term [ $ML^{-1}$ ].
<i>cTolA</i>	Maximum desired absolute change in the value of the concentration, <i>c</i> [ $ML^{-3}$ ], between two successive iterations for nonlinear adsorption (Table 9.7).
<i>cTolR</i>	Maximum desired relative change in the value of the concentration, <i>c</i> [ $ML^{-3}$ ], between two successive iterations for nonlinear adsorption (Table 9.7).
<i>CumQrR</i>	Cumulative total potential transpiration from the entire flow domain, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup> ( <i>CumQRP</i> in Tables 10.3 and 10.7).
<i>CumQrT</i>	Cumulative total potential flux across the atmospheric boundary, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup> ( <i>CumQAP</i> in Tables 10.3 and 10.7).
<i>CumQvR</i>	Cumulative total actual transpiration from the entire flow domain, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup> ( <i>CumQR</i> in Tables 10.3 and 10.7).

Table 11.4. (continued)

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<i>CumR</i>	Amount of solute removed from the entire flow domain by root water uptake during one time step, $[ML^{-1}]$ or $[M]^t$ .
<i>Cum0</i>	Amount of solute removed from the entire flow domain by zero-order reactions during one time step, $[ML^{-1}]$ or $[M]^t$ .
<i>Cum1</i>	Amount of solute removed from the entire flow domain by first-order reactions during one time step, $[ML^{-1}]$ or $[M]^t$ .
<i>DeltC</i>	Sum of the absolute changes in concentrations as summed over all elements, $[ML^{-1}]$ or $[M]^t$ (see equation (6.32)).
<i>DeltW</i>	Sum of the absolute changes in water content as summed over all elements, $[L^2]$ or $[L^3]^t$ (see equation (5.25)).
<i>dHenry</i>	Time change of the equilibrium distribution constant, $k_s$ , within one time step $[T^{-1}]$ .
<i>dh</i>	Spacing (logarithmic scale) between consecutive pressure heads in the internally generated tables of the hydraulic properties [-] (see equation (5.28)).
<i>dMul</i>	Dimensionless number by which $\Delta t$ is multiplied if the number of iterations is less than or equal to 3 [-] (Table 9.3).
<i>dMul2</i>	Dimensionless number by which $\Delta t$ is multiplied if the number of iterations is greater than or equal to 7 [-] (Table 9.3).
<i>dt</i>	Time increment, $\Delta t$ [T] (Table 9.3).
<i>dtMax</i>	Maximum permitted time increment, $\Delta t_{max}$ [T] (Table 9.3).
<i>dtMaxC</i>	Maximum permitted time increment, $\Delta t_{max}$ for solute transport [T] (see equation (6.34)).
<i>dtMin</i>	Minimum permitted time increment, $\Delta t_{min}$ [T] (Table 9.3).
<i>dtOld</i>	Old time increment [T].
<i>dtOpt</i>	Optimal time increment [T].
<i>EI</i>	Potential surface flux per unit atmospheric boundary $[LT^{-1}]$ ( $=rTop$ ).
<i>Epsi</i>	Temporal weighing coefficient [-] (Table 9.7).
<i>EpsH</i>	Absolute change in the nodal pressure head between two successive iterations [L].
<i>EpsTh</i>	Absolute change in the nodal water content between two successive iterations [-].
<i>fExp</i>	Adsorption isotherm coefficient, $\beta$ [-] (Table 9.7).
<i>Frac</i>	Dimensionless fraction of the adsorption sites classified as Type-1, e.i. sites with instantaneous sorption (Table 9.7).
<i>GamG</i>	First-order rate constant for gaseous phase, $\mu_g$ $[T^{-1}]$ (Table 9.7).
<i>GamG1</i>	Rate constant, $\mu_g$ $[T^{-1}]$ , representing first-order decay for the first solute and zero-order production for the second solute in the gaseous phase (Table 9.7).
<i>GamL</i>	First-order rate constant for dissolved phase, $\mu_w$ $[T^{-1}]$ (Table 9.7).
<i>GamL1</i>	Rate constant, $\mu_w$ $[T^{-1}]$ , representing first-order decay for the first solute and zero-order production for the second solute in the dissolved phase (Table 9.7).

Table 11.4. (continued)

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<i>GamS</i>	First-order rate constant for the solid phase concentration, $\mu_s$ [ $T^{-1}$ ] (Table 9.7).
<i>GamS1</i>	Rate constant, $\mu_s$ [ $T^{-1}$ ], representing first-order decay for the first solute and zero-order production for the second solute in the solid phase (Table 9.7).
<i>GWL</i>	Time-dependent prescribed head boundary condition [L] for nodes indicated by $Kode(n) = +3$ (Table 9.12).
<i>GWL0L</i>	Parameter in equation (7.1) [L] (Table 9.12).
<i>hCritA</i>	Minimum allowed pressure head at the soil surface [L] (Table 9.12).
<i>hCritS</i>	Maximum allowed pressure head at the soil surface [L] (Table 9.12).
<i>hE</i>	Mean element value of the pressure head [L].
<i>Henry</i>	Equilibrium distribution constant between liquid and gas phase, $k_e$ [-] (Table 9.7).
<i>hMeanG</i>	Mean value of the pressure head calculated over a set of nodes for which $Kode(n) = \pm 3$ [L] ( $hKode3$ in Tables 10.1 and 10.7).
<i>hMeanR</i>	Mean value of the pressure head within the root zone [L] ( $hRoot$ in Table 10.1 and 10.7).
<i>hMeanT</i>	Mean value of the pressure head calculated over a set of nodes for which $Kode(n) = \pm 4$ [L] ( $hAtm$ in Tables 10.1 and 10.7).
<i>hTab1</i>	Lower limit [L] of the pressure head interval for which tables of hydraulic properties is generated internally for each material ( $ha$ in Table 9.2).
<i>hTabN</i>	Upper limit [L] of the pressure head interval for which tables of hydraulic properties is generated internally for each material ( $hb$ in Table 9.2).
<i>hTot</i>	Mean pressure head in the entire flow domain [L] ( $hMean$ in Table 10.6).
<i>Kk</i>	Unsaturated hydraulic conductivity corresponding to $\theta_k$ [ $LT^{-1}$ ] (see Section 2.3) (Table 9.2).
<i>Ks</i>	Saturated hydraulic conductivity [ $LT^{-1}$ ] (Table 9.2).
<i>m</i>	Parameter in the soil water retention function [-] (see Section 2.3) (Table 9.2).
<i>n</i>	Parameter in the soil water retention function [-] (see Section 2.3) (Table 9.2).
<i>Omega</i>	Mass transfer coefficient for the nonequilibrium adsorption, $\omega$ [ $T^{-1}$ ] (Table 9.7).
<i>Peclet</i>	Maximum local Peclet number [-] (Table 10.4).
<i>PeCr</i>	Stability criteria (see Section 6.3.6) (Table 9.7).
<i>Prec</i>	Precipitation [ $LT^{-1}$ ] (Table 9.12).
<i>P0</i>	Value of the pressure head [L], $h_1$ , below which roots start to extract water from the soil (Table 9.4).
<i>P2H</i>	Value of the limiting pressure head [L], $h_3$ , below which the roots cannot extract water at the maximum rate (assuming a potential transpiration rate of $r2P$ ) (Table 9.4).
<i>P2L</i>	As above, but for a potential transpiration rate of $r2L$ (Table 9.4).
<i>P3</i>	Value of the pressure head [L], $h_4$ , below which root water uptake ceases (usually equal to the wilting point) (Table 9.4).

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Table 11.4. (continued)

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$Q_a$	Parameter in the soil water retention function [-] (see Section 2.3) (Table 9.2).
$Q_k$	Volumetric water content corresponding to $K_k$ [-] (see Section 2.3) (Table 9.2).
$Q_m$	Parameter in the soil water retention function [-] (see Section 2.3) (Table 9.2).
$Q_r$	Residual soil water content [-] (Table 9.2).
$Q_s$	Saturated soil water content [-] (Table 9.2).
$R$	Universal gas constant [ $\text{ML}^2\text{T}^{-2}\text{K}^{-1}\text{M}^{-1}$ ] ( $8.314 \text{ kg m}^2 \text{ s}^{-2}\text{K}^{-1}\text{mol}^{-1}$ ).
$rLen$	Width of soil surface associated with transpiration, [L] or [ $\text{L}^2$ ] <sup>t</sup> (Table 9.11).
$\rho$	Bulk density of material $M$ [ $\text{ML}^{-3}$ ].
$RootCh$	Amount of solute removed from a particular subelement during one time step by root water uptake, [ $\text{ML}^{-1}$ ] or [ $\text{M}$ ] <sup>t</sup> .
$rQWL$	Time-dependent prescribed flux boundary condition [ $\text{LT}^{-1}$ ] for nodes were $Kode(n) = -3$ (Table 9.12).
$rRoot$	Potential transpiration rate [ $\text{LT}^{-1}$ ] (Table 9.12).
$rSoil$	Potential evaporation rate [ $\text{LT}^{-1}$ ] (Table 9.12).
$rTop$	Potential surface flux per unit atmospheric boundary [ $\text{LT}^{-1}$ ] ( $rAtm$ in Table 10.2).
$r2H$	Potential transpiration rate [ $\text{LT}^{-1}$ ] (see Table 9.4).
$r2L$	Potential transpiration rate [ $\text{LT}^{-1}$ ] (see Table 9.4).
$t$	Time, $t$ , at current time-level [T].
$tAtm$	Time for which the $i$ -th data record is provided [T] (Table 9.12).
$TauG$	Tortuosity factor in the gas phase [-].
$TauW$	Tortuosity factor in the liquid phase [-].
$TE$	Average temperature of an element [K].
$tFix$	Next time resulting from time discretizations 2 and 3 [T] (see Section 5.3.3).
$tInit$	Starting time of the simulation [T] (Table 9.12).
$tMax$	Maximum duration of the simulation [T].
$TNewE$	Amount of heat in a particular element, [ $\text{MLT}^{-2}$ ] or [ $\text{ML}^2\text{T}^{-2}$ ] <sup>t</sup> .
$tOld$	Previous time-level [T].
$TolH$	Maximum desired absolute change in the value of the pressure head, $h$ [L], between two successive iterations during a particular time step (Table 9.1).
$TolTh$	Maximum desired absolute change in the value of the water content, $\theta$ [-], between two successive iterations during a particular time step (Table 9.1).
$tPeriod$	Time interval for the completion of one temperature cycle [T].
$tPulse$	Time duration of the concentration pulse [T] (Table 9.7).

Table 11.4. (continued)

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<i>Tr</i>	Reference temperature 293.15 K (20°C).
<i>TTot</i>	Average temperature in the entire flow domain [K] (Table 10.6).
<i>TVol</i>	Amount of heat in the entire flow domain, $[MLT^{-2}]$ or $[ML^2T^{-2}]^\dagger$ (Table 10.6).
<i>Vabs</i>	Absolute value of the nodal Darcy fluid flux density $[LT^{-1}]$ .
<i>vMeanR</i>	Actual transpiration rate $[LT^{-1}]$ ( <i>vRoot</i> in Table 10.2).
<i>vNewE</i>	Volume of water in a particular element at the new time-level, $[L^2]$ or $[L^3]^\dagger$ .
<i>vOldE</i>	Volume of water in a particular element at the old time-level, $[L^2]$ or $[L^3]^\dagger$ .
<i>Volume</i>	Volume of water in the entire flow domain, $[L^2]$ or $[L^3]^\dagger$ (Table 10.6).
<i>wBalR</i>	Relative error in the water mass balance in the entire flow domain [%] (see equation (5.25)).
<i>wBalT</i>	Absolute error in the water mass balance in the entire flow domain, $[L^2]$ or $[L^3]^\dagger$ (see equation (5.24)).
<i>wCumA</i>	Sum of the absolute values of all fluxes across the flow boundaries, including those resulting from sources and sinks in the region, $[L^2]$ or $[L^3]^\dagger$ (see equation (5.25)).
<i>wCumT</i>	Sum of all cumulative fluxes across the flow boundaries, including those resulting from sources and sinks in the region, $[L^2]$ or $[L^3]^\dagger$ (see equation (5.24)).
<i>wVoll</i>	Initial volume of water in the flow domain, $[L^2]$ or $[L^3]^\dagger$ .
<i>xKs</i>	Adsorption isotherm coefficient, $k_s$ $[L^3M^{-1}]$ (Table 9.7).
<i>xNu</i>	Adsorption isotherm coefficient, $\eta$ $[L^3M^{-1}]$ (Table 9.7).
<i>xMuG</i>	Zero-order rate constant for the gaseous phase concentration, $\gamma_g$ $[ML^3T^{-1}]$ (Table 9.7).
<i>xMuL</i>	Zero-order rate constant for the liquid phase concentration, $\gamma_w$ $[ML^3T^{-1}]$ (Table 9.7).
<i>xMul</i>	Parameter which depends on the type of flow system being analyzed, [-] or $[L]^\dagger$ (see equations (5.11) and (5.12)).
<i>xMuS</i>	Zero-order rate constant for the solid phase concentration, $\gamma_s$ $[T^{-1}]$ (Table 9.7).

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<sup>†</sup> for plane and axisymmetric flow, respectively

Table 11.5. List of significant logical variables.

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<i>AtmInf</i>	Logical variable indicating whether or not the input file ATMOSPH.IN is provided (Table 9.1).
<i>CheckF</i>	Logical variable indicating whether or not the grid input data are to be printed for checking (Table 9.1).
<i>DrainF</i>	Logical variable indicating whether drains are, or are not, present in the transport domain (Table 9.1); if drains are present, they are represented by an electrical resistance network analog.
<i>Explic</i>	Logical variable indicating whether an explicit or implicit scheme was used for solving the water flow equation.
<i>FluxF</i>	Logical variable indicating whether or not detailed flux information is to be printed (Table 9.1).
<i>FreeD</i>	Logical variable indicating whether a unit hydraulic gradient (free drainage) is, or is not, invoked at the bottom of the transport domain (Table 9.1).
<i>ItCrit</i>	Logical variable indicating whether or not convergence was achieved.
<i>lArtD</i>	Logical variable indicating whether or not an artificial dispersion is to be added in order to satisfy the stability criteria <i>PeCr</i> (see Section 6.3.6).
<i>lChem</i>	Logical variable indicating whether or not the solute transport equation is to be solved (Table 9.1).
<i>lConst</i>	Logical variable indicating whether or not there is a constant number of nodes at any transverse line.
<i>lConv</i>	Logical variable indicating whether or not convergence was achieved in case of nonlinear or non-equilibrium adsorption (Table 9.1).
<i>lEquil</i>	Logical variable indicating whether equilibrium or non-equilibrium adsorption is considered.
<i>lLinear</i>	Logical variable indicating whether a linear or nonlinear adsorption is considered.
<i>lOrt</i>	Logical variable indicating whether indirect iterative methods (the conjugate gradient method for symmetric matrices or the ORTHOMIN method for asymmetric matrices) or direct methods (Gaussian elimination for banded matrices) are used to solve the system of linear equations.
<i>lTDep</i>	Logical variable indicating whether or not the solute transport properties are considered to be temperature dependent.
<i>lTemp</i>	Logical variable indicating whether or not the heat transport equation is to be solved (Table 9.1).
<i>lUpW</i>	Logical variable indicating if upstream weighing or the standard Galerkin formulation is to be used (Table 9.7).
<i>lWat</i>	Logical variable indicating if steady-state or transient water flow is to be considered (Table 9.1).
<i>lWDep</i>	Logical variable indicating whether or not the soil hydraulic properties are considered to be temperature dependent.
<i>qGWLF</i>	Logical variable indicating whether or not the discharge-groundwater level relationship is used as bottom boundary condition (Table 9.12).

Table 11.5. (continued)

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<i>SeepF</i>	Logical variable indicating whether or not a seepage face is to be expected (Table 9.1).
<i>ShortF</i>	Logical variable indicating whether or not the printing of time-level information is to be suppressed on each time level (Table 9.1).
<i>SinkF</i>	Logical variable indicating whether or not plant water uptake will take place (Table 9.12).

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Table 11.6. List of significant arrays.

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<i>A(MBandD,NumNPD)</i>	Coefficient matrix.
<i>Ac(NumNPD)</i>	Nodal values of the product $\theta R$ [-].
<i>Area(10)</i>	Areas of the specified subregions, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup> (Table 10.6).
<i>Axz(NumNPD)</i>	Nodal values of the dimensionless scaling factor $\alpha_x$ associated with the pressure head [-] (Table 9.9).
<i>B(NumNPD)</i>	Coefficient vector.
<i>Beta(NumNPD)</i>	Nodal values of the normalized rootwater uptake distribution, [ $L^2$ ] or [ $L^3$ ] <sup>†</sup> (Table 9.9).
<i>Bxz(NumNPD)</i>	Nodal value of the scaling factor $\alpha_x$ associated with the saturated hydraulic conductivity [-] (Table 9.9).
<i>Cap(NumNPD)</i>	Nodal values of the soil water hydraulic capacity [ $L^{-1}$ ].
<i>CapTab(NTabD,NMatD)</i>	Internal table of the soil water hydraulic capacity [ $L^{-1}$ ].
<i>cBound(NSD,9)</i>	Values of the time independent concentration boundary condition [ $ML^{-3}$ ] (Table 9.7).
<i>cCumA(NSD)</i>	Sum of the absolute values of all cumulative solute fluxes across the flow boundaries, including those resulting from sources and sinks in the flow domain, [ $ML^{-1}$ ] or [ $M$ ] <sup>†</sup> (see equation (6.32)).
<i>cCumT(NSD)</i>	Sum of all cumulative solute fluxes across the boundaries, including those resulting from sources and sinks in the flow domain, [ $ML^{-1}$ ] or [ $M$ ] <sup>†</sup> (see right hand side of equation (6.31)).
<i>ChemS(NSD,NumKD)</i>	Cumulative boundary solute fluxes, [ $ML^{-1}$ ] or [ $M$ ] <sup>†</sup> (Table 10.5).
<i>ChPar(NSD*16+4,NMatD)</i>	Parameters which describe the solute transport properties of the porous media (Table 9.7).
<i>cht(NSD)</i>	Time-dependent concentration for the first-type boundary condition assigned to nodes for which $Kode(n) = +3$ [ $ML^3$ ] (Table 9.12).
<i>cMean(8,10)</i>	Mean concentrations of specified subregions [ $ML^{-3}$ ] (Table 10.6).
<i>cNew(NumNPD)</i>	Nodal values of the concentration at the new time level [ $ML^{-3}$ ].
<i>Con(NumNPD)</i>	Nodal values of the hydraulic conductivity [ $LT^{-1}$ ].
<i>ConO(NumNPD)</i>	Nodal values of the hydraulic conductivity at old time level [ $LT^{-1}$ ].
<i>ConAxx(NumEID)</i>	Nodal values of the component $K_{xx}^A$ of the anisotropy tensor $K^A$ [-].
<i>ConAxz(NumEID)</i>	Nodal values of the component $K_{xz}^A$ of the anisotropy tensor $K^A$ [-].
<i>ConAzz(NumEID)</i>	Nodal values of the component $K_{zz}^A$ of the anisotropy tensor $K^A$ [-].
<i>Conc(NSD,NumNPD)</i>	Nodal values of the concentration [ $ML^{-3}$ ] (Table 9.9).
<i>ConSat(NMatD)</i>	Saturated hydraulic conductivities of the material [ $LT^{-1}$ ].
<i>ConSub(8,10)</i>	Amounts of solute in the specified subregions, [ $ML^{-1}$ ] or [ $M$ ] <sup>†</sup> (Table 10.6).
<i>ConTab(NTabD,NMatD)</i>	Internal table of the hydraulic conductivity [ $LT^{-1}$ ].

Table 11.6. (continued)

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<i>ConVol(8)</i>	Amount of solute in the entire flow domain, $[ML^{-1}]$ or $[M]^\dagger$ ( <i>ConVol</i> in Table 10.6).
<i>cPrec(NSD)</i>	Solute concentration of rainfall water $[ML^{-3}]$ (Table 9.12).
<i>cPrevO(NumNPD)</i>	Nodal values of the concentration of the previous solute in the decay chain at the old time-level $[ML^{-3}]$ .
<i>crt(NSD)</i>	Time-dependent concentration of the drainage flux, or some other time-dependent prescribed flux for nodes were $Kode(n) = -3 [ML^{-3}]$ (Table 9.12).
<i>cTemp(NumNPD)</i>	Nodal values of the concentration at the previous iteration $[ML^{-3}]$ .
<i>cTot(8)</i>	Mean concentration in the flow domain $[ML^{-3}]$ ( <i>cMean</i> in Table 10.6).
<i>CumCh0(NSD)</i>	Cumulative amount of solute removed from the entire flow domain by zero-order reactions, $[ML^{-1}]$ or $[M]^\dagger$ (Table 10.5).
<i>CumCh1(NSD)</i>	Cumulative amount of solute removed from the entire flow domain by first-order reactions, $[ML^{-1}]$ or $[M]^\dagger$ (Table 10.5).
<i>CumChR(NSD)</i>	Cumulative amount of solute removed from the entire flow domain by root water uptake, $[ML^{-1}]$ or $[M]^\dagger$ (Table 10.5).
<i>CumQ(NumKD)</i>	Cumulative boundary fluxes, $[L^2]$ or $[L^3]^\dagger$ (Table 10.3).
<i>cVolI(NSD)</i>	Initial amount of solute in the entire flow domain, $[ML^{-1}]$ or $[M]^\dagger$ .
<i>Dispxx(NumNPD)</i>	Nodal values of the component $D_{xx}$ of the dispersion tensor $[L^2T^{-1}]$ .
<i>Dispzz(NumNPD)</i>	Nodal values of the component $D_{zz}$ of the dispersion tensor $[L^2T^{-1}]$ .
<i>Dispzz(NumNPD)</i>	Nodal values of the component $D_{xz}$ of the dispersion tensor $[L^2T^{-1}]$ .
<i>DS(NumNPD)</i>	Vector $\{D\}$ in the global matrix equation for water flow, $[L^2T^{-1}]$ or $[L^3T^{-1}]^\dagger$ (see equation (5.9)); also used for the diagonal of the coefficient matrix $[Q]$ in the global matrix equation for solute transport, $[L^2]$ or $[L^3]^\dagger$ (see equation (6.5)).
<i>Dxz(NumNPD)</i>	Nodal values of the scaling factor $\alpha_\theta$ associated with the water content (Table 9.9).
<i>E(3,3)</i>	Element contributions to the global matrix $A$ for water flow $[L^2]$ (see equation (5.5)).
<i>EfDim(2,NDr)</i>	Effective diameter of drains and side lengths of the finite element mesh representing the drain (Table 9.6).
<i>F(NumNPD)</i>	Diagonal of the coefficient matrix $[F]$ in the global matrix equation for water flow, $[L^2]$ or $[L^3]^\dagger$ (see equation (5.7)).
<i>Fc(NumNPD)</i>	Nodal values of the part of the parameter $F$ (representing first-order reactions) $[T^{-1}]$ (see equation (3.13)).
<i>Fc1(NumNPD)</i>	Nodal values of the parameter $F$ $[T^{-1}]$ (see equation (3.13)).
<i>Gc(NumNPD)</i>	Nodal values of the part of the parameter $G$ (representing zero-order reactions) $[ML^{-3}T^{-1}]$ (see equation (3.8), (3.14)).
<i>Gc1(NumNPD)</i>	Nodal values of the parameter $G$ $[ML^{-3}T^{-1}]$ (see equation (3.8), (3.14)).
<i>hMean(10)</i>	Mean values of the pressure head in specified subregions $[L]$ (Table 10.6).
<i>hMean(NumKD)</i>	Mean values of the pressure head along a certain type of boundary $[L]$ (Table 10.6).

Table 11.6. (continued)

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$hNew(NumNPD)$	Nodal values of the pressure head [L] at the new time-level (Table 9.9).
$hOld(NumNPD)$	Nodal values of the pressure head [L] at the old time-level.
$hSat(NMatD)$	Air-entry values for each material [L].
$hTab(NTabD)$	Internal table of the pressure head [L].
$hTemp(NumNPD)$	Nodal values of the pressure head [L] at the previous iteration.
$IU(13)$	Vector which contains identification numbers of output files.
$KEIDr(NDr, NEID)$	Global numbers of elements surrounding a particular drain (Table 9.6).
$KodCB(NumBPD)$	Codes which identify the type of invoked boundary conditions, and refer to the vector $cBound$ for time-independent solute transport boundary conditions (Table 9.7).
$Kode(NumNPD)$	Codes which specify the type of invoked boundary conditions, (Table 9.9).
$KodTB(NumBPD)$	Codes which identify the type of invoked boundary conditions, and refer to the vector $TBound$ for time-independent heat transport boundary conditions (Table 9.8).
$KX(NumEID,4)$	Global nodal numbers of element corner nodes (Table 9.10).
$KXB(NumBPD)$	Global nodal numbers of sequentially numbered boundary nodes for which $Kode(n) \neq 0$ (Table 9.11).
$LayNum(NumEID)$	Subregion numbers assigned to each element (Table 9.10).
$ListNE(NumNPD)$	Number of subelements adjacent to a particular node.
$MatNum(NumNPD)$	Indices for material whose hydraulic and transport properties are assigned to a particular node (Table 9.9).
$ND(NDr)$	Global number of a drain (Table 9.6).
$NEID(NDr)$	Number of elements surrounding a drain (Table 9.6).
$Node(NObsD)$	Observation nodes for which the pressure head, the water content, temperature and concentration are printed at each time level.
$NP(NSeepD, NumSPD)$	Sequential global numbers of nodes on the seepage face (Table 9.5).
$NSP(NSeepD)$	Numbers of nodes on seepage face (Table 9.5).
$Par(10,NMatD)$	Parameters which describe the hydraulic properties of the medium (Table 9.2).
$POptm(NMatD)$	Values of the pressure head [L], $h_2$ , below which roots start to extract water at the maximum possible rate (Table 9.4).
$Q(NumNPD)$	Nodal values of the recharge/discharge rate, $[L^2T^{-1}]$ or $[L^3T^{-1}]^\dagger$ (Table 9.9).
$Qc(NSD, NumNPD)$	Nodal values of solute fluxes, $[ML^{-1}T^{-1}]$ or $[MT^{-1}]^\dagger$ .
$Sink(NumNPD)$	Nodal values of the sink term $[T^{-1}]$ (see equation (2.3)).
$SMean(NumKD)$	Total solute fluxes, $[ML^{-1}T^{-1}]$ or $[MT^{-1}]^\dagger$ (Table 10.5).
$SolIn(NumEID)$	Element values of the initial amount of solute, $[ML^{-1}]$ or $[M]^\dagger$ (Table 10.6).

Table 11.6. (continued)

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<i>Sorb(NSD,NumNPD)</i>	Nodal values of the sorbed concentration for type-2 sorption sites [-] (Table 9.9).
<i>SorbN(NumNPD)</i>	Nodal values of the sorbed concentration for type-2 sorption sites at the last iteration [-] (Table 9.9).
<i>SubCha(10)</i>	Inflow/outflow to/from specified subregions, $[L^2T^{-1}]$ or $[L^3T^{-1}]^t$ (Table 10.6).
<i>SubT(10)</i>	Amount of heat in specified subregions, $[MLT^2]$ or $[ML^2T^{-2}]^t$ (Table 10.6).
<i>SubVol(10)</i>	Volume of water in specified subregions, $[L^2]$ or $[L^3]^t$ (Table 10.6).
<i>SWidth(NumKD)</i>	Length of a boundary associated with a certain type of boundary condition, $[L]$ or $[L^2]^t$ .
<i>TBound(6)</i>	Values of the time-independent temperature boundary condition [K] (Table 9.8).
<i>TDep(NSD*16+4)</i>	Activation energy for transport and chemical parameters $[ML^2T^{-2}M^{-1}]$ (see Section 3.4).
<i>TempN(NumNPD)</i>	Nodal values of the temperature at the new time level [K].
<i>TempO(NumNPD)</i>	Nodal values of the temperature at the old time level [K].
<i>TheTab(NTabD,NMatD)</i>	Internal table of the soil water content [-].
<i>ThNew(NumNPD)</i>	Nodal values of the water content at the new time level [-].
<i>ThOld(NumNPD)</i>	Nodal values of the water content at the old time level [-].
<i>thr(NMatD)</i>	Residual water contents for specified materials [-].
<i>thSat(NMatD)</i>	Saturated water contents for specified materials [-].
<i>TMean(10)</i>	Average temperature in a specified subregion [K].
<i>TPar(10,NMatD)</i>	Parameters which describe the heat transport properties of the porous media (Table 9.8).
<i>TPrint(MPL)</i>	Specified print-times [T] (Table 9.3).
<i>vMean(NumKD)</i>	Values of boundary fluxes across a certain type of boundary, $[L^2T^{-1}]$ or $[L^3T^{-1}]^t$ .
<i>VxN(NumNPD)</i>	Nodal values of the <i>x</i> -component of the Darcian velocity vector at the new time level $[LT^{-1}]$ .
<i>VxO(NumNPD)</i>	Nodal values of the <i>x</i> -component of the Darcian velocity vector at the old time level $[LT^{-1}]$ .
<i>VzN(NumNPD)</i>	Nodal values of the <i>z</i> -component of the Darcian velocity vector at the new time level $[LT^{-1}]$ .
<i>VzO(NumNPD)</i>	Nodal values of the <i>z</i> -component of the Darcian velocity vector at the old time level $[LT^{-1}]$ .
<i>WatIn(NumEID)</i>	Element values of the initial volume of water, $[L^2]$ or $[L^3]^t$ .
<i>WeTab(3,2*NumEID)</i>	Weighing factors associated with the sides of subelements [-].
<i>Width(NumBPD)</i>	Width of the boundary associated with boundary nodes, $[L^2]$ or $[L^3]^t$ (Table 9.11).

Table 11.6. (continued)

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$x(NumNP)$	$x$ -coordinates [L] of the nodal points (Table 9.9).
$y(NumNP)$	$y$ or $z$ -coordinates [L] of the nodal points (Table 9.9).

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<sup>†</sup> for plane and axisymmetric flow, respectively

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## APPENDIX - MESH GENERATOR

To overcome potential problems with the correct definition and numbering of finite elements and their corner nodes in input file GRID.IN, we provided a separate finite element generator, GENER2 which produces the nodes and elements for a quadrilateral domain.

The mesh generator assumes that the initial pressure heads, concentrations and temperatures, as well as the recharge, the material number, the root water uptake distribution, and the scaling factors for the hydraulic property functions are all constant in each horizontal layer. The initial conditions, root distribution, recharge, material number and related parameters should, in general, be given for each horizontal layer. However, a short-cut is possible when two horizontal layers (e.g.,  $n_1$  and  $n_2$ ), not adjacent to each other, are located such that  $n_2$  is greater than  $n_1 + 1$ . The program will automatically generate layers between  $n_1$  and  $n_2$ , provided all of the following conditions are met simultaneously: (1) all horizontal layers between layers  $n_1$  and  $n_2$  are spaced at equal intervals, (2) values of the input variables  $hNew(n)$ ,  $Beta(n)$ ,  $Axz(n)$ ,  $Bxz(n)$ ,  $Dxz(n)$ ,  $Temp(n)$ , and  $Conc(1,n)$  through  $Conc(NS,n)$  vary linearly between layers  $n_1$  and  $n_2$ , and (3) the values of  $Kode(n)$ ,  $Q(n)$  and  $MatNum(n)$  are the same for all  $n = n_1, n_1 + 1, \dots, n_2 - 1$  (see also Table A.1).

Table A.1 gives a description of an input file for the finite element mesh generator GENER2. The input file GENER2.IN must be placed in directory CHAIN\_2D.IN. The resulting file GRID.IN can then be modified using any word- or data-processing software. The input files for example problems 1 through 7 are given in Tables A.2 through A.8. The input files GRID.IN, generated with GENER2 for example problems 1 through 6, can be used directly as input files for CHAIN\_2D without further modification. The GRID.IN file for example 7 needs additional modifications involving the boundary conditions.

Table A.1. Input file 'GENER2.IN' for finite element mesh generator.

Record	Type	Variable	Description
1,2	-	-	Comment lines.
3	Logical	<i>lAxisym</i>	Set equal to .true. if axisymmetrical flow domain is to be analyzed.
4	-	-	Comment line.
5	Real	<i>Angle</i>	Angle in degrees between $K_1^4$ and the $x$ -coordinate axis assigned to all elements.
5	Real	<i>ConA1</i>	First principal component, $K_1^4$ , of the dimensionless tensor $\mathbf{K}^4$ which describes the local anisotropy of the hydraulic conductivity assigned to all elements.
5	Real	<i>ConA2</i>	Second principal component, $K_2^4$ .
6	-	-	Comment line.
7	Integer	<i>NS</i>	Number of solutes in a chain reaction.
7	Logical	<i>lEquil</i>	.true. if equilibrium or no adsorption is considered for solute transport. .false. if nonequilibrium adsorption is considered for at least one solute species. If <i>lEquil</i> = .false., then initial values of the sorbed concentration for type-2 sorption sites must be specified.
8	-	-	Comment line.
9	Integer	<i>NColX</i>	Number of nodal points in the direction of the horizontal axes, $x$ .
9	Integer	<i>NLinZ</i>	Number of nodal points in the direction of the vertical axes, $z$ .
10	-	-	Comment line.
11	Real	<i>x(1)</i>	$x$ -coordinate of the left-most node [L].
11	Real	<i>z(NLinZ)</i>	$z$ -coordinate of the bottom node [L].
12,13	-	-	Comment lines.
14	Real	<i>dx(i)</i>	Array of $\Delta x$ increments [L], $i = 1, 2, \dots, (NColX-1)$ . Input subsequently from left to the right.
15	-	-	Comment lines.
16	Real	<i>dz(i)</i>	Array of $\Delta z$ increments [L], $i = 1, 2, \dots, (NLinZ-1)$ . Input subsequently from top to the bottom.
17,18	-	-	Comment lines.
19	Integer	<i>n</i>	Number of the horizontal layer starting at the upper boundary and continuing down to the bottom.
19	Integer	<i>Kode(n)</i>	Code specifying the type of boundary condition applied to nodes of a particular horizontal layer $n$ . Permissible values are 0, $\pm 1, \pm 2, \pm 3, \pm 4, \dots, \pm NumKD$ (see Section 8.3).

Table A.1. (continued)

Record	Type	Variable	Description
19	Real	<i>hOld(n)</i>	Initial value of the pressure head assigned to nodes of a particular horizontal layer <i>n</i> [L].
19	Real	<i>Q(n)</i>	Prescribed recharge/discharge rates assigned to nodes of a horizontal layer <i>n</i> , [L <sup>2</sup> T <sup>-1</sup> ]. <i>Q(n)</i> is negative when directed out of the system. When no value for <i>Q(n)</i> is needed, set <i>Q(n)</i> equal to zero.
19	Int	<i>MatNum(n)</i>	Material number assigned to nodes of a particular horizontal layer <i>n</i> .
19	Real	<i>Beta(n)</i>	Value of the water uptake distribution, <i>b(x,z)</i> , in the soil root zone assigned to nodes of a particular horizontal layer <i>n</i> [L <sup>-2</sup> ]. Set <i>Beta(n)</i> equal to zero if horizontal layer <i>n</i> lies outside the root zone.
19	Real	<i>Axz(n)</i>	Nodal value of the dimensionless scaling factor $\alpha_h$ associated with the pressure head assigned to nodes of a particular horizontal layer <i>n</i> [-].
19	Real	<i>Bxz(n)</i>	Nodal value of the dimensionless scaling factor $\alpha_K$ associated with the saturated hydraulic conductivity assigned to nodes of a particular horizontal layer <i>n</i> [-].
19	Real	<i>Dxz(n)</i>	Nodal value of the dimensionless scaling factor $\alpha_o$ associated with the water content assigned to nodes of a particular horizontal layer <i>n</i> [-].
19	Real	<i>Temp(n)</i>	Initial value of the temperature assigned to nodes of a particular horizontal layer <i>n</i> [°C].
19	Real	<i>Conc(1,n)</i>	Initial value of the concentration of the first solute assigned to nodes of a particular horizontal layer <i>n</i> [ML <sup>-3</sup> ]. This variable does not need to be specified if <i>NS</i> = 0.
19	Real	<i>Conc(2,n)</i>	Initial value of the concentration of the second solute assigned to nodes of a particular horizontal layer <i>n</i> [ML <sup>-3</sup> ]. This variable does not need to be specified if <i>NS</i> < 2.
.	.	.	.
.	.	.	.
.	.	.	.
19	Real	<i>Conc(NS,n)</i>	Initial value of the concentration of the last solute assigned to nodes of a particular horizontal layer <i>n</i> [ML <sup>-3</sup> ].
19	Real	<i>Sorb(1,n)</i>	Initial value of the sorbed concentration of the first solute for type-2 sorption sites assigned to nodes of a particular horizontal layer <i>n</i> [ML <sup>-3</sup> ]. This variable does not need to be specified if <i>NS</i> = 0 or <i>lEquil</i> = .true.
19	Real	<i>Sorb(2,n)</i>	Initial value of the sorbed concentration of the second solute for type-2 sorption sites assigned to nodes of a particular horizontal layer <i>n</i> [ML <sup>-3</sup> ]. This variable does not need to be specified if <i>NS</i> < 2 or <i>lEquil</i> = .true.
.	.	.	.
.	.	.	.
.	.	.	.
19	Real	<i>Sorb(NS,n)</i>	Initial value of the sorbed concentration of the last solute for type-2 sorption sites assigned to nodes of a particular horizontal layer <i>n</i> [ML <sup>-3</sup> ].

Table A.1. (continued)

Record	Type	Variable	Description
In general, record 19 information is required for each horizontal layer $n$ , starting with $n=1$ and continuing sequentially until $n=NLinZ$ . Record 19 information for certain horizontal layers may be skipped if several conditions are satisfied (see beginning of this section).			

Table A.2. Input file 'GENER2.IN' for the first example.

```
*** INPUT FILE 'GENER2.IN' ****
AxisymmetricFlow
f
Angle Aniz1 Anizz
0. 1. 1.
(NS LEquil      (number of solutes and equilibrium or nonequilibrium)
0 f
NCol NLin      (number of nodes in horizontal and vertical direction)
2 56
x(1) z(NLin)      (x, z-coordinate of left bottom node)
0 0
*** SPACE INCREMENTS ****
dx-array
1
dz-array
4*.25 2*.5 39*1 10*2
*** LINE ATTRIBUTES ****
LineNumber Code hInit Q MatNum Beta Axz Bxz Dxz Temp
1 1 .75 0. 1 0. 1. 1. 1. 0.
2 0 -150 0. 1 0. 1. 1. 1. 0.
55 0 -150 0. 1 0. 1. 1. 1. 0.
56 -2 -150 0. 1 0. 1. 1. 1. 0.
*** END OF FILE 'GENER2.IN' ****
```

Table A.3. Input file 'GENER2.IN' for the second example.

```
*** INPUT FILE 'GENER2.IN' ****
AxisymmetricFlow          (Type of flow being analyzed)
f
Angle Aniz1 Anizz           (Anisotropy Information)
0.    1.    1.
NS    lEquil    (number of solutes and equilibrium or nonequilibrium)
0    f
NCol   NLin    (number of nodes in horizontal and vertical direction)
2    33
x(1)  z(NLin)           (x, z-coordinate of left bottom node)
0    0
*** SPACE INCREMENTS ****
dx-array                   (number of items is NCol-1)
1
dz-array                   (number of items is NLin-1)
2*1 2*2 4 4*5 19*10 5 3 2*1
*** LINE ATTRIBUTES ****
LineNumber  Code  hInit   Q  MatNum Beta  Axz  Bxz  Dxz  Temp
1      -4     -55.  0.  1   0.  1.  1.  1.  0.
2      0     -54.  0.  1   0.  1.  1.  1.  0.
3      0     -53.  0.  1   1.  1.  1.  1.  0.
10     0     -25.  0.  1   1.  1.  1.  1.  0.
11     0     -15.  0.  1   0.  1.  1.  1.  0.
12     0     -5.   0.  2   0.  1.  1.  1.  0.
32     0     174.  0.  2   0.  1.  1.  1.  0.
33     -3    175.  0.  2   0.  1.  1.  1.  0.
*** END OF FILE 'GENER2.IN' ****
```

Table A.4. Input file 'GENER2.IN' for the third example.

```

*** INPUT FILE 'GENER2.IN' ****
AxisymmetricFlow
      f
Angle Aniz1 Anizz
      0.    1.    1.
      (Anisotropy Information)
NS    LEquil   (number of solutes and equilibrium or nonequilibrium)
      1      t
NCol  NLin    (number of nodes in horizontal and vertical direction)
      15     21
x(1) z(NLin) (x, z-coordinate of left bottom node)
      0     -200
*** SPACE INCREMENTS ****
dx-array
      4*10 5 2 4 2*5 7 8 10 15 20
      (number of items is NCol-1)
dz-array
      12*5 4*10 4*25
      (number of items is NLin-1)
*** LINE ATTRIBUTES ****
LineNumber  Code hInit Q MatNum Beta Axz Bxz Dxz Temp Conc
      1       1   0.  0.  1   0.  1.  1.  1.  0.  0.
      2       0   0.  0.  1   0.  1.  1.  1.  0.  0.
      20      0   0.  0.  1   0.  1.  1.  1.  0.  0.
      21      2   0.  0.  1   0.  1.  1.  1.  0.  0.
*** END OF FILE 'GENER2.IN' ****

```

**Table A.5. Input file 'GENER2.IN' for the forth example.**

```
*** INPUT FILE 'GENER2.IN' ****
AxisymmetricFlow
f
Angle Aniz1 Anizz
0. 1. 1.
          (Anisotropy Information)
NS LEquil (number of solutes and equilibrium or nonequilibrium)
3 t
NCol NLin (number of nodes in horizontal and vertical direction)
2 201
x(1) z(NLin) (x, z-coordinate of left bottom node)
0 -200
*** SPACE INCREMENTS ****
dx-array (number of items is NCol-1)
1
dz-array (number of items is NLin-1)
200*1
*** LINE ATTRIBUTES ****
LineN Code hInit Q MatNum Beta Axz Bxz Dxz Temp Conc1 Conc2 Conc3
1 1 0.0 0. 1 0. 1. 1. 1. 0. 0. 0. 0.
2 0 0.0 0. 1 0. 1. 1. 1. 0. 0. 0. 0.
200 0 0.0 0. 1 0. 1. 1. 1. 0. 0. 0. 0.
201 2 0.0 0. 1 0. 1. 1. 1. 0. 0. 0. 0.
*** END OF FILE 'GENER2.IN' ***
```

Table A.6. Input file 'GENER2.IN' for the fifth example.

```
*** INPUT FILE 'GENER2.IN' ****
AxisymmetricFlow          (Type of flow being analyzed)
  f
Angle Aniz1 Anizz          (Anisotropy Information)
  0.    1.    1.
NS    lEquil    (number of solutes and equilibrium or nonequilibrium)
  1      t
NCol   NLin     (number of nodes in horizontal and vertical direction)
  2      44
x(1)  z(NLin)           (x, z-coordinate of left bottom node)
  0    -10.75
*** SPACE INCREMENTS ****
dx-array                  (number of items is NCol-1)
  1
dz-array                  (number of items is NLin-1)
  43*.25
*** LINE ATTRIBUTES ****
LineN Code hInit  Q  MatNum Beta Axz Bxz Dxz Temp Conc
  1   1    0.0  0.   1.   0.   1.   1.   1.   0.   0.
  2   0    0.0  0.   1.   0.   1.   1.   1.   0.   0.
  43  0    0.0  0.   1.   0.   1.   1.   1.   0.   0.
  44  2    0.0  0.   1.   0.   1.   1.   1.   0.   0.
*** END OF FILE 'GENER2.IN' ****
```

Table A.7. Input file 'GENER2.IN' for the sixth example.

```
*** INPUT FILE 'GENER2.IN' ****
AxisymmetricFlow
f
Angle Aniz1 Anizz
0. 1. 1.
          (Anisotropy Information)
NS    LEquil   (number of solutes and equilibrium or nonequilibrium)
1     f
NCol  NLin    (number of nodes in horizontal and vertical direction)
2     16
x(1) z(NLin) (x, z-coordinate of left bottom node)
0     -30.
*** SPACE INCREMENTS ****
dx-array
1
dz-array
15*2
*** LINE ATTRIBUTES ****
LineN Code hInit Q MatNum Beta Axz Bxz Dxz Temp Conc Sorb
1     1    0.0  0.  1   0.  1.  1.  1.  0.  0.  0.
2     0    0.0  0.  1   0.  1.  1.  1.  0.  0.  0.
15    0    0.0  0.  1   0.  1.  1.  1.  0.  0.  0.
16    2    0.0  0.  1   0.  1.  1.  1.  0.  0.  0.
*** END OF FILE 'GENER2.IN' ****
```

Table A.8. Input file 'GENER2.IN' for the seventh example.

```

*** INPUT FILE 'GENER2.IN' ****
AxisymmetricFlow          (Type of flow being analyzed)
t
Angle Aniz1 Anizz           (Anisotropy Information)
0.   1.   1.
NS    LEquil    (number of solutes and equilibrium or nonequilibrium)
3     t
NCol  NLin     (number of nodes in horizontal and vertical direction)
20    19
x(1) z(NLin)            (x, z-coordinate of left bottom node)
0    100
*** SPACE INCREMENTS ****
dx-array                  (number of items is NCol-1)
5 5 4 3 2 2 2 3 4 5 6 7 8 9 10 11 12 13 14
dz-array                  (number of items is NLin-1)
3*2 4 4*5 10*10
*** LINE ATTRIBUTES ****
LineN Code hInit Q MatNum Beta Axz Bxz Dxz Temp Conc1 Conc2 Conc3
1   -6  -147.4 0.  1   0.  1.  1.  1.  20.  0.0  0.0  0.0
2   0   -145.5 0.  1   0.  1.  1.  1.  20.  0.0  0.0  0.0
3   0   -143.4 0.  1   0.  1.  1.  1.  20.  0.0  0.0  0.0
4   0   -141.0 0.  1   0.  1.  1.  1.  20.  0.0  0.0  0.0
5   0   -135.6 0.  1   0.  1.  1.  1.  20.  0.0  0.0  0.0
6   0   -127.7 0.  1   0.  1.  1.  1.  20.  0.0  0.0  0.0
7   0   -119.0 0.  1   0.  1.  1.  1.  20.  0.0  0.0  0.0
8   0   -109.9 0.  1   0.  1.  1.  1.  20.  0.0  0.0  0.0
9   0   -100.5 0.  1   0.  1.  1.  1.  20.  0.0  0.0  0.0
10  0   -82.8  0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
11  0   -71.0  0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
12  0   -60.3  0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
13  0   -49.8  0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
14  0   -39.6  0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
15  0   -29.5  0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
16  0   -19.4  0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
17  0   -9.4   0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
18  0   0.6   0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
19  0   10.2  0.  2   0.  1.  1.  1.  20.  0.0  0.0  0.0
*** END OF FILE 'GENER2.IN' ****

```