



POLITECNICO
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DEPARTMENT OF
MECHANICAL ENGINEERING

ADVANCED DYNAMICS OF MECHANICAL SYSTEMS

**Finite Elements Method (FEM)
in structural dynamics:
software implementation in
Matlab environment – part 1**

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PROCEDURE FOR BUILDING A FE MODEL OF A STRUCTURE

1. Mesh generation
[USER INPUT: *.inp]
2. Definition of the global and local reference systems
[USER INPUT: *.inp]
3. Removal of external constraints and introduction of corresponding constraint forces
[USER INPUT: *.inp]
4. Energy functions formulation in the local nodal coordinates of each element
[FEM PROGRAM: loadstructure()]
5. Coordinate transformation from the local to the global reference system
[FEM PROGRAM: assem(), calling el_tra()]
6. Matrix assembling, for the entire structure
[FEM PROGRAM: assem()]

```
Main.m

% structure data
m = ...

% check max element length
Lmax = ...

% build *.inp file (mesh, constraints)

loadstructure()
    % nodes definition
    % elements definition

% draw structure
dis_stru()

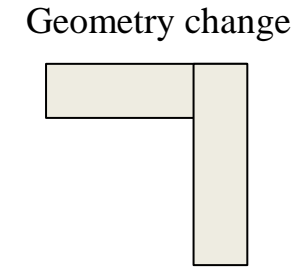
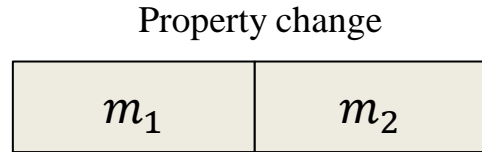
% build and assemble matrices
assem()
el_tra() % build M and K local ref.

% assemble total M and K
```

MESH GENERATION

We must consider

1. Discontinuities
2. Element length



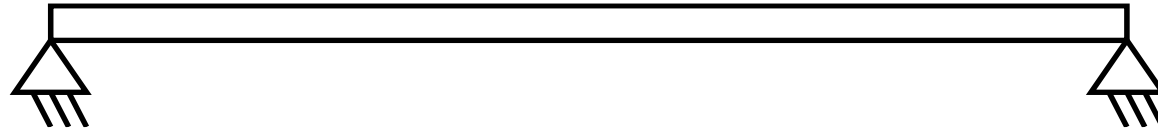
The element must work in quasi-static region!

- The element first natural frequency is $\omega_k^{(1)} = \left(\frac{\pi}{L_k}\right)^2 \sqrt{\frac{EJ_k}{m_k}}$
- $\omega_k^{(1)}$ should be sufficiently greater than the maximum frequency of interest Ω_{max}
- Typically, we can produce a mesh such that $\frac{\omega_k^{(1)}}{\Omega_{max}} \geq 1.5$
- Thus

$$L_{max} = \sqrt{\frac{\pi^2}{1.5 \Omega_{max}} \sqrt{\left(\frac{EJ}{m}\right)_{min}}}$$

EXAMPLE – PROPERTIES OF THE SYSTEM

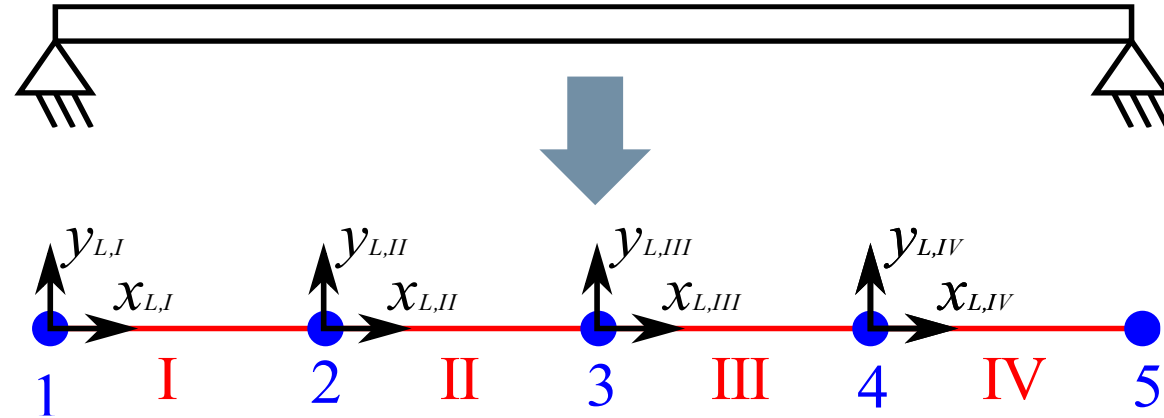
A simply supported aluminum beam with rectangular constant cross-section



Parameter	symbol	unit	value
Length	L	mm	1200
Thickness	h	mm	8
Width	b	mm	40
Density	ρ	kg/m ³	2700
Young's Modulus	E	GPa	68

$$\Omega_{max} = 100 \, 2\pi \quad \Rightarrow \quad L_{max} = \sqrt{\frac{\pi^2}{1.5 \, \Omega_{max}} \sqrt{\left(\frac{EJ}{m}\right)_{min}}} = 348 \, mm$$

EXAMPLE – MESHING



We must define:

- Nodes:
 - Constraints (x, y, θ)
 - Coordinates (x, y)
- Elements:
 - Connected nodes
 - Properties (m, EA, EJ)

Constraint on x: 1 true, 0 false

Node number

```

*NODES
n - constr.(x,y,theta) - coord. x,y
1 1 0 0 0
2 0 0 0 0.3 0
3 ..
    
```

coordinates

Element number

```

*BEAMS
n - input node - output node - property
1 1 2 1
2 2 3 1
3 ..
    
```

Element property number

Connected nodes

```

*PROPERTIES
1 0.864 2.176e7 1.1605e2
    
```

Property number

m, EA, EJ

INPUT FILE: ADMS_FEM_01.INP

File extension

```
! FEM(1)
! 1st Exercise
! -----
! list of nodes :
*NODES
! n. of node - constraint code (x,y,theta) - x coordinate - y coordinate.
1 1 1 0 0.0 0.0
2 0 0 0 0.3 0.0
3 0 0 0 0.6 0.0
4 0 0 0 0.9 0.0
5 1 1 0 1.2 0.0
! end card *NODES
*ENDNODES
! -----
! list of elements :
*BEAMS
! n. of elem. - n. of input node - n. of output node - n. of prop.
1 1 2 1
2 2 3 1
3 3 4 1
4 4 5 1
*ENDBEAMS
! -----
! List of properties :
*PROPERTIES
! N. of prop. - m - EA - EJ
1 0.864 2.176e7 1.1605e2
*ENDPROPERTIES
```

Start comments with «!»: the compiler ignores the following characters
NO white lines admitted

*NODES start definition of nodes

*ENDNODES end definition of nodes

*BEAMS start definition of beam finite elements

*ENDBEAMS end definition of beam finite elements

*PROPERTIES start definition of element properties

*ENDPROPERTIES end definition of beam finite elements properties

INPUT FILE PROCESSING

- **loadstructure()**

Processes the *.inp file and returns some useful variables

```
[file_i,xy,nnod,sizew,idb,ndof,incid,l,gamma,m,EA,EJ,posit,nbeam,pr]=loadstructure;
```

- **assem()**

Taking info from previous function outputs, assembles M and K matrices

```
[M,K]=assem(incid,l,m,EA,EJ,gamma,idb);
```

The final matrices M and K are of the whole system (free and constrained), as we'll see in the following.

INPUT FILE PROCESSING: LOADSTRUCTURE()

loadstructure() function

Call

```
[file_i,xy,nnod,sizew,idb,ndof,incid,l,gamma,m,EA,EJ,posit,nbeam,pr]=loadstructure;
```

Outputs

file_i	name of the *.inp file analysed
xy	$N \times 2$ matrix containing the coordinates of the nodes
nnod	total number of nodes of the structure
sizew	maximum dimension of the structure
idb	$N \times 3$ matrix, numbering each degree of freedom (free and constrained) with different progressive numbers. N is the number of nodes, 3 are the degrees of freedom for each node $(1, 2, 3) = (x, y, \theta)$
ndof	number of total degrees of freedom
incid	incidence matrix $N \times 6$, same idea as for idb, but with N number of elements and 6 the degrees of freedom of each element: $(1, 2, 3, 4, 5, 6) = (x_1, y_1, \theta_1, x_2, y_2, \theta_2)$

INPUT FILE PROCESSING: LOADSTRUCTURE()

loadstructure() function

Call

```
[file_i,xy,nnod,sizew,idb,ndof,incid,l,gamma,m,EA,EJ,posit,nbeam,pr]=loadstructure;
```

Outputs

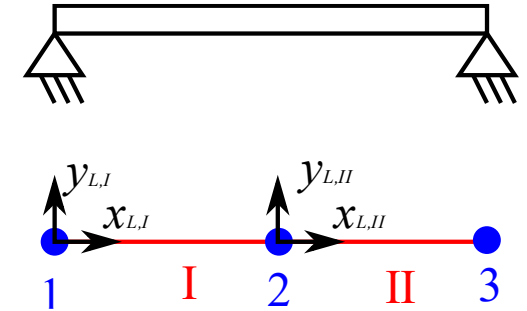
l	vector containing the length of each element
gamma	vector containing the rotation angle of each element with respect to the global reference system
m	vector containing the mass per unit length of each element
EA, EJ	vectors containing EA and EJ for each elements
posit	$N \times 2$ matrix containing the xy positions of the elements
nbeam	number of elements
pr	vector containing the properties of each element

INPUT FILE PROCESSING: LOADSTRUCTURE()

What are **idb** and incidence matrix **incid** used for? Consider this simple example

$$\text{idb} = \begin{bmatrix} 6 & 7 & 1 \\ 2 & 3 & 4 \\ 8 & 9 & 5 \end{bmatrix}$$

$$\text{incid} = \begin{bmatrix} 6 & 7 & 1 & 2 & 3 & 4 \\ 2 & 3 & 4 & 8 & 9 & 5 \end{bmatrix}$$



If we want to know which is the index of the row, in the assembled matrices, corresponding to the θ DoF of the second node (mid span), we use **idb** matrix as:

$$\text{index} = \text{idb}(2, 3) = 4$$

Whereas if we want to know which is the index of the row corresponding to the y DoF of the second node of the second element (II), we would type:

$$\text{index} = \text{incid}(2, 3+2) = 9$$

EXAMPLE: MAIN CODE

```
clear all; close all; clc

L = 1.2;           % [m]
E = 68e9;          % [Pa]
b = 40e-3;         % [m]
h = 8e-3;          % [m]
r = 2700;          % [kg/m^3]
m = r*b*h;         % [kg/m]
J = 1/12*b*h^3;    % [m^4]
A = b*h;           % [m^2]
EA = E*A;          % [N]
EJ = E*J;          % [Nm^2]

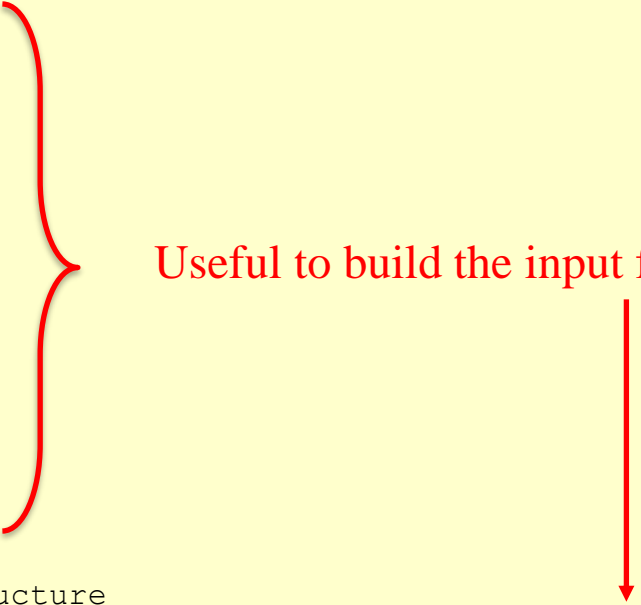
Omax = 100*2*pi;
a = 1.5;
Lmax = sqrt(pi^2/a/Omax * sqrt(EJ/m));

% load the input file and assemble the structure
[file_i,xy,nnod,sizew,idb,ndof,incid,l,gamma,m,EA,EJ,posit,nbeam,pr]=loadstructure;

% draw the structure
dis_stru(posit,l,gamma,xy,pr,idb,ndof);

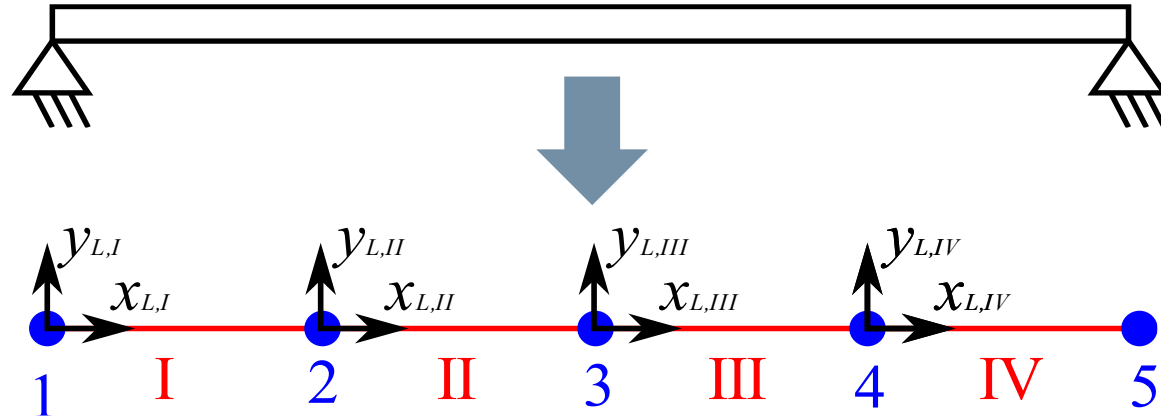
% assemble mass and stiffness matrices
[M,K]=assem(incid,l,m,EA,EJ,gamma,idb);
```

Useful to build the input file



PLOT THE UNDEFORMED STRUCTURE

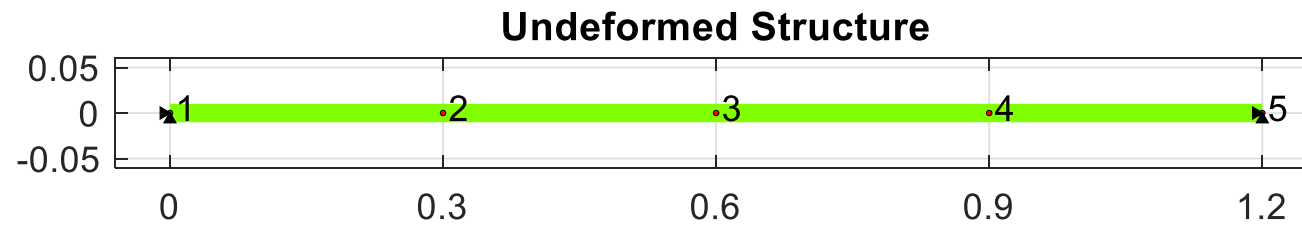
1. Meshing



4 beam elements (300 mm)
and 5 nodes

2. Coding

```
dis_stru(posit,l,gamma,xy,pr,idb,ndof);
```



ASSEMBLY: MASS MATRIX M

```
[M,K]=assem(incid,l,m,EA,EJ,gamma,idb);
```

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
1	2.2217e-04	0	0.0024	-1.6663e-04	0	0	0	0	0	0	0	0	0	0.0041	0
2	0	0.1728	0	0	0.0432	0	0	0	0	0	0	0	0.0432	0	0
3	0.0024	0	0.1925	0	0	0.0333	-0.0024	0	0	0	0	0	0	0.0333	0
4	-1.6663e-04	0	0	4.4434e-04	0	0.0024	-1.6663e-04	0	0	0	0	0	0	-0.0024	0
5	0	0.0432	0	0	0.1728	0	0	0.0432	0	0	0	0	0	0	0
6	0	0	0.0333	0.0024	0	0.1925	3.4694e-18	0	0.0333	-0.0024	0	0	0	0	0
7	0	0	-0.0024	-1.6663e-04	0	3.4694e-18	4.4434e-04	0	0.0024	-1.6663e-04	0	0	0	0	0
8	0	0	0	0	0.0432	0	0	0.1728	0	0	0.0432	0	0	0	0
9	0	0	0	0	0	0.0333	0.0024	0	0.1925	-4.3368e-18	0	-0.0024	0	0	0.0333
10	0	0	0	0	0	-0.0024	-1.6663e-04	0	-4.3368e-18	4.4434e-04	0	-1.6663e-04	0	0	0.0024
11	0	0	0	0	0	0	0	0.0432	0	0	0.0864	0	0	0	0
12	0	0	0	0	0	0	0	0	-0.0024	-1.6663e-04	0	2.2217e-04	0	0	-0.0041
13	0	0.0432	0	0	0	0	0	0	0	0	0	0	0.0864	0	0
14	0.0041	0	0.0333	-0.0024	0	0	0	0	0	0	0	0	0	0.0963	0
15	0	0	0	0	0	0	0	0	0.0333	0.0024	0	-0.0041	0	0	0.0963

The mass matrix M is symmetric

➤ Similarly, also the stiffness matrix K is symmetric