

# Summary

- Topics:
  1. Bio-medical imaging
  2. Image processing
  3. Geometric modeling and computer graphics
  4. Mesh generation
    - Marching Cubes/Dual Contouring
    - Tri/Tet Meshing
    - Quad/Hex Meshing
    - Quality Improvement
  5. Computational mechanics
  6. Bio-medical applications

# **Topic 5: Introduction to Finite Element Method**

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# Overview

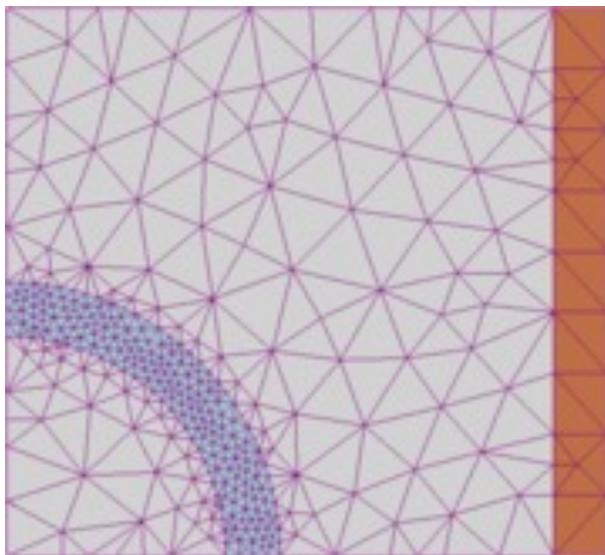
- What is Finite Element Method?
- A One-dimensional Model Problem
  - Boundary value problem (BVP)
  - Weak/variational formulation
  - Galerkin approximations
  - Basis functions
  - Finite element calculations
  - Flow chart
  - Interpretation of the approximate solution
  - Accuracy of the finite element approximation

# Finite Element Method

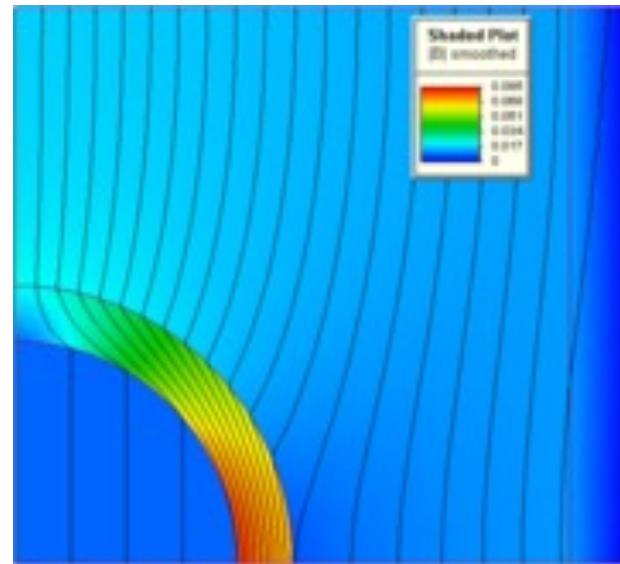
- The finite element method is a general technique for constructing approximate solutions to **boundary-value problems (BVP)**.
- This method involves dividing the domain of the solution into a finite number of simple subdomains, the finite elements, and using the variational concepts to construct an approximation of the solution over the collection of finite elements.

# Finite Element Method

- Due to the generality and richness of the ideas underlying the method, it has been used with remarkable success in solving a wide range of problems in a lot of research fields.



2D mesh for the right image (mesh is denser around the object of interest)



2D FEM solution for a magnetostatic configuration (lines denote the direction of calculated flux density and color—its magnitude)

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# The Model Problem

- A one-dimensional “two-point” boundary-value problem characterized by
  - a simple linear ordinary differential equation of second order
  - a pair of boundary conditions

# The Statement of The Model Problem

- Consider the problem of finding a function  $u = u(x)$ ,  $0 \leq x \leq 1$ , satisfying the following differential equation and boundary conditions:

$$\begin{cases} -u'' + u = f(x), & 0 < x < 1 \\ u(0) = 0, & u(1) = 0 \end{cases}$$

where  $u'' = d^2u / dx^2$ , and  $f(x) = x$ .

- A problem such as this might be the study of the deflection of a string on an elastic foundation or of the temperature distribution in a rod.

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# Weak/Variational Statement

- The requirement that a solution  $u$  to the BVP satisfies the differential equation at *every point*  $x$ ,  $0 < x < 1$ , is too **strong**.
- To overcome this difficulty, we shall reformulate the BVP in a way that will admit weaker conditions on the solution and its derivatives. Such reformulation is called **weak** or **variational** formulation.
- Whenever a smooth “classical” solution to a problem exists, it is also the solution of the weak problem.

# Weak/Variational Formulation

- Consider the same BVP problem:

$$\begin{aligned} -u'' + u &= x, & 0 < x < 1 \\ u(0) &= 0. & u(1) = 0 \end{aligned} \quad (1)$$

- The weak format is:

$$\int_0^1 (-u'' + u)v dx = \int_0^1 xv dx$$

for all members  $v$  of a suitable class of functions.  $v$  is the *weight function* or the *test function*.

# Weak Statement

- The set of functions with zero values at  $x = 0$  and  $x = 1$  is denoted by the symbol  $H$ .
- The weak statement assumes the more compact form: find  $u$  such that:

$$\left. \begin{array}{l} \int_0^1 (-u'' + u - x)v dx = 0 \quad \text{for all } v \in H \\ u(0) = 0 \\ u(1) = 0 \end{array} \right\} \quad (2)$$

# A Symmetric Variational Formulation

- If  $u$  and  $v$  are sufficiently smooth functions, then the standard integration-by-parts formula holds:

$$\int_0^1 -u''v dx = \int_0^1 u'v' dx - u'v \Big|_0^1.$$

- If we demand the test functions vanish at the endpoints,  $v(0) = v(1) = 0$ , then we have:

$$\int_0^1 -u''v dx = \int_0^1 u'v' dx$$

for all admissible test functions  $v$ .

## Weak Statement

- The weak statement becomes: find  $u \in H_0^1$  such that

$$\int_0^1 (u'v' + uv - xv) dx = 0 \quad \text{for all } v \in H_0^1 \quad (3)$$

- The set of admissible function  $u \in H_0^1$  is defined as the set of all functions that vanish at the endpoints and whose 1<sup>st</sup> derivatives are *square-integrable*. Thus, a function  $w$  is a member of  $H_0^1$  if

$$\int_0^1 (w')^2 dx < \infty \quad \text{and} \quad w(0) = 0 = w(1)$$

# Properties of $H_0^1$

- **A linear space of functions:** if  $v_1$  and  $v_2$  are arbitrary test functions and  $\alpha$  and  $\beta$  are arbitrary constants, then  $\alpha v_1 + \beta v_2$  is also a test function.
- **Infinite-dimensional:** if we introduce the set of functions

$$\Psi_n(x) = \sqrt{2} \sin n\pi x, \quad n = 1, 2, 3, \dots$$

(trigonometric function) and  $v$  is a smooth test function in  $H_0^1$ , then it is easily verified that  $v$  can be represented in the form

$$v(x) = \sum_{n=1}^m a_n \Psi_n(x) \quad a_n = \int_0^1 v(x) \Psi_n(x) dx$$

- An infinity of coefficients  $a_n$  must be specified in order to define any function  $v \in H_0^1$ ; therefore the space  $H_0^1$  of admissible function is infinite-dimensional.

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# Galerkin Approximations

- Given an infinite set of functions in  $H_0^1$

$$\{\phi_1(x), \phi_2(x), \phi_3(x), \dots\}$$

Each test function  $v$  in  $H_0^1$  can be represented as a linear combination of them

$$v(x) = \sum_{i=1}^{\infty} \beta_i \phi_i(x)$$

where  $\beta_i$  are constants,  $\phi_i$  are called *basis functions*.

# Approximation with Finite Number Basis Functions

- If we take only a finite number  $N$  of terms, then we will obtain only an approximation of  $v$

$$v_N(x) = \sum_{i=1}^N \beta_i \phi_i(x). \quad (4)$$

- The  $N$  basis functions

$$\{\phi_1(x), \phi_2(x), \dots, \phi_N(x)\}$$

define an  $N$ -dimensional subspace  $H_0^N$  of  $H_0^1$ .

# Galerkin Approximations

- Galerkin's method consists of seeking an approximate solution to

$$\int_0^1 (u'v' + uv - xv) dx = 0 \quad \text{for all } v \in H_0^1$$

in a finite-dimensional subspace  $H_0^N$  of the space of  $H_0^1$  admissible functions rather than in the whole space  $H_0^1$ .

- We seek an approximate solution in  $H_0^N$ ,

$$u_N(x) = \sum_{i=1}^N \alpha_i \phi_i(x). \quad (5)$$

# Galerkin Approximations

- The weak form becomes: find  $u_N \in H_0^N$  such that

$$\int_0^1 (u'_N v'_N + u_N v_N) dx = \int_0^1 x v_N dx \quad \text{for all } v_N \in H_0^N \quad (6)$$

$$u_N(x) = \sum_{i=1}^N \alpha_i \phi_i(x)$$

- Since the  $\phi_i$  are known,  $u_N$  will be completely determined once the  $N$  coefficients  $\alpha_i$  are determined.  $\alpha_i$  are referred to as the *degrees of freedom* of the approximation.

# Galerkin Approximations

- Plug the approximate solution  $u_N$  and the test function  $v_N$  into the weak statement (Eqn.6):

$$\int_0^1 (u'_N v'_N + u_N v_N) dx = \int_0^1 x v_N dx$$

$$u_N(x) = \sum_{i=1}^N \alpha_i \phi_i(x) \quad \rightarrow \quad \downarrow \quad \leftarrow \quad v_N(x) = \sum_{i=1}^N \beta_i \phi_i(x)$$

$$\begin{aligned} & \int_0^1 \left\{ \frac{d}{dx} \left[ \sum_{i=1}^N \beta_i \phi_i(x) \right] \frac{d}{dx} \left[ \sum_{j=1}^N \alpha_j \phi_j(x) \right] + \left[ \sum_{i=1}^N \beta_i \phi_i(x) \right] \left[ \sum_{j=1}^N \alpha_j \phi_j(x) \right] \right. \\ & \left. - x \sum_{i=1}^N \beta_i \phi_i(x) \right\} dx = 0 \quad \text{for all } \beta_i, i = 1, 2, \dots, N \end{aligned}$$

Expanding it and factoring the coefficients  $\beta_i$  gives

$$\sum_{i=1}^N \beta_i \left( \sum_{j=1}^N \left\{ \int_0^1 [\phi'_i(x) \phi'_j(x) + \phi_i(x) \phi_j(x)] dx \right\} \alpha_j - \int_0^1 x \phi_i(x) dx \right) = 0$$

for all  $\beta_i, i = 1, 2, \dots, N$

$$\phi'_i(x) = d\phi_i(x)/dx.$$



# Galerkin Approximations

- Rewrite it in the more compact form:

$$\sum_{i=1}^N \beta_i \left( \sum_{j=1}^N K_{ij} \alpha_j - F_i \right) = 0$$

for all choices of  $\beta_i$ , where

$$K_{ij} = \int_0^1 [\phi'_i(x) \phi'_j(x) + \phi_i(x) \phi_j(x)] dx \quad (7)$$

$$F_i = \int_0^1 x \phi_i dx \quad i, j = 1, 2, \dots, N$$

- The  $N \times N$  matrix  $K = [K_{ij}]$  is referred to as the stiffness matrix, and the  $N \times 1$  column vector  $F = \{F_i\}$  is referred to as the load/force vector.

# Galerkin Approximations

- Because  $\beta_i$  is arbitrary, we can obtain

$$\sum_{j=1}^N K_{ij} \alpha_j = F_i, \quad i = 1, 2, \dots, N$$



$$\alpha_i = \sum_{l=1}^N (K^{-1})_{il} F_l$$

- The approximation solution is:

$$u_N(x) = \sum_{i=1}^N \alpha_i \phi_i(x).$$

# Galerkin Methods

- The stiffness matrix is symmetric. This symmetry provides us with an opportunity for reducing the computational effort.
- For a symmetric formulation, the Galerkin's method provides the best possible approximation of the solution.
- For a symmetric formulation, the spaces of trial functions and test functions coincide, only one set of basis functions  $\phi_i$  need to be constructed.

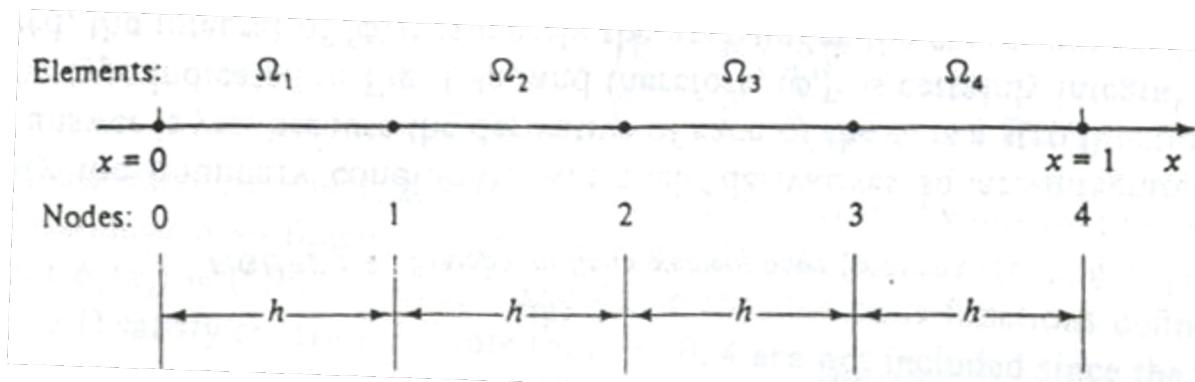
$$K_{ij} = \int_0^1 [\phi'_i(x)\phi'_j(x) + \phi_i(x)\phi_j(x)] dx$$

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# Finite Element Basis Functions

- The basis function can be defined piecewise over subregions of the domain called finite elements, e.g., piecewise polynomials of low degree.
- First we partition the domain ( $0 \leq x \leq 1$ ) into elements (mesh generation).

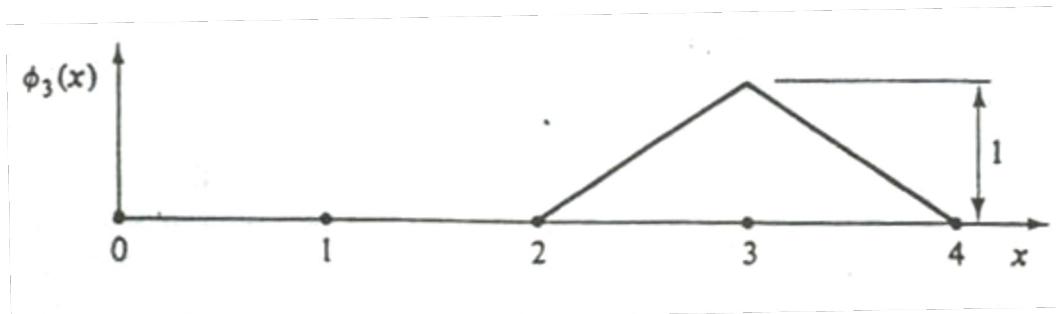
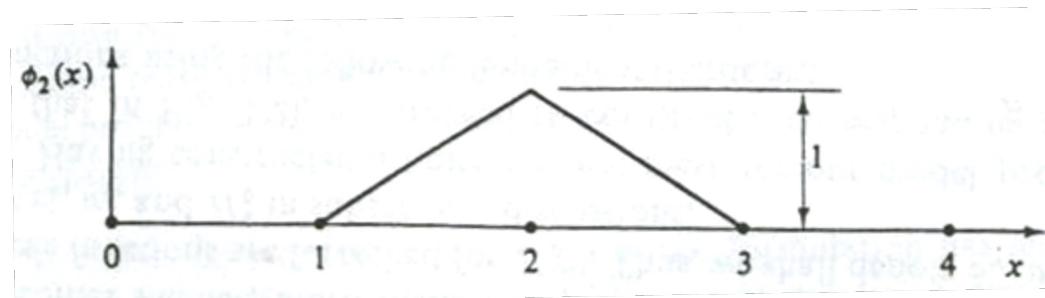
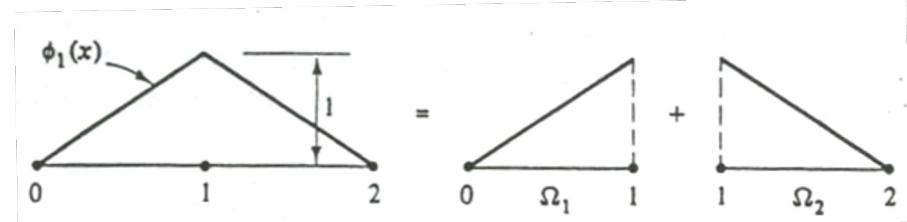
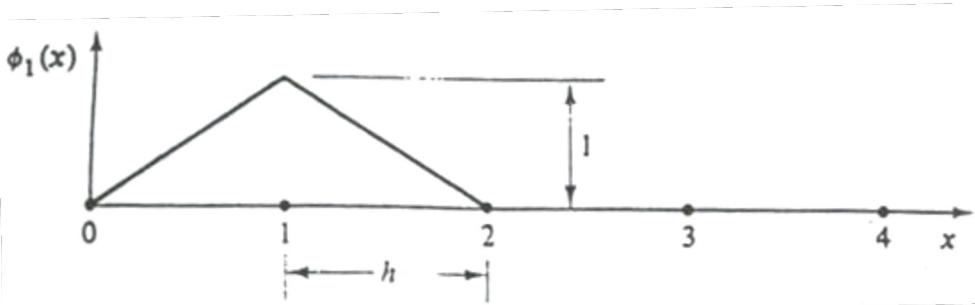


A finite element partition of the domain comprised of 4 elements with nodes at element endpoints.

# Finite Element Basis Functions

- Fundamental criteria to construct a set of basis functions:
  - The basis functions are generated by simple functions defined piecewise – element by element – over the finite element mesh.
  - The basis functions are smooth enough to be members of the class  $H_0^1$  of test functions.
  - The basis functions are chosen in such a way that the parameters  $\alpha_i$  define the approximation solution precisely at the nodal points.

# Finite Element Basis Functions (“Hat”-Function)



# Finite Element Basis Functions

- If the coordinates of the nodes are denoted  $x_i$  ( $i = 0, 1, 2, 3, 4$ ), then the basis functions shown for  $i = 1, 2$ , and  $3$  are given by

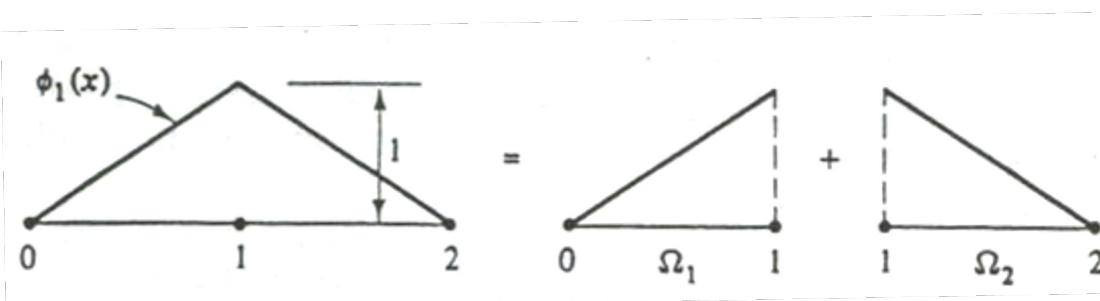
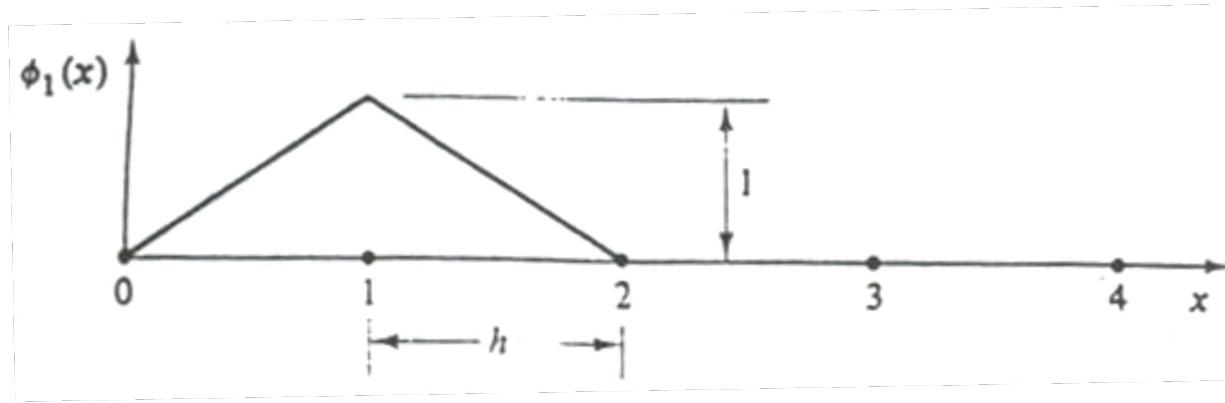
$$\phi_i(x) = \begin{cases} \frac{x - x_{i-1}}{h_i} & \text{for } x_{i-1} \leq x \leq x_i \\ \frac{x_{i+1} - x}{h_{i+1}} & \text{for } x_i \leq x \leq x_{i+1} \\ 0 & \text{for } x \leq x_{i-1} \text{ and } x \geq x_{i+1} \end{cases}$$

$$\phi'_i(x) = \begin{cases} \frac{1}{h_i} & \text{for } x_{i-1} < x < x_i \\ \frac{-1}{h_{i+1}} & \text{for } x_i < x < x_{i+1} \\ 0 & \text{for } x < x_{i-1} \text{ and } x > x_{i+1} \end{cases}$$

where  $h_i = x_i - x_{i-1}$  is the length of element  $\Omega_i$ .

# Check Criteria 1

- **Criteria 1:** The basis functions are generated by simple functions defined piecewise – element by element – over the finite element mesh.

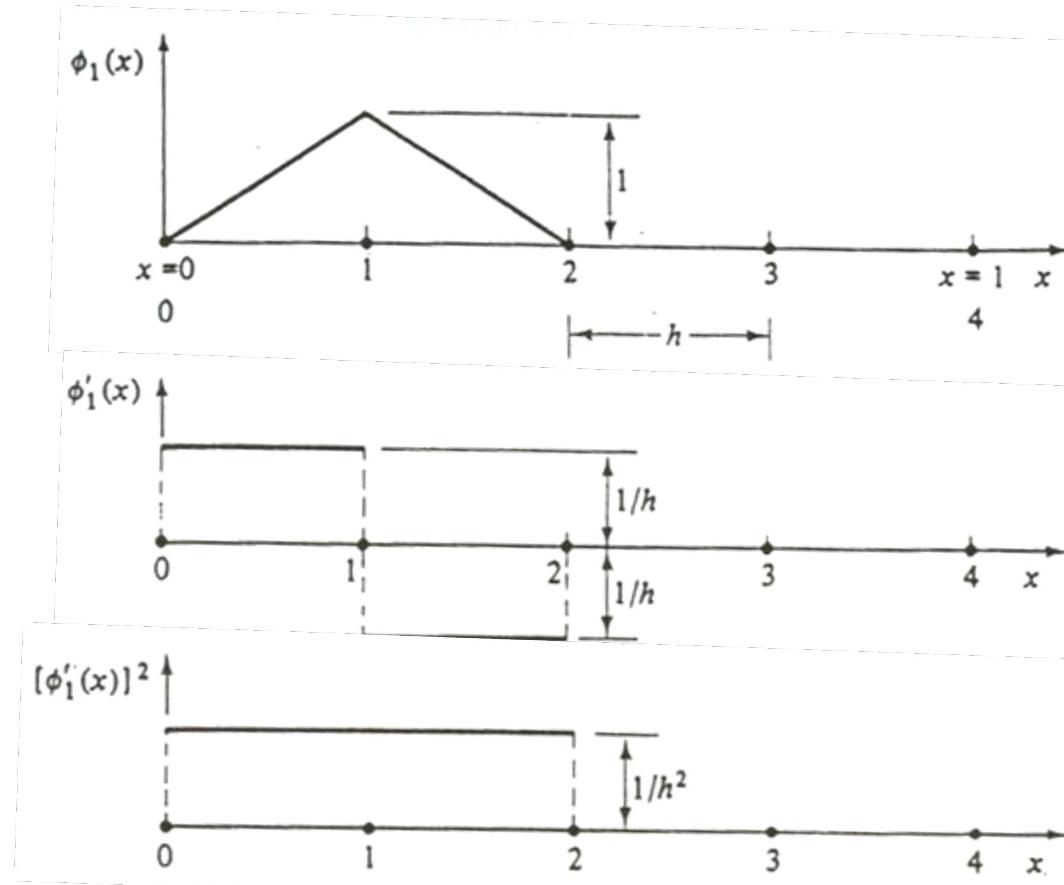


## Check Criteria 2

- **Criteria 2:** The basis functions are smooth enough to be members of the class  $H_0^1$  of test functions.
  - In order that  $\phi_i \in H_0^1$ ,  $i = 1, 2, 3$ , each must have square-integrable 1<sup>st</sup> derivatives and must vanish at  $x=0$  and  $x=1$ . It is obvious that the hat-function satisfies this condition.
  - Are their derivatives *square-integrable*? [Yes]

## Check Criteria 2

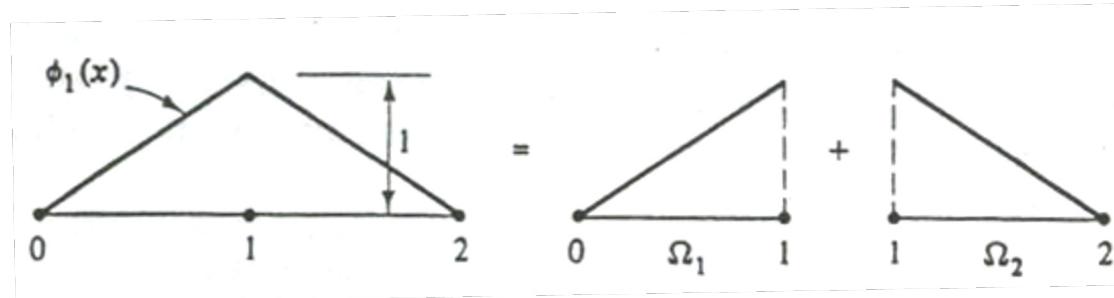
- Are their derivatives square-integrable? [Yes]



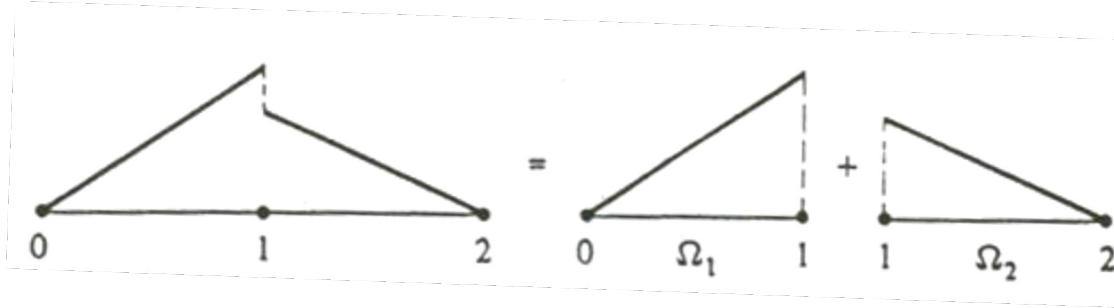
$$\int_0^1 [\phi'_1(x)]^2 dx = \frac{1}{h^2} 2h = 2h^{-1} < \infty$$



# Square-integrable vs Non-square-integrable



Square-integrable basis functions



Non-square-integrable basis functions

## Check Criteria 3

- **Criteria 3:** The basis functions are chosen in such a way that the parameters  $\alpha_i$  define the approximation solution precisely at the nodal points.
  - This criterion is not difficult to satisfy if each basis function is unity at one node and zero at all other nodes. If  $x_j$  is the  $x$ -coordinate of node  $j$ , then

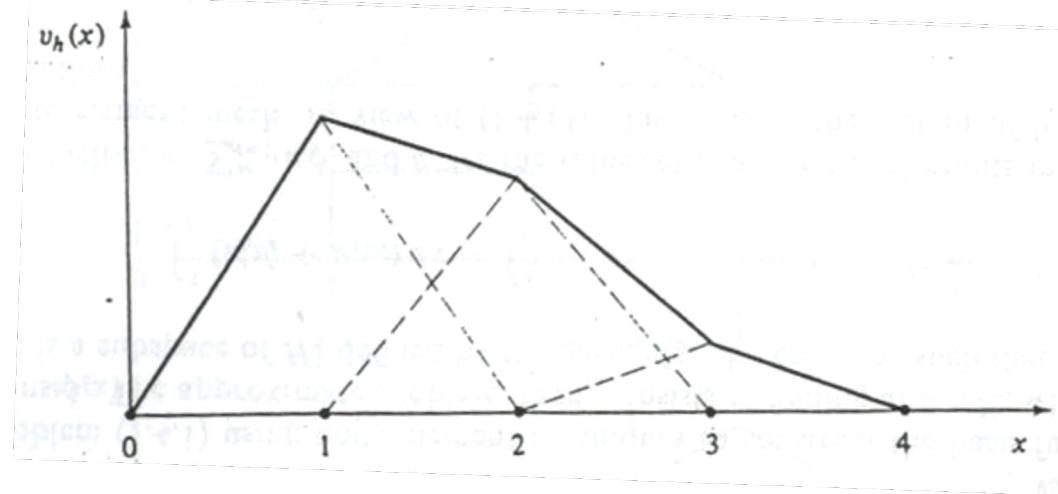
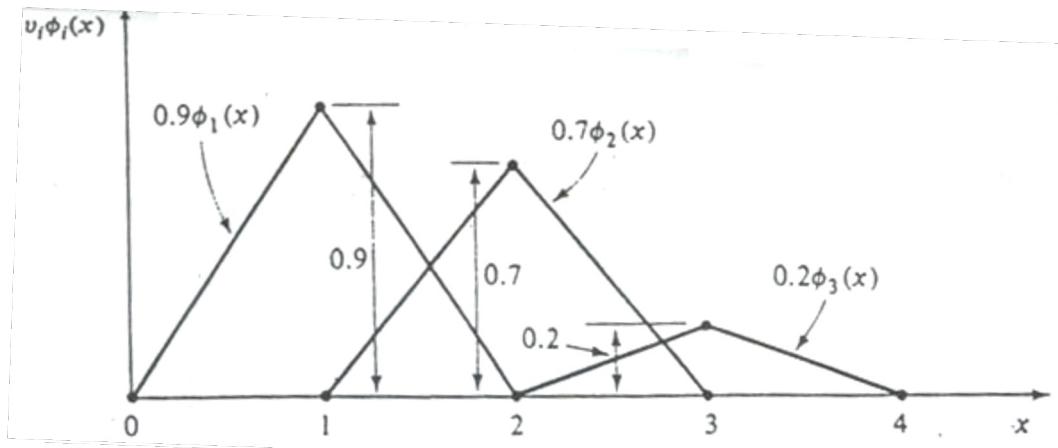
$$\phi_i(x_j) = \begin{cases} 1 & \text{if } i = j \\ 0 & \text{if } i \neq j \end{cases}$$

- Note that  $i = 0, 4$  are not included since the basis functions are required to satisfy the homogeneous end conditions.

# Basis Functions

Suppose  $N = 3$ ,  $\beta_1=0.9$ ,  $\beta_2=0.7$ , and  $\beta_3=0.2$ .

$$v_h(x) = \sum_{i=1}^N \beta_i \phi_i(x). \rightarrow v_h(x) = 0.9\phi_1(x) + 0.7\phi_2(x) + 0.2\phi_3(x)$$



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# Finite Element Calculations

- The weak form is: find  $u_h \in H_0^h$  such that

$$\int_0^1 (u'_h v'_h + u_h v'_h) dx = \int_0^1 x v_h dx \quad \text{for all } v_h \in H_0^h$$

$$u_h(x) = \sum_{i=1}^N u_i \phi_i(x).$$

$u_i$  are the values of  $u_h$  at the nodal points in the finite element mesh. This leads to the system of linear equations

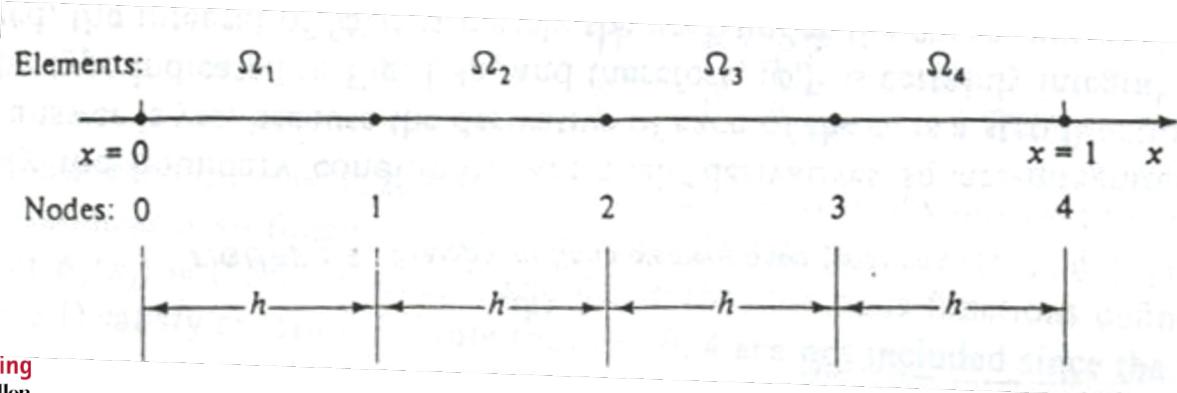
$$\sum_{j=1}^N K_{ij} \alpha_j = F_i, \quad i = 1, 2, \dots, N$$

# Stiffness Matrix and Load Vector

- Summability of stiffness

$$\begin{aligned} K_{ij} &= \int_0^1 (\phi'_i \phi'_j + \phi_i \phi_j) dx \\ &= \int_0^h (\phi'_i \phi'_j + \phi_i \phi_j) dx + \int_h^{2h} (\phi'_i \phi'_j + \phi_i \phi_j) dx \\ &\quad + \int_{2h}^{3h} (\phi'_i \phi'_j + \phi_i \phi_j) dx + \int_{3h}^1 (\phi'_i \phi'_j + \phi_i \phi_j) dx \\ &= \sum_{e=1}^4 \int_{\Omega_e} (\phi'_i \phi'_j + \phi_i \phi_j) dx \end{aligned}$$

$\int_{\Omega_e}$  denotes integration over element  $\Omega_e$ .



# Stiffness Matrix and Load Vector

- Let the term

$$K_{ij}^e = \int_{\Omega_e} (\phi_i' \phi_j' + \phi_i \phi_j) dx$$

represent components of the element stiffness matrix for finite element  $\Omega_e$ .

$$K_{ij} = \sum_{e=1}^4 K_{ij}^e$$

Similarly

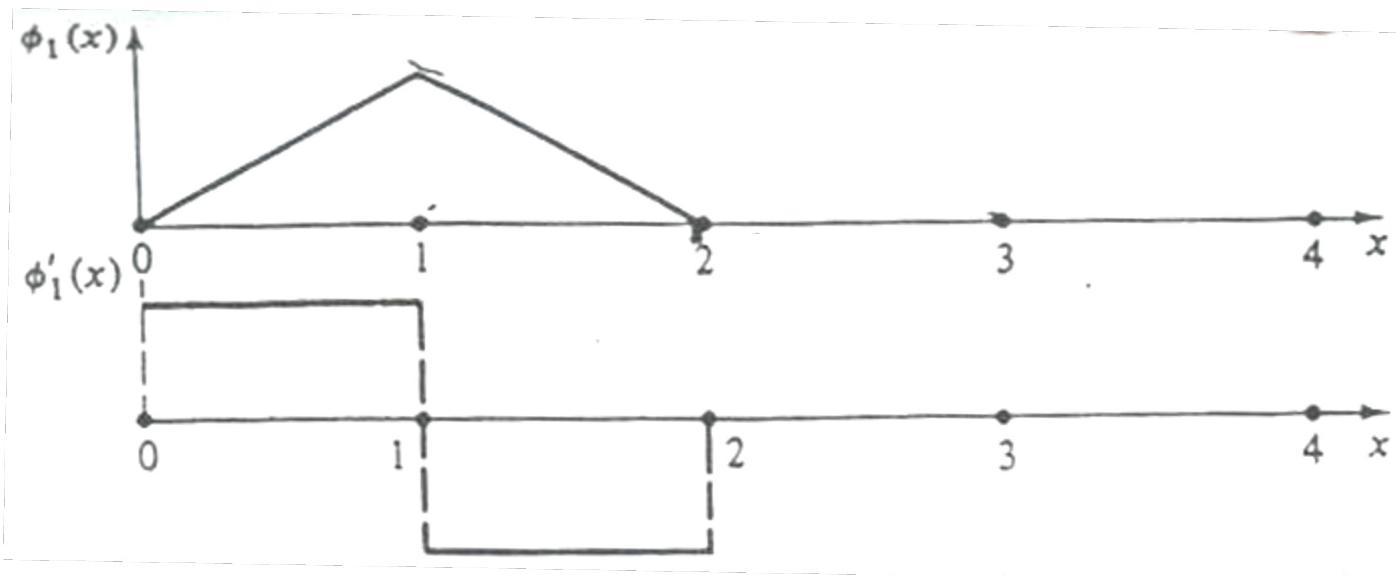
$$F_i = \sum_{e=1}^4 F_i^e, \quad F_i^e = \int_{\Omega_e} x \phi_i dx$$

# Stiffness Matrix and Load Vector

- The stiffness matrix and load vector can be computed as the sum of contributions from each element.
- First construct  $K^e$  and  $F^e$  for a typical element  $\Omega_e$ , then construct  $K$  and  $F$ .

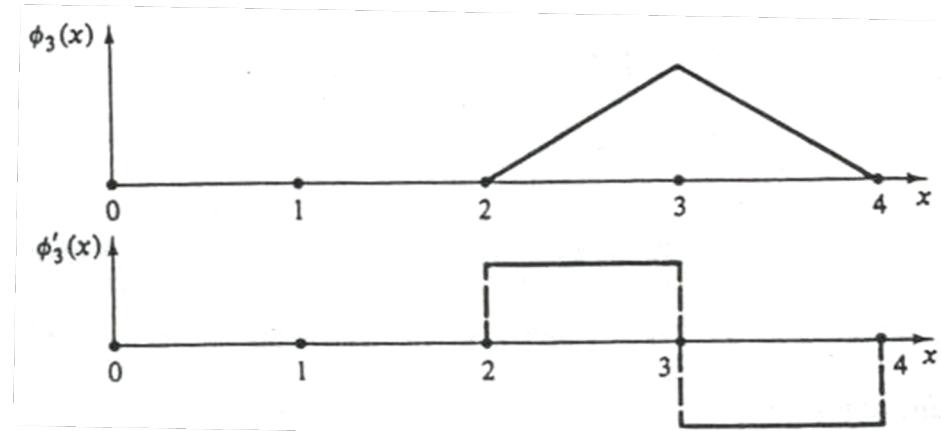
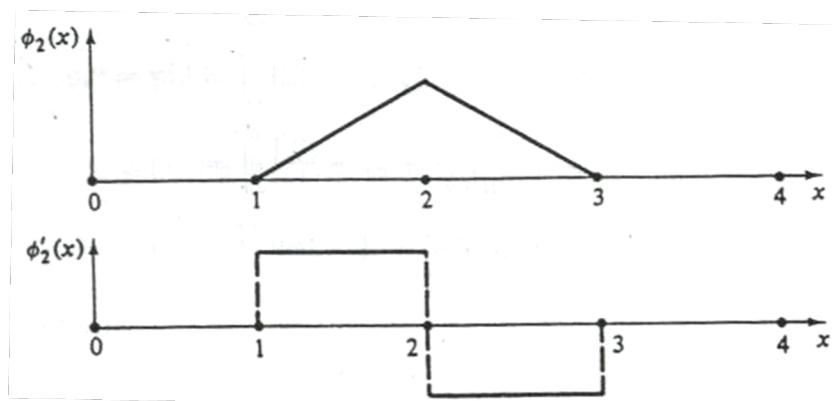
# Sparseness of K

- Regarding the stiffness matrix  $K_{ij}$ ,  $i, j = 1, 2, 3$ , we must compute 9 numbers.
- We found that  $\phi_1$  and  $\phi'_1$  are not zero only in elements  $\Omega_1$  and  $\Omega_2$ , which are adjacent to node 1. There  $K_{12}, K_{21}$  are nonzero, but  $K_{13}=K_{31}=0$ .



# Sparseness of K

- Similarly,  $\phi_2$  and  $\phi'_2$  are not zero only in elements  $\Omega_2$  and  $\Omega_3$ , which are adjacent to node 2.
- $\phi_3$  and  $\phi'_3$  are not zero only in elements  $\Omega_3$  and  $\Omega_4$ , which are adjacent to node 3.
- If we number nodes sequentially, the constructed stiffness matrix is to be **banded**.



## Symmetry of K

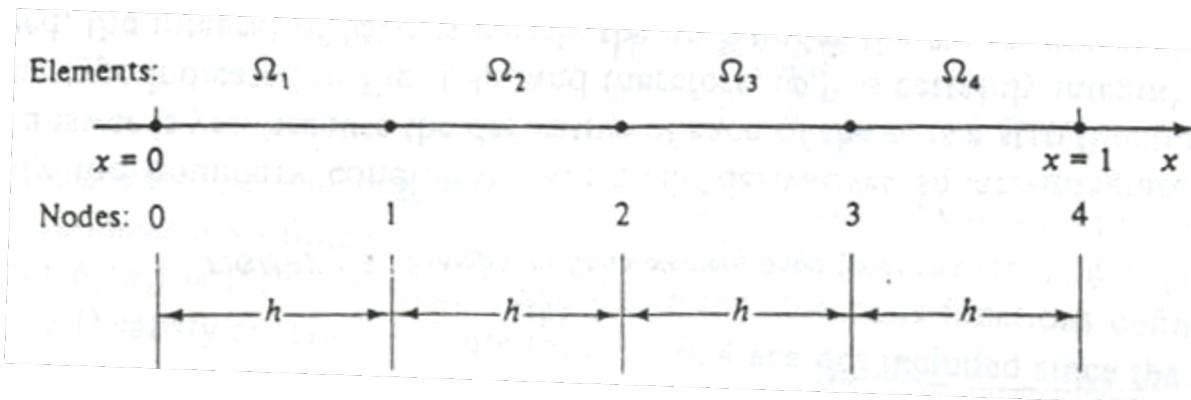
- Interchanging  $i$  and  $j$  in the integral expression for  $K_{ij}$  does not change the value calculated, so that  $K_{ij} = K_{ji}$  and the stiffness matrix is symmetric.

$$K_{ij} = \int_0^1 [\phi'_i(x)\phi'_j(x) + \phi_i(x)\phi_j(x)] dx$$

# Solving The Model Problem

- The weak form is: find  $u_N \in H_0^N$  such that

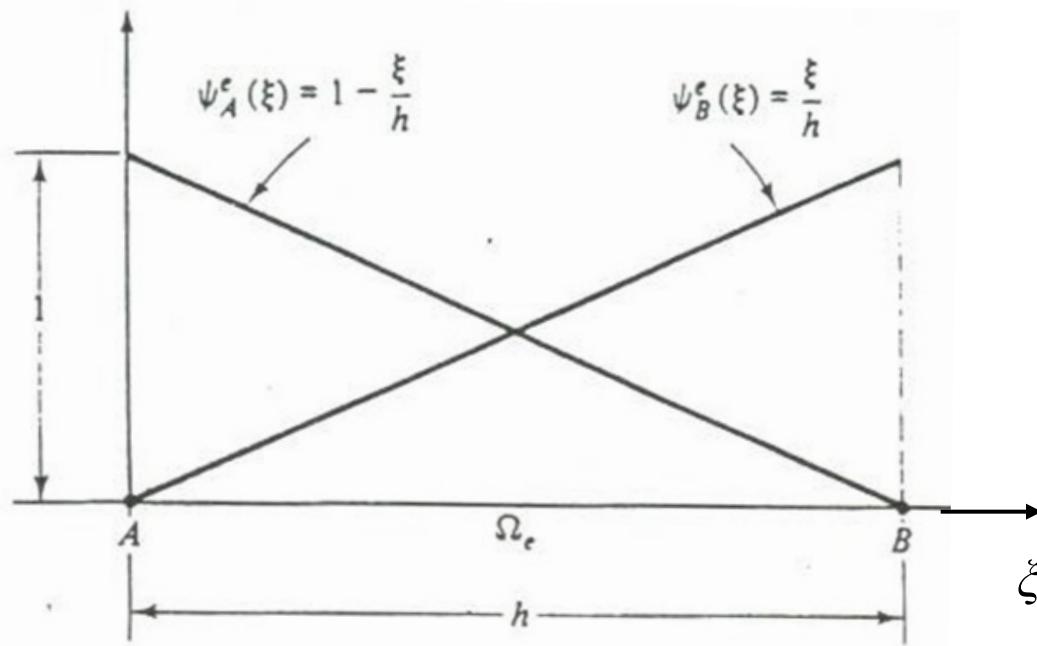
$$\int_0^1 (u'_N v'_N + u_N v_N) dx = \int_0^1 x v_N dx \quad \text{for all } v_N \in H_0^N$$



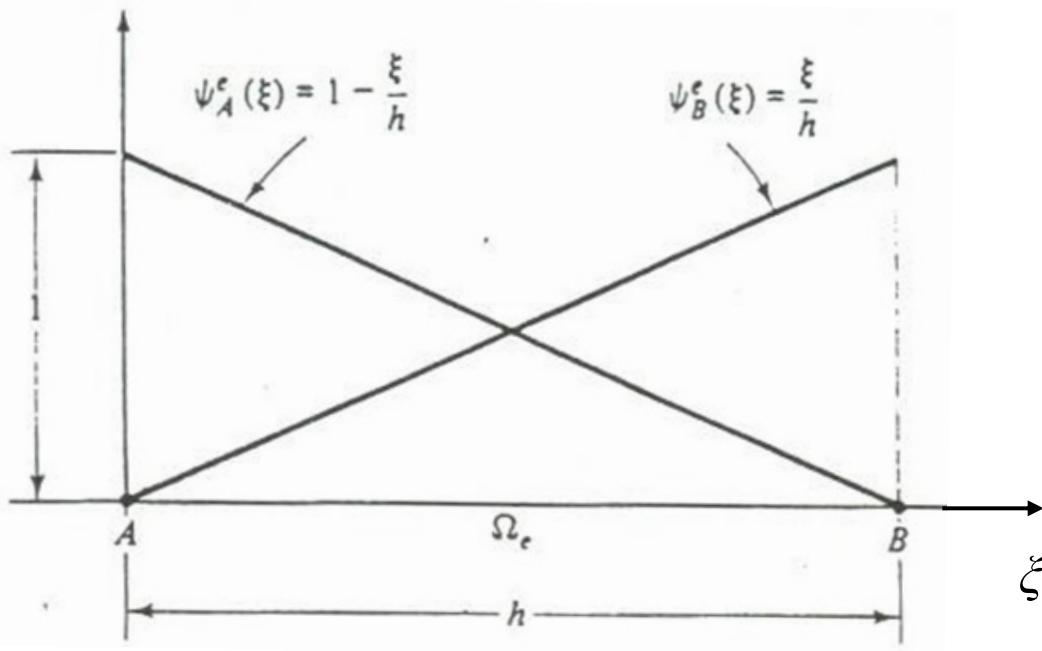
A finite element partition of the domain comprised of 4 elements with nodes at element endpoints.

# One Representative Element

- Let  $\xi$  be a local coordinate on this representative element with its origin at the left node  $A$  of  $\Omega_e$ . As  $x$  traverses  $x_A$  to  $x_B$ ,  $\xi$  goes from 0 to  $h$ , or  $\xi = x - x_A$ .  $\Psi_A^e$  and  $\Psi_B^e$  are the shape functions defined for  $\Omega_e$ .



# One Representative Element



$$\psi_A^\epsilon(\xi) = 1 - \frac{\xi}{h}, \quad \psi_B^\epsilon(\xi) = \frac{\xi}{h}$$

$$\psi_A'^\epsilon(\xi) = -\frac{1}{h}, \quad \psi_B'^\epsilon(\xi) = \frac{1}{h}$$

# One Representative Element

Stiffness matrix:  $k_{AA}^e = \int_0^h \{[\psi_A'(\xi)]^2 + [\psi_A(\xi)]^2\} d\xi$

$$= \int_0^h \left[ \frac{1}{h^2} + \left(1 - \frac{\xi}{h}\right)^2 \right] d\xi = \frac{1}{h} + \frac{h}{3}$$

$$k_{AB}^e = k_{BA}^e = \int_0^h [\psi_A'(\xi)\psi_B'(\xi) + \psi_A(\xi)\psi_B(\xi)] d\xi$$

$$= \int_0^h \left[ \left(-\frac{1}{h}\right) \frac{1}{h} + \left(1 - \frac{\xi}{h}\right) \frac{\xi}{h} \right] d\xi = -\frac{1}{h} + \frac{h}{6}$$

$$k_{BB}^e = \int_0^h \{[\psi_B'(\xi)]^2 + [\psi_B(\xi)]^2\} d\xi = \frac{1}{h} + \frac{h}{3}$$

Load vector:

$$F_A^e = \int_0^h (x_A + \xi) \left(1 - \frac{\xi}{h}\right) d\xi = \frac{h}{6}(2x_A + x_B)$$

$$F_B^e = \int_0^h (x_A + \xi) \left(\frac{\xi}{h}\right) d\xi = \frac{h}{6}(x_A + 2x_B)$$

# One Representative Element

Construct local element stiffness matrix and load vector:

$$\mathbf{k}^e = \begin{bmatrix} \frac{1}{h} + \frac{h}{3} & -\frac{1}{h} + \frac{h}{6} \\ -\frac{1}{h} + \frac{h}{6} & \frac{1}{h} + \frac{h}{3} \end{bmatrix}, \quad \mathbf{f}^e = \frac{h}{6} \begin{bmatrix} 2x_A + x_B \\ x_A + 2x_B \end{bmatrix}$$

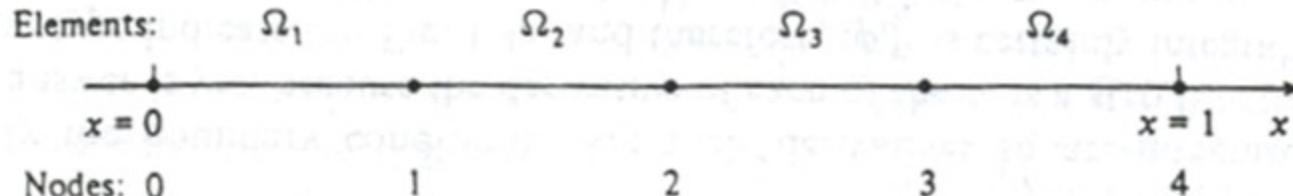
then we can calculate the element matrix for each element,  $h = 1/4$ .

*Element  $\Omega_1$ ,*

$$\mathbf{K}^1 = [K_{ij}^1] = \frac{1}{24} \begin{bmatrix} 98 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}, \quad \mathbf{F}^1 = \{F_i^1\} = \frac{1}{96} \begin{bmatrix} 2 \\ 0 \\ 0 \end{bmatrix}$$

*Element  $\Omega_2$ ,*

$$\mathbf{K}^2 = [K_{ij}^2] = \frac{1}{24} \begin{bmatrix} 98 & -95 & 0 \\ -95 & 98 & 0 \\ 0 & 0 & 0 \end{bmatrix}, \quad \mathbf{F}^2 = \{F_i^2\} = \frac{1}{96} \begin{bmatrix} 4 \\ 5 \\ 0 \end{bmatrix}$$



# Stiffness Matrix and Load Vector

Element  $\Omega_3$ ,

$$\mathbf{K}^3 = [K_{ij}^3] = \frac{1}{24} \begin{bmatrix} 0 & 0 & 0 \\ 0 & 98 & -95 \\ 0 & -95 & 98 \end{bmatrix}, \quad \mathbf{F}^3 = \{F_i^3\} = \frac{1}{96} \begin{bmatrix} 0 \\ 7 \\ 8 \end{bmatrix}$$

Element  $\Omega_4$ ,

$$\mathbf{K}^4 = [K_{ij}^4] = \frac{1}{24} \begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 98 \end{bmatrix}, \quad \mathbf{F}^4 = \{F_i^4\} = \frac{1}{96} \begin{bmatrix} 0 \\ 0 \\ 10 \end{bmatrix}$$

$$\mathbf{K} = [K_{ij}] = \mathbf{K}^1 + \mathbf{K}^2 + \mathbf{K}^3 + \mathbf{K}^4 = \frac{1}{24} \begin{bmatrix} 196 & -95 & 0 \\ -95 & 196 & -95 \\ 0 & -95 & 196 \end{bmatrix}$$

$$\mathbf{F} = \{F_i\} = \mathbf{F}^1 + \mathbf{F}^2 + \mathbf{F}^3 + \mathbf{F}^4 = \frac{1}{96} \begin{bmatrix} 6 \\ 12 \\ 18 \end{bmatrix}$$

Elements:

$\Omega_1$

$\Omega_2$

$\Omega_3$

$\Omega_4$

$$x = 0$$

$$x = 1 \quad x$$

Nodes: 0

1

2

3

4



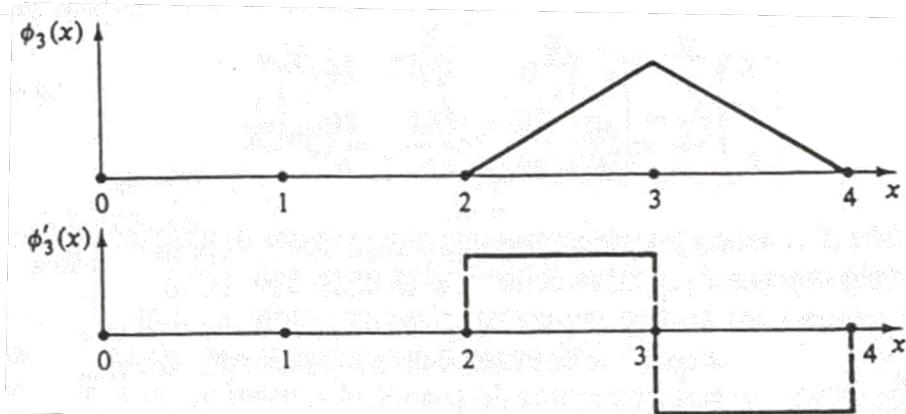
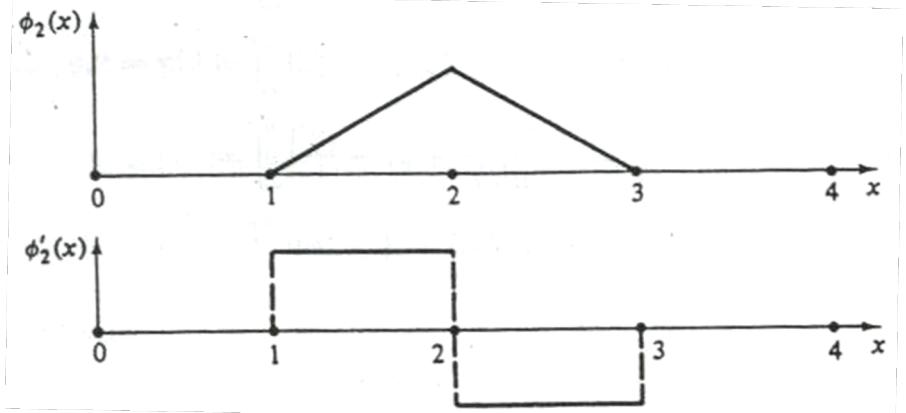
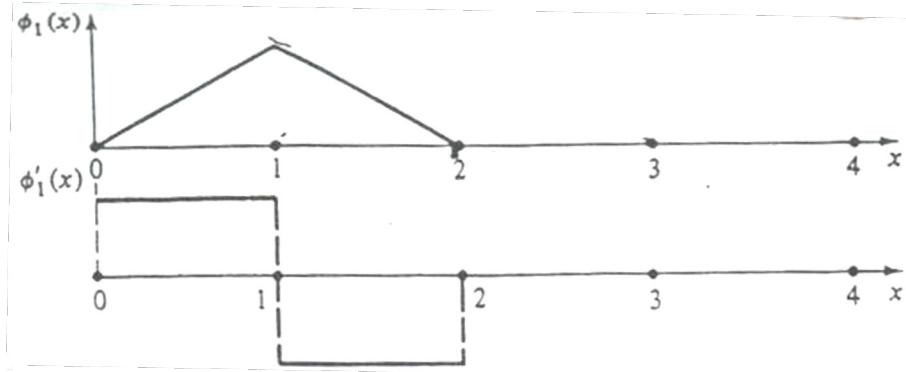
# Final Linear System

$$\frac{1}{24} \begin{bmatrix} 196 & -95 & 0 \\ -95 & 196 & -95 \\ 0 & -95 & 196 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix} = \frac{1}{16} \begin{bmatrix} 1 \\ 2 \\ 3 \end{bmatrix}$$

$$u \Rightarrow \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix} = \begin{bmatrix} 0.0353 \\ 0.0569 \\ 0.0505 \end{bmatrix}$$

$$u_h(x) = 0.0353\phi_1(x) + 0.0569\phi_2(x) + 0.0505\phi_3(x)$$

$\phi_i$  are “hat”-functions.



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## Review: The Model Problem and Weak Form

- Consider the problem of finding a function  $u = u(x)$ ,  $0 \leq x \leq 1$ , satisfying the following differential equation and boundary conditions:

$$\left. \begin{aligned} -u'' + u &= f(x), & 0 < x < 1 \\ u(0) &= 0, & u(1) &= 0 \end{aligned} \right\}$$

where  $u'' = d^2u / dx^2$ , and  $f(x) = x$ .

- The weak statement becomes: find  $u \in H_0^1$  such that

$$\int_0^1 (u'v' + uv - xv) dx = 0 \quad \text{for all } v \in H_0^1$$

# Review: Galerkin Approximations

- Plug the approximate solution  $u_N$  and the test function  $v_N$  into the weak statement (finite-dimensional):

$$\int_0^1 (u'_N v'_N + u_N v_N) dx = \int_0^1 x v_N dx$$

$$u_N(x) = \sum_{j=1}^N \alpha_j \phi_j(x) \quad \rightarrow \quad \downarrow \quad \leftarrow \quad v_N(x) = \sum_{i=1}^N \beta_i \phi_i(x)$$

$$\begin{aligned} & \int_0^1 \left\{ \frac{d}{dx} \left[ \sum_{i=1}^N \beta_i \phi_i(x) \right] \frac{d}{dx} \left[ \sum_{j=1}^N \alpha_j \phi_j(x) \right] + \left[ \sum_{i=1}^N \beta_i \phi_i(x) \right] \left[ \sum_{j=1}^N \alpha_j \phi_j(x) \right] \right. \\ & \quad \left. - x \sum_{i=1}^N \beta_i \phi_i(x) \right\} dx = 0 \quad \text{for all } \beta_i, i = 1, 2, \dots, N \end{aligned}$$

$$\sum_{j=1}^N K_{ij} \alpha_j = F_i, \quad i = 1, 2, \dots, N \quad \rightarrow \quad \alpha_j = \sum_{l=1}^N (K^{-1})_{jl} F_l$$

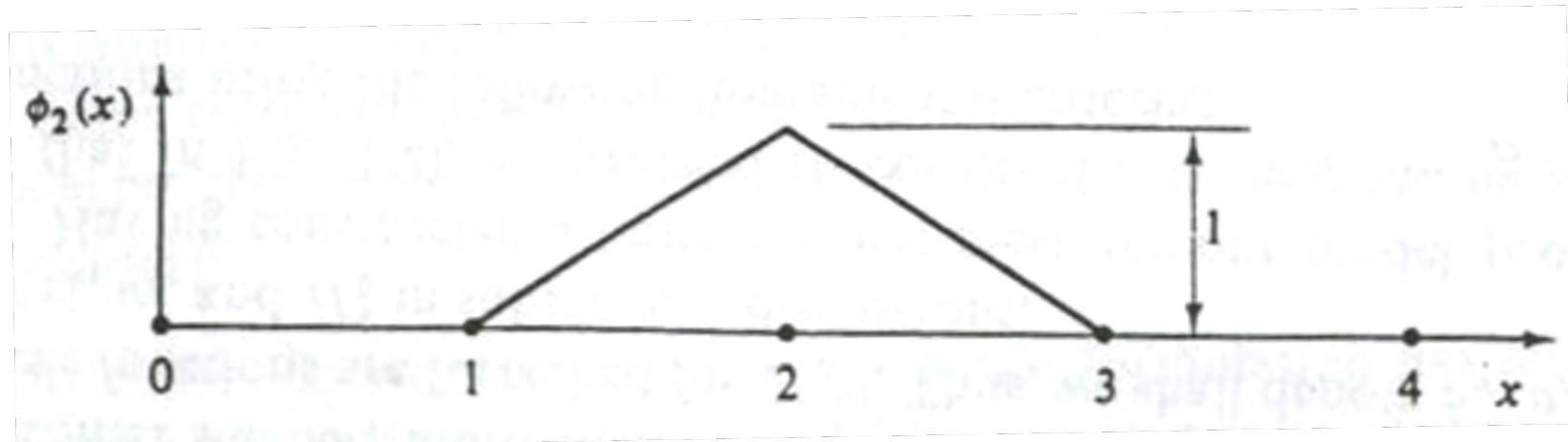
$$\text{where } K_{ij} = \int_0^1 [\phi'_i(x) \phi'_j(x) + \phi_i(x) \phi_j(x)] dx$$

$$F_i = \int_0^1 x \phi_i dx$$

$$i, j = 1, 2, \dots, N$$

# Review: Three Criteria for Basis Functions

- The basis functions are generated by simple functions defined piecewise – element by element – over the finite element mesh.
- The basis functions are smooth enough to be members of the class  $H_0^1$  of test functions.
- The basis functions are chosen in such a way that the parameters  $\alpha_i$  defines the approximation solution precisely at the nodal points.



# Review: Finite Element Calculations

- The weak form leads to the system of linear equations

$$\sum_{j=1}^N K_{ij} \alpha_j = F_i, \quad i = 1, 2, \dots, N$$

- Let the term

$$K_{ij}^e = \int_{\Omega_e} (\phi'_i \phi'_j + \phi_i \phi_j) dx$$

represent components of the element stiffness matrix for finite element  $\Omega_e$ .

$$K_{ij} = \sum_{e=1}^4 K_{ij}^e$$

Similarly

$$F_i = \sum_{e=1}^4 F_i^e, \quad F_i^e = \int_{\Omega_e} x \phi_i dx$$

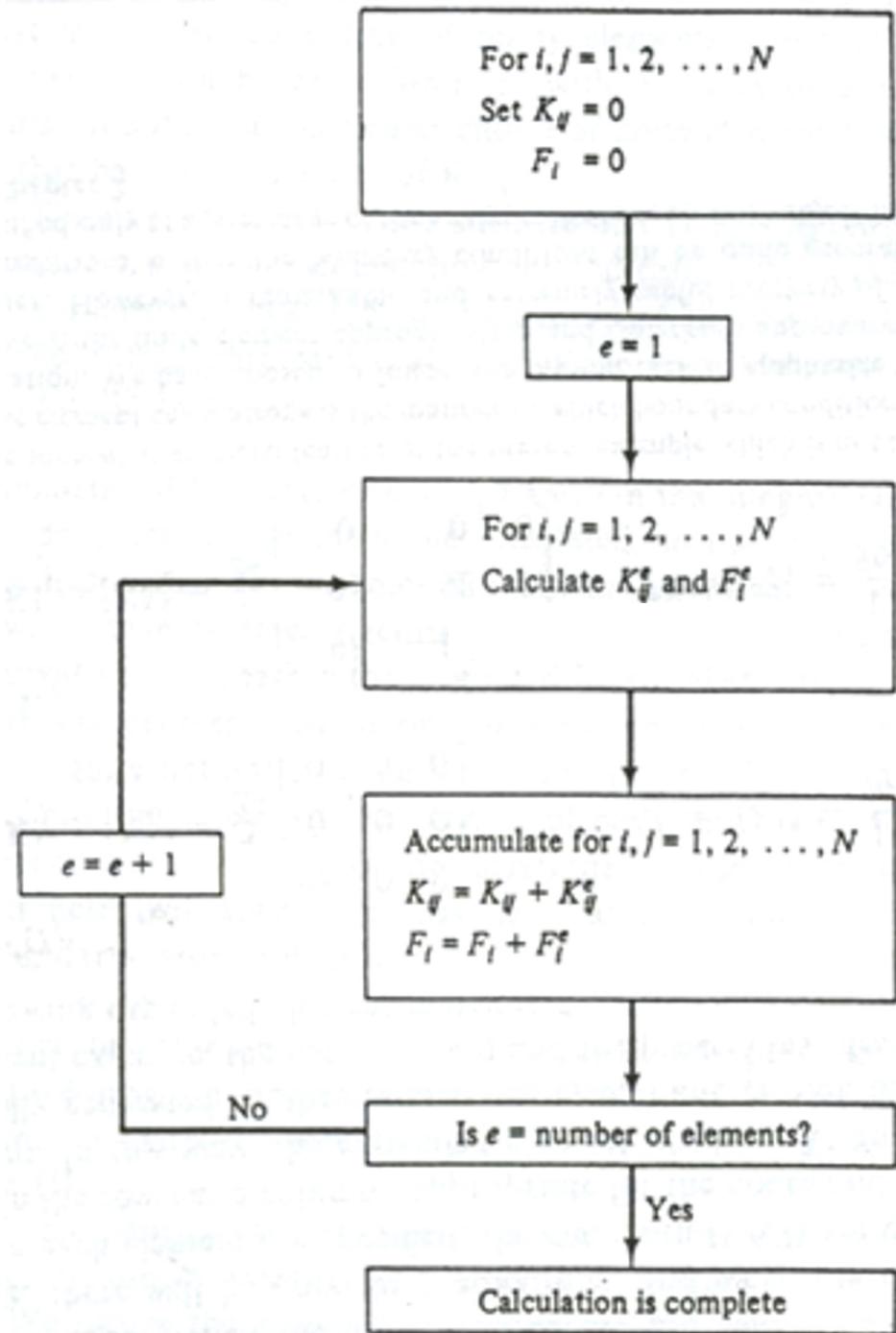


# Flow Chart

Flow chart shows element by element calculation and assembly of  $K$  and  $F$ .

$$K_{ij}^e = \int_{\Omega_e} (\phi_i' \phi_j' + \phi_i \phi_j) dx$$

$$F_i^e = \int_{\Omega_e} x \phi_i dx$$



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# The Model Problem

- The weak form is: find  $u_h \in H_0^h$  such that

$$\int_0^1 (u'_h v'_h + u_h v_h) dx = \int_0^1 x v_h dx \quad \text{for all } v_h \in H_0^h$$

- We have obtained the solution of the model problem. We can check graphs of the solution of its derivative.

$$\mathbf{u} \Rightarrow \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix} = \begin{bmatrix} 0.0353 \\ 0.0569 \\ 0.0505 \end{bmatrix}$$

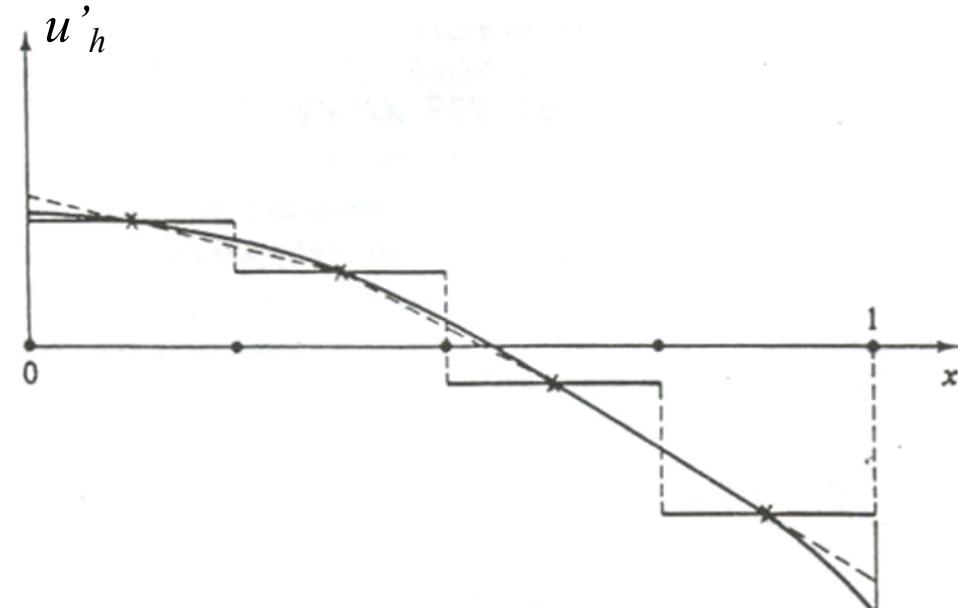
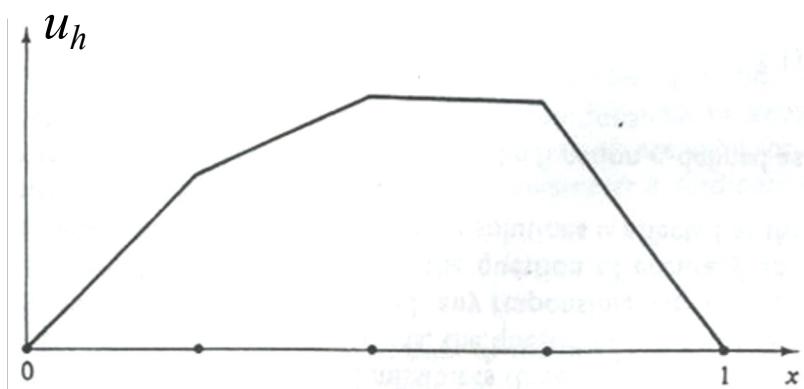
$$u_h(x) = 0.0353\phi_1(x) + 0.0569\phi_2(x) + 0.0505\phi_3(x)$$

$\phi_i$  are “hat”-functions.

# The Approximate Solution

$$u_h(x) = 0.0353\phi_1(x) + 0.0569\phi_2(x) + 0.0505\phi_3(x)$$

- The approximate solution is a fairly smooth function – there are no apparent oscillations or regions of very high gradients. (finer mesh gives better results)
- There is one extremum of the solution, a maximum value of 0.0569 at  $x = 0.5$ .
- The derivative of the solution is large near the endpoints, the largest absolute value occurring near  $x = 1.0$ .



## Interpretation of The Approximate Solution

- Suppose the model problem has arisen in the analysis of a stretched string, supported on an elastic foundation and subjected to a transverse load whose distribution is given by  $f(x) = x$ .
- The solution  $u$  is the transverse deflection of the string and its derivative  $u'$  is proportional to the stress in the string.
- The total strain energy in the system (i.e., in the string and the elastic support) is given by

$$U = \frac{1}{2} \int_0^1 [(u')^2 + u^2] dx$$

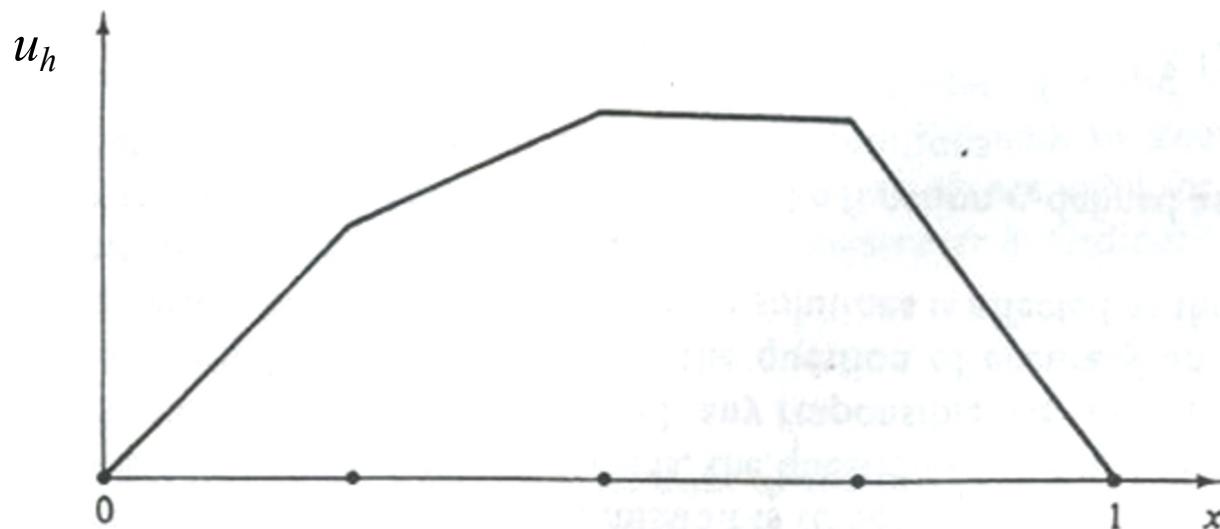
# Questions?

- What is the location and the value of the maximum deflection? ( $u$ )
- What is the location and the value of the maximum stress? ( $u'$ )
- What is the value of total strain energy in the system? (  $U = \frac{1}{2} \int_0^1 [(u')^2 + u^2] dx$  )

# Question 1

- *What is the location and the value of the maximum deflection?*

The maximal  $u_h$  is at node 2,  $x = 0.5$ , and  $u_2 = 0.0569$ .

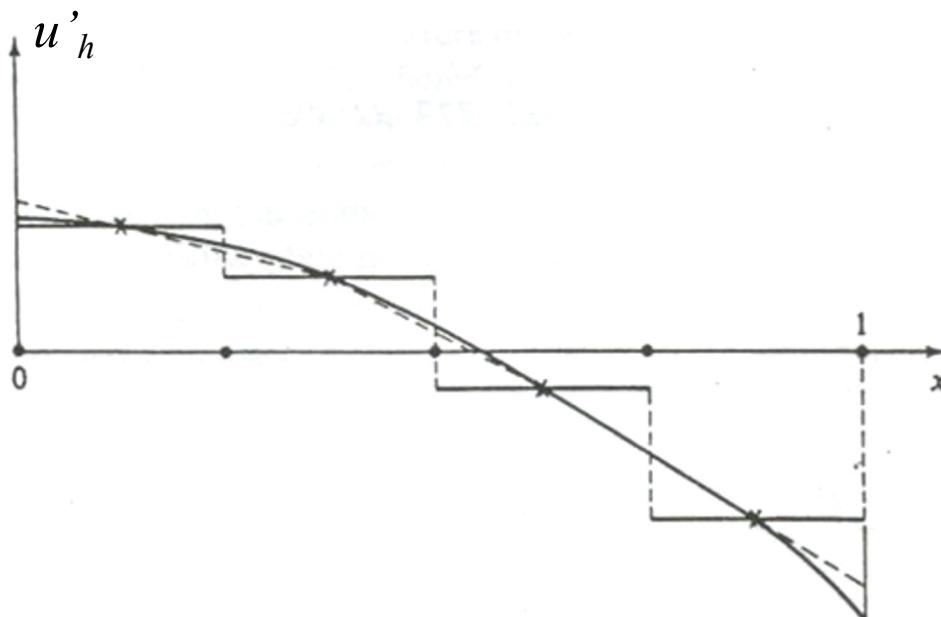


## Question 2

- *What is the location and the value of the maximum stress?*

The maximum value of  $|u'_h|$  is 0.202 and this value occurs throughout the element  $\Omega_4$ . We assign it to the midpoint of the element ( $x = 0.875$ ).

We expect the maximum stress to occur at the point  $x = 1$ , so we could extrapolate the values of stress to the boundary point at  $x = 1$  (a smooth curve or piecewise straight lines)



## Question 3

- What is the value of total strain energy in the system?

$$U = \frac{1}{2} \int_0^1 [(u')^2 + u^2] dx$$

Let  $\phi$  denote the vector of basis functions and  $u$  the vector of nodal values of the solution,

$$\phi = \begin{bmatrix} \phi_1 \\ \phi_2 \\ \phi_3 \end{bmatrix}, \quad u = \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix}$$

$$u_h(x) = u^T \phi(x)$$

$$\begin{aligned} U_h &= \frac{1}{2} \int_0^1 [(u'_h)^2 + u_h^2] dx \\ &= \frac{1}{2} u^T \int_0^1 (\phi'^T \phi' + \phi^T \phi) dx u \\ &= \frac{1}{2} u^T K u \end{aligned}$$

$$Ku = F$$



$$\begin{aligned} U_h &= \frac{1}{2} u^T F \\ &= 0.0094 \end{aligned}$$



# Interpretation of The Approximation Solution

- Calculations and observations indicate that a great deal of useful information can be extracted from a careful examination of properties of the solution.

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# Approximation Accuracy

- How is the error in the approximate solutions effected as the number of elements in the mesh is increased?
- The error  $e$  is defined as the difference between the exact and the approximate solutions:

$$e(x) = u(x) - u_h(x).$$

energy norm:

$$\|e\|_E = \left\{ \int_0^1 [(e')^2 + e^2] dx \right\}^{1/2}$$

mean-square ( $L^2$ ) norm:

$$\|e\|_0 = \left( \int_0^1 e^2 dx \right)^{1/2}$$

maximum norm:

$$\|e\|_\infty = \max_{0 \leq x \leq 1} |e(x)|$$

# Asymptotic Estimates

- The error can be estimated although the exact solution is unknown.
- Suppose that the domain of the problem is discretized by a finite element mesh consisting of elements of equal length  $h$ . The error estimates will be of the form

$$\|e\| \leq Ch^P$$

where  $C$  is a constant depending upon the problem and generally  $p$  is a positive integer that depends on the chosen basis functions. When  $h$  tends to zero, then  $\|e\|$  tends to zero.

# **a-priori** Estimates and **a-posteriori** Estimates

$$\|e\| \leq Ch^P$$

- The above estimates require no information from the actual finite element solution, they are known prior to the construction of the solution, and are called **a-priori** estimates.
- More detailed estimates of accuracy can be based on information obtained from the finite element solution. Such estimates are called **a-posteriori** estimates.

## a-priori Error Estimates

- In the case of our model problem, the following a-priori error estimates can be shown to hold for piecewise-linear basis functions:

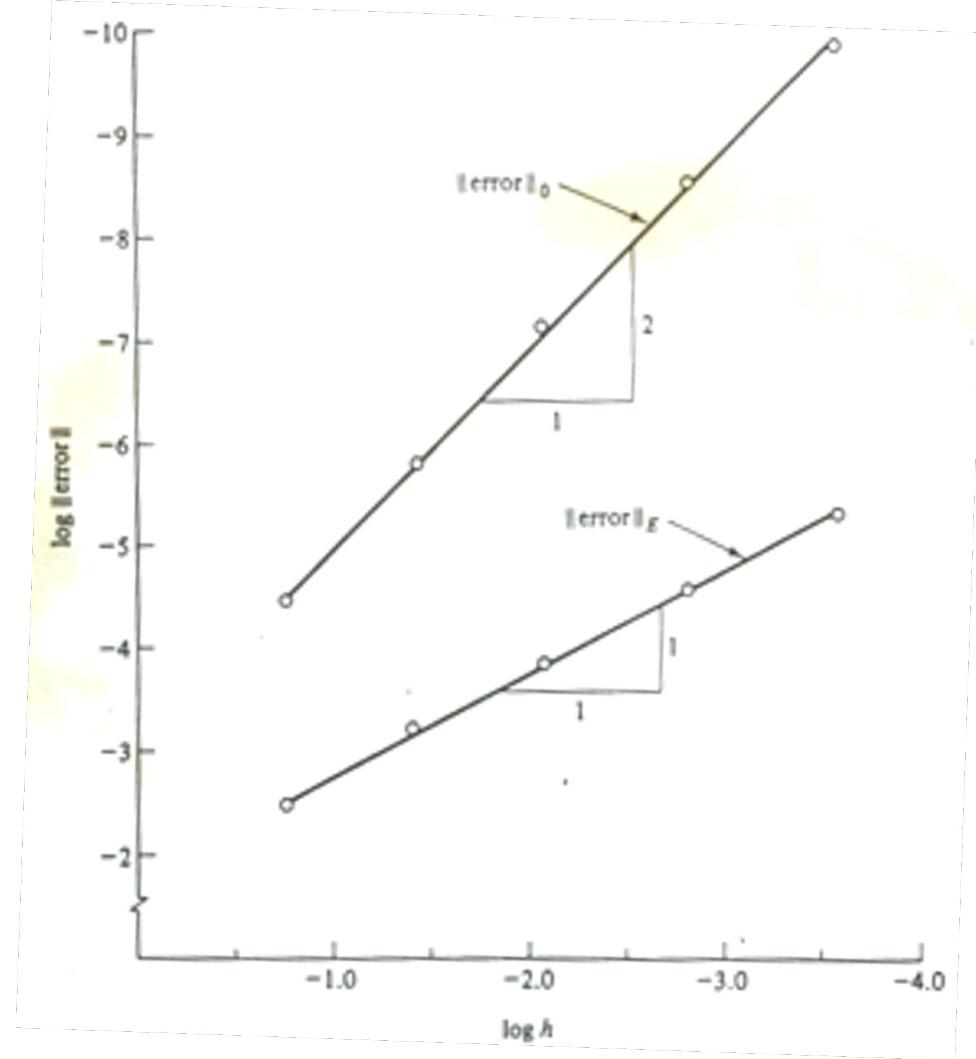
$$\left. \begin{aligned} \|e\|_E &\leq C_1 h \\ \|e\|_0 &\leq C_2 h^2 \\ \|e\|_\infty &\leq C_3 h^2 \end{aligned} \right\}$$

# Log-log Plots of The Errors

- Considering the error estimates:

$$\|e\| \leq E(h) = Ch^P$$

$$\log E(h) = p \log h + \log C$$



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# Summary

- The same algorithm can be extended to 2D/3D.
- The finite element method has been used in a lot of research fields:
  - Mechanical Engineering
  - Civil Engineering and Architecture
  - Materials Science
  - Geological Engineering
  - Petroleum Engineering
  - Electrical Engineering and Computer Sciences
  - Biomedical Engineering
  - Biology, Chemistry, Physics, etc.

# New Techniques of FEM

- XFEM: developed by Ted Belytschko in Northwestern University  
<http://dilbert.engr.ucdavis.edu/~suku/xfem/>
- Immersed FEM: developed by Wing Kam Liu in Northwestern University.
- Isogeometric Analysis: developed by Thomas J.R. Hughes in University of Texas at Austin.  
(Integration of CAD and FEM)  
<http://users.ices.utexas.edu/~hughes/>

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- [http://en.wikipedia.org/wiki/Finite\\_element\\_method](http://en.wikipedia.org/wiki/Finite_element_method)
- XFEM: developed by Ted Belytschko in Northwestern University <http://dilbert.engr.ucdavis.edu/~suku/xfem/>
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