

## ETABS Shear Wall Design

### ACI 318-14 Pier Design

#### Pier Details

Story ID	Pier ID	Centroid X (m)	Centroid Y (m)	Length (m)	Thickness (m)	LLRF
Cielo P1	PFel-A20-2	27,77885	29,44554	1,05	0,26	1

#### Material Properties

$E_c$ (tonf/m <sup>2</sup> )	$f'_c$ (tonf/m <sup>2</sup> )	Lt.Wt Factor (Unitless)	$f_y$ (tonf/m <sup>2</sup> )	$f_{ys}$ (tonf/m <sup>2</sup> )
2194996,45	2109,21	1	42184,18	42184,18

#### Design Code Parameters

$\phi_T$	$\phi_C$	$\phi_v$	$\phi_v$ (Seismic)	$IP_{MAX}$	$IP_{MIN}$	$P_{MAX}$
0,9	0,65	0,75	0,6	0,04	0,0025	0,8

#### Pier Leg Location, Length and Thickness

Station Location	ID	Left X <sub>1</sub> m	Left Y <sub>1</sub> m	Right X <sub>2</sub> m	Right Y <sub>2</sub> m	Length m	Thickness m
Top	Leg 1	27,67868	28,93019	27,87902	29,9609	1,05	0,26
Bottom	Leg 1	27,67868	28,93019	27,87902	29,9609	1,05	0,26

#### Flexural Design for P, M<sub>3</sub> and M<sub>2</sub>

Station	D/C	Flexural	$P_u$ tonf	$M_{u2}$ tonf-m	$M_{u3}$ tonf-m
Top	1,242	1.4Y+1.2D+1.0L	-23,5068	0,393	-15,0001
Bottom	0,846	-1.4Y+1.2D+1.0L	2,0538	-0,5182	-18,9486

**Design Inadequacy Message: Pier fails in flexure or P-M-M interaction !!**

#### Shear Design

Station Location	ID	Rebar m <sup>2</sup> /m	Shear Combo	$P_u$ tonf	$M_u$ tonf-m	$V_u$ tonf	$\phi V_c$ tonf	$\phi V_n$ tonf
Top	Leg 1	0,00065	-1.4Y+1.2D+1.0L	43,1674	23,0646	22,0606	12,6155	29,8899
Bottom	Leg 1	0,00065	-1.4Y+1.2D+1.0L	2,0538	-18,9486	21,9695	12,6155	29,8899

#### Boundary Element Check (ACI 18.10.6.3, 18.10.6.4)

Station Location	ID	Edge Length (m)	Governing Combo	$P_u$ tonf	$M_u$ tonf-m	Stress Comp tonf/m <sup>2</sup>	Stress Limit tonf/m <sup>2</sup>	C Depth m	C Limit m
Top-Left	Leg 1	Not Required	-1.4Y+1.2D+1.0L	43,1674	23,0646	-324,65	421,84	0,17497	0,23333
Top-Right	Leg 1	Not Calculated	-1.4Y+1.2D+1.0L	43,1674	23,0646	640,9	421,84	0,17497	0,23333
Bottom-Left	Leg 1	0,04266	-1.4Y+1.2D+1.0L	2,0538	-18,9486	404,14	421,84	0,08531	0,23333
Bottom-Right	Leg 1	0,05483	1.4Y+1.2D+1.0L	15,1757	15,9736	389,94	421,84	0,10967	0,23333