# Lab 8 - Guidelines

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## 0.1 Lab 8: Guidelines for Open Channel Flow Experiments

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#### 0.2 Part I: Flow across a weir

In the first part of the experiment, you will compare the flow rate measured across the weir with the flow rate obtained from the apparatus. Note that **Manning's equation is not required for this part**.

• How do you read the flow rate from the apparatus?

The flow rate from the manometer is calculated using the formula:

$$\log_{10}(Q) = \frac{\log_{10}(\Delta H) - 1.47}{2.096} \implies \boxed{Q_m = 10^{\frac{\log_{10}(\Delta H) - 1.47}{2.096}}, \quad (cfs)}$$

Where  $\Delta H\left(ft\right)$  is the difference between the readings of the two manometers:  $\Delta H=H_{1}-H_{2}$ 

• Flow across the V-notch weir:

The flow rate across the V-notch weir is determined using the following equation:

$$Q_w = \frac{8}{15} \sqrt{2g} C_e \tan\left(\frac{\theta}{2}\right) (h+k)^{5/2}$$

Here,  $C_e$  is a coefficient based on the angle  $\theta$  of the weir in degrees. k is the head correction coefficient. The values for  $C_e$  and k are functions of the angle  $\theta$  of the V-notch weir:

$$C_e = 0.6072 - 0.0008745 \cdot \theta + 0.000006104 \cdot \theta^2$$

$$k = \frac{0.002}{\sin\left(\frac{\theta}{2}\right)}, \quad \text{(for } 40^{\circ} \le \theta \le 90^{\circ}), \quad (ft)$$

Since the angle of our V-notch weir is 90°, we say:

$$Q_w = \frac{8}{15} \sqrt{2g} \, C_e \, (h+k)^{5/2}, \quad C_e = 0.578, \quad \text{and } k = 0.862 \, mm \, (0.00283 \, ft)$$

Note: You will have to plot two curves on the same figure:

- (1)  $\log(Q_m)(h)$ : manometer flow rates vs weir water heights
- (2)  $\log(Q_w)(h)$ : weir flow rates vs weir water heights.

## 0.3 Part 2: Channel roughness

In the second part of the experiment, you will estimate the channel roughness coefficient n. To do so,

• Calculate flow rates using the equation from Part I based on the manometer readings:

 $\log_{10}(Q) = \frac{\log_{10}(\Delta H) - 1.47}{2.096}$  - Then, compute the wetted perimeter P, the area A, and the hydraulic radius  $R = \frac{A}{P}$  from the measured h and the channel width B.

Knowing the flow rate Q, the bed slope  $S_0$ , and other calculated dimensions above, **use Manning's** equation to solve for n.

$$Q = \frac{K_n}{n} \cdot A \cdot R^{2/3} \cdot S_0 \implies \boxed{n = \frac{K_n}{Q} \cdot A \cdot R^{2/3} \cdot S_0}$$

Here,  $K_n$  is a unit conversion quantity. Take  $K_n=1.0$  if using S.I units or  $K_n=1.49$  for US customary units.

#### Remark:

To verify your results, rewrite Manning's equation as a linear relationship and then plot it. To do this, we pose Y=Q and  $x=K_n\cdot A\cdot R^{2/3}\cdot S_0$ , then Manning's equation can be written as a linear equation of the form:

$$Y = \frac{1}{n}x$$

- Use the data you gathered in part II, regardless of the slopes, and plot Q versus  $x = K_n \cdot A \cdot R^{2/3} \cdot S_0$ .
- Compare the slope of the plotted curve with the  $\frac{1}{n}$  to critique and validate your estimated mean roughness coefficient.

#### 0.3.1 References

- (1) Discharge Characteristics of Triangular-notch Thin-plate Weirs
- (2) Open Channel Flow: Instructor Notebook Dr. Cleveland

## 0.3.2 Practical Example

[11]: #------#

# SCRIPTS

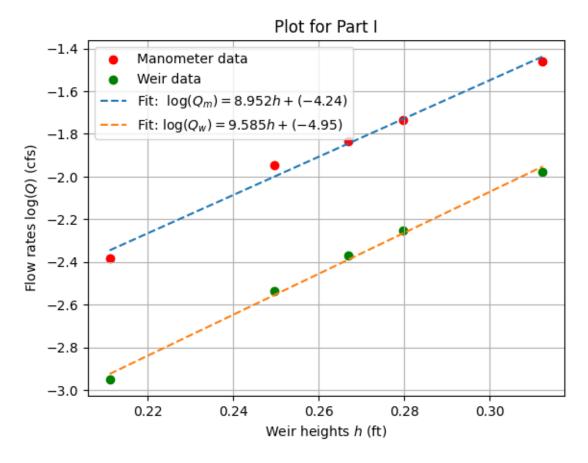
#-----
import numpy as np

import math

```
g = 32.2 # ft/s2
k = 0.00283 \# ft
Ce = 0.578 # discharge coefficient
h = [64.32, 76.10, 81.38, 85.28, 95.25] # mm (weir heights)
h = [i*0.00328084 \text{ for } i \text{ in } h]
                                           # ft
#print(h)
# Manometer reading
H_1 = [2.3, 2.43, 2.47, 2.53, 2.75] # ft
H_2 = [2.1, 1.93, 1.84, 1.75, 1.37]
# Denivelation from the manometer
delta_h = [i-j \text{ for } i,j \text{ in } zip(H_1, H_2)] \#H_1-H_2
# Compute flow rates from the manometers
Qm = 10**((np.log10(delta_h) -1.47) / 2.096)
# Flow across the weir
Qw = [8/15 * math.sqrt(2*g) * Ce * (i+k)**2.5 for i in h]
# print Qm
print("- The flow across the manometers Qm=", np.round(Qm, 4), "cfs")
print("- The flow across the weir Qw=", np.round(Qw, 4), "cfs")
```

- The flow across the manometers  $Qm = [0.0923 \ 0.1429 \ 0.1596 \ 0.1767 \ 0.232]$  cfs
- The flow across the weir  $Qw = [0.0523 \ 0.0793 \ 0.0936 \ 0.105 \ 0.1381]$  cfs

```
plt.title("Plot for Part I")
plt.xlabel("Weir heights $h$ (ft)")
plt.ylabel("Flow rates $\\log(Q)$ (cfs)")
plt.legend()
plt.grid(True)
plt.show()
```

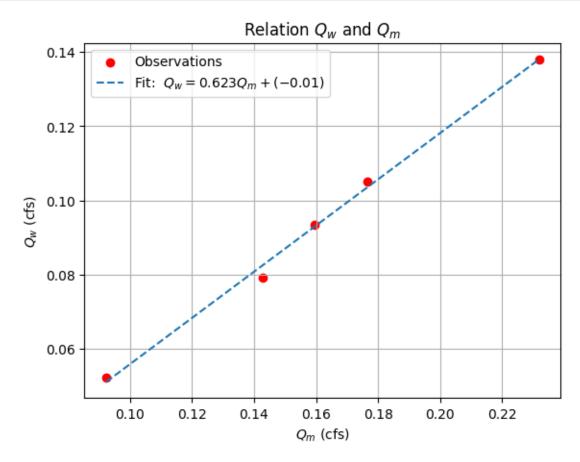


```
[8]: # Let us fit a linear regression that predicts Qw based on Qm
    x = Qm
    y = Qw

# Regression of degree 1
    m, b = np.polyfit(x, y, 1) # manometers

# plots
    plt.scatter(Qm, Qw, label="Observations", color="red")
    plt.plot(x, [m*i+b for i in x], "--", label=f'Fit: $Q_w={m:.3f}Q_m+({b:.2f})$')
    plt.title("Relation $Q_w$ and $Q_m$")
    plt.xlabel("$Q_m$ (cfs)")
```

```
plt.ylabel("$Q_w$ (cfs)")
plt.legend()
plt.grid(True)
plt.show()
```



### Note:

As established from the linear regression, the relationship between the flow rate over the weir  $Q_w$  and the upstream flow rate from the manometer readings  $(Q_m)$  is given by:

$$Q_w = 0.623Q_m - 0.01,$$

This relationship allows us to estimate the flow reaching the rocky-bed region under experimental conditions where the weir is present. The estimated  $Q_w$  can then be used to calculate Manning's coefficient n for the rocky-bed region.

In the absence of the weir, the total upstream flow  $Q_m$  would directly enter the rocky-bed region, as there would be no division or obstruction caused by the weir. In this case, Manning's coefficient will be estimated using this direct flow value.

[]: