



THE UNIVERSITY OF BRITISH COLUMBIA

## Bidirectional monotonic and cyclic shear testing of soils: State of Knowledge

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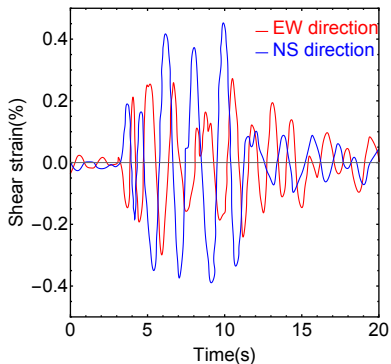
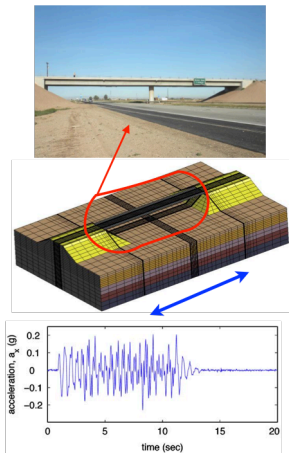


Vancouver, BC, Canada. October 3, 2016

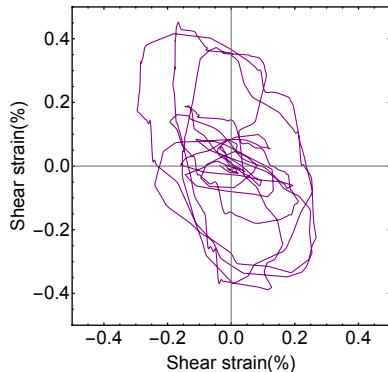


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# Motivation



(a) Strain history



(b) Strain orbit

Nanbu earthquake, 1995 (Matsuda et al., 2012)

Modified from Bebamzadeh et al. (2014);  
Rahmani et al. (2014)

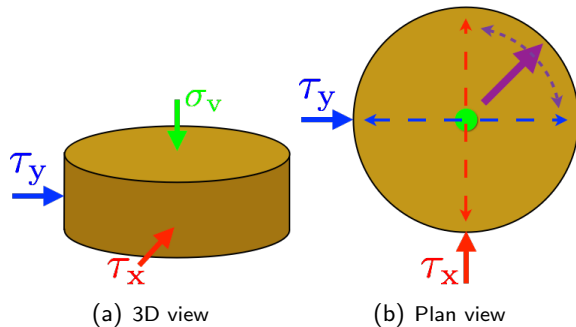
# Outline

- 1 Bidirectional shear test
- 2 Initial stress state
- 3 Undrained shearing
  - Monotonic shear test
  - Cyclic shear test
- 4 Summary and ongoing research

# Laboratory bidirectional shear test

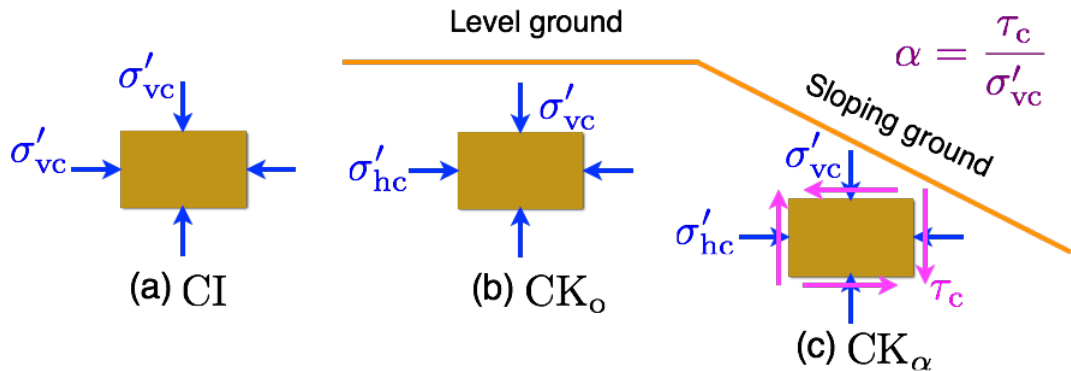


**Fig. 1.1** Bidirectional shear device (Kammerer, 2002)



**Fig. 1.2** Modified from GDS instruments

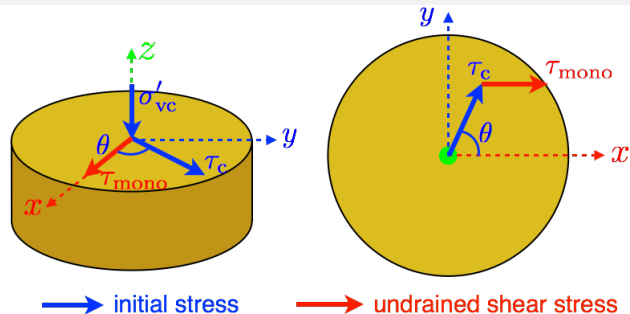
## Initial stress state



**Fig. 2.1** Initial stress conditions: (a) isotropic condition; (b)  $K_o$  condition; (c)  $K_\alpha$  condition. (modified from Boulanger et al. (1991))

# Monotonic shear test

- Initial stress state:  $CK_{\alpha}$
- Angle between  $\tau_c$  and  $\tau_{mono}$ :  $\theta$



**Tab. 1** Bidirectional monotonic shear tests on soils

Data source	Material	$\sigma'_{vc}$ [kPa]	$\tau_c / \sigma'_{vc}$	$\theta$ [°]	Shear rate [%/hr]
DeGroot (1989)	Boston blue clay	294.2	0.2	0, 30, 60, 90, 120, 150, 180	5
Li et al. (2016)	Leighton Buzzard sand	200	0.05, 0.1	0, 30, 60, 90, 120, 150, 180	3.53

# Cyclic shear test - loading paths

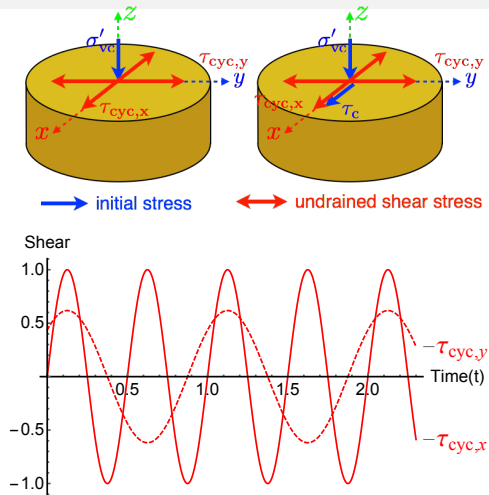


Fig. 3.1 Illustration

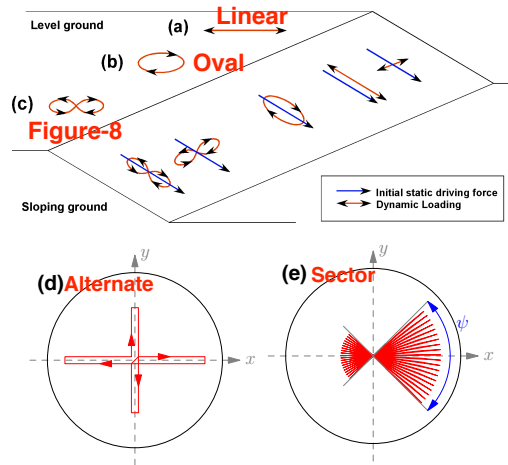


Fig. 3.2 Loading paths (modified from Kammerer (2002))

# Cyclic shear test - database

**Tab. 2** Bidirectional cyclic shear tests on soils: [database](#)

Data source	Material	Initial State	$\sigma'_{vc}$ [kPa]	$\alpha$	Test Type	f [Hz]	CSR <sub>x</sub>	CSR <sub>y</sub>
Ishihara and Yamazaki (1980)	Fuji River sand	CI	200	0	Oval	0.25	0.092	0.081
			200	0	Alternate	0.25	0.144	0.143
Ishihara and Nagase (1988)	Fuji River sand	CI	196	0	Irregular	-	-	-
Boulanger et al. (1991)	Sacramento River sand	CK <sub><math>\alpha</math></sub>	206	0.1	Linear	0.2	0	0.1,0.15,0.118 0.141,0.16,0.18 0.082,0.1,0.109 0.124,0.125
			206	0.2	Linear	0.2	0	0.135,0.139,0.15 0.15,0.157,0.187 0.187 0.05,0.08,0.082 0.082,0.1,0.1 0.11,0.115,0.131
		CK <sub><math>\alpha</math></sub>	206	0.3	Linear	0.2	0	0.131,0.143,0.15 0.15,0.15,0.163 0.17,0.21,0.211 0.218
			87	0	Oval	0.1	0.24	0.239
			79	0	Oval	0.1	0.274	0.26
			83	0	Oval	0.1	0.133	0.124
Kammerer (2002)	Monterey 0/30 sand	CK <sub><math>\alpha</math></sub>	87	0.02	Oval	0.1	0.134	0.227
			83	0.02	Oval	0.1	0.365	0.185



# Summary and ongoing research

## Summary

- Experimental data about bidirectional shear test is available within two categories
  - *Monotonic shearing on specimens with  $CK_\alpha$*
  - *Cyclic shearing on specimens with  $CI$ ,  $CK_o$  and  $CK_\alpha$*
- A comprehensive database is collected in a well organized way

## Ongoing research

- Simulate element tests with complex loading paths to evaluate numerical models
- Model boundary value problems

Thank you!

## Figure-8 path

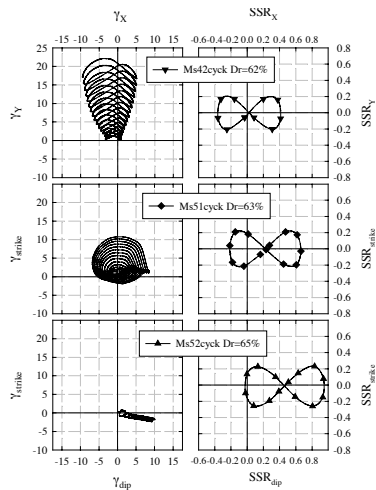


Fig. 4.1 Figure-8 from Kammerer (2002)

Bidirectional shear test of soils

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