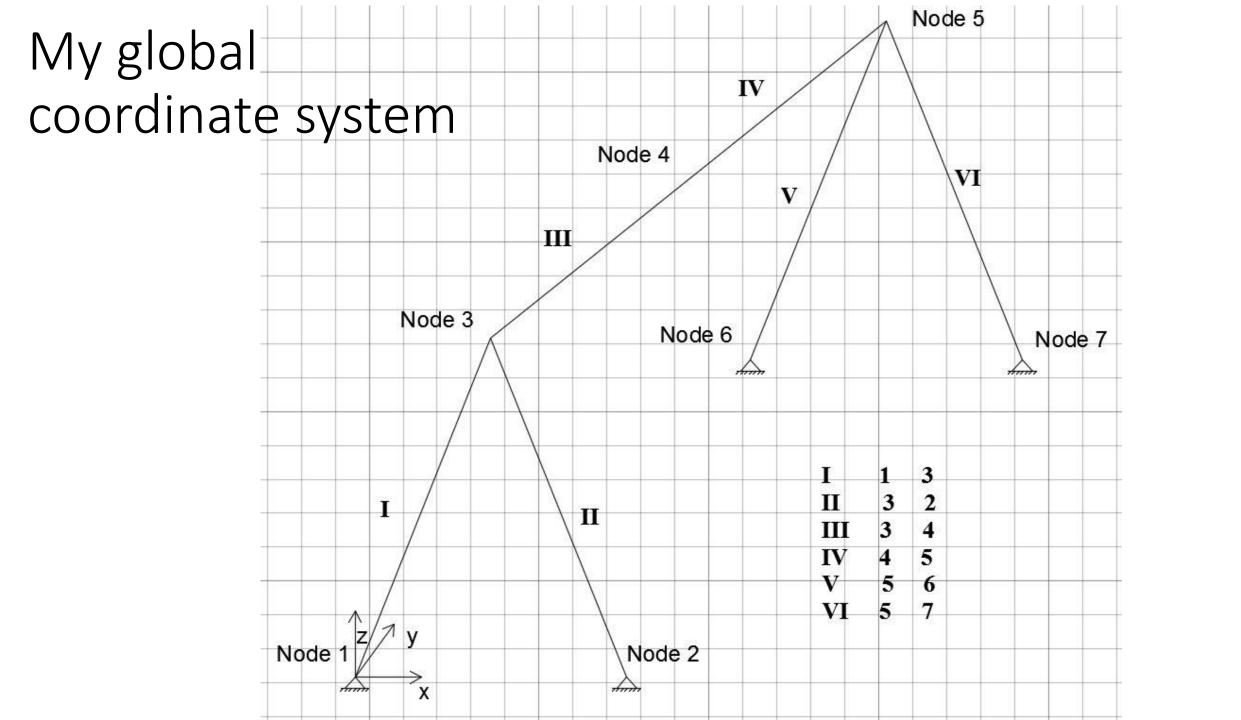
## Final Project

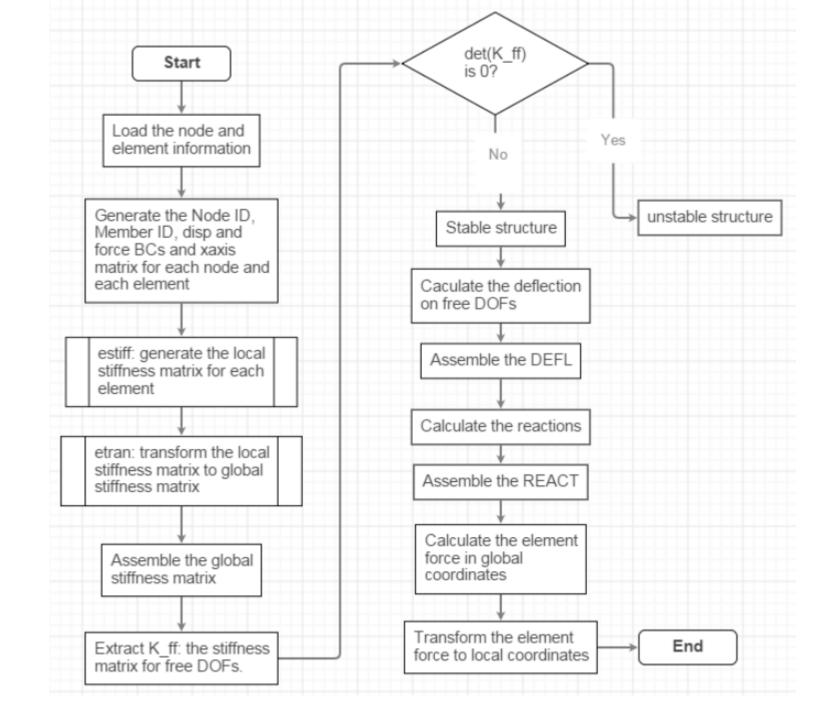
ECI211 Fall 2015

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## My flowchart



#### Load the node and element information

- % load the input information.
- load nnodes.txt;
- load coord.txt;
- load concen.txt;
- load fixity.txt;
- load nele.txt;
- load ends.txt;
- load Area.txt;
- load Izz.txt;
- load lyy.txt;
- load J.txt;
- load E.txt;
- load v.txt;
- load beta\_ang.txt;

## Generate the node id. Each line represents the 6 DOFs for one node.

- node\_id=zeros(nnodes,6);
- for i=1:nnodes
- for j=1:6
- node\_id(i,j)=(i-1)\*6+j;
- end
- end

1	2	3	4	5	6
7	8	9	10	11	12
13	14	15	16	17	18
19	20	21	22	23	24
25	26	27	28	29	30
31	32	33	34	35	36
37	38	39	40	41	42

## Generate the member id. Each line represents the 12 DOFs for one element.

- mem\_id=zeros(nele,12);
- for i=1:nele
- mem\_id(i,1:6)=node\_id(ends(i,1),1:6);
- mem\_id(i,7:12)=node\_id(ends(i,2),1:6);
- end

1	2	3	4	5	6	13	14	15	16	17	18	
13	14	15	16	17	18	7	8	9	10	11	12	
13	14	15	16	17	18	19	20	21	22	23	24	
19	20	21	22	23	24	25	26	27	28	29	30	
25	26	27	28	29	30	31	32	33	34	35	36	
25	26	27	28	29	30	37	38	39	40	41	42	

Generate the displacement boundary conditions.

Each line in fixity represents the disp BC on 6 DOFs of one node.

Orepresents that the DOF is fixed.

NaN represents that the DOF is free.

- fixity\_tran=fixity';
- D=fixity\_tran(:);

- free\_dof=find(isnan(D));
- fixed\_dof=find(D==0);

0	0	0	NaN	NaN	NaN	
0	0	0	NaN	NaN	NaN	
NaN	NaN	NaN	NaN	NaN	NaN	
NaN	NaN	NaN	NaN	NaN	NaN	
NaN	NaN	NaN	NaN	NaN	NaN	
0	0	0	NaN	NaN	NaN	
0	0	0	NaN	NaN	NaN	

1 2 3 7 8 9 31 32 33 37 38 39

4 5 6 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 34 35 36 40 41 42

Generate the force boundary conditions.

Each line in concen represents the force BC on 6 DOFs of one node.

Orepresents that the DOF does not has external force.

The number represents that the force values on the DOF.

```
concen_tran=concen';
```

```
P_total=concen_tran(:);
```

•

P\_free=P\_total(free\_dof);

0	0	0	0	0	0
0	0	0	0	0	0
0	0	0	0	0	0
0	0	-4.5	0	0	0
0	0	0	0	0	0
0	0	0	0	0	0
0	0	0	0	0	0

Generate the xaxis. xaxis will be used in the rotation matrix. The rotation matrix is to transform the stiffness matrix from global to local coordinate system.

- for i=1:nele
- xaxis(i,:)=(coord(ends(i,2),:)-coord(ends(i,1),:));
- Length(i) = norm(xaxis(i,:));
- xaxisunit(i,:)=xaxis(i,:)/Length(i);
- end

0.3713	0	0.9284
0.3713	0	-0.9284
0.0000	1	0.0000
0.0000	1	0.0000
-0.3713	0	-0.9284
0.3713	0	-0.9284

## Generate the local stiffness matrix for each element.

- k\_stack\_local=zeros(nele,12,12);
- gamma\_stack\_local=zeros(nele,12,12);
- for i=1:nele
- k\_stack\_local(i,:,:)=estiff(Area(i),Izz(i),Iyy(i),J(i),E(i),v(i),Length(i));
- gamma\_stack\_local(i,:,:)=etran(beta\_ang(i),xaxisunit(i,1:3));
- end

# Transform the local stiffness matrix to global stiffness matrix.

```
k_stack_global=zeros(nele,12,12);
for i=1:nele
k_stack_global(i,:,:)=(squeeze(gamma_stack_local(i,:,:))')...
*squeeze(k_stack_local(i,:,:))...
*squeeze(gamma_stack_local(i,:,:));
```

end

```
size(squeeze(k_stack_local(i,:,:)))

12    12
size(k_stack_local(i,:,:))

1    12    12
```

## Assemble the global stiffness matrix together.

- ndof=6\*nnodes;
- k\_total=zeros(ndof,ndof);
- for i=1:nele
- k\_total(mem\_id(i,1:6),mem\_id(i,1:6))=k\_total(mem\_id(i,1:6),mem\_id(i,1:6))+squeeze(k\_stack\_global(i,1:6,1:6));
- k\_total(mem\_id(i,1:6),mem\_id(i,7:12))=k\_total(mem\_id(i,1:6),mem\_id(i,7:12))+squeeze(k\_stack\_global(i,1:6,7:12));
- k\_total(mem\_id(i,7:12),mem\_id(i,1:6))=k\_total(mem\_id(i,7:12),mem\_id(i,1:6))+squeeze(k\_stack\_global(i,7:12,1:6));
- k\_total(mem\_id(i,7:12),mem\_id(i,7:12))= k\_total(mem\_id(i,7:12),mem\_id(i,7:12))+squeeze(k\_stack\_global(i,7:12,7:12));

$$\begin{bmatrix} K_{11} & K_{12} & K_{13} & K_{14} & K_{15} & K_{16} \\ K_{21} & K_{22} & K_{23} & K_{24} & K_{25} & K_{26} \\ K_{31} & K_{32} & K_{33} & K_{34} & K_{35} & K_{36} \\ K_{41} & K_{42} & K_{43} & K_{44} & K_{45} & K_{46} \\ K_{51} & K_{52} & K_{53} & K_{54} & K_{55} & K_{56} \\ K_{61} & K_{62} & K_{63} & K_{64} & K_{65} & K_{66} \end{bmatrix} \begin{bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6 \end{bmatrix} = \begin{bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \\ F_5 \\ F_6 \end{bmatrix}$$

## Extract the K\_ff.

K\_ff=k\_total(free\_dof,free\_dof);

K\_sf=k\_total(fixed\_dof,free\_dof);

C = det(A) returns the determinant for A.

If the structure is stable, det(A) is positive.

If the structure is unstable, det(A) is near 0.

```
if abs(det(K_ff))<(10^(-15))</li>
   AFLAG=0;
   display('Unstable Structure.');
   display('Maybe you should fix more DOFs .');
• else
   AFLAG=1;
   display('The structure is stable.');
end
```

C = rcond(A) returns the reciprocal condition number of A. If A is well conditioned, rcond(A) is near 1.0. If A is badly conditioned, rcond(A) is near 0.

```
• if abs(rcond(K_ff))<(10^(-15))
    BFLAG=0;
   display('The matrix is badly conditioned.');
   display('The results may have a large error. ');
   display('Some element may have a way bigger stiffness than others.');
   display('Maybe you need to check the units of element properties.');
else
    BFLAG=1;
   display('The matrix is well-conditioned.');
end
```

Put the deflection to its appropriate location. Include the fixed DOF.

- defl=K\_ff\P\_fr
- defl\_vector\_total=zeros(ndof,1);
- defl\_vector\_total(free\_dof)=defl;

Each line in DEFL' represents the 6 DOFs at each node.

- DEFL=reshape(defl\_vector\_total,6,nnodes);
- DEFL=DEFL';
- display(DEFL)

#### Generate the reaction.

```
react=K_sf*defl;
```

- react\_vector\_total=zeros(ndof,1);
- react\_vector\_total(fixed\_dof)=react;
- REACT=reshape(react\_vector\_total,6,nnodes);
- REACT=REACT';
- display(REACT)

Calculate the element force. Transform the element from from global to local coordiante system.

```
ELE_FOR=zeros(nele,12);
• for i=1:nele
    d_global_tmp=zeros(1,12);
    d_global_tmp(1:6)=DEFL(ends(i,1),1:6);
    d_global_tmp(7:12)=DEFL(ends(i,2),1:6);
    d_global_tmp=d_global_tmp';
    f_local_tmp=squeeze(k_stack_local(i,:,:))*squeeze(gamma_stack_local(i,:,:))*d_global_tmp;
    for j=1:12
    ELE_FOR(i,j)=f_local_tmp(j);
    end

    end

display(ELE_FOR)
```

- The structure is stable.
- The matrix is well-conditioned.

#### • DEFL =

	1	1 2		4	5	6
1	0	0	0	7.5737e-04	2.5418e-06	-0.0013
2	0	0	0	7.5737e-04	-2.5418e-06	0.0013
3	-9.3650e-18	0.0026	-0.0123	-0.0026	-2.8740e-21	-3.4050e-19
4	3.7910e-16	-5.3331e-12	-4.4649	-5.2095e-16	-1.3301e-21	9.6745e-21
5	-2.1273e-18	-0.0026	-0.0123	0.0026	-5.3117e-23	2.8733e-19
6	0	0	0	-7.5737e-04	2.5418e-06	0.0013
7	0	0	0	-7.5737e-04	-2.5418e-06	-0.0013
_						

#### • REACT =

	1	2	3	4	5	6
1	0.4498	0.2505	1.1250	0	0	0
2	-0.4498	0.2505	1.1250	0	0	0
3	0	0	0	0	0	0
4	0	0	0	0	0	0
5	0	0	0	0	0	0
6	0.4498	-0.2505	1.1250	0	0	0
7	-0.4498	-0.2505	1.1250	0	0	0

• ELE\_FOR =

	1	2	3	4	5	6	7	8	9	10	11	12
1	1.2116	0.2505	1.7670e-04	3.6971e-14	1.7152e-17	3.3173e-14	-1.2116	-0.2505	-1.7670e-04	-3.6971e-14	-0.4758	674.5522
2	1.2116	-0.2505	-1.7670e-04	-1.4211e-14	0.4758	-674.5522	-1.2116	0.2505	1.7670e-04	1.4211e-14	-1.1102e-16	2.8422e-14
3	0.5010	1.2575e-16	2.2500	-1.9951e-16	-1.2526e+03	3.5484e-14	-0.5010	-1.2575e-16	-2.2500	1.9951e-16	-2.1224e+03	1.5314e-13
4	0.5010	-1.4200e-16	-2.2500	-1.6503e-16	2.1224e+03	-1.5314e-13	-0.5010	1.4200e-16	2.2500	1.6503e-16	1.2526e+03	-5.9851e-14
5	1.2116	0.2505	1.7670e-04	0	-0.4758	674.5522	-1.2116	-0.2505	-1.7670e-04	0	3.3307e-16	-2.8422e-14
6	1.2116	0.2505	-1.7670e-04	-2.8422e-14	0.4758	674.5522	-1.2116	-0.2505	1.7670e-04	2.8422e-14	-8.8818e-16	-8.5265e-14
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## Thanks!

• Questions?