



# 有限元方法及应用

题 目:	有限元大作业
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时 间:	2024. 2. 23

## 空间 8 节点六面体单元分析

**问题描述：**如图 1 所示的一个空间块体，在右端部受两个集中力  $F$  作用，其中的参数为： $E=1\times 10^{10}$  Pa,  $\mu=0.25$ ,  $t=0.2$  m,  $F=1\times 10^5$  N。基于 Matlab，用一个空间 8 节点六面体单元计算各个节点位移、支座反力以及单元的应力。

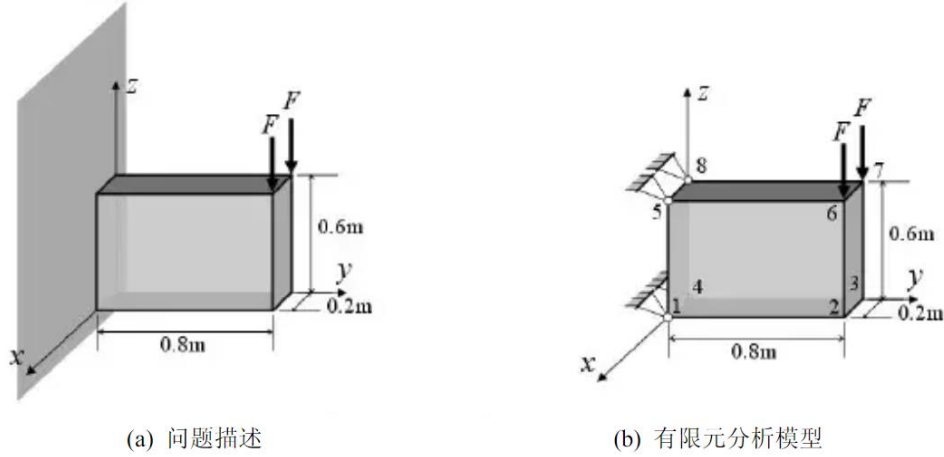


图 1 右端部受集中力作用的空间块体

### 理论分析：

#### (1) 结构的离散化与编号

将结构离散为一个 8 节点六面体单元，节点编号如图 1(b) 所示，节点的几何坐标见表 1。

表 1 节点的坐标

节点	x	y	z
1	0.2	0	0
2	0.2	0.8	0
3	0	0.8	0
4	0	0	0
5	0.2	0	0.6
6	0.2	0.8	0.6
7	0	0.8	0.6
8	0	0	0.6

节点位移列阵

$$q^e = [u_1 \ v_1 \ w_1 \ u_2 \ v_2 \ w_2 \ \dots \ u_8 \ v_8 \ w_8]^T$$

总的节点载荷列阵

$$P^e = [P_{x1} \ P_{y1} \ P_{z1} \ P_{x2} \ P_{y2} \ P_{z2} \ \dots \ P_{x8} \ P_{y8} \ P_{z8}]^T$$

## （2）计算单元的刚度矩阵（以国际标准单位）

首先在MATLAB环境下，输入弹性模量E、泊松比NU，然后针对题中单元节点坐标，调用函数Hexahedral3D8Node\_Stiffness，就可以得到单元的刚度矩阵k1（24×24）。

## （3）建立整体刚度方程

由于该结构共有8个节点，则总共的自由度数为24，因此，结构总的刚度矩阵为KK(24×24)，先对KK清零，然后调用函数Hexahedral3D8Node\_Assembly进行刚度矩阵的组装。由于本题中只用了一个单元，因此总体刚度矩阵KK与单元刚度k1相同。

## （4）边界条件的处理及刚度方程求解

由图1(b)可以看出，节点1、节点4、节点5和节点8的三个方向的位移将为零，即 $u_1 = v_1 = w_1 = 0, u_4 = v_4 = w_4 = 0, u_5 = v_5 = w_5 = 0, u_8 = v_8 = w_8 = 0$ 。因此，将针对节点2、节点3、节点6和节点7的位移进行求解，这4个节点的位移将对应KK矩阵中的4至9行、4至9列，4至9行、16至21列，16至21行、4至9列，以及16至21行、16至21列，则需从KK(24×24)中提出，置给k，然后生成对应的载荷列阵p，再采用高斯消去法进行求解，即MATLAB中的反斜线符号“\”求解。

## （5）支反力的计算

由方程 $\mathbf{K} \cdot \mathbf{q} = \mathbf{P}$ 可知，在得到整个结构的节点位移后，由原整体刚度方程就可以计算出对应的支反力；先将上面得到的位移结果与位移边界条件的节点位移进行组合(注意位置关系)，可以得到整体的位移列阵U(24×1)，再代回原整体刚度方程，计算出所有的节点力P(24×1)，按式的对应关系就可以找到对应的支反力。

## （6）各单元的应力计算

先从整体位移列阵U(24×1)中提取出单元的位移列阵，然后，调用计算单元应力的函数Hexahedral3D8Node\_Stress，就可以得到各个单元的应力分量。

### 代码：

```
function k =  
Hexahedral3D8Node_Stiffness(E,NU,x1,y1,z1,x2,y2,z2,x3,y3,z3,x4,y4,z4,  
x5,y5,z5,x6,y6,z6,x7,y7,z7,x8,y8,z8)  
syms s t n;  
N1=(1+s)*(1-t)*(1-n)/8;  
N2=(1+s)*(1+t)*(1-n)/8;  
N3=(1-s)*(1+t)*(1-n)/8;  
N4=(1-s)*(1-t)*(1-n)/8;  
N5=(1+s)*(1-t)*(1+n)/8;
```

```

N6=(1+s)*(1+t)*(1+n)/8;
N7=(1-s)*(1+t)*(1+n)/8;
N8=(1-s)*(1-t)*(1+n)/8;
x=N1*x1+N2*x2+N3*x3+N4*x4+N5*x5+N6*x6+N7*x7+N8*x8;
y=N1*y1+N2*y2+N3*y3+N4*y4+N5*y5+N6*y6+N7*y7+N8*y8;
z=N1*z1+N2*z2+N3*z3+N4*z4+N5*z5+N6*z6+N7*z7+N8*z8;
J=[diff(x,s),diff(y,s),diff(z,s);diff(x,t),diff(y,t),diff(z,t);diff(x
,n),diff(y,n),diff(z,n)];
Jdet=det(J);
J
Jdet
a=diff(y,t)*diff(z,n)-diff(z,t)*diff(y,n);
b=diff(y,s)*diff(z,n)-diff(z,s)*diff(y,n);
c=diff(y,s)*diff(z,t)-diff(z,s)*diff(y,t);
d=diff(x,t)*diff(z,n)-diff(z,t)*diff(x,n);
e=diff(x,s)*diff(z,n)-diff(z,s)*diff(x,n);
f=diff(x,s)*diff(z,t)-diff(z,s)*diff(x,t);
g=diff(x,t)*diff(y,n)-diff(y,t)*diff(x,n);
h=diff(x,s)*diff(y,n)-diff(y,s)*diff(x,n);
l=diff(x,s)*diff(y,t)-diff(y,s)*diff(x,t);
Ns=[N1,N2,N3,N4,N5,N6,N7,N8];
Bs=sym(zeros(6,3,8));
for i=1:8
    Bs(:,:,i)=[a*diff(Ns(i),s)-
b*diff(Ns(i),t)+c*diff(Ns(i),n),0,0;0,-
d*diff(Ns(i),s)+e*diff(Ns(i),t)-f*diff(Ns(i),n),0;
0,0,g*diff(Ns(i),s)-h*diff(Ns(i),t)+l*diff(Ns(i),n);
-d*diff(Ns(i),s)+e*diff(Ns(i),t)-
f*diff(Ns(i),n),a*diff(Ns(i),s)-b*diff(Ns(i),t)+c*diff(Ns(i),n),0;
0,g*diff(Ns(i),s)-h*diff(Ns(i),t)+l*diff(Ns(i),n),-
d*diff(Ns(i),s)+e*diff(Ns(i),t)-f*diff(Ns(i),n);
g*diff(Ns(i),s)-
h*diff(Ns(i),t)+l*diff(Ns(i),n),0,a*diff(Ns(i),s)-
b*diff(Ns(i),t)+c*diff(Ns(i),n)]/Jdet;
end
B=[Bs(:,:,1),Bs(:,:,2),Bs(:,:,3),Bs(:,:,4),Bs(:,:,5),Bs(:,:,6),Bs(:,:,
,7),Bs(:,:,8)];
B
D=(E/((1+NU)*(1-2*NU)))*[1-NU,NU,NU,0,0,0;NU,1-NU,NU,0,0,0;NU,NU,1-
NU,0,0,0;0,0,0,0.5-NU,0,0;0,0,0,0,0.5-NU,0;0,0,0,0,0,0.5-NU];
D
BD=Jdet*transpose(B)*D*B;
z=(int(int(int(BD,n,-1,1),t,-1,1),s,-1,1));
z

```

```

k=double(z);
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
function z = Hexahedral3D8Node_Assembly(KK,k,i,j,l,m,n,o,p,q)
DOF=[3*i-2:3*i,3*j-2:3*j,3*l-2:3*l,3*m-2:3*m,3*n-2:3*n,3*o-2:3*o,3*p-
2:3*p,3*q-2:3*q];
for n1=1:24
    for n2=1:24
        KK(DOF(n1),DOF(n2))=KK(DOF(n1),DOF(n2))+k(n1,n2);
    end
end
z=KK;
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
function
stress=Hexahedral3D8Node_Stress(E,NU,x1,y1,z1,x2,y2,z2,x3,y3,z3,x4,y4
,z4,x5,y5,z5,x6,y6,z6,x7,y7,z7,x8,y8,z8,u)
syms s t n;
N1=(1+s)*(1-t)*(1-n)/8;
N2=(1+s)*(1+t)*(1-n)/8;
N3=(1-s)*(1+t)*(1-n)/8;
N4=(1-s)*(1-t)*(1-n)/8;
N5=(1+s)*(1-t)*(1+n)/8;
N6=(1+s)*(1+t)*(1+n)/8;
N7=(1-s)*(1+t)*(1+n)/8;
N8=(1-s)*(1-t)*(1+n)/8;
x=N1*x1+N2*x2+N3*x3+N4*x4+N5*x5+N6*x6+N7*x7+N8*x8;
y=N1*y1+N2*y2+N3*y3+N4*y4+N5*y5+N6*y6+N7*y7+N8*y8;
z=N1*z1+N2*z2+N3*z3+N4*z4+N5*z5+N6*z6+N7*z7+N8*z8;
J=[diff(x,s),diff(y,s),diff(z,s);diff(x,t),diff(y,t),diff(z,t);diff(x
,n),diff(y,n),diff(z,n)];
Jdet=det(J);
a=diff(y,t)*diff(z,n)-diff(z,t)*diff(y,n);
b=diff(y,s)*diff(z,n)-diff(z,s)*diff(y,n);
c=diff(y,s)*diff(z,t)-diff(z,s)*diff(y,t);
d=diff(x,t)*diff(z,n)-diff(z,t)*diff(x,n);
e=diff(x,s)*diff(z,n)-diff(z,s)*diff(x,n);
f=diff(x,s)*diff(z,t)-diff(z,s)*diff(x,t);
g=diff(x,t)*diff(y,n)-diff(y,t)*diff(x,n);
h=diff(x,s)*diff(y,n)-diff(y,s)*diff(x,n);
l=diff(x,s)*diff(y,t)-diff(y,s)*diff(x,t);
Ns=[N1,N2,N3,N4,N5,N6,N7,N8];
Bs=sym(zeros(6,3,8));
for i=1:8
    Bs(:, :, i)=[a*diff(Ns(i),s)-
b*diff(Ns(i),t)+c*diff(Ns(i),n),0,0;0,-

```

```

d*diff(Ns(i),s)+e*diff(Ns(i),t)-f*diff(Ns(i),n),0;
    0,0,g*diff(Ns(i),s)-h*diff(Ns(i),t)+l*diff(Ns(i),n);
    -d*diff(Ns(i),s)+e*diff(Ns(i),t)-
f*diff(Ns(i),n),a*diff(Ns(i),s)-b*diff(Ns(i),t)+c*diff(Ns(i),n),0;
    0,g*diff(Ns(i),s)-h*diff(Ns(i),t)+l*diff(Ns(i),n),-
d*diff(Ns(i),s)+e*diff(Ns(i),t)-f*diff(Ns(i),n);
    g*diff(Ns(i),s)-
h*diff(Ns(i),t)+l*diff(Ns(i),n),0,a*diff(Ns(i),s)-
b*diff(Ns(i),t)+c*diff(Ns(i),n)]/Jdet;
    end
B=[Bs(:, :, 1),Bs(:, :, 2),Bs(:, :, 3),Bs(:, :, 4),Bs(:, :, 5),Bs(:, :, 6),Bs(:, :,
, 7),Bs(:, :, 8)];
D=(E/( (1+NU)*(1-2*NU)))*[1-NU,NU,NU,0,0,0;NU,1-NU,NU,0,0,0;NU,NU,1-
NU,0,0,0;0,0,0,0.5-NU,0,0;0,0,0,0,0.5-NU,0;0,0,0,0,0,0.5-NU];
w=D*B*u;
wcent=subs(w,{s,t,n},{0,0,0})
    stress=double(wcent)
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%run_code
clear;clc;
E=1.0e10;NU=0.25;lx=0.2;ly=0.8;lz=0.6;
k1=Hexahedral3D8Node_Stiffness(E,NU,lx,0,0,lx,ly,0,0,ly,0,0,0,0,lx,0,
lz,lx,ly,lz,0,ly,lz,0,0,lz);
KK=zeros(24,24);
KK=Hexahedral3D8Node_Assembly(KK,k1,1,2,3,4,5,6,7,8);
k=[KK(4:9,4:9),KK(4:9,16:21);KK(16:21,4:9),KK(16:21,16:21)];
p=[0;0;0;0;0;0;0;0;-1e5;0;0;-1e5];
u=k\p
U=[0;0;0;u(1:6);0;0;0;0;0;0;u(7:12);0;0;0];
P=KK*U
u1=U(1:24);
stress1=Hexahedral3D8Node_Stress(E,NU,lx,0,0,lx,ly,0,0,ly,0,0,0,0,lx,
0,lz,lx,ly,lz,0,ly,lz,0,0,lz,u1)

```

## 代码输出结果:

各个节点位移（单位为m）:

$$u_2 = 0.0223, v_2 = -0.2769, w_2 = -0.6728,$$

$$u_3 = -0.0223, v_3 = -0.2769, w_3 = -0.6728,$$

$$u_6 = -0.0129, v_6 = 0.3108, w_6 = -0.7774,$$

$$u_7 = 0.0129, v_7 = 0.3108, w_7 = -0.7774$$

各个节点支座反力（单位为N）:

$$R_{x1} = -0.2509, R_{y1} = 1.3333, R_{z1} = 0.6938,$$

$$R_{x4} = 0.2509, R_{y4} = 1.3333, R_{z4} = 0.6938,$$

$$R_{x5} = 0.3455, R_{y5} = -1.3333, R_{z5} = 0.3062,$$

$$R_{x8} = -0.3455, R_{y8} = -1.3333, R_{z8} = 0.3062$$

单元 1 的中心点的应力分量为（单位为 Pa）：

$$\sigma_x = 19700, \sigma_y = 0, \sigma_z = -867300,$$

$$\tau_{xy} = 0, \tau_{zx} = 1666700, \tau_{yz} = 0$$