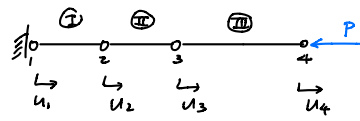
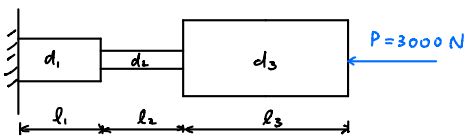


PROBLEM 1



	$d (m)$	$A (m^2)$	$l (m)$	$E (Pa)$
1	4×10^{-2}	$4 \times 10^{-4} \pi$	0.1	80×10^9
2	2×10^{-2}	$1 \times 10^{-4} \pi$	0.1	
3	6×10^{-2}	$9 \times 10^{-4} \pi$	0.2	

1.1. elementary stiffness matrices

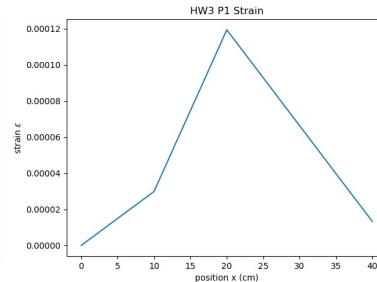
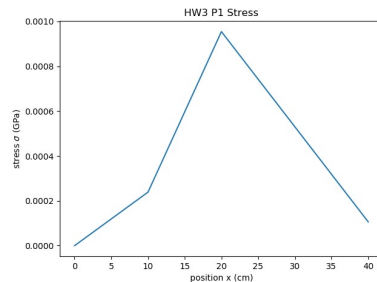
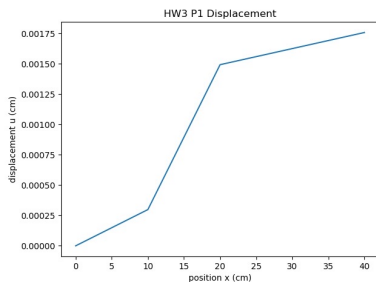
$$\begin{aligned}
 \text{I: } [k]^{\text{I}} &= \frac{EA_1}{l_1} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = k_1 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}, & k_1 &= 3.2 \times 10^8 \pi \text{ N/m} \\
 \text{II: } [k]^{\text{II}} &= \frac{EA_2}{l_2} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = k_2 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}, & k_2 &= 8.0 \times 10^7 \pi \text{ N/m} \\
 \text{III: } [k]^{\text{III}} &= \frac{EA_3}{l_3} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = k_3 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}, & k_3 &= 3.6 \times 10^8 \pi \text{ N/m}
 \end{aligned}$$

$$\begin{aligned}
 1.2. \quad [K] &= \begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 & k_1+k_2 & -k_2 & 0 \\ 0 & -k_2 & k_2+k_3 & -k_3 \\ 0 & 0 & -k_3 & k_3 \end{bmatrix} = \begin{bmatrix} 3.2 \times 10^8 & -3.2 \times 10^8 & 0 & 0 \\ -3.2 \times 10^8 & 4.0 \times 10^8 & -8.0 \times 10^7 & 0 \\ 0 & -8.0 \times 10^7 & 4.4 \times 10^8 & -3.6 \times 10^8 \\ 0 & 0 & -3.6 \times 10^8 & 3.6 \times 10^8 \end{bmatrix} \cdot \pi \text{ N/m} \\
 \{F\} &= \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -P \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -3000 \end{Bmatrix} \text{ N}
 \end{aligned}$$

$$\begin{aligned}
 1.3. \quad [K]\{u\} &= \{F\} \Rightarrow \{u\} = [K]^{-1}\{F\} \\
 &\text{enforce displacement B.C. at node I: } u_1 = 0 \\
 &\Rightarrow \text{row 1 \& col 1 in } [K] \text{ and } F[1] \text{ all go to zero for consistency}
 \end{aligned}$$

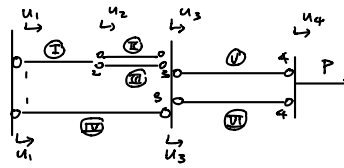
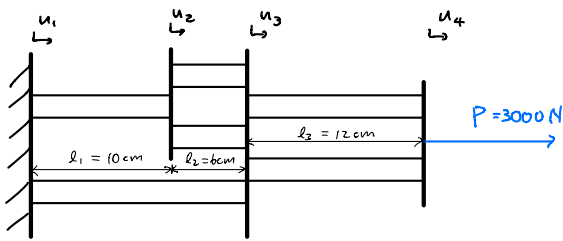
$$\begin{bmatrix} k_1+k_2 & -k_2 & 0 \\ -k_2 & k_2+k_3 & -k_3 \\ 0 & -k_3 & k_3 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \\ u_4 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ -P \end{Bmatrix} \Rightarrow \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 2.98 \times 10^{-8} \\ 1.49 \times 10^{-7} \\ 1.76 \times 10^{-7} \end{Bmatrix} \text{ m} = \begin{Bmatrix} 0 \\ 0.000298 \\ 0.00149 \\ 0.00176 \end{Bmatrix} \text{ cm}$$

$$1.4. \quad \epsilon = \frac{\partial u}{\partial x} = \frac{\text{diff}(u)}{\text{diff}(x)} \quad \sigma = E \epsilon$$



Stress & strain plots look the same except for stress (σ) is scaled by E .

PROBLEM 2



$$\left. \begin{aligned} E &= 70 \text{ GPa} = 7 \times 10^{10} \text{ Pa} \\ A &= 0.1 \text{ cm}^2 = 1 \times 10^{-5} \text{ m}^2 \end{aligned} \right\} \text{ for all elements}$$

2.1.

elementary stiffness matrices:

$$\textcircled{I}: [k]^{\textcircled{I}} = \frac{EA}{l_1} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = k_1 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}^{\textcircled{1} \textcircled{2}}$$

$$\textcircled{II}: [k]^{\textcircled{II}} = \frac{EA}{l_2} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = k_2 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}^{\textcircled{2} \textcircled{3}}$$

$$\textcircled{III}: [k]^{\textcircled{III}} = \frac{EA}{l_2} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = k_3 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}^{\textcircled{3} \textcircled{4}}$$

$$\textcircled{IV}: [k]^{\textcircled{IV}} = \frac{EA}{l_1 + l_2} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = k_4 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}^{\textcircled{1} \textcircled{3}}$$

$$\textcircled{V}: [k]^{\textcircled{V}} = \frac{EA}{l_3} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = k_5 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}^{\textcircled{3} \textcircled{6}}$$

$$\textcircled{VI}: [k]^{\textcircled{VI}} = \frac{EA}{l_3} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = k_6 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}^{\textcircled{4} \textcircled{6}}$$

i	$l_i \text{ (m)}$	$k_i \text{ (10}^6 \text{ N/m)}$
1	0.10	7
2	0.06	11.667
3	0.06	11.667
4	0.16	4.375
5	0.12	5.833
6	0.12	5.833

assemble global $[K]$

$$[K] = \begin{bmatrix} \textcircled{1} & \textcircled{2} & \textcircled{3} & \textcircled{4} \\ k_1 + k_4 & -k_1 & -k_4 & 0 \\ -k_1 & k_1 + k_2 + k_3 & -k_2 - k_3 & 0 \\ -k_4 & -k_2 - k_3 & k_2 + k_3 + k_4 + k_5 + k_6 & -k_5 - k_6 \\ 0 & 0 & -k_5 - k_6 & k_5 + k_6 \end{bmatrix}^{\textcircled{1} \textcircled{2} \textcircled{3} \textcircled{4}}$$

force vector

$$\{F\} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ P \end{Bmatrix}$$

apply displacement B.C. @ node 1 : $u_1 = 0$

\Rightarrow row 1 & col 1 in $[K]$ go to 0

$$\begin{bmatrix} k_1 + k_2 + k_3 & -k_2 - k_3 & 0 \\ -k_2 - k_3 & k_2 + k_3 + k_4 + k_5 + k_6 & -k_5 - k_6 \\ 0 & -k_5 - k_6 & k_5 + k_6 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \\ u_4 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ P \end{Bmatrix}$$

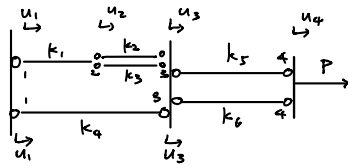
$$\begin{bmatrix} k_1 + k_2 + k_3 & -k_2 - k_3 & 0 \\ -k_2 - k_3 & k_2 + k_3 + k_4 + k_5 + k_6 & -k_5 - k_6 \\ 0 & -k_5 - k_6 & k_5 + k_6 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \\ u_4 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ P \end{Bmatrix}$$

$$\Rightarrow \begin{bmatrix} 30.333 & -23.333 & 0 \\ -23.333 & 39.375 & -11.667 \\ 0 & -11.667 & 11.667 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \\ u_4 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 3000 \end{Bmatrix} \Rightarrow$$

$$\begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0.000236 \\ 0.000307 \\ 0.000565 \end{Bmatrix} \text{ m} = \begin{Bmatrix} 0 \\ 0.0236 \\ 0.0307 \\ 0.0565 \end{Bmatrix} \text{ cm}$$

PROBLEM 2 CONT'D

2.2.



parallel : $k_{eq} = k_i + k_j$

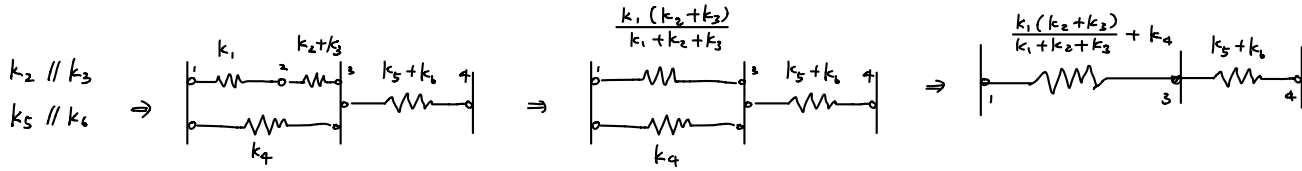
$u_{eq} = u_1 = u_2$

$F_{eq} = F_1 + F_2$

series : $k_{eq} = \frac{k_i k_j}{k_i + k_j}$

$u_{eq} = u_1 + u_2$

$F_{eq} = F_1 = F_2$



$$k_{eq} = \frac{\left(\frac{k_1 (k_2 + k_3)}{k_1 + k_2 + k_3} + k_4 \right) \cdot (k_5 + k_6)}{\left(\frac{k_1 (k_2 + k_3)}{k_1 + k_2 + k_3} + k_4 \right) + (k_5 + k_6)}$$

$k_{eq} = 5.314 \times 10^6 \text{ N/m}$

Hook's law : $u_4 = \frac{P}{k_{eq}}$

$u_3 = \frac{P}{k_{1234}}$

$u_2 = \frac{k_2 + k_3}{k_1 + k_2 + k_3} u_3$

$k_1 u_2 = (k_2 + k_3)(u_3 - u_2)$

$(k_1 + k_2 + k_3) u_2 = (k_2 + k_3) u_3$

$$\begin{Bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0.000236 \\ 0.000307 \\ 0.000565 \end{Bmatrix} \text{ m} = \begin{Bmatrix} 0 \\ 0.0236 \\ 0.0307 \\ 0.0565 \end{Bmatrix} \text{ cm}$$

* same as FEM