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Module Name: Multivariate Methods for Data Analysis

Individual Assignment

Multiple Linear Regression & Factor Analysis (SPSS)

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1 Dataset Context & Metadata

1.1 Dataset Description

The dataset used in this study is the *Concrete Compressive Strength Dataset*, obtained from the UCI Machine Learning Repository (<https://archive.ics.uci.edu/dataset/165/concrete+compressive+strength>). It contains laboratory test results of concrete mixtures and their corresponding compressive strength values. The dataset consists of 1030 instances, each representing a mixture defined by its ingredient composition and curing age, with the compressive strength measured in megapascals (MPa).

The data is multivariate in nature and includes eight quantitative input variables representing the concrete ingredients and age, and one quantitative output variable representing the concrete compressive strength.

1.2 Metadata

The following table summarises the quantitative variables included in the dataset. At least one dependent variable and eight independent variables are included, satisfying the requirements for both Multiple Linear Regression (Q1) and Factor Analysis (Q2).

Variable Name (Label)	Type	SPSS Measure	Role (DV/IV)
Cement (kg/m ³)	Numeric	Scale	Independent
Blast Furnace Slag (kg/m ³)	Numeric	Scale	Independent
Fly Ash (kg/m ³)	Numeric	Scale	Independent
Water (kg/m ³)	Numeric	Scale	Independent
Superplasticizer (kg/m ³)	Numeric	Scale	Independent
Coarse Aggregate (kg/m ³)	Numeric	Scale	Independent
Fine Aggregate (kg/m ³)	Numeric	Scale	Independent
Age (days)	Numeric	Scale	Independent
Concrete Compressive Strength (MPa)	Numeric	Scale	Dependent Variable

Table 1: Metadata for the Concrete Compressive Strength Dataset (verified in SPSS).

1.3 Preprocessing

Minimal preprocessing was required as the dataset was already clean and free of missing values. All variables were confirmed as quantitative (Scale) in SPSS and kept for analysis.

2 Question 1: Multiple Linear Regression

2.1 Introduction (Q1a)

Concrete is one of the most commonly used construction materials in the world because of its versatility, durability, and cost-effectiveness. Among its properties, compressive strength is considered the most crucial, as it directly determines the structural integrity and service life of concrete infrastructure. Ensuring the desired strength of concrete is important for safety, as subpar performance can result in structural failures with detrimental economic and human consequences. Traditional assessment methods rely on standardized compression tests, usually performed at 28 days, but these procedures are time-consuming, labor intensive, and sensitive to experimental errors (Sah & Hong, 2024).

To address these challenges, researchers have turned to predictive modeling approaches that estimate compressive strength based on input variables such as cement, aggregates, water, admixtures, and curing age. These models enable early strength estimation, which is helpful in making choices during construction and helps with mix optimisation before the complete curing period has passed. Predictive models present an opportunity to increase the efficiency and reliability of quality assurance processes by reducing the need for prolonged laboratory testing.

However, standard empirical models often struggle to accurately capture complex, nonlinear interactions between concrete components, limiting their effectiveness for prediction. As Nikoopayan Tak et al. (2025) states, such methods struggle to account for variability across mixes, illustrating the importance of strong computational tools that are able to accurately model these interactions. As a result, both statistical and machine learning approaches have been used in recent years, with multiple studies demonstrating that emerging methods surpass classical models in accuracy. Simpler regression-based models, on the other hand, continue to be useful because of their interpretability and ease of application in real-world engineering situations.

Building on this foundation, this study applies Multiple Linear Regression (MLR) to the UCI Concrete Compressive Strength dataset. By analysing the predictive influence of cement, slag, fly ash, water, superplasticizer, aggregates, and curing time, the analysis aims to identify the most significant determinants of compressive strength. In doing so, this study seeks to demonstrate how MLR can balance interpretability with predictive performance, providing practical information for data-driven improvements in concrete mix design and contributing to safer and more efficient construction practices

2.2 Problem Statement (Q1b)

A constant challenge in construction is the time and cost associated with measuring concrete compressive strength through standard laboratory tests, which typically require a 28-day curing period (Sah & Hong, 2024). This delay leaves projects vulnerable to safety and financial risks, as inadequate strength may only be identified after critical stages of construction.

The problem addressed in this study is the need for a reliable and interpretable predictive model that can estimate compressive strength from known proportions of the material and curing age. Multiple Linear Regression (MLR) is well suited for this task because as it quantifies the relationship between strength (dependent variable) and eight quantitative predictors (cement, slag, fly ash, water, superplasticizer, coarse aggregate, fine aggregate, and age).

Using the UCI Concrete Compressive Strength dataset with 1030 observations, this research applies MLR to identify the most significant predictors of strength and to provide a practical, data-driven tool that supports safer and more efficient decision-making in concrete mix design.

2.3 Research Objectives and Hypotheses (Q1c)

The aim of this study is to develop a Multiple Linear Regression (MLR) model to predict concrete compressive strength using quantitative variables from the UCI dataset. To achieve this aim, the following objectives and hypotheses are established.

- **Objective 1:** To evaluate the effect of material composition variables (cement, slag, fly ash, water, superplasticizer, coarse aggregate, and fine aggregate) on compressive strength.
 - H_0 : Material composition variables have no significant effect on compressive strength.
 - H_1 : At least one material composition variable has a significant effect on compressive strength.
- **Objective 2:** To examine the effect of curing age (days) on compressive strength.
 - H_0 : Curing age has no significant effect on compressive strength.
 - H_1 : Curing age has a significant effect on compressive strength.
- **Objective 3:** To test whether the combination of material composition and curing age collectively explains significant variance in compressive strength.
 - H_0 : Material composition and curing age together do not significantly explain compressive strength.

- H_1 : Material composition and curing age together significantly explain compressive strength.

2.4 Analysis Results and Interpretations (Q1d)

2.4.1 Dataset Evidence (Q1d-i)

Figure 1 shows the SPSS overview confirming that the dataset contains 1030 cases and 9 variables. All variables were defined as *Scale* (quantitative) in SPSS, as shown in Figure 2.



Figure 1: SPSS dataset overview confirming 1030 cases and 9 variables.

	Name	Type	Width	Decimals	Label	Values	Missing	Columns	Align	Measure	Role
1	cement	Numeric	8	2	Cement (kg/m ³)	None	None	11	Right	Scale	Input
2	slag	Numeric	8	2	Blast Furnace Slag (kg/m ³)	None	None	11	Right	Scale	Input
3	flyash	Numeric	8	2	Fly Ash (kg/m ³)	None	None	11	Right	Scale	Input
4	water	Numeric	8	2	Water (kg/m ³)	None	None	11	Right	Scale	Input
5	superplast	Numeric	8	3	Superplasticizer (kg/m ³)	None	None	11	Right	Scale	Input
6	coarseagg	Numeric	8	2	Coarse Aggregate (kg/m ³)	None	None	11	Right	Scale	Input
7	fineagg	Numeric	8	2	Fine Aggregate (kg/m ³)	None	None	11	Right	Scale	Input
8	age	Numeric	11	0	Curing Age (days)	None	None	11	Right	Scale	Input
9	strength	Numeric	8	12	Compressive Strength (MPa)	None	None	11	Right	Scale	Input

Figure 2: SPSS Variable View showing all variables as quantitative (Scale).

2.4.2 Stepwise Multiple Linear Regression (Q1d-ii)

A stepwise multiple linear regression was conducted in SPSS with *Compressive Strength (MPa)* as the dependent variable. The independent variables included eight quantitative predictors: cement (kg/m³), blast furnace slag (kg/m³), fly ash (kg/m³), water (kg/m³), superplasticizer (kg/m³), coarse aggregate (kg/m³), fine aggregate (kg/m³), and curing age (days). These variables represent the mixture proportions and curing conditions of concrete. SPSS applied the stepwise method to retain only statistically significant predictors that improved the overall model fit, with the detailed results presented in the following subsections.

2.4.3 Assumption Testing (Q1d-iii)

To ensure the validity of the regression results, the four key assumptions of multiple linear regression were examined: normality, linearity, homoscedasticity, and multicollinearity.

Normality of residuals

As the sample size exceeded 50 ($N = 1030$), the Kolmogorov–Smirnov test was considered more appropriate than the Shapiro–Wilk test for assessing normality. The result ($p = .025$) indicated a statistically significant deviation from perfect normality. However, the Normal P–P plot of the standardized residuals (Figure 3) showed points closely followed the diagonal line, suggesting that the residuals were approximately normally distributed. Therefore, the normality assumption was considered satisfied.

Tests of Normality						
	Kolmogorov-Smirnov ^a			Shapiro-Wilk		
	Statistic	df	Sig.	Statistic	df	
Unstandardized Residual	.031	1030	.025	.996	1030	.007

a. Lilliefors Significance Correction

Table 2: Kolmogorov–Smirnov and Shapiro–Wilk tests of normality for regression residuals.

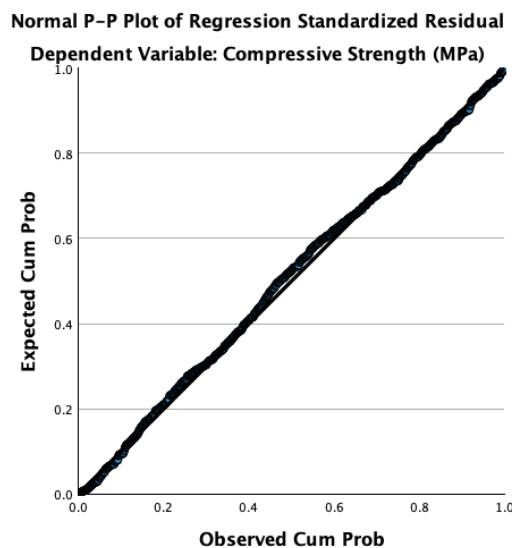


Figure 3: Normal P–P plot of regression standardized residuals (Model 6).

Linearity and Homoscedasticity

The scatterplot of standardized residuals against predicted values (Figure 4) displayed a random pattern without systematic curvature or U-shapes, confirming linearity. The spread of residuals was relatively constant across predicted values, with no funnel-shaped pattern present, indicating that the assumption of homoscedasticity (equal variance of errors) was also satisfied.

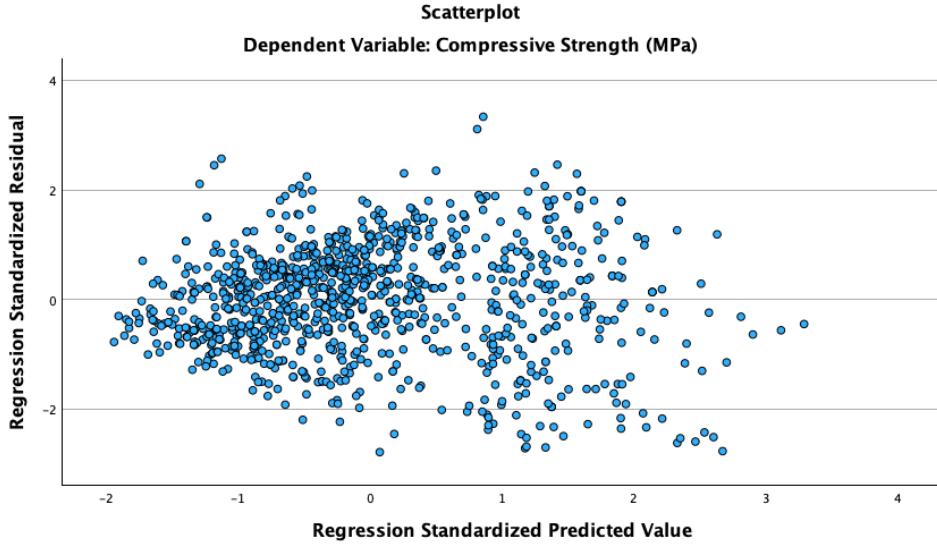


Figure 4: Scatterplot of standardized residuals versus predicted values (Model 6).

Multicollinearity

The coefficients output for the final regression model (Model 6) reported Variance Inflation Factor (VIF) values all below 2.5 and tolerance values above 0.1, as shown in Table 3. These results confirm that multicollinearity was not a concern in the final regression model.

Model	Coefficients ^a							Collinearity Statistics	
	B	Std. Error	Standardized Coefficients Beta	t	Sig.	95.0% Confidence Interval for B Lower Bound	Upper Bound	Tolerance	VIF
1	(Constant)	13.443	1.297		10.365	<.001	10.898	15.988	
	Cement (kg/m ³)	.080	.004	.498	18.405	<.001	.071	.088	1.000
2	(Constant)	9.190	1.250		7.350	<.001	6.737	11.644	
	Cement (kg/m ³)	.075	.004	.468	18.534	<.001	.067	.083	.991
3	(Constant)	5.098	1.147		4.446	<.001	2.848	7.348	
	Cement (kg/m ³)	.069	.004	.431	18.976	<.001	.062	.076	.981
4	(Constant)	-2.001	1.208		-1.657	.098	-4.371	.369	
	Cement (kg/m ³)	.081	.004	.508	23.076	<.001	.074	.088	.905
5	(Constant)	35.492	4.304		8.246	<.001	27.046	43.938	
	Cement (kg/m ³)	.081	.003	.509	24.007	<.001	.075	.088	.905
6	(Constant)	29.030	4.212		6.891	<.001	20.764	37.296	
	Cement (kg/m ³)	.105	.004	.660	24.821	<.001	.097	.114	.534
a. Dependent Variable: Compressive Strength (MPa)									

Table 3: Collinearity statistics (Tolerance and VIF) for predictors in Model 6.

2.4.4 Model Summary Interpretation (Q1d-iv)

Table 4 presents the regression progress across six steps. The final model (Model 6) achieved $R = 0.784$ and an adjusted $R^2 = 0.612$, indicating that the predictors collectively explain 61.2% of the variance in compressive strength. The standard error of the estimate decreased from 14.50 in the initial model to 10.41 in the final model, confirming the improved predictive accuracy as relevant predictors were retained.

These findings address Objective 3, showing that material composition and curing age jointly account for a substantial proportion of variability in compressive strength. The null hypothesis (H_0) that these predictors do not explain compressive strength is therefore rejected.

Model Summary ^g				
Model	R	R Square	Adjusted R Square	Std. Error of the Estimate
1	.498 ^a	.248	.247	14.495430857581399
2	.593 ^b	.351	.350	13.470522571358584
3	.694 ^c	.482	.480	12.045095584476996
4	.742 ^d	.551	.549	11.216192566254911
5	.764 ^e	.584	.582	10.798373985681206
6	.784 ^f	.614	.612	10.409784091934055

a. Predictors: (Constant), Cement (kg/m³)

b. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³)

c. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days)

d. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³)

e. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³)

f. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³), Fly Ash (kg/m³)

g. Dependent Variable: Compressive Strength (MPa)

Table 4: Model summary of stepwise regression predicting compressive strength.

2.4.5 Model Adequacy and Significance (Q1d-v)

Hypotheses:

- H_0 : The model does not fit the data.
- H_1 : The model fits the data.

ANOVA ^a						
Model		Sum of Squares	df	Mean Square	F	Sig.
1	Regression	71172.222	1	71172.222	338.726	<.001 ^b
	Residual	216000.806	1028	210.118		
	Total	287173.028	1029			
2	Regression	100818.766	2	50409.383	277.807	<.001 ^c
	Residual	186354.263	1027	181.455		
	Total	287173.028	1029			
3	Regression	138316.508	3	46105.503	317.784	<.001 ^d
	Residual	148856.520	1026	145.084		
	Total	287173.028	1029			
4	Regression	158224.978	4	39556.245	314.430	<.001 ^e
	Residual	128948.050	1025	125.803		
	Total	287173.028	1029			
5	Regression	167769.631	5	33553.926	287.757	<.001 ^f
	Residual	119403.398	1024	116.605		
	Total	287173.028	1029			
6	Regression	176317.061	6	29386.177	271.181	<.001 ^g
	Residual	110855.968	1023	108.364		
	Total	287173.028	1029			

a. Dependent Variable: Compressive Strength (MPa)

b. Predictors: (Constant), Cement (kg/m³)

c. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³)

d. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days)

e. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³)

f. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³)

g. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³), Fly Ash (kg/m³)

Table 5: ANOVA results for stepwise regression predicting compressive strength (Model 6).

The ANOVA results in Table 5 confirm the statistical adequacy of the regression model. For the final stepwise model (Model 6), the regression sum of squares was 176,317.061 compared to a residual sum of 110,855.968, yielding an F-statistic of 271.181 with $df = (6, 1023)$ and a significance level of $p < 0.001$.

This leads to the rejection of H_0 as there is sufficient evidence to conclude that the independent variables (cement, blast furnace slag, fly ash, water, superplasticizer, and curing age) collectively provide a significant prediction of compressive strength. Therefore, the regression model is a good fit for the data and directly addresses Objective 3.

2.4.6 Coefficient Interpretation (Q1d-vi)

Model	Coefficients ^a							Collinearity Statistics		
	B	Unstandardized Coefficients Std. Error	Standardized Coefficients Beta	t	Sig.	95.0% Confidence Interval for B Lower Bound	Upper Bound	Tolerance	VIF	
1	(Constant)	13.443	1.297		10.365	<.001	10.898	15.988		
	Cement (kg/m ³)	.080	.004	.498	18.405	<.001	.071	.088	1.000	1.000
2	(Constant)	9.190	1.250		7.350	<.001	6.737	11.644		
	Cement (kg/m ³)	.075	.004	.468	18.534	<.001	.067	.083	.991	1.009
3	Superplasticizer (kg/m ³)	.902	.071	.323	12.782	<.001	.764	1.041	.991	1.009
	(Constant)	5.098	1.147		4.446	<.001	2.848	7.348		
4	Cement (kg/m ³)	.069	.004	.431	18.976	<.001	.062	.076	.981	1.019
	Superplasticizer (kg/m ³)	1.112	.064	.397	17.246	<.001	.985	1.238	.951	1.052
5	Curing Age (days)	.098	.006	.370	16.077	<.001	.086	.110	.953	1.050
	(Constant)	-2.001	1.208		-1.657	.098	-4.371	.369		
6	Cement (kg/m ³)	.081	.004	.508	23.076	<.001	.074	.088	.905	1.105
	Superplasticizer (kg/m ³)	1.060	.060	.379	17.610	<.001	.941	1.178	.946	1.057
7	Curing Age (days)	.098	.006	.372	17.368	<.001	.087	.110	.953	1.050
	Blast Furnace Slag (kg/m ³)	.053	.004	.275	12.580	<.001	.045	.061	.919	1.088
8	Water (kg/m ³)	.197	.022	-.252	-9.047	<.001	-.240	-.154	.524	1.908
	(Constant)	35.492	4.304		8.246	<.001	27.046	43.938		
9	Cement (kg/m ³)	.081	.003	.509	24.007	<.001	.075	.088	.905	1.105
	Superplasticizer (kg/m ³)	.614	.076	.219	8.073	<.001	.465	.763	.549	1.821
10	Curing Age (days)	.109	.006	.413	19.554	<.001	.098	.120	.910	1.100
	Blast Furnace Slag (kg/m ³)	.060	.004	.311	14.520	<.001	.052	.068	.888	1.127
11	Water (kg/m ³)	-.197	.022	-.252	-9.047	<.001	-.240	-.154	.524	1.908
	Fly Ash (kg/m ³)	.069	.008	.263	8.881	<.001	.054	.084	.430	2.328

a. Dependent Variable: Compressive Strength (MPa)

Table 6: Coefficients table for stepwise regression predicting compressive strength (Model 6).

Objective 1: Effect of material composition

The final model (Model 6) in Figure 6 shows that cement ($\beta = 0.660$, $p < .001$) was the strongest predictor of compressive strength. Blast furnace slag ($\beta = 0.447$, $p < .001$) and fly ash ($\beta = 0.263$, $p < .001$) also improved strength, reflecting their pozzolanic contribution. Superplasticizer had a smaller positive effect ($p = .005$), while water reduced strength ($\beta = -0.279$, $p < .001$), in line with the weakening effect of excess water on concrete. Aggregates were excluded, indicating their limited influence once other variables were controlled.

These results are consistent with the existing literature. Sah and Hong (2024). found that cement content had the strongest positive association with compressive strength, while water content showed a clear negative effect, confirming the dominant role of the water–cement ratio. Similarly, Nikoopayan Tak et al. (2025) reported that additional cementitious materials such as blast furnace slag and fly ash improve strength through pozzolanic reactions, with superplasticizer providing additional but smaller gains.

Objective 2: Effect of curing age

Curing age ($\beta = 0.429$, $p < .001$) was also significant, confirming that strength increases with time through continued hydration. This supports the hypothesis and agrees with Nikoopayan Tak et al. (2025), who observed consistent strength gains with longer curing periods and noted that their results were in line with prior investigations reporting similar trends.

2.5 Conclusions and Recommendations (Q1e)

Conclusions

This study demonstrated that multiple linear regression can effectively predict concrete compressive strength from mixture proportions and curing age. The final stepwise model explained 61.2% of the variance, identifying cement content, curing age, slag, and fly ash as significant positive predictors, while water had a negative effect. These results confirmed the hypotheses for all the objectives and were consistent with previous research emphasising the central role of cementitious materials and curing conditions in determining compressive strength .

Recommendations

The findings highlight practical approaches for companies in the industry. Strength can be increased by optimising cement and water ratios, extending curing periods, and incorporating supplementary cementitious materials such as slag and fly ash. Water content should be carefully controlled, while superplasticizers can be used to improve workability without compromising performance.

Limitations & Future Research

It is important to note this study relied on a secondary dataset generated under laboratory conditions, which may not capture field variability such as temperature, humidity, or long-term durability. Furthermore, stepwise regression may exclude variables that could be relevant in other contexts, and the linear approach may not fully account for complex material interactions. Future research could expand on this work by using larger and more diverse datasets, including field measurements. Exploring non-linear and machine learning methodologies, as well as integrating durability and sustainability indicators, could provide a more comprehensive understanding of concrete performance.

3 Question 2: Factor Analysis

3.1 Purpose of Factor Analysis (Q2a)

Factor analysis is a multivariate technique used to identify underlying structures among a set of observed variables. It is particularly appropriate for this study because the dataset contains several correlated predictors (cement, slag, fly ash, water, superplasticizer, coarse aggregate, fine aggregate, and curing age), which may reflect latent dimensions such as “cementitious materials,” “aggregates,” or “curing conditions.”

The main purpose of factor analysis is to reduce dimensionality while retaining most of the variance present in the original dataset. By grouping correlated variables into common factors, factor analysis simplifies complex datasets, reduces redundancy, and allows a clearer interpretation of how different inputs relate to one another. In this context, it helps to uncover the main components that influence compressive strength, ensuring that subsequent analyses focus on interpretable components rather than individual overlapping predictors.

3.2 Handling of Non-Metric Variables (Q2b)

All independent variables used in this study are metric and measured on a scale level, which makes them appropriate for factor analysis. Consequently, no non-metric variables were included in the procedure.

Factor analysis assumes continuous, normally distributed data and linear relationships among variables. Non-metric variables, such as categorical or nominal data, are generally excluded because they violate these assumptions and can distort the factor structure. If such variables were present, they would either be removed or transformed into dummy or ordinal-coded variables to allow for an appropriate analysis.

3.3 Execution of Factor Analysis (Q2c)

To evaluate the structure of the independent variables, factor analysis was conducted using Principal Component Analysis (PCA) with Varimax rotation. The following SPSS outputs provide evidence of sampling adequacy, factor extraction, and loadings.

Communalities

	Initial	Extraction
Cement (kg/m ³)	1.000	.932
Blast Furnace Slag (kg/m ³)	1.000	.911
Fly Ash (kg/m ³)	1.000	.659
Water (kg/m ³)	1.000	.836
Superplasticizer (kg/m ³)	1.000	.772
Coarse Aggregate (kg/m ³)	1.000	.908
Fine Aggregate (kg/m ³)	1.000	.520
Curing Age (days)	1.000	.512

Extraction Method: Principal Component Analysis.

Table 8: Communalities table showing proportion of variance explained for each variable.

Component	Total Variance Explained									
	Total	Initial Eigenvalues			Extraction Sums of Squared Loadings			Rotation Sums of Squared Loadings		
		% of Variance	Cumulative %	Total	% of Variance	Cumulative %	Total	% of Variance	Cumulative %	
1	2.280	28.499	28.499	2.280	28.499	28.499	1.951	24.385	24.385	
2	1.416	17.703	46.201	1.416	17.703	46.201	1.429	17.860	42.246	
3	1.340	16.753	62.954	1.340	16.753	62.954	1.415	17.692	59.938	
4	1.014	12.677	75.631	1.014	12.677	75.631	1.255	15.694	75.631	
5	.952	11.895	87.526							
6	.790	9.877	97.403							
7	.178	2.222	99.624							
8	.030	.376	100.000							

Extraction Method: Principal Component Analysis.

Table 9: Total variance explained by extracted components.

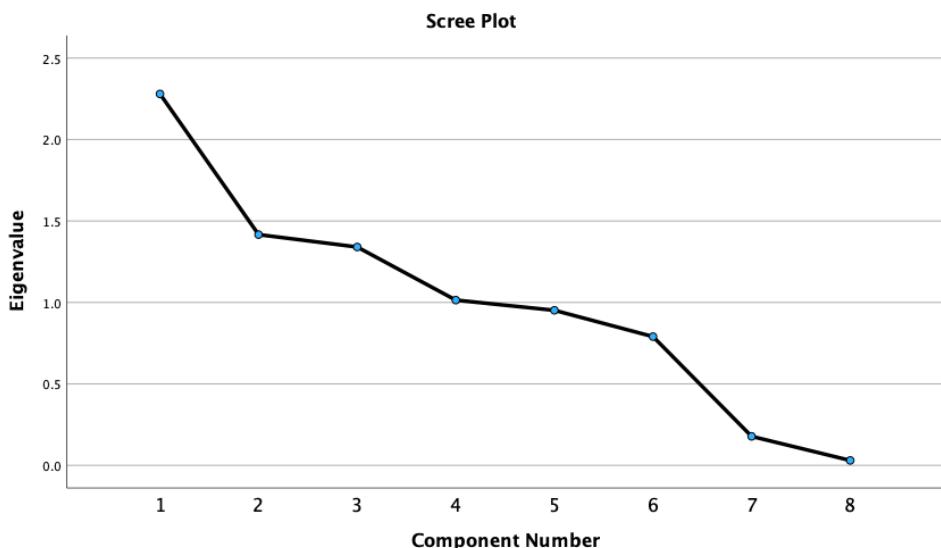


Figure 5: Scree plot of eigenvalues indicating factor retention.

Component Matrix^a

	Component			
	1	2	3	4
Cement (kg/m ³)	-.148	.133	.943	-.055
Blast Furnace Slag (kg/m ³)	-.268	-.816	-.201	-.365
Fly Ash (kg/m ³)	.596	.171	-.472	.228
Water (kg/m ³)	-.826	-.063	-.247	.298
Superplasticizer (kg/m ³)	.764	-.338	.271	-.038
Coarse Aggregate (kg/m ³)	-.057	.750	-.200	-.550
Fine Aggregate (kg/m ³)	.607	.023	-.006	.388
Curing Age (days)	-.440	.150	.117	.532

Extraction Method: Principal Component Analysis.

a. 4 components extracted.

Table 10: Unrotated component matrix.

Rotated Component Matrix^a

	Component			
	1	2	3	4
Cement (kg/m ³)	.084	.940	.196	.047
Blast Furnace Slag (kg/m ³)	.033	-.076	-.922	.234
Fly Ash (kg/m ³)	.269	-.662	.384	.032
Water (kg/m ³)	-.878	-.016	-.247	.056
Superplasticizer (kg/m ³)	.781	-.003	.094	.391
Coarse Aggregate (kg/m ³)	.028	-.041	.152	-.939
Fine Aggregate (kg/m ³)	.334	-.265	.470	.342
Curing Age (days)	-.613	.168	.257	.205

Extraction Method: Principal Component Analysis.

Rotation Method: Varimax with Kaiser Normalization.

a. Rotation converged in 7 iterations.

Table 11: Rotated component matrix (Varimax rotation).

Component Transformation Matrix

Component	1	2	3	4
1	.848	-.322	.389	.163
2	-.170	.045	.700	-.693
3	.220	.930	.203	.212
4	-.452	-.169	.564	.670

Extraction Method: Principal Component Analysis.

Rotation Method: Varimax with Kaiser Normalization.

Table 12: Component transformation matrix.

3.4 Grouping of Variables (Q2d)

Rotated Component Matrix^a

	Component			
	1	2	3	4
Cement (kg/m ³)	.084	.940	.196	.047
Blast Furnace Slag (kg/m ³)	.033	-.076	-.922	.234
Fly Ash (kg/m ³)	.269	-.662	.384	.032
Water (kg/m ³)	-.878	-.016	-.247	.056
Superplasticizer (kg/m ³)	.781	-.003	.094	.391
Coarse Aggregate (kg/m ³)	.028	-.041	.152	-.939
Fine Aggregate (kg/m ³)	.334	-.265	.470	.342
Curing Age (days)	-.613	.168	.257	.205

Extraction Method: Principal Component Analysis.

Rotation Method: Varimax with Kaiser Normalization.

a. Rotation converged in 7 iterations.

Table 13: Rotated component matrix (Grouping).

■ Factor 1: Water–Admixture–Curing

Water (-0.878), Superplasticizer (0.781), and Curing Age (-0.613) clustered together, representing the balance between mix fluidity, chemical admixtures, and time-dependent hydration.

■ Factor 2: Primary Binder

Cement (0.940) and Fly Ash (-0.662) loaded strongly, reflecting the role of cement and its partial replacement material in strength development.

■ Factor 3: Supplementary Binder and Fines

Blast Furnace Slag (-0.922) and Fine Aggregate (0.470) grouped under this factor, highlighting the contribution of secondary binders and finer particles to concrete properties.

■ Factor 4: Coarse Aggregate

Coarse Aggregate (-0.939) loaded almost exclusively on this factor, capturing the structural skeleton of the mix.

3.5 Interpretation of Outputs (Q2e)

3.5.1 Interpretation of Communalities (Q2i)

As shown in Table 8, Cement (0.932), Blast Furnace Slag (0.911), and Coarse Aggregate (0.908) had very high extraction values, meaning over 90% of their variance was explained by the extracted factors. Water (0.836) and Superplasticizer (0.772) were also well represented.

Fly Ash (0.659), Fine Aggregate (0.520), and Curing Age (0.512) had lower values, but still above the recommended 0.50 threshold. This indicates that just over half of their variance was explained by the model, which is considered adequate for retention.

According to the factor analysis rule of thumb, communalities below 0.50 suggest removing a variable. However, since all variables here exceeded 0.50, none required exclusion from the analysis.

3.5.2 Interpretation of Eigenvalues and Scree Plot (Q2ii)

The eigenvalue results (Table 9) show that the first four components had values greater than 1 (2.280, 1.416, 1.340, and 1.014), together accounting for 75.6% of the total variance. According to the Kaiser criterion, these four components should be retained, while the remaining components with eigenvalues below 1 contribute minimal explanatory power.

The scree plot (Figure 5) displays a clear decline in eigenvalues across the first four components, after which the values drop below 1 and level off. This pattern confirms that retaining four factors provides a balanced representation of the dataset. Thus, both the Kaiser rule and scree test support a four-factor solution, consistent with the rotated component matrix (Table 13).

3.6 Improving Factorability & Reliability (Q2f-i)

Reliability could be improved in two ways. First, variables with relatively low communalities, such as Fine Aggregate (.520) and Curing Age (.512), may weaken the stability of the factor solution and should be reconsidered or measured with greater precision in future analyses.

Second, although the large sample size ($N = 1030$) already exceeds the guidelines, which improves the stability of the factor solution. Research shows that variables with high communalities need fewer cases, while weaker variables require larger samples to achieve accurate results (Lorenzo-Seva & Ferrando, 2024). This confirms that the dataset is strong, though addressing weaker variables could further improve its reliability.

The robustness of factor solutions depends strongly on both sample size and variable quality. Larger samples help stabilize communalities and factor loadings, making the solution less sensitive to sampling error. At the same time, including variables with weak or inconsistent

loadings can introduce noise, reducing clarity and reliability. A balance of sufficient cases and carefully selected variables therefore provides the most stable and interpretable factor structure.

3.7 Cross-Loadings (Q2f-ii, iii)

3.7.1 Understanding and Addressing Cross-Loadings

A cross-loading occurs when a variable loads substantially on more than one factor, making it unclear which construct it belongs to. Thresholds above 0.40 on multiple components are commonly treated as problematic.

In the unrotated component matrix (Table 10), Fly Ash loaded 0.596 on Component 1 and -0.472 on Component 3, Coarse Aggregate loaded 0.750 on Component 2 and -0.550 on Component 4 and Curing age loaded -0.440 on Component 1 and 0.532 on Component 4 exceeding the 0.40 threshold, indicating cross-loadings. After Varimax rotation (Table 11), the issue was reduced but not fully eliminated. Fly Ash (-0.662 on Component 2; 0.384 on Component 3) Fine Aggregate (0.470 on Component 3; 0.334 on Component 1) and Superplasticizer (0.781 on Component 1; 0.391 on Component 4) showed moderate loadings on more than one factor. Although one loading in each case fell just below the 0.40 threshold, it is still large enough to suggest an influence from two factors.

Such cross-loadings can distort interpretation by blurring the meaning of the components. For example, Fly Ash may appear to represent both the binder dimension and the fines dimension, however this limits the ability to treat each factor as a distinct component.

3.7.2 Reducing Cross-Loadings

Cross-loadings reduces the clarity of factor analysis because a variable seems to belong to more than one factor. Two main strategies can be applied. First, variables that show consistent overlap, such as Fly Ash and Fine Aggregate in this dataset, can be reconsidered or removed, since excluding vague items improves the stability of the solution. Second, the choice of rotation method can reduce cross-loadings.

While Varimax is the most common rotation, it is sensitive to sampling error because it minimises individual cross-loadings. Beauducel and Hilger (2023) demonstrated that oblique mean target (OMT) rotation, which minimises the average cross-loadings across factors, produces more robust and interpretable results. This suggests that in future analyses, cross-loadings could be reduced either by refining weak variables or by using more flexible rotation techniques such as OMT, which may provide clearer factor solutions than Varimax.

References

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Appendices

Appendix A: SPSS Outputs for Q1 (Regression)

Appendix A: Multiple Linear Regression Outputs

This appendix presents the complete SPSS outputs for the multiple linear regression analysis. All figures are labelled for reference in Section 1(d).

Descriptive Statistics

	Mean	Std. Deviation	N
Compressive Strength (MPa)	35.81783582611362	16.705679174867940	1030
Cement (kg/m ³)	281.1656	104.50714	1030
Blast Furnace Slag (kg/m ³)	73.8955	86.27910	1030
Fly Ash (kg/m ³)	54.1871	63.99647	1030
Water (kg/m ³)	181.5664	21.35557	1030
Superplasticizer (kg/m ³)	6.20311	5.973492	1030
Coarse Aggregate (kg/m ³)	972.9186	77.75382	1030
Fine Aggregate (kg/m ³)	773.5789	80.17543	1030
Curing Age (days)	45.66	63.170	1030

Table 14: Descriptive statistics of dependent and independent variables.

Correlations									
	Compressive Strength (MPa)	Cement (kg/m³)	Blast Furnace Slag (kg/m³)	Fly Ash (kg/m³)	Water (kg/m³)	Superplasticizer (kg/m³)	Coarse Aggregate (kg/m³)	Fine Aggregate (kg/m³)	Curing Age (days)
Pearson Correlation	Compressive Strength (MPa)	1.000	.498	.135	-.106	-.290	.366	-.165	-.167
	Cement (kg/m³)	.498	1.000	-.275	-.397	-.082	.093	-.109	-.223
	Blast Furnace Slag (kg/m³)	.135	-.275	1.000	-.324	.107	.043	-.284	-.282
	Fly Ash (kg/m³)	-.106	-.397	-.324	1.000	-.257	.377	-.010	.079
	Water (kg/m³)	-.290	-.082	.107	-.257	1.000	-.657	-.182	.451
	Superplasticizer (kg/m³)	.366	.093	.043	.377	-.657	1.000	-.266	.223
	Coarse Aggregate (kg/m³)	-.165	-.109	-.284	-.010	-.182	-.266	1.000	-.179
	Fine Aggregate (kg/m³)	-.167	-.223	-.282	.079	-.451	.223	-.179	1.000
Sig. (1-tailed)	Curing Age (days)	.329	.082	-.044	-.154	.278	-.193	-.003	-.156
	Compressive Strength (MPa)	.	<.001	<.001	<.001	<.001	<.001	<.001	<.001
	Cement (kg/m³)	.000	.	.000	.000	.004	.001	.000	.000
	Blast Furnace Slag (kg/m³)	.000	.000	.	.000	.000	.082	.000	.000
	Fly Ash (kg/m³)	.000	.000	.000	.	.000	.000	.375	.006
	Water (kg/m³)	.000	.004	.000	.000	.	.000	.000	.000
	Superplasticizer (kg/m³)	.000	.001	.082	.000	.000	.	.000	.000
	Coarse Aggregate (kg/m³)	.000	.000	.000	.375	.000	.000	.	.461
N	Fine Aggregate (kg/m³)	.000	.000	.000	.006	.000	.000	.000	.
	Curing Age (days)	.000	.004	.078	.000	.000	.000	.461	.000
	Compressive Strength (MPa)	1030	1030	1030	1030	1030	1030	1030	1030
	Cement (kg/m³)	1030	1030	1030	1030	1030	1030	1030	1030
	Blast Furnace Slag (kg/m³)	1030	1030	1030	1030	1030	1030	1030	1030
	Fly Ash (kg/m³)	1030	1030	1030	1030	1030	1030	1030	1030
	Water (kg/m³)	1030	1030	1030	1030	1030	1030	1030	1030
	Superplasticizer (kg/m³)	1030	1030	1030	1030	1030	1030	1030	1030
	Coarse Aggregate (kg/m³)	1030	1030	1030	1030	1030	1030	1030	1030
	Fine Aggregate (kg/m³)	1030	1030	1030	1030	1030	1030	1030	1030
	Curing Age (days)	1030	1030	1030	1030	1030	1030	1030	1030

Table 15: Correlation matrix between compressive strength and predictors.

Variables Entered/Removed^a

Model	Variables Entered	Variables Removed	Method
1	Cement (kg/m³)	.	Stepwise (Criteria: Probability-of-F-to-enter <= .050, Probability-of-F-to-remove >= .100).
2	Superplasticizer (kg/m³)	.	Stepwise (Criteria: Probability-of-F-to-enter <= .050, Probability-of-F-to-remove >= .100).
3	Curing Age (days)	.	Stepwise (Criteria: Probability-of-F-to-enter <= .050, Probability-of-F-to-remove >= .100).
4	Blast Furnace Slag (kg/m³)	.	Stepwise (Criteria: Probability-of-F-to-enter <= .050, Probability-of-F-to-remove >= .100).
5	Water (kg/m³)	.	Stepwise (Criteria: Probability-of-F-to-enter <= .050, Probability-of-F-to-remove >= .100).
6	Fly Ash (kg/m³)	.	Stepwise (Criteria: Probability-of-F-to-enter <= .050, Probability-of-F-to-remove >= .100).

a. Dependent Variable: Compressive Strength (MPa)

Table 16: Stepwise entry and removal of predictors.

Model Summary^g

Model	R	R Square	Adjusted R Square	Std. Error of the Estimate
1	.498 ^a	.248	.247	14.495430857581399
2	.593 ^b	.351	.350	13.470522571358584
3	.694 ^c	.482	.480	12.045095584476996
4	.742 ^d	.551	.549	11.216192566254911
5	.764 ^e	.584	.582	10.798373985681206
6	.784 ^f	.614	.612	10.409784091934055

a. Predictors: (Constant), Cement (kg/m³)

b. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³)

c. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days)

d. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³)

e. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³)

f. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³), Fly Ash (kg/m³)

g. Dependent Variable: Compressive Strength (MPa)

Table 17: Model summary for stepwise regression (Models 1–6).

ANOVA^a

Model		Sum of Squares	df	Mean Square	F	Sig.
1	Regression	71172.222	1	71172.222	338.726	<.001 ^b
	Residual	216000.806	1028	210.118		
	Total	287173.028	1029			
2	Regression	100818.766	2	50409.383	277.807	<.001 ^c
	Residual	186354.263	1027	181.455		
	Total	287173.028	1029			
3	Regression	138316.508	3	46105.503	317.784	<.001 ^d
	Residual	148856.520	1026	145.084		
	Total	287173.028	1029			
4	Regression	158224.978	4	39556.245	314.430	<.001 ^e
	Residual	128948.050	1025	125.803		
	Total	287173.028	1029			
5	Regression	167769.631	5	33553.926	287.757	<.001 ^f
	Residual	119403.398	1024	116.605		
	Total	287173.028	1029			
6	Regression	176317.061	6	29386.177	271.181	<.001 ^g
	Residual	110855.968	1023	108.364		
	Total	287173.028	1029			

a. Dependent Variable: Compressive Strength (MPa)

b. Predictors: (Constant), Cement (kg/m³)c. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³)d. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days)e. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³)f. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³)g. Predictors: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³), Fly Ash (kg/m³)

Table 18: ANOVA results for stepwise regression.

Coefficients^a

Model	Unstandardized Coefficients		Standardized Coefficients Beta	t	Sig.	95.0% Confidence Interval for B		Collinearity Statistics	
	B	Std. Error				Lower Bound	Upper Bound	Tolerance	VIF
1	(Constant)	13.443	1.297	10.365	<.001	10.898	15.988		
	Cement (kg/m ³)	.080	.004	.498	18.405	<.001	.071	.088	1.000
2	(Constant)	9.190	1.250	7.350	<.001	6.737	11.644		
	Cement (kg/m ³)	.075	.004	.468	18.534	<.001	.067	.083	.991
	Superplasticizer (kg/m ³)	.902	.071	.323	12.782	<.001	.764	1.041	.991
3	(Constant)	5.098	1.147	4.446	<.001	2.848	7.348		
	Cement (kg/m ³)	.069	.004	.431	18.976	<.001	.062	.076	.981
	Superplasticizer (kg/m ³)	1.112	.064	.397	17.246	<.001	.985	1.238	.951
	Curing Age (days)	.098	.006	.370	16.077	<.001	.086	.110	.953
4	(Constant)	-2.001	1.208	-1.657	.098	-4.371	.369		
	Cement (kg/m ³)	.081	.004	.508	23.076	<.001	.074	.088	.905
	Superplasticizer (kg/m ³)	1.060	.060	.379	17.610	<.001	.941	1.178	.946
	Curing Age (days)	.098	.006	.372	17.368	<.001	.087	.110	.953
	Blast Furnace Slag (kg/m ³)	.053	.004	.275	12.580	<.001	.045	.061	.919
5	(Constant)	35.492	4.304	8.246	<.001	27.046	43.938		
	Cement (kg/m ³)	.081	.003	.509	24.007	<.001	.075	.088	.905
	Superplasticizer (kg/m ³)	.614	.076	.219	8.073	<.001	.465	.763	.549
	Curing Age (days)	.109	.006	.413	19.554	<.001	.098	.120	.910
	Blast Furnace Slag (kg/m ³)	.060	.004	.311	14.520	<.001	.052	.068	.888
	Water (kg/m ³)	-.197	.022	-.252	-9.047	<.001	-.240	-.154	.524
6	(Constant)	29.030	4.212	6.891	<.001	20.764	37.296		
	Cement (kg/m ³)	.105	.004	.660	24.821	<.001	.097	.114	.534
	Superplasticizer (kg/m ³)	.239	.085	.085	2.826	.005	.073	.405	.412
	Curing Age (days)	.113	.005	.429	20.987	<.001	.103	.124	.902
	Blast Furnace Slag (kg/m ³)	.086	.005	.447	17.386	<.001	.077	.096	.572
	Water (kg/m ³)	-.218	.021	-.279	-10.332	<.001	-.260	-.177	.517
	Fly Ash (kg/m ³)	.069	.008	.263	8.881	<.001	.054	.084	.430

a. Dependent Variable: Compressive Strength (MPa)

Table 19: Coefficients table for regression models (final model highlighted).

Excluded Variables^a

Model		Beta In	t	Sig.	Partial Correlation	Collinearity Statistics		Minimum Tolerance
						Tolerance	VIF	
1	Blast Furnace Slag (kg/m ³)	.294 ^b	11.051	<.001	.326	.924	1.082	.924
	Fly Ash (kg/m ³)	.109 ^b	3.735	<.001	.116	.842	1.188	.842
	Water (kg/m ³)	-.251 ^b	-9.641	<.001	-.288	.993	1.007	.993
	Superplasticizer (kg/m ³)	.323 ^b	12.782	<.001	.370	.991	1.009	.991
	Coarse Aggregate (kg/m ³)	-.112 ^b	-4.141	<.001	-.128	.988	1.012	.988
	Fine Aggregate (kg/m ³)	-.059 ^b	-2.141	.032	-.067	.950	1.052	.950
	Curing Age (days)	.290 ^b	11.329	<.001	.333	.993	1.007	.993
2	Blast Furnace Slag (kg/m ³)	.271 ^c	10.936	<.001	.323	.919	1.088	.913
	Fly Ash (kg/m ³)	-.062 ^c	-2.024	.043	-.063	.669	1.495	.669
	Water (kg/m ³)	-.069 ^c	-2.079	.038	-.065	.567	1.763	.566
	Coarse Aggregate (kg/m ³)	-.030 ^c	-1.153	.249	-.036	.922	1.085	.922
	Fine Aggregate (kg/m ³)	-.151 ^c	-5.772	<.001	-.177	.891	1.123	.891
	Curing Age (days)	.370 ^c	16.077	<.001	.449	.953	1.050	.951
3	Blast Furnace Slag (kg/m ³)	.275 ^d	12.580	<.001	.366	.919	1.088	.905
	Fly Ash (kg/m ³)	-.041 ^d	-1.495	.135	-.047	.667	1.498	.667
	Water (kg/m ³)	-.177 ^d	-5.887	<.001	-.181	.543	1.842	.543
	Coarse Aggregate (kg/m ³)	-.012 ^d	-.504	.614	-.016	.920	1.087	.885
	Fine Aggregate (kg/m ³)	-.116 ^d	-4.884	<.001	-.151	.882	1.133	.882
4	Fly Ash (kg/m ³)	.228 ^e	7.387	<.001	.225	.435	2.298	.435
	Water (kg/m ³)	-.252 ^e	-9.047	<.001	-.272	.524	1.908	.524
	Coarse Aggregate (kg/m ³)	.086 ^e	3.739	<.001	.116	.824	1.214	.824
	Fine Aggregate (kg/m ³)	-.004 ^e	-.167	.868	-.005	.741	1.349	.741
5	Fly Ash (kg/m ³)	.263 ^f	8.881	<.001	.268	.430	2.328	.412
	Coarse Aggregate (kg/m ³)	-.011 ^f	-.455	.650	-.014	.641	1.560	.408
	Fine Aggregate (kg/m ³)	-.103 ^f	-4.062	<.001	-.126	.628	1.593	.444
6	Coarse Aggregate (kg/m ³)	.016 ^g	.639	.523	.020	.631	1.584	.346
	Fine Aggregate (kg/m ³)	.015 ^g	.521	.603	.016	.457	2.186	.313

a. Dependent Variable: Compressive Strength (MPa)

b. Predictors in the Model: (Constant), Cement (kg/m³)

c. Predictors in the Model: (Constant), Cement (kg/m³), Superplasticizer (kg/m³)

d. Predictors in the Model: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days)

e. Predictors in the Model: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³)

f. Predictors in the Model: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³)

g. Predictors in the Model: (Constant), Cement (kg/m³), Superplasticizer (kg/m³), Curing Age (days), Blast Furnace Slag (kg/m³), Water (kg/m³), Fly Ash (kg/m³)

Table 20: Variables excluded during stepwise regression.

Model	Dimension	Eigenvalue	Condition Index	Variance Proportions				
				(Constant)	Cement (kg/m³)	Superplasticizer (kg/m³)	Curing Age (days)	Blast Furnace Slag (kg/m³)
1	1	1.937	1.000	.03	.03			
	2	.063	5.563	.97	.97			
2	1	2.576	1.000	.02	.02	.05		
	2	.362	2.666	.04	.06	.93		
3	1	2.956	1.000	.01	.01	.03	.03	
	2	.698	2.058	.00	.00	.21	.61	
3	2	.285	3.221	.06	.10	.74	.35	
	3	.062	6.448	.95	.93	.01		
4	1	3.401	1.000	.01	.01	.02	.02	.02
	2	.717	2.178	.00	.00	.13	.62	.07
4	3	.549	2.489	.00	.01	.16	.01	.70
	4	.282	3.474	.04	.11	.68	.34	.02
5	1	4.325	1.000	.00	.01	.01	.01	.00
	2	.718	2.454	.00	.00	.08	.57	.07
5	3	.549	2.806	.00	.01	.09	.01	.68
	4	.332	3.611	.00	.04	.36	.37	.03
5	5	.073	7.717	.01	.93	.01	.00	.19
	6	.003	36.016	.98	.01	.45	.03	.01
6	1	4.736	1.000	.00	.00	.01	.01	.00
	2	.880	2.320	.00	.00	.04	.18	.05
6	3	.682	2.636	.00	.00	.01	.35	.24
	4	.361	3.622	.00	.06	.01	.31	.19
6	5	.305	3.941	.00	.00	.40	.11	.02
	6	.033	11.995	.03	.92	.23	.01	.49
6	7	.003	37.721	.97	.01	.31	.03	.00

a. Dependent Variable: Compressive Strength (MPa)

Table 21: Collinearity diagnostics for stepwise regression.

Residuals Statistics ^a					
	Minimum	Maximum	Mean	Std. Deviation	N
Predicted Value	10.41257762908936	78.84008026123047	35.81783582611354	13.089995019369182	1030
Residual	-29.013690948486328	34.726081848144530	.0000000000000004	10.379390496869727	1030
Std. Predicted Value	-1.941	3.287	.000	1.000	1030
Std. Residual	-2.787	3.336	.000	.997	1030

a. Dependent Variable: Compressive Strength (MPa)

Figure 6: Residuals statistics for the regression model.

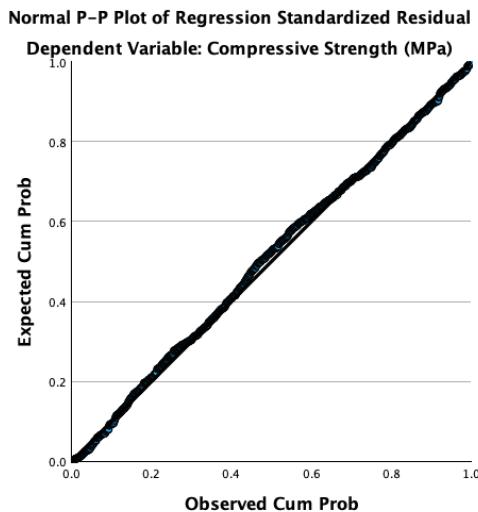


Figure 7: Normal P-P plot of regression standardized residuals.

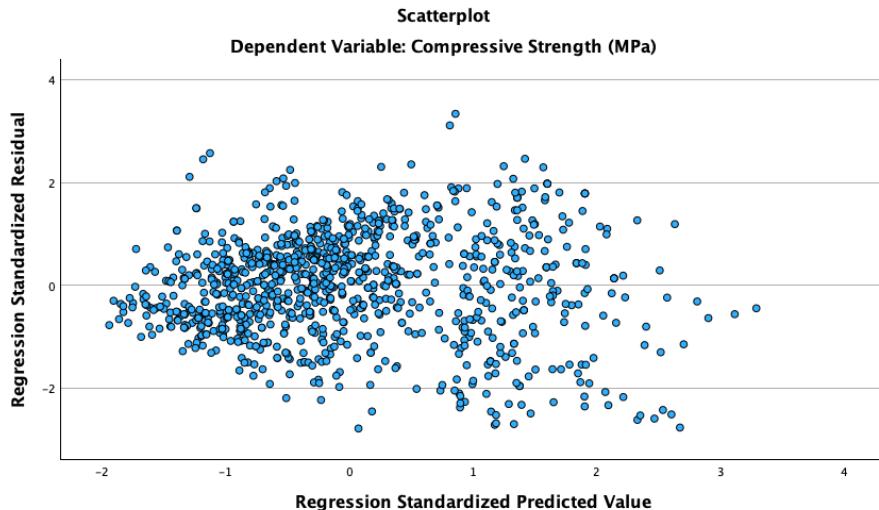


Figure 8: Scatterplot of standardized residuals versus predicted values.

Case Processing Summary

	Cases				Total N	Percent
	N	Valid Percent	N	Missing Percent		
Unstandardized Residual	1030	100.0%	0	0.0%	1030	100.0%

Table 22: Case processing summary for residuals.

Descriptives

Unstandardized Residual		Descriptives		Statistic	Std. Error
		Mean	95% Confidence Interval for Mean		
	Mean	.0000000	.32340985		
	95% Confidence Interval for Mean				
	Lower Bound	-.6346181			
	Upper Bound	.6346181			
	5% Trimmed Mean	.1328851			
	Median	.6501919			
	Variance	107.732			
	Std. Deviation	10.37939050			
	Minimum	-29.01369			
	Maximum	34.72608			
	Range	63.73977			
	Interquartile Range	13.03322			
	Skewness			-.174	.076
	Kurtosis			.025	.152

Table 23: Descriptive statistics of unstandardized residuals.

Tests of Normality						
	Kolmogorov-Smirnov ^a			Shapiro-Wilk		
	Statistic	df	Sig.	Statistic	df	Sig.
Unstandardized Residual	.031	1030	.025	.996	1030	.007

a. Lilliefors Significance Correction

Table 24: Kolmogorov–Smirnov and Shapiro–Wilk tests of normality.

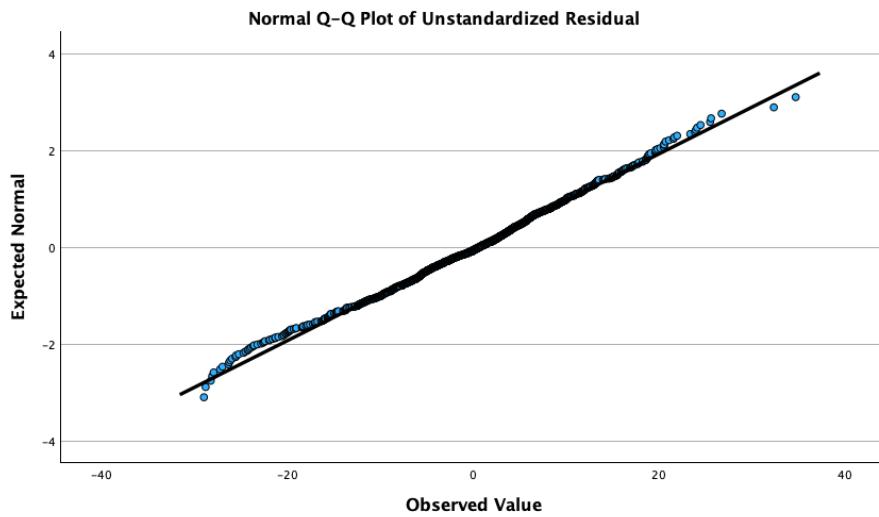


Figure 9: Normal Q–Q plot of residuals.

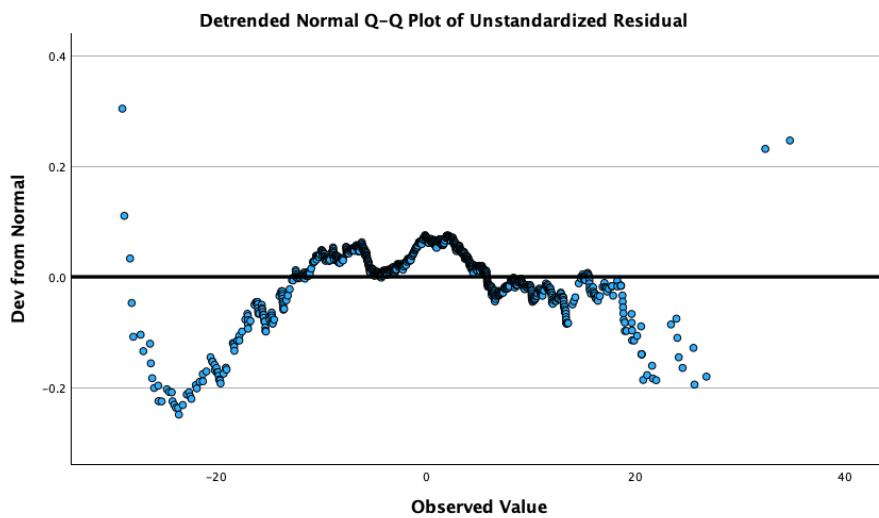


Figure 10: Detrended Q–Q plot of residuals.

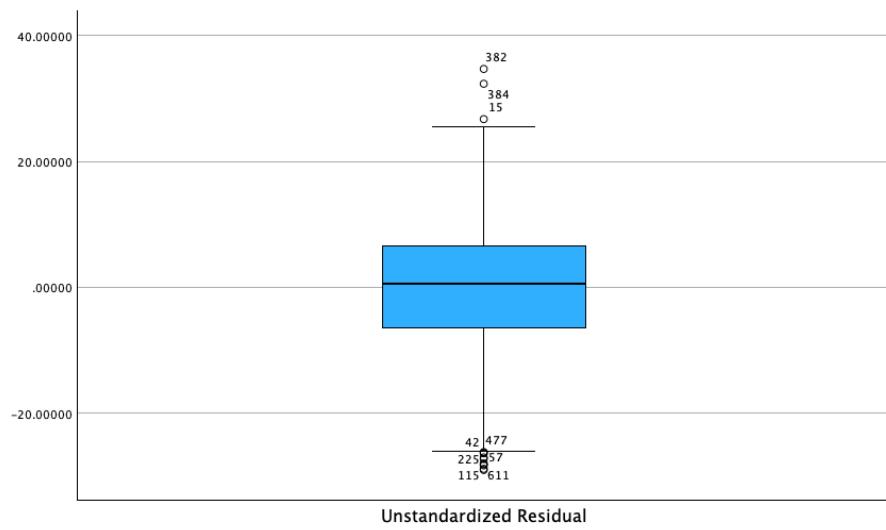


Figure 11: Boxplot of unstandardized residuals.