

Nonlinear Legendre Spectral Finite Elements for Wind Turbine Blade Dynamics



Qi Wang¹, Nick Johnson²
Michael A. Sprague¹, Jason Jonkman¹

¹National Renewable Energy Laboratory

²Colorado School of Mines

AIAA SciTech 2015
Kissimmee, FL
5–9 January 2015

Motivation

- ▶ Beam model currently used in FAST
 - Euler-Bernoulli beam model with shortening effect
 - Two degree-of-freedom
 - Assumed-mode method
- ▶ Beam models used in other wind turbine tools
 - Multibody-formulation
 - Linear beam models
 - Constraints introduced between linear beams to describe large deflections and rotations
 - Finite element method

Objective

- ▶ Objective: create efficient high-fidelity beam models for wind turbine blade analysis that can
 - Capture geometrical nonlinearity systematically
 - Capture anisotropic and heterogeneous behavior of composite materials rigorously
 - Modeling moving beams (translation and rotation)
 - Achieve the speed of computational design without significant loss of accuracy comparing to the ultimate accuracy obtained by 3D nonlinear FEA
 - Compatible with the FAST modularization framework



NWTC static and dynamic blade tests showing typical large, elastic deflections.

Approach

- ▶ Implementation
 - Geometrically Exact Beam Theory (GEBT)

Approach

- ▶ Implementation
 - Geometrically Exact Beam Theory (GEBT)
 - ▶ First proposed in 1973 (Reissner, 1973)

Approach

- ▶ Implementation
 - Geometrically Exact Beam Theory (GEBT)
 - ▶ First proposed in 1973 (Reissner, 1973)
 - ▶ Extended to composite beams (Hodges, 2006)

Approach

- ▶ Implementation
 - Geometrically Exact Beam Theory (GEBT)
 - ▶ First proposed in 1973 (Reissner, 1973)
 - ▶ Extended to composite beams (Hodges, 2006)
 - ▶ Displacement-based implementation (?)

Approach

► Implementation

- Geometrically Exact Beam Theory (GEBT)
 - First proposed in 1973 (Reissner, 1973)
 - Extended to composite beams (Hodges, 2006)
 - Displacement-based implementation (?)
 - Mixed implementation (Yu and Blair, 2012)
- Legendre Spectral Finite Element (LSFE) (?)

► Implementation

- Geometrically Exact Beam Theory (GEBT)
 - First proposed in 1973 (Reissner, 1973)
 - Extended to composite beams (Hodges, 2006)
 - Displacement-based implementation (?)
 - Mixed implementation (Yu and Blair, 2012)
- Legendre Spectral Finite Element (LSFE) (?)
 - A p -version high-order finite element

► Implementation

- Geometrically Exact Beam Theory (GEBT)
 - First proposed in 1973 (Reissner, 1973)
 - Extended to composite beams (Hodges, 2006)
 - Displacement-based implementation (?)
 - Mixed implementation (Yu and Blair, 2012)
- Legendre Spectral Finite Element (LSFE) (?)
 - A p -version high-order finite element
 - Successfully applied to simulation of fluid dynamics, geophysics, elastodynamics

Approach

- ▶ Implementation
 - Geometrically Exact Beam Theory (GEBT)
 - ▶ First proposed in 1973 (Reissner, 1973)
 - ▶ Extended to composite beams (Hodges, 2006)
 - ▶ Displacement-based implementation (?)
 - ▶ Mixed implementation (Yu and Blair, 2012)
 - Legendre Spectral Finite Element (LSFE) (?)
 - ▶ A p -version high-order finite element
 - ▶ Successfully applied to simulation of fluid dynamics, geophysics, elastodynamics
 - ▶ Limited usage in structural dynamics
 - FAST Modularization Framework (Jonkman, 2013)
 - ▶ State-space formulation for tight-coupling scheme
 - ▶ Time integrator for first-order PDEs
- ▶ Result: BeamDyn, which can be used as a structural module of FAST

► Governing Equation

$$\begin{aligned}\dot{\underline{h}} - \underline{F}' &= \underline{f} \\ \underline{\dot{g}} + \tilde{u}\underline{h} - \underline{M}' - (\tilde{x}'_0 + \tilde{u}')\underline{F} &= \underline{m}\end{aligned}$$

► Constitutive Equation

$$\begin{aligned}\begin{Bmatrix} \underline{h} \\ \underline{g} \end{Bmatrix} &= \underline{\underline{\mathcal{M}}} \begin{Bmatrix} \underline{\dot{u}} \\ \underline{\omega} \end{Bmatrix} \\ \begin{Bmatrix} \underline{F} \\ \underline{M} \end{Bmatrix} &= \underline{\underline{\mathcal{C}}} \begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix}\end{aligned}$$

► Governing Equation

$$\begin{aligned}\underline{\dot{h}} - \underline{F}' &= \underline{f} \\ \underline{\dot{g}} + \underline{\ddot{u}}\underline{h} - \underline{M}' - (\underline{\tilde{x}}'_0 + \underline{\tilde{u}}')\underline{F} &= \underline{m}\end{aligned}$$

► Constitutive Equation

$$\begin{aligned}\begin{Bmatrix} \underline{h} \\ \underline{g} \end{Bmatrix} &= \underline{\underline{\mathcal{M}}} \begin{Bmatrix} \underline{\dot{u}} \\ \underline{\omega} \end{Bmatrix} \\ \begin{Bmatrix} \underline{F} \\ \underline{M} \end{Bmatrix} &= \underline{\underline{\mathcal{C}}} \begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix}\end{aligned}$$

- \mathcal{M} and \mathcal{C} are 6×6 sectional mass and stiffness matrices, respectively

► Governing Equation

$$\begin{aligned}\dot{\underline{h}} - \underline{F}' &= \underline{f} \\ \underline{\dot{g}} + \ddot{\underline{u}}\underline{h} - \underline{M}' - (\tilde{x}'_0 + \tilde{u}')\underline{F} &= \underline{m}\end{aligned}$$

► Constitutive Equation

$$\begin{aligned}\begin{Bmatrix} \underline{h} \\ \underline{g} \end{Bmatrix} &= \underline{\underline{\mathcal{M}}} \begin{Bmatrix} \underline{\dot{u}} \\ \underline{\omega} \end{Bmatrix} \\ \begin{Bmatrix} \underline{F} \\ \underline{M} \end{Bmatrix} &= \underline{\underline{\mathcal{C}}} \begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix}\end{aligned}$$

- \mathcal{M} and \mathcal{C} are 6×6 sectional mass and stiffness matrices, respectively
- Elastic couplings are captured

► Governing Equation

$$\begin{aligned}\dot{\underline{h}} - \underline{F}' &= \underline{f} \\ \underline{\dot{g}} + \ddot{\underline{u}}\underline{h} - \underline{M}' - (\tilde{\underline{x}}'_0 + \tilde{\underline{u}}')\underline{F} &= \underline{m}\end{aligned}$$

► Constitutive Equation

$$\begin{aligned}\begin{Bmatrix} \underline{h} \\ \underline{g} \end{Bmatrix} &= \underline{\underline{\mathcal{M}}} \begin{Bmatrix} \underline{\dot{u}} \\ \underline{\omega} \end{Bmatrix} \\ \begin{Bmatrix} \underline{F} \\ \underline{M} \end{Bmatrix} &= \underline{\underline{\mathcal{C}}} \begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix}\end{aligned}$$

- \mathcal{M} and \mathcal{C} are 6×6 sectional mass and stiffness matrices, respectively
- Elastic couplings are captured
- Timoshenko-like beam model

► Strain Measures

$$\begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix} = \begin{Bmatrix} \underline{x}'_0 + \underline{u}' - (\underline{\underline{R}} \underline{\underline{R}}_0) \bar{l}_1 \\ \underline{k} \end{Bmatrix}$$

► Governing Equation

$$\begin{aligned}\dot{\underline{h}} - \underline{F}' &= \underline{f} \\ \underline{\dot{g}} + \ddot{\underline{u}}\underline{h} - \underline{M}' - (\tilde{\underline{x}}'_0 + \tilde{\underline{u}}')\underline{F} &= \underline{m}\end{aligned}$$

► Constitutive Equation

$$\begin{aligned}\begin{Bmatrix} \underline{h} \\ \underline{g} \end{Bmatrix} &= \underline{\underline{\mathcal{M}}} \begin{Bmatrix} \underline{\dot{u}} \\ \underline{\omega} \end{Bmatrix} \\ \begin{Bmatrix} \underline{F} \\ \underline{M} \end{Bmatrix} &= \underline{\underline{\mathcal{C}}} \begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix}\end{aligned}$$

► Strain Measures

$$\begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix} = \begin{Bmatrix} \underline{x}'_0 + \underline{u}' - (\underline{\underline{R}} \underline{\underline{R}}_0) \bar{l}_1 \\ \underline{k} \end{Bmatrix}$$

- \mathcal{M} and \mathcal{C} are 6×6 sectional mass and stiffness matrices, respctively
- Elastic couplings are captured
- Timoshenko-like beam model
- Geometrically exact: deformed beam geometry is represented exactly

► Governing Equation

$$\begin{aligned} \dot{\underline{h}} - \underline{F}' &= \underline{f} \\ \underline{\dot{g}} + \ddot{\underline{u}}\underline{h} - \underline{M}' - (\tilde{\underline{x}}'_0 + \tilde{\underline{u}}')\underline{F} &= \underline{m} \end{aligned}$$

► Constitutive Equation

$$\begin{aligned} \begin{Bmatrix} \underline{h} \\ \underline{g} \end{Bmatrix} &= \underline{\underline{\mathcal{M}}} \begin{Bmatrix} \underline{\dot{u}} \\ \underline{\omega} \end{Bmatrix} \\ \begin{Bmatrix} \underline{F} \\ \underline{M} \end{Bmatrix} &= \underline{\underline{\mathcal{C}}} \begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix} \end{aligned}$$

► Strain Measures

$$\begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix} = \begin{Bmatrix} \underline{x}'_0 + \underline{u}' - (\underline{\underline{R}} \underline{\underline{R}}_0) \bar{l}_1 \\ \underline{k} \end{Bmatrix}$$

- \mathcal{M} and \mathcal{C} are 6×6 sectional mass and stiffness matrices, respectively
- Elastic couplings are captured
- Timoshenko-like beam model
- Geometrically exact: deformed beam geometry is represented exactly
- Small strains

State-space Formulation

- ▶ Governing Equation

$$\underline{\underline{M}} \underline{\underline{a}} + \underline{\underline{f}}(\underline{\underline{q}}, \underline{\underline{v}}, t) = 0$$

- ▶ State-Space Form

$$\underline{\underline{q}}^T = \begin{bmatrix} \underline{\underline{u}}^T & \underline{\underline{p}}^T \end{bmatrix}$$

$$\underline{\underline{v}}^T = \begin{bmatrix} \dot{\underline{\underline{u}}}^T & \underline{\underline{\omega}}^T \end{bmatrix}$$

$$\underline{\underline{a}}^T = \begin{bmatrix} \ddot{\underline{\underline{u}}}^T & \dot{\underline{\underline{\omega}}}^T \end{bmatrix}$$

$$\underline{\underline{A}} \dot{\underline{\underline{x}}}(t) = \underline{\underline{f}}(\underline{\underline{x}}(t), t)$$

$$\underline{\underline{x}}(t) \equiv \begin{Bmatrix} \underline{\underline{q}}(t) \\ \underline{\underline{v}}(t) \end{Bmatrix}$$

$$\underline{\underline{A}}(\underline{\underline{x}}(t)) = \begin{bmatrix} \underline{\underline{D}} & \underline{\underline{0}} \\ \underline{\underline{0}} & \underline{\underline{M}} \end{bmatrix}$$

$$\underline{\underline{D}}(\underline{\underline{x}}(t)) = \int_0^l \underline{\underline{N}}^T \begin{bmatrix} \underline{\underline{I}} \\ \underline{\underline{0}} \\ \underline{\underline{0}} \\ \underline{\underline{H}} \end{bmatrix} \underline{\underline{N}} dx_1$$

$$\underline{\underline{f}}(\underline{\underline{x}}(t), t) = \begin{Bmatrix} \int_0^l \underline{\underline{N}}^T \underline{\underline{v}} dx_1 \\ \underline{\underline{F}}(\underline{\underline{x}}(t), t) \end{Bmatrix}$$

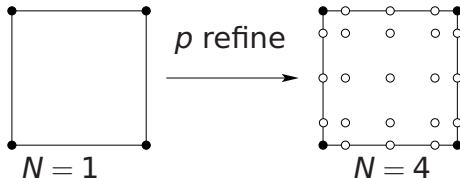
- ▶ Second-order Adams-Moulton (AM2)

$$\underline{\underline{A}}_{k+1} (\underline{\underline{x}}_{k+1} - \underline{\underline{x}}_k - \frac{\Delta t}{2} \dot{\underline{\underline{x}}}_k) = \frac{\Delta t}{2} \underline{\underline{f}}(\underline{\underline{x}}_{k+1}, t_{k+1})$$

Legendre Spectral Finite Elements

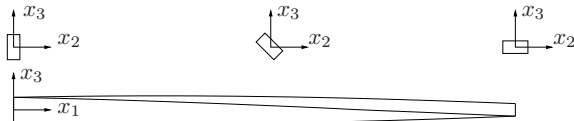
LSFE methods combine the geometric flexibility of the FE method with the accuracy of global spectral methods.

- ▶ Solution improved through increased basis polynomial order (p -refinement)
- ▶ LSFEs employ Lagrangian interpolant shape functions with nodes at Gauss-Lobatto-Legendre (GLL) points
- ▶ *Exponential* convergence rates for sufficiently smooth solutions



Example 1: Initially Twisted Beam

► Sketch of Initially Twisted Beam



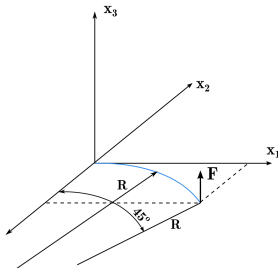
► Result

Table: Comparison of tip displacements of an initially twisted beam

	u_1 (m)	u_2 (m)	u_3 (m)
BeamDyn	-1.132727	-1.715123	-3.578671
ANSYS	-1.134192	-1.714467	-3.584232
Percent Error	0.129%	0.038%	0.155%

Example 1: Initially Curved Beam

► Sketch of Initially Curved Beam



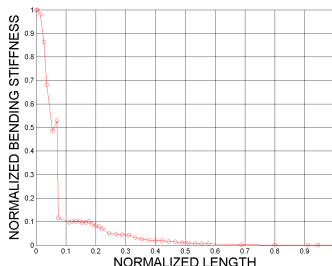
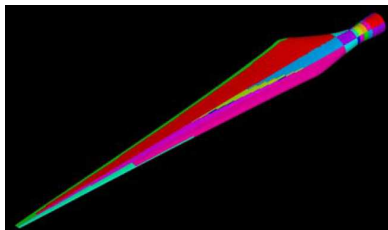
► Result

Table: Comparison of tip displacements of an initially curved beam

	u_1 (inches)	u_2 (inches)	u_3 (inches)
BeamDyn (one LSFE)	-23.7	13.5	53.4
Bathe-Bolourchi ?	-23.5	13.4	53.4

Example 2: CX-100

► Sketch of CX-100

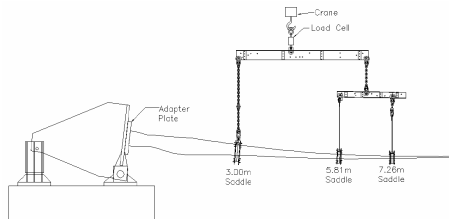


► Sectional Properties at 2.2 m

$$C = 10^3 \times \begin{bmatrix} 193,000 & -75.4 & 12.2 & -75.2 & -1970 & -3500 \\ -75.4 & 19,500 & 4,760 & 62.6 & 67.3 & 11.3 \\ 12.2 & 4,760 & 7,210 & -450 & 17.0 & 2.68 \\ -75.2 & 62.6 & -450 & 518 & 1.66 & -1.11 \\ -1,970 & 67.3 & 17.0 & 1.66 & 2,280 & -879 \\ -3,500 & 11.6 & 2.68 & -1.11 & -875 & 4,240 \end{bmatrix}$$

Example 2: CX-100 (Continued)

► Static Test Configuration



► Deflection

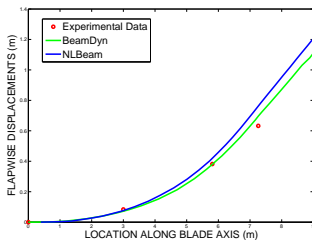
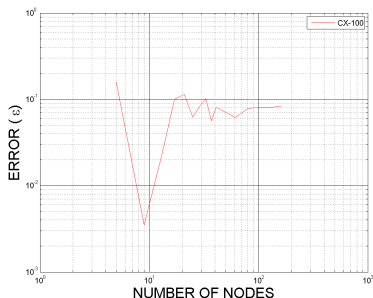


Table: Experimental and BeamDyn simulation results for the CX-100 static test

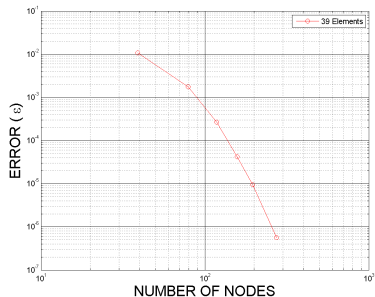
	u_3 at saddle #1 (m)	u_3 at saddle #2 (m)	u_3 at saddle #3 (m)
Experimental	0.083530	0.381996	0.632460
BeamDyn	0.072056	0.381074	0.698850
Percent Error	13.74%	0.24%	10.5%

Example 2: Convergence Study

► Validation

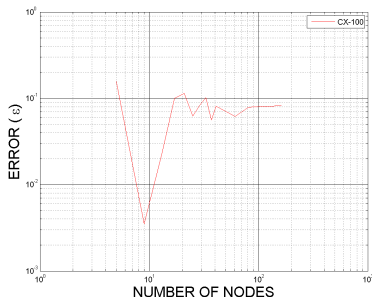


► Verification



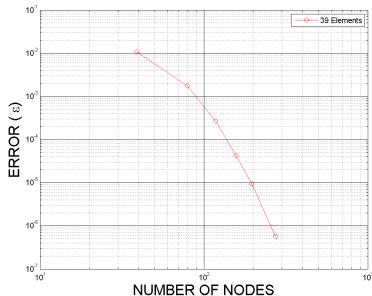
Example 2: Convergence Study

► Validation



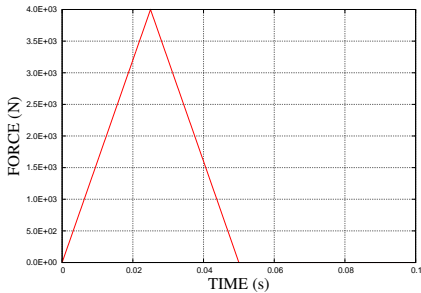
- Sharp gradients in sectional properties
- Erratic data

► Verification



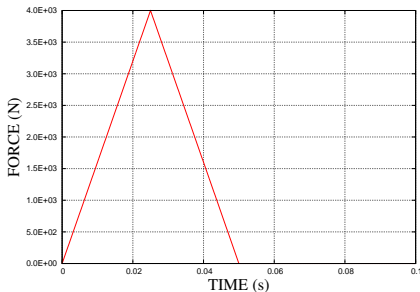
Example 3: Damping Effect

► Cantilever Beam Under Impulsive Load



Example 3: Damping Effect

► Cantilever Beam Under Impulsive Load



► Viscous Damping

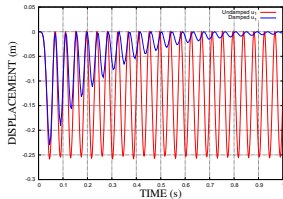
$$\underline{f}_d = \underline{\mu} \underline{C} \left\{ \begin{matrix} \dot{e} \\ \dot{\kappa} \end{matrix} \right\}$$

► RMS Error

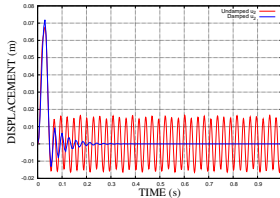
$$\epsilon_{RMS} = \sqrt{\frac{\sum_{k=0}^{n_{max}} [u_3^k - u_b(t^k)]^2}{\sum_{k=0}^{n_{max}} [u_b(t^k)]^2}}$$

Example 3: Root forces and moments

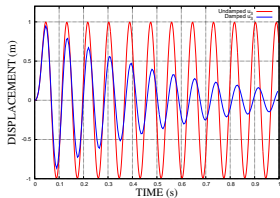
► u_1



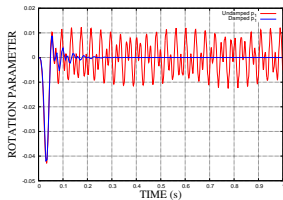
► u_2



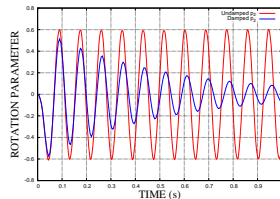
► u_3



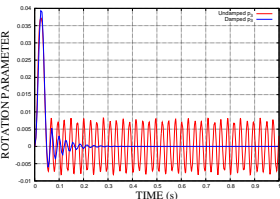
► p_1



► p_2



► p_3



Example 4: NREL 5-MW Blade

Example 4: NREL 5-MW Blade

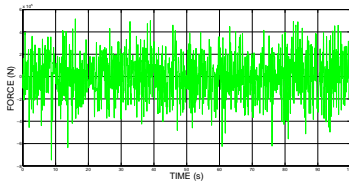
- ▶ NREL 5-MW Blade; Cantilevered at root

Example 4: NREL 5-MW Blade

- ▶ NREL 5-MW Blade; Cantilevered at root
- ▶ White noise force applied at the free tip along flap direction

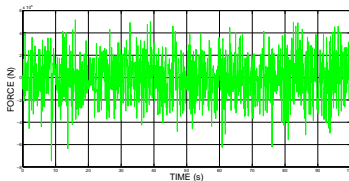
Example 4: NREL 5-MW Blade

- ▶ NREL 5-MW Blade; Cantilevered at root
- ▶ White noise force applied at the free tip along flap direction
- ▶ Time History of Applied Force

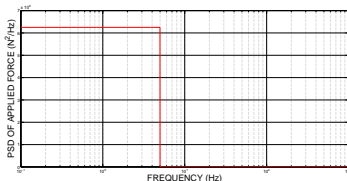


Example 4: NREL 5-MW Blade

- ▶ NREL 5-MW Blade; Cantilevered at root
- ▶ White noise force applied at the free tip along flap direction
- ▶ Time History of Applied Force



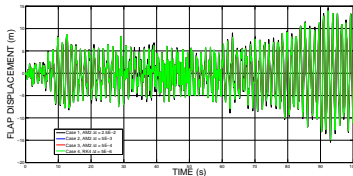
- ▶ PSD of Applied Force



Example 4: NREL 5-MW Blade (Continued)

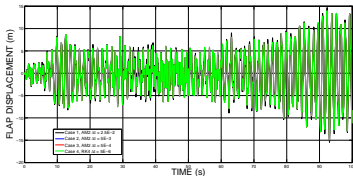
Example 4: NREL 5-MW Blade (Continued)

► Flapwise Response

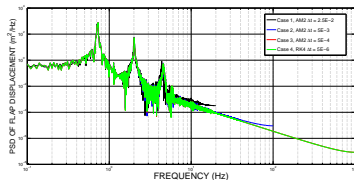


Example 4: NREL 5-MW Blade (Continued)

► Flapwise Response

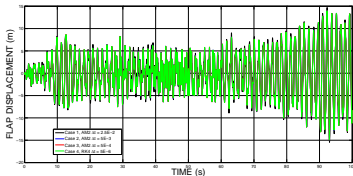


► PSD of Flapwise Response

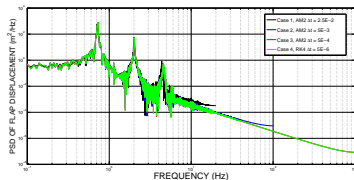


Example 4: NREL 5-MW Blade (Continued)

► Flapwise Response



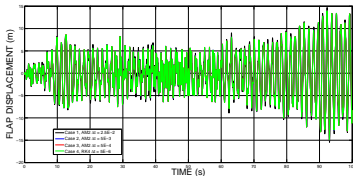
► PSD of Flapwise Response



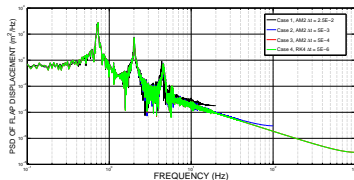
- For an implicit AM2 time step beyond 0.005 s, the solution is nearly identical to the fully resolved explicit RK4 solution

Example 4: NREL 5-MW Blade (Continued)

► Flapwise Response



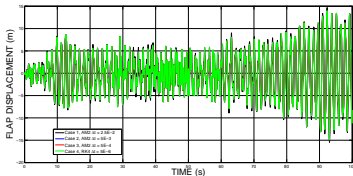
► PSD of Flapwise Response



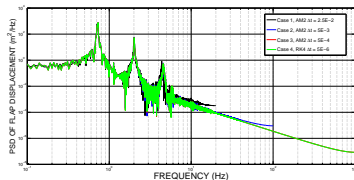
- For an implicit AM2 time step beyond 0.005 s, the solution is nearly identical to the fully resolved explicit RK4 solution
- For an AM2 time step of 0.025 s, the solution remains stable and tracks the other solutions, but error grows at higher frequencies

Example 4: NREL 5-MW Blade (Continued)

► Flapwise Response



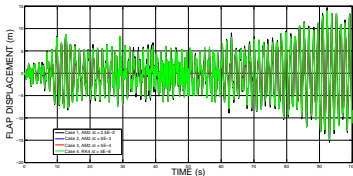
► PSD of Flapwise Response



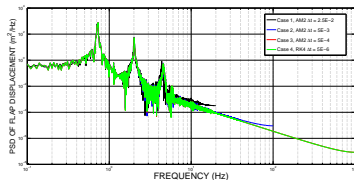
- For an implicit AM2 time step beyond 0.005 s, the solution is nearly identical to the fully resolved explicit RK4 solution
- For an AM2 time step of 0.025 s, the solution remains stable and tracks the other solutions, but error grows at higher frequencies
- The spikes at 0.7 Hz and 2 Hz correspond to the first and second blade flapwise natural frequencies

Example 4: NREL 5-MW Blade (Continued)

► Flapwise Response



► PSD of Flapwise Response



- For an implicit AM2 time step beyond 0.005 s, the solution is nearly identical to the fully resolved explicit RK4 solution
- For an AM2 time step of 0.025 s, the solution remains stable and tracks the other solutions, but error grows at higher frequencies
- The spikes at 0.7 Hz and 2 Hz correspond to the first and second blade flapwise natural frequencies
- The spike above 5 Hz—above the frequency range of excitation—is brought about by nonlinear effects

Example 4: NREL 5-MW Blade (Continued)

► Convergence Rate

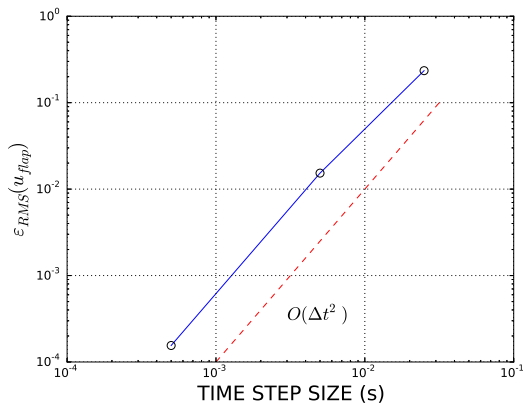


Figure: Normalized RMS error of flapwise displacement histories as a function of time step size for AM2 time integrator. The dashed line shows ideal second-order convergence.

Example 4: NREL 5-MW Blade (Continued)

► Solver Statistics

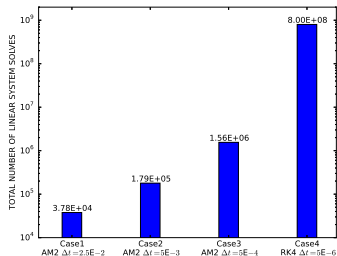


Figure: Total number of linear system solves.

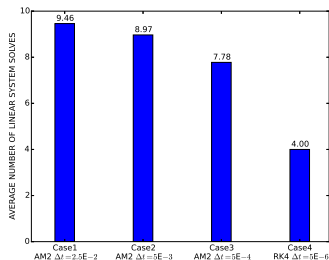


Figure: Average number of linear system solves per step.

Summary

► Conclusion

- Based on **geometrically exact beam theory**, BeamDyn is capable of dealing with **geometric nonlinear** beam problems with arbitrary magnitude of displacements and rotations for both static and dynamic analyses
- Along with a preprocessor like PreComp or VABS, BeamDyn takes **full elastic coupling effects** into account
- The governing equations are reformulated into **state-space form**, thus, making it amendable into FAST for tight-coupling analysis
- The space is discretized by **spectral finite elements**, which is a p-version finite element, so that **exponential convergence rate** can be expected for smooth solutions
- **Different time integrators** have been implemented in BeamDyn; users will have options based on their needs
- BeamDyn is implemented following the programming requirements (data structures and interfaces) of the **FAST modularization framework**

Summary (Continued)

► Future Work

- Coupling BeamDyn to FAST
- Full-Turbine validation
- Enhancement of numerical performance

Questions?

Acknowledgments

- ▶ Funded by the U.S. Department of Energy under Contract No. DE-AC36-08-GO28308 with the National Renewable Energy Laboratory.
- ▶ Support was provided through an NREL Laboratory Directed Research and Development grant *High-Fidelity Computational Modeling of Wind-Turbine Structural Dynamics*

References

- Hodges, D. H., 2006. Nonlinear Composite Beam Theory. AIAA.
- Jelenić, G., Crisfield, M. A., 1999. Geometrically exact 3d beam theory: implementation of a strain-invariant finite element for statics and dynamics. *Computer Methods in Applied Mechanics and Engineering* 171, 141–171.
- Jonkman, J. M., January 2013. The new modularization framework for the fast wind turbine cae tool. In: *Proceedings of the 51st AIAA Aerospace Sciences Meeting including the New Horizons Forum and Aerospace Exposition*. Grapevine, Texas.
- Reissner, E., 1973. On one-dimensional large-displacement finite-strain beam theory. *Studies in Applied Mathematics LII*, 87–95.
- Yu, W., Blair, M., 2012. Gebt: A general-purpose nonlinear analysis tool for composite beams. *Composite Structures* 94, 2677–2689.