```
function [DEFL,REACT,ELE_FOR,AFLAG] = ud_3d1el(...

nnodes,coord,concen,fixity,nele,ends,A,Izz,Iyy,J,Cw,IsSym,Ysc,Zsc,Betay,Betaz,Betaw
,Zzz,Zyy,Ayy,Azz,...
      E,v,Fy,YldSurf,Wt,webdir,beta_ang,w,thermal,truss,anatype);

% Code developed by Mrunmayi Mungekar and Devasmit Dutta
%
% UD_3D1EL performs a user defined three-dimensional
% first-order elastic analysis of a structural system.
%

%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%
%  Functions Called
%              < to be defined by the student >
%
%  Dictionary of Variables
%     Input Information:
%       nnodes         ==  total number of nodes
%       coord(i,1:3)   ==  node i's coordinates
%                            coord(i,1) = X coordinate
%                            coord(i,2) = Y coordinate
%                            coord(i,3) = Z coordinate
%       concen(i,1:6)  ==  concentrated loads for node i's 6 d.o.f.
%                            concen(i,1) = force in global X direction
%                            concen(i,2) = force in global Y direction
%                            concen(i,3) = force in global Z direction
%                            concen(i,4) = moment about global X axis
%                            concen(i,5) = moment about global Y axis
%                            concen(i,6) = moment about global Z axis
%       fixity(i,1:6)  ==  prescribed displacements for node i's 6 d.o.f.
%                          Note: A free d.o.f. will have a value of NaN
%                          and hence, you will find the Matlab function
%                          isnan very useful.
%                          Examples: If fixity(15,3) is set to NaN, then node 15's
%                                      Z-disp component is free;
%                                    If fixity(2,6) is set to 0.0, then node 2's
%                                      Z-rotation component is supported;
%                                    If fixity(5,2) is set to -2.1, then node 5's
%                                      Y-disp component is supported and defined
%                                      with a settlement of -2.1 units.
%                            fixity(i,1) = prescribed disp. in global X direction
%                            fixity(i,2) = prescribed disp. in global Y direction
%                            fixity(i,3) = prescribed disp. in global Z direction
%                            fixity(i,4) = prescribed rotation about global X axis
%                            fixity(i,5) = prescribed rotation about global Y axis
%                            fixity(i,6) = prescribed rotation about global Z axis
%       nele           ==  total number of elements
%       ends(i,1:14)   ==  element i's nodal information
%                            ends(i,1) = start node #
%                            ends(i,2) = finish node #
%                            ends(i,3) = flag to indicate whether or not flexural
%                            moments are released at start node.  ends(i,3)=0 both
not
%                            released (rigid connection); ends(i,3)=1 both flexural
%                            moments are released (pinned connection); ends(i,3)=2
%                            at least one of the flexural moments are partially or
```

```
fully
%                           released (see below for connection stiffness
attributes)
%                           ends(i,4) = flag to indicate whether or not flexural
%                           moments are released at finish node.  ends(i,4)=0 both
not
%                           released (rigid connection); ends(i,4)=1 both flexural
%                           moments are released (pinned connection); ends(i,4)=2
%                           at least one of the flexural moments are partially or
fully
%                           released (see below for connection stiffness
attributes)
%                           ends(i,5) = flag to indicate the degree of warping
%                           restraint at start node.  ends(i,5)=0 warping free;
%                           ends(i,5)=1 warping fixed; ends(i,5)=2 warping
continuous
%                           ends(i,6) = flag to indicate the degree of warping
%                           restraint at finish node.  ends(i,6)=0 warping free;
%                           ends(i,6)=1 warping fixed; ends(i,6)=2 warping
continuous
%                           ends(i,7) = rotational spring stiffness at the start
%                           node and about element i's local z-z axis.
%                           ends(i,8) = rotational spring stiffness at the start
%                           node and about element i's local y-y axis.
%                           ends(i,9) = rotational spring stiffness at the finish
%                           node and about element i's local z-z axis.
%                           ends(i,10) = rotational spring stiffness at the finish
%                           node and about element i's local y-y axis.
%                           ends(i,11) = connection moment capacity Mpz at the
start
%                           node and about element i's local z-z axis.
%                           ends(i,12) = connection moment capacity Mpy at the
start
%                           node and about element i's local y-y axis.
%                           ends(i,13) = connection moment capacity Mpz at the
finish
%                           node and about element i's local z-z axis.
%                           ends(i,14) = connection moment capacity Mpy at the
finish
%                           node and about element i's local y-y axis.
%       A(i)            ==  element i's cross sectional area
%       Izz(i)          ==  element i's moment of inertia about its local z-z axis
%       Iyy(i)          ==  element i's moment of inertia about its local y-y axis
%       J(i)            ==  element i's torsional constant
%       Cw(i)           ==  element i's warping constant
%       Zzz(i)          ==  element i's plastic section modulus about its local z-z
axis
%       Zyy(i)          ==  element i's plastic section modulus about its local y-y
axis
%       Ayy(i)          ==  element i's effective shear area along its local y-y
axis
%       Azz(i)          ==  element i's effective shear area along its local z-z
axis
%       E(i)            ==  element i's material elastic modulus, Young's Modulus
%       v(i)            ==  element i's material Poisson's ratio
%       Fy(i)           ==  element i's material yield strength
%       YldSurf(i)      ==  element i's yield surface maximum values
%                               YldSurf(i,1) = maximum P/Py value
%                               YldSurf(i,2) = maximum Mz/Mpz value
```

```
%                               YldSurf(i,3) = maximum My/Mpy value
%       Wt(i)           ==  element i's material weight density
%                           (Assume that gravity is directed in the negative global
Y dir)
%       webdir(i,1:3)   ==  element i's unit web vector.  This is a unit vector
%                           that defines the element's local y-y axis with respect
%                           to the global coordinate system.  It is based on the
%                           structure's undeformed geometry.
%                               webdir(i,1) = x component of element's unit web
vector
%                               webdir(i,2) = y component of element's unit web
vector
%                               webdir(i,3) = z component of element's unit web
vector
%                           NOTE: An element's 3x3 rotation matrix, [g], is
constructed
%                           as follows: First, calculate a unit vector, x_vect, that
%                           describes the element's local x-axis. Second, take the
%                           cross product of x_vect and webdir(i,:) to obtain
z_vect,
%                           i.e. z_vect = cross(x_vect,webdir(i,:)). Third, set
z_vect
%                           to a unit vector, i.e. z_vect = z_vect/norm(z_vect).
%                           Finally, the first row of [g] is x_vect, its second row
is
%                           webdir(i,:), and its third row is z_vect.
%       beta_ang(i)     ==  element i's web rotation angle.  These values are
%                           provided for those students who are required to
calculate
%                           their own unit web vectors (see above).  It is based
%                           on the structure's undeformed geometry.
%                           Note:  MASTAN2 uses the following convention for
%                                  defining a member's default web orientation:
%                                  A vector defing the element's local y-axis
%                                  with respect to the global coordinate system
%                                  will have a positive component in the global
%                                  Y direction.  If the element's local x-axis,
%                                  its length axis, is aligned with the global Y
%                                  axis, then element's local y-axis is aligned
%                                  with global negative X axis.  After this initial
%                                  orientation, element i may be rotated about
%                                  its local x-axis by the amount defined by
%                                  its web rotation angle, beta_ang(i).  The
%                                  angle is in radians and assumes a right-hand
%                                  convention about the local x-axis which runs from
%                                  the element's start node to its finish node.
%       w(i,1:3)          ==  element i's uniform load which references its
%                             local coordinate system
%                               w(i,1) = x component of uniform load
%                               w(i,2) = y component of uniform load
%                               w(i,3) = z component of uniform load
%       thermal(i,1:4)    ==  element i's thermal strain effects which reference its
%                             local coordinate system
%                               thermal(i,1) = coefficient of thermal expansion
%                               thermal(i,2) = change in temperature at centroid
%                               thermal(i,3) = linear temperature gradient in local
y-dir
%                                            = (T_up_y - T_btm_y) / depth_y
%                               thermal(i,4) = linear temperature gradient in local
```

```
z-dir
%                                              = (T_up_z - T_btm_z) / width_z
%       truss            ==  flag to indicate if structure is a truss or not
%                              truss = 0   System is not a truss
%                              truss = 1   System is a truss
%       anatype          ==  flag to indicate which type of analysis is requested
%                              anatype = 1  First-Order Elastic
%                              anatype = 2  Second-Order Elastic
%                              anatype = 3  First-Order Inelastic
%                              anatype = 4  Second-Order Inelastic
%                              anatype = 5  Elastic Buckling (Eigenvalue)
%                              anatype = 6  Inelastic Buckling (Eigenvalue)
%
%     Local Information:
%              < to be defined by the student >
%
%     Output Information:
%       DEFL(i,1:6)      ==  node i's calculated 6 d.o.f. deflections
%                              DEFL(i,1) = displacement in X direction
%                              DEFL(i,2) = displacement in Y direction
%                              DEFL(i,3) = displacement in Z direction
%                              DEFL(i,4) = rotation about X direction
%                              DEFL(i,5) = rotation about Y direction
%                              DEFL(i,6) = rotation about Z direction
%       REACT(i,1:6)     ==  reactions for supported node i's 6 d.o.f.
%                              REACT(i,1) = force in X direction
%                              REACT(i,2) = force in Y direction
%                              REACT(i,3) = force in Z direction
%                              REACT(i,4) = moment about X direction
%                              REACT(i,5) = moment about Y direction
%                              REACT(i,6) = moment about Z direction
%       ELE_FOR(i,1:1?)  ==  element i's internal forces and moments
%                            Note: All values reference the element's local
%                                  coordinate system.
%                              ELE_FOR(i,1)  = x-force at start node
%                              ELE_FOR(i,2)  = y-force at start node
%                              ELE_FOR(i,3)  = z-force at start node
%                              ELE_FOR(i,4)  = x-moment at start node
%                              ELE_FOR(i,5)  = y-moment at start node
%                              ELE_FOR(i,6)  = z-moment at start node
%                              ELE_FOR(i,7)  = x-force at end node
%                              ELE_FOR(i,8)  = y-force at end node
%                              ELE_FOR(i,9)  = z-force at end node
%                              ELE_FOR(i,10) = x-moment at end node
%                              ELE_FOR(i,11) = y-moment at end node
%                              ELE_FOR(i,12) = z-moment at end node
%                            If you are not programming warping torsion, the
ELE_FOR
%                            array needs to contain only 12 columns, i.e.
ELE_FOR(i,1:12)
%                            For those programming warping torsion, the bimoments
and
%                            rates of twist should be stored as follows.
%                              ELE_FOR(i,13) = bimoment at start node
%                              ELE_FOR(i,14) = bimoment at end node
%                              ELE_FOR(i,15) = rate of twist at start node
%                              ELE_FOR(i,16) = rate of twist at end node
%       AFLAG            ==  logical flag to indicate if a successful
%                            analysis has been completed
```

```
%                              AFLAG = 1     Successful
%                              AFLAG = 0     Unstable Structure
%                              AFLAG = inf   No analysis code available
%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%%%%%%
%
%  Start by defining all output arrays to be empty
%
DEFL=[]; REACT=[]; ELE_FOR=[];

memb_id = MD_member_id(nnodes, nele, ends);

disp('member id');
disp(memb_id);

concen_applied_load_dof = MD_concen_load_dof(concen, nnodes);

FEF = zeros(6*nnodes,1);

for i =1:nele
   start_node = ends(i,1);
   end_node = ends(i,2);
   start_coord = coord(start_node,:);
   end_coord = coord(end_node,:);
   L = norm(end_coord - start_coord);
   memberlocalFEF = MD_computeMemberFEFs(w(i,:),L);
   gamma = MD_etran(start_coord,end_coord,webdir(i,:));
   memberglobalFEF = gamma'*memberlocalFEF;
   FEF(memb_id(i,:),1) = memberglobalFEF + FEF(memb_id(i,:),1);
end

disp('concen_applied_load_dof');
disp(concen_applied_load_dof);

disp('FEF');
disp(FEF);

D = fixity';
D = D(:);
freeDOF = find(isnan(D));
supportDOF = D == 0;
displacedDOF = find(D~=0 & ~isnan(D));

kstructureglobal = zeros(6*nnodes,6*nnodes);
for i =1:nele
   start_node = ends(i,1);
   end_node = ends(i,2);
   start_coord = coord(start_node,:);
   end_coord = coord(end_node,:);
   L = norm(end_coord - start_coord);

   kele_local = MD_estiff(A(i), Izz(i), Iyy(i), J(i), Ayy(i), Azz(i), E(i), v(i),
L)
   gamma = MD_etran(start_coord,end_coord,webdir(i,:));
   kele_global = gamma'*kele_local*gamma;
   kstructureglobal(memb_id(i,:),memb_id(i,:)) = kele_global +
kstructureglobal(memb_id(i,:),memb_id(i,:));
```

```
end

Kff = kstructureglobal(freeDOF,freeDOF);
Kfn = kstructureglobal(freeDOF,displacedDOF);
Knf = kstructureglobal(displacedDOF,freeDOF);
Ksf = kstructureglobal(supportDOF,freeDOF);
Ksn = kstructureglobal(supportDOF,displacedDOF);
Knn = kstructureglobal(displacedDOF,displacedDOF);

Pf = concen_applied_load_dof(freeDOF);
FEFf = FEF(freeDOF);
FEFs = FEF(supportDOF);
FEFn = FEF(displacedDOF);
Delta_n = D(displacedDOF);
Delta_f = (Kff)\(Pf - FEFf - Kfn*Delta_n);

disp('Delta_f');
disp(vpa(Delta_f,4));

Rs = FEFs + Ksf*Delta_f + Ksn*Delta_n;
Rn = FEFn + Knf*Delta_f + Knn*Delta_n;

disp('Rs');
disp(vpa(Rs,4));

disp('Rn');
disp(vpa(Rn,4));

disp('back-calculating freeDOF load')
disp(vpa(FEFf + Kff*Delta_f + Kfn*Delta_n,4))

AFLAG = inf;
%
%  STUDENT NOTE:
%     In order for this routine to become fully active AFLAG
%     must be changed.
%
%
%  Student's code starts here...
%
%
%
%  Good luck CE Student!!!
%
```