## **CURRENT CODES**

**SOIL UNIT WEIGHT** 

SUBGRADE MODULUS

LATERAL EARTH PRESSURE - AT REST (BASEMENT WALLS)

LATERAL EARTH PRESSURE - PASSIVE

COEFFICIENT OF SLIDING FRICTION

ALLOWABLE SOIL BEARING PRESSURE

- INTERNATIONAL BUILDING CODE 2015
- ASCE STANDARD ASCE/SEI 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
- 2012 NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION NATIONAL DESIGN SPECIFICATION (NDS) SUPPLEMENT: DESIGN VALUES FOR WOOD CONSTRUCTION 2015
- ACI 318-11: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY • 530/530.1-11: BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTURES AND
- RELATED COMMENTARIES AWS D1.1/D1.1M:2015 STRUCTURAL WELDING CODE - STEEL ANSI/AISC 360-10 - SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS

ANSI/AISC 360-10 - SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS	•
DESIGN - LOADS & CRITERIA	
1-LIVE LOAD	
FLOOR FRAMING (RETAIL, OFFICE, RESTAURANT, RECREATIONAL)	100 PSF
FLOOR FRAMING (RESIDENTIAL AREAS)	40 PSF
STAIRWAYS, EXITS	100 PSF
BALCONIES	75 PSF
PRIVATE GARAGES (PASSENGER VEHICLES ONLY)	40 PSF
INTERIOR PARTITION WALLS (UNIFORMLY DISTRIBUTED WEIGHT)	15 PSF
CORRIDORS FIRST FLOOR	100 PSF
CORRIDORS 2ND AND 3RD FLOORS	40 PSF
CORNICES	60 PSF
2-SNOW LOAD	
DESIGN SNOW LOAD	42 PSF (BALANCED)
(pF) FLAT ROOF SNOW LAOD (pf = 0.7CeCtlspg)	42 PSF
(Ce) SNOW EXPOSURE FACTOR	1.0
(Is) SNOW LOAD IMPORTANCE FACTOR	1.0
(Ct) ROOF THERMAL FACTOR	1.0
(Pg) GROUND \$NOW	60 PSF
(Cs) SLOPED ROOF FACTOR	1.0
3-WIND LOADS	
(IW) WIND IMPORTANCE FACTOR	1.0
RISK CATEGORY II	
BASIC WIND SPEED (3-SEC. GUST, ULTIMATE)	115 MPH
BASIC WIND SPEED (3-SEC. GUST, NOMINAL)	90 MPH
MEAN ROOF HEIGHT	33FT
WIND EXPOSURE CATEGORY	В
WIND EXPOSURE CLASSIFICATION	ENCLOSED
(Kz) VELOCITY EXPOSURE COEFFICIENT	0.720
(Kzt) TOPOGRAPHIC FACTOR	1.0
DESIGN PROCEDURE	METHOD 1 (SIMPLIED)
4-EARTHQUAKE LOADS	
OCCUPANCY CATEGORY	II
(Ie) SEISMIC IMPORTANCE FACTOR	1
(Ss) SHORT PERIOD SPECTRAL ACCELERATION	0.045 g
SPECTRAL RESPONSE ACCELERATIONS SS, AT SHORT PERIODS	0.038 g
SITE CLASSIFICATIONS	В
(Fa) SITE COEFFICIENT	1.0
(Fv) SITE COEFFICIENT	1.0
(Sds) DESIGN SPECTRAL RESPONSE ACCELERATION - SHORT PERIODS	0.030 g
(Sd1) DESIGN SPECTRAL RESPONSE ACCELERATION - AT 1 SEC. PERIODS	0.025 g
SEISMIC DESIGN CATEGORY	Α
SEISMIC-FORCE-RESISTING SYSTEM	WOOD STRUCTURAL PANEL (WSP) SHEATHED SHEAR WALLS
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE PROCEDURE (ELFP)
5-SOIL	

125 PCF (ASSUMED)

62.5 FPS/FT OF DEPTH (ASSUMED)

340 PSF (ASSUMED)

0.30 (ASSUMED)

260 PCI (ASSUMED)

3000 PSF

MATERIAL STRENGTHS	
CONCRETE - EXT. SLABS ON GRADE	
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	YES 4% to 6%
CONCRETE - FOOTINGS	fic = 2 000 BCI
MIN. COMPRESSIVE STRENGTH AT 28 DAYS  MAX. WATER-CEMENTITIOUS RATIO	f'c = 3,000 PSI 0.59
MAX. AGGREGATE SIZE	1 1/2"
SLUMP LIMIT	5" +/-1"
AIR CONTENT	NO
CONCRETE - INTERIOR SLABS ON GRADE	
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	NO
CONCRETE - PIERS, WALLS, AND COLUMNS	
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	YES 4% to 6%
CONCRETE - SLURRY	
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 1,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.55
MAX. AGGREGATE SIZE	1 1/2"
SLUMP LIMIT	6" +/-1"
AIR CONTENT	NO
CONCRETE - STAIR LANDING/TREADS MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	NO
CONCRETE - TOPPING	110
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	NO
STEEL - REINFORCING STEEL	· · · · · · · · · · · · · · · · · · ·
ALL ASTM A615, GRADE 60, DEFORMED	Fy = 60,000 PSI
WELDED WIRE REINFORCEMENT, FLAT SHEETS	Fy = 60,000 PSI
STEEL - STRUCTURAL BOLTS	
HIGH STRENGTH BOLTS, NUTS, & WASHERS	ASTM A325
ZINC-COATED HIGH STRENGTH BOLTS, NUTS, & WASHERS	ASTM A325
STAINLESS STEEL BOLTS, NUTS, & WASHERS	ASTM F593
SHEAR CONNECTORS (GRADES 1015 THRU 1020)	ASTM A108
THREADED RODS	ASTM A36
CLEVIS & TURNBUCKLES (GRADE 1035)	ASTM A108
EYE BOLTS & NUTS (GRADE 1030)	ASTM A108
ANCHOR BOLTS (GRADE 36)	ASTM F1554
STEEL - STRUCTURAL STEEL	F F0 000 PC!
ROLLED WIDE FLANGE SHAPES, ASTM A992 GRADE 50	Fy = 50,000 PSI
CHANNELS, ANGLES, AND S SHAPES, ASTM A36 PLATE AND BAR, ASTM A36	Fy = 36,000 PSI Fy = 36,000 PSI
TUBE SHAPES, ASTM A500 GRADE B	Fy = 46,000 PSI
PIPE ASTM A53, TYPE E or S, GRADE B	Fy = 46,000 PSI
I II E ASIM ASS, TIFE E OFS, GRADE D	Fy = 36,000 PSI

	DEFLECTION LIMITS		
DESCRIPTION	LIVE LOADS	SNOW OR DEAD LOADS	

RIGID MATERIALS (BRICK/MASONRY)	N/A	L/600	N/A
FLEXIBLE MATERIALS	N/A	L/360	N/A
FLOOR			
RIGID MATERIALS (BRICK/MASONRY)	L/600	L/600	L/600
WITH GYPSUM BOARD CEILINGS	L/540	N/A	L/360
FLEXIBLE MATERIALS	L/540	N/A	L/360
LINTELS, HEADER, AND/OR BEAM			
RIGID MATERIALS (BRICK/MASONRY)	L/600	L/600	L/600
FLEXIBLE MATERIALS	L/360	L/360	L/240
ROOF			
WITH GYPSUM BOARD CEILINGS	L/360	L/360	L/240
FLEXIBLE CEILINGS	L/360	L/360	L/240
NO CEILING	L/240	L/240	L/180
RIGID MATERIALS (BRICK/MASONRY)	L/600	L/600	L/600

NET PRESSURE	COEFFICIENTS Cnet	
AREA	C <sub>net</sub> + INTERNAL PRESSURE	C <sub>net</sub> - INTERNA PRESSURE
WINDWARD WALL	0.43	0.73
LEEWARD WALL	-0.51	-0.21
SIDEWALL	-0.66	-0.35
PARAPET WINDWARD WALL	1.	28
PARAPET LEEWARD WALL	-0	.85
FLAT ROOF	-1.09	-0.79

NET PRESSURE	COEFFICIENTS P <sub>net</sub>	
AREA	P <sub>net</sub> + INTERNAL PRESSURE	P <sub>net</sub> - INTERNAL PRESSURE
WINDWARD WALL	10.5 psf	17.8 psf
LEEWARD WALL	-12.4 psf	-5.1 psf
SIDEWALL	-16.1 psf	-8.5 psf
PARAPET WINDWARD WALL	3	1.2 psf
PARAPET LEEWARD WALL	-2	0.7 psf
FLAT ROOF	-26.6 psf	-19.3 psf

## FOOTINGS, FOUNDATIONS AND EARTHWORK:

• DO NOT POUR ANY FOOTINGS ON FROZEN SOILS ALL FOOTINGS POURED BELOW LOCAL FROST LINE

• EXPOSED BEARING STRATUM SHOULD BE VERIFIED BY CONTRACTOR AND SOILS ENGINEER TO SATISFY NET ALLOWABLE SOIL BEARING CAPACITY AS OUTLINED IN THESE CONSTRUCTION DOCUMENTS, AS WELL AS THE PROJECT'S GEO-TECHNICAL REPORT. IF EXCAVATED BEARING SOILS FALL BELOW OUTLINED CAPACITIES, CONTINUE EXCAVATION UNTIL BEARING CAPACITIES ARE MET. EXTEND FOOTINGS DOWN TO NEW BEARING ELEVATION, OR USE ENGINEERED FILL TO MAKE UP THE DIFFERENCE. ALL ENGINEERED FILL TO BE DESIGNED BY SOILS ENGINEER AND TO BE FREE OF DRAINING GRANULAR MATERIAL COMPACTED TO 95% OF MODIFIED PROCTOR DENSITIES. • SEE DESIGN LOADS AND CRITERIA FOR DETAILS RELATIVE TO ASSUMED SOIL CONDITIONS, AND GEO-TECHNICAL

RECOMMENDATIONS. COORDINATE ANY SOIL CAPACITIES THAT FALL BELOW THOSE OUTLINED WITH THE STRUCTURAL

• WHEN APPLICABLE, BACK-FILLING SHOULD BE DONE SIMULTANEOUSLY ON BOTH SIDE OF A CONCRETE FOUNDATION

• DO NOT PLACE BACK FILL AGAINST BASEMENT WALLS UNTIL 1ST FLOOR FRAMING SYSTEM IS IN PLACE. • SHORING AND UNDERPINNING DESIGNED TO PREVENT 1/4" MAX MOVEMENT IN EITHER THE VERT/HORZ. DIRECTION • ALL BACK FILL WITHIN 3FT OF FOUNDATION WALLS TO BE GRANULAR FILL APPROVED BY THE PROJECT'S SOIL'S ENGINEER AND COMPACTED TO 90% OF STANDARD PROCTOR DENSITY

## **REBAR CONTINUITY:**

UNLESS OTHERWISE NOTED, ALL REINFORCING SHOULD BE CONTINOUS. CONTACT LAPSE SLICING TO BE USED AT CORNERS AND INTERSECTION - SEE TYPICAL DETAIL. ALL SPLICES TO BE CLASS B SPLICES SHOP DRAWINGS SHOULD SHOW THE LOCATION AND OVERLAP DISTANCE OF ALL LAP SPLICES.

		REBAI	CONTINU	YTIL				
REBAR LOCATION	#3	#4	#5	#6	#7	#8	#9	#10
3,000 - 3,500 PSI								
	0.011	0.011	0.411	4=11	7011	0.411	4.4.011	4.50
TOP BARS	22"	20"	34"	47"	70"	94"	118"	150"
				0.411	C CU	71"	115"	143"
OTHER BARS	17"	22"	28"	34"	55"	/ 1	113	143
OTHER BARS 4,000 - 4,500 PSI	17"	22"	28"	34"	55	/1	115	143
	16"	19"	26"	34"	60"	80"	103"	158"

TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THE REINFORCEMENT. TABLE ABOVE DOES NOT APPLY TO EPOXY COATED REBAR

MECHANICAL CONNECTIONS CAN BE USED IN PLACE OF LAP SPLICES. SHOP DRAWING, WITH APPROPROATE ICC-ES REPORT, TO BE SUBMITTED TO ARCHITECT AND ENGINEER FOR REVIEW. CONNECTIONS SHALL DEVELOP 125 PERCENT OF THE SPECIFIED YIELD STRIGHTH OF THE BAR IN TENSION.













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