



STRUCTURAL ENGINEERING

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From: Ana

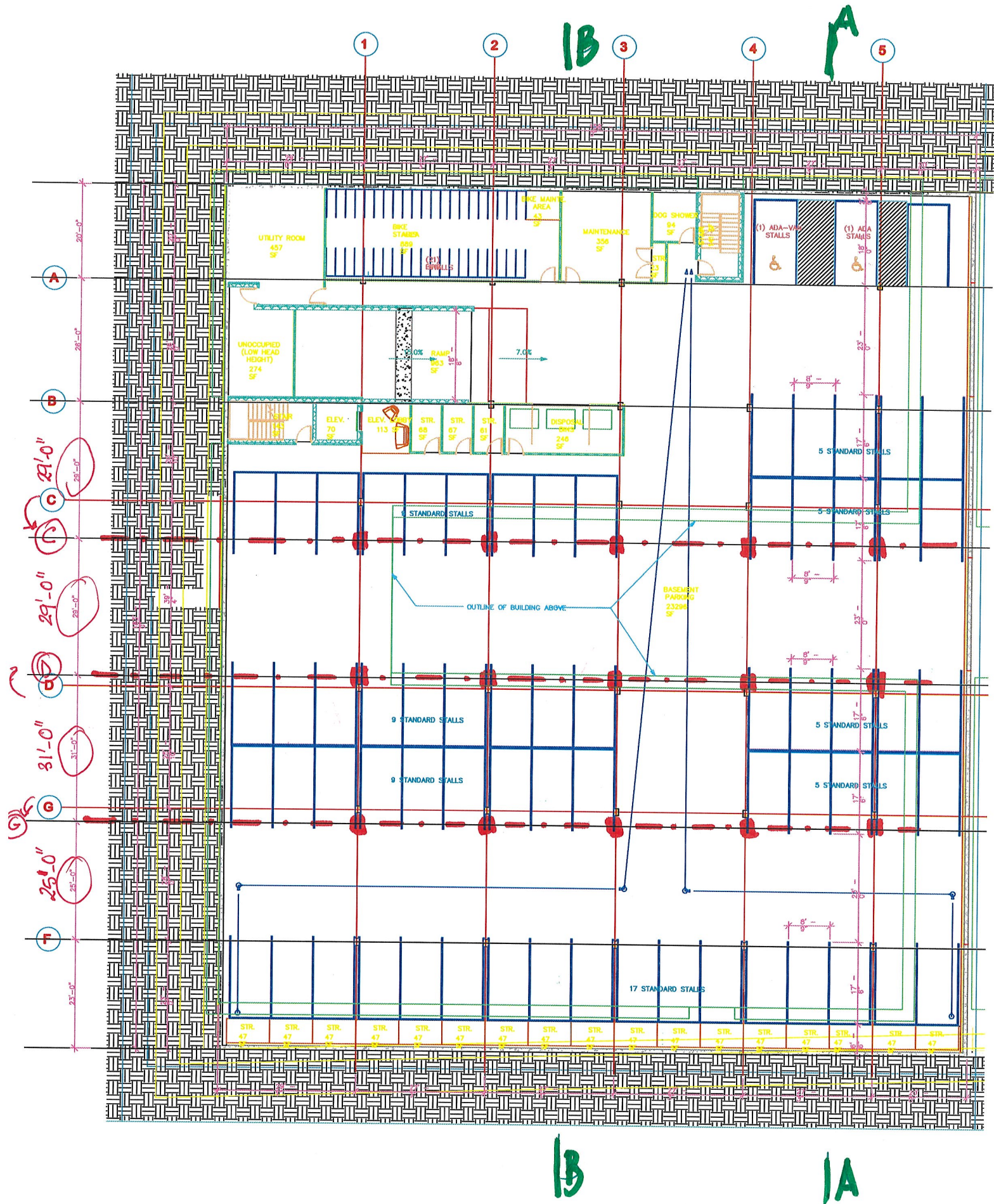
Subject: Proposed column layout.

Date: May 23, 2019

The column layout shown in figure 0 is proposed, based on a preliminary calculation of internal forces in sections A-A and B-B of the 1st floor. The objective is to achieve the most similar internal forces possible in all the spans in order to have types of hollowcore deck and reinforcement in compression layer as uniform as possible.

Figures 1 to 4 show the loads considered in the simple beam model of section A-A, figs. 5 to 6 show the results of internal forces obtained in the worst case. Figs. 1 to 6 are analogous for section B-B.

Figure 0: column layout



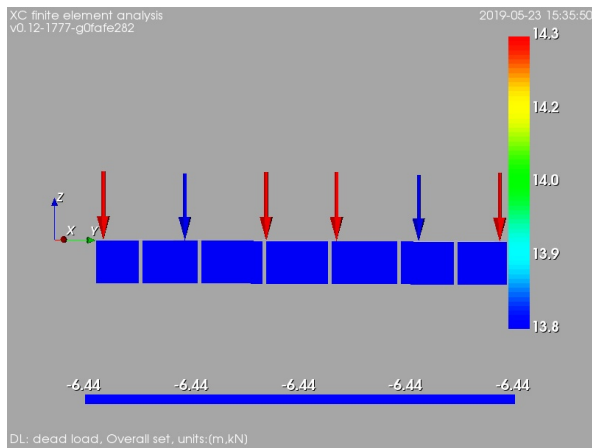


Figure 1: Sect. A-A. D: dead load

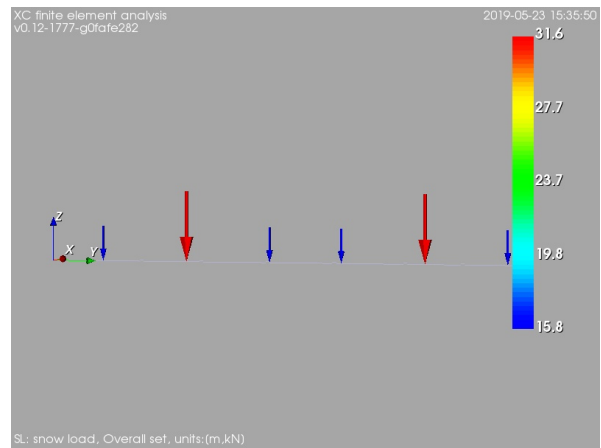


Figure 4: Sect. A-A. L_r : snow load

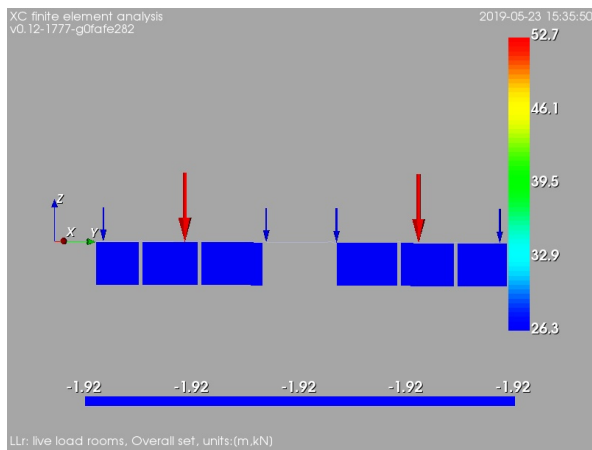


Figure 2: Sect. A-A. L_r : live load rooms

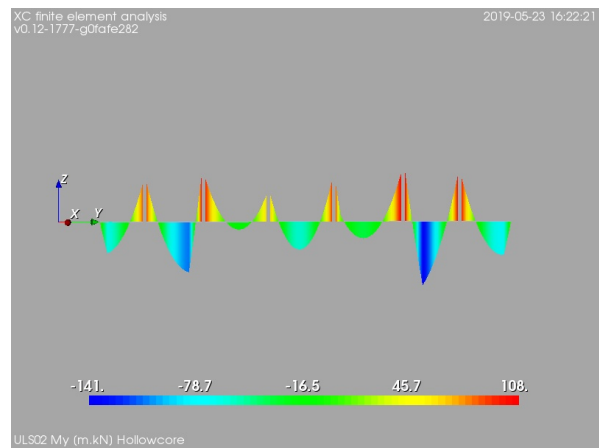


Figure 5: Sect. A-A. Maximum bending moment (ULS: $1.2*D+1.6*L_r+1.6*L_t+0.5*S$)

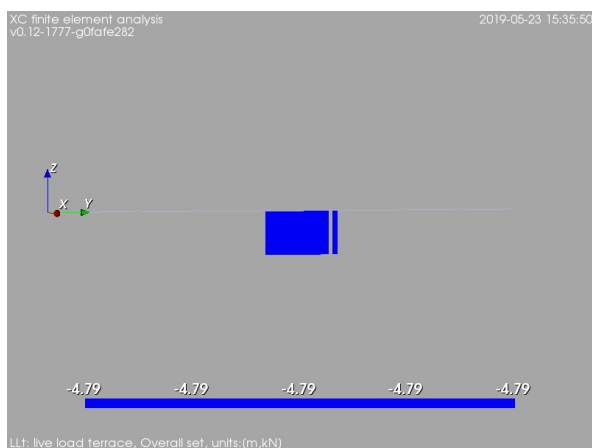


Figure 3: Sect. A-A. L_t : live load terrace

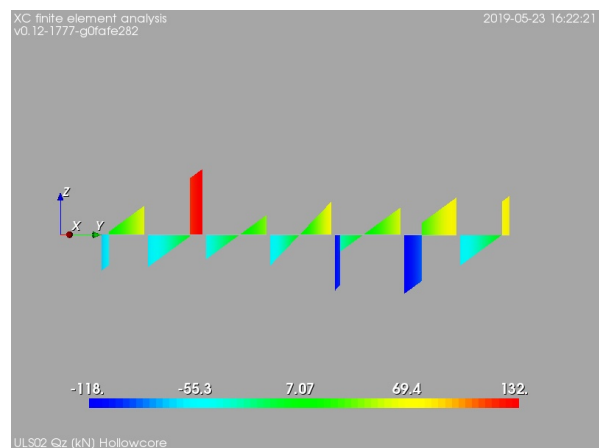


Figure 6: Sect. A-A. Maximum shear force (ULS: $1.2*D+1.6*L_r+1.6*L_t+0.5*S$)

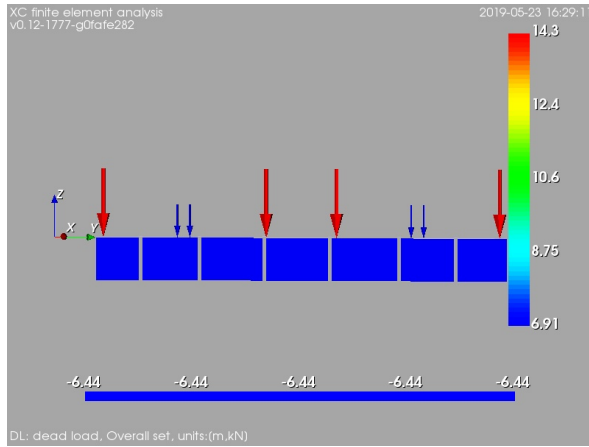


Figure 7: Sect. B-B. D: dead load

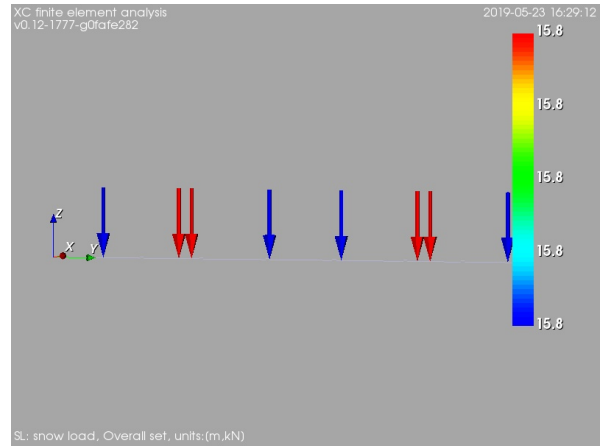


Figure 10: Sect. B-B. L_r : snow load

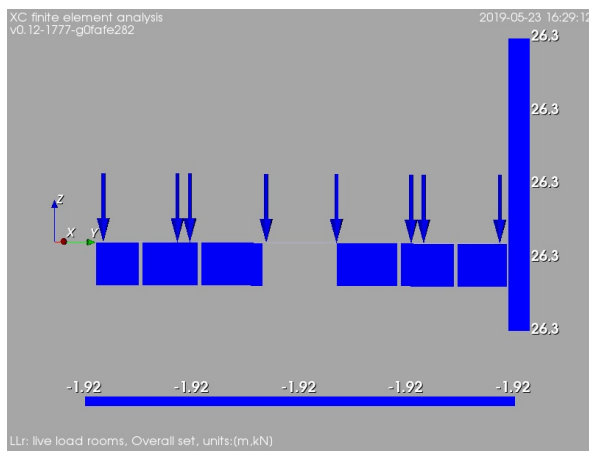


Figure 8: Sect. B-B. L_r : live load rooms

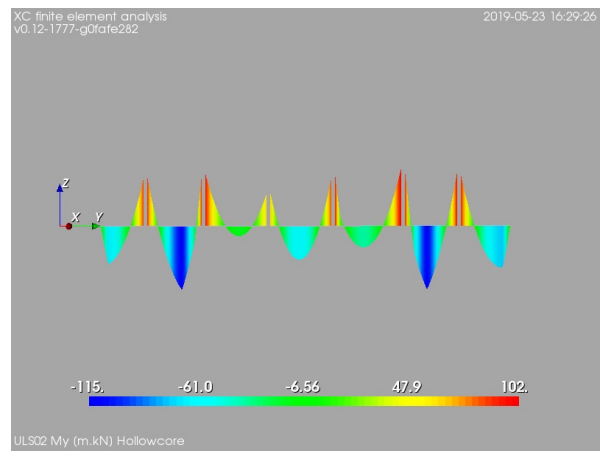


Figure 11: Sect. B-B. Maximum bending moment (ULS: $1.2 \cdot D + 1.6 \cdot L_r + 1.6 \cdot L_t + 0.5 \cdot S$)

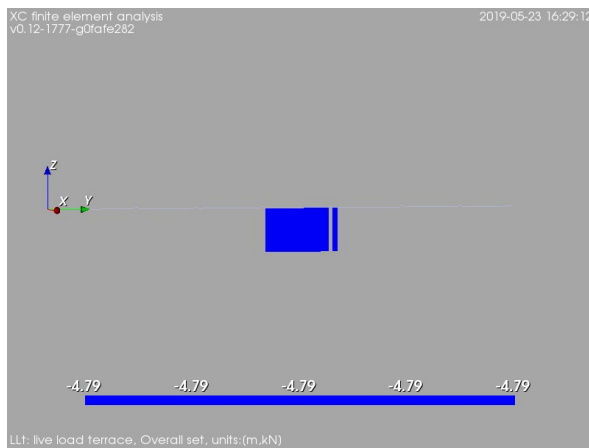


Figure 9: Sect. B-B. L_t : live load terrace

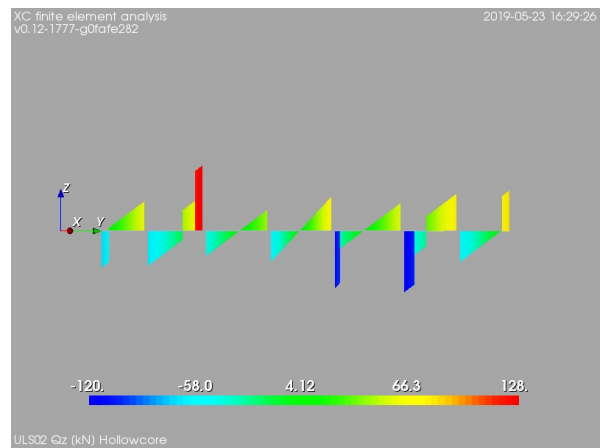


Figure 12: Sect. B-B. Maximum shear force (ULS: $1.2 \cdot D + 1.6 \cdot L_r + 1.6 \cdot L_t + 0.5 \cdot S$)