CURRENT CODES

- INTERNATIONAL BUILDING CODE 2015
 ASCESTANDARD ASCESSES 7-10 MINIMUM DE
- ASCE STANDARD ASCE/SEI 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES

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 ASCE STANDARD ASCE STRUCTURES FOR BUILDINGS FOR BUILD
- 2012 NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION
 NATIONAL DESIGN SPECIFICATION (NDS) SUPPLEMENT: DESIGN VALUES FOR WOOD CONSTRUCTION 2015
- ACI 318-11: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY
 530/530.1-11: BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTURES AND RELATED COMMENTARIES

1-LIVE LOAD	
DESIGN - LOADS & CRITERIA	
 AWS D1.1/D1.1M:2015 STRUCTURAL WELDING CODE - STEEL ANSI/AISC 360-10 - SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS 	

DESIGN - LOADS & CRITERIA			
1 111/5 1 0 4 D			
1-LIVE LOAD FLOOR FRAMING (RETAIL, OFFICE, RESTAURANT, RECREATIONAL)	100 PSF		
FLOOR FRAMING (RESIDENTIAL AREAS)	40 PSF		
STAIRWAYS, EXITS	100 PSF		
BALCONIES	75 PSF		
PRIVATE GARAGES (PASSENGER VEHICLES ONLY)	40 PSF		
INTERIOR PARTITION WALLS (UNIFORMLY DISTRIBUTED WEIGHT)	15 PSF		
CORRIDORS FIRST FLOOR	100 PSF		
CORRIDORS 2ND AND 3RD FLOORS	40 PSF		
CORNICES	60 PSF		
2-SNOW LOAD			
DESIGN SNOW LOAD	42 PSF (BALANCED)		
(pF) FLAT ROOF SNOW LAOD (pf = 0.7CeCtlspg)	42 PSF		
(Ce) SNOW EXPOSURE FACTOR	1.0		
(Is) SNOW LOAD IMPORTANCE FACTOR	1.0		
(Ct) ROOF THERMAL FACTOR	1.0		
(Pg) GROUND SNOW	60 PSF		
(Cs) SLOPED ROOF FACTOR	1.0		
3-WIND LOADS			
(Iw) WIND IMPORTANCE FACTOR	1.0		
RISK CATEGORY II			
BASIC WIND SPEED (3-SEC. GUST, ULTIMATE)	115 MPH		
BASIC WIND SPEED (3-SEC. GUST, NOMINAL)	90 MPH		
MEAN ROOF HEIGHT	33FT		
WIND EXPOSURE CATEGORY	В		
WIND EXPOSURE CLASSIFICATION	ENCLOSED		
(Kz) VELOCITY EXPOSURE COEFFICIENT	0.720		
(Kzt) TOPOGRAPHIC FACTOR	1.0		
DESIGN PROCEDURE	METHOD 1 (SIMPLIED)		
4-EARTHQUAKE LOADS			
OCCUPANCY CATEGORY	ll ll		
(Ie) SEISMIC IMPORTANCE FACTOR	1		
(Ss) SHORT PERIOD SPECTRAL ACCELERATION	0.045 g		
SPECTRAL RESPONSE ACCELERATIONS SS, AT SHORT PERIODS	0.038 g		
SITE CLASSIFICATIONS	В		
(Fa) SITE COEFFICIENT	1.0		
(Fv) SITE COEFFICIENT	1.0		
(Sds) DESIGN SPECTRAL RESPONSE ACCELERATION - SHORT PERIODS	0.030 g		
(Sd1) DESIGN SPECTRAL RESPONSE ACCELERATION - AT 1 SEC. PERIODS	0.025 g		
SEISMIC DESIGN CATEGORY	A		
SEISMIC-FORCE-RESISTING SYSTEM	WOOD STRUCTURAL PANEL (WSP)		
	SHEATHED SHEAR WALLS		
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE PROCEDURE (ELFP)		
5-SOIL			
CONTINUE WEIGHT			
SOIL UNIT WEIGHT	125 PCF (ASSUMED)		
LATERAL EARTH PRESSURE - AT REST (BASEMENT WALLS)			
	125 PCF (ASSUMED) 62.5 FPS/FT OF DEPTH (ASSUMED) 340 PSF (ASSUMED)		
LATERAL EARTH PRESSURE - AT REST (BASEMENT WALLS)	62.5 FPS/FT OF DEPTH (ASSUMED)		
LATERAL EARTH PRESSURE - AT REST (BASEMENT WALLS) LATERAL EARTH PRESSURE - PASSIVE	62.5 FPS/FT OF DEPTH (ASSUMED) 340 PSF (ASSUMED)		

MATERIAL STRENGTHS	
CONCRETE - EXT. SLABS ON GRADE	
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	YES 4% to 6%
CONCRETE - FOOTINGS MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 3,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.59
MAX. AGGREGATE SIZE	1 1/2"
SLUMP LIMIT	5" +/-1"
AIR CONTENT	NO
CONCRETE - INTERIOR SLABS ON GRADE	
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	NO
CONCRETE - PIERS, WALLS, AND COLUMNS	
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	YES 4% to 6%
CONCRETE - SLURRY	
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 1,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.55
MAX. AGGREGATE SIZE	1 1/2"
SLUMP LIMIT	6" +/-1"
AIR CONTENT CONCRETE - STAIR LANDING/TREADS	NO
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	NO NO
CONCRETE - TOPPING	
MIN. COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAX. WATER-CEMENTITIOUS RATIO	0.48
MAX. AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	NO
STEEL - REINFORCING STEEL	
ALL ASTM A615, GRADE 60, DEFORMED	Fy = 60,000 PSI
WELDED WIRE REINFORCEMENT, FLAT SHEETS	Fy = 60,000 PSI
STEEL - STRUCTURAL BOLTS	
HIGH STRENGTH BOLTS, NUTS, & WASHERS	ASTM A325
ZINC-COATED HIGH STRENGTH BOLTS, NUTS, & WASHERS	ASTM A325
STAINLESS STEEL BOLTS, NUTS, & WASHERS	ASTM F593
SHEAR CONNECTORS (GRADES 1015 THRU 1020)	ASTM A108
THREADED RODS	ASTM A36
CLEVIS & TURNBUCKLES (GRADE 1035)	ASTM A108
EYE BOLTS & NUTS (GRADE 1030)	ASTM A108
ANCHOR BOLTS (GRADE 36)	ASTM F1554
STEEL - STRUCTURAL STEEL	F FO 000 PC!
ROLLED WIDE FLANGE SHAPES, ASTM A992 GRADE 50	Fy = 50,000 PSI
CHANNELS, ANGLES, AND S SHAPES, ASTM A36	Fy = 36,000 PSI
PLATE AND BAR, ASTM A500 CRADE B	Fy = 36,000 PSI
TUBE SHAPES, ASTM A500 GRADE B PIPE ASTM A53, TYPE E or S, GRADE B	Fy = 46,000 PSI
FIFE WALM WAS TIEFFOLD GRADER	Fy = 46,000 PSI

DEFLECTION LIMITS				
DESCRIPTION	LIVE LOADS	SNOW OR DEAD LOADS		

RIGID MATERIALS (BRICK/MASONRY)	N/A	L/600	N/A
FLEXIBLE MATERIALS	N/A	L/360	N/A
FLOOR			
RIGID MATERIALS (BRICK/MASONRY)	L/600	L/600	L/600
WITH GYPSUM BOARD CEILINGS	L/540	N/A	L/360
FLEXIBLE MATERIALS	L/540	N/A	L/360
LINTELS, HEADER, AND/OR BEAM			
RIGID MATERIALS (BRICK/MASONRY)	L/600	L/600	L/600
FLEXIBLE MATERIALS	L/360	L/360	L/240
ROOF			
WITH GYPSUM BOARD CEILINGS	L/360	L/360	L/240
FLEXIBLE CEILINGS	L/360	L/360	L/240
NO CEILING	L/240	L/240	L/180
RIGID MATERIALS (BRICK/MASONRY)	L/600	L/600	L/600

NET PRESSURE	COEFFICIENTS Cnet	
AREA	C _{net} + INTERNAL PRESSURE	C _{net} - INTERNA PRESSURE
WINDWARD WALL	0.43	0.73
LEEWARD WALL	-0.51	-0.21
SIDEWALL	-0.66	-0.35
PARAPET WINDWARD WALL	1.	28
PARAPET LEEWARD WALL	-0.85	
FLAT ROOF	-1.09	-0.79

NET PRESSURE	COEFFICIENTS P _{net}	
AREA	P _{net} + INTERNAL PRESSURE	P _{net} - INTERNA PRESSURE
WINDWARD WALL	10.5 psf	17.8 psf
LEEWARD WALL	-12.4 psf	-5.1 psf
SIDEWALL	-16.1 psf	-8.5 psf
PARAPET WINDWARD WALL	31.2 psf	
PARAPET LEEWARD WALL	-2	0.7 psf
FLAT ROOF	-26.6 psf	-19.3 psf

FOOTINGS, FOUNDATIONS AND EARTHWORK:

DO NOT POUR ANY FOOTINGS ON FROZEN SOILSALL FOOTINGS POURED BELOW LOCAL FROST LINE

• EXPOSED BEARING STRATUM SHOULD BE VERIFIED BY CONTRACTOR AND SOILS ENGINEER TO SATISFY NET ALLOWABLE SOIL BEARING CAPACITY AS OUTLINED IN THESE CONSTRUCTION DOCUMENTS, AS WELL AS THE PROJECT'S GEO-TECHNICAL REPORT. IF EXCAVATED BEARING SOILS FALL BELOW OUTLINED CAPACITIES, CONTINUE EXCAVATION UNTIL BEARING CAPACITIES ARE MET. EXTEND FOOTINGS DOWN TO NEW BEARING ELEVATION, OR USE ENGINEERED FILL TO MAKE UP THE DIFFERENCE. ALL ENGINEERED FILL TO BE DESIGNED BY SOILS ENGINEER AND TO BE FREE OF DRAINING GRANULAR MATERIAL COMPACTED TO 95% OF MODIFIED PROCTOR DENSITIES.

• SEE DESIGN LOADS AND CRITERIA FOR DETAILS RELATIVE TO ASSUMED SOIL CONDITIONS, AND GEO-TECHNICAL

RECOMMENDATIONS. COORDINATE ANY SOIL CAPACITIES THAT FALL BELOW THOSE OUTLINED WITH THE STRUCTURAL ENGINEER.

WHEN APPLICABLE, BACK-FILLING SHOULD BE DONE SIMULTANEOUSLY ON BOTH SIDE OF A CONCRETE FOUNDATION
 WALL

DO NOT PLACE BACK FILL AGAINST BASEMENT WALLS UNTIL 1ST FLOOR FRAMING SYSTEM IS IN PLACE.
SHORING AND UNDERPINNING DESIGNED TO PREVENT 1/4" MAX MOVEMENT IN EITHER THE VERT/HORZ. DIRECTION
ALL BACK FILL WITHIN 3FT OF FOUNDATION WALLS TO BE GRANULAR FILL APPROVED BY THE PROJECT'S SOIL'S ENGINEER AND COMPACTED TO 90% OF STANDARD PROCTOR DENSITY

REBAR CONTINUITY:

UNLESS OTHERWISE NOTED, ALL REINFORCING SHOULD BE CONTINOUS. CONTACT LAPSE SLICING TO BE USED AT CORNERS AND INTERSECTION - SEE TYPICAL DETAIL. ALL SPLICES TO BE CLASS B SPLICES SHOP DRAWINGS SHOULD SHOW THE LOCATION AND OVERLAP DISTANCE OF ALL LAP SPLICES.

REBAR CONTINUITY								
REBAR LOCATION	#3	#4	#5	#6	#7	#8	#9	#10
3,000 - 3,500 PSI								
-/								
TOP BARS	22"	20"	34"	47"	70"	94"	118"	150
TOP BARS OTHER BARS	22" 17"	20" 22"	34" 28"	47" 34"	70" 55"	94" 71"	118" 115"	
						, -		
OTHER BARS						, -		150' 143'

TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THE REINFORCEMENT. TABLE ABOVE DOES NOT APPLY TO EPOXY COATED REBAR

MECHANICAL CONNECTIONS CAN BE USED IN PLACE OF LAP SPLICES. SHOP DRAWING, WITH APPROPROATE ICC-ES REPORT, TO BE SUBMITTED TO ARCHITECT AND ENGINEER FOR REVIEW. CONNECTIONS SHALL DEVELOP 125 PERCENT OF THE SPECIFIED YIELD STRIGHTH OF THE BAR IN TENSION.















Structural Engineer of Record: Ennovation 4729 Dale-Curtain Dr, McFarland, WI 53558 kfrey@ennovationbuilt.com





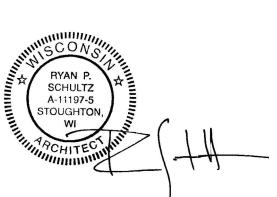
ELECTRICAL
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E8403 State Rd 85 | Mondovi, WI 54755



bhalgren@prismdesign-electrical.com | 715.797.0602

Plumbing Engineer: TAILORED ENGINEERING 1600 Aspen Commons | Ste 210 | Middleton, WI 53562 bnovak@tailoredeng.com | 608.209.7500



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Date	Description
07.08.2019	Footing/Foundation Permit
08.21.2019	Permit
05.15.2020	Permit Revision #4
07.02.2020	Permit Revision #5