DESIGN DATA

APPLICABLE CODES/STANDARDS:
.....INTERNATIONAL BUILDING CODE - 2018
.....ASCE 7-16 MIN DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE/SEI

STRUCTURAL DESIGN STANDARDS (DESIGN SHALL CONFORM TO THE CURRENT EDITION UNDER THE APPLICABLE CODE)
.....ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY
.....ACI 530/530.1 BLDG CODE REQUIREMENTS AND SPECS FOR MASONRY STRUCTURES (AND RELATED COMMENTARIES)
.....ANSI/AISC 360-16 SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS
.....AWS D1.1/D1.1M STRUCTURAL WELDING CODE-STEEL

BUILDING DESIGN LOADS/CRITERIA

DESIGN LIVE LOADS:

.....FLOOR FRAMING (RETAIL, OFFICE, RESTAURANT, RECREATIONAL)	100 psf
.....FLOOR FRAMING (RESIDENTIAL AREAS)	40 psf
.....STAIRWAYS, EXITS	100 psf
.....BALCONIES	40 psf
.....PRIVATE GARAGES (PASSENGER VEHICLES ONLY)	40 psf
.....INTERIOR PARTITION WALLS (UNIFORMLY DISTRIBUTED WEIGHT)	15 psf
.....CORRIDORS FIRST FLOOR	100 psf
.....CORRIDORS 2nd 3rdFLOORS	40 psf
.....CORNICES	60 psf

40 PSF IS THE CODE REQUIRED LIVE LOAD FOR BALCONIES. HOWEVER FOR THIS CLIMATE AREA I LIKE TO USE A LIVE LOAD OF 75 PSF. THIS CAN ACCOUNT FOR SNOW ACCUMULATION AND/OR OVERLOAD CONDITIONS THAT CAN HAPPEN ON BALCONIES

SNOW LOADS & DESIGN DATA:

.....DESIGN SNOW LOAD	42 psf (BALANCED SNOW LOA
.....FLAT ROOF SNOW LOAD (Pf) = (0.7*Ce*Ct*Is*Pg)	42 psf
.....SNOW EXPOSURE FACTOR (Ce)	1.0
.....SNOW LOAD IMPORTANCE FACTOR (Is)	1.0
.....ROOF THERMAL FACTOR (Ct)	1.0
.....GROUND SNOW (Pg)	60 psf
.....SLOPED ROOF FACTOR (Cs)	1.0

WIND DESIGN DATA:

.....WIND IMPORTANCE FACTOR (Iw)	1.0
.....RISK CATEGORY	II
.....BASIC WIND SPEED (3-SECOND GUST, ULTIMATE)	115 MPH
.....BASIC WIND SPEED (3-SECOND GUST, NOMINAL)	90 MPH
.....MEAN ROOF HEIGHT	33 FT
.....WIND EXPOSURE CATEGORY	B
.....WIND EXPOSURE CLASSIFICATION	ENCLOSED
.....VELOCITY EXPOSURE COEFFICIENT Kz	0.720
.....TOPOGRAPHIC FACTOR (Kzt)	1.0
.....DESIGN PROCEDURE	METHOD 1 (SIMPLIFIED PROCEDURE)

NET PRESSURE COEFFICIENTS C_{net}		
AREA	C_{+} INTERNAL $_{net}$ PRESSURE	C_{-} INTERNAL $_{net}$ PRESURE
WINDWARD WALL	0,43	0,73
LEEWARD WALL	-0,51	-0,21
SIDEWALL	-0,66	-0,35
PARAPET WINDWARD WALL	1,28	
PARAPET LEEWARD WALL	-0,85	
FLAT ROOF	-1,09	-0,79

DESIGN WIND PRESSURES P_{net}		
AREA	P_{+} INTERNAL $_{net}$ PRESSURE	P_{-} INTERNAL $_{net}$ PRESURE
WINDWARD WALL	10.5 psf	17.8 psf
LEEWARD WALL	-12.4 psf	-5.1 psf
SIDEWALL	-16.1 psf	-8.5 psf
PARAPET WINDWARD WALL	31.2 psf	
PARAPET LEEWARD WALL	-20.7 psf	
FLAT ROOF	-26.6 psf	-19.3 psf

EARTHQUAKE DESIGN DATA:

.....OCCUPANCY CATEGORY	II
.....SEISMIC IMPORTANCE FACTOR (Ie)	1
.....MAPPED SPECTRAL ACCELERATIONS AT SHORT PERIODS (Ss)	0.045 g
.....MAPPED SPECTRAL ACCELERATIONS AT (1) SECOND PERIODS (S1)	0.038 g
.....SITE CLASSIFICATIONS	B
.....SITE COEFFICIENT (Fa)	1,0
.....SITE COEFFICIENT (Fv)	1,0
.....DESIGN SPECTRAL RESPONSE COEFFICIENT AT SHORT PERIODS (Sds)	0.030 g
.....DESIGN SPECTRAL RESPONSE COEFFICIENT AT (1) SECOND PERIODS (Sd1)	0.025 g
.....SEISMIC DESIGN CATEGORY	A
.....BASIC SEISMIC-FORCE-RESISTING SYSTEM	LIGHT FRAME WOOD WALLS WITH STRUCTURAL WOOD SHEAR PANELS
.....ANALYSIS PROCEDURE FOR SEISMIC DESIGN	EQUIVALENT LATERAL FORCE ANALYSIS

SOIL DESIGN VALUES:

.....SOIL UNIT WEIGHT	125 PCF (ASSUMED)
.....LATERAL EARTH PRESSURE	
.....AT-REST (BASEMENT WALLS)	62,5 PSF/FT OF DEPTH (ASSUMED)
.....PASSIVE	340 PSF (ASSUMED)
.....COEFFICIENT OF SLIDING FRICTION	0.30 (ASSUMED)
.....SUBGRADE MODULUS	260 PCI (ASSUMED)
.....ALLOWABLE SOIL BEARING PRESSURE	3000 PSF

REFER TO SOILS REPORT NO. 17002 DATED 2/10/2017 PREPARED BY ITCO ALLIED ENGINEERING CO. FOR DESCRIPTION OF SOIL CONDITIONS, GEOTECHNICAL RECOMMENDATIONS, AND DESIGN VALUES

DEFLECTION LIMITS			
MEMBERS	LIVE	SNOW or WIND	DEAD + LIVE or SNOW
ROOF MEMBERS			
SUPPORTING GYPSUM BOARD CEILINGS	L/360	L/360	L/240
SUPPORTING FLEXIBLE CEILINGS	L/360	L/360	L/240
NOT SUPPORTING CEILING	L/240	L/240	L/180
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC.)	L/600	L/600	L/600
FLOOR MEMBERS			
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC.)	L/600	L/600	L/600
SUPPORTING GYPSUM BOARD CEILINGS	L/540	N/A	L/360
SUPPORTING FLEXIBLE MATERIALS	L/540	N/A	L/360
LINTEL/HEADER/BEAM MEMBERS			
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC.)	L/600	L/600	L/600
SUPPORTING FLEXIBLE MATERIALS (EIFS, SIDING, ETC.)	L/360	L/360	L/240
EXTERIOR WALLS			
WITH RIGID FINISHES (BRICK, MASONRY, ETC.)	N/A	L/600	N/A
WITH FLEXIBLE FINISHES (EIFS, SIDING, ETC.)	N/A	L/360	N/A

MATERIAL STRENGTHS

CAST-IN-PLACE CONCRETE:

FOOTINGS

MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 3,000 PSI
MAXIMUM WATER-CEMENTITIOUS RATIO	0.59
MAXIMUM AGGREGATE SIZE	1 1/2"
SLUMP LIMIT	5" +/-1"
AIR CONTENT	NO

TERIOR PIERS, WALLS, AND COLUMNS

MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAXIMUM WATER-CEMENTITIOUS RATIO	0.48
MAXIMUM AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	YES 4% to 6%

TERIOR SLABS ON GRADE

MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
MAXIMUM WATER-CEMENTITIOUS RATIO	0.48
MAXIMUM AGGREGATE SIZE	3/4"
SLUMP LIMIT	4" +/-1"
AIR CONTENT	NO

CONCRETE TOPPING

.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.48
.....MAXIMUM AGGREGATE SIZE	3/4"
.....SLUMP LIMIT	4" +/-1"
.....AIR CONTENT	NO

STAIR LANDINGS AND TREADS

.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.48
.....MAXIMUM AGGREGATE SIZE	3/4"
.....SLUMP LIMIT	4" +/-1"
.....AIR CONTENT	NO

EXTERIOR SLABS ON GRADE

.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 4,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.48
.....MAXIMUM AGGREGATE SIZE	3/4"
.....SLUMP LIMIT	4" +/-1"
.....AIR CONTENT	YES 4% to 6%

SLURRY

.....MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS	f'c = 1,000 PSI
.....MAXIMUM WATER-CEMENTITIOUS RATIO	0.55
.....MAXIMUM AGGREGATE SIZE	1 1/2"
.....SLUMP LIMIT	6" +/-1"
.....AIR CONTENT	NO

STEEL/METAL:

REINFORCING STEEL:

.....ALL ASTM A615, GRADE 60, DEFORMED	Fy = 60,000 PSI
.....STEEL WELDED WIRE REINFORCEMENT, FLAT SHEETS	Fy = 60,000 PSI

STRUCTURAL STEEL:

.....ROLLED WIDE FLANGE SHAPES, ASTM A992	GRADE 50 Fy = 50,000 PSI
.....CHANNELS, ANGLES, AND S SHAPES, ASTM A36	Fy = 36,000 PSI
.....PLATE AND BAR, ASTM A36	Fy = 36,000 PSI
.....TUBE SHAPES, ASTM A500 GRADE B	Fy = 46,000 PSI
.....PIPE ASTM A53, TYPE E or S, GRADE B	Fy = 46,000 PSI
.....ALL OTHER ROLLED SHAPES, ASTM A36	Fy = 36,000 PSI

STRUCTURAL BOLTS:

.....HIGH STRENGTH BOLTS, NUTS, & WASHERS	ASTM A325
.....ZINC-COATED HIGH STRENGTH BOLTS, NUTS, & WASHERS	ASTM A325
.....STAINLESS STEEL BOLTS, NUTS, & WASHERS	ASTM F593
.....SHEAR CONNECTORS (GRADES 1015 THRU 1020)	ASTM A108
.....THREADED RODS	ASTM A36
.....CLEVIS & TURNBUCKLES (GRADE 1035)	ASTM A108
.....EYE BOLTS & NUTS (GRADE 1030)	ASTM A108
.....ANCHOR BOLTS (GRADE 36)	ASTM F1554

WELDED CONNECTIONS:

.....WELDING ELECTRODES	E70XX E80XX FOR WELDING REINF

MASONRY: f'm = 2,000 PSI

MASONRY MORTAR:
.....TYPE "M" MORTAR BELOW GRADE
.....TYPE "M" or "S" ABOVE GRADE

GROUT BELOW BASE PLATES & BEARING PLATES:
.....NONMETALLIC, SHRINKAGE-RESISTANT ASTM C1107

FOUNDATION AND EARTHWORK:

1. ALL EXTERIOR FOOTINGS MUST BEAR BELOW LOCAL FROST LINE RELATIVE TO ADJACENT FINISH EXTERIOR GRADE.
2. DO NOT PLACE ANY FOOTINGS ON FROZEN SUBGRADE.
3. BACK FILLING SHALL BE DONE SIMULTANEOUSLY ON BOTH SIDES OF FOUNDATION WALLS.
4. DO NOT PLACE BACK FILL AGAINST BASEMENT WALLS UNTIL THE TOP AND BOTTOM OF THE WALL ARE ADEQUATELY BRACED BY THE SLAB ON GRADE AND THE FLOOR FRAMING AT THE TOP OF THE WALL.
5. REMOVE ANY EXISTING CONCRETE 2'-0" BELOW NEW CONCRETE FOOTINGS AND SLABS ON GRADE, UNLESS NOTED OTHERWISE.
6. SHORING/OR UNDERPINNING SHALL BE DESIGNED TO LIMIT HORIZONTAL AND VERTICAL MOVEMENT OF EXISTING CONSTRUCTION TO 1/4" MAXIMUM IN ANY DIRECTION.
7. CENTER PIER AND COLUMN FOOTINGS ON COLUMN CENTERLINES AND WALL FOOTINGS ON WALL CENTERLINES UNLESS SPECIFICALLY NOTED OTHERWISE.
8. ALL BACK FILL WITHIN 3'-0" OF RETAINING WALLS AND BASEMENT WALLS SHALL BE FREE DRAINING GRANULAR MATERIAL APPROVED BY A SOILS ENGINEER AND COMPACTED TO 90% STANDARD PROCTOR.
9. TOP OF FOOTING ELEVATIONS SHOWN ON THESE CONSTRUCTION DOCUMENTS REPRESENT MINIMUM FOOTING DEPTHS FOR FROST PROTECTION AND BEST JUDGMENT OF A SUITABLE BEARING STRATUM. ACTUAL GRADE CONDITIONS AND SUITABLE BEARING STRATUM MUST BE VERIFIED BY THE CONTRACTOR AND A SOILS ENGINEER AT THE TIME OF EXCAVATION.
10. FOOTING EXCAVATIONS MUST EXTEND TO COMPETENT BEARING MATERIAL. CONTRACTOR SHALL HIRE A SOILS ENGINEER TO FIELD VERIFY NET ALLOWABLE SOIL BEARING CAPACITY STATED ON THESE CONSTRUCTION DOCUMENTS AND IN GEOTECHNICAL REPORT FOR THIS PROJECT. IF SUITABLE BEARING STRATUM DOES NOT EXIST AT FOOTING ELEVATIONS STATED ON CONSTRUCTION DOCUMENTS, EXCAVATIONS SHALL BE EXTENDED UNTIL SOIL WITH STATED BEARING CAPACITY IS REACHED. PLACE COMPACTED FILL BELOW FOOTINGS OR EXTEND FOOTINGS DOWN TO SUITABLE BEARING STRATUM. ENGINEERED FILL BELOW SLABS ON GRADE AND FOOTINGS SHALL BE FREE DRAINING GRANULAR MATERIAL COMPACTED TO 95% MODIFIED PROCTOR AND PLACED PER THE SOIL ENGINEERS RECOMMENDATIONS. ALL FIELD CONDITIONS THAT WILL AFFECT DESIGN AS PRESENTED MUST BE COORDINATED WITH STRUCTURAL ENGINEER.
11. REFER TO DESIGN DATA FOR DESCRIPTION OF SOIL CONDITIONS, GEOTECHNICAL RECOMMENDATIONS, AND DESIGN VALUES.

CONTINUITY:

ALL REINFORCING SHALL BE CONTINUOUS UNLESS NOTED OTHERWISE. CONTINUITY AT CORNERS AND INTERSECTIONS SHALL BE ACHIEVED USING CORNER BARS AND CONTACT LAP SPLICES, SEE TYPICAL DETAIL. CONTINUITY AT OTHER LOCATIONS MAY BE ACHIEVED USING CONTACT LAP SPLICES SHOWN ON APPROVED SHOP DRAWINGS. LOCATION OF LAP SPLICES SHALL BE SHOWN ON THE SHOP DRAWINGS. UNLESS NOTED OTHERWISE, THE FOLLOWING LAP SPLICES SHALL BE USED: (ALL LAP SPLICES ARE CLASS B SPLICES)

LOCATION:	#3	#4	#5	#6	#7	#8	#9	#10	#11
3,000 & 3,500 PSI CONCRETE:									
- TOP BARS (*):	21"	19"	35"	46"	71"	93"	118"	149"	184"
- OTHER BARS:	16"	22"(**)	27"	35"	55"	71"	91"	115"	142"
4,000 & 4,500 PSI CONCRETE:									
- TOP BARS (*):	16"	19"	25"	36"	61"	80"	102"	129"	159"
- OTHER BARS:	16"	16"(**)	19"	28"	47"	62"	78"	99"	123"

(*) TOP BARS ARE HORIZONTAL REINFORCING WHERE MORE THAN 12" OF CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCING.

(**) FOR #4 EPOXY COATED REBAR, USE 27" SPLICE LENGTH AT 3,000 AND 3,500 PSI CONC. AND 19" AT 4,000 AND 4,500 PSI.

MECHANICAL CONNECTIONS MAY BE USED IN LIEU OF LAP SPLICES PROVIDED APPROVAL IS OBTAINED FROM THE ARCHITECT/ENGINEER. CONNECTIONS SHALL DEVELOP IN TENSION 125 PERCENT OF THE SPECIFIED YIELD STRENGTH OF THE BAR. ALL MECHANICAL CONNECTIONS SHALL BE SHOWN ON THE SHOP DRAWINGS AND BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS AND THE PRODUCT'S ICC-ES REPORT. SUBMIT THE PRODUCT'S ICC-ES REPORT FOR MECHANICAL SPLICE PRODUCTS WITH SHOP DRAWINGS.

TYPICALLY I WOULD SEE SOME LOADING INFORMATION AND PERFORMANCE CRITERIA FOR THE WOOD TRUSS DESIGNER. IT CAN BE PLACED ON THIS SHEET OR IT COULD BE PLACED ON THE SCHEDULE SHEET. THE TRUSS SUPPLIER NEEDS TOP CHORD DEAD LOAD AND BOTTOM CHORD DEAD LOADS NOTED. ALSO DEFLECTION CRITERIA IS USUALLY INCLUDED. DEFLECTION FOR FLOORS TYPICALLY IS LIVE LOAD < L/480 w/ TOTAL LOAD OF L/240. FOR ROOFS SNOW LOAD < L360 AND TOTAL LOAD OF L/240

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Ennovation Engineering Services Incorporated
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Date	Description
07.08.2019	Footing and Foundation Plan Permit

STRUCTURAL NOTES

CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

S000

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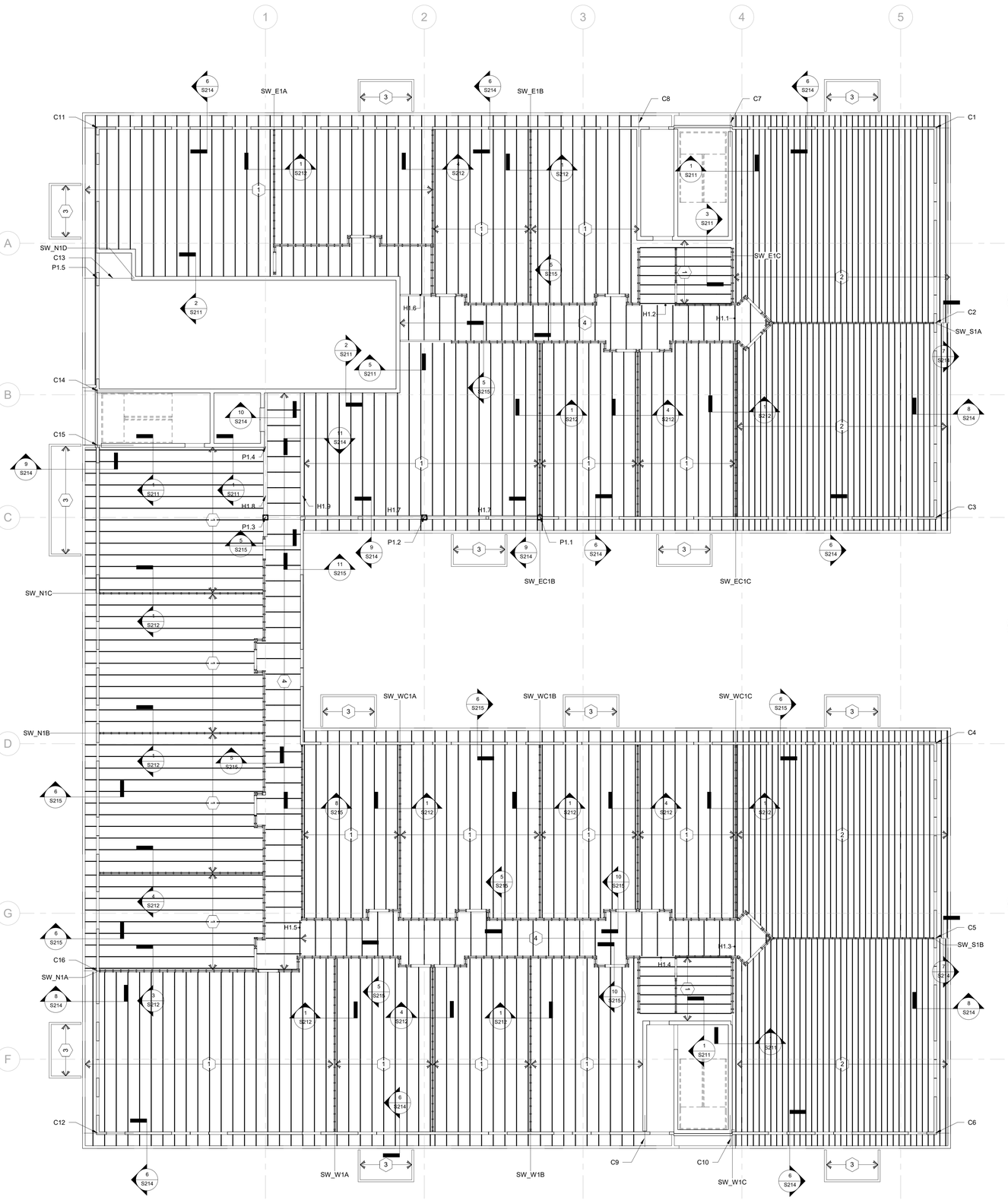
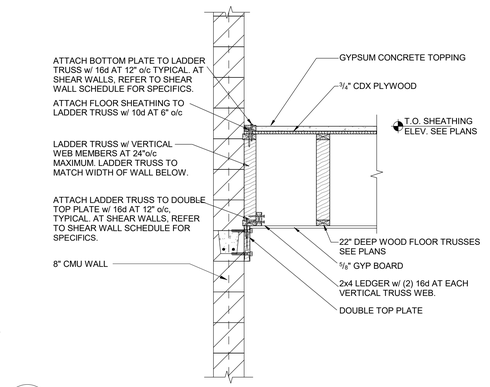
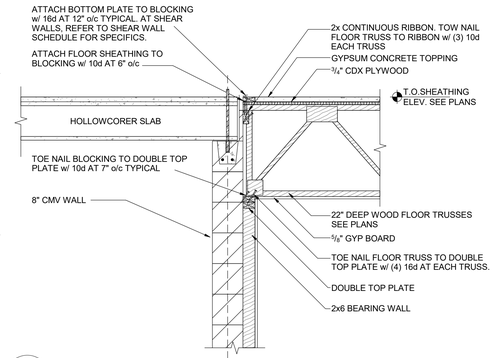


0 S211 FRAMING PLAN - 2ND FLOOR
1/8" = 1'-0"



1 S211 FLOOR FRAMING PARALLEL TO CMU WALL

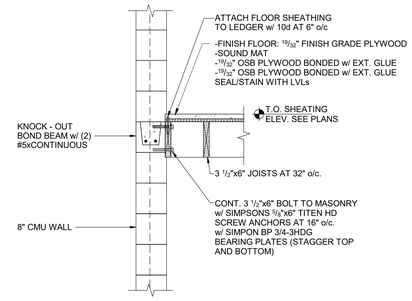


2 S211 FLOOR FRAMING/HOLLOWCORE SLAB

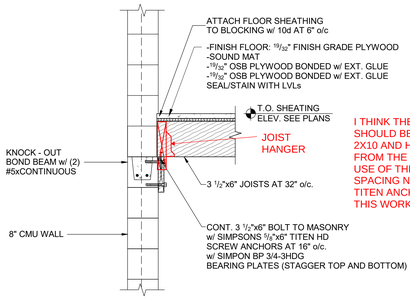
WOOD FLOOR PLAN KEYED NOTES:

1. 22" DEEP PARALLEL CHORD FLOOR TRUSSES AT 24" o/c.
2. 22" DEEP PARALLEL CHORD FLOOR TRUSSES AT 12" o/c.
3. PREFABRICATED SIDE HUNG METAL BALCONY, BY OTHERS
4. 3½"x6" JOISTS AT 32" o/c.

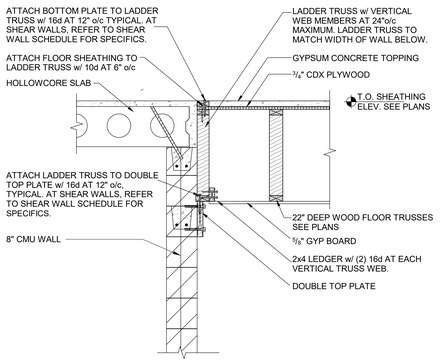
"SEE HEADER/POST SCHEDULE IN S216 SHEET"
"SEE SHEAR WALL SCHEDULE IN S216 SHEET"



3 S211 CORRIDOR JOISTS PARALLEL TO CMU WALL



4 S211 CORRIDOR JOISTS BEARING ON CMU WALL



5 S211 FLOOR FRAMING/HOLLOWCORE. SLAB

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Date	Description
08.15.2019	75% CD Set

FRAMING PLAN - 2ND FLOOR
CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

S211

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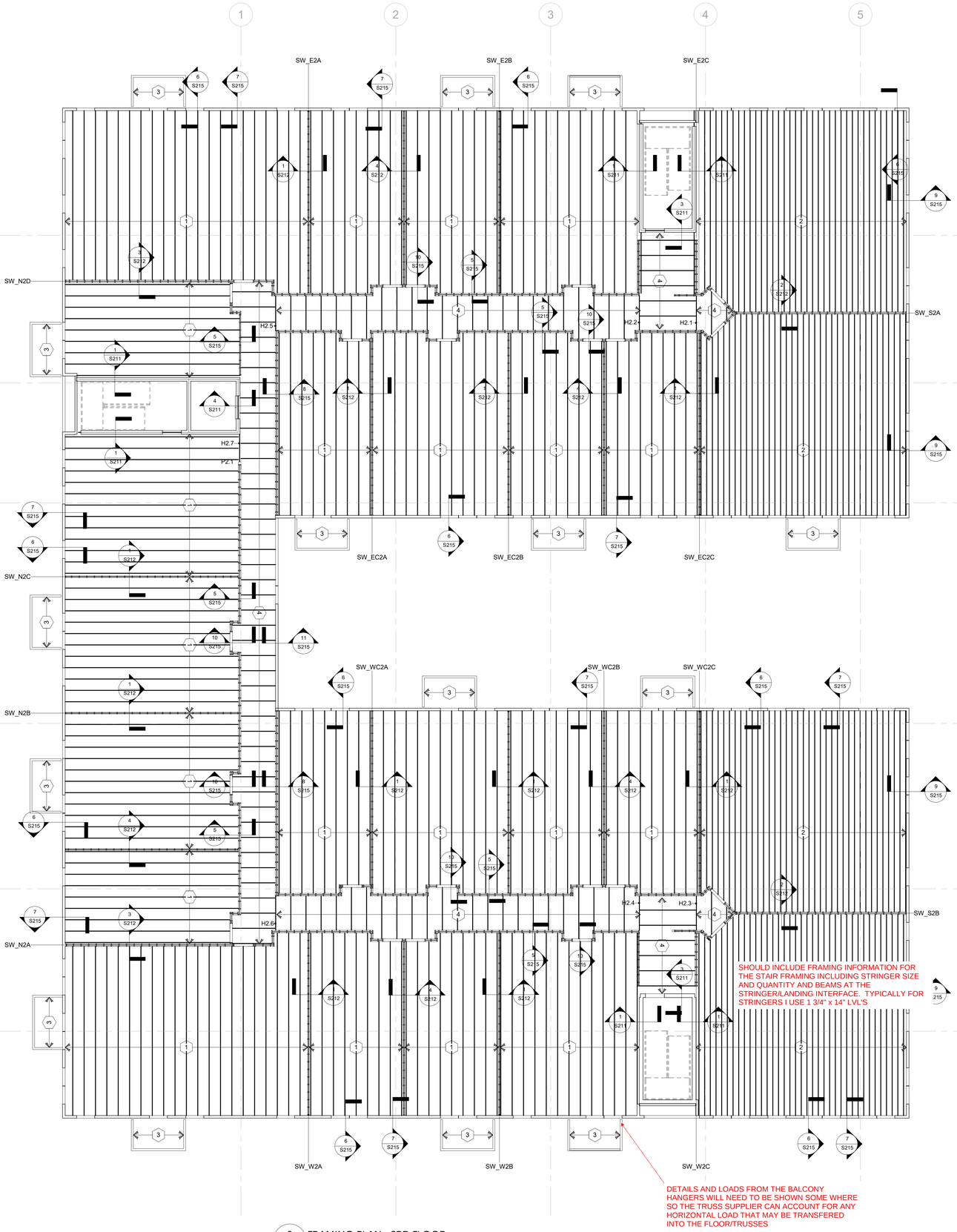
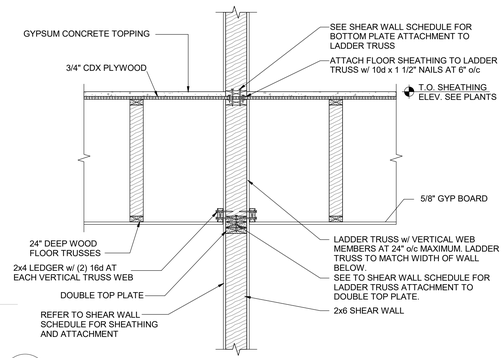


0 S212 FRAMING PLAN - 3RD FLOOR
1/8" = 1'-0"

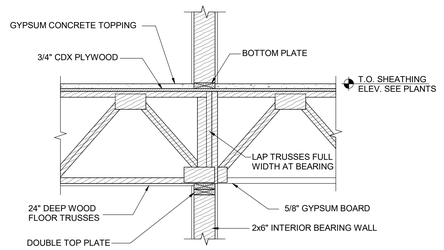


1 S212 FLOOR FRAMING. INTERIOR SHEAR WALL

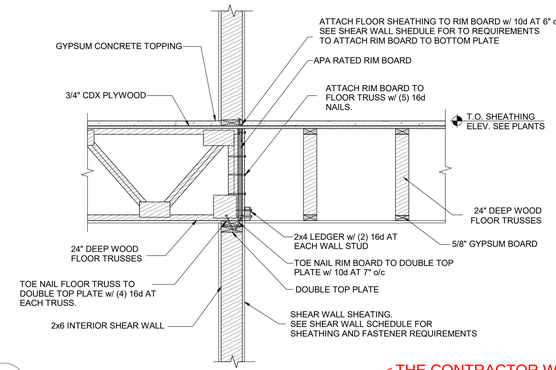
WOOD FLOOR PLAN KEYED NOTES:

1. 24" DEEP PARALLEL CHORD FLOOR TRUSSES AT 24" o/c.
2. 24" DEEP PARALLEL CHORD FLOOR TRUSSES AT 12" o/c.
3. PREFABRICATED SIDE HUNG METAL BALCONY, BY OTHERS
4. 3 1/2"x6" JOISTS AT 32" o/c.

"SEE HEADER/POST SCHEDULE IN S216 SHEET"
"SEE SHEAR WALL SCHEDULE IN S216 SHEET"

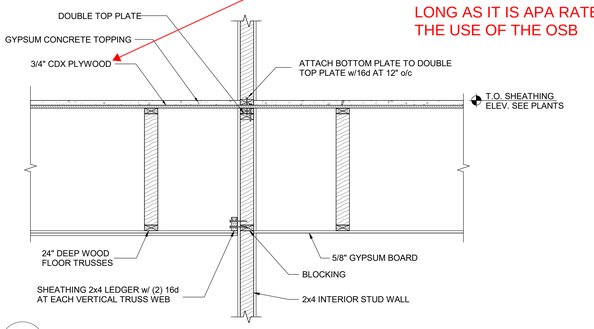


2 S212 FLOOR FRAMING. INTERIOR BEARING WALL



3 S212 FLOOR FRAMING INTERIOR SHEAR WALL

THE CONTRACTOR WILL PROBABLY REQUEST IF THEY CAN USE OSB SHEATING IN LIEU OF THE PLYWOOD. THE OSB USE IS PRETTY TYPICAL IN THIS AREA. AS LONG AS IT IS APA RATED I ALLOW THE USE OF THE OSB



4 S212 FLOOR FRAMING. INTERIOR PARTITION WALL

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Structural Engineer: Structural Engineering
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Structural Engineer: Structural Engineering
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kfrey@ennovationbuilt.com

Mechanical Engineer: HOVLAND'S HVAC
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jhansen@hovlands-inc.com | 715.552.5595

Electrical Engineer: PRISM DESIGN ELECTRICAL CONSULTANS INC
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bhalgren@prismdesign-electrical.com | 715.797.0602

Plumbing Engineer: TAILORED ENGINEERING
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bnovak@tailoredeng.com | 608.209.7500

Date	Description
08.15.2019	75% CD Set

FRAMING PLAN - 3RD FLOOR
CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

S212

14/08/2019 16:50:45

SHOW ROOF TRUSS FRAMING OVER STAIR TOWERS

SHOW ROOF TRUSS FRAMING OVER STAIR TOWERS

THE ELEVATOR/CONTRACTOR WILL BE LOOKING FOR AN ELEVATOR HOIST BEAM. I TYPICALLY USE W8x24 FOR THIS TYPE

SW_E3A SW_E3B SW_E3C SW_S3A SW_N3D SW_EC3A SW_EC3B SW_EC3C SW_N3C

P3.1 H3.1 P3.2 H3.8 H3.5 H3.4 H3.10 P3.7

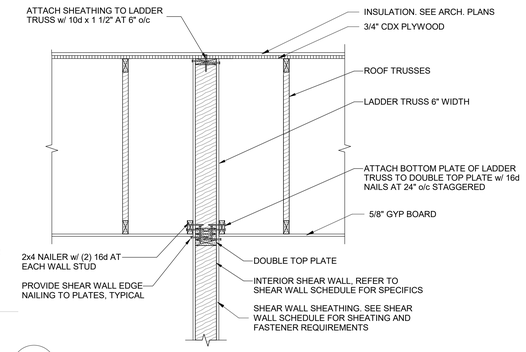
SW_WC3A SW_WC3B SW_WC3C SW_N3B SW_S3B SW_N3A SW_W3A SW_W3B SW_W3C

P3.3 H3.2 P3.4 H3.9 H3.7 H3.6 P3.5 H3.3 P3.6

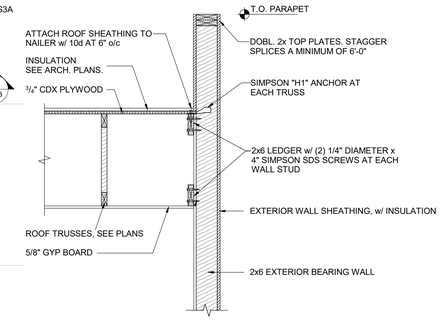
FOR THE ROOF FRAMING I THINK THE TRUSSES CAN BE SPACED AT 24" O.C. IN LIEU OF THE 12" O.C. NOTED

SHOW ROOF TRUSS FRAMING OVER STAIR TOWERS

0 S213 FRAMING PLAN - ROOF
1/8" = 1'-0"

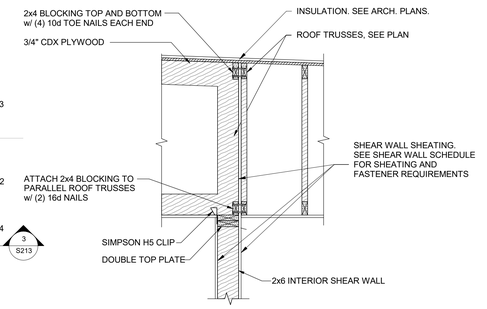


1 S213 ROOF FRAMING. INTERIOR BEARING WALL

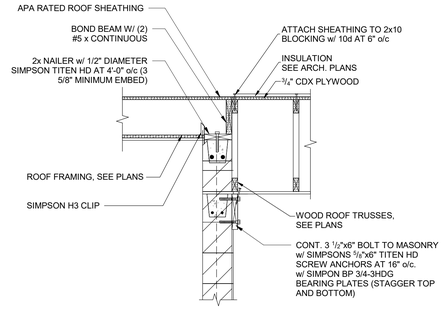


3 S213 ROOF FRAMING. EXTERIOR BEARING WALL

TOE NAILS EACH END



5 S213 ROOF FRAMING. INTERIOR SHEAR WALL



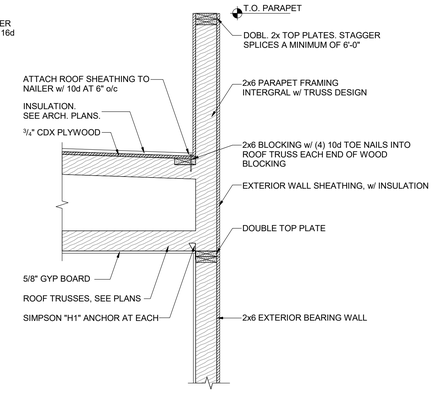
7 S213 ROOF FRAMING. CMU STAIR WALL

WOOD FLOOR PLAN KEYED NOTES:

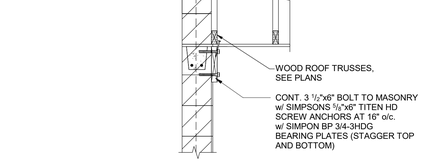
1. VARIABLE DEPTH PARALLEL CHORD FLOOR TRUSSES AT 24" o/c.
2. VARIABLE DEPTH PARALLEL CHORD FLOOR TRUSSES AT 12" o/c.
3. ~~PREFABRICATED SIDE HUNG METAL BALCONY, BY OTHERS~~
4. 3½"x6" JOISTS AT 32" o/c.

"SEE HEADER/POST SCHEDULE IN S216 SHEET"
"SEE SHEAR WALL SCHEDULE IN S216 SHEET"

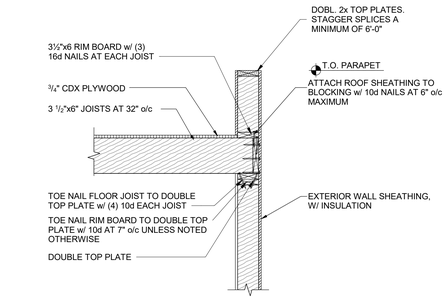
CHANGE REFERENCE TO FLOORS AND USE ROOF



2 S213 ROOF FRAMING. EXTERIOR BEARING WALL



4 S213 ROOF FRAMING. CMU WALL



6 S213 ROOF FRAMING. JOISTS ON EXTERIOR BEARING WALL

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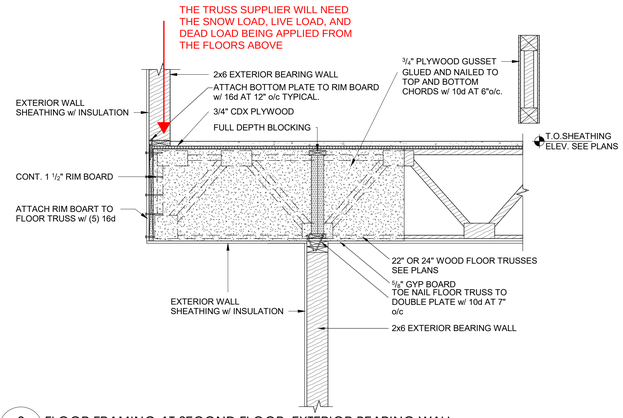
Date	Description
08.15.2019	75% CD Set

FRAMING PLAN - ROOF

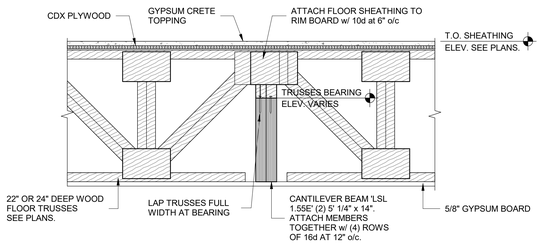
CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

S213

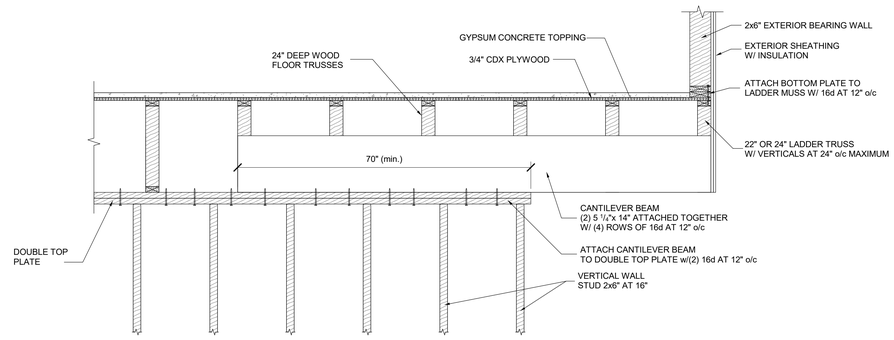
14/08/2019 16:50:53



6 S214 FLOOR FRAMING AT SECOND FLOOR. EXTERIOR BEARING WALL



7 S214 CANTILEVER. SECTION

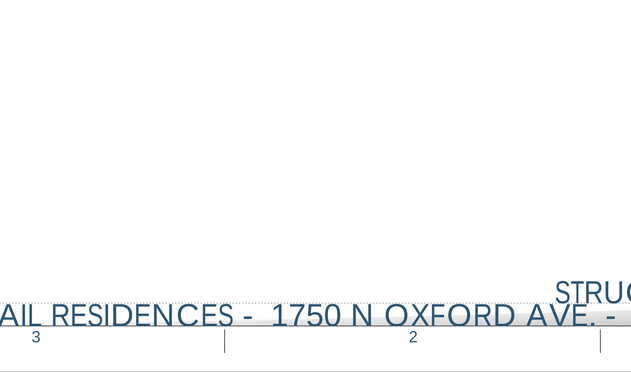


8 S214 CANTILEVER. LATERAL VIEW

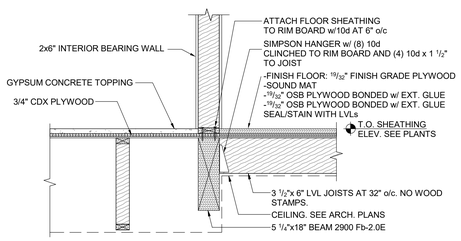
9 S214 FLOOR FRAMING AT LOBBY. COURTYARD STEEL BEAM

C380x60 DETAIL

USE U.S. STANDARD DESIGNATION FOR THE CHANNELS



10 S214 FLOOR FRAMING AT LOBBY. CORRIDOR STEEL BEAM



11 S214 FLOOR FRAMING AT LOBBY. INTERIOR BEARING WALL

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ROYAL CONSTRUCTION
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Cedar corporation
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Date	Description
08.15.2019	75% CD Set

STRUCTURAL DETAILS

CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

S214

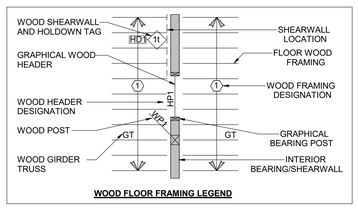
14/08/2019 16:50:59

WOOD SHEAR WALL SCHEDULE

Shear wall ID	Sheathing material	Panel thickness (in)	Blocking	Minimum fastener penetration in framing member or blocking (in)	Fastener type and size	Panel edge fastener spacing (in)	Nominal unit shear capacity v_n (plf)	Hold-down anchor capacity (klp)	Number of bolts (1 in. diameter, 4 inch embedment depth)	Bolt spacing (in)	Bottom plate attachment (foundation) / Bottom plate attachment (floor to floor)
SW_N3A	Wood structural panels – sheathing	3/8	YES	1-3/8	8d	4	840	2	-	-	wood screws 20 (d= 0.32 in) at 25 in. o/c; 30 fasteners in 2 rows.
SW_N3B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	-	-	16d (d= 0.268 in) nails at 24 in. o/c; 16 fasteners in 1 row.
SW_N3C	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	-	"	16d (d= 0.268 in) nails at 21 in. o/c; 35 fasteners in 2 rows.
SW_N3D	Wood structural panels – sheathing	3/8	YES	1-3/8	8d	4	840	2	-	-	wood screws 20 (d= 0.32 in) at 25 in. o/c; 30 fasteners in 2 rows.
SW_N2A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	4	1430	4	-	-	wood screws 20 (d= 0.32 in) at 14 in. o/c; 52 fasteners in 2 rows.
SW_N2B	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	-	-	-	16d (d= 0.268 in) nails at 13 in. o/c; 28 fasteners in 1 row.
SW_N2C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	1	-	-	16d (d= 0.268 in) nails at 12 in. o/c; 59 fasteners in 2 rows.
SW_N2D	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	4	1430	4	-	-	wood screws 20 (d= 0.32 in) at 14 in. o/c; 52 fasteners in 2 rows.
SW_N1A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	7	10	36	SDWS log screw (d= 0.197 in) at 12 in. o/c; 58 fasteners in 2 rows.
SW_N1B	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	-	11	36	16d (d= 0.268 in) nails at 19 in. o/c; 39 fasteners in 2 rows.
SW_N1C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	3	11	36	wood screws 20 (d= 0.32 in) at 19 in. o/c; 40 fasteners in 2 rows.
SW_N1D	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	7	10	36	SDWS log screw (d= 0.197 in) at 12 in. o/c; 60 fasteners in 2 rows.
SW_S3A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	2	-	-	wood screws 20 (d= 0.32 in) at 21 in. o/c; 36 fasteners in 2 rows.
SW_S3B	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	2	-	-	wood screws 20 (d= 0.32 in) at 21 in. o/c; 36 fasteners in 2 rows.
SW_S2A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	6	-	-	SDWS log screw (d= 0.197 in) at 13 in. o/c; 54 fasteners in 2 rows.
SW_S2B	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	6	-	-	SDWS log screw (d= 0.197 in) at 13 in. o/c; 54 fasteners in 2 rows.
SW_S1A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	11	10	36	SDWS log screw (d= 0.197 in) at 8 in. o/c; 76 fasteners in 2 rows.
SW_S1B	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	11	10	36	SDWS log screw (d= 0.197 in) at 8 in. o/c; 76 fasteners in 2 rows.
SW_E3A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	4	1430	3	-	-	wood screws 20 (d= 0.32 in) at 16 in. o/c; 46 fasteners in 2 rows.
SW_E3B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	-	-	16d (d= 0.268 in) nails at 12 in. o/c; 30 fasteners in 1 row.
SW_E3C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	4	1430	6	-	-	SDWS log screw (d= 0.197 in) at 15 in. o/c; 32 fasteners in 2 rows.
SW_E2A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	7	-	-	SDWS log screw (d= 0.197 in) at 11 in. o/c; 64 fasteners in 2 rows.
SW_E2B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	1	-	-	16d (d= 0.268 in) nails at 14 in. o/c; 51 fasteners in 2 rows.
SW_E2C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	11	-	"	SDWS log screw (d= 0.197 in) at 9 in. o/c; 54 fasteners in 2 rows.
SW_E1A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	13	7	36	SDWS log screw (d= 0.197 in) at 7 in. o/c; 64 fasteners in 2 rows.
SW_E1B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	11	36	16d (d= 0.268 in) nails at 32 in. o/c; 12 fasteners in 1 row.
SW_E1C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	9	11	36	SDWS log screw (d= 0.197 in) at 10 in. o/c; 72 fasteners in 2 rows.
SW_W3A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	4	1430	3	-	-	wood screws 20 (d= 0.32 in) at 16 in. o/c; 46 fasteners in 2 rows.
SW_W3B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	-	-	16d (d= 0.268 in) nails at 12 in. o/c; 30 fasteners in 1 row.
SW_W3C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	4	1430	6	-	-	SDWS log screw (d= 0.197 in) at 15 in. o/c; 32 fasteners in 2 rows.
SW_W2A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	7	-	-	SDWS log screw (d= 0.197 in) at 11 in. o/c; 64 fasteners in 2 rows.
SW_W2B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	1	-	-	16d (d= 0.268 in) nails at 14 in. o/c; 51 fasteners in 2 rows.
SW_W2C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	11	-	-	SDWS log screw (d= 0.197 in) at 9 in. o/c; 54 fasteners in 2 rows.
SW_W1A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	13	9	30	SDWS log screw (d= 0.197 in) at 7 in. o/c; 64 fasteners in 2 rows.
SW_W1B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	11	36	16d (d= 0.268 in) nails at 32 in. o/c; 12 fasteners in 1 row.
SW_W1C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	9	11	36	SDWS log screw (d= 0.197 in) at 10 in. o/c; 72 fasteners in 2 rows.
SW_EC3A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	0	-	-	16d (d= 0.268 in) nails at 18 in. o/c; 42 fasteners in 2 rows.
SW_EC3B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	-	-	16d (d= 0.268 in) nails at 60 in. o/c; 7 fasteners in 1 row.
SW_EC3C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	3	-	-	wood screws 20 (d= 0.32 in) at 19 in. o/c; 40 fasteners in 2 rows.
SW_EC2A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	2	-	-	wood screws 20 (d= 0.32 in) at 21 in. o/c; 36 fasteners in 2 rows.
SW_EC2B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	-	"	16d (d= 0.268 in) nails at 32 in. o/c; 12 fasteners in 1 row.
SW_EC2C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	6	-	-	SDWS log screw (d= 0.197 in) at 12 in. o/c; 58 fasteners in 2 rows.
SW_EC1A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	11	6	36	SDWS log screw (d= 0.197 in) at 9 in. o/c; 42 fasteners in 2 rows.
SW_EC1B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	11	36	16d (d= 0.268 in) nails at 22 in. o/c; 17 fasteners in 1 row.
SW_EC1C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	11	11	36	SDWS log screw (d= 0.197 in) at 9 in. o/c; 82 fasteners in 2 rows.
SW_WC3A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	0	-	-	16d (d= 0.268 in) nails at 18 in. o/c; 42 fasteners in 2 rows.
SW_WC3B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	0	560	-	-	-	16d (d= 0.268 in) nails at 60 in. o/c; 7 fasteners in 1 row.
SW_WC3C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	6	950	3	-	-	wood screws 20 (d= 0.32 in) at 19 in. o/c; 40 fasteners in 2 rows.
SW_WC2A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	2	-	-	wood screws 20 (d= 0.32 in) at 21 in. o/c; 36 fasteners in 2 rows.
SW_WC2B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	-	-	16d (d= 0.268 in) nails at 32 in. o/c; 12 fasteners in 1 row.
SW_WC2C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	3	1860	6	-	-	SDWS log screw (d= 0.197 in) at 12 in. o/c; 58 fasteners in 2 rows.
SW_WC1A	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	11	6	36	SDWS log screw (d= 0.197 in) at 9 in. o/c; 42 fasteners in 2 rows.
SW_WC1B	Wood structural panels – sheathing	3/8	NO	1-3/8	8d	6	560	-	11	36	16d (d= 0.268 in) nails at 22 in. o/c; 17 fasteners in 1 row.
SW_WC1C	Wood structural panels – sheathing	19/32	YES	1-1/2	10d	2	2435	11	11	36	SDWS log screw (d= 0.197 in) at 9 in. o/c; 82 fasteners in 2 rows.

WOOD FLOOR PLAN NOTES:

1. TYPICAL FLOOR CONSTRUCTION: 1" GYPSUM CONCRETE TOPPING (120 PCF MAXIMUM DENSITY) ON 3/4" TONGUE & GROOVE APA RATED WOOD FLOOR SHEATHING (PLYWOOD OR OSB). GLUE & SCREW FLOOR SHEATHING TO WOOD FLOOR STRUCTURE. SHEATHING TO BE ATTACHED TO FLOOR MEMBERS w/ SIMPSON STRONG-TIE STRONG-DRIVE WSNTL FASTENERS ON A 6"/6" o/c PATTERN (EDGE/FIELD).
3. TYPICAL STAIR LANDING CONSTRUCTION: 3/4" TONGUE & GROOVE APA RATED WOOD FLOOR SHEATHING (PLYWOOD OR OSB). GLUE & SCREW FLOOR SHEATHING TO WOOD FLOOR STRUCTURE. SHEATHING TO BE ATTACHED TO FLOOR MEMBERS w/ SIMPSON STRONG-TIE STRONG-DRIVE WSNTL FASTENERS ON A 6"/6" o/c PATTERN (EDGE/FIELD).
4. REFER TO ARCHITECTURAL DRAWINGS FOR STAIR FRAMING AND CONFIGURATION.
5. "HPX" DENOTES A WOOD HEADER/POST CONSTRUCTION. REFER TO WOOD HEADER/POST SCHEDULE FOR HEADER & POST DESIGNATION.
6. "WPX" DENOTES A WOOD POST. REFER TO WOOD HEADER/POST SCHEDULE FOR WOOD POST DESIGNATION ONLY.
7. ALL EXTERIOR WOOD STUD WALLS SHALL HAVE (1) LAYER OF 1/2" APA RATED SHEATHING (PLYWOOD OR OSB) ON THE EXTERIOR WALL FACE. REFER TO STANDARD DETAILS FOR TYPICAL BEARING WALL CONSTRUCTION AND SHEATHING ATTACHMENT. IF WALL IS NOT SPECIFICALLY DESIGNATED AS A SHEAR WALL, ATTACH SHEATHING TO WALL STUDS w/ 10d COMMON NAILS ON 6"/12" PATTERN (EDGES/FIELD). NAILS TO HAVE A MINIMUM PENETRATION INTO FRAMING MEMBER OF 1-1/2".
8. REFER TO TYPICAL WOOD WALL DETAILS FOR FRAMING AROUND AN OPENING THROUGH A WOOD STUD BEARING WALL. TYPICAL.
9. REFER TO EXTERIOR MISCELLANEOUS VENEER LINTEL SCHEDULE FOR ALL OPENINGS IN EXTERIOR VENEER.
10. ALL WOOD POSTS SHALL LINE UP FLOOR TO FLOOR DOWN TO THE TOP OF CONCRETE FOUNDATION WALL OR TOP OF PRECAST PLANK LEVEL. PROVIDE SOLID BLOCKING OF SAME SIZE AS POST IN TRUSS SPACES.
11. AT INTERIOR BEARING WALLS WHERE FLOOR TRUSSES BEAR ON WALL FROM EITHER SIDE, LAP TRUSSES AND BEAR EACH TRUSS FULL WIDTH OF WALL, TYPICAL.
12. PROVIDE 2x6 STRONGBACK BRIDGING FULL LENGTH OF BUILDING. NAIL TO VERTICAL TRUSS WEB w/ (3) 16d NAILS. PROVIDE BRIDGING EQUALLY SPACED ALONG TRUSS SPAN AS REQUIRED BY DESIGN.
13. AT EXTERIOR DECKS, PROVIDE COMPOSITE OR PRESSURE TREATED 5/4" WOOD DECKING.
14. ALL WOOD HEADERS SHOWN IN STUD WALLS ARE DROPPED HEADERS AND SHALL BE PLACED AT WALL OPENING HT UNLESS NOTED OTHERWISE. REFER TO ARCH DRAWINGS FOR OPENING HEIGHTS. ALL OTHER HEADERS SHALL BE FLUSH w/ BOTTOM OF FLOOR FRAMING, TYPICAL.
15. ALL VERTICAL MASONRY WALL REINFORCEMENT SHALL RUN CONTINUOUS THROUGH BOND BEAMS AND EXTEND FULL HEIGHT OF THE WALL. GROUT CORES SOLID AT ALL VERTICAL REINFORCING.
16. "MW-X" INDICATES MASONRY WALL REINFORCEMENT TYPE. REFER TO SCHEDULE FOR SIZE & SPACING.
17. GROUT MASONRY CORES SOLID AT ALL MECHANICAL ANCHOR LOCATIONS, TYPICAL.
18. (XXX'-XX") INDICATES THE TOP OF STEEL BEAM ELEVATION.



WOOD FLOOR FRAMING LEGEND

WOOD FRAMING HEADER/POST SCHEDULE

MARK	MATERIAL	WIDTH (in)	DEPTH (in)	BEARING	REMARKS
H3.1, H3.2, H3.3	LVL	1.75	14	P3.1 to P3.6	Third floor enclosed balconies. South facade.
H3.4 to H3.9	LVL	3.5	7.25	2x6 stud	Bearing stud width will match wall studs width.
H3.10	LVL	3.5	14	P3.7/CMU wall	Notched in CMU wall
H2.1 to H2.6	LVL	3.5	7.25	2x6 stud	Bearing stud width will match wall studs width.
H2.7	LVL	3.5	14	P3.7/CMU wall	Notched in CMU wall
H1.1 to H1.6	LVL	3.5	7.25	2x6 stud	Bearing stud width will match wall studs width.
H1.7	ASTM A992 steel	(2) C15x50	15	P1.1, P1.2, P1.3	
H1.8	ASTM A992 steel	W14x30	13.8	P1.3, P1.4	
H1.9	LVL	5.25	18	H17, bearing wall	
Facade headers (span < 3.5 feet)	LSL	<3.5	11.875	PFH	
Interior headers (span < 4.0 feet)	LSL	4	16	PIH	
Cantilevers					
C2,C5	LVL	2x5.25	14	SW_S1A and SW_S1B shear walls	
C1, C3, C4, C6	LVL	5.25	14	facade bearing walls	
C7, C8, C9, C10, C15	LVL	3.5	14	CMU wall	Bolted to masonry
C11, C12	LVL	5.25	14	facade bearing walls	
C13	LVL	5.25	18	SW_N2D shear wall	At shear wall bottom
C14	LVL	3.5	18	CMU wall	Bolted to masonry
C16	LVL	5.25	14	SW_N1A shear wall	
Posts					
P3.1 to p3.6	saw lumber	6	6		Third floor enclosed balconies. South facade.
P3.7	saw lumber	4	6		
P2.1	saw lumber	4	6		
P1.1, P1.2, P1.3	A500 Rect. HSS Grade B	HSS8x8x3/16			
P1.4	A500 Rect. HSS Grade B	HSS7x7x3/16			
P1.5	A500 Rect. HSS Grade B	HSS8x8x3/16			
PFH	A500 Rect. HSS Grade B	4	6		Facade header supports (span<3.5 feet)
PIH	A500 Rect. HSS Grade B	4	6		Interior header supports (span<4 feet)

I DID NOT SEE ANYTHING ON THE PLANS OR SCHEDULES ADDRESSES THE DOOR AND WINDOW HEADER SIZES; INCLUDING SHOULDER (JACK STUDS) STUDS OR KING STUD REQUIREMENTS

I DID NOT SEE ANYTHING ON THE PLANS OR SCHEDULES ADDRESSES THE LOAD BEARING STUD SCHEDULE INCLUDING TOP AND BOTTOM PLATE MATERIALS

INFORMATION REGARDING THE TRUSS ALIGNMENT WITH THE FLOOR TRUSSES AND JOISTS. DO YOU WANT TRUSSES AND STUDS TO ALIGN. THIS CAN BE AN ISSUE WITH TRUSS SPACING AT 24" O.C. AND STUDS AT 16" O.C

FOR THE SHEAR WALLS I DID NOT SEE ANYTHING ADDRESSING JAMB STUDS AND TIE DOWN INFORMATION/DETAILS AT THE FLOORS

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Date	Description
08.15.2019	75% CD Set

STRUCTURAL NOTES
CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

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