

DESIGN DATA

APPLICABLE CODES/STANDARDS:
....INTERNATIONAL BUILDING CODE - 2016
....ASCE 7-16 MIN DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE/SEI

STRUCTURAL DESIGN STANDARDS (DESIGN SHALL CONFORM TO THE CURRENT EDITION UNDER THE APPLICABLE CODE):
....ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY
....ACI 530/531 BLDG CODE REQUIREMENTS AND SPECS FOR MASONRY STRUCTURES (AND RELATED COMMENTARIES)
....ANSI/AISC 360-16 SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS
....AWS D1.1/D1.1M STRUCTURAL WELDING CODE-STEEL

DEFLECTION LIMITS			
MEMBERS	LIVE	SNOW or WIND	DEAD + LIVE or SNOW
ROOF MEMBERS			
SUPPORTING GYPSUM BOARD CEILINGS	L/360	L/360	L/240
SUPPORTING FLEXIBLE CEILINGS	L/360	L/360	L/240
NOT SUPPORTING CEILING	L/240	L/240	L/180
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC.)	L/600	L/600	L/600
FLOOR MEMBERS			
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC.)	L/600	L/600	L/600
SUPPORTING GYPSUM BOARD CEILINGS	L/540	N/A	L/360
SUPPORTING FLEXIBLE MATERIALS	L/540	N/A	L/360
LINTEL/HEADER/BEAM MEMBERS			
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC.)	L/600	L/600	L/600
SUPPORTING FLEXIBLE MATERIALS (EIFS, SIDING, ETC.)	L/360	L/360	L/240
EXTERIOR WALLS			
WITH RIGID FINISHES (BRICK, MASONRY, ETC.)	N/A	L/600	N/A
WITH FLEXIBLE FINISHES (EIFS, SIDING, ETC.)	N/A	L/360	N/A

BUILDING DESIGN LOADS/CRITERIA

DESIGN LIVE LOADS:
....FLOOR FRAMING (RETAIL, OFFICE, RESTAURANT, RECREATIONAL) 100 psf

....FLOOR FRAMING (RESIDENTIAL AREAS) 40 psf

....STAIRWAYS, EXITS 100 psf

....BALCONIES 40 psf

....PRIVATE GARAGES (PASSENGER VEHICLES ONLY) 40 psf

....INTERIOR PARTITION WALLS (UNIFORMLY DISTRIBUTED WEIGHT) 15 psf

....CORRIDORS FIRST FLOOR 100 psf

....CORRIDORS 2nd & 3rd FLOORS 40 psf

....CORNICES 60 psf

....SLOPED ROOF FACTOR (Cs) 1.0

SNOW LOADS & DESIGN DATA:
....DESIGN SNOW LOAD 42 psf (BALANCED SNOW LOAD)

....FLAT ROOF SNOW LOAD (Pf) = (0.7Ce'Cl'sPg) 42 psf

....SNOW EXPOSURE FACTOR (Ce) 1.0

....SNOW LOAD IMPORTANCE FACTOR (Is) 1.0

....ROOF THERMAL FACTOR (Ci) 1.0

....GROUND SNOW (Pg) 60 psf

....SLOPED ROOF FACTOR (Cs) 1.0

WIND DESIGN DATA:
....WIND IMPORTANCE FACTOR (Iw) 1.0

....RISK CATEGORY II

....BASIC WIND SPEED (5-SECOND GUST, ULTIMATE) 115 MPH

....BASIC WIND SPEED (5-SECOND GUST, NOMINAL) 90 MPH

....MEAN ROOF HEIGHT 33 FT

....WIND EXPOSURE CATEGORY B

....WIND EXPOSURE CLASSIFICATION ENCLOSED

....VELOCITY EXPOSURE COEFFICIENT Kz 0.720

....TOPOGRAPHIC FACTOR (Kt) 1.0

....DESIGN PROCEDURE METHOD 1 (SIMPLIFIED PROCEDURE)

NET PRESSURE COEFFICIENTS C_{net}		
AREA	C_{net} INTERNAL PRESSURE	C_{net} INTERNAL PRESSURE
WINDWARD WALL	0.43	0.73
LEEWARD WALL	-0.51	-0.21
SIDEWALL	-0.66	-0.35
PARAPET WINDWARD WALL	1.28	
PARAPET LEEWARD WALL	-0.85	
FLAT ROOF	-1.09	-0.79

DESIGN WIND PRESSURES P_{net}		
AREA	P_{net} INTERNAL PRESSURE	P_{net} INTERNAL PRESSURE
WINDWARD WALL	10.5 psf	17.8 psf
LEEWARD WALL	-12.4 psf	-5.1 psf
SIDEWALL	-16.1 psf	-8.5 psf
PARAPET WINDWARD WALL	31.2 psf	
PARAPET LEEWARD WALL	-20.7 psf	
FLAT ROOF	-26.6 psf	-19.3 psf

EARTHQUAKE DESIGN DATA:
....OCCUPANCY CATEGORY II
....SEISMIC IMPORTANCE FACTOR (Ie) 1
....MAPPED SPECTRAL ACCELERATIONS AT SHORT PERIODS (Ss) 0.045 g
....MAPPED SPECTRAL ACCELERATIONS AT (1) SECOND PERIODS (S1) 0.038 g
....SITE CLASSIFICATION B
....SITE COEFFICIENT (Ps) 1.0
....DESIGN SPECTRAL RESPONSE COEFFICIENT AT SHORT PERIODS (Sds) 0.030 g
....DESIGN SPECTRAL RESPONSE COEFFICIENT AT (1) SECOND PERIODS (Sd1) 0.025 g
....SEISMIC DESIGN CATEGORY A
....BASIC SEISMIC-FORCE-RESISTING SYSTEM LIGHT FRAME WOOD WALLS WITH STRUCTURAL WOOD SHEAR PANELS EQUIVALENT LATERAL FORCE ANALYSIS
....ANALYSIS PROCEDURE FOR SEISMIC DESIGN

SOIL DESIGN VALUES:
....SOIL UNIT WEIGHT 125 PCF (ASSUMED)
....LATERAL EARTH PRESSURE
....AT-REST (BASEMENT WALLS) 62.5 PSF/FT OF DEPTH (ASSUMED)
....PASSIVE 340 PSF (ASSUMED)
....COEFFICIENT OF SLIDING FRICTION 0.30 (ASSUMED)
....SUBGRADE MODULUS 260 PCI (ASSUMED)
....ALLOWABLE SOIL BEARING PRESSURE 3000 PSF

REFER TO SOILS REPORT NO. 17002 DATED 2/10/2017 PREPARED BY ITCO ALLIED ENGINEERING CO. FOR DESCRIPTION OF SOIL CONDITIONS, GEOTECHNICAL RECOMMENDATIONS, AND DESIGN VALUES

FOUNDATION AND EARTHWORK

- ALL EXTERIOR FOOTINGS MUST BEAR BELOW LOCAL FROST LINE RELATIVE TO ADJACENT FINISH EXTERIOR GRADE.
- DO NOT PLACE ANY FOOTINGS ON FROZEN SUBGRADE.
- BACK FILLING SHALL BE DONE SIMULTANEOUSLY ON BOTH SIDES OF FOUNDATION WALLS.
- DO NOT PLACE BACK FILL AGAINST BASEMENT WALLS UNTIL THE TOP AND BOTTOM OF THE WALL ARE ADEQUATELY BRACED BY THE SLAB ON GRADE AND THE FLOOR FRAMING AT THE TOP OF THE WALL.
- REMOVE ANY EXISTING CONCRETE 2'-0" BELOW NEW CONCRETE FOOTINGS AND SLABS ON GRADE, UNLESS NOTED OTHERWISE.
- SHORING/OR UNDERPINNING SHALL BE DESIGNED TO LIMIT HORIZONTAL AND VERTICAL MOVEMENT OF EXISTING CONSTRUCTION TO 1/4" MAXIMUM IN ANY DIRECTION.
- CENTER PIER AND COLUMN FOOTINGS ON COLUMN CENTERLINES AND WALL FOOTINGS ON WALL CENTERLINES UNLESS SPECIFICALLY NOTED OTHERWISE.
- ALL BACK FILL WITHIN 3'-0" OF RETAINING WALLS AND BASEMENT WALLS SHALL BE FREE DRAINING GRANULAR MATERIAL APPROVED BY A SOILS ENGINEER AND COMPACTED TO 90% STANDARD PROCTOR.
- TOP OF FOOTING ELEVATIONS SHOWN ON THESE CONSTRUCTION DOCUMENTS REPRESENT MINIMUM FOOTING DEPTHS FOR FROST PROTECTION AND BEST JUDGMENT OF A SUITABLE BEARING STRATUM. ACTUAL GRADE CONDITIONS AND SUITABLE BEARING STRATUM MUST BE VERIFIED BY THE CONTRACTOR AND A SOILS ENGINEER AT THE TIME OF EXCAVATION.

10. FOOTING EXCAVATIONS MUST EXTEND TO COMPETENT BEARING MATERIAL. CONTRACTOR SHALL HIRE A SOILS ENGINEER TO FIELD VERIFY NET ALLOWABLE SOIL BEARING CAPACITY STATED ON THESE CONSTRUCTION DOCUMENTS AND IN GEOTECHNICAL REPORT FOR THIS PROJECT. THE SUITABLE BEARING STRATUM DOES NOT EXIST AT FOOTING ELEVATION STATED ON CONSTRUCTION DOCUMENTS. EXCAVATION SHOULD BE EXTENDED UNTIL SOIL WITH APPROPRIATE BEARING CAPACITY IS REACHED. PLACE GRANULAR MATERIAL ON TOP OF FOOTINGS OR EXTEND FOOTINGS DOWN TO SUITABLE BEARING STRATUM. ENGINEERED FILL BELOW SLABS ON GRADE AND FOOTINGS SHALL BE FREE DRAINING GRANULAR MATERIAL COMPAKTED TO 95% MODIFIED PROCTOR AND PLACED PER THE SOIL ENGINEERS RECOMMENDATIONS. ALL FIELD CONDITIONS THAT WILL AFFECT DESIGN AS PRESENTED MUST BE COORDINATED WITH STRUCTURAL ENGINEER.

11. REFER TO DESIGN DATA FOR DESCRIPTION OF SOIL CONDITIONS, GEOTECHNICAL RECOMMENDATIONS, AND DESIGN VALUES.

CONTINUITY:

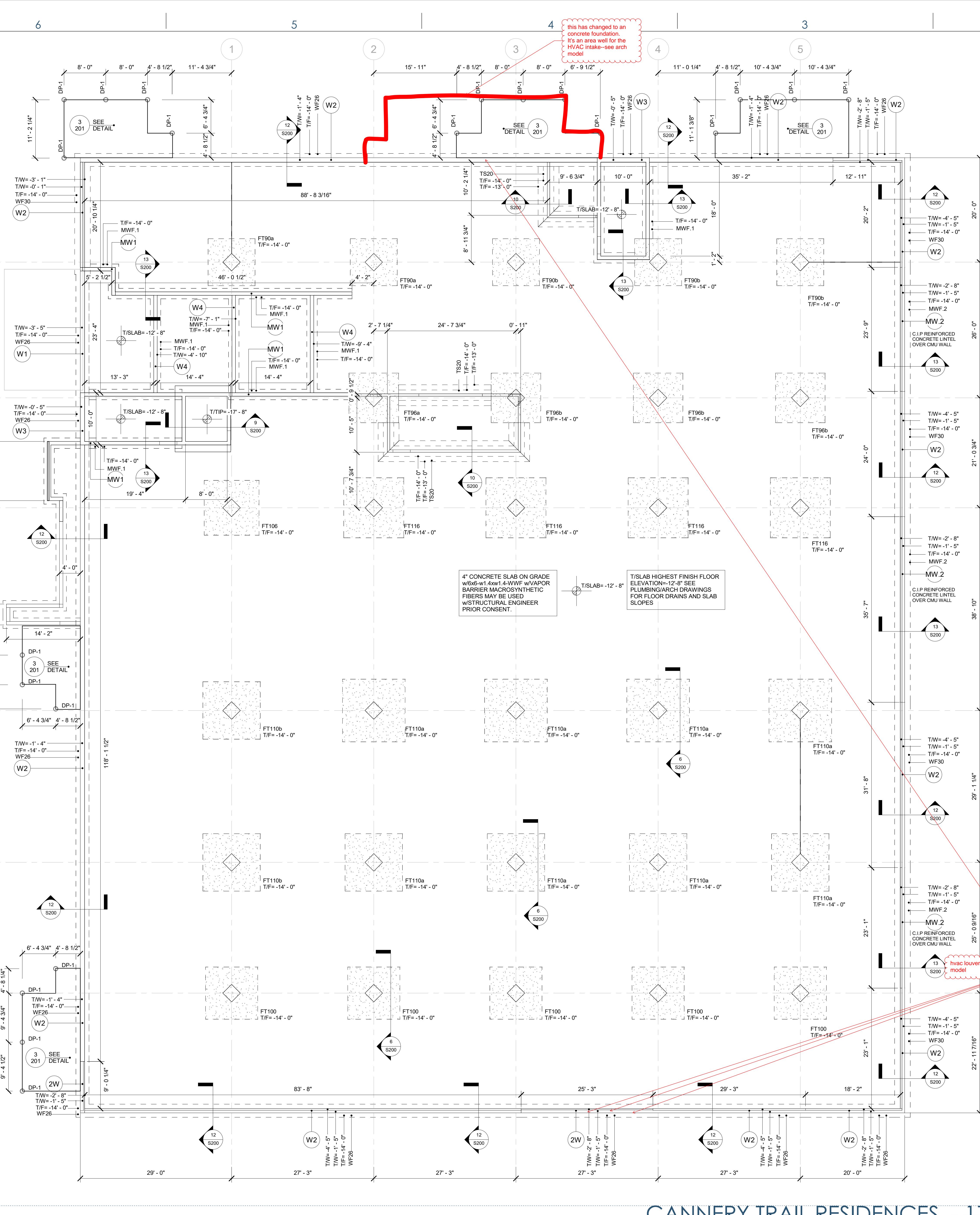
ALL REINFORCING SHALL BE CONTINUOUS UNLESS NOTED OTHERWISE. CONTINUITY AT CORNERS AND INTERSECTIONS SHALL BE ACHIEVED USING CORNER BARS AND CONTACT LAP SPLICES. SEE TYPICAL DETAIL. CONTINUITY AT OTHER LOCATIONS MAY BE ACHIEVED USING CONTACT LAP SPLICES SHOWN ON APPROVED SHOP DRAWINGS. LOCATION OF LAP SPLICES SHALL BE SHOWN ON THE SHOP DRAWINGS. UNLESS NOTED OTHERWISE, THE FOLLOWING LAP SPLICES SHALL BE USED: (ALL LAP SPLICES ARE CLASS B 3 SPLICES)

LOCATION:	#3	#4	#5	#6	#7	#8	#9	#10	#11
3,000 & 3,500 PSI CONCRETE:									
- TOP BARS (*)	21"	19"	35"	46"	71"	93"	118"	149"	184"
- OTHER BARS:	16"	22"	27"	35"	55"	71"	91"	115"	142"
4,000 & 4,500 PSI CONCRETE:									
- TOP BARS (*)	16"	19"	25"	36"	61"	80"	102"	129"	159"
- OTHER BARS:	16"	16"	19"	28"	47"	62"	78"	99"	123"

(* TOP BARS ARE HORIZONTAL REINFORCING WHERE MORE THAN 12" OF CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCING.
(**) FOR #4 EPOXY COATED REBAR, USE 27" SPLICE LENGTH AT 3,000 AND 3,500 PSI CONCRETE AND 19" AT 4,000 AND 4,500 PSI.

MECHANICAL CONNECTIONS MAY BE USED IN LIEU OF LAP SPLICES PROVIDED APPROVAL IS OBTAINED FROM THE ARCHITECT/ENGINEER. CONNECTIONS SHALL DEVELOP IN TENSION 125 PERCENT OF THE SPECIFIED YIELD STRENGTH OF THE REINFORCING. MECHANICAL SPLICES SHALL BE SHOWN ON THE SHOP DRAWINGS AND BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS AND THE PRODUCT'S ICC-ES REPORT. SUBMIT THE PRODUCT'S ICC-ES REPORT FOR MECHANICAL SPLICE PRODUCTS WITH SHOP DRAWINGS.

WELDING CONNECTIONS:
....WELDING ELECTRODES E70XX E80XX FOR WELDING REINF
MASONRY:
....fm = 2,000 PSI
MASONRY MORTAR:
....TYPE "M



MASONRY WALL REINFORCING SCHEDULE

MARK	WALL THICKNESS	VERTICAL REINFORCEMENT & SPACING	REINFORCEMENT LOCATION IN CELL CENTER
MW.1	8"	#5 AT 48" o/c MAX	
MW.2	8"	#6 AT 16" o/c MAX	INSIDE FACE

MASONRY WALL REINFORCEMENT SCHEDULE NOTES:
1. GROUP CONCRETE MASONRY UNITS SOLID FULL HEIGHT OF BUILDING AT REINFORCEMENT LOCATIONS.
2. UNLESS OTHERWISE SPECIFIED, PROVIDE VERTICAL REINFORCEMENT.
3. PROVIDED WHERE MASONRY UNIT WALL REINFORCING ABOVE AND BELOW ALL MASONRY OPENINGS: EXTEND LARGE OF 24" OR 40 BAR DIA.
4. REFER TO STRUCTURAL NOTES SHEET FOR LAPS IN STEEL REINFORCEMENT.
5. PROVIDE STANDARD (W1) HORIZONTAL JOINT REINFORCING AT 10' ON CENTER VERTICALLY (8' ON CENTER IN PARAPET WALLS) UNO.
6. MASONRY FIREWALL CONSTRUCTION ASSUMES MASONRY BLOCKS COMPRISED OF LIMESTONE.

MASONRY WALL FOOTING SCHEDULE

MARK	WIDTH	THICKNESS	LONGITUDINAL
MWF.1	2'-0"	1'-0"	(2) #5
MWF.2	3'-0"	1'-2"	(3) #5

MASONRY WALL FOOTING SCHEDULE NOTES:
1. REFER TO STRUCTURAL NOTES SHEET FOR LAPS IN STEEL REINFORCEMENT.
2. REFER TO FOUNDATION PLAN FOR TOP OF FOOTING ELEVATIONS.
3. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE

THICKENED SLAB SCHEDULE

MARK	DIMENSIONS	REINFORCEMENT	REMARKS
TS20	2'-0"	1'-0"	(2) #5

THICKENED SLAB SCHEDULE NOTES:
1. REFER TO STRUCTURAL NOTES SHEET FOR LAPS IN STEEL REINFORCEMENT.
2. REFER TO FOUNDATION PLAN FOR TOP OF FOOTING ELEVATIONS.
3. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE

WALL FOOTING SCHEDULE

MARK	WIDTH	THICKNESS	LONGITUDINAL	TRANSVERSE
WF26	2'-6"	1'-2"	(3) #5	#5's AT 12' BOTTOM FACE
WF30	3'-0"	1'-2"	(3) #5	#5's AT 12' BOTTOM FACE

COLUMN FOOTING SCHEDULE NOTES:
1. REFER TO STRUCTURAL NOTES SHEET FOR MINIMUM COVER REQUIREMENTS.
2. REFER TO FOUNDATION PLAN FOR TOP OF FOOTING ELEVATIONS.
3. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
4. ALL LAPS IN STEEL REINFORCING SHALL BE CLASS "B" LAP SPLICES UNLESS NOTED OTHERWISE.

COLUMN FOOTING SCHEDULE

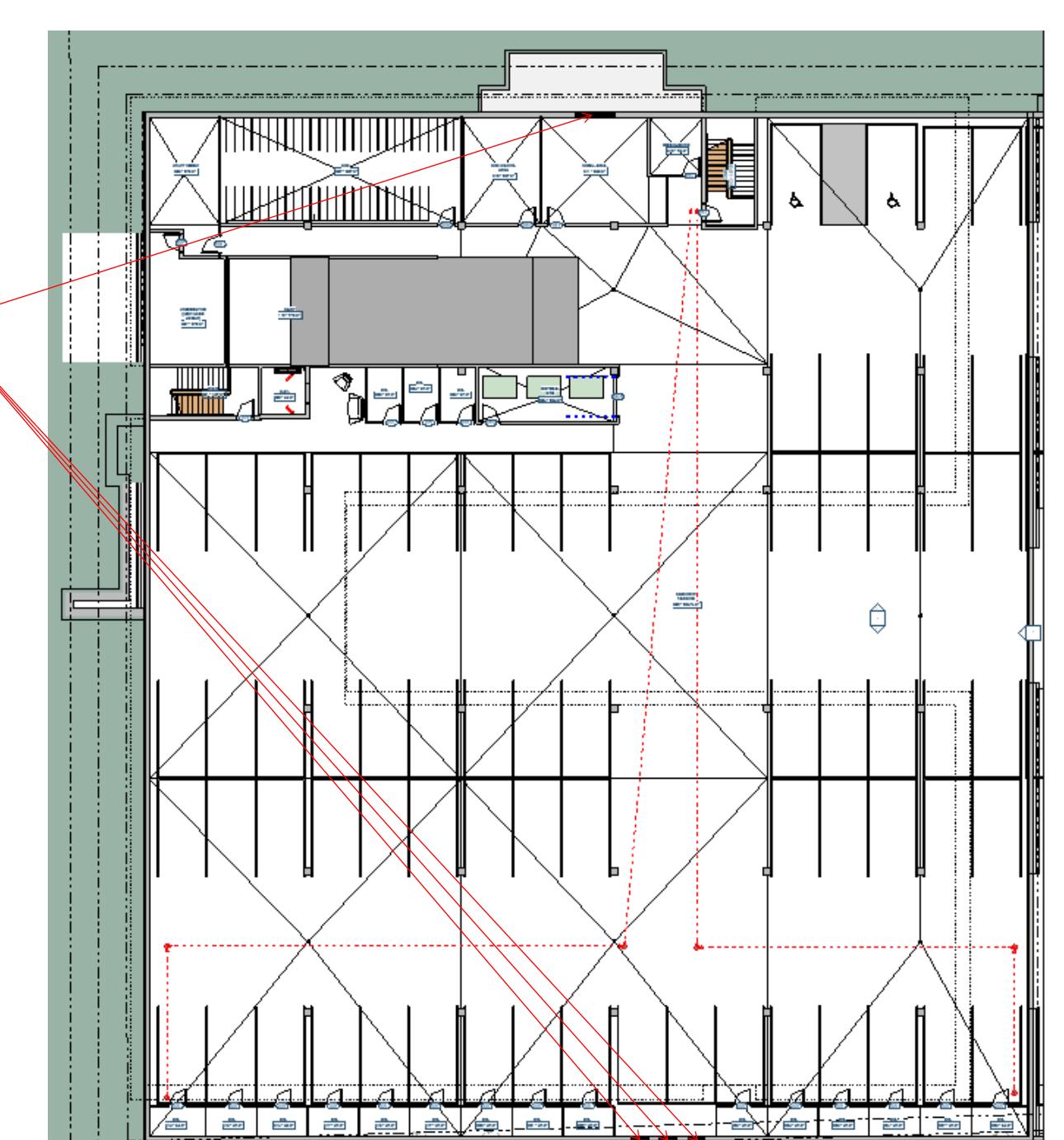
MARK	W	L	D	BOTTOM REINFORCING	COLUMNS
FT90a	9'-0"	9'-0"	1'-8"	(10) #7	(10) #7 A1 A2
FT90b	9'-0"	9'-0"	1'-8"	(10) #8	(10) #8 A3 A4 A5
FT96a	9'-6"	9'-6"	1'-8"	(10) #7	(10) #7 B2
FT96b	9'-6"	9'-6"	1'-8"	(10) #8	(10) #8 B3 B4 B5
FT100	10'-0"	10'-0"	2'-1"	(11) #8	(11) #8 F1 F2 F3 F4 F5
FT106	10'-6"	10'-6"	2'-3"	(11) #8	(11) #8 C1
FT110a	11'-0"	11'-0"	2'-1"	(12) #8	(12) #8 G2 G3 G4 G5
FT110b	11'-0"	11'-0"	2'-3"	(12) #8	(12) #8 D1 G1
FT116	11'-6"	11'-6"	2'-1"	(12) #8	(12) #8 C2 C3 C4 C5
FT120	12'-0"	12'-0"	2'-3"	(13) #8	(13) #8 D2 D3 D4 D5

COLUMN FOOTING SCHEDULE NOTES:
1. REFER TO STRUCTURAL NOTES SHEET FOR LAPS IN STEEL REINFORCEMENT.
2. REFER TO FOUNDATION PLAN FOR TOP OF FOOTING ELEVATIONS.
3. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE

CONCRETE WALL REINFORCING SCHEDULE

MARK	TYPE	THICKNESS	REINFORCEMENT	REMARKS
W1	CONCRETE	10"	5#'s AT 18" o.c.	Inside face
W2	CONCRETE	10"	5#'s AT 12" o.c.	5#'s AT 12" o.c. inside face
W3	CONCRETE	10"	6#'s AT 12" o.c.	inside face
W4	CONCRETE	8"	4#'s AT 12" o.c.	3#'s AT 12" o.c. centered in wall thickness

CONCRETE WALL REINFORCING SCHEDULE NOTES:
1. REFER TO STRUCTURAL NOTES SHEET FOR LAPS IN STEEL REINFORCEMENT.
2. COORDINATE AND VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND EXIST. CONDITIONS



FOUNDATION PLAN
CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

Date: 07.08.2019 Description: Footing and Foundation Plan Permit

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CAPITAL GROUP
Developer: W Capital Group
tyler@wcapitalgroup.com | 608.345.6454

OpeningDesign
316 Washington Ave | Suite 675
Madison, WI 53703
ryan@openingdesign.com | 773.425.6046

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345 Greenway Street | Eau Claire, WI 54701
jim@royalbuilt.com | 715.225.6377

Cedar Corporation
Civil Engineer: CEDAR CORPORATION
404 Wilson Avenue | Menomonie, WI 54761
kevin.colm@cedarcorp.com | 715.235.9081

Structural Engineer: Structural Engineering
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l.pereztato@xengineering.xyz | +34 610 56 26 37

Hovland's
Heating - Ventilation - Air Conditioning
Mechanical Engineer: HOVLAND'S HVAC
10954 E Melby Street | Chippewa Falls, WI 54729
jansen@hovlands-inc.com | 715.532.5595

Electrical Engineer: PRISM DESIGN ELECTRICAL CONSULTANTS INC
E403 39th Rd 85 | Mondovi, WI 54755
bholgren@prismdesign-electrical.com | 715.797.0602

Plumbing Engineer: TAILORED ENGINEERING
1400 Aspen Commons | Ste 210 | Middleton, WI 53562
bnovak@tailoredeng.com | 608.209.7500

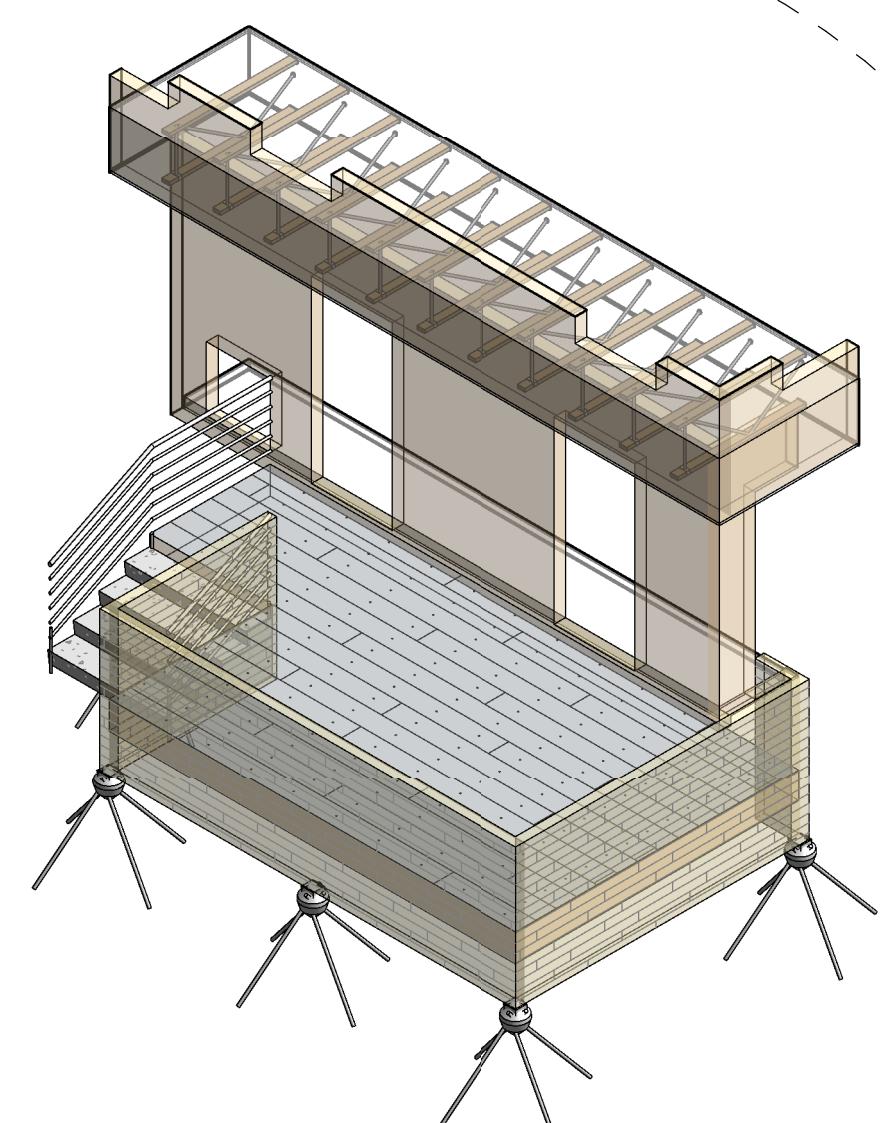
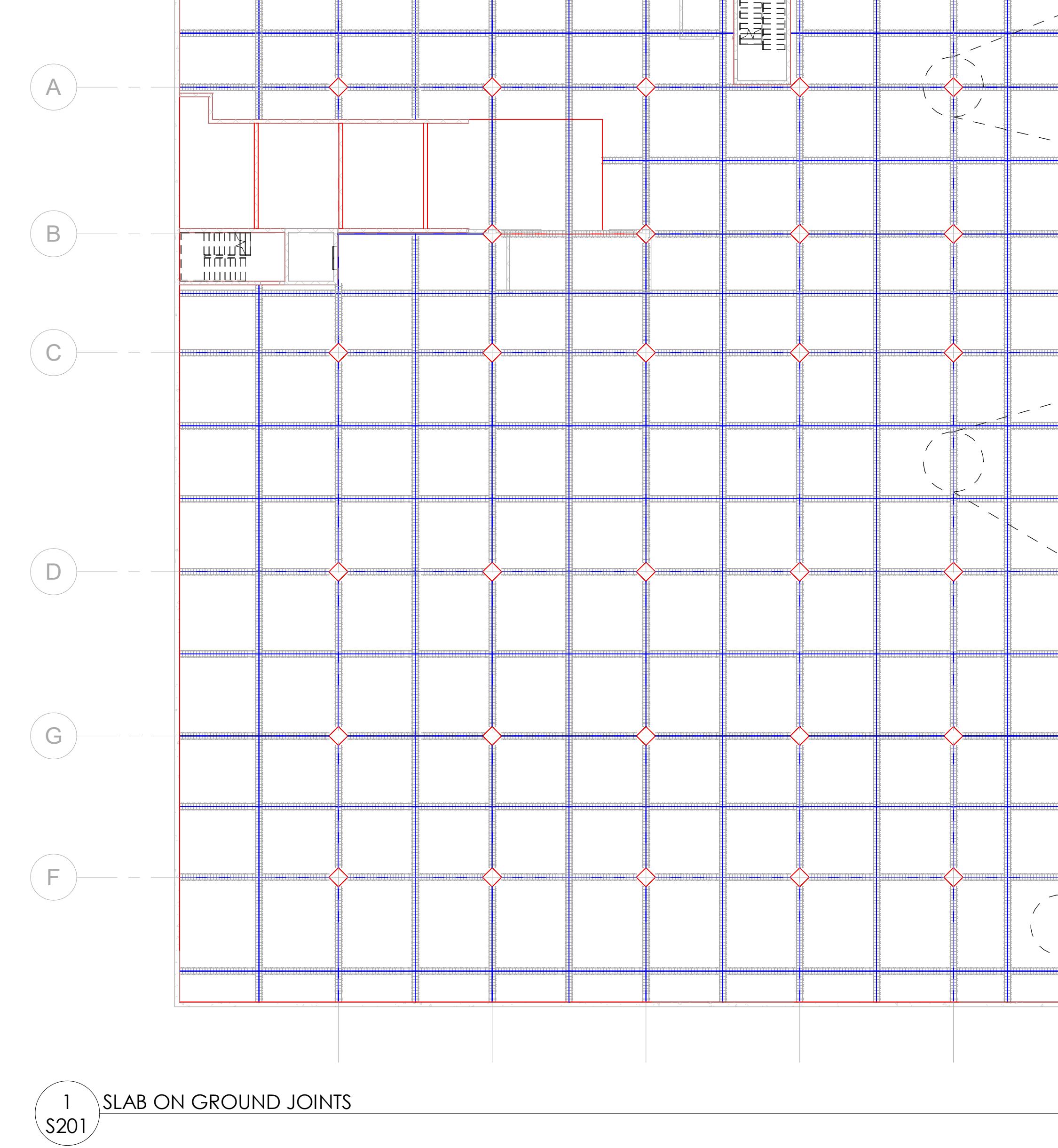
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07.08.2019	Footing and Foundation Plan Permit

STRUCTURAL DETAILS

CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

JOINT TYPES:

- █ CONTRACTION OR CONSTRUCTION JOINT
- █ ISOLATION JOINT



2 S201 DIAMOND PIERS

CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

STRUCTURAL DETAILS rev01
S201

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
Innovation
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4727 Dale-Curtin Dr. McFarland, WI 53558
khey@innovation-built.com

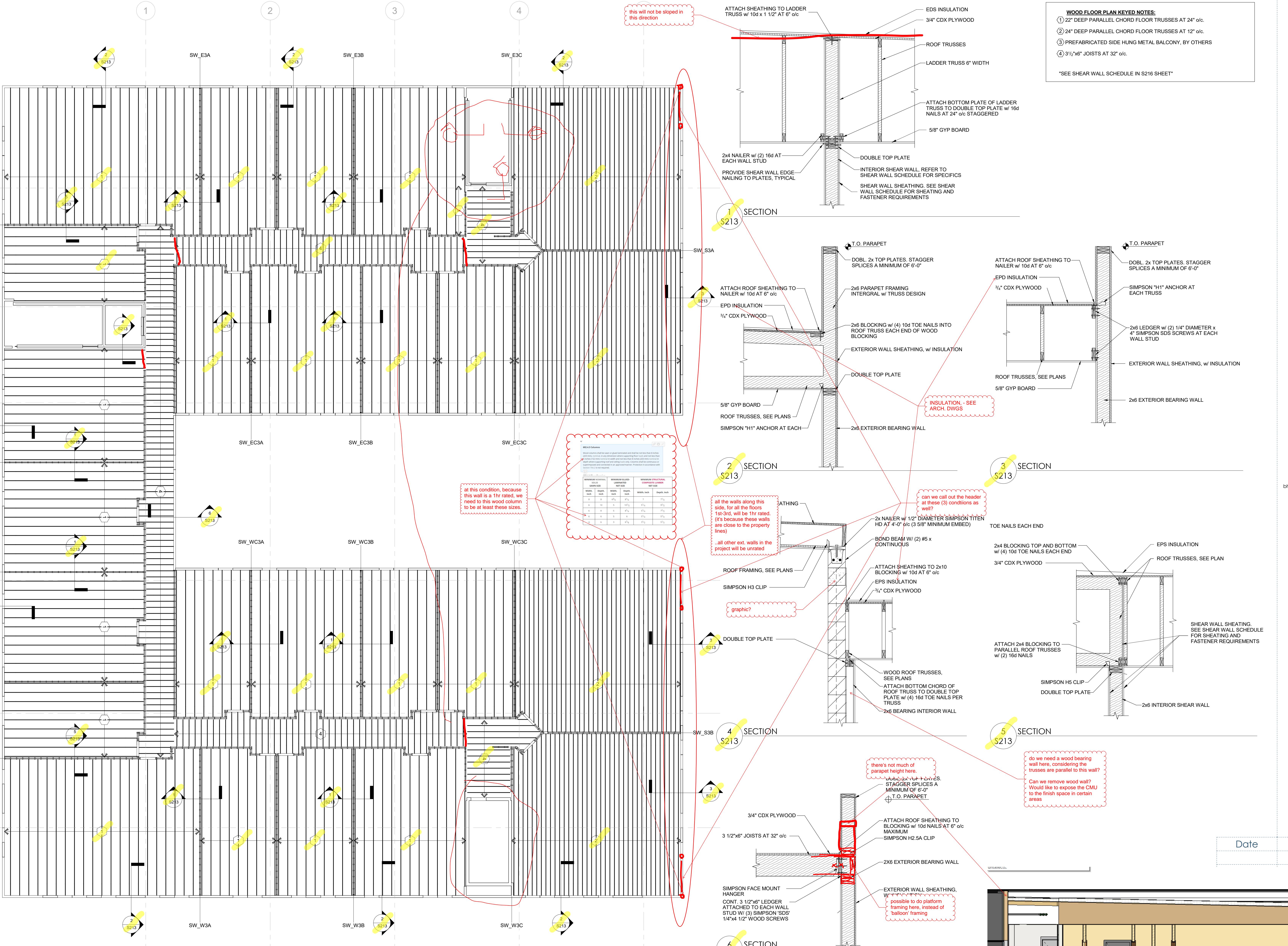
Hovland's
Mechanical Engineer: HOVLAND'S HVAC
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Date	Description
07.08.2019	Footing and Foundation Plan Permit





0 FRAMING PLAN - ROOF
S213 1/8" = 1'-0"

CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

FRAMING PLAN - ROOF
S213

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Structural Engineer: Structural Engineering
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l.perezato@xcengineering.xyz | +34 610 56 26 37

Innovation Engineering Services Incorporated
Structural Engineer: Structural Engineering
4727 Dale-Curtain Dr, McFarland, WI 53558
khsley@innovation-built.com

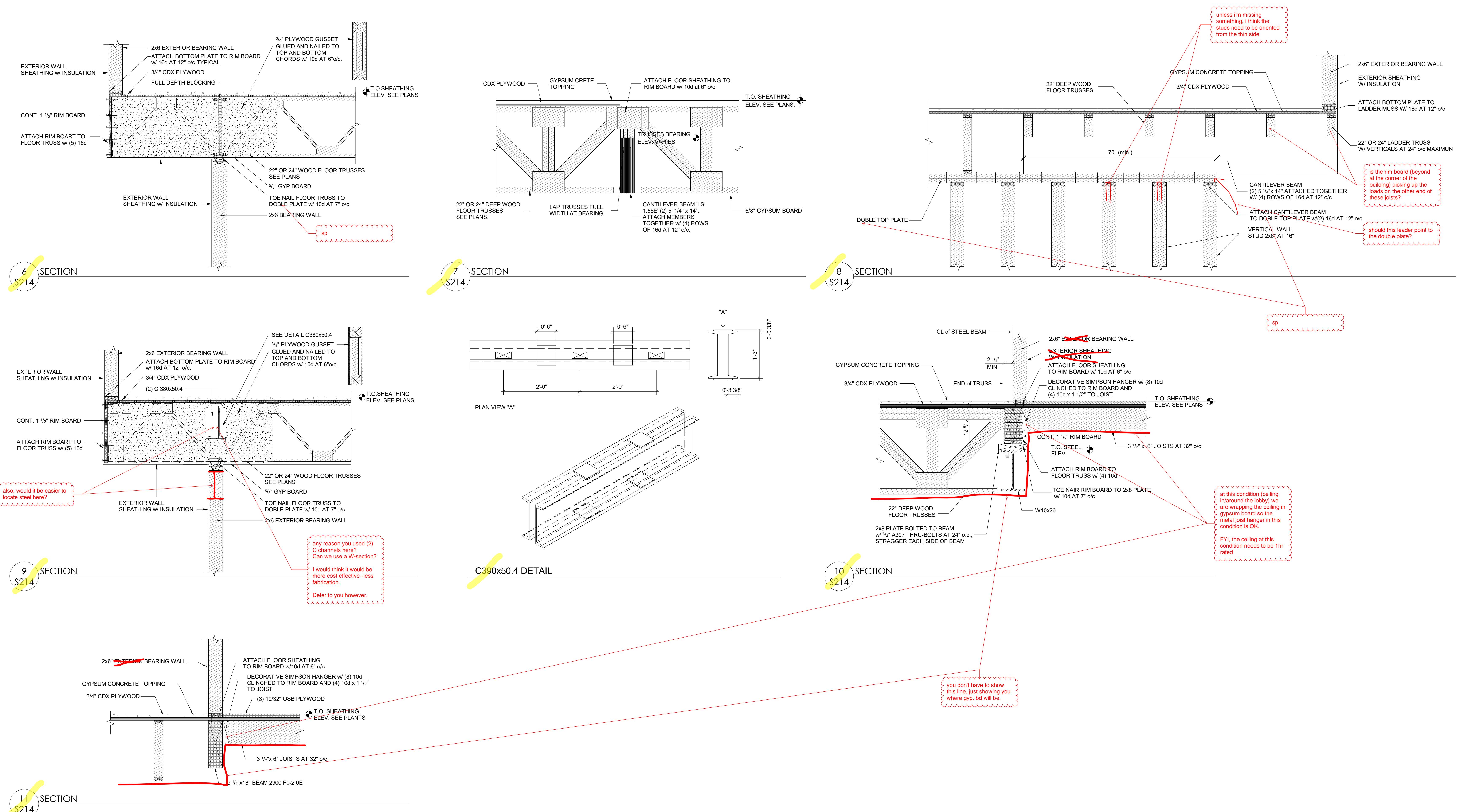
Hovland's Heating - Ventilation + Air Conditioning
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1400 Aspen Commons | Ste 210 | Middleton, WI 53562
bnovak@tailoredeng.com | 608.209.7500

Date _____
Description _____

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CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

S214

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The logo consists of a stylized red letter 'W' formed by two building silhouettes. The left building has vertical windows, and the right building has horizontal windows. Below the logo, the words "CAPITAL GROUP" are written in a bold, serif font.

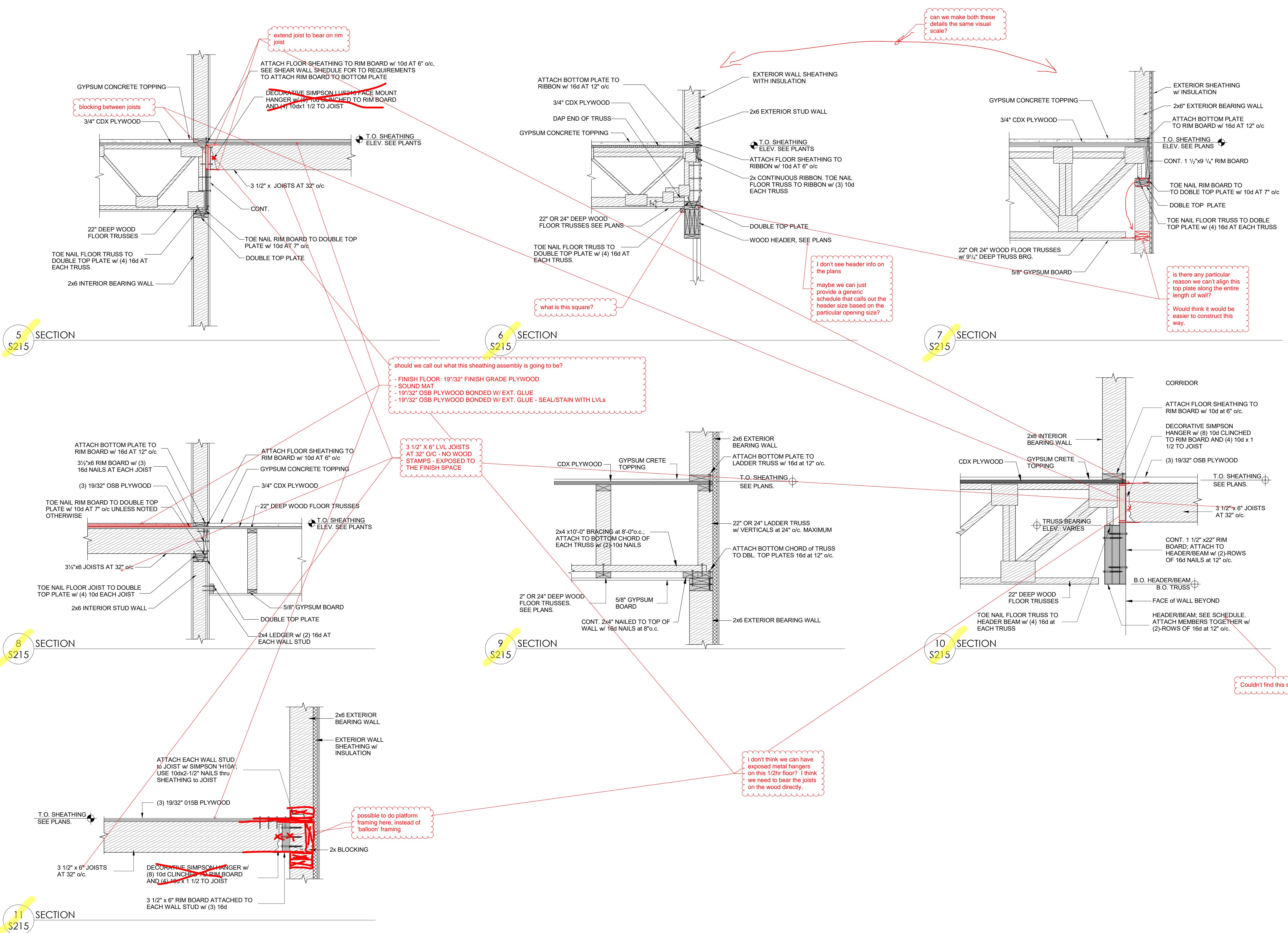
The logo for Cedar Corporation features the word "cedar" in a large, bold, sans-serif font where the "c" is stylized with a circular cutout. Below it, the word "corporation" is written in a smaller, lowercase, sans-serif font.

Civil Engineer: CEDAR CORPORATION
1 Wilson Avenue | Menomonie, WI 54751
n.ouim@cedarcorp.com | 715-235-9081

The logo for Ennovation Engineering Services Incorporated features the word "Ennovation" in a large, bold, blue sans-serif font, with "Engineering Services Incorporated" in a smaller, black sans-serif font below it. The text is set against a white background that is framed by a thick, dark blue curved shape resembling a stylized 'E' or a house roofline. Below the main text is a thin, light blue curved shape.

Electrical Engineer: PRISM DESIGN ELECTRICAL
CONSULTANTS INC
8403 State Rd 85 | Mondovi, WI 54755
n@prismdesign-electrical.com | 715.797.0602

The logo for Tailored Engineering features a stylized 'T' and 'E' inside a circle on the left, followed by the company name 'TAILORED ENGINEERING' in a bold, sans-serif font.



CANNERY TRAIL RESIDENCES - 1750 N OXFORD AVE. - EAU CLAIRE, WI

STRUCTURAL DETAILS



CAPITAL GROUP
Developer: W Capital Group
tyler@wcapitalgroup.com | 800.345.6454



Architect: OpeningDesign
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WOOD SHEAR WALL SCHEDULE										
Shear wall ID	Sheathing material	Panel thickness	Blocking	Minimum fastener penetration in framing type or blocking	Fastener type and size	Panel edge fastener spacing	Nominal nail shear capacity λ_w	Hold-down anchor capacity λ_h	Number of hold-downs in diameter 4 inch embedment depth	Bottom plate attachment (floor to floor)
SW_N3A	Wood structural panels - sheathing	3/8	YES	1-3/8	8d 4	840	2	-	-	wood screws 20 (d= 0.32 in) at 25 in. o/c; 30 fasteners in 2 rows.
SW_N3B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d 6	560	-	-	-	16d (d= 0.268 in) nails at 12 in. o/c; 35 fasteners in 2 rows.
SW_N3C	Wood structural panels - sheathing	3/8	NO	1-3/8	8d 6	560	-	-	-	16d (d= 0.268 in) nails at 12 in. o/c; 35 fasteners in 2 rows.
SW_N3D	Wood structural panels - sheathing	3/8	YES	1-3/8	8d 4	840	2	-	-	wood screws 20 (d= 0.32 in) at 25 in. o/c; 30 fasteners in 2 rows.
SW_N2A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	4	1430	4	-	wood screws 20 (d= 0.32 in) at 14 in. o/c; 52 fasteners in 2 rows.
SW_N2B	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	6	950	-	-	wood screws 20 (d= 0.32 in) at 14 in. o/c; 52 fasteners in 2 rows.
SW_N2C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	6	950	1	-	16d (d= 0.268 in) nails at 12 in. o/c; 59 fasteners in 2 rows.
SW_N2D	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	4	1430	4	-	wood screws 20 (d= 0.32 in) at 14 in. o/c; 52 fasteners in 2 rows.
SW_N1A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	3	1860	7	10	16d (d= 0.268 in) nails at 12 in. o/c; 58 fasteners in 2 rows.
SW_N1B	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	6	950	-	11	16d (d= 0.268 in) nails at 10 in. o/c; 39 fasteners in 2 rows.
SW_N1C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	6	950	3	11	wood screws 20 (d= 0.32 in) at 10 in. o/c; 32 fasteners in 2 rows.
SW_N1D	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	3	1860	7	10	SDWS log screw (d= 0.197 in) at 10 in. o/c; 60 fasteners in 2 rows.
SW_S3A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	6	950	2	-	wood screws 20 (d= 0.32 in) at 21 in. o/c; 36 fasteners in 2 rows.
SW_S3B	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	6	950	2	-	SDWS log screw (d= 0.197 in) at 21 in. o/c; 36 fasteners in 2 rows.
SW_S2A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	3	1860	6	-	SDWS log screw (d= 0.197 in) at 13 in. o/c; 54 fasteners in 2 rows.
SW_S2B	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	3	1860	6	-	SDWS log screw (d= 0.197 in) at 13 in. o/c; 54 fasteners in 2 rows.
SW_S1A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	11	10	SDWS log screw (d= 0.197 in) at 8 in. o/c; 76 fasteners in 2 rows.
SW_S1B	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	11	10	wood screws 20 (d= 0.32 in) at 8 in. o/c; 30 fasteners in 1 row.
SW_E3A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	4	1430	3	-	wood screws 20 (d= 0.32 in) at 8 in. o/c; 76 fasteners in 2 rows.
SW_E3B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d	6	560	-	-	16d (d= 0.268 in) nails at 12 in. o/c; 30 fasteners in 1 row.
SW_E3C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	4	1430	6	-	SDWS log screw (d= 0.197 in) at 10 in. o/c; 32 fasteners in 2 rows.
SW_E2A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	3	1860	7	-	SDWS log screw (d= 0.197 in) at 11 in. o/c; 64 fasteners in 2 rows.
SW_E2B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d	6	560	1	-	16d (d= 0.268 in) nails at 14 in. o/c; 51 fasteners in 2 rows.
SW_E2C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	11	-	SDWS log screw (d= 0.197 in) at 9 in. o/c; 64 fasteners in 2 rows.
SW_E1A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	13	7	16d (d= 0.268 in) nails at 10 in. o/c; 64 fasteners in 2 rows.
SW_E1B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d	6	560	-	11	16d (d= 0.268 in) nails at 32 in. o/c; 12 fasteners in 1 row.
SW_E1C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	9	11	SDWS log screw (d= 0.197 in) at 8 in. o/c; 72 fasteners in 2 rows.
SW_W3A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	4	1430	3	-	wood screws 20 (d= 0.32 in) at 10 in. o/c; 42 fasteners in 2 rows.
SW_W3B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d	6	560	-	-	16d (d= 0.268 in) nails at 12 in. o/c; 30 fasteners in 1 row.
SW_W3C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	4	1430	6	-	SDWS log screw (d= 0.197 in) at 15 in. o/c; 32 fasteners in 2 rows.
SW_W2A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	3	1860	7	-	SDWS log screw (d= 0.197 in) at 11 in. o/c; 64 fasteners in 2 rows.
SW_W2B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d	6	560	1	-	16d (d= 0.268 in) nails at 14 in. o/c; 51 fasteners in 2 rows.
SW_W2C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	11	-	SDWS log screw (d= 0.197 in) at 9 in. o/c; 54 fasteners in 2 rows.
SW_W1A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	13	9	16d (d= 0.268 in) nails at 14 in. o/c; 72 fasteners in 2 rows.
SW_W1B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d	6	560	-	11	SDWS log screw (d= 0.197 in) at 7 in. o/c; 72 fasteners in 2 rows.
SW_W1C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	9	11	wood screws 20 (d= 0.32 in) at 10 in. o/c; 72 fasteners in 2 rows.
SW_EC3A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	6	950	0	-	16d (d= 0.268 in) nails at 10 in. o/c; 42 fasteners in 2 rows.
SW_EC3B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d	6	560	-	-	16d (d= 0.268 in) nails at 60 in. o/c; 7 fasteners in 1 row.
SW_EC3C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	6	950	3	-	SDWS log screw (d= 0.197 in) at 19 in. o/c; 40 fasteners in 2 rows.
SW_EC2A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	3	1860	2	-	wood screws 20 (d= 0.32 in) at 21 in. o/c; 36 fasteners in 2 rows.
SW_EC2B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d	6	560	-	-	16d (d= 0.268 in) nails at 32 in. o/c; 12 fasteners in 1 row.
SW_EC2C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	3	1860	6	-	SDWS log screw (d= 0.197 in) at 12 in. o/c; 58 fasteners in 2 rows.
SW_EC1A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	11	6	16d (d= 0.268 in) nails at 10 in. o/c; 42 fasteners in 2 rows.
SW_EC1B	Wood structural panels - sheathing	3/8	NO	1-3/8	8d	6	560	-	11	SDWS log screw (d= 0.197 in) at 22 in. o/c; 42 fasteners in 1 row.
SW_EC1C	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	2	2435	11	11	wood screws 20 (d= 0.32 in) at 9 in. o/c; 42 fasteners in 2 rows.
SW_WC3A	Wood structural panels - sheathing	19/32	YES	1-1/2	10d	6	950	0	-	16d (d= 0.268 in) nails at 10 in. o/c; 42 fasteners in 2 rows.
SW_WC3B	Wood structural panels - sheathing	3/8	NO	1						