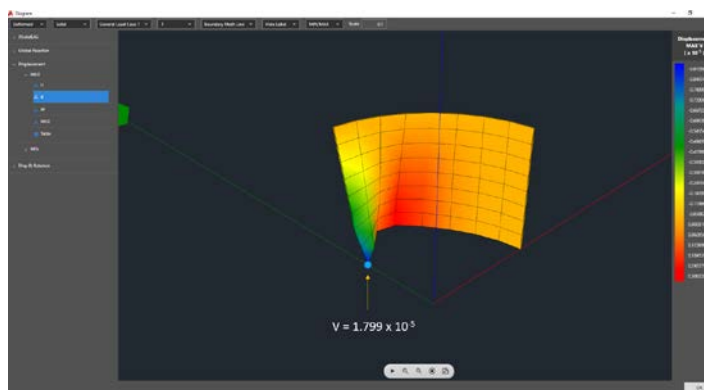
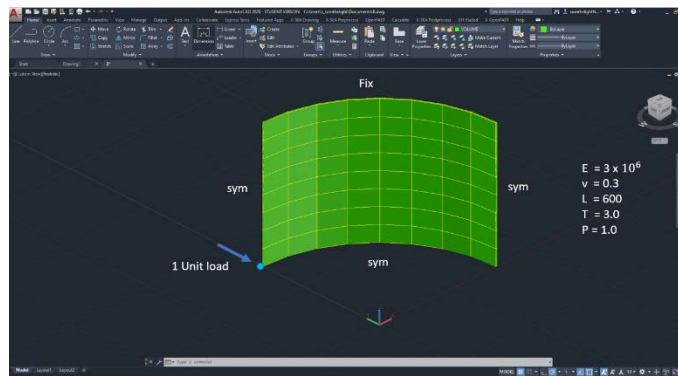


AutoCAD Embedded Finite Element Structural Analysis Software for Offshore & Onshore Structure

“X-SEA AutoCAD”

Verification Report of 3-D 8-node Solid element (XSolid)



Title: Cook's Membrane Problem**Problem Description**

The Cook's membrane problem shown in Figure 1 is solved to determine the sensitivity of finite elements to geometric distortion of the current element on a flat surface. This is a situation that involves a significant amount of shear deformation; it is used to demonstrate the element's capability in modeling membrane deformation.

thickness $t = 0.1$; $E = 1.0$; $\nu = 0.3$. Loading $F = 1.0$.

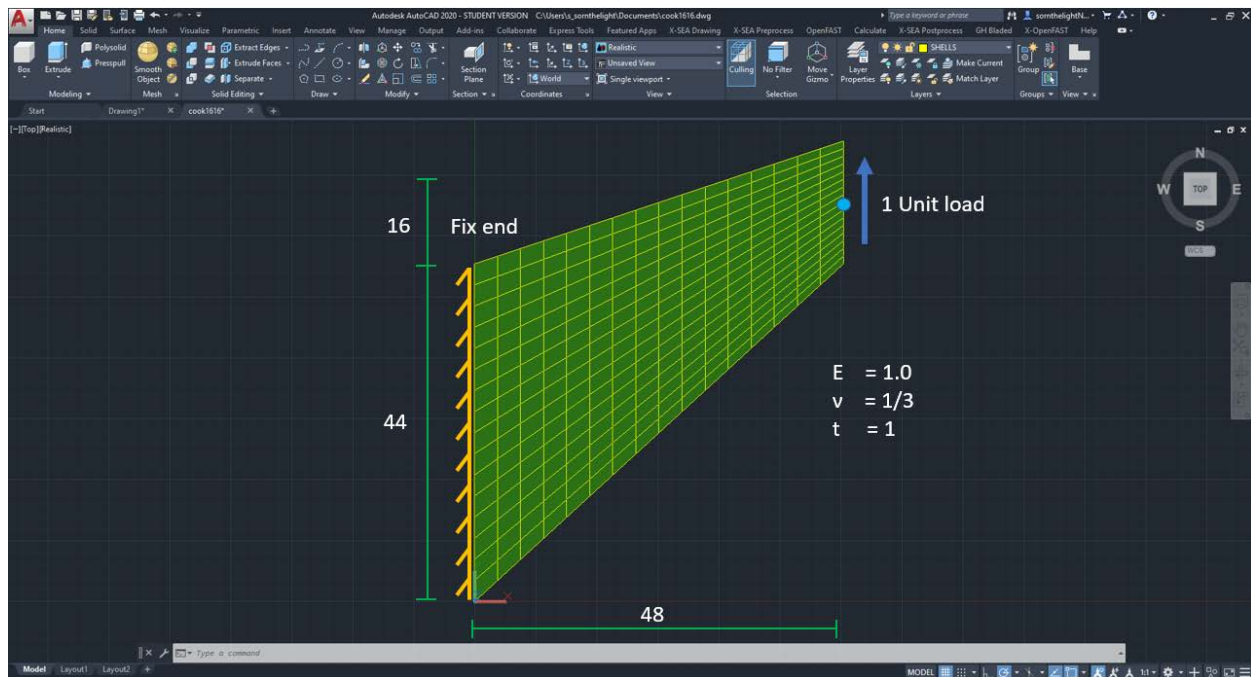


Fig.1.1 Cook's membrane Model

Results

In Table 1.1, numerical results are listed with the vertical displacement at the tip (reference solution = 23.91). The current element performs admirably.

Table 1.1 Result of Cook's Membrane Problem ($V = 23.91$)

| Element size | Solution | Normalize Solution |
|--------------|--------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 2x2x1 | 20.870 | 0.873 |
| 4x4x1 | 23.051 | 0.964 |
| 8x8x1 | 23.809 | 0.996 |
| 16x16x1 | 24.791 | 1.037 |

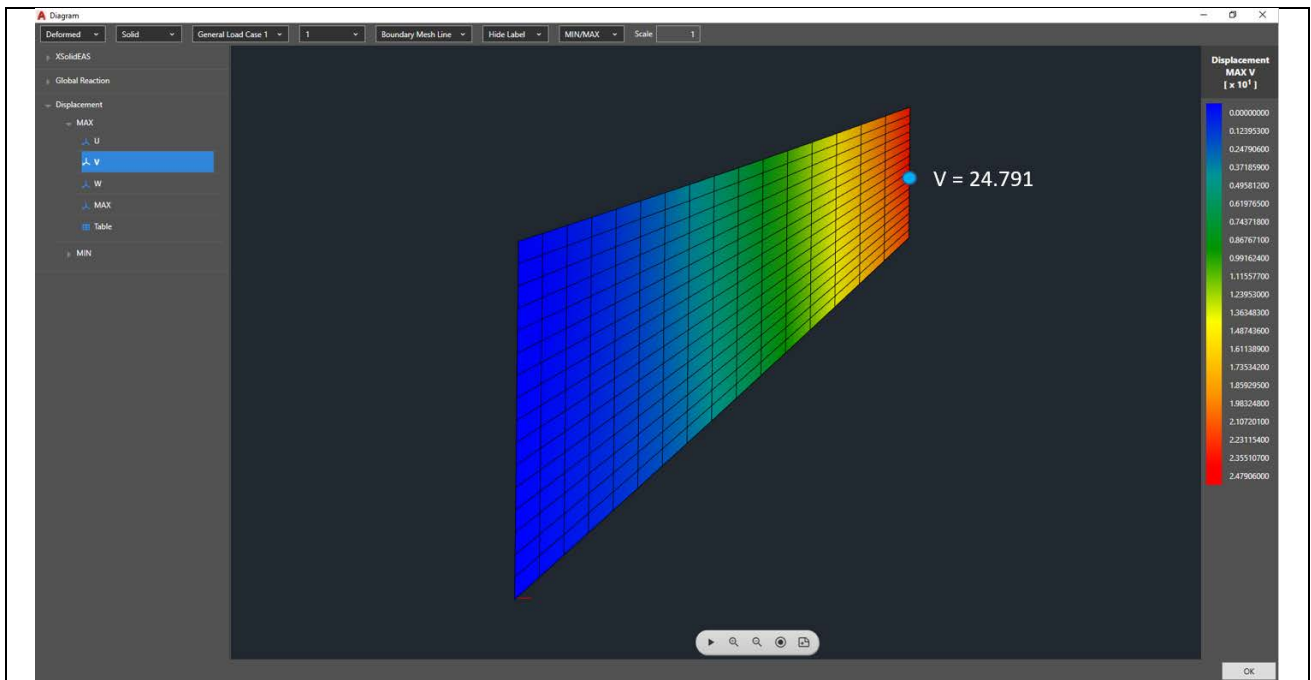


Fig.1.2 Deformation of Cook's membrane model

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Title: Curved beam problem (In-Plane)**Problem Description**

In Figure 2.1, the curve beam problem is subjected to an in-plane shear tip force. Due to the inherent mesh distortion, this problem can be used to investigate the effect of slight irregularity in the element geometry. The present result demonstrates the proposed element's superior performance.

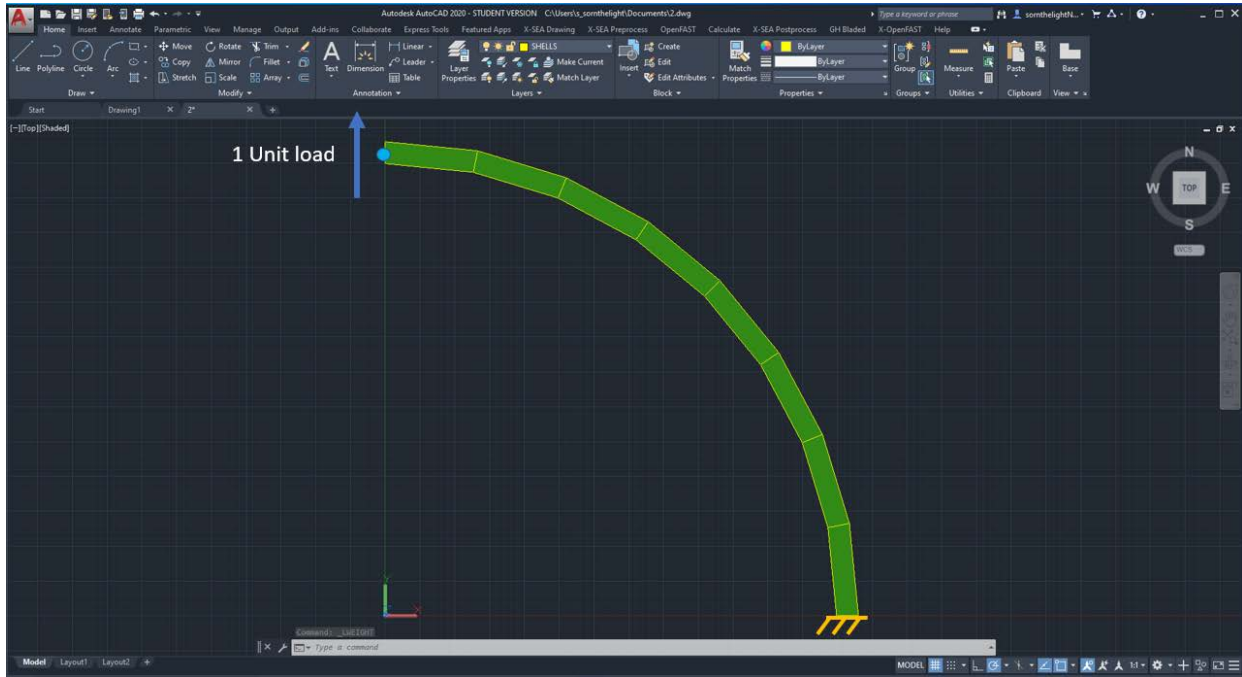


Fig. 2.1 Curved beam problem (In-Plane) Model

Results

The analytical vertical displacement solution at free end and the finite element solutions by X-SEA is show in the picture follows.

Table 2.1 Result of curved beam under in-plane shear ($V = 0.08734$)

| Element size | Solution | Normalize Solution |
|--------------|--------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 6x1x1 | 0.076870 | 0.880 |
| 8x1x1 | 0.084132 | 0.963 |
| 12x1x1 | 0.087296 | 0.999 |

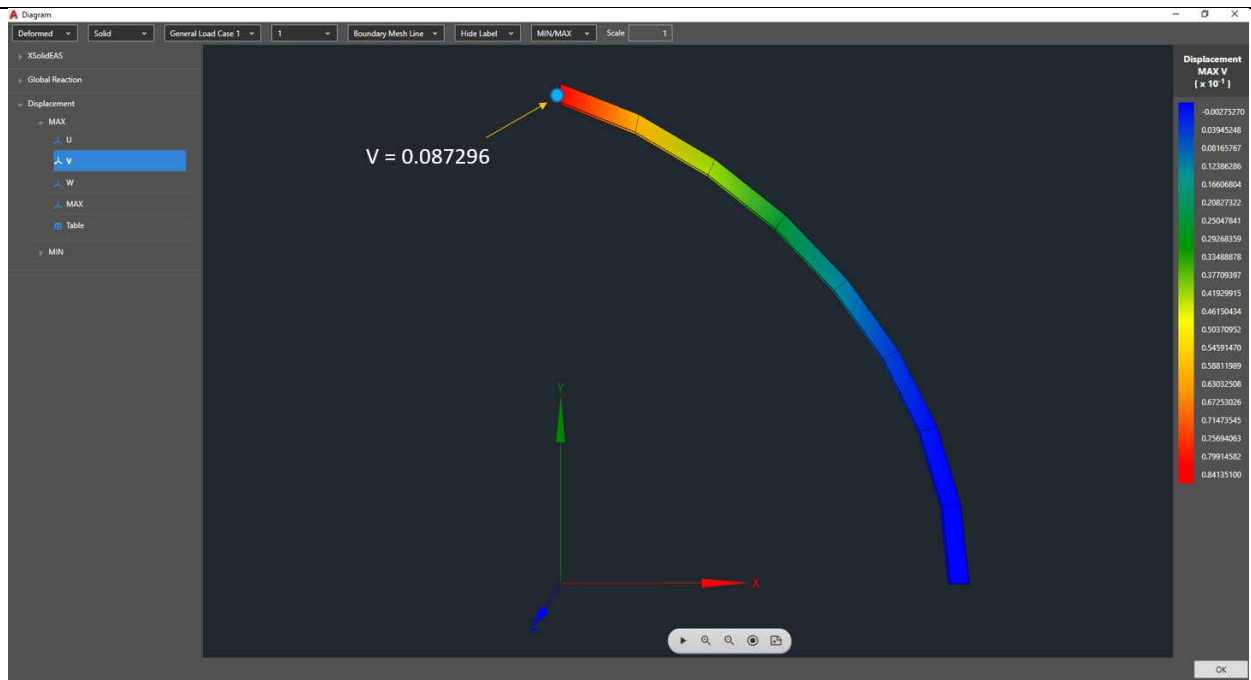


Fig.2.2 Deformation of Curved beam problem result (In-Plane) Model

Title: Curved beam problem (Out-Plane)**Problem Description**

In Figure 3.1, the curved beam problem is subjected to an out-of-plane shear tip force. Due to the inherent mesh distortion, this problem can be used to investigate the effect of minor irregularity in element geometry. The present results demonstrate the proposed element's superior performance.

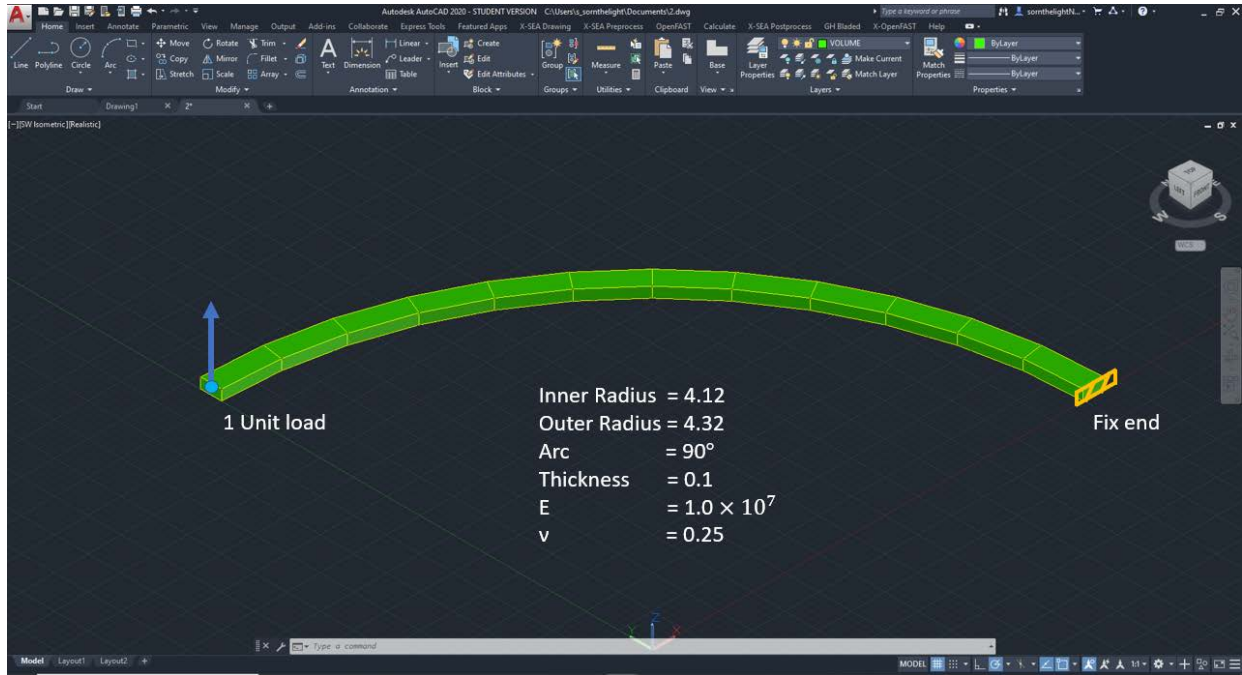


Fig. 3.1 Curved beam problem (Out-Plane) Model

Results

The analytical out-plane displacement solution at free end and the finite element solutions by X-SEA is show in the picture follows.

Table 3.1 Result of curved beam under out-plane shear ($W = 0.5022$)

| Element size | Solution | Normalize Solution |
|--------------|--------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 6x1x1 | 0.42482 | 0.846 |
| 8x1x1 | 0.46180 | 0.920 |
| 12x1x1 | 0.47844 | 0.953 |

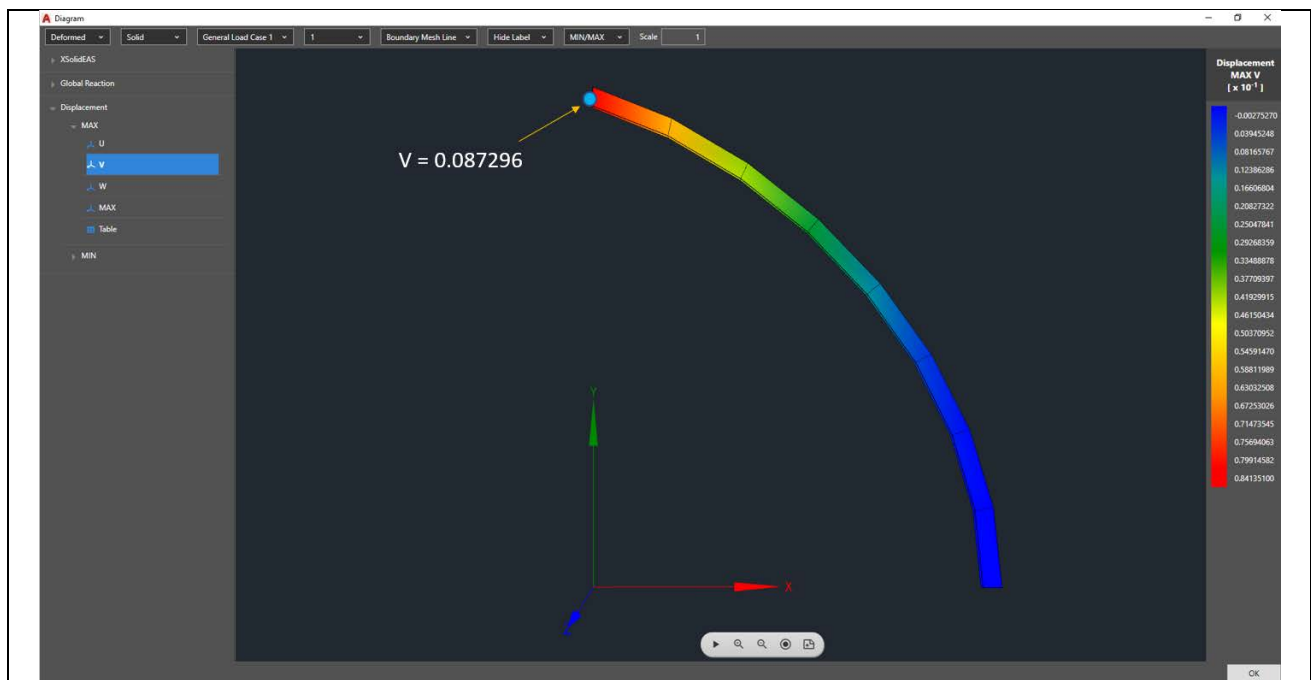


Fig.3.2 Deformation of Curved beam problem (Out-Plane) Model

Title: Twist Beam**Problem Description**

To investigate the effect of element warping, the twisted beam problem is proposed. The shear forces acting on elements with a thickness of 0.32 and 0.0032 are investigated in-plane and out-of-plane. The present results demonstrate the current solid element's superior performance.

- (a) For the case of thickness $t = 0.32$, In-Plane shear

Under in-plane shear force at the free end, the analytic solution deflection of free end is 0.005424.

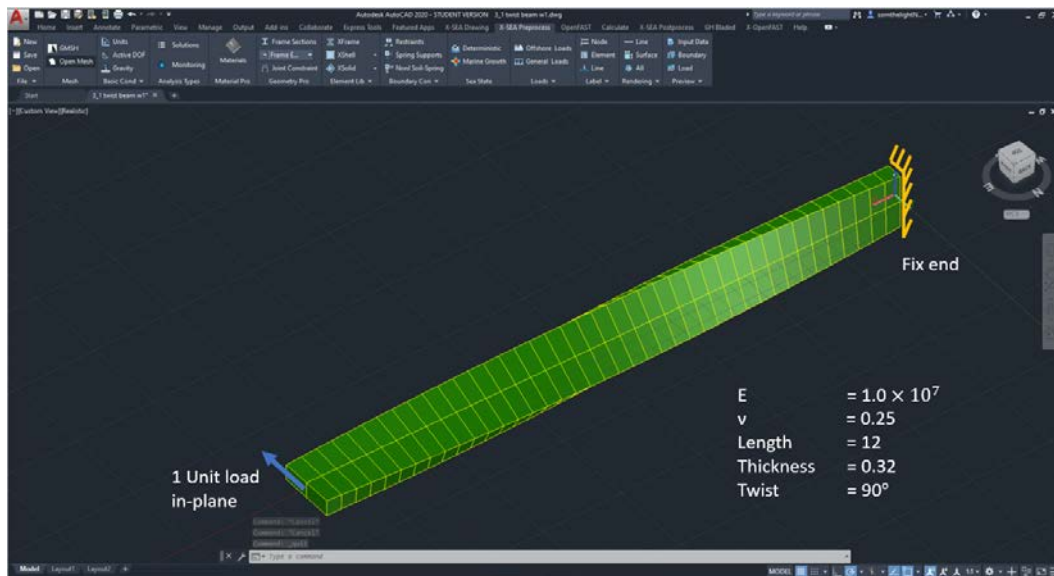


Fig. 4.1 Twist Beam with 0.32 thickness against in-plane unit load Model

- (b) For the case of thickness $t = 0.32$, Out-of-plane Shear

Under Out-plane shear force at the free end, the analytic solution deflection of free end is 0.001754.

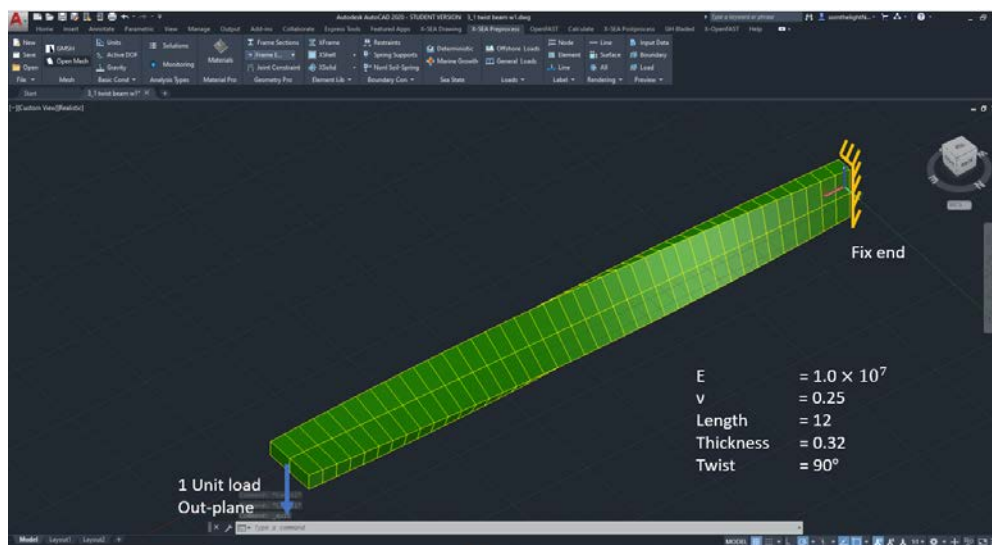


Fig. 4.2 Twist Beam with 0.32 thickness against out-plane unit load Model

(c) For the case of thickness $t = 0.0032$, In-Plane Shear

Under Out-plane shear force at the free end, the analytic solution deflection of free end is 0.5256×10^4 .

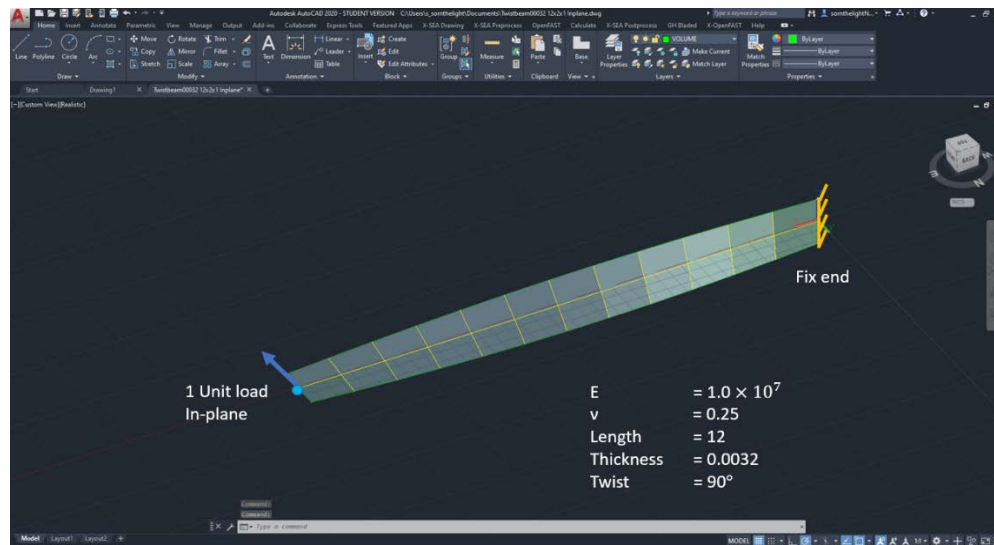


Fig. 4.3 Twist Beam with 0.0032 thickness against in-plane unit load Model

(d) For the case of thickness $t = 0.0032$, Out-Plane shear

Under Out-plane shear force at the free end, the analytic solution deflection of free end is 0.1294×10^4 .

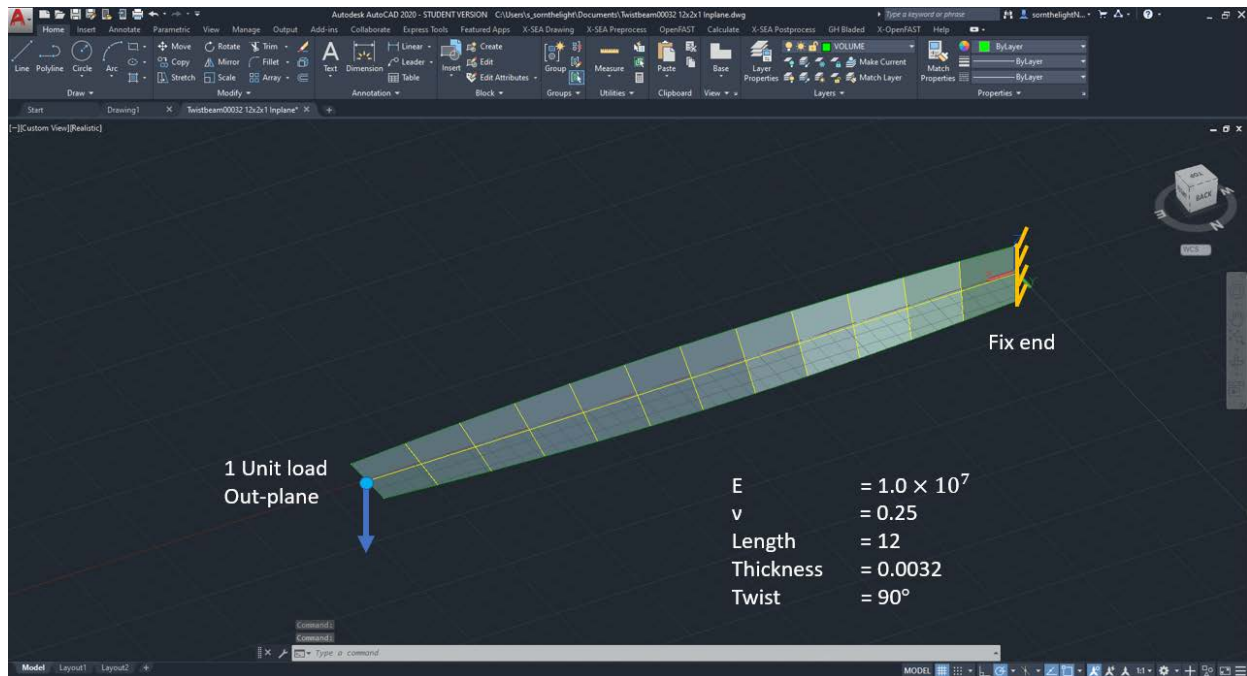


Fig. 4.4 Twist Beam with 0.0032 thickness against in-plane unit load Model

Results

The analytical vertical displacement solution at center and the finite element solutions by X-SEA using a 8x8x1 mesh is show in the picture follows.

(a) For the case of thickness $t = 0.32$, In-Plane shear

Under in-plane shear force at the free end, the analytic solution deflection of free end is 0.005424.

Table 4.1 Result of Twisted Beam with thickness = 0.32 In-plane ($V = 5.424 \times 10^{-3}$)

| Element size | Solution | Normalize Solution |
|--------------|------------------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 4x2x1 | 5.416×10^{-3} | 0.998 |
| 8x2x1 | 5.417×10^{-3} | 0.999 |
| 12x2x1 | 5.420×10^{-3} | 0.999 |
| 40x2x1 | 5.423×10^{-3} | 1.000 |

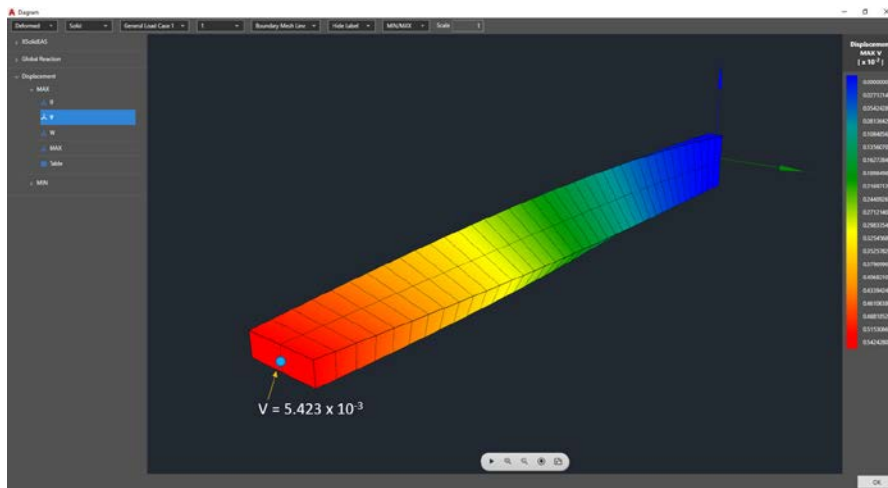


Fig. 4.5 Deformation of Twist Beam with 0.32 thickness against in-plane unit load Model

(b) For the case of thickness $t = 0.32$, Out-of plane Shear

Under Out-plane shear force at the free end, the analytic solution deflection of free end is 0.001754.

Table 4.2 Result of Twisted Beam with thickness = 0.32 Out-plane ($V = 1.754 \times 10^{-3}$)

| Element size | Solution | Normalize Solution |
|--------------|------------------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 4x2x1 | 1.650×10^{-3} | 0.940 |
| 8x2x1 | 1.724×10^{-3} | 0.983 |
| 12x2x1 | 1.738×10^{-3} | 0.991 |
| 40x2x1 | 1.750×10^{-3} | 0.998 |

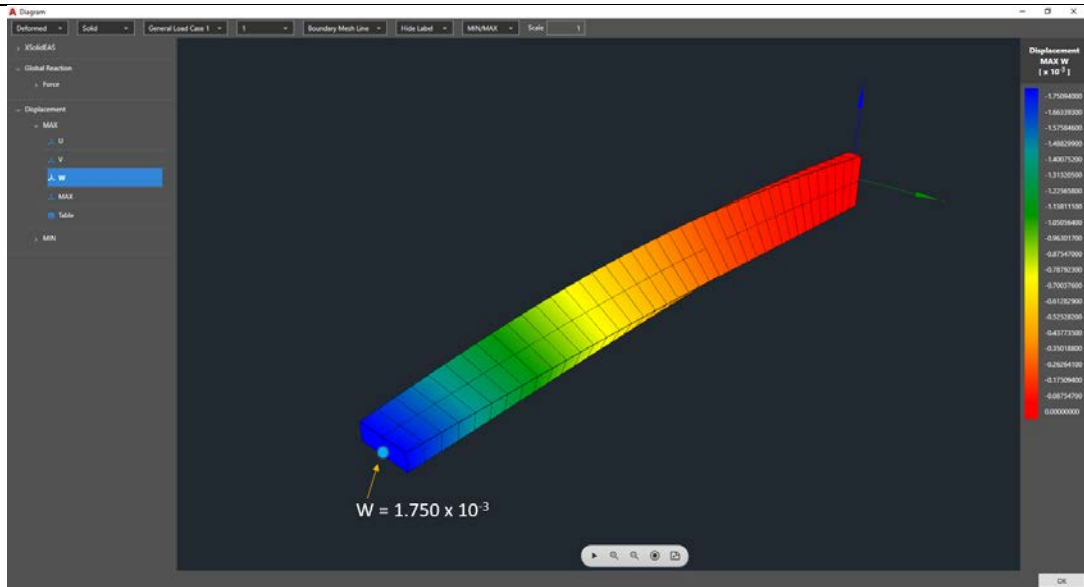


Fig. 4.6 Deformation Twist Beam with 0.32 thickness against out-plane unit load result Model

- (c) For the case of thickness $t = 0.0032$, In-Plane Shear
Under Out-plane shear force at the free end, the analytic solution deflection of free end is 0.5256×10^4 .

Table 4.3 Result of Twisted Beam with thickness = 0.0032 In-plane ($V = 0.5256 \times 10^4$)

| Element size | Solution | Normalize Solution |
|--------------|---------------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 2x2x1 | 0.823×10^4 | 0.157 |
| 4x2x1 | 1.270×10^4 | 0.242 |
| 8x2x1 | 4.753×10^4 | 0.904 |
| 12x2x1 | 4.837×10^4 | 0.920 |
| 40x2x1 | 4.943×10^4 | 0.941 |

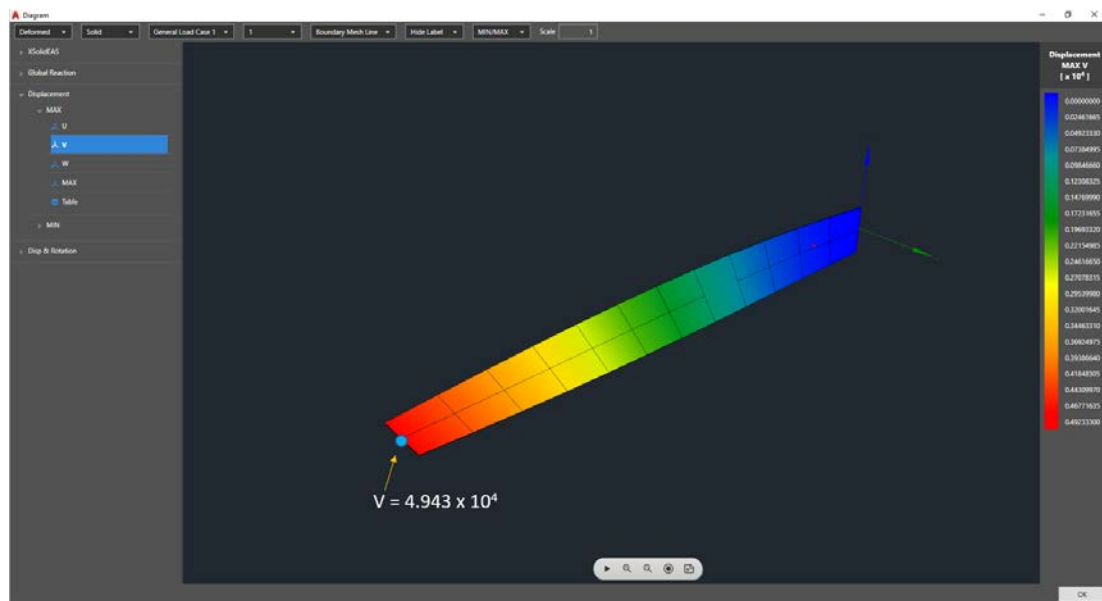


Fig. 4.7 Deformation Twist Beam with 0.0032 thickness against in-plane unit load result Model

- (d) For the case of thickness $t = 0.0032$, Out-Plane shear
Under Out-plane shear force at the free end, the analytic solution deflection of free end is 0.1294×10^4 .

Table 4.4 Result of Twisted Beam with thickness = 0.0032 Out-plane ($V = 0.1294 \times 10^4$)

| Element size | Solution | Normalize Solution |
|--------------|---------------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 2x2x1 | 0.017×10^4 | 0.139 |
| 4x2x1 | 0.073×10^4 | 0.565 |
| 8x2x1 | 0.117×10^4 | 0.902 |
| 12x2x1 | 0.120×10^4 | 0.925 |
| 40x2x1 | 0.123×10^4 | 0.951 |

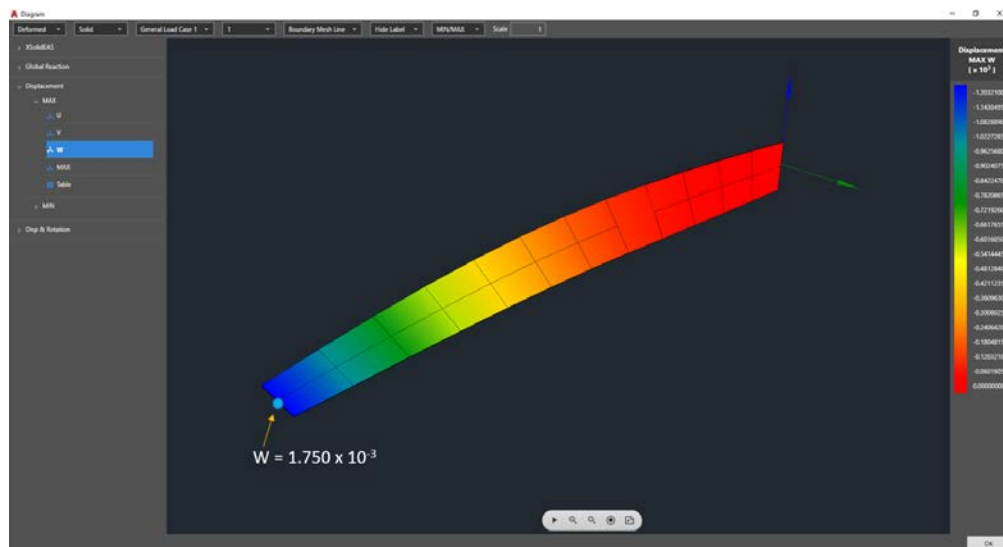


Fig. 4.8 Deformation Twist Beam with 0.0032 thickness against out-plane unit load result Model

Title: Bending Rhombic Plate**Problem Description**

A simply supported rhombic plate in Figure 5.1 is subjected to a uniformly distributed load. This is a rather challenging test due to the singularity of its solution at the obtuse vertices.

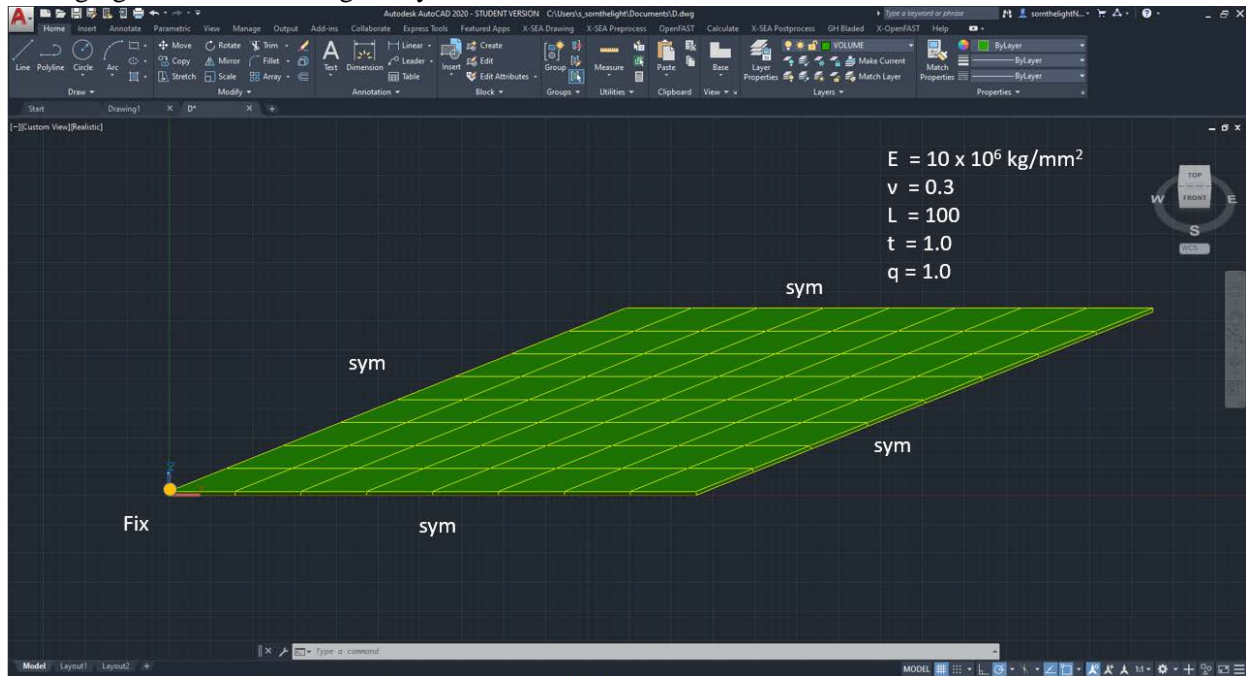


Fig.5.1 Rhombic Plate Model

Results

The numerical results of plate center is presented in Table 5.1 and figure 5.2 The reference solution is $W_c = 0.04455$

Table 5.1 Result of Bending Rhombic Plate Model ($W = 0.04455$)

| Element size | Solution | Normalize Solution |
|--------------|--------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 4x4x1 | 0.04002 | 0.898 |
| 8x8x1 | 0.04228 | 0.949 |
| 16x16x1 | 0.04408 | 0.989 |
| 20x20x1 | 0.04451 | 0.999 |
| 32x32x1 | 0.04519 | 1.014 |

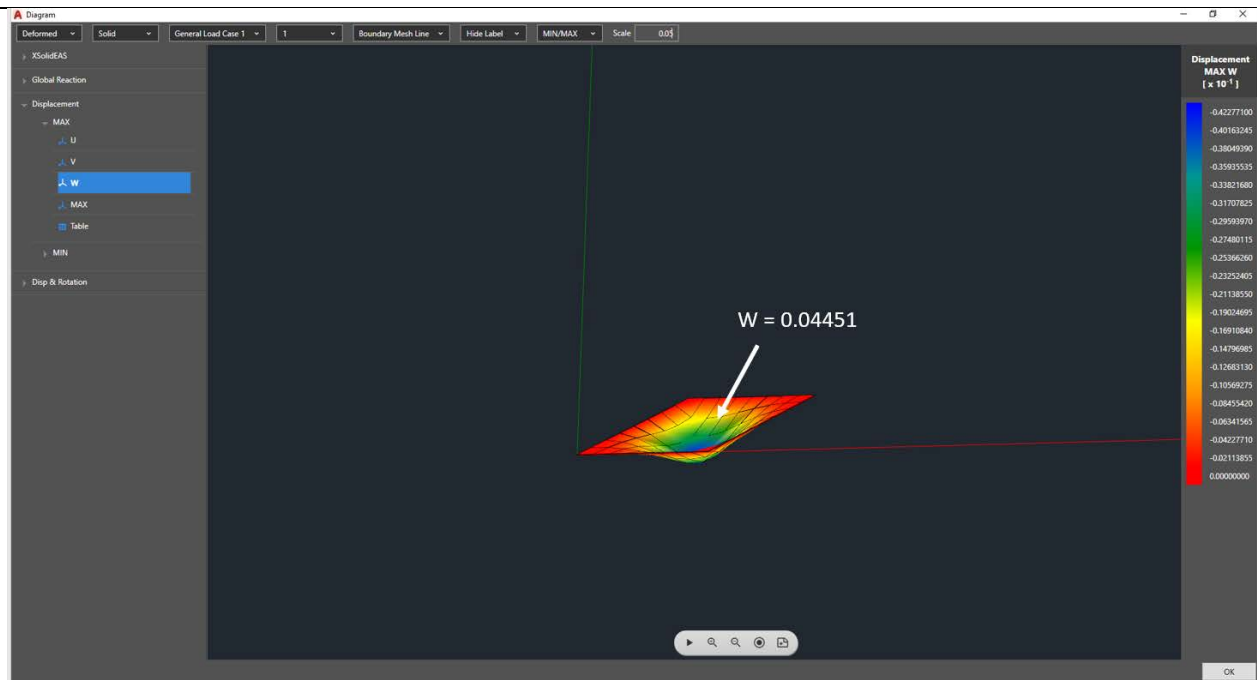


Fig.5.2 Deformation Bending Rhombic Plate result Model

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Title: Bending of Rectangular Plate**Problem Description**

The clamped rectangular plate problem under central point loading (Figure 6.1 and Figure 6.2) is applied to test shear locking by changing of aspect ratio. Two aspect ratio of $b/a = 1$ and 5 are considered.

(a) For the case of $b/a = 1.0$

The reference vertical deflection at the center of plate is 5.60×10^{-6} .

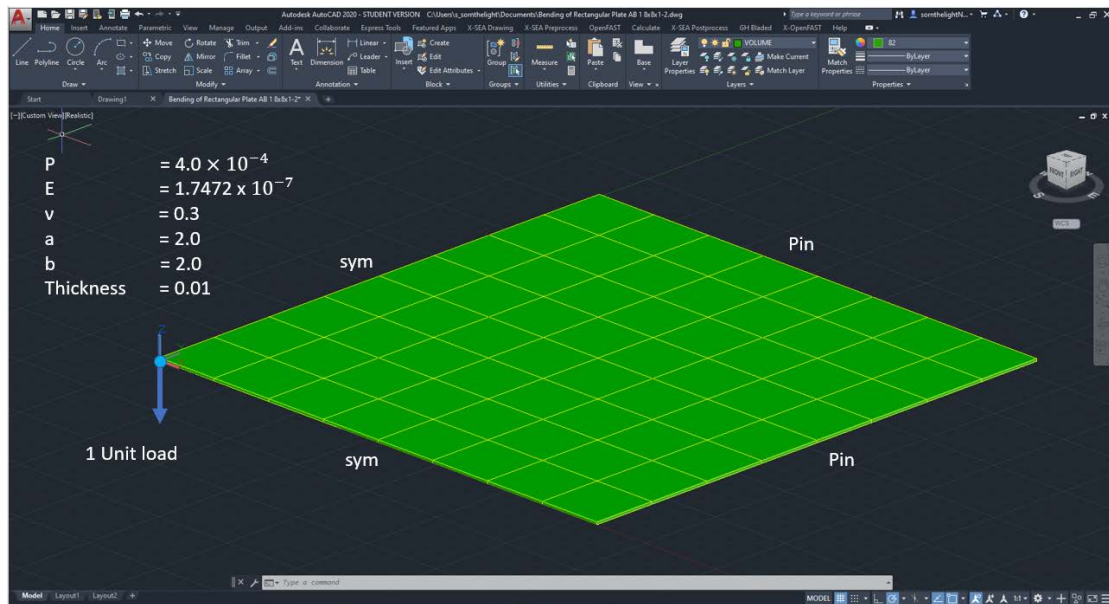


Fig. 6.1 Bending of Rectangular Plate ($b/a = 1.0$) Model

(b) For the case of $b/a = 5.0$

The reference vertical deflection at the center of plate is 7.23×10^{-6} .

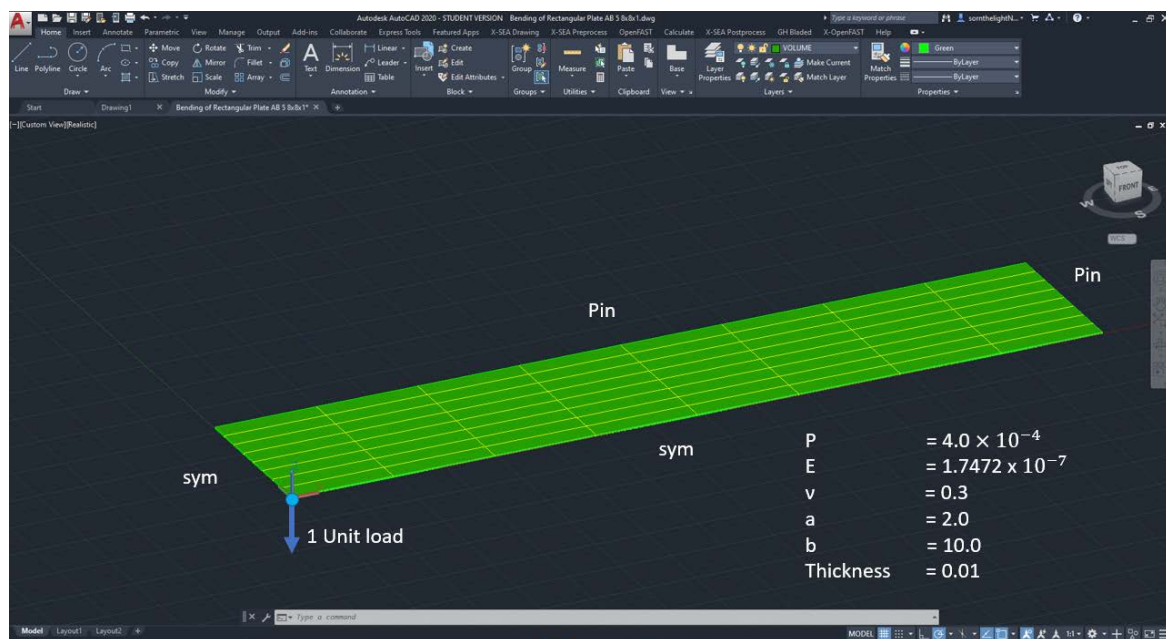


Fig. 6.2 Bending of Rectangular Plate ($b/a = 5.0$) Mode

Results

| Element size | Solution | Normalize Solution |
|--------------|------------------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 2x2x1 | 4.961×10^{-6} | 0.886 |
| 4x4x1 | 5.448×10^{-6} | 0.973 |
| 6x6x1 | 5.539×10^{-6} | 0.989 |
| 8x8x1 | 5.571×10^{-6} | 0.995 |

Table 6.1 Result of Bending of Rectangular Plate $b/a = 1$ ($W = 5.60 \times 10^{-6}$)

| Element size | Solution | Normalize Solution |
|--------------|------------------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 2x2x1 | 2.322×10^{-6} | 0.321 |
| 4x4x1 | 6.146×10^{-6} | 0.850 |
| 6x6x1 | 6.705×10^{-6} | 0.927 |
| 8x8x1 | 6.921×10^{-6} | 0.957 |
| 16x16x1 | 7.152×10^{-6} | 0.989 |

Table 5.2 Result of Bending of Rectangular Plate $b/a = 5$ ($W = 7.23 \times 10^{-6}$)

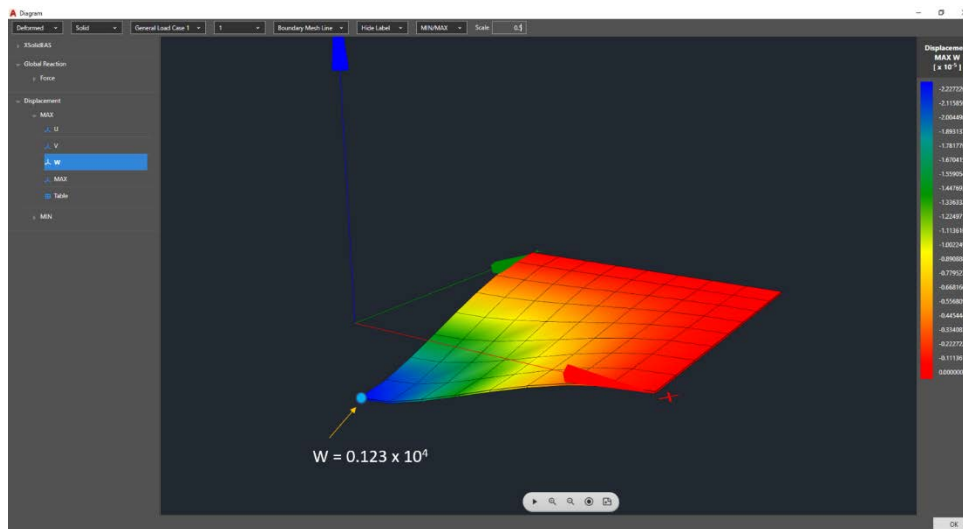


Fig.6.3 Deformation of Bending of Rectangular Plate ($b/a = 1.0$) Mode

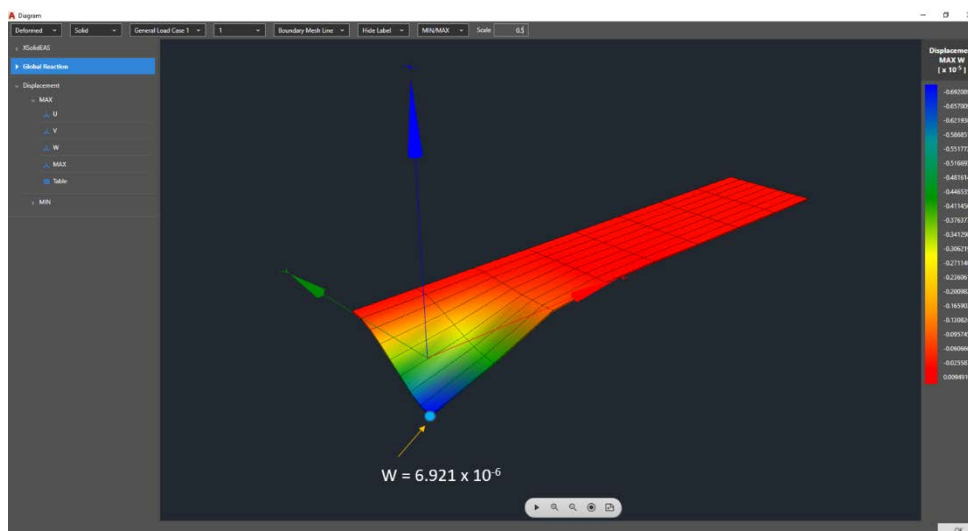
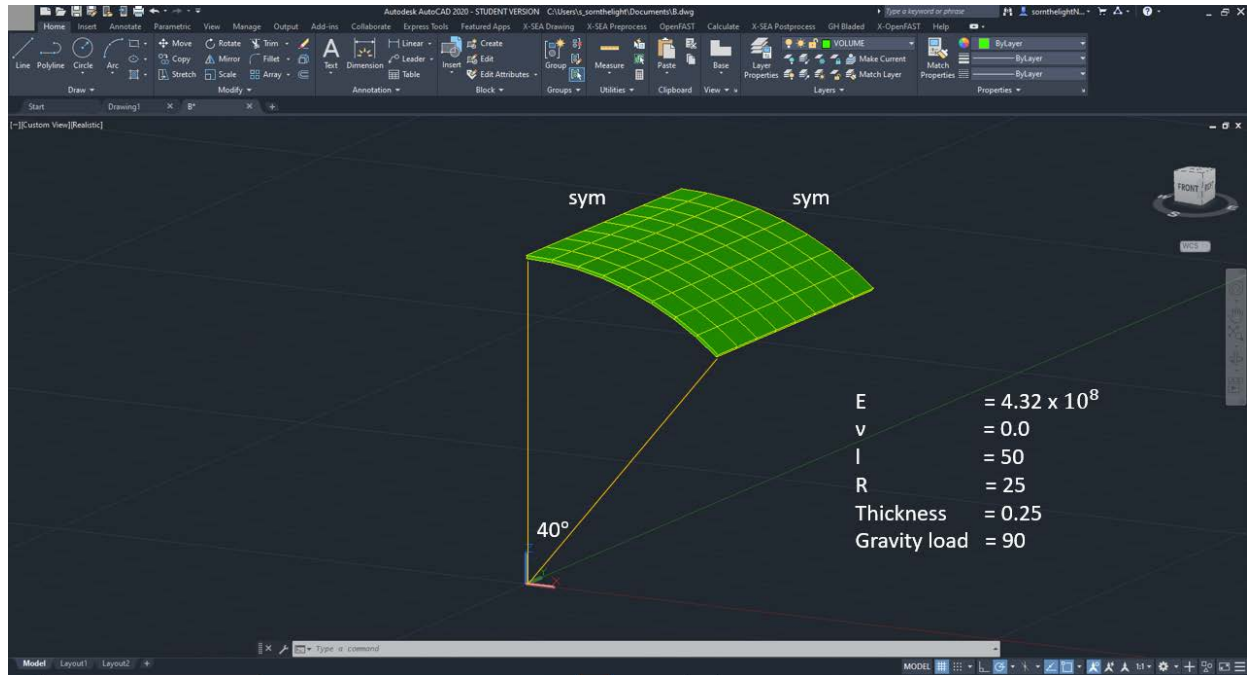


Fig. 6.4 Deformation of Bending of Rectangular Plate ($b/a = 5.0$)

Title: Scordellis-Lo Roof Problem**Problem Description**

Both membrane and bending deformations are important to the solution of the Scordellis-Lo roof problem shown in figure 7. As the membrane deformation contributed significantly, this problem can be used to determine the element capability in modeling membrane state in curved shells.

**Fig. 7.1** Scordellis-Lo Roof Problem Model**Results**

The reference solution of vertical deflection at point A is 0.3024.

Table 7.1 Result of Scordellis-Lo Roof Problem ($W = 0.3024$)

| Element size | Solution | Normalize Solution |
|--------------|--------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 4x4x1 | 0.3224 | 1.066 |
| 6x6x1 | 0.3157 | 1.044 |
| 8x8x1 | 0.3154 | 1.043 |
| 16x16x1 | 0.3128 | 1.034 |

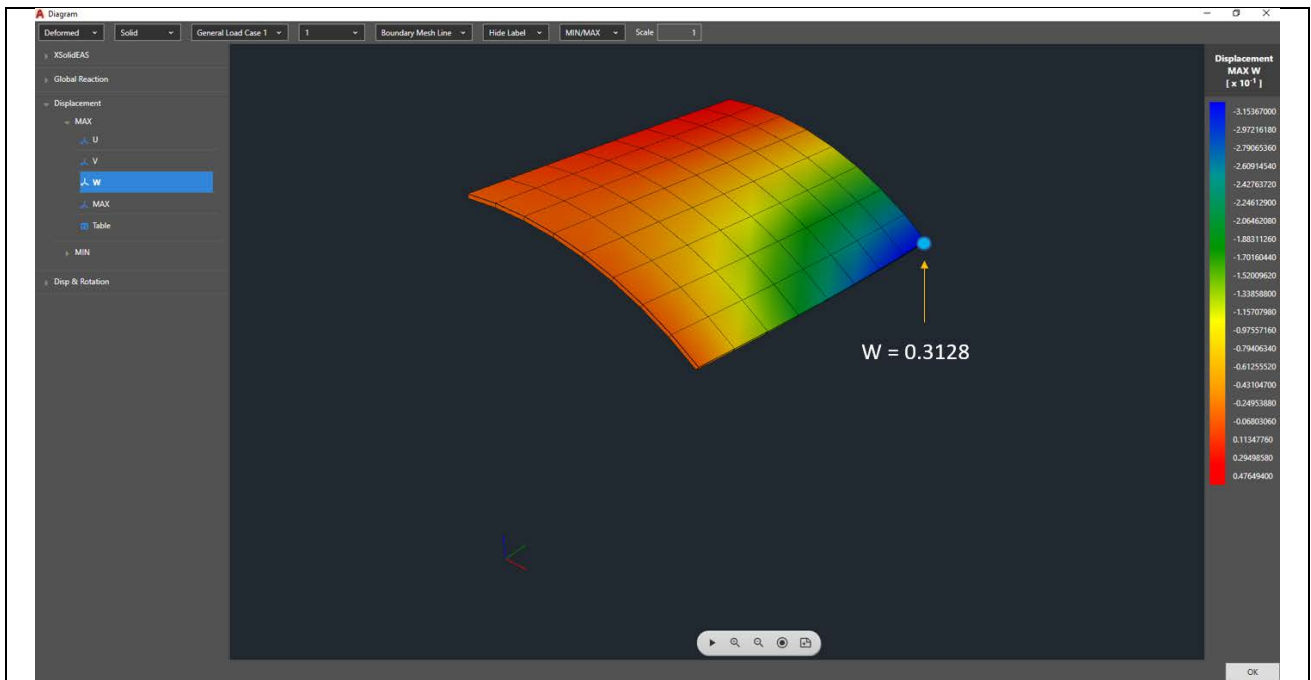


Fig. 7.2 Deformation of Scordellis-Lo Roof Model

Title: Pinched Cylinder with End Diaphragm**Problem Description**

This problem is test for the element ability in modeling inextensional bending mode and complex membrane states. The vertical deflection is investigated.

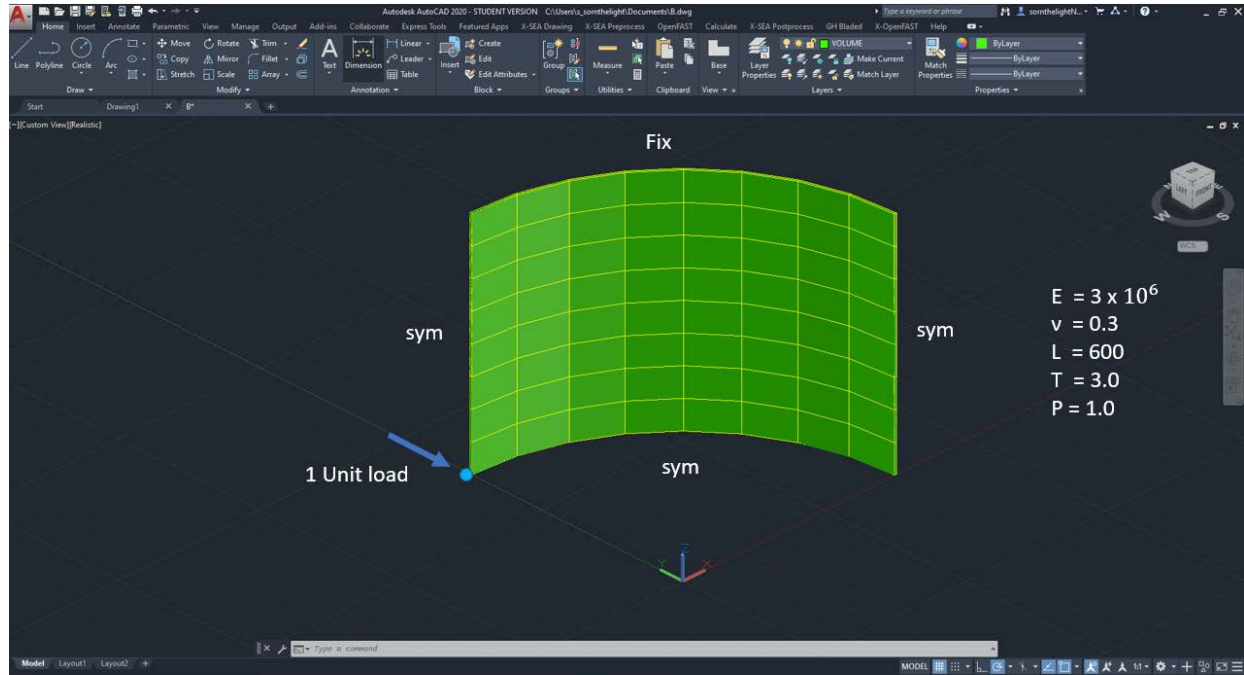


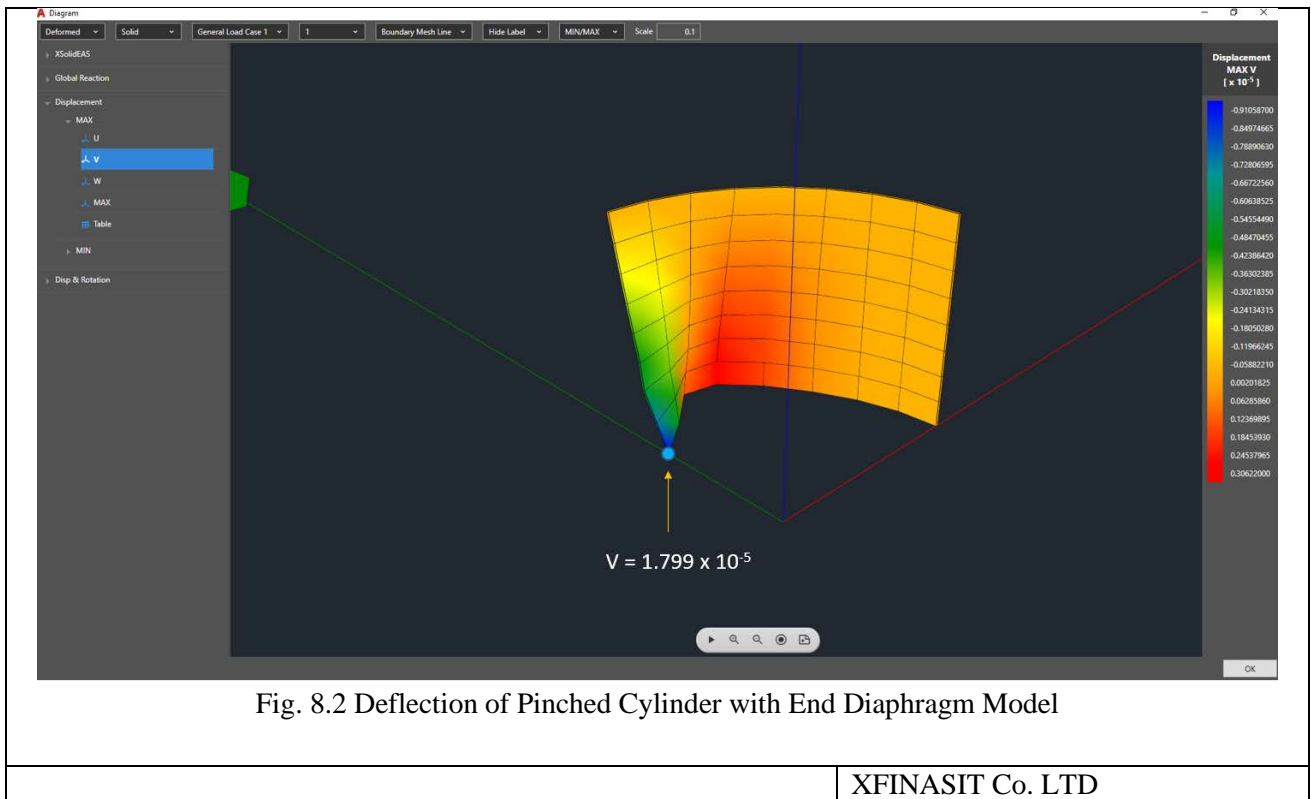
Fig. 8.1 Pinched Cylinder with End Diaphragm Model

Results

The reference value is 0.18248×10^{-4} . Result with different meshes and elements are presented in table 7.1 and figure 7.2.

Table 8.1 Pined cylinder with End diaphragm Problem ($W = 1.8248 \times 10^{-5}$)

| Element size | Solution | Normalize Solution |
|--------------|------------------------|--------------------|
| | XSolid-8-EAS | XSolid-8-EAS |
| 4x4x1 | 1.953×10^{-5} | 0.107 |
| 8x8x1 | 9.106×10^{-5} | 0.499 |
| 16x16x1 | 1.662×10^{-5} | 0.911 |
| 32x32x1 | 1.799×10^{-5} | 0.986 |



Title: Geometrical nonlinear analysis of Clamped Plate Under Uniform Pressure**Problem Description**

A geometrical nonlinear analysis of clamped plate shown in figure 9 is carried out. One quadrant is modeled due to the symmetry. The vertical deflection at the center is investigated by reference Timosehnko and Woinowsky.

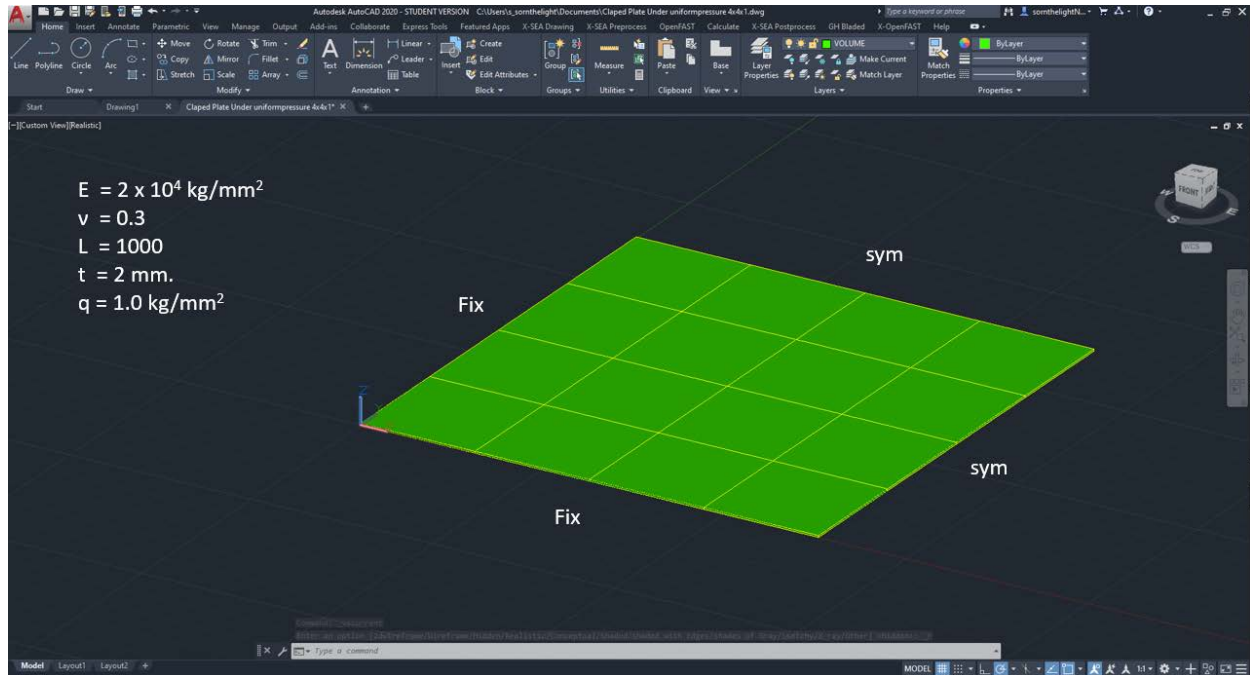


Fig.9.1 Clamped Plate Under Uniform Pressure Model

Results

Numerical results are taken out to list in table 9.1 as comparison to show the performance of present elements.

Table 9.1 Pressure load for different displacement ($\times 10^{-5}$)

| Disp. | XSOLID-8-EAS |
|-------|--------------|
| 0.5 | 0.609 |
| 1.0 | 1.333 |
| 1.5 | 2.289 |
| 2.0 | 3.603 |
| 2.2 | 4.259 |
| 2.4 | 5.002 |
| 2.6 | 5.842 |
| 2.8 | 6.787 |
| 3.0 | 7.848 |

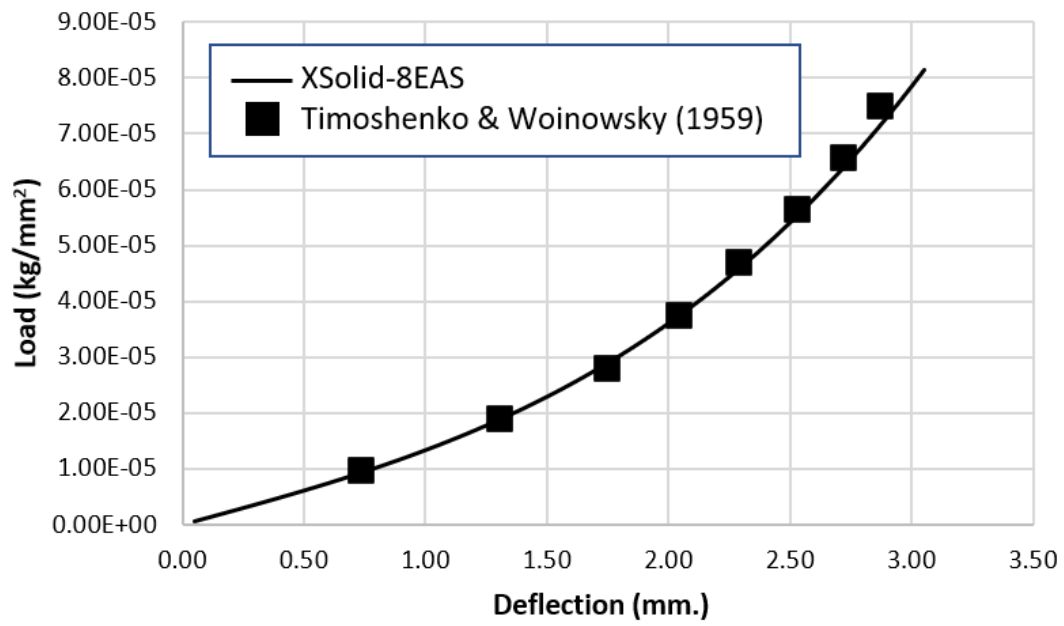


Fig.9.2 Load-deflection curve of clamped plate (at center)

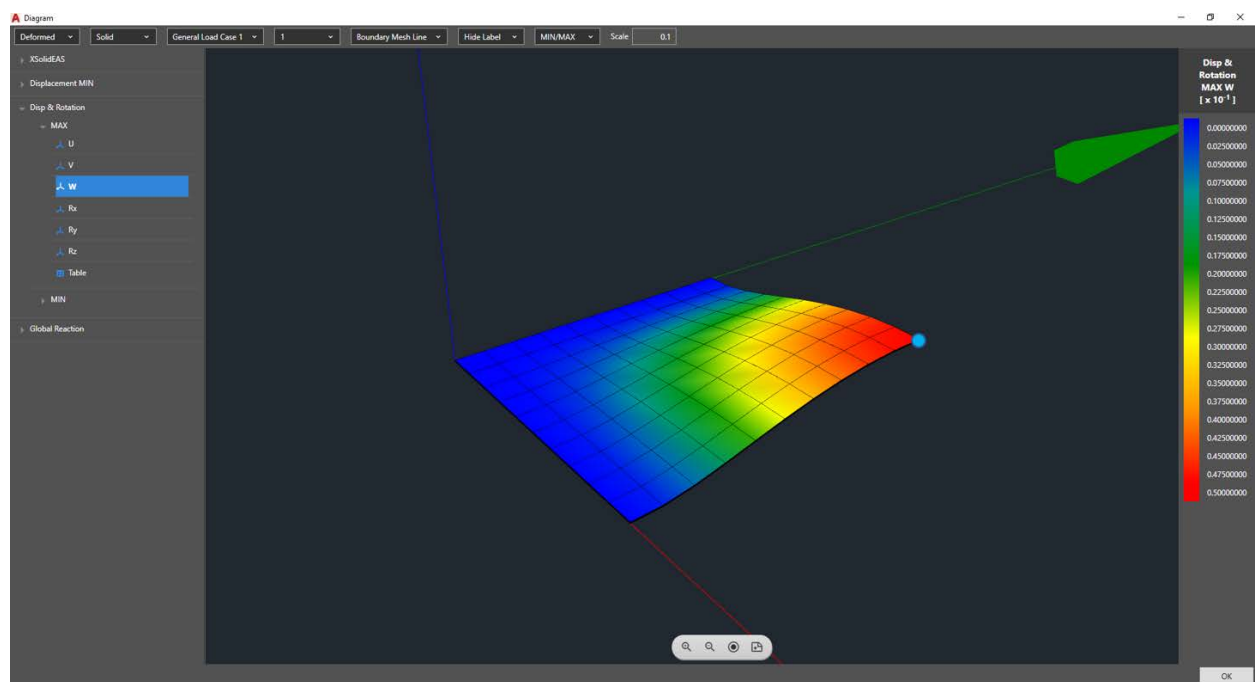
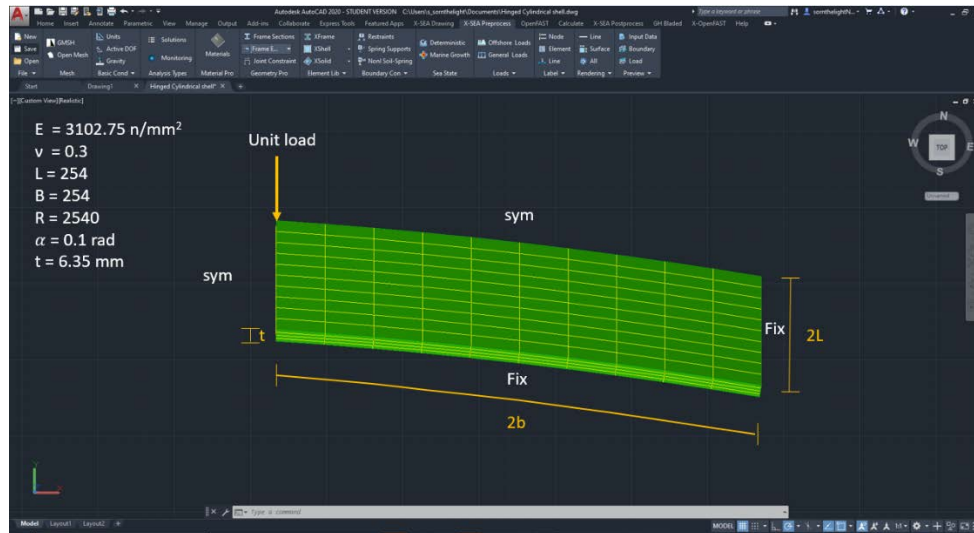


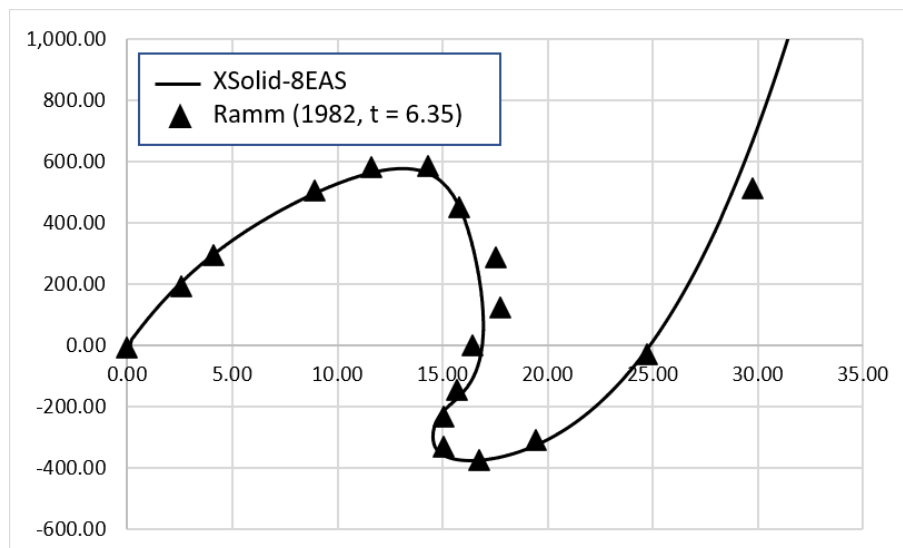
Fig.9.3 Deformation Clamped Plate Under Uniform Pressure Model

Title: Geometrical nonlinear analysis of Hinged Cylindrical Shell**Problem Description**

A geometrical nonlinear analysis of hinged shell is carried out. The cylindrical shell is hinged at the straight edges and free at the curved edges. One quadrant of the shell is modeled. The mesh size is $10 \times 10 \times 4$. The geometry and the material are shown in Figure 10.1

**Fig.10.1** Hinged Cylindrical Shell Model**Results**

The result obtained from present solid element X-Solid-8-EAS is plotted. The deflection of point A is investigated. The results obtained are plotted in figure 10.2

**Fig.10.2** Load-deflection curve of hinged cylindrical shell ($t = 6.35$) at loaded point